CR 316

INDEX OF SHEETS

SEE SHEET 2



STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT

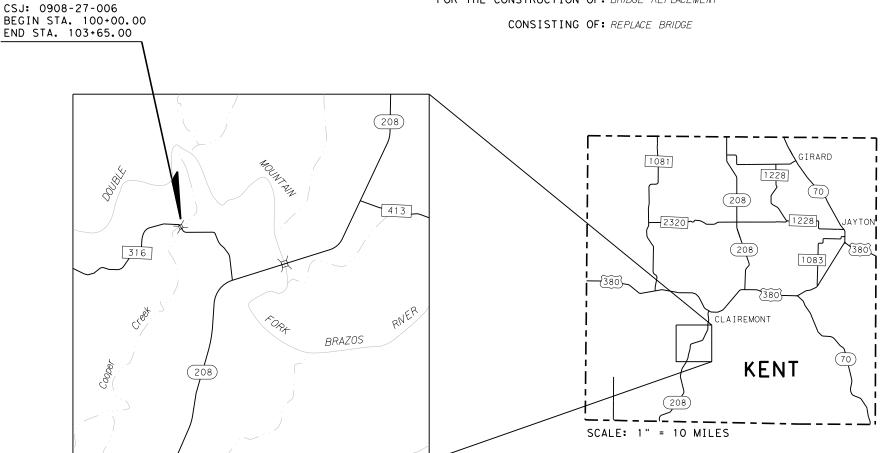
PROJECT NO. BR 2023(941)

NET LENGTH OF ROADWAY = NET LENGTH OF BRIDGE = NET LENGTH OF PROJECT = 300.00 ft = 00.057 mi 65.00 ft = 00.012 mi 365.00 ft = 00.069 mi

CR 316 KENT COUNTY

LIMITS: AT COOPER CREEK

FOR THE CONSTRUCTION OF: BRIDGE REPLACEMENT



DESIGN SPEED = MOIEC CURRENT A.D.T. (2020) = 10 AADT PROJECTED A.D.T. (2042) = 30 AADT FUNCTIONAL CLASS = RURAL LOCAL EXISTING NBI# =08-132-0-AA01-13-001 PROPOSED NBI# =08-132-0-AA01-13-003

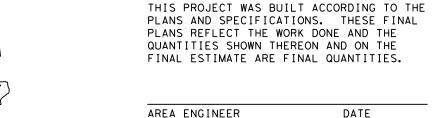
CONTRACTOR :

PROJECT NO. BR 2023 (941) STATE COUNTY DISTRICT TEXAS KENT ABL CONTROL SECTION JOB 0908 27 006 CR 316

FINAL PLANS

LETTING DATE:	DECEMBER 2023
DATE CONTRACTOR BEGAN	WORK:
DATE WORK WAS COMPLETE	D:
DATE WORK WAS ACCEPTED	!
FINAL CONTRACT COST: \$	

CERTIFICATION FOR FINAL PLANS



THE DISTRICT TRAFFIC SAFETY COMMITTEE HAS REVIEWED THE TRAFFIC CONTROL PLAN FOR THIS PROJECT AND IT IS IN COMPLIENCE WITH CURRENT DEAS IN TO STANDARDS.

Michael Wittie, P.E. 62ACOMMIJIEE CHAIRMAN

9/26/2023 DATE

DATE



CONCURRENCE:

Texas Department of Transportation

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RECOMMENDED FOR LETTING: 9/26/2023

RECOMMENDED FOR LETTING: 9/26/2023

-575MICHAENA AD HAITHCOCK, P.E. DIRECTOR OF T P & D

APPROVED FOR LETTING: 9/26/2023

408SIEWARIGAL CHAPMAN, P.E. AREA ENGINEER

9/20/2023

Layne Coulter 6HENORARES 4HEAYNE COULTER KENT COUNTY JUDGE

SUBMITTED FOR LETTING: 9/20/2023

— 34AMÆGEANSA©D4SMAYFIÉLD. P.E. TXDOT PROJECT MANAGER

Megan C. Mayfield, P.E.

OF6FMOMAS70430 ALLBRITTON, P.E.

Michael Haithcock

DISTRICT ENGINEER

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF TRANSPORTATION. NOVEMBER 1, 2014 AND SPECIFICATION ITEMS LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION CONTRACTS (FORM FHWA 1273, OCTOBER 23, 2023).

SCALE: 1" = 1 MILE

EXCEPTIONS: N/A EQUATIONS: N/A RAILROAD CROSSINGS: N/A

71

TRF

GENERAL TITLE SHEET INDEX OF SHEETS 3 PROJECT LAYOUT TYPICAL SECTIONS 6-9 **GENERAL NOTES** 10 **ESTIMATE & QUANTITY SHEET** 11 QUANTITY SUMMARY 12 **BRIDGE SUMMARY** TRAFFIC CONTROL PLAN 13 DETOUR LAYOUT/TCP NARRATIVE TRAFFIC CONTROL PLAN STANDARDS BC (1)-21THRU BC (12)-21 **#** 14-25 **#** 26 TCP (2-7)-23 **ROADWAY DETAILS** 27 SURVEY CONTROL DATA 28 REMOVAL LAYOUT 29-30 ALIGNMENT DATA 31 PLAN AND PROFILE 32 FBMS-23 **ROADWAY DETAILS STANDARDS #** 33 GF(31)-19 **#** 34 GF(31) TR TL-2-19 **#** 35 SGT (10S)31-16 **#** 36 SGT (11S)31-18 **#** 37 SGT (12S)31-18 **BRIDGE** DRAINAGE AREA MAP 39-44 BRIDGE HYDRAULIC DATA **BRIDGE LAYOUT** SOIL BORING DATA 46-47 48 BEARING SEAT ELEVATIONS 49 SIDD-14 **STRUCTURE STANDARDS \$** 50-51 ABB-24 **#** 52-53 SBBS-B20-24 BBSDS-B20-24 **#** 54 **#** 55-57 BB-B20 **#** 58 BBEB **#** 59 **BBRAS** # 60-61 CSAB **#** 62-63 FD **#** 64 SEJ-M **#** 65 CRR SD-EBR **#** 66-67 **#** 68-70 T223

PAVEMENT MARKINGS & DELINEATION STANDARDS

72 D&OM (1)-20 # 73 D&OM (2)-20 # 74 D&OM (3)-20 # 75 D&OM (5)-20 # 76 D&OM (VIA)-20

ENVIRONMENTAL ISSUES

* 77-78 STORMWATER POLLUTION PREVENTION PLAN (SWP3)
 * 79 ENVIRONMENTAL PERMITS, ISSUES AND COMMITMENTS
 * 80 ENVIRONMENTAL LAYOUT SHEET

ENVIRONMENTAL ISSUES STANDARDS

81 EC (1)-16 **#** 82-84 EC (9)-16



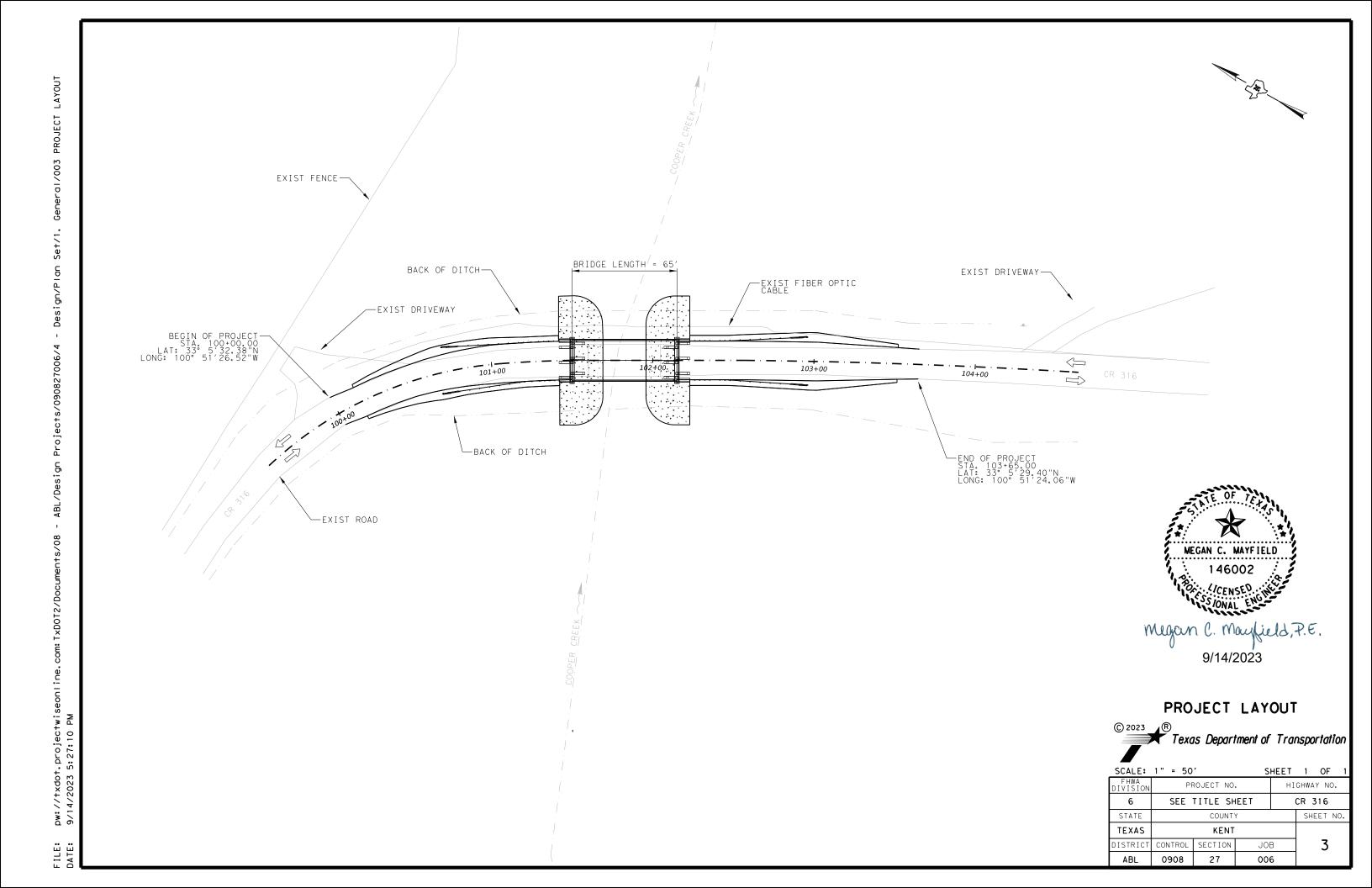
THE STANDARD SHEETS SPECIFICALLY IDENTIFIED ON THIS SHEET WITH A # HAVE BEEN SELECTED BY ME OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.

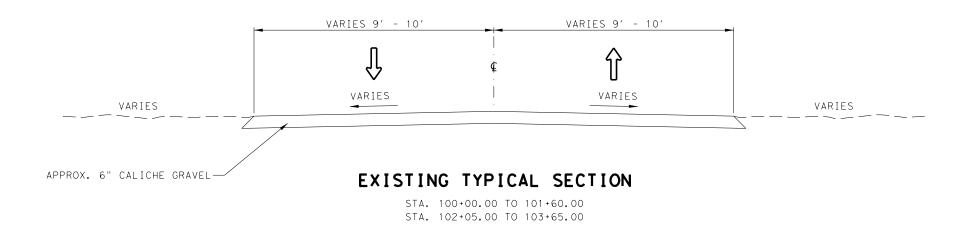
Megan C. Mayfield, P.E. 10/20/2023
DATE

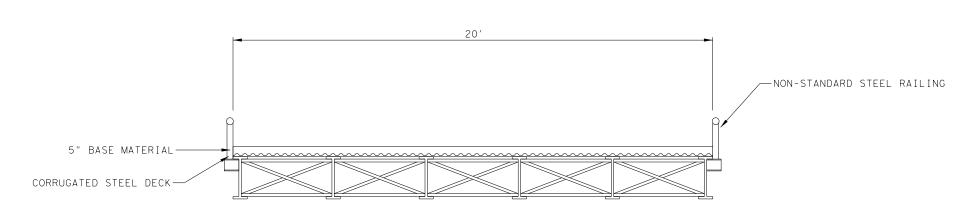
INDEX OF SHEETS



FHWA IVISION	PF	ROJECT NO	ΗI	GHWAY NO.	
6	SEE TITLE SHEET				CR 316
STATE	COUNTY				SHEET NO.
TEXAS					
ISTRICT	CONTROL	SECTION JOB			2
ABL	0908	27	006		







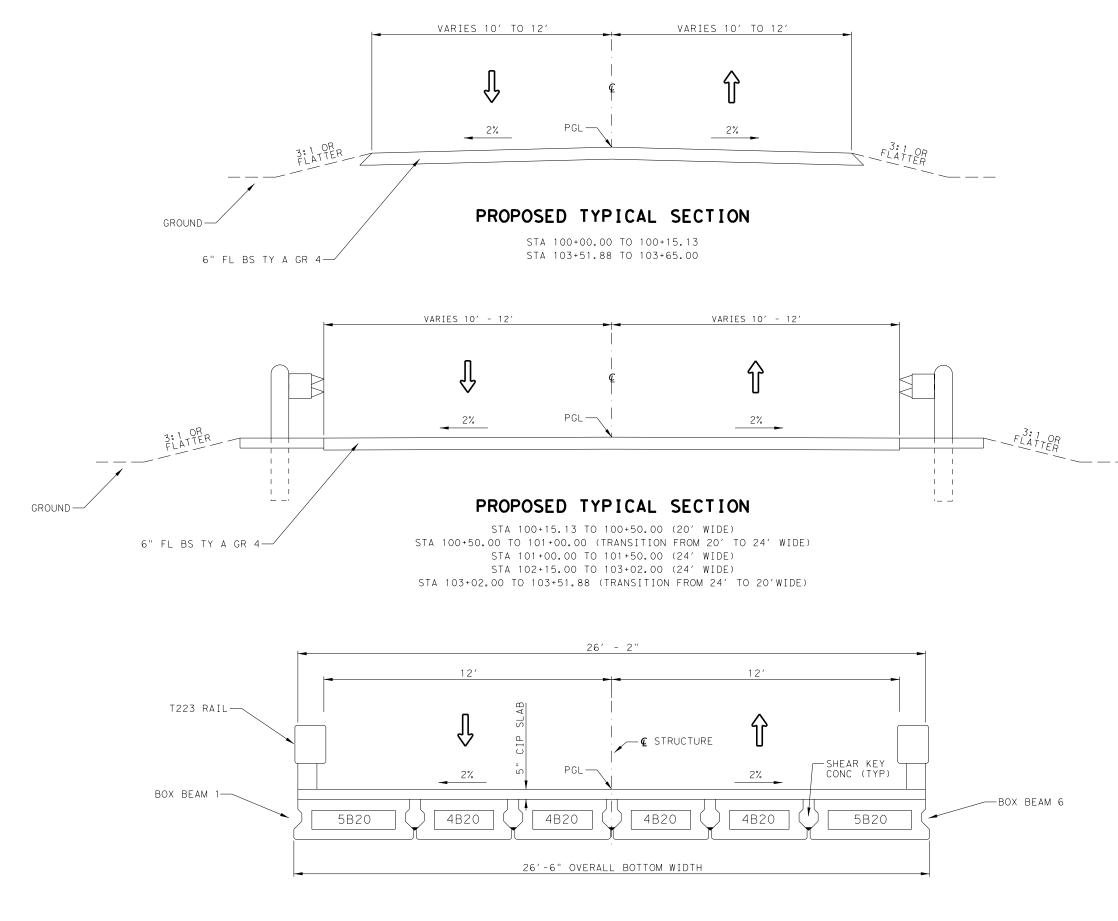
EXISTING TYPICAL SECTION

STA. 101+60.00 TO 102+05.00



CR 316 TYPICAL SECTION

SCALE:	1" = 4'		SI	HEET	1 OF 2
FHWA DIVISION	PF	ROJECT NO	нІ	GHWAY NO.	
6	SEE TITLE SHEET				CR 316
STATE	COUNTY				SHEET NO.
TEXAS		KENT			
DISTRICT	CONTROL SECTION JOB			В] 4
ABL	0908 27 006			6	



PROPOSED TYPICAL SECTION

STA. 101+50.00 TO 102+15.00



CR 316 TYPICAL SECTION

© 2023 R	as Department of	Transportation
16.	из реригинені ог	т тапърагтанан

SCALE:	1" = 4'		SI	HEET	2	OF	2
FHWA DIVISION	PF	•	НΙ	GHWA	AY NO.		
6	SEE	SEE TITLE SHEET			CR	316	
STATE	COUNTY				SH	EET NO).
TEXAS	KENT						
DISTRICT	CONTROL SECTION JO			В		5	
ABL	0908	27 006					

CCSJ: 0908-27-006 County: Kent Highway: CR 316 General

ABILENE DISTRICT GENERAL NOTES **2014 SPECIFICATIONS**

Contractor questions on this project are to be addressed to the following individual(s):

Stewart Chapman, P.E. / Phone: 325-573-0143 / Stewart.Chapman@txdot.gov Maxie Allen, P.E. / Phone: 325-573-0142 / Maxie.Allen@txdot.gov Jose Cabrera, P.E. / Phone: 325-573-0143 / Jose.Cabrera@txdot.gov (Snyder Area Office)

Contractor questions will be accepted through email, phone, and in person by the above individuals.

For Q&A's on Proposals navigate to

https://tableau.txdot.gov/views/ProjectInformationDashboard/NoticetoContractors Use the dashboard to navigate to the project you are interested in by scrolling or filtering the dashboard using the controls on the left. Hover over the blue hyperlink for the project you want to view the Q&A for and click on the link in the window that pops up.

All relevant project documentation including contract time, cross sections, etc will be posted on the districts FTP website. https://ftp.dot.state.tx.us/pub/txdot-info/Pre-Letting%20Responses/

Failure to make necessary corrections to SW3P based on SW3P inspections will be cause for withholding the monthly estimate until such corrections have been made.

Failure to make necessary corrections to traffic control items based on barricade inspections will be cause for withholding the monthly estimate until such corrections have been made.

Provide ingress/egress to the adjacent properties in areas under construction. Phased construction of driveways and streets shall be required to provide uninterrupted access to adjacent properties. Coordinate work with the property owners before beginning any construction in the vicinity of the drive.

Environmental

Endangered and Protected Species

1. Migratory Bird Treaty Act (MBTA) - Establishment of a Federal prohibition, unless permitted by regulations, to "pursue, hunt, take, capture, kill, attempt to take, capture or kill, possess, offer for sale, sell, offer to purchase, purchase, deliver for shipment, ship, cause to be shipped, deliver for transportation, transport, cause to be transported, carry, or cause to be carried by any means whatever, receive for shipment, transportation or carriage, or export, at any time, or in any manner, any migratory bird, included in the

> General Notes Sheet A

CCSJ: 0908-27-006 County: Kent Highway: CR 316

> terms of this Convention . . . for the protection of migratory birds . . . or any part, nest, or egg of any such bird." (16 U.S.C. 703)

Best Management Practices

- 1. Comply with the SW3P and associated sheets.
- 2. Migratory Birds:
 - a. Bird nesting season is typically 15Feb through 15Sep annually.
 - b. The Contractor will avoid disturbing, destroying, removing, or relocating migratory birds and active nests found in trees, culverts, bridges, on the ground, or anywhere they are encountered.
 - c. Perform all tree trimming and other vegetation clearing activities during the nonbreeding season (typically 15Sep-15Feb annually). Perform any inactive nest removal and bird exclusion methods to prevent birds from establishing nests. Phasing of work during construction may be necessary to stay in compliance.
 - d. When active nests are unexpectedly encountered on-site during construction, the Contractor will stop work and immediately notify the Engineer. Take measures to avoid disturbance of these birds, their occupied nest, eggs, and/or young, in accordance with the Migratory Bird Treaty Act, Texas Parks and Wildlife Code, and TxDOT policy.
 - e. The Engineer will notify the Contractor when work may resume.
 - f. The Contractor should be prepared to prevent migratory birds from building nests by utilizing nest prevention methods, such as bird-deterrent netting and bird-repelling sprays and/or gels, between 15Feb and 15Sep. The Contractor can discuss other preventative measures with the Engineer and/or District Environmental Staff.
- 3. Other Best Management Practices for State Protected Species
 - a. If Black Tailed Prairie Dog (BTPD) burrows or pocket gopher mounds are found near or within the project area, place barrier fencing to discourage the individual animals of moving into or through the construction area.
 - b. While seeding or revegetating, if BTPD or pocket gopher mounds are discovered near or within the planned area, a vegetative barrier should be planted to discourage the dispersal of the species within the TxDOT ROW.
 - c. If any animals are within the project area, avoid harming when encountered. Let them leave the area without harassment. Avoid any unnecessary impacts to dens or burrows.
 - d. Examine heavy equipment stored on site before use, particularly after rain events when reptile and amphibian movements occur more often, to ensure use will not harm individuals that might be seeking temporary refuge.
 - e. Avoid or minimize disturbing or removing downed trees, rotting stumps, and leaf litter where feasible.

General Notes Sheet B

epartment of Transportation

CR 316

CCSJ: 0908-27-006 County: Kent Highway: CR 316

- f. For open trenches and excavated pits, install escape ramps at an angle of less than 45 degrees (1:1) in areas left uncovered. Visually inspect excavation areas for the presence of wildlife prior to backfilling.
- g. Rubbish found near bridges on TxDOT ROW should be removed and disposed of properly to minimize the risk of pollution. Rubbish does not include brush piles or
- h. If Texas tortoises (Gopherus berlandieri) or box turtles (Terrepene spp.) are present in a project area, they should be removed from the area and relocated between 100 and 200 meters from the project area.
- i. Minimize the amount of vegetation cleared. Removal of native vegetation, particularly mature native trees and shrubs should be avoided.

Item 5, "Control of Work"

Use Method C for construction surveying.

All known utilities are identified in the plans, including the crossing of power lines. Use this information to identify potential issues with power poles and power lines prior to bidding.

Make necessary arrangements with utility owners regarding temporary protections such as bracing power poles, and de-energizing power lines. The Department will not reimburse the cost of such temporary protections to the Contractor, unless the Engineer determines that inadequate information was available at the time the project was bid. "Call Before You Dig" "Call 811"

Drilled shaft locations or excavation areas must be staked prior to the notification so that the underground utilities can be located in relationship to the proposed work. Preserve and document the marked utility locations to prevent unnecessary secondary notifications. Notify the Engineer of conflicts between proposed work and underground utilities.

"When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at

Alternate Precast Proposal Submission (txdot.gov)

Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor."

Item 6, "Control of Materials"

To comply with the latest provisions of Build America, Buy America Act (BABA Act) of the Bipartisan Infrastructure Law, the contractor must submit a notarized original of the TxDOT Construction Material Buy America Certification Form for all items classified as construction materials. This form is not required for materials classified as a manufactured product.

> General Notes Sheet C

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Refer to the Buy America Material Classification Sheet for clarification on material categorization.

The Buy America Material Classification Sheet is located at the below link.

https://www.txdot.gov/business/resources/materials/buy-america-material-classificationsheet.html for clarification on material categorization.

Item 7, "Legal Relations and Responsibilities"

Provide one SW3P Notification Board for this project. Notification Boards are to be placed at locations within the right-of-way but outside the clear zone as directed by the Engineer. Consider this work to be subsidiary to the various bid items of the contract.

The Contractor's attention is directed to the Texas Aggregate Quarry Pit Safety Act. Any pit or quarry meeting the definition of an unacceptable unsafe location as defined in the Act is subject to regulations set forth in this Act. A copy of the Texas Administrative Code, Title 43, Part, 1, Chapter 21, Subchapter M may be viewed at

https://texreg.sos.state.tx.us/public/readtac\$ext.ViewTAC?tac_view=5&ti=43&pt=1&ch=21&sc h=M&rl=Y

No significant traffic generator events identified

Hard hats are required at all times during construction when construction personnel are in TxDOT Right-of-Way.

Item 8 "Prosecution and Progress"

Each contract awarded by the Department stands on its own and as such, is separate from other contracts. A Contractor awarded multiple contracts must be capable and sufficiently staffed to concurrently process and/or execute all contracts at the same time.

Coordinate and update the work schedule with the project inspector daily. Give a minimum of 24 hours of notice to project inspector if work requiring inspection or testing is to be performed. Failure to do so may cause that work to be delayed or postponed if TxDOT personnel are not available. Work performed without suitable inspection, as determined by the Engineer, may be ordered removed and replaced at Contractor's expense.

Item 9, "Measurement and Payment"

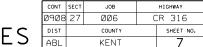
The progress payment period shall end on the 25th of each month, unless directed by the Area Office Engineer. Material on Hand (MOH) is due two business days before estimate cut off.

Item 100, "Preparing Right of Way"

The Contractor's attention is directed to potential regulations against burning within the project limits. Abide by all local ordinances and county imposed burn bans. When burning is prohibited, dispose of material in accordance with regulations set forth by other regulatory

> General Notes Sheet D





CCSJ: 0908-27-006 County: Kent Highway: CR 316

agencies including the Texas Commission for Environmental Quality. The cost of burning or disposal of any product is subsidiary to various bid items.

Item 247, "Flexible Base"

Provide flexible base with a minimum Plasticity index of 4

Ride quality is waved for this project.

The flexible base material in this contract has been estimated to be 128 cubic yards (compacted). The estimated quantity of flexible base is for the roadway. The measured area for payment is the crown width only. The tapers, etc., are not included in the measurements for the flexible base and are considered subsidiary to this item.

Item 416, "Drilled Shaft Foundations"

Riprap will be paid for under item 432.

All soil, water, and slurry removed from drilled shafts shall be captured and disposed of properly. No discharge of these materials into, or in close proximity to, the surrounding water will be allowed.

Item 420, "Concrete Substructures"

In addition to the elements shown in table 1, the following elements are Plans Quantity Elements.

• Bent Concrete

Item 421, "Hydraulic Cement Concrete"

Use a cement meeting the requirements of Ty II when Mix Design Option 7 is selected for cast in place concrete.

Class C fly ash and Type I cement will not be allowed for any mix unless approved by the Engineer.

As a minimum, curing facility includes concrete curing tank, heater and a concrete recording thermometer. Provide a recorder with the capability to chart temperatures for 24 hours, 7 days and 30 day periods of time.

Air Entrainment requirements are waived with exception to bridge deck concrete, and rails, top slabs of direct traffic culverts and approach slabs. Air Entrainment is required for all slip formed concrete (bridge rail, concrete traffic barrier, pavement, etc.).

Item 422, "Concrete Superstructures"

Saw-cut groves are not required.

General Notes Sheet E **CCSJ:** 0908-27-006 County: Kent Highway: CR 316

Provide either a carpet drag or broom finish for micro-texture to deck and approach slabs.

Item 440, "Reinforcement for Concrete"

Provide epoxy coated reinforcement for all reinforcement in abutment caps, wingwalls, and backwalls (drilled shaft reinforcement excluded); interior bent caps (column and drilled shaft reinforcement excluded); cast-in-place portions of bridge deck (PCP reinforcement and bridge girder reinforcement excluded); bridge railing; and approach slab.

Item 496, "Removing Structures"

The contractor will be required to provide a demo plan for bridge structures to be approved by the engineer.

Item 502, "Barricades, Signs and Traffic Handling"

Provide the Engineer with written notification seven (7) days in advance of major traffic changes. A major traffic change is defined as the temporary (greater than one day) or permanent relocation of traffic lanes typically in an urban setting. The notice will, at a minimum, include the expected date, time and scope of the traffic change. The Department will utilize the information provided to inform the traveling public of the changes. Failure to provide advance notice, or to provide accurate information, will result in delaying the work until such time that the public has been notified.

Additional signs, barricades and traffic handling may be necessary to complete the work shown herein and will be provided by the contractor as required and will be considered subsidiary to this item.

Relocate existing roadside signs to temporary supports as approved by the engineer.

All safety appurtenances such as signs, delineators, object markers and route markers will be in place prior to opening each phase of the construction to traffic, unless otherwise directed.

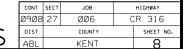
The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

The Contractor's person responsible for TCP compliance must be available by local telephone and have a response time within 45 minutes.

Equip all work vehicles within 30 feet of the traveled way with a functioning amber strobe light or rotating beacon visible from all directions.

> General Notes Sheet F





CCSJ: 0908-27-006 County: Kent Highway: CR 316

Repair barricades within the timeline shown on the barricade inspection report. Failure to comply will cease all work until barricades are repaired to the satisfaction of the Department. Replace all damaged traffic control devices immediately. Remove any damaged traffic control devices from the project within 24 hours.

Conflicting guide signs shall be covered as approved by the Engineer. This work shall be subsidiary to Item 502.

Item 506, "Temporary Erosion, Sedimentation, and Environmental Controls"

On site concrete washout shall be allowed on this project as shown in the Environmental Layout Sheet, or as directed, in writing, by the Engineer.

Item 644, "Small Roadside Sign Supports and Assemblies"

Remove entire small sign foundation.

Item 658, "Delineator and Object Marker Assemblies"

Delineators and object marker assemblies will use winged channel posts. The winged channel posts will be 1.12 lb/ft and 6.5 ft in length.

All MBGF delineation shall be GF2 mounted on posts.

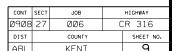
Use a minimum 2 inch long lag screws with washers to attach flexible GF2 barrier reflectors to wooden post. For steel posts, use an approved adhesive, or other method approved by Engineer.

Concrete Barrier Reflectors shall be equivalent to Shure-tite CTB "Cup Mount" Delineator (8"). Attach delineators to concrete rail with concrete anchors as approved by the Engineer.

General Notes

Sheet G

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Estimate & Quantity Sheet

CONTROLLING PROJECT ID 0908-27-006

DISTRICT Abilene
HIGHWAY CR 316

COUNTY Kent

		CONTROL SECTION	N JOB	0908-27	7-006		
		PROJ	ECT ID	A00190	0600		
	COUNTY				ıt	TOTAL EST.	TOTAL
		HIG	HWAY	CR 3	16	0 20 14 10000000000000000000000000000000	FINAL
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL		
	100-6001	PREPARING ROW	AC	1.000		1.000	
	110-6001	EXCAVATION (ROADWAY)	CY	16.000		16.000	
	110-6002	EXCAVATION (CHANNEL)	CY	824.000		824.000	
	132-6004	EMBANKMENT (FINAL)(DENS CONT)(TY B)	CY	558.000		558.000	
	150-6002	BLADING	HR	16.000		16.000	
	247-6237	FL BS (CMP IN PLC)(TY A OR B GR 4)(6")	SY	948.000		948.000	
	400-6005	CEM STABIL BKFL	CY	49.000		49.000	
	409-6002	PRESTR CONC PIL (18 IN SQ)	LF	360.000		360.000	
	420-6013	CL C CONC (ABUT)	CY	27.200		27.200	
	420-6066	CL C CONC (RAIL FOUNDATION)	CY	18.600		18.600	
	422-6005	REINF CONC SLAB (BOX BEAM)	SF	1,723.000		1,723.000	
	425-6001	PRESTR CONC BOX BEAM (4B20)	LF	260.000		260.000	
	425-6002	PRESTR CONC BOX BEAM (5B20)	LF	130.000		130.000	
	432-6002	RIPRAP (CONC)(5 IN)	CY	54.000		54.000	
	434-6002	ELASTOMERIC BEARING (LAMINATED)	EA	18.000		18.000	
	450-6006	RAIL (TY T223)	LF	162.000		162.000	
	454-6018	SEALED EXPANSION JOINT (4 IN) (SEJ - M)	LF	52.000		52.000	
	496-6009	REMOV STR (BRIDGE 0 - 99 FT LENGTH)	EA	1.000		1.000	
	500-6001	MOBILIZATION	LS	1.000		1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	МО	5.000		5.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	578.000		578.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	578.000		578.000	
	506-6041	BIODEG EROSN CONT LOGS (INSTL) (12")	LF	370.000		370.000	
	506-6043	BIODEG EROSN CONT LOGS (REMOVE)	LF	370.000		370.000	
	540-6001	MTL W-BEAM GD FEN (TIM POST)	LF	50.000		50.000	
	540-6007	MTL BEAM GD FEN TRANS (TL2)	EA	4.000		4.000	
	544-6001	GUARDRAIL END TREATMENT (INSTALL)	EA	4.000		4.000	
	644-6076	REMOVE SM RD SN SUP&AM	EA	3.000		3.000	
	658-6014	INSTL DEL ASSM (D-SW)SZ (BRF)CTB (BI)	EA	12.000		12.000	
	658-6062	INSTL DEL ASSM (D-SW)SZ 1(BRF)GF2(BI)	EA	12.000		12.000	
	18	EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART)	LS	1.000		1.000	
		SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING)	LS	1.000		1.000	



DISTRICT	COUNTY	CCSJ	SHEET
Abilene	Kent	0908-27-006	10

	SUMMARY OF ROADWAY ITEMS								
	100	110	110	132	150	247	540	540	544
	6001	6001	6002	6004	6002	6237	6001	6007	6001
LOCATION	PREPARING ROW	EXCAVATION (ROADWAY)	EXCAVATION (CHANNEL)	EMBANKMENT (FINAL) (DENS CONT) (TY B)	BLADING	FL BS (CMP IN PLC) (TY A OR B GR 4) (6")		MTL BEAM GD FEN TRANS (TL2)	GUARDRAIL END TREATMENT (INSTALL)
	AC	CY	CY	CY	HR	SY	LF	EA	EA
COOPER CREEK	1	16	824	558	16	948	50	4	4
PROJECT TOTALS	1	16	824	558	16	948	50	4	4

	SUMMARY OF EROSION CONTROL ITEMS					
	506	506	506	506		
	6038	6039	6041	6043		
LOCATION	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDMT CONT FENCE (REMOVE)	BIODEG EROSN CONT LOGS (INSTL) (12')	BIODEG EROSN CONT LOGS (REMOVE)		
	LF	LF	LF	LF		
COOPER CREEK	578	578	370	370		
PROJECT TOTALS	578	578	370	370		

SUMMARY OF SIGNING ITEMS				
	658	658		
	6014	6062		
LOCATION	INSTL DEL ASSM (D-SW) SZ (BRF) CTB (BI)	INSTL DEL ASSM (D-SW) SZ 1 (BRF) GF2 (BI)		
	EA	EA		
COOPER CREEK	12	12		
PROJECT TOTALS	12	12		

EMOVAL ITEMS
644
6076
REMOVE SM RD SN SUP&AM
EA
3
3

QUANTITY SUMMARY



SCALE:	N/A		S	HEET	1	OF	1		
FHWA DIVISION	PF	ROJECT NO		нІ	HIGHWAY NO.				
6	SEE	SEE TITLE SHEET							
STATE		SHEET NO.							
TEXAS		KENT							
DISTRICT	CONTROL	DL SECTION JOB				11			
ABL	0908	27	000	6					

CSJ	PLAN PROFILE SHEET			DESIGN		BRIDGE LOCATION	STATION		CLEAR LENGTH RDWY WIDTH		LOADING	400-6005 CEM STABIL BKFL	409-6002 PRESTR CONC PIL (18 IN SQ)	CL C CONC (ABUT)
		EXISTING	PROPOSED	EXISTING	PROPOSED		BEGIN	END	FT	FT		CY	LF	CY
0908-27-006	48	08-132-0-AA01-13-001	08-132-0-AA01-13-003	SINGLE SPAN STEEL GIRDER & STRINGER BRIDGE ON STEEL SUBSTRUCTURE	65' CONC. BOX BEAM BRIDGE	CR 316 AT COOPER CREEK	101+50.00	102+15.00	65	24	HL -93	49	360	27.2
TOTALS												49	360	27.2

CSJ (CONT'D FROM ABOVE)	420-6066 CL C CONC (RAIL FOUNDATION)	422-6005 REFIN CONC SLAB (BOX BEAM)	425-6001 PRESTR CONC BOX BEAM (4B20)	425-6002 PRESTR CONC BOX BEAM (5B20)	432-6002 RIPRAP (CONC) (5IN)	434-6002 ELASTOMERTIC BEARING (LAMINATED)	450-6006 RAIL (TY T223)	454-6018 SEALED EXPANSION JOINT (4IN) (SEJ-M)	496-6009 REMOV STR (BRIDGE 0-99FT LENGTH)
	CY	SF	LF	LF	CY	EA	LF	LF	EA
0908-27-006	18.6	1723	260	130	54	18	162	52	1
TOTALS	18.6	1723	260	130	54	18	162	52	1



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Texas Department of Transportation

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TEXAS		KENT						
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2. The development and design of the Traffic Control Plan (TCP) is the responsibility of the Engineer.

- 3. The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop, sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.

of this standard is governed by the "Texas Engineering Practice Act". No warranty of any by TxDOI for any purpose whatsoever. TxDOI assumes no responsibility for the conversion addard to other formats or for incorrect results or damages resulting from its use. (27006/4 - Design/Plan Set/2, TCP/O14 025 BC (1)-(12) -2)

- 6. When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- 7. The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- 9. The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES, CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:

- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel," or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

- Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD) DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS) MATERIAL PRODUCER LIST (MPL) ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)" STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD) TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD) TRAFFIC ENGINEERING STANDARD SHEETS

SHEET 1 OF 12



Standard

BARRICADE AND CONSTRUCTION
GENERAL NOTES
AND REQUIREMENTS

BC(1)-21

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ROAD

CLOSED R11-2

Type 3

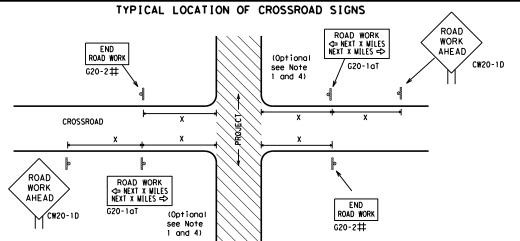
devices

Barricade or

channelizina

CW13-1P

Channelizing Devices



- \sharp May be mounted on back of "ROAD WORK AHEAD" (CW20-1D) sign with approval of Engineer. (See note 2 below)
- The typical minimum signing on a crossroad approach should be a "ROAD WORK AHEAD" (CW20-1D) sign and a (G20-2) "END ROAD WORK" sign, unless noted otherwise in plans.
- 2. The Engineer may use the reduced size 36" x 36" ROAD WORK AHEAD (CW20-1D) sign mounted back to back with the reduced size 36" x 18" "END ROAD WORK" (G20-2) sign on low volume crossroads (see Note 4 under "Typical Construction Warning Sign Size and Spacing"). See the "Standard Highway Sign Designs for Texas" manual for sign details. The Engineer may omit the advance warning signs on low volume crossroads. The Engineer will determine whether a road is low volume as per TMUTCD Part 5. This information shall be shown in the plans.
- Based on existing field conditions, the Engineer/Inspector may require additional signs such as FLAGGER AHEAD, LOOSE GRAVEL, or other appropriate signs. When additional signs are required, these signs will be considered part of the minimum requirements. The Engineer/Inspector will determine the proper location and spacing of any sign not shown on the BC sheets, Traffic Control Plan sheets or the Work Zone Standard Sheets.
- The "ROAD WORK NEXT X MILES" (G20-1aT) sign shall be required at high volume crossroads to advise motorists of the length of construction in either direction from the intersection. The Engineer will determine whether a roadway is considered high volume.
- Additional traffic control devices may be shown elsewhere in the plans for higher volume crossroads.

SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING DOWNSTREAM OF THE CSJ LIMITS

ROAD

WORK

AHEAD

CW20-1D

When work occurs in the intersection area, appropriate traffic control devices, as shown elsewhere in the plans or as determined by the Engineer/Inspector, shall be in place.

BEGIN T-INTERSECTION WORK ZONE ★ ★ G20-9TP ★ ★ R20-5T FINES DOUBL X R20-5aTP MORKERS ARE PRESENT ROAD WORK ← NEXT X WILES X X G20-2bT WORK ZONE G20-1bTI INTERSECTED 1000' - 1500' - Hwy 1 Block - City 1000'-1500' - Hwy 1 Block - City ROADWAY \Rightarrow ROAD WORK G20-16TR NEXT X MILES => WORK ZONE G20-2bT * * Limit BEGIN G20-5T * * G20-9TP ZONE TRAFFI G20-6T * * R20-5T FINES DOUBLE X X R20-5aTP WHEN WORKERS ROAD WORK G20-2

CSJ LIMITS AT T-INTERSECTION

STAY ALERT

TALK OR TEXT LATER

END |

WORK ZONE G20-26T * *

G20-10

OBEY

SIGNS

STATE LAW

 \Rightarrow

R20-3T

- 1. The Engineer will determine the types and location of any additional traffic control devices, such as a flagger and accompanying signs, or other signs, that should be used when work is being performed at or near an intersection.
- 2. If construction closes the road at a T-intersection, the Contractor shall place the "CONTRACTOR NAME"(G20-6T) sign behind the Type 3 Barricades for the road closure (see BC(10) also). The "ROAD WORK NEXT X MILES" left arrow(G20-1bTL) and "ROAD WORK NEXT X MILES" right arrow (G20-1bTR)" signs shall be replaced by the detour signing called for in the plans.

TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING 1,5,6

SIZE

onventional

48" x 48"

Expressway/ Freeway	Posted Speed	Sign∆ Spacing "X"
	MPH	Feet (Apprx.)
48" × 48"	30	120
70	35	160
	40	240
	45	320
48" × 48"	50	400
70 2 70	55	500 ²
	60	600 ²
	65	700 ²
48" × 48"	70	800 ²
	75	900 ²
	80	1000 ²
	*	* 3

SPACING

- CW7. CW8. 48" x 48 36" × 36' CW9, CW11 CW14 CW3, CW4, CW5, CW6, 48" x 48" 48" x 48 CW8-3, CW10, CW12
- * For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.
- \triangle Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

Sign

Number

or Series

CW20' CW21

CW22

CW23

CW25

CW1, CW2,

- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- 3. Distance between signs should be increased as required to have 1/2 mile or more advance warning.
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D) signs may be used on low volume crossroads at the discretion of the Engineer as per TMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.
- 6. See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design

SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING AT THE CSJ LIMITS WORK AREAS IN MULTIPLE LOCATIONS WITHIN CSJ LIMITS X X G20-9TP SPEED STAY ALERT ROAD LIMIT R4-1 DO NOT PASS appropriate: OBEY TRAFFIC **X X** R20-5T WORK WARNING * * G20-5T ROAD WORK CW1-4L AHEAD DOUBLE SIGNS * * R20-5aTP ME PRESENT CW20-1D ROAD STATE LAW TALK OR TEXT LATER CW13-1P R2-1++ ROAD ★ ★ G20-6T WORK R20-3T * * WORK G20-10T * * AHEAD AHEAD Type 3 Barricade or WPH CW13-1P CW20-1D channelizing devices \Diamond \Diamond \Diamond \Leftrightarrow \Rightarrow \Leftrightarrow \Rightarrow \Rightarrow Beginning of NO-PASSING SPEED END G20-2bt * * R2-1 LIMIT line should $\otimes \times \times$ coordinate ROAD WORK When extended distances occur between minimal work spaces, the Engineer/Inspector should ensure additional with sign ROAD WORK AHEAD"(CW20-1D)signs are placed in advance of these work areas to remind drivers they are still G20-2 X X location NOTES within the project limits. See the applicable TCP sheets for exact location and spacing of signs and

★ ★G20-9TP

¥ ¥R20-5T

X X R20-5aTP SHEN SHEEN ARE PRESENT

SPEED

LIMIT

-CSJ Limi

R2-1

BEGIN ROAD WORK NEXT X MILES

* *G20-5T

* *G20-6T

END

ROAD WORK

G20-2 * *

ROAD

WORK

√2 MILE

CW20-1E

ZONE

TRAFFI

FINES

DOUBLE

SPEED R2-1

LIMIT

The Contractor shall determine the appropriate distance to be placed on the G20-1 series signs and "BEGIN ROAD WORK NEXT X MILES" (G20-5T) sign for each specific project. This distance shall replace the "X" and shall be rounded to the nearest whole mile with the approval of the Engineer.

- The "BEGIN WORK ZONE" (G20-9TP) and "END WORK ZONE" (G20-2b1 shall be used as shown on the sample layout when advance signs are required outside the CSJ Limits. They inform the motorist of entering or leaving a part of the work zone lying outside the CSJ Limits where traffic fines may double if workers are present.
- CSJ limit signing is required for highway construction and maintenance work, with the exception of mobile operations.
- Area for placement of "ROAD WORK AHEAD" (CW20-1D) sign and other signs or devices as called for on the Traffic
- Contractor will install a regulatory speed limit sign at the end of the work zone.

	LEGEND
Ι	Type 3 Barricade
000	Channelizing Devices
4	Sign
X	See Typical Construction Warning Sign Size and Spacing chart or the TMUTCD for sign spacing requirements.

SHEET 2 OF 12



BARRICADE AND CONSTRUCTION PROJECT LIMIT

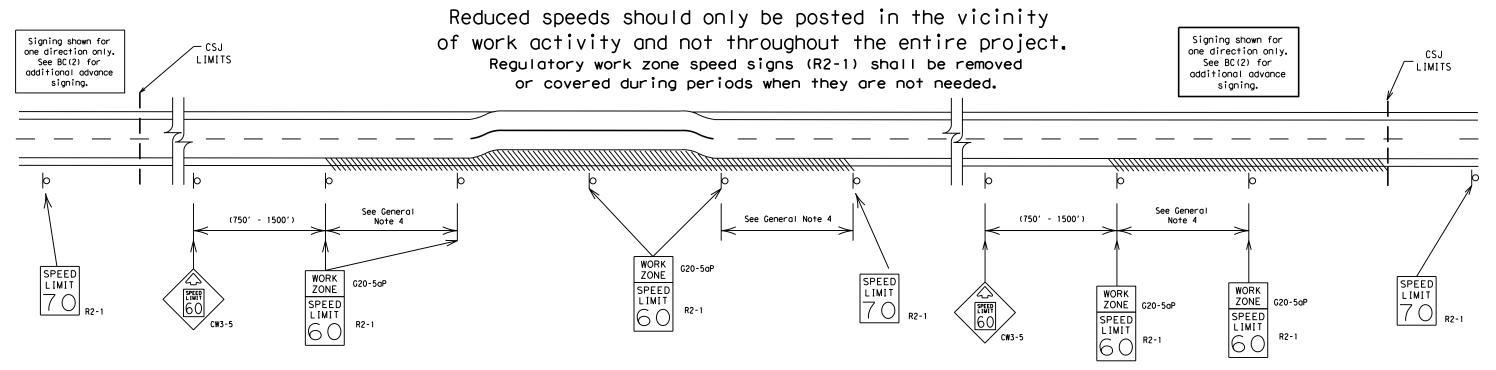
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TYPICAL APPLICATION OF WORK ZONE SPEED LIMIT SIGNS

Work zone speed limits shall be regulatory, established in accordance with the "Procedures for Establishing Speed Zones," and approved by the Texas Transportation Commission, or by City Ordinance when within Incorporated City Limits.



GUIDANCE FOR USE:

LONG/INTERMEDIATE TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit should be included on the design of the traffic control plans when restricted geometrics with a lower design speed are present in the work zone and modification of the geometrics to a higher design speed is not feasible.

Long/Intermediate Term Work Zone Speed Limit signs, when approved as described above, should be posted and visible to the motorist when work activity is present. Work activity may also be defined as a change in the roadway that requires a reduced speed for motorists to safely negotiate the work area, including:

- a) rough road or damaged pavement surface
- b) substantial alteration of roadway geometrics (diversions)
- c) construction detours
- d) grade
- e) width
- f) other conditions readily apparent to the driver

As long as any of these conditions exist, the work zone speed limit signs should remain in place.

SHORT TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit may be included on the design of the traffic control plans when workers or equipment are not behind concrete barrier, when work activity is within 10 feet of the traveled way or actually in the traveled way.

Short Term Work Zone Speed Limit signs should be posted and visible to the motorists only when work activity is present. When work activity is not present, signs shall be removed or covered. (See Removing or Covering on BC(4)).

GENERAL NOTES

- Regulatory work zone speed limits should be used only for sections of construction projects where speed control is of major importance.
- Regulatory work zone speed limit signs shall be placed on supports at a 7 foot minimum mounting height.
- 3. Speed zone signs are illustrated for one direction of travel and are normally posted for each direction of travel.
- 4. Frequency of work zone speed limit signs should be:

40 mph and greater 0.2 to 2 miles

35 mph and less 0.2 to 1 mile

- 5. Regulatory speed limit signs shall have black legend and border on a white reflective background (See "Reflective Sheeting" on BC(4)).
- Fabrication, erection and maintenance of the "ADVANCE SPEED LIMIT" (CW3-5) sign, "WORK ZONE" (G20-5aP) plaque and the "SPEED LIMIT" (R2-1) signs shall not be paid for directly, but shall be considered subsidiary to Item 502.
- 7. Turning signs from view, laying signs over or down will not be allowed, unless as otherwise noted under "REMOVING OR COVERING" on BC(4).
- 8. Techniques that may help reduce traffic speeds include but are not limited to:
 A. Law enforcement.
 - B. Flagger stationed next to sign.
 - C. Portable changeable message sign (PCMS).
 - D. Low-power (drone) radar transmitter.
 - E. Speed monitor trailers or signs.
- Speeds shown on details above are for illustration only.
 Work Zone Speed Limits should only be posted as approved for each project.
- 10. For more specific guidance concerning the type of work, work zone conditions and factors impacting allowable regulatory construction speed zone reduction see TxDOT form #1204 in the TxDOT e-form system.

SHEET 3 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION WORK ZONE SPEED LIMIT

BC(3)-21

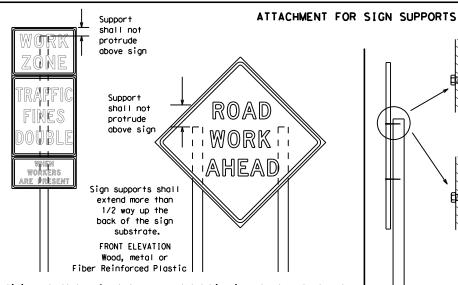
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this standard i y TxDOI for any rd to other form

TYPICAL MINIMUM CLEARANCES FOR LONG TERM AND INTERMEDIATE TERM SIGNS 12' min. ROAD ROAD ROAD ROAD WORK minimum WORK WORK WORK from AHEAD AHEAD AHEAD curb AHEAD min. * * XX 7.0' min. 7.0' min. 9.0' max. 6' or 7.0' min. 9.0' max. 6.0' min. greater 9.0' max. Paved Paved shou I der shoul de

* When placing skid supports on unlevel ground, the leg post lengths must be adjusted so the sign appears straight and plumb. Objects shall NOT be placed under skids as a means of leveling.

* * When plaques are placed on dual-leg supports, they should be attached to the upright nearest the travel lane. Supplemental plaques (advisory or distance) should not cover the surface of the parent sign.



Splicing embedded perforated square metal tubing in order to extend post height will only be allowed when the splice is made using four bolts, two above and two below the spice point. Splice must be located entirely behind the sign substrate, not near the base of the support. Splice insert lengths should be at least 5 times nominal post size, centered on the splice and of at least the same gauge material.

SIDE ELEVATION

Wood

substrates to other types of sign supports Nails shall NOT be allowed. Each sign shall be attached directly to the sign support. Multiple

Attachment to wooden supports

will be by bolts and nuts

or screws. Use TxDOT's or

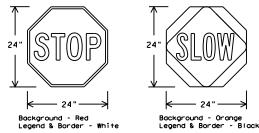
manufacturer's recommended

procedures for attaching sign

signs shall not be joined or spliced by any means. Wood supports shall not be extended or repaired by splicing or other means.

STOP/SLOW PADDLES

- 1. STOP/SLOW paddles are the primary method to control traffic by flaggers. The STOP/SLOW paddle size should be 24" x 24". STOP/SLOW paddles shall be retroreflectorized when used at night.
- 3. STOP/SLOW paddles may be attached to a staff with a minimum length of 6' to the bottom of the sign.
- 4. Any lights incorporated into the STOP or SLOW paddle faces shall only be as specifically described in Section 6E.03 Hand Signaling Devices in the TMUTCD.



SHEETING RE	QUIREMEN'	TS (WHEN USED AT NIGHT)
USAGE	COLOR	SIGN FACE MATERIAL
BACKGROUND	RED	TYPE B OR C SHEETING
BACKGROUND	ORANGE	TYPE B _{FL} OR C _{FL} SHEETING
LEGEND & BORDER	WHITE	TYPE B OR C SHEETING
LEGEND & BORDER	BLACK	ACRYLIC NON-REFLECTIVE FILM

CONTRACTOR REQUIREMENTS FOR MAINTAINING PERMANENT SIGNS WITHIN THE PROJECT LIMITS

- Permanent signs are used to give notice of traffic laws or regulations, call attention to conditions that are potentially hazardous to traffic operations, show route designations, destinations, directions, distances, services, points of interest, and other geographical, recreational, specific service (LOGO), or cultural information. Drivers proceeding through a work zone need the same, if not better route guidance as normally installed on a roadway without construction.
- When permanent regulatory or warning signs conflict with work zone conditions, remove or cover the permanent signs until the permanent sign message matches the roadway condition. For details for covering large guide signs see the TS-CD standard.
- When existing permanent signs are moved and relocated due to construction purposes, they shall be visible to motorists at all times.
- If existing signs are to be relocated on their original supports, they shall be installed on crashworthy bases as shown on the SMD Standard sheets. The signs shall meet the required mounting heights shown on the BC Sheets or the SMD Standards. This work should be paid for under the appropriate pay item for relocating existing signs.
- If permanent signs are to be removed and relocated using temporary supports. the Contractor shall use crashworthy supports as shown on the BC standard sheets, TLRS standard sheets or the CW7TCD list. The signs shall meet the required mounting heights shown on the BC, or the SMD standard sheets during construction. This work should be paid for under the appropriate pay item for relocating existing signs.
- Any sign or traffic control device that is struck or damaged by the Contractor or his/her construction equipment shall be replaced as soon as possible by the Contractor to ensure proper guidance for the motorists. This will be subsidiary to Item 502.

GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports.
- All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and guide the traveling public safely through the work zone.
- The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question reaardina installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so the Engineer can verify the correct procedures are being followed.
- The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used for identification shall be 1 inch.
- The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

<u>DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)</u>

- The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in regard to crashworthiness and duration of work requirements.
- a. Long-term stationary work that occupies a location more than 3 days.
- Intermediate-term stationary work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting more than one hour.
- Short-term stationary daytime work that occupies a location for more than 1 hour in a single daylight period.
- Short, duration work that occupies a location up to 1 hour.
- Mobile work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except as shown for supplemental plagues mounted below other signs.
- The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above
- the ground. Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to appropriate Long-term/Intermediate sign height.
- Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

SIZE OF SIGNS

The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

SIGN SUBSTRATES

- The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports.
- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave.
- All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6" centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- 1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300
- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).
- White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background.
- 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL} , shall be used for rigid signs with orange backgrounds.

SIGN LETTERS

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any intersections where the sign may be seen from approaching traffic.
- Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely covered when not required.
- When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.
- Burlap shall NOT be used to cover signs.
- Duct tape or other adhesive material shall NOT be affixed to a sign face. Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a
- constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular
- impact. Rubber (such as tire inner tubes) shall NOT be used. Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured
- with rubber bases may be used when shown on the CWZTCD list. Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

SHEET 4 OF 12

Traffic Safety Division Standard



BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

BC(4)-21

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9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	ABL		KENT			17

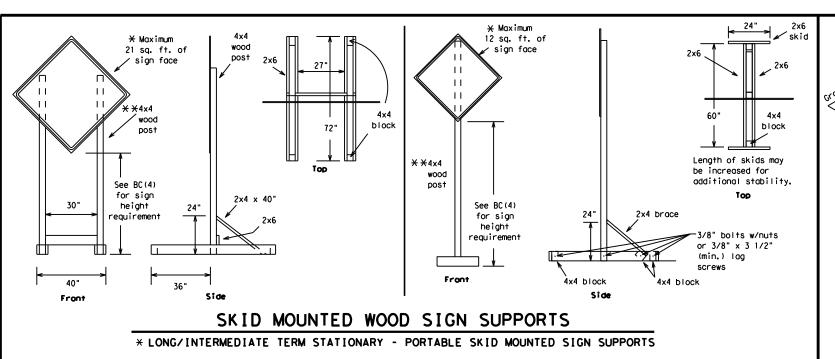


opposite sides going in opposite directions. Minimum

weld, do not

back fill puddle.

weld starts here



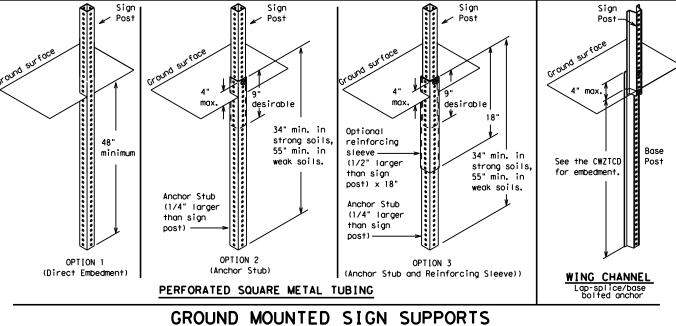
-2" x 2"

12 ga. upright

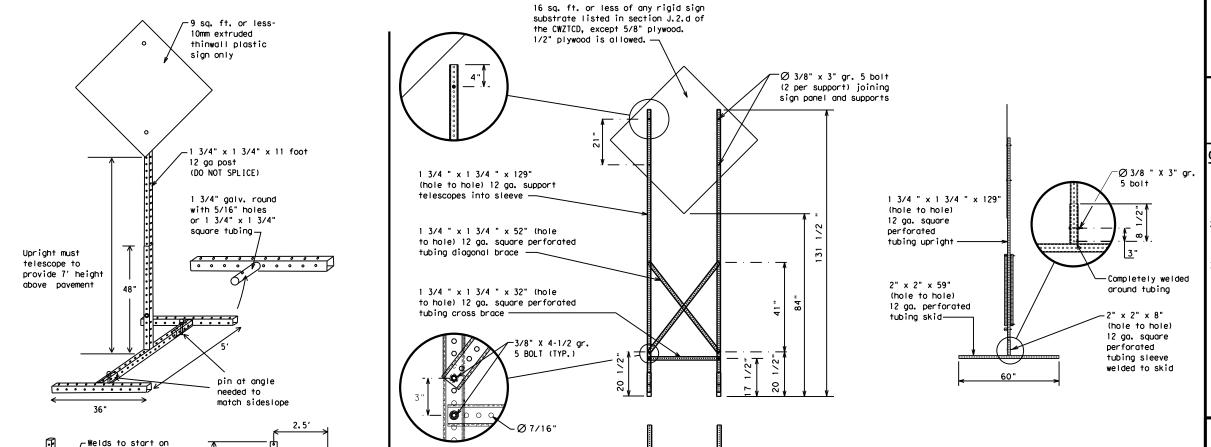
2"

SINGLE LEG BASE

Side View



Refer to the CWZTCD and the manufacturer's installation procedure for each type sign support. The maximum sign square footage shall adhere to the manufacturer's recommendation. Two post installations can be used for larger signs.



WEDGE ANCHORS

Both steel and plastic Wedge Anchor Systems as shown on the SMD Standard Sheets may be used as temporary sign supports for signs up to 10 square feet of sign face. They may be set in concrete or in sturdy soils if approved by the Engineer. (See web address for "Traffic Engineering Standard Sheets" on BC(1)).

OTHER DESIGNS

MORE DETAILS OF APPROVED LONG/INTERMEDIATE AND SHORT TERM SUPPORTS CAN BE FOUND ON THE CWZTCD LIST. SEE BC(1) FOR WEBSITE LOCATION.

GENERAL NOTES

- Nails may be used in the assembly of wooden sign supports, but 3/8" bolts with nuts or 3/8" x 3 1/2" lag screws must be used on every joint for final
- No more than 2 sign posts shall be placed within a 7 ft. circle, except for specific materials noted on the CWZTCD List.
- When project is completed, all sign supports and foundations shall be removed from the project site. This will be considered subsidiary to Item 502.
 - ★ See BC(4) for definition of "Work Duration."
 - Wood sign posts MUST be one piece. Splicing will NOT be allowed. Posts shall be painted white.
 - ☐ See the CWZTCD for the type of sign substrate that can be used for each approved sign support.

SHEET 5 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION TYPICAL SIGN SUPPORT

BC(5)-21

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	SKID	MOUNTED	PERFORATED	SQUARE	STEEL	TUBING	SIGN	SUPPORTS
--	------	---------	------------	--------	-------	--------	------	----------

32'

* LONG/INTERMEDIATE TERM STATIONARY - PORTABLE SKID MOUNTED SIGN SUPPORTS

PORTABLE CHANGEABLE MESSAGE SIGNS

No warranty of any for the conversion om its use.

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO, "FOR." "AT." etc.
- Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by
- 4. Use the word "EXIT" to refer to an exit ramp on a freeway: i.e., "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- Do not use the word "Danger" in message.
 Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- 15. PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

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designation # IH-number, US-number, SH-number, FM-number

RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES

MERGE

RIGHT

DETOUR

X EXITS

USE

EXIT XXX

STAY ON

US XXX

SOUTH

TRUCKS

USF

US XXX N

WATCH

FOR

TRUCKS

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DELAYS

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USE

OTHER

ROUTES

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LANE

Action to Take/Effect on Travel

List

FORM

X LINES

RIGHT

USE

XXXXX

RD EXIT

USE EXIT

I-XX

NORTH

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I-XX F

TO I-XX N

WATCH

FOR

TRUCKS

EXPECT

DELAYS

PREPARE

TO

STOP

END

SHOULDER

USE

WATCH

FOR

WORKERS

(The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

FREEWAY CLOSED X MILE	FRONTAGE ROAD CLOSED	ROADWORK XXX FT	ROAD REPAIRS XXXX FT
ROAD CLOSED AT SH XXX	SHOULDER CLOSED XXX FT	FLAGGER XXXX FT	LANE NARROWS XXXX FT
ROAD CLSD AT FM XXXX	RIGHT LN CLOSED XXX FT	RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE
RIGHT X LANES CLOSED	RIGHT X LANES OPEN	MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT
CENTER LANE CLOSED	DAYTIME LANE CLOSURES	LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT
NIGHT LANE CLOSURES	I-XX SOUTH EXIT CLOSED	DETOUR X MILE	ROUGH ROAD XXXX FT
VARIOUS LANES CLOSED	EXIT XXX CLOSED X MILE	ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN
EXIT CLOSED	RIGHT LN TO BE CLOSED	BUMP XXXX FT	US XXX EXIT X MILES
MALL DRIVEWAY CLOSED	X LANES CLOSED TUE - FRI	TRAFFIC SIGNAL XXXX FT	LANES SHIFT

APPLICATION GUIDELINES

Phase Lists".

1. Only 1 or 2 phases are to be used on a PCMS.

2. The 1st phase (or both) should be selected from the

is not included in the first phase selected.

and should be understandable by themselves.

no more than one week prior to the work.

"Road/Lane/Ramp Closure List" and the "Other Condition List".

a minimum of 1000 ft. Each PCMS shall be limited to two phases,

of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for

6. For advance notice, when the current date is within seven days

3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice

4. A Location Phase is necessary only if a distance or location

5. If two PCMS are used in sequence, they must be separated by

* LANES SHIFT in Phase 1 must be used with STAY IN LANE in Phase 2.

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- appropriate.
- be interchanged as appropriate.

- 9. Distances or AHEAD can be eliminated from the message if a

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS

SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

FULL MATRIX PCMS SIGNS

same size arrow.

BLVD

CLOSED

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under "PORTABLE CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the Engineer, it shall maintain the legibility/visibility requirement listed above When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and shall not substitute
- for, or replace that sign. 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(7), for the

WORDING ALTERNATIVES

2. Roadway designations IH, US, SH, FM and LP can be interchanged as

Phase 2: Possible Component Lists

Location

List

ΔΤ

FM XXXX

BEFORE

RAILROAD

CROSSING

NEXT

MILES

PAST

IIS XXX

EXIT

XXXXXXX

TO

XXXXXXX

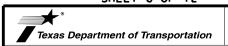
IIS XXX

TΩ

FM XXXX

- EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can
- 4. Highway names and numbers replaced as appropriate.
- 5. ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- AHEAD may be used instead of distances if necessary. 7. FI and MI. MILE and MILES interchanged as appropriate.
- 8. AT. BEFORE and PAST interchanged as needed.
- location phase is used.

SHEET 6 OF 12



Traffic Safety Division Standard

* * Advance

Notice List

TUE-FRI

XX AM-

X PM

APR XX-

X PM-X AM

BEGINS

MONDAY

BEGINS

ΜΔΥ ΧΧ

MAY X-X

XX PM -

XX AM

NFXT

FRI-SUN

XX AM

XX PM

NEXT

TUE

AUG XX

TONIGHT

XX PM-

XX AM

Warning

List

SPEED

LIMIT

XX MPH

MAXIMUM

SPEED

XX MPH

MINIMUM

SPEED

XX MPH

ADVISORY

SPEED

XX MPH

RIGHT

IANF

EXIT

USF

CAUTION

DRIVE

SAFELY

DRIVE

WITH

CARE

* * See Application Guidelines Note 6.

BARRICADE AND CONSTRUCTION PORTABLE CHANGEABLE MESSAGE SIGN (PCMS)

BC(6)-21

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Type C Warning Light or

Warning reflector may be round

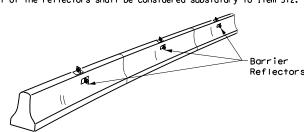
or square. Must have a yellow

reflective surface area of at least

30 square inches

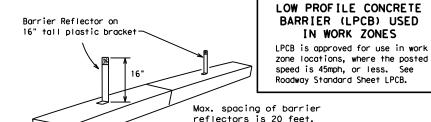
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- Barrier Reflectors shall be pre-qualified, and conform to the color and reflectivity requirements of DMS-8600. A list of pregualified Barrier Reflectors can be found at the Material Producer List web address shown on BC(1).
- 2. Color of Barrier Reflectors shall be as specified in the TMUTCD. The cost of the reflectors shall be considered subsidiary to Item 512.



CONCRETE TRAFFIC BARRIER (CTB)

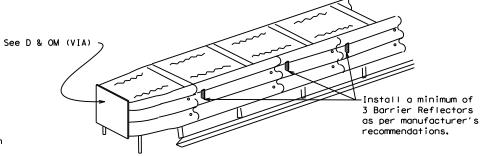
- 3. Where traffic is on one side of the CTB, two (2) Barrier Reflectors shall be mounted in approximately the midsection of each section of CTB. An alternate mounting location is uniformly spaced at one end of each CTB. This will allow for attachment of a barrier grapple without damaging the reflector. The Barrier Reflector mounted on the side of the CTB shall be located directly below the reflector mounted on top of the barrier, as shown in the detail above.
- Where CTB separates two-way traffic, three barrier reflectors shall be mounted on each section of CTB. The reflector unit on top shall have two yellow reflective faces (Bi-Directional) while the reflectors on each side of the barrier shall have one yellow reflective face, as shown in the detail above.
- 5. When CTB separates traffic traveling in the same direction, no barrier reflectors will be required on top of the CTB.
- 6. Barrier Reflector units shall be yellow or white in color to match the edgeline being supplemented.
- 7. Maximum spacing of Barrier Reflectors is forty (40) feet.
- 8. Pavement markers or temporary flexible-reflective roadway marker tabs shall NOT be used as CTB delineation.
- 9. Attachment of Barrier Reflectors to CTB shall be per manufacturer's
- 10. Missing or damaged Barrier Reflectors shall be replaced as directed by the Engineer
- 11. Single slope barriers shall be delineated as shown on the above detail.



manufacturer's recommendations.

Attach the delineators as per

IN WORK ZONES



LOW PROFILE CONCRETE BARRIER (LPCB)

DELINEATION OF END TREATMENTS

END TREATMENTS FOR CTB'S USED IN WORK ZONES

End treatments used on CTB's in work zones shall meet the apppropriate crashworthy standards as defined in the Manual for Assessing Safety Hardware (MASH), Refer to the CWZTCD List for approved end treatments and manufacturers.

BARRIER REFLECTORS FOR CONCRETE TRAFFIC BARRIER AND ATTENUATORS

WARNING LIGHTS

- 1. Warning lights shall meet the requirements of the TMUTCD.
- 2. Warning lights shall NOT be installed on barricades.
- 3. Type A-Low Intensity Flashing Warning Lights are commonly used with drums. They are intended to warn of or mark a potentially hazardous area. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "FL". The Type A Warning Lights shall not be used with signs manufactured with Type B_{FL} or C_{FL} Sheeting meeting the requirements of Departmental Material Specification DMS-8300.
- 4. Type-C and Type D 360 degree Steady Burn Lights are intended to be used in a series for delineation to supplement other traffic control devices. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "SB".
- 5. The Engineer/Inspector or the plans shall specify the location and type of warning lights to be installed on the traffic control devices.
- 6. When required by the Engineer, the Contractor shall furnish a copy of the warning lights certification. The warning light manufacturer will certify the warning lights meet the requirements of the latest ITE Purchase Specifications for Flashing and Steady-Burn Warning Lights.
- 7. When used to delineate curves, Type-C and Type D Steady Burn Lights should only be placed on the outside of the curve, not the inside.
- 8. The location of warning lights and warning reflectors on drums shall be as shown elsewhere in the plans.

WARNING LIGHTS MOUNTED ON PLASTIC DRUMS

- 1. Type A flashing warning lights are intended to warn drivers that they are approaching or are in a potentially hazardous area.
- 2. Type A random flashing warning lights are not intended for delineation and shall not be used in a series.
- 3. A series of sequential flashing warning lights placed on channelizing devices to form a merging taper may be used for delineation. If used, the successive flashing of the sequential warning lights should occur from the beginning of the taper to the end of the merging taper in order to identify the desired vehicle path. The rate of flashing for each light shall be 65 flashes per minute, plus or minus 10 flashes.
- 4. Type C and D steady-burn warning lights are intended to be used in a series to delineate the edge of the travel lane on detours, on lane changes, on lane closures, and on other similar conditions.
- 5. Type A, Type C and Type D warning lights shall be installed at locations as detailed on other sheets in the plans.
- 6. Warning lights shall not be installed on a drum that has a sign, chevron or vertical panel.
- 7. The maximum spacing for warning lights on drums should be identical to the channelizing device spacing.

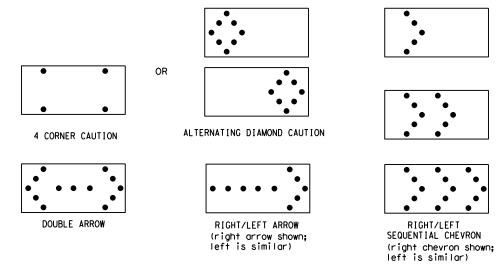
WARNING REFLECTORS MOUNTED ON PLASTIC DRUMS AS A SUBSTITUTE FOR TYPE C (STEADY BURN) WARNING LIGHTS

- 1. A warning reflector or approved substitute may be mounted on a plastic drum as a substitute for a Type C, steady burn warning light at the discretion of the Contractor unless otherwise noted in the plans.
- 2. The warning reflector shall be yellow in color and shall be manufactured using a sign substrate approved for use with plastic drums listed
- 3. The warning reflector shall have a minimum retroreflective surface area (one-side) of 30 square inches.
- 4. Round reflectors shall be fully reflectorized, including the area where attached to the drum.
- 5. Square substrates must have a minimum of 30 square inches of reflectorized sheeting. They do not have to be reflectorized where it attaches to the drum.
- 6. The side of the warning reflector facing approaching traffic shall have sheeting meeting the color and retroreflectivity requirements for DMS 8300-Type B or Type C.
- 7. When used near two-way traffic, both sides of the warning reflector shall be reflectorized.
- 8. The warning reflector should be mounted on the side of the handle nearest approaching traffic.
- 9. The maximum spacing for warning reflectors should be identical to the channelizing device spacing requirements.

Arrow Boards may be located behind channelizing devices in place for a shoulder taper or merging taper, otherwise they shall be delineated with four (4) channelizing devices placed perpendicular to traffic on the upstream side of traffic.

- 1. The Flashing Arrow Board should be used for all lane closures on multi-lane roadways, or slow moving maintenance or construction activities on the travel lanes.

 2. Flashing Arrow Boards should not be used on two-lane, two-way roadways, detours, diversions
- or work on shoulders unless the "CAUTION" display (see detail below) is used.
- The Engineer/Inspector shall choose all appropriate signs, barricades and/or other traffic control devices that should be used in conjunction with the Flashing Arrow Board.
- 4. The Flashing Arrow Board should be able to display the following symbols:



- 5. The "CAUTION" display consists of four corner lamps flashing simultaneously, or the Alternating Diamond Caution mode as shown.
- The straight line caution display is NOT ALLOWED.
- The Flashing Arrow Board shall be capable of minimum 50 percent dimming from rated lamp voltage.
 The flashing rate of the lamps shall not be less than 25 nor more than 40 flashes per minute.
 Minimum lamp "on time" shall be approximately 50 percent for the flashing arrow and equal

- intervals of 25 percent for each sequential phase of the flashing chevron.

 9. The sequential arrow display is NOT ALLOWED.

 10. The flashing arrow display is the TxDOT standard; however, the sequential chevron display may be used during daylight operations.
- 11. The Flashing Arrow Board shall be mounted on a vehicle, trailer or other suitable support.
 12. A Flashing Arrow Board SHALL NOT BE USED to laterally shift traffic.
 13. A full matrix PCMS may be used to simulate a Flashing Arrow Board provided it meets visibility,
- flash rate and dimming requirements on this sheet for the same size arrow. 14. Minimum mounting height of trailer mounted Arrow Boards should be 7 feet from roadway
- to bottom of panel.

	REQUIREMENTS							
TYPE	MINIMUM SIZE	MINIMUM NUMBER OF PANEL LAMPS	MINIMUM VISIBILITY DISTANCE					
В	30 × 60	13	3/4 mile					
С	48 × 96	15	1 mile					

ATTENTION Flashing Arrow Boards shall be equipped with automatic dimming devices.

WHEN NOT IN USE, REMOVE THE ARROW BOARD FROM THE RIGHT-OF-WAY OR PLACE THE ARROW BOARD BEHIND CONCRETE
TRAFFIC BARRIER OR GUARDRAIL.

FLASHING ARROW BOARDS

SHEET 7 OF 12

TRUCK-MOUNTED ATTENUATORS

- Truck-mounted attenuators (TMA) used on TxDOT facilities must meet the requirements outlined in the Manual for Assessing Safety Hardware (MASH).
- Refer to the CWZTCD for the requirements of Level 2 or Level 3 TMAs.
- 3. Refer to the CWZTCD for a list of approved TMAs.
- 4. TMAs are required on freeways unless otherwise noted in the plans.
- 5. A TMA should be used anytime that it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the work performance.
- The only reason a TMA should not be required is when a work area is spread down the roadway and the work crew is an extended distance from the TMA.



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION ARROW PANEL. REFLECTORS. WARNING LIGHTS & ATTENUATOR

BC(7)-21

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GENERAL NOTES 1. For long term stationary work zones on freeways, drums shall be used as

the primary channelizing device. 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections,

one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the

- cones in proper position and location. 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 5. Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- 6. The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

Pre-qualified plastic drums shall meet the following requirements:

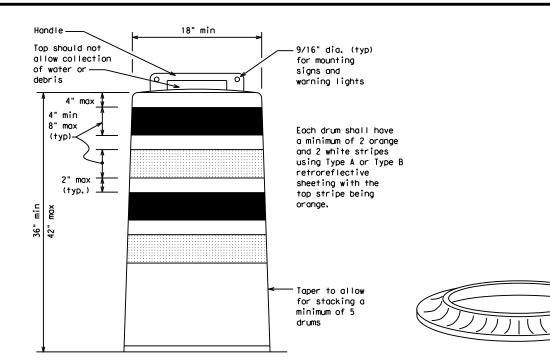
- 1. Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- 3. Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

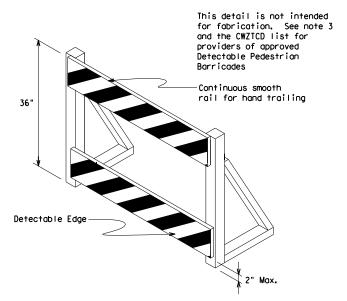
RETROREFLECTIVE SHEETING

- 1. The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting

BALLAST

- 1. Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- 2. Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- 5. When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.





DETECTABLE PEDESTRIAN BARRICADES

- 1. When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
- 2. Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- 3. Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian
- 5. Warning lights shall not be attached to detectable pedestrian barricades.
- 6. Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.



18" x 24" Sign (Maximum Sign Dimension) Chevron CW1-8, Opposing Traffic Lane Divider, Driveway sign D70a, Keep Right R4 series or other signs as approved by Engineer

See Ballast



12" x 24" Vertical Panel mount with diagonals sloping down towards travel way

Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums

SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS

- 1. Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD.
- 2. Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL} Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
- 3. Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
- 4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
- 5. Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each
- 6. Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2
- 7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum, A minimum of three (3) should be used at each location called for in the plans.
- 8. R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.

SHEET 8 OF 12

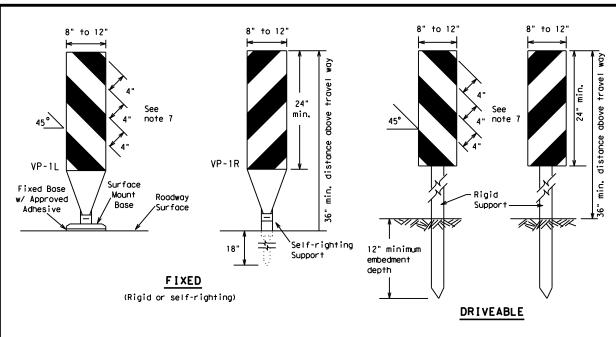


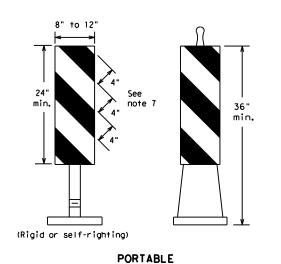
Traffic Safety

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC(8)-21

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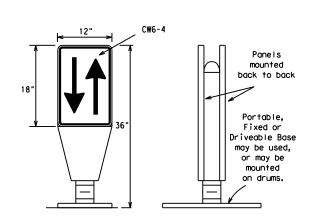




- 1. Vertical Panels (VP's) are normally used to channelize traffic or divide opposing lanes of traffic.
- 2. VP's may be used in daytime or nighttime situations. They may be used at the edge of shoulder drop-offs and other areas such as lane transitions where positive daytime and nighttime delineation is required. The Engineer/Inspector shall refer to the Roadway Design Manual for additional requirements on the use VP's for drop-offs.
- 3. VP's should be mounted back to back if used at the edge of cuts adjacent to two-way two lane roadways. Stripes are to be reflective orange and reflective white and should always slope downward toward the travel lane.
- 4. VP's used on expressways and freeways or other high speed roadways, may have more than 270 square inches of retroreflective area facing traffic.

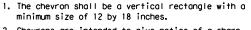
 5. Self-righting supports are available with portable base.
- See "Compliant Work Zone Traffic Control Devices List"
- 6. Sheeting for the VP's shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300, unless noted otherwise,
- 7. Where the height of reflective material on the vertical panel is 36 inches or greater, a panel stripe of 6 inches shall be used.

VERTICAL PANELS (VPs)



- 1. Opposing Traffic Lane Dividers (OTLD) are delineation devices designed to convert a normal one-way roadway section to two-way operation. OTLD's are used on temporary centerlines. The upward and downward arrows on the sign's face indicate the direction of traffic on either side of the divider. The base is secured to the pavement with an adhesive or rubber weight to minimize movement caused by a vehicle impact or wind gust.
- 2. The OTLD may be used in combination with 42"
- 3. Spacing between the OTLD shall not exceed 500 feet. 42" cones or VPs placed between the OTLD's should not exceed 100 foot spacing.
- 4. The OTLD shall be orange with a black nonreflective legend. Sheeting for the OTLD shall be retroreflective Type B_{FL} or Type C_{FL} conforming to Departmental Material Specification DMS-8300. unless noted otherwise. The legend shall meet the requirements of DMS-8300.

OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

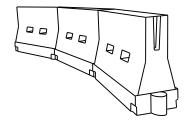


- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the out side of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type B_{FL} or Type C_{FL} conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- 6. For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS

GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.



LONGITUDINAL CHANNELIZING DEVICES (LCD)

36'

Fixed Base w/ Approved Adhesive

(Driveable Base, or Flexible

Support can be used)

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- 2. LCDs may be used instead of a line of cones or drums.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- 2. Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings.
- 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

Posted Speed	Formula	D	esirab er Len **	le	Suggested Maximum Spacing of Channelizing Devices		
		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	
30	2	150′	165′	1801	30'	60′	
35	L= WS ²	2051	2251	2451	35′	70′	
40	80	265′	295′	3201	40′	80′	
45		450′	495′	540′	45′	90′	
50		500′	550′	6001	50°	100′	
55	L=WS	550′	6051	660′	55 <i>°</i>	110′	
60	L - 11 3	600'	660′	720′	60′	120′	
65		650′	715′	7801	65′	130′	
70		700′	770′	840′	70′	140'	
75		750′	825′	900′	75′	150′	
80		800′	880′	960′	80′	160′	

XX Taper lengths have been rounded off. L=Length of Taper (FT.) W=Width of Offset (FT.) S=Posted Speed (MPH)

SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12



Traffic Safety Division Standard

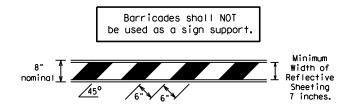
BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC (9) -21

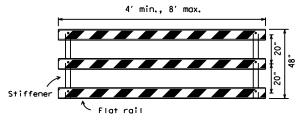
ILE:	bc-21.dgn	DN: T	×DOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
C) T×DOT	November 2002	CONT	SECT	JOB		HIC	GHWAY
9-07 7-13	REVISIONS 8-14 5-21	0908	27	006		CR	316
		DIST		COUNTY		SHEET NO.	
		ABL		KENT			22

TYPE 3 BARRICADES

- 1. Refer to the Compliant Work Zone Traffic Control Devices List (CWZTCD) for details of the Type 3 Barricades and a list of all materials used in the construction of Type 3 Barricades.
- 2. Type 3 Barricades shall be used at each end of construction projects closed to all traffic.
- 3. Barricades extending across a roadway should have stripes that slope downward in the direction toward which traffic must turn in detouring When both right and left turns are provided, the chevron striping may slope downward in both directions from the center of the barricade. Where no turns are provided at a closed road, striping should slope downward in both directions toward the center of roadway.
- Striping of rails, for the right side of the roadway, should slope downward to the left. For the left side of the roadway, striping should slope downward to the right.
- Identification markings may be shown only on the back of the barricade rails. The maximum height of letters and/or company logos used for identification shall be 1".
- 6. Barricades shall not be placed parallel to traffic unless an adequate clear zone is provided.
- Warning lights shall NOT be installed on barricades.
- Where barricades require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand is recommended. The $\,$ sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight. Sand bags shall not be stacked in a manner that covers any portion of a barricade rails reflective sheeting. Rock, concrete, iron, steel or other solid objects will NOT be permitted. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs. Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall not be used for sandbags. Sandbags shall only be placed along or upon the base supports of the device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners.
- Sheeting for barricades shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300 unless otherwise noted.

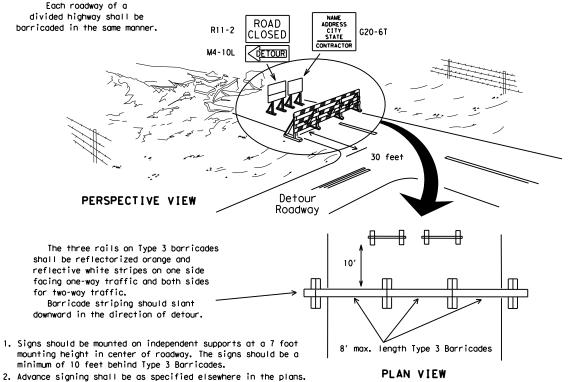


TYPICAL STRIPING DETAIL FOR BARRICADE RAIL



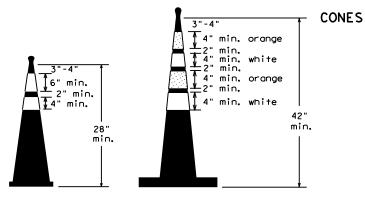
Stiffener may be inside or outside of support, but no more than 2 stiffeners shall be allowed on one barricade.

TYPICAL PANEL DETAIL

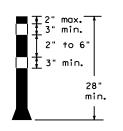


TYPE 3 BARRICADE (POST AND SKID) TYPICAL APPLICATION

1. Where positive redirectional capability is provided, drums may be omitted. 2. Plastic construction fencing may be used with drums for safety as required in the plans. 3. Vertical Panels on flexible support may be substituted for drums when the Typical shoulder width is less than 4 feet. Plastic Drum 4. When the shoulder width is greater than 12 feet. steady-burn lights PERSPECTIVE VIEW may be omitted if drums are used. 5. Drums must extend the length These drums are not required of the culvert widening. on one-way roadway LEGEND Plastic drum Plastic drum with steady burn light um of two drums s locross the work or yellow warning reflector Steady burn warning light or yellow warning reflector Increase number of plastic drums on the side of approaching traffic if the crown width makes it necessary. (minimum of 2 and maximum of 4 drums) PLAN VIEW CULVERT WIDENING OR OTHER ISOLATED WORK WITHIN THE PROJECT LIMITS



Two-Piece cones



One-Piece cones

2" min.

Tubular Marker

FOR SKID OR POST TYPE BARRICADES

Alternate Alternate Drums, vertical panels or 42" cones Approx. Approx. 50' at 50' maximum spacing 50' Min. 2 drums or 1 Type 3 or 1 Type 3 barricade STOCKPILE On one-way roads Desirable downstream drums stockpile location Channelizing devices parallel to traffic or barricade may be is outside should be used when stockpile is omitted here clear zone. within 30' from travel lane. \Diamond ➾

TRAFFIC CONTROL FOR MATERIAL STOCKPILES

28" Cones shall have a minimum weight of 9 1/2 lbs.

42" 2-piece cones shall have a minimum weight of 30 lbs. including base.

- 1. Traffic cones and tubular markers shall be predominantly orange, and meet the height and weight requirements shown above.
- 2. One-piece cones have the body and base of the cone molded in one consolidated unit. Two-piece cones have a cone shaped body and a separate rubber base, or ballast, that is added to keep the device upright and in place.
- 3. Two-piece cones may have a handle or loop extending up to 8" above the minimum height shown, in order to aid in retrieving the device.
- 4. Cones or tubular markers shall have white or white and orange reflective bands as shown above. The reflective bands shall have a smooth, sealed outer surface and meet the requirements of Departmental Material Specification DMS-8300 Type A or Type B.
- 5. 28" cones and tubular markers are generally suitable for short duration and short-term stationary work as defined on BC(4). These should not be used for intermediate-term or long-term stationary work unless personnel is on-site to maintain them in their proper upright position.
- 6. 42" two-piece cones, vertical panels or drums are suitable for all work zone durations.
- 7. Cones or tubular markers used on each project should be of the same size and shape.

SHEET 10 OF 12



BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

Traffic Safety Division Standard

BC(10)-21

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TxDOT	November 2002	CONT	SECT	JOB		HIC	YAWH	
	REVISIONS	0908	27	006		CR	316	
	8-14	DIST	DIST COUNTY				SHEET NO.	
	5-21	ABL		KENT			23	

Kind is a Kind is a Color of this solution is an analysis solution.

w://txdot.projectwiseonline.com:TxDOT2/

WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Additional supplemental pavement marking details may be found in the plans or specifications.
- Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans,
- When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- 1. Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

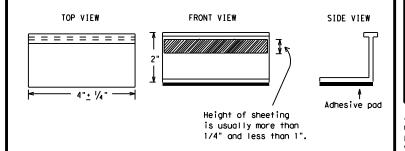
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- 4. Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10. Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECURE TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARKER TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guidemarks shall meet the requirements of DMS-8242.
- Tabs detailed on this sheet are to be inspected and accepted by the Engineer or designated representative. Sampling and testing is not normally required, however at the option of the Engineer, either "A" or "B" below may be imposed to assure quality before placement on the roadway.
 - A. Select five (5) or more tabs at random from each lot or shipment and submit to the Construction Division, Materials and Pavement Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix five (5) tabs at 24 inch intervals on an asphaltic pavement in a straight line. Using a medium size passenger vehicle or pickup, run over the markers with the front and rear tires at a speed of 35 to 40 miles per hour, four (4) times in each direction. No more than one (1) out of the five (5) reflective surfaces shall be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. See Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARKS

- Raised pavement markers used as guidemarks shall be from the approved product list, and meet the requirements of DMS-4200.
- All temporary construction raised pavement markers provided on a project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applied or butyl rubber pad for all surfaces, or thermoplastic for concrete surfaces.

Guidemarks shall be designated as:
YELLOW - (two amber reflective surfaces with yellow body).
WHITE - (one silver reflective surface with white body).

DEPARTMENTAL MATERIAL SPECIFICATIO	NS
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
TRAFFIC BUTTONS	DMS-4300
EPOXY AND ADHESIVES	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS	DMS-8241
TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242

A list of prequalified reflective raised pavement markers, non-reflective traffic buttons, roadway marker tabs and other pavement markings can be found at the Material Producer List web address shown on BC(1).

SHEET 11 OF 12



Traffic Safety Division Standard

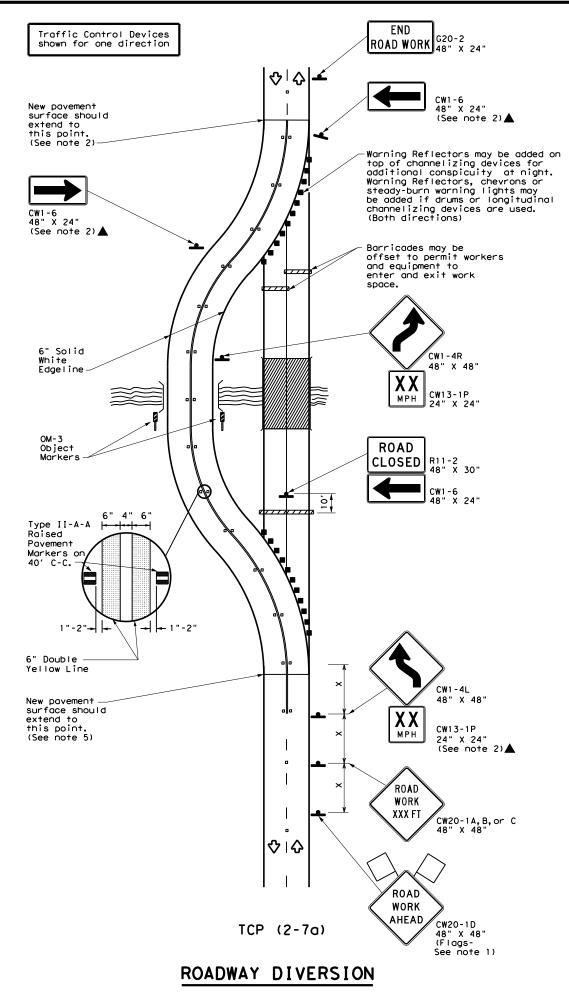
BARRICADE AND CONSTRUCTION PAVEMENT MARKINGS

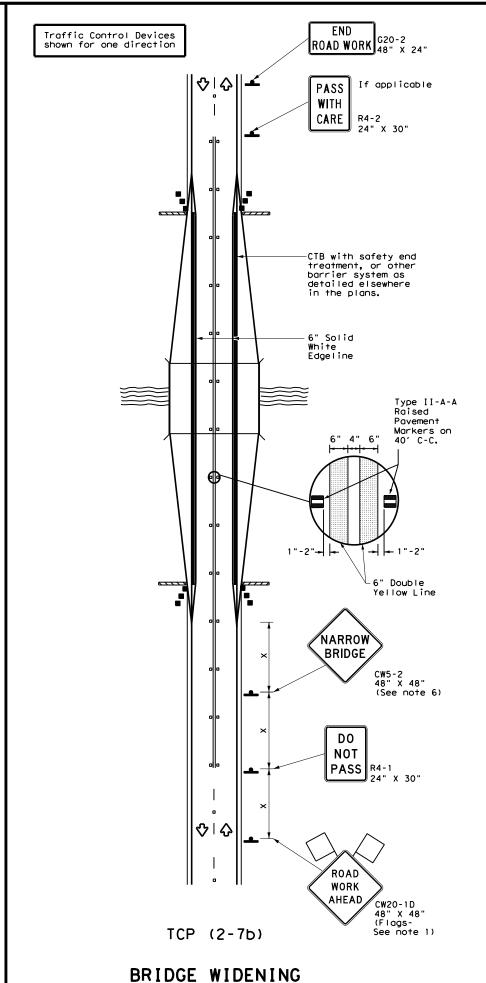
BC(11)-21

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TxDOT February 1998	CONT SECT JOB			HIGHWAY		
REVISIONS 98 9-07 5-21	0908	27	006		CR	316
·98 9-07 5-21 ·02 7-13	DIST		COUNTY			SHEET NO.
02 8-14	ABL		KENT			24

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STANDARD WORK ZONE PAVEMENT MARKINGS DETAILS Type Y buttons Type II-A-A 000/100// DOUBLE PAVEMENT NO-PASSING REFLECTOR 17FD PAVEMENT LINE Type I-C, I-A or II-A-A Type W or Y buttons RAISED EDGE LINE SOL I D PAVEMENT OR SINGLE LINES 60" REFLECTORIZED NO-PASSING LINE PAVEMENT White or Yellow Type I-C Type W buttons WIDE RAISED PAVEMENT LINE REFLECTORIZED (FOR LEFT TURN CHANNELIZING LINE OR CHANNELIZING LINE USED TO MARKINGS DISCOURAGE LANE CHANGING,) White 30"<u>+</u> 3' 30"+/-3" Type I-C or II-A-A 0 Q 0 9 0 RAISED **CENTER** PAVEMENT | 5' | 5' | MARKERS √Type W or Y buttons LINE OR LANE REFLECTORIZED LINE MARKINGS White or Yellow Type I-C or II-A-A **BROKEN** (when required) LINES RAISED п _ ‡8 п П 1-2" _ MARKERS **AUXILIARY** Type I-C or II-C-OR LANEDROP REFLECTORIZED LINE PAVEMENT REMOVABLE MARKINGS 5′ <u>+</u> 6" WITH RAISED **PAVEMENT MARKERS** If raised pavement markers are used Raised Pavement Markers to supplement REMOVABLE markings, the markers shall be applied to the top of the tape at the approximate mid length of tape used for broken lines or at 20 foot spacing for solid lines. This allows an easier 20' ± 1' removal of raised pavement markers Centerline only - not to be used on edge lines **SHEET 12 OF 12** Traffic Safety Division Standard Texas Department of Transportation BARRICADE AND CONSTRUCTION PAVEMENT MARKING PATTERNS Raised payement markers used as standard pavement markings shall be from the approved products list and meet the requirements of Item 672 "RAISED PAVEMENT MARKERS." BC(12)-21 DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO ©⊺xDOT February 1998 JOB HIGHWAY CR 316 0908 27 006 1-97 9-07 5-21 2-98 7-13 11-02 8-14 25





	LEGEND						
~~~	Type 3 Barricade		Channelizing Devices				
	Heavy Work Vehicle		Truck Mounted Attenuator (TMA)				
	Trailer Mounted Flashing Arrow Board	••••	Raised Pavement Markers Ty II-AA				
<b>-</b>	Sign	♡	Traffic Flow				
$\Diamond$	Flag	ПО	Flagger				

Posted Speed	Formula	Taper Lengths Channelizing Sy Devices		Desirable Spacing of Channelizing XX Devices		Minimum Sign Spacing "x"	Suggested Longitudinal Buffer Space	
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"
30	WS ²	150′	1651	180′	30'	60′	120′	90′
35	L = WS	2051	225′	245'	35′	70′	160′	120′
40	80	265′	295′	3201	40′	80′	240'	155′
45		450′	4951	540'	45′	90′	320′	195′
50		500′	550′	6001	50′	100′	400′	240′
55	L=WS	550′	6051	660′	55′	110′	500′	295′
60	- 113	600'	660′	720′	60′	120'	600′	350′
65		650′	715′	780′	65′	130′	700′	410′
70		700′	770′	840′	70′	140′	800′	475′
75		750′	825′	900'	75′	150′	900'	540′

* Conventional Roads Only

** Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

		TYPICAL L	ISAGE	
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY
			✓	✓

#### GENERAL NOTES

- 1. Flags attached to signs where shown are REQUIRED.
- All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved by the Engineer.

#### TCP (2-7a)

- Raised pavement markers shall be placed 40 feet c-c on centerline throughout project.
- Roadway diversion design requirements should be based on posted speed limit or prevailing speed.
- New pavement surface should be extended across existing roadway edge to a point where existing pavement markings left in place during project do not conflict with construction area pavement marking.

#### TCP (2-7b)

The CW5-2 "Narrow Bridge" sign may be omitted if lane and shoulder widths are maintained.



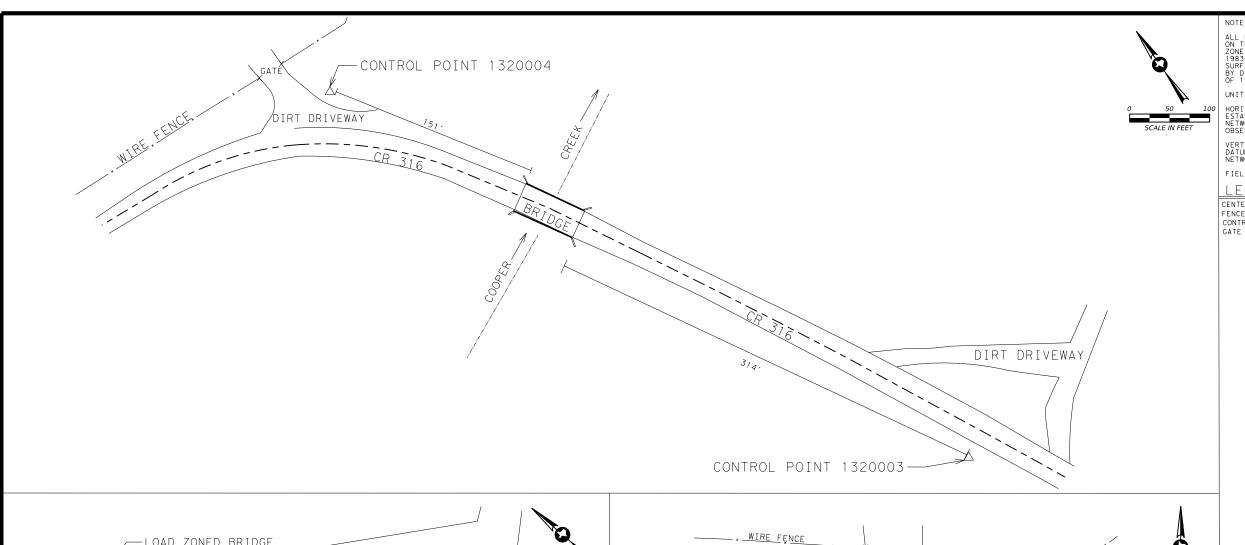
Traffic Safety Division Standard

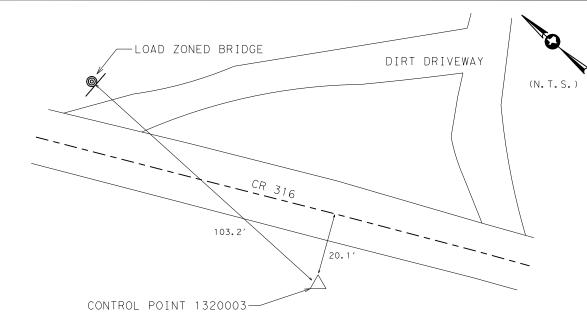
TRAFFIC CONTROL PLAN DIVERSIONS AND NARROW BRIDGES

TCP (2-7) -23

FILE: +cp2-7-23.dgn	DN:		CK:	DW:		CK:
© TxDOT April 2023	CONT	SECT	JOB	JOB		GHWAY
12-85 4-98 2-18	0908	27	006		CR	316
8-95 3-03 4-23	DIST		COUNTY			SHEET NO.
1-97 2-12	ABL		KENT	•		26

167





# CONTROL POINT: 1320003

CP# 1320003 IS A 5/8"REBAR WITH A TXDOT ALUMINUM CAP IN CONCRETE. LOCATED ON THE WEST SIDE OF CR 316 AND BEING ± 314' FEET FROM END OF BRIDGE OVER COOPER CREEK.

GRID COORDINATES NORTHING 7,087,987.47 NORTHING

ELEVATION 2,041.49

1,246,886.61 EASTING

SURFACE COORDINATES 7,089,475.95 1,247,148.46

LATITUDE

LONGITUDE 33° 05′ 27. 9879" 100° 51′ 23. 3512"

# CONTROL POINT: 1320004

CP# 1320004 IS A 5/8"REBAR WITH A TXDOT ALUMINUM CAP IN CONCRETE. LOCATED NORTH EAST OF CR 316 AND BEING ± 151' FROM END OF BRIDGE OVER COOPER CREEK.

GATE

DIRT DRIVEWAY

38.7

37.4

SURFACE COORDINATES GRID COORDINATES 7,088,450.99 NORTHING NORTHING EASTING 1,246,671.66 EASTING ELEVATION 2,033.72

1,246,933.46

LATITUDE LONGITUDE 7,089,939.56 33°05′32.5256" 100°51′25.9996"

ALL BEARINGS AND COORDINATES SHOWN ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRA ZONE (4202)NORTH AMERICAN DATUM OF 1983 (NAD 1983) 2011, ALL DISTANCES AND COORDINATES AR SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY A COMBINED ADJUSTMENT FACTOR OF 1.00021.

UNIT OF MEASURE IS U.S. SURVEY FEET

HORIZONTAL CONTROL OF THIS PROJECT WAS ESTABLISHED BY TXDOT VIRTUAL REFERENCE SYSTEM NETWORK BASED ON AVERAGED THREE 180 EPOCH OBSERVATIONS

VERTICAL CONTROL IS NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88); GEOID 12B; TXDOT VRS NETWORK BASED ON THREE 180 EPOCH OBSERVATION FIELD SURVEYS WERE PERFORMED IN NOVEMBER 202

 $\stackrel{\textstyle \wedge}{\searrow}$ 

CENTER LINE FENCE CONTROL POINT



(N.T.S.)

-CONTROL POINT 1320004

## 9/14/2023

© 2023 ® TEXAS DEPARTMENT OF TRANSPORTATION
ABILENE DISTRICT
4250 North Clack Street
Abilene, Texas 79601
PHONE: (325) 676 - 6800

#### CR 316 COOPER CREEK SURVEY CONTROL DATA

FED. RD. DIV. NO.	STATE	F	EDERAL	_ AID F	PROJECT NO.	HIGHWAY NO.
6	TEXAS		SEE	CR 316		
DISTRICT NO	COUNT	Y CONTROL SECTION JOB NO. NO.				SHEET NO.
8	KENT		0908	27	006	27

Chord Direction:

Radial Direction:

Tangent Ahead Direction:

HORIZONTAL ALIGNMENT REPORT Alignment name: CR 316 ALIGN Alignment description: CR 316 Horizontal Alignment Report Created: Friday, June 2, 2023 Time: 05:28:11 PM STATION X Y PC 99+45.750 R1 1246843.935 7089942.997 PΙ 100+28.684 R1 1246923.156 7089918.458 CC 1246774.392 7089718.490 PCC 101+05.207 R1 1246969.432 7089849.635 Radius: 235.031 38°52'20.439" Right Delta: 24°22'40.805" Degree of Curvature (Arc): 159.457 Length: 82.934 Tangent: 156.416 Chord: Middle Ordinate: 13.394 External: 14.203 Tangent Back Direction: S72°47'21.147"E Radial Direction: S17°12'38.853"W Chord Direction: S53°21'10.928"E Radial Direction: S56°04'59.292"W Tangent Ahead Direction: S33°55'00.708"E PCCBL CL-4 101+05.207 R1 1246969.432 7089849.635 PΙ 101+26.501 R1 1246981.333 7089831.977 CC 1246357.303 7089437.086 PTBL CL-101+47.783 R1 1246992.196 7089813.662 Radius: 738.172 03°18'17.045" Right Delta: 07°45'42.644" Degree of Curvature (Arc): 42.577 Length: 21.294 Tangent: 42.571 Chord: Middle Ordinate: 0.307 0.307 External: Tangent Back Direction: S33°58'41.954"E Radial Direction: S56°01'18.046"W Chord Direction: S32°19'33.431"E Radial Direction: S59°19'35.092"W Tangent Ahead Direction: S30°40'24.908"E PT101+47.783 R1 1246992.196 7089813.662 102+57.325 R1 1247046.196 7089718.355 PC Tangential Direction: S29°32'08.473"E Tangential Length: 109.542 102+57.325 R1 PC 1247046.196 7089718.355 ΡI 103+60.720 R1 1247096.304 7089627.914 CC 1243590.655 7087803.850 PT104+64.067 R1 1247141.613 7089534.975 3950.455 Radius: 02°59'54.628" Right Delta: Degree of Curvature (Arc): 01°27'01.292" Length: 206.742 Tangent: 103.395 Chord: 206.719 Middle Ordinate: 1.352 1.353 External: Tangent Back Direction: S28°59'17.417"E Radial Direction: S61°00'42.583"W

S27°29'20.104"E

S64°00'37.210"W

S25°59'22.790"E



# CR 316 ALIGNMENT DATA



SCALE:	N/A		SI	HEET	1	OF 2
FHWA DIVISION	PF	ROJECT NO.	•	ΗI	GHW	AY NO.
6	SEE	TITLE SH	IEET		CR	316
STATE		COUNT	Y		SH	IEET NO.
TEXAS		KENT				
DISTRICT	CONTROL	SECTION	JOI	3		29
ABL	0908	27	000	6		

VERTICAL ALIGNMENT REPORT Alignment name: CR 316 PROFILE

Alignment description: Report Created: Friday, July 21, 2023

VPT

101+53.808 R1

2038.515

Time: 3:05:57 PM

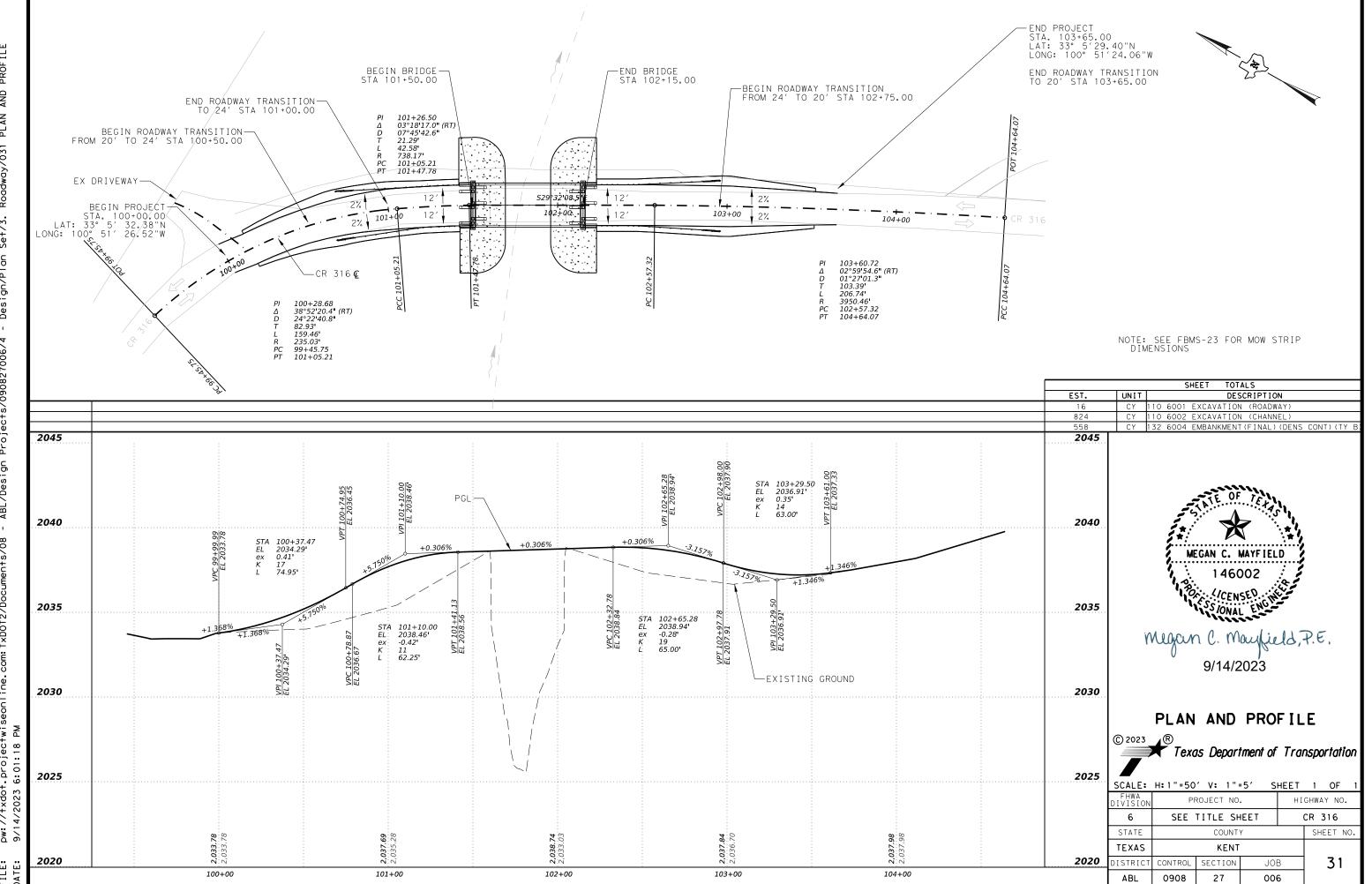
	STATION	ELEVATION		STATION	ELEVATION
POT	99+45.750 R1	2033.728	Length:	74.296	
VPI	99+57.140 R1	2033.494	Entrance Grade:	0.052	
Tangent Grade:	-0.021		Exit Grade:	0.004	
Tangent Length:	11.390		K Value =:	15.543	
			Middle Ordinate (E):	-0.444	
VPI	99+57.140 R1	2033.494			
VPI	99+60.130 R1	2033.422	VPT	101+53.808 R1	2038.515
Tangent Grade:	-0.024		VPC	102+21.399 R1	2038.770
Tangent Length:	2.990		Tangent Grade:	0.004	
			Tangent Length:	67.591	
VPI	99+60.130 R1	2033.422			
VPI	99+68.530 R1	2033.442	VPC	102+21.399 R1	2038.770
Tangent Grade:	0.002		VPI	102+55.280 R1	2038.898
Tangent Length:	8.399		VPT	102+89.161 R1	2037.990
			VHP	102+29.767 R1	2038.786
VPI	99+68.530 R1	2033.442	Length:	67.761	
VPI	99+79.919 R1	2033.447	Entrance Grade:	0.004	
Tangent Grade:	0.000		Exit Grade:	-0.027	
Tangent Length:	11.390		K Value =:	22.158	
			Middle Ordinate (E):	-0.259	
VPI	99+79.919 R1	2033.447			
VPI	99+88.905 R1	2033.435	VPT	102+89.161 R1	2037.990
Tangent Grade:	-0.001		VPC	102+94.108 R1	2037.858
Tangent Length:	8.986		Tangent Grade:	-0.027	
			Tangent Length:	4.948	
VPI	99+88.905 R1	2033.435			
VPI	99+91.309 R1	2033.754	VPC	102+94.108 R1	2037.858
Tangent Grade:	0.034		VPI	103+29.500 R1	2036.909
Tangent Length:	9.335		VPT	103+64.892 R1	2037.385
			VLP	103+41.229 R1	2037.226
VPI	99+98.240 R1	2033.754	Length:	70.783	
VPC	100+01.461 R1	2033.798	Entrance Grade:	-0.027	
Tangent Grade:	0.014		Exit Grade:	0.013	
Tangent Length:	3.221		K Value =:	17.580	
			Middle Ordinate (E):	0.356	
VPC	100+01.461 R1	2033.798			
VPI	100+37.470 R1	2034.290	VPT	103+64.892 R1	2037.385
VPT	100+73.479 R1	2036.148	VPI	103+73.140 R1	2037.496
			Tangent Grade:	0.013	
Length:	72.018		Tangent Length:	8.248	
Entrance Grade:	0.014				
Exit Grade:	0.052		VPI	103+73.140 R1	2037.496
K Value =:	19.003		VPI	104+11.170 R1	2038.177
ddle Ordinate (E):	0.341		Tangent Grade:	0.018	
			Tangent Length:	38.030	
VPT	100+73.479 R1	2036.148			
VPC	100+79.512 R1	2036.459	VPI	104+11.170 R1	2038.177
Tangent Grade:	0.052		POT	104+64.067 R1	2039.771
Tangent Length:	6.033		Tangent Grade:	0.030	
	100.50 510 51	0006 175	Tangent Length:	52.897	
VPC	100+79.512 R1	2036.459			
VPI	101+16.660 R1	2038.375			
מתעז	101152 000 D1	2020 515			



# CR 316 ALIGNMENT DATA



SCALE:	N/A		SI	HEET	2	OF	2
FHWA DIVISION	PI	ROJECT NO	•	ΗI	GHWA	AY NO.	•
6	SEE	TITLE SH	IEET		CR	316	
STATE		COUNTY				EET N	١٥.
TEXAS		KENT					
DISTRICT	CONTROL	SECTION	JOI	В		30	
ABL	0908	27	00	6			



BUTTON HEAD BOLT

FBBO4 = 18'

NOTE: SEE GENERAL NOTE 3 FOR SPLICE & POST BOLT DETAILS.

NOTE: GF(31), MID-SPAN RAIL SPLICES ARE

MID-SPAN

RAIL SPLICE DETAIL

% " X 1 ¼" BUTTON HEAD SPLICE BOLTS WITH RECCESSED NUTS.

REQUIRED WITH 6'-3" POST SPACINGS.

3. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF

7. IF SHOWN ELSEWHERE IN THE PLANS OR AS DIRECTED BY THE ENGINEER, THE GUARD FENCE MAY BE FLARED

POSITIONED SO THAT THE FACE OF CURB IS LOCATED DIRECTLY BELOW OR BEHIND THE FACE OF THE RAIL. RAIL PLACED OVER CURBS SHALL BE INSTALLED SO THAT THE POST BOLT IS LOCATED APPROXIMATELY 25

DEPTH, WHICHEVER MAYBE LESS. ANY EXCESS POST LENGTH, AFTER MEETING THESE DEPTHS, MAY BE FIELD

MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210 ONLY PRODUCERS

13. FOR THE LOW FILL CULVERT OPTION, POSTS LOCATED PARTIALLY OR WHOLLY BETWEEN PRECAST BOX CULVERT UNITS, THE USE OF A CAST-IN-PLACE CONCRETE CLOSURE BETWEEN BOXES IS REQUIRED. THE LENGTH OF THE CAST-IN-PLACE CONCRETE CLOSURE SHALL ACCOMMODATE THE PLACEMENT OF THE LOW FILL CULVERT OPTION.

OF THE EDGE OF PAVEMENT OR FOR A PAVEMENT OVERLAY, USE A 10-FOOT STRAIGHTEDGE TO EXTEND THE PAVEMENT/SHOULDER SLOPE TO THE BACK OF RAIL, MEASURE FROM THE BOTTOM OF STRAIGHTEDGE TO THE TOP

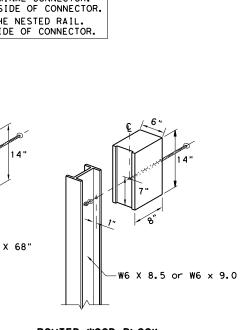
NOTE: CULVERTS OF 25 FT. OR LESS, SEE GF(31)LS STANDARD FOR "LONG SPAN" OPTION.

METAL BEAM GUARD FENCE

FILE: gf3119.dgn	DN: Tx	DOT	ck: KM	DW:	۷P	ck:CGL/AG
©T×DOT: NOVEMBER 2019	CONT	SECT	JOB		HIGHWAY	
REVISIONS	0908	27	006		CR 316	
	DIST		COUNTY			SHEET NO.
	ABL		KENT			33

#### GENERAL NOTES

- THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST, OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. THE EXACT POSITION OF TRANSITIONS SHALL BE AS SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER. REFER TO GF (31) STANDARD SHEET.
- 2. RAIL ELEMENT SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED IN THE PLANS.
- 3. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING." FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM REQUIRING CONSTRUCTION OF THE TRANSITION.
- BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 3/4" WASHER (FWC16a) AND NOT MORE THAN 1" BEYOND IT. TRIM BOLT LENGTH TO MEET REQUIRED LENGTH.
- POSTS SHALL NOT BE SET IN CONCRETE, OF ANY DEPTH.
- CROWN SHALL BE WIDENED TO ACCOMMODATE TRANSITIONS.
- WHERE SOLID ROCK IS ENCOUNTERED, CONTACT THE DESIGN DIVISION FOR ADDITIONAL GUIDANCE. (512) 416-2678
- UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. THE CONSTRUCTION DIVISION, TXDOT, MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210. ONLY PRODUCERS ON THE MPL CAN FURNISH COMPOSITE MATERIAL BLOCKS.
- 9. REFER TO GF(31)STANDARD SHEET & BRIDGE RAILING DETAILS FOR ADDITIONAL DETAILS.
- 10. FOR ROUND WOOD POSTS SYSTEMS, ALL ROUND WOOD POSTS SHALL BE 7  $\frac{1}{2}$ " DIA. MINIMUM THROUGHOUT THE TRANSITION.



LOW-SPEED TRANSITION



METAL BEAM GUARD FENCE THRIE-BEAM TRANSITION TL-2 MASH COMPLIANT

GF (31) TR TL2-19

E: gf31trt1219.dgn	DN: Tx	xDOT CK: KM DW: VP			VP	VP CK:CGL/AG		
TXDOT: NOVEMBER 2019	CONT	SECT JOB I			HIGHWAY			
REVISIONS	0908	27	006		C	R 316		
	DIST		COUNTY			SHEET NO.		
	ABL		KENT			34		

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NOTE: STEEL I-BEAM POST W6 X 8.5 (6'-0") PN:533G STANDARD WOOD BLOCKOUTS (6"X8"X14") PN:4076I %" X 10" HGR BOLT PN: 3500G LINE AT THE BACK OF POST #2 THRU #8 HGR NUT PN: 3340G FROM THE CENTERLINE OF POST(1) & POST(0) AT (POSTS 2 THRU 8) ANCHOR PADDLE ANGLE STRUT PN: 15204A-PN: 15202G POST(8) POST (7) POST (5) POST (3) SEE DETAIL 1 POST (1) DO NOT BOLT POST(0) PLAN VIEW BEGIN LENGTH OF NEED ANCHOR RAIL TO - POST (2) TRAFFIC FLOW MASH TEST LEVEL 3 (TL-3) LENGTH OF SoftStop TERMINAL (50'-9 1/2") 50'-9 1/2" STANDARD INSTALLATION LENGTH (MASH TL-3 SoftStop) END PAYMENT FOR SGT BEGIN STANDARD ANCHOR RAIL WITH SLOTS - (THREADED THRU HEAD)
SEE SoftStop MANUAL FOR COMPLETE DETAILS MIDDLE SLOT CUTOUT OUTSIDE SLOTS CUTOUT-(1) 1 3/4" X 6'-10 1/4" (2)1/2" X 6'-9 %" SEE GN(3) MBGF LAPPED IN DIRECTION OF TRAFFIC FLOW 25'-0" DOWNSTREAM W-BEAM GUARDRAIL PN:61G SoftStop ANCHOR RAIL (12GA) PN: 15215G & NOTE:B 3'-1 1/2"(+/-) ANCHOR PADDLE -PN: 15204A SEE NOTE: C END OF ANCHOR RAIL PN: 15215G DO NOT BOLT ANCHOR RAIL TO RAIL 25'-0"— PN: 61G -- RAIL 25'-0" PN: 15215G SEE A **HEIGHT** SEE DETAIL 2 POST(2) RAIL HEIGHT 13% DIA. YIELDING 13/6" DIA. — YIELDING ∠ (8) 5/8"× 1- 1/4" HGR BOLTS ∠ (8) % "x 1- ¼" GR BOLTS PN: 3360G HOLES PN: 3360G HOLES DEPTH %" HEX NUTS PN: 3340G %" HEX NUTS PN: 3340G (TYP 1-8) SEE 3 6'-1%" POST(1) POST (2) 6'-0" (SYTP) POST (8) POST (7) POST(4) POST(3) 4' -9 1/2" SYTP HARDWARE FOR POST(2) THRU POST(8) **ELEVATION VIEW** PN: 15000G PN: 15203G (1) %"x 10" HGR BOLT PN: 3500G (1) %" HGR HEX NUT PN: 3340G PART OTY ANGLE STRUT (1) 3/8" × 1 3/4" -PN: 15202G POST (0) 6' -5 3/8" NOTE: DO NOT BOLT ANCHOR RAIL PANEL TO POST (2) PN 3391G ALTERNATE BLOCKOUT PN: 152054 SEE GENERAL NOTE: 6 (2) % " WASHERS | | 6" X 8" X 14' (1) % " HEX NUT 5%6" × 1 - 1/2" HEX HD BOLT-GR-5 ANCHOR PLATE WASHER PN 4372G -4" X 7 1/2" X 14" HGR HEX NUT BLOCKOUT 1/2" THICK PN: 15206G BLOCKOUT COMPOSITE ANCHOR KEEPER WOOD -PN: 105286 1" ROUND WASHER F463 PN: 4902G PN: 4076B PN 3340G PLATE (24 GA)-(2) % " ~ ROUND WASHERS PN: 6777B NOTE:
DO NOT BOLT
ANCHOR RAIL TO PN: 15207G DETAIL 1 PN: 3240G (2) %6" x 2 ½" HEX HD BOLT GR-5 AI TERNATE SHOWN AT POST(1) - POST (2) BLOCKOUT BLOCKOUT WOOD W-BEAM RAIL 6" X 8" X 14" - BLOCKOUT WOOD NEAR GROUND PN: 105285G W-BEAM RAIL DETAIL 2 GENERAL NOTE: 6 %" X 10" %" HGR NUT PN: 3340G -HGR POST BOLT SHOWN AT POST (1 (2) 1/6 " ROUND WASHER HGR POST BOLT PN: 3500G HGR POST BOLT (WIDE) PN: 3240G PN: 3500G - 5% " HGR NUT PN: 3340G %" HGR NUT PN: 3340G POST 32" HEIGHT -1" NUT PN:3908G SHALL BE SECURELY TIGHTENED ANCHOR PADDLE-PN: 15204A HE I GHT (2) 56" HEX NUT A563 GR. DH PN: 3245G 31" RAIL 31" RAIL %"DIAMETER YIELDING HOLES AFTER FINAL ASSEMBLY HEIGHT HEIGHT LOCATED IN FLANGES BUT NOT DEFORMING THE KEEPER PLATE. (4 PLIES) POST 17" - 1/2"
HE I GHT SEE A (HOLES APROXIMATELY CENTERED AT FINISHED GRADE) FINISHED FINISHED FINISHED GRADE PN: 15202G GRADE GRADE ⅓6" DIA. (2) 3/4" × 2 1/2" HEX BOLT (TYP) PN: 3717G YIELDING HOLES 4' - 9 1/2" POST(2) (4) 3/4" FLAT WASHER (TYP) PN: 3701G (3, 4, 5, 6, 7 & 8) (2) ¾" HEX NUT (TYP) PN: 3704G POST(1) 6'- 1 3% " POST DEPTH (2) ANCHOR POST ANGLE PN: 15201G ISOMETRIC VIEW SECTION VIEW B-B SECTION VIEW A-A POST (1 & 2) 6'-0" (W6 X 8.5) 6'-0" (W6 X 8.5) I-BEAM POST PN: 533G (SYTP) I-BEAM POST PN: 15000G W6 X 8.5 I-BEAM POST SHOWING FRONT VIEW POST(1) STANDARD WOOD BLOCKOUT NOTE: DO NOT BOLT ANCHOR RAIL PANEL TO POST (2) 4'-9 1/2" (W6 X 8.5) (SYTP) I-BEAM POST PN: 15203G NOTE: NO BLOCKOUT INSTALLED AT POST(1) NOTE: NO BLOCKOUT INSTALLED AT POST (1) DETAIL 3 AT POST (0) 50' APPROACH GRADING APPROX 5'-10"-6'-5 38" (W6 X 15) I-BEAM POST PN: 15205A STANDARD MBGF TRAFFIC FLOW APPROACH GRADING (1V:10H OR FLATTER)
SEE PRODUCT ASSEMBLY MANUAL EDGE OF PAVEMENT NOTE: ADJUST WIDTH ACCORDINGLY WHEN OFFSET IS USED. (OFFSET "OPTION" SHOWN) RAIL OFFSET FOR ADDITIONAL GUIDANCE, THIS STANDARD IS A BASIC REPRESENTATION OF THE SOf+S+op END TERMINAL, IT IS NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL. APPROACH GRADING AT GUARDRAIL END TREATMENTS

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: TRINITY HIGHWAY AT 1 (888) 323-6374. 2525 N. STEMMONS FREEWAY, DALLAS, TX 75207
- FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE; SOf+Stop END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL. PN: 620237B
- 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WIT ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
- 6. A COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL AND REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE.
- 8. POSTS SHALL NOT BE SET IN CONCRETE.
- IT IS ACCEPTABLE TO INSTALL THE SOFTSTOP IMPACT HEAD PARALLEL TO THE GRADE LINE OR WITH AN UPWARD TILT.
- 10. DO NOT ATTACH THE SOFTSTOP SYSTEM DIRECTLY TO A RIGID BARRIER.
- 11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE SOF†S†op SYSTEM BE CURVED.
- 12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.

NOTE: A	THE INSTALLATION HEIGHT OF FULLY ASSEMBLED ANCHOR POST WILL VARY FROM 3-7/4" MIN. TO 4" MAX. ABOVE FINISHED GRADE.
NOTE: B	PART PN: 5852B RIGHT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING) PART PN: 5851B LEFT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING)
NOTE: C	W-BEAM SPLICE LOCATED BETWEEN LINE POST(4) AND LINE POST(5) GUARDRAIL PANEL 25'-0" PN: 61G ANCHOR RAIL 25'-0" PN: 15215G LAP GUARDRAIL IN DIRECTION OF TRAFFIC FLOW.

MAIN SYSTEM COMPONENTS

PART	QTY	MAIN SYSTEM COMPONENTS					
620237B	1	PRODUCT DESCRIPTION ASSEMBLY MANUAL (LATEST REV.)					
15208A	1	SoftStop HEAD (SEE MANUAL FOR RIGHT-LEFT APPROACH)					
15215G	1	SoftStop ANCHOR RAIL (12GA) WITH CUTOUT SLOTS					
61G	1	SoftStop DOWNSTREAM W-BEAM RAIL (12GA) (25'- 0")					
15205A	1	POST #0 - ANCHOR POST (6'- 5 %")					
15203G	1	POST #1 - (SYTP) (4'- 9 ½")					
15000G	1	POST #2 - (SYTP) (6'- 0")					
533G	6	POST #3 THRU #8 - I-BEAM (W6 x 8.5) (6'- 0")					
4076B	7	BLOCKOUT - WOOD (ROUTED) (6" x 8" x 14")					
6777B	7	BLOCKOUT - COMPOSITE (4" $\times$ 7 $\frac{1}{2}$ " $\times$ 14")					
15204A	1	ANCHOR PADDLE					
15207G	1	ANCHOR KEEPER PLATE (24 GA)					
15206G	1	ANCHOR PLATE WASHER ( 1/2" THICK )					
15201G	2	ANCHOR POST ANGLE (10" LONG)					
15202G	1	ANGLE STRUT					
HARDWARE							
4902G	1	1" ROUND WASHER F436					
3908G	1	1" HEAVY HEX NUT A563 GR. DH					
3717G	2	¾" × 2 ½" HEX BOLT A325					
3701G	4	3/4" ROUND WASHER F436					
3704G	2	34" HEAVY HEX NUT A563 GR. DH					
3360G	16	%" × 1 ¼" W-BEAM RAIL SPLICE BOLTS HGR					
3340G	25	%" W-BEAM RAIL SPLICE NUTS HGR					
3500G	7	%" × 10" HGR POST BOLT A307					
3391G	1	%" × 1 ¾" HEX HD BOLT A325					
4489G	1	%" × 9" HEX HD BOLT A325					
4372G	4	%" WASHER F436					
105285G	2	%6" × 2 1/2" HEX HD BOLT GR-5					
105286G	1	%6" × 1 1/2" HEX HD BOLT GR-5					
3240G	6	% " ROUND WASHER (WIDE)					
3245G	3	% " HEX NUT A563 GR. DH					
5852B	1_	HIGH INTENSITY REFLECTIVE SHEETING - SEE NOTE: B					

Texas Department of Transportation

TRINITY HIGHWAY SOFTSTOP END TERMINAL MASH - TL-3

SGT (10S) 31-16

: sgt10s3116	DN: TxD	OT	ck: KM	DW: VP		ck: MB/VP	
TxDOT: JULY 2016	CONT	SECT	JOB		HIGHWAY		
REVISIONS	0908	27	006		CR 316		
	DIST	COUNTY			SHEET NO.		
	ABL		KENT			35	

#### GENERAL NOTES

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: LINDSAY TRANSPORTATION SOLUTIONS (LTS) - BARRIER SYSTEMS, INC. AT (707) 374-6800
- FOR INSTALLATION, REPAIR, & MAINTENANCE REFER TO THE; MAX-TENSION INSTALLATION INSTRUCTION MANUAL. P/N MANMAX REV D (ECN 3516).
- APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURE'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- 5. ALL STEEL COMPONENTS ARE GALVANIZED PER ASTM A123 OR EQUIVALENT UNLESS OTHERWISE STATED.
- 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POST WITH COMPOSITE BLOCKOUTS.
- COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- 8. REFER TO INSTALLATION MANUAL FOR SPECIFIC PANEL LAPPING GUIDANCE.
- IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL FOR INSTALLATION GUIDANCE.
- 10. POSTS SHALL NOT BE SET IN CONCRETE.
- 11. A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POST TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST
- 12. MAX-TENSION SYSTEM SHALL NEVER BE INSTALLED WITHIN A CURVED SECTION OF GUARDRAIL.
- 13. IF A DELINEATION MARKER IS REQUIRED, MARKER SHALL BE IN ACCORDANCE WITH TEXAS MUTCD.
- 14. THE SYSTEM IS SHOWN WITH 12'-6" MBGF PANELS, 25'-0" MBGF PANELS ARE ALSO ALLOWED.
- 15. A MINIMUM OF 12'-6" OF 12GA. MBGF IS REQUIRED IMMEDIATELY DOWNSTREAM OF THE MAX-TENSION SYSTEM.

TEM#	PART NUMBER	DESCRIPTION	QTY
1	BSI-1610060-00	SOIL ANCHOR - GALVANIZED	1
2	BSI-1610061-00	GROUND STRUT - GALVANIZED	1
3	BSI-1610062-00	MAX-TENSION IMPACT HEAD	1
4	BSI-1610063-00	W6×9 I-BEAM POST 6FTGALVANIZED	1
5	BSI-1610064-00	TSS PANEL - TRAFFIC SIDE SLIDER	1
6	BSI-1610065-00	ISS PANEL - INNER SIDE SLIDER	1
7	BSI-1610066-00	TOOTH - GEOMET	1
8	BSI-1610067-00	RSS PLATE - REAR SIDE SLIDER	1
9	B061058	CABLE FRICTION PLATE - HEAD UNIT	1
10	BSI-1610069-00	CABLE ASSEMBLY - MASH X-TENSION	2
11	BSI-1012078-00	X-LITE LINE POST-GALVANIZED	8
12	B090534	8" W-BEAM COMPOSITE-BLOCKOUT XT110	8
13	BSI-4004386	12'-6" W-BEAM GUARD FENCE PANELS 12GA.	4
14	BSI-1102027-00	X-LITE SQUARE WASHER	1
15	BSI-2001886	5/8" X 7" THREAD BOLT HH (GR.5)GEOMET	1
16	BSI-2001885	34" X 3" ALL-THREAD BOLT HH (GR.5)GEOMET	4
17	4001115	%" X 1 1/4" GUARD FENCE BOLTS (GR. 2) MGAL	48
18	2001840	%" X 10" GUARD FENCE BOLTS MGAL	8
19	2001636	% " WASHER F436 STRUCTURAL MGAL	2
20	4001116	% " RECESSED GUARD FENCE NUT (GR. 2)MGAL	59
21	BSI-2001888	%" X 2" ALL THREAD BOLT (GR.5)GEOMET	1
22	BSI-1701063-00	DELINEATION MOUNTING (BRACKET)	1
23	BSI-2001887	1/4" X 3/4" SCREW SD HH 410SS	7
24	4002051	GUARDRAIL WASHER RECT AASHTO FWRO3	1
25	SEE NOTE BELOW	HIGH INTENSITY REFLECTIVE SHEETING	1
26	4002337	8" W-BEAM TIMBER-BLOCKOUT, PDB01B	8
27	BSI-4004431	25' W-BEAM GUARDRAIL PANEL, 8-SPACE, 12GA.	2
28	MANMAX Rev-(D)	MAX-TENSION INSTALLATION INSTRUCTIONS	1

Texas Department of Transportation

Design Division Standard

# MAX-TENSION END TERMINAL MASH - TL-3

SGT (11S) 31-18

FILE: sg+11s3118.dgn	DN: TxE	TOO	ck: KM	DW: T×DO		ck: CL	
C TxDOT: FEBRUARY 2018	CONT	SECT	JOB		Н	IGHWAY	
REVISIONS	0908	27	006	(		CR 316	
	DIST		COUNTY			SHEET NO.	
	ABL		KENT			36	

STANDARD

POST 8

POST 8

3'-4'

1/2" X 1 1/4" A325 BOLT (m)-

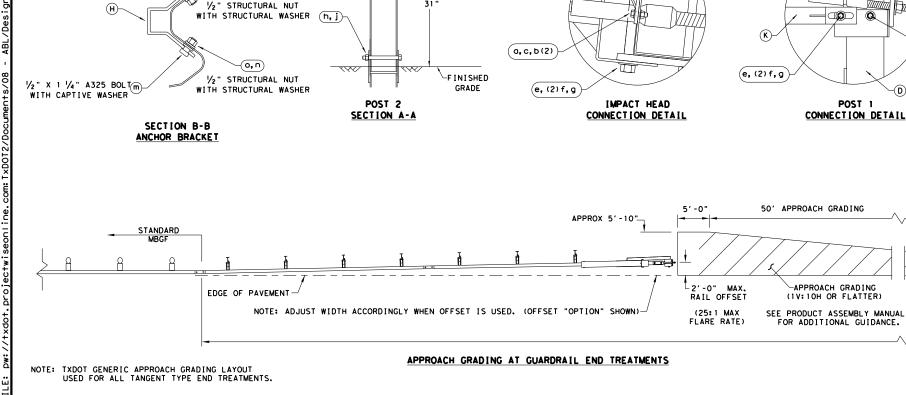
WITH CAPTIVE WASHER

(POST 3-8)

INSTALLATION DEPTH

3'-1 /2" T

31" MBGF



50'-0'

POST 5

POST 5

PLAN VIEW

(O)

W-BEAM MGS RAIL SECTION 12'-6"

 $\mathcal{A}_{0}$ 

POST 4

POST 4

- FINISHED

**ELEVATION VIEW** 

GRADE

POST 3

POST 3

 $\sqrt{N}$ 

W-BEAM MGS RAIL SECTION 9'-4 1/2"

 $\sqrt{N}$ 

d, (8), g(8)

POST 2

SEE IMPACT HEAD-

CONNECTION

IMPACT HEAD

TRAFFIC FLOW

OBJECT (

(c)

1.1

POST

(G)

CONNECTION

- POST

SOIL PLATE ON

DOWNSTREAM SIDE

ALTERNATIVE ITEMS NOT SHOWN. *

* ITEM(P) 8" WOOD-BLOCKOUT

* X ITEM(Q) 25'GUARD FENCE PANEL

(H,m(8),n(8),o(8))

DETAIL

**(B**)

W-BEAM GUARDRAIL END SECTION

12' -6"

BEGIN LENGTH OF NEED

,–(B)

(E)-

DEPTH

6'-0"

В

POST 2

**Q** 

POST 1

STRUT

NOTE: SEE (GENERAL NOTE 14) FOR DRIVING CAP INFORMATION.

DEPTH

2'-0'

q, g ) HARDWARE FOR (POST 8) THRU (POST 3)

POST 6

POST 6

POST

POST 7

- 1. ITEM (M) COMPOSITE BLOCKOUTS INSTALLED

AT LINE POST(8) THRU LINE POST(3).

2. ITEM P WOOD BLOCKOUTS CAN BE USED AS ALTERNATE.

(d, g)

 $\sqrt{0}$ 

W-BEAM MGS RAIL SECTION

* NOTES:

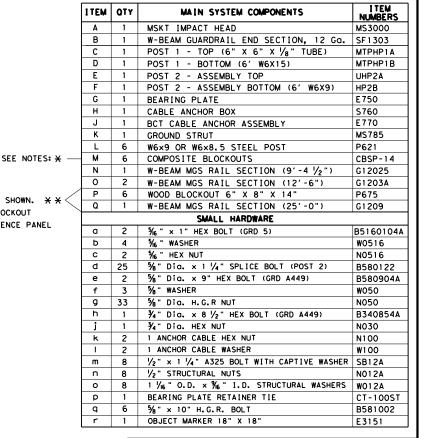
-END PAYMENT FOR MSKT INSTALLATION

,-(o)

FINISHED

GRADE

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: ROAD SYSTEMS, INC. (432)263-2435. 3616 OLD HOWARD COUNTY AIRPORT, BIG SPRING, TX 79720
- 2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE; MSKT END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL (PUBLICATION~062717).
- 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- 5. HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
- 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POSTS WITH COMPOSITE BLOCKOUTS.
- 7. A COMPOSITE MATERIAL BLOCKOUTS THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- 8. IF SOLID ROCK IS ENCOUNTERED IN THE AREA OF (POST 1) AND / OR (POST 2) CONTACT THE MANUFACTURER, & REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE
- 9. POSTS SHALL NOT BE SET IN CONCRETE.
- 10. SYSTEM MUST BE ATTACHED TO STANDARD 31" MBGF.
- 11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE MSKT SYSTEM BE CURVED.
- 12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.
- 13. THE SYSTEM IS SHOWN WITH TWO 12'-6" MBGF PANELS, ONE 25'-0" MBGF PANEL IS ALSO ALLOWED IN THEIR PLACE.
- A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POSTS 3-8 TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST. SPECIAL DRIVING CAP TO BE USED ON LOWER POSTS 1 & 2 TO PREVENT DAMAGE TO THE WELDED PLATES.



Texas Department of Transportation

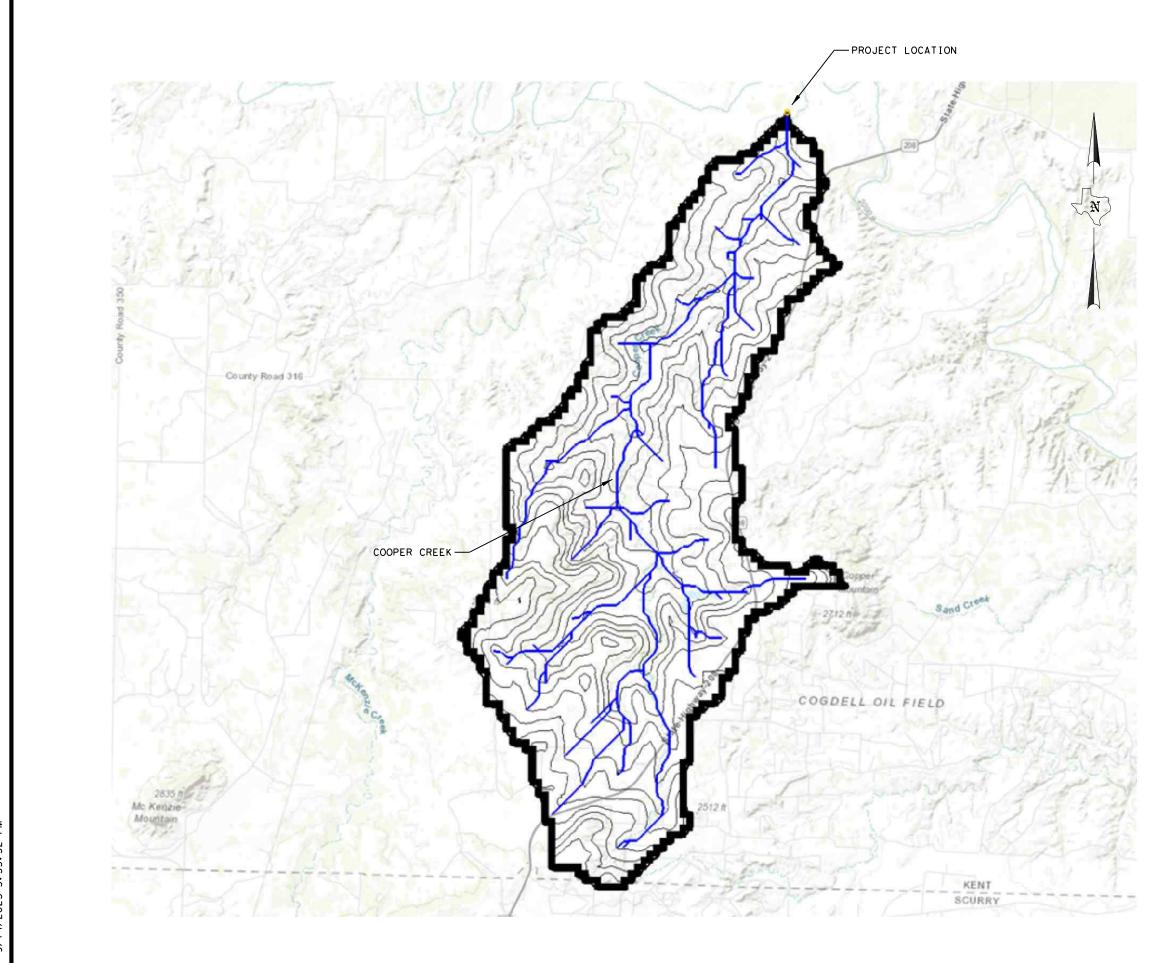
SINGLE GUARDRAIL TERMINAL MSKT-MASH-TL-3

SGT (12S) 31-18

E: sg+12s3118,dgn	DN:Tx	DOT	CK:KM	DW	:VP	CK: CL
T×DOT: APRIL 2018	CONT	SECT	HIGHWAY			
REVISIONS	0908	27	006	CI	R 316	
	DIST		COUNTY		SHEET NO.	
	ABL		KENT			37

NOTE: THIS STANDARD IS A BASIC REPRESENTATION OF THE MSKT END TERMINAL, IT IS NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL.

TRAFFIC FLOW



WMS P	EAK FLOWS
EVENT	PEAK DISCHARGE FLOW (CFS)
2 YR	41.18
10 YR	330.24
25 YR	653.95
50 YR	970.27
100 YR	1372.57

### NOTES:

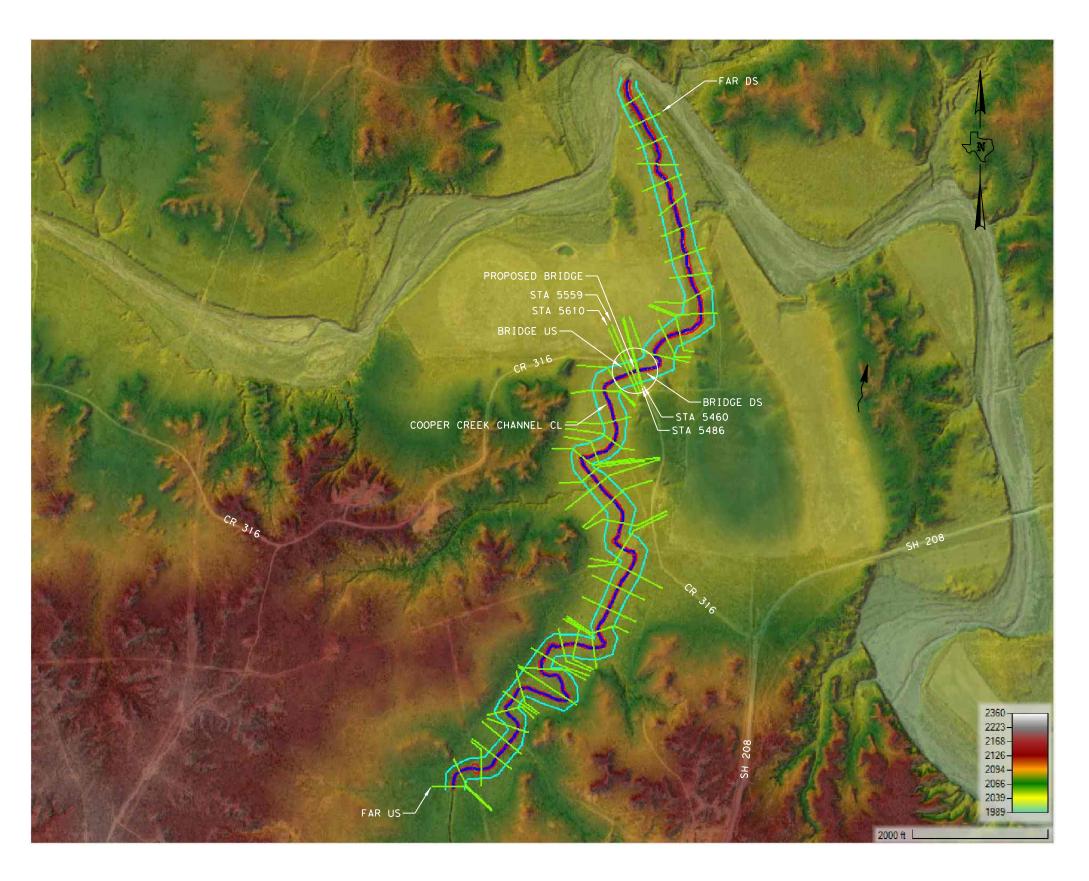
- 1. DRAINAGE AREA ANALYSIS DONE IN WMS 11.1.6
- 2. ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM
- 3. TIME OF CONCENTRATION IS 17.675 HOURS
- 4. MAX STREAM LENGTH IS 10.78 MILES
- 5. CURVE NUMBER IS 48.393 BASED ON LAND USE DATA
- 6. AREA OF WATERSHED IS 19.21 SQ. MI



### CR 316 DRAINAGE AREA MAP



_							
SCALE:	N/A		S	HEET	1	OF	1
FHWA DIVISION	PF	ROJECT NO	•	нІ	GHWA	Y NO.	
6	SEE	TITLE SH	IEET	(	CR :	316	
STATE		COUNT		SH	EET NO		
TEXAS							
DISTRICT	CONTROL	SECTION	3		38		
ABL	0908	27	000	5			



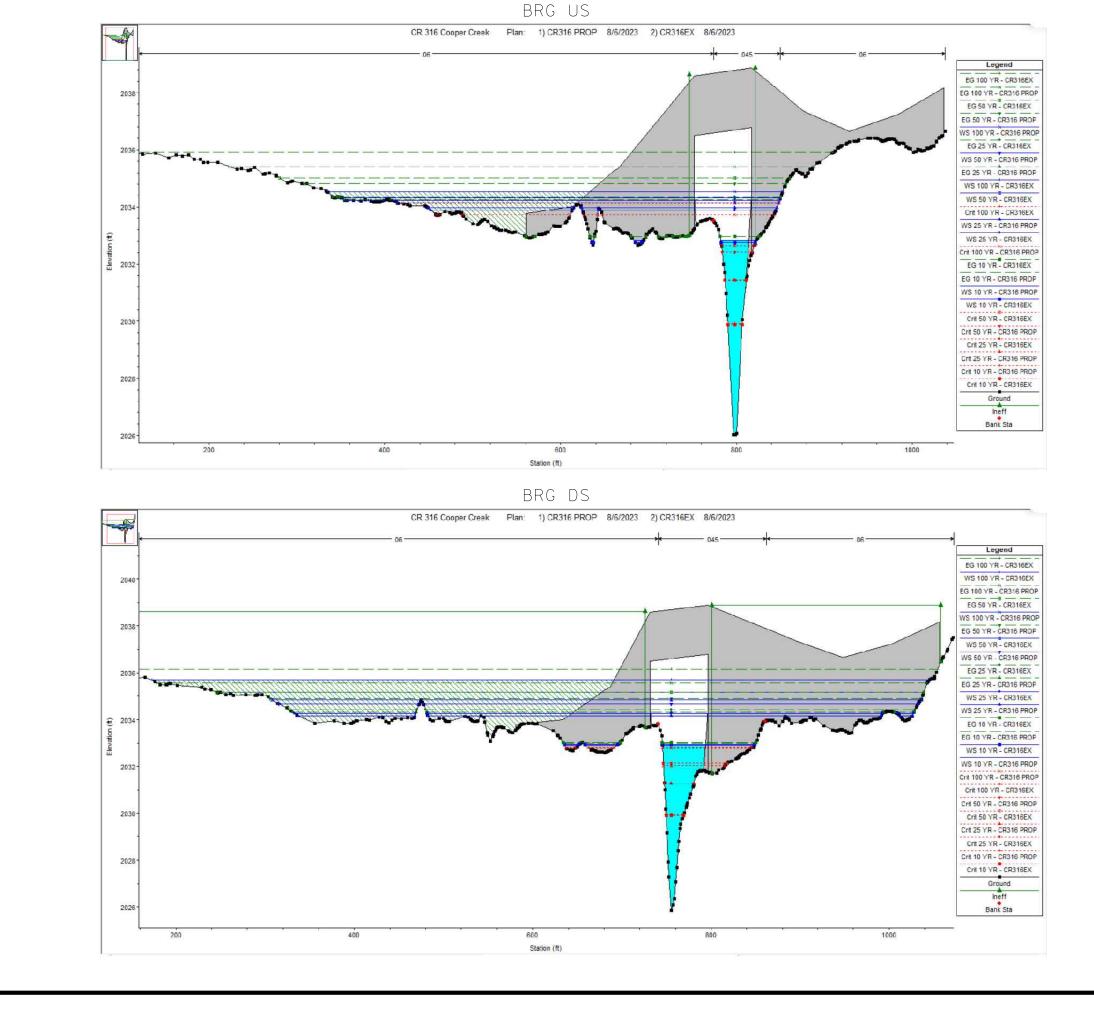
### NOTES:

- 1. HEC-RAS VERSION 6.3.1 WAS USED TO MODEL STORMWATER FLOWS AT THIS BRIDGE SITE FOR EXISTING AND PROPOSED CONDITIONS
- 2. CRITICAL HYDRAULIC DATA CALCULATED FOR EXISTING AND PROPOSED STRUCTURES ARE PROVIDED IN THE TABLES ON THE FOLLOWING HYDRAULIC DATA SHEETS
- 3. THIS SITE IS LOCATED IN A UNMAPPED FEMA





SCALE:	N/A		SF	HEET	1	OF 6			
FHWA DIVISION	PF	ROJECT NO	ΗI	IGHWAY NO.					
6	SEE	TITLE SH		CR	316				
STATE		COUNT	Y		IEET NO.				
TEXAS		KENT							
DISTRICT	CONTROL	SECTION	3		39				
ABL	0908	27	000	6					



#### NOTES:

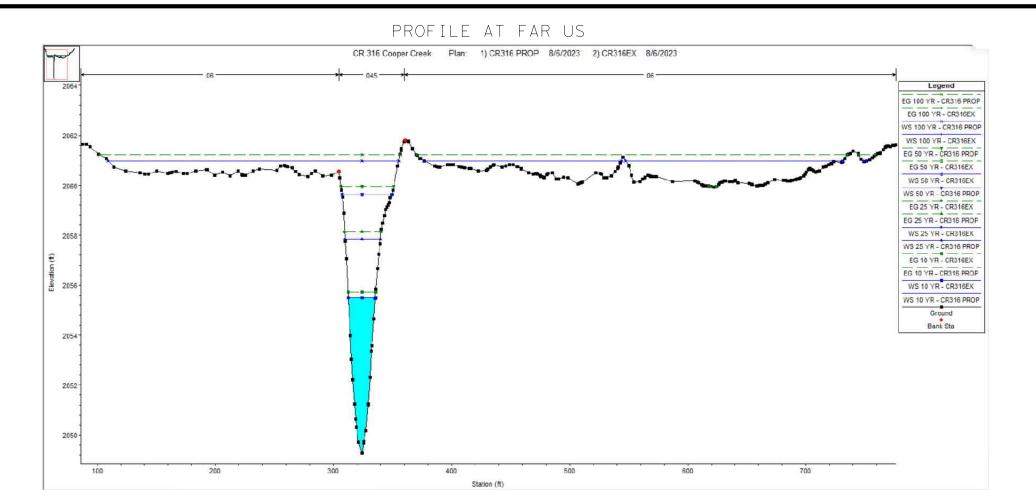
1. DATA COMES FROM HYDRAULIC ANALYSIS PREFORMED ON HEC-RAS VERSION 6.3.1



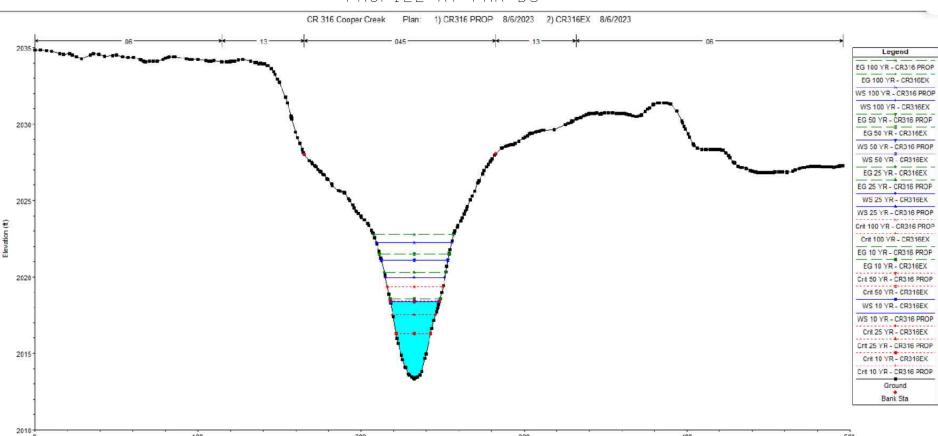


SCALE:	N/A		SH	HEET	2	OF	6
FHWA DIVISION	PF	ROJECT NO		ΗI	GHW	АУ ИО	•
6	SEE		CR	316			
STATE			SH	EET N	١٥.		
TEXAS							
DISTRICT	CONTROL	В		40			
ABL	0908	27	000	6			





### PROFILE AT FAR DS



Station (ft)





SCALE:	N/A			Sł	HEET	3	OF	6
FHWA DIVISION	PF	ROJECT N	10.		ΗI	GHW	AA NO	
6	SEE	TITLE S		CR	316			
STATE			SH	EET N	١٥.			
TEXAS								
DISTRICT	CONTROL	В		41				
ABL	0908	27		00	6			

	Ι	Ι					Τ		1	uuru ra	ible for Coo	POI OIGEN					T		Т		1	
			Q To	otal	Min Ch	Elev	w.s.	Elev	Crit	W.S.	E.G.	Elev	E.G.	Slope	Vel	Chnl	Flow	Area	Top \	Width	Froude	e # Chl
Reach	River	Profile	(cf	is)	(f	ft)	(f	t)	(1	ft)	(f	t)	(ft	/ft)	(ft.	s)	(sq	ıft)	(1	ft)		
	Sta		CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP
Cooper Creek	15579	10 YR	330.24	330.24	2049.28	2049.28	2055.48	2055.48			2055.71	2055.71	0.002709	0.002709	3.80	3.80	86.88	86.88	22.73	22.73	0.34	0.34
Cooper Creek	15579	50 YR	970.27	970.27	2049.28	2049.28	2059.62	2059.62			2059.95	2059.95	0.002756	0.002756	4.63	4.63	209.64	209.64	42.08	42.08	0.37	0.37
Cooper Creek	15579	100 YR	1372.57	1372.57	2049.28	2049.28	2060.97	2060.97			2061.21	2061.21	0.002053	0.002053	4.25	4.25	555.60	555.60	599.50	599.50	0.32	0.32
Cooper Creek	15324	10 YR	330.24	330.24	2049.09	2049.09	2054.70	2054.70			2054.96	2054.96	0.003138	0.003138	4.09	4.09	80.70	80.70	20.43	20.43	0.36	0.36
Cooper Creek	15324	50 YR	970.27	970.27	2049.09	2049.09	2058.73	2058.73			2059.18	2059.18	0.003245	0.003245	5.39	5.39	179.94	179.94	29.95	29.95	0.39	0.39
Cooper Creek	15324	100 YR	1372.57	1372.57	2049.09	2049.09	2059.97	2059.97	2056.07	2056.07	2060.50	2060.50	0.003899	0.003899	5.97	5.97	285.61	285.61	268.23	268.23	0.43	0.43
Cooper Creek	15004	10 YR	330.24	330.24	2047.47	2047.47	2053.80	2053.80			2054.03	2054.03	0.002657	0.002657	3.82	3.82	86.34	86.34	21.42	21.42	0.34	0.34
Cooper Creek	15004	50 YR	970.27	970.27	2047.47	2047.47	2057.79	2057.79			2058.20	2058.20	0.002828	0.002828	5.12	5.12	189.35	189.35	30.80	30.80	0.36	0.36
Cooper Creek	15004	100 YR	1372.57	1372.57	2047.47	2047.47	2058.97	2058.97	2054.89	2054.89	2059.37	2059.37	0.002901	0.002901	5.35	5.35	399.85	399.85	375.08	375.08	0.37	0.37
Cooper Creek	14688	10 YR	330.24	330.24	2046.39	2046.39	2053.04	2053.04			2053.24	2053.24	0.002298	0.002298	3.61	3.61	91.49	91.49	22.10	22.10	0.31	0.31
Cooper Creek	14688	50 YR	970.27	970.27	2046.39	2046.39	2056.94	2056.94			2057.33	2057.33	0.002633	0.002633	5.00	5.00	196.06	196.06	55.50	55.50	0.35	0.35
Cooper Creek	14688	100 YR	1372.57	1372.57	2046.39	2046.39	2057.95	2057.95	2053.91	2053.91	2058.42	2058.42	0.003185	0.003185	5.70	5.70	311.35	311.35	267.62	267.62	0.39	0.39
Cooper Creek	14419	10 YR	330.24	330.24	2045.49	2045.49	2052.30	2052.30			2052.48	2052.48	0.003448	0.003448	3.42	3.42	96.48	96.48	22.11	22.11	0.29	0.29
Cooper Creek	14419	50 YR	970.27	970.27	2045.49	2045.49	2056.00	2056.00			2056.40	2056.40	0.004729	0.004729	5.04	5.04	193.15	193.15	54.65	54.65	0.35	0.35
Cooper Creek	14419	100 YR	1372.57	1372.57	2045.49	2045.49	2057.25	2057.25			2057.48	2057.48	0.003264	0.003264	4.34	4.34	535.31	535.31	527.12	527.12	0.30	0.30
Cooper Creek	14167	10 YR	330.24	330.24	2044.95	2044.95	2051.44	2051.44			2051.67	2051.67	0.002967	0.002967	3.89	3.89	84.80	84.80	22.82	22.82	0.36	0.36
Cooper Creek	14167	50 YR	970.27	970.27	2044.95	2044.95	2054.97	2054.97			2055.41	2055.41	0.003311	0.003311	5.28	5.28	183.75	183.75	33.74	33.74	0.40	0.40
Cooper Creek	14167	100 YR	1372.57	1372.57	2044.95	2044.95	2056.12	2056.12	2052.69	2052.69	2056.60	2056.60	0.003638	0.003638	5.74	5.74	332.28	332.28	371.15	371.15	0.42	0.42
Cooper Creek	13829	10 YR	330.24	330.24	2043.90	2043.90	2050.00	2050.00			2050.35	2050.35	0.005292	0.005292	4.76	4.76	69.32	69.32	20.80	20.80	0.46	0.46
Cooper Creek	13829	50 YR	970.27	970.27	2043.90	2043.90	2053.48	2053.48			2054.05	2054.05	0.004893	0.004893	6.07	6.07	165.00	165.00	59.77	59.77	0.47	0.47
Cooper Creek	13829	100 YR	1372.57	1372.57	2043.90	2043.90	2054.91	2054.91	2051.96	2051.96	2055.37	2055.37	0.003767	0.003767	5.78	5.78	360.08	360.08	370.47	370.47	0.42	0.42
Cooper Creek	13598	10 YR	330.24	330.24	2043.67	2043.67	2049.19	2049.19			2049.42	2049.42	0.00296	0.00296	3.86	3.86	85.45	85.45	24.08	24.08	0.36	0.36
Cooper Creek	13598	50 YR	970.27	970.27	2043.67	2043.67	2052.69	2052.69			2053.12	2053.12	0.003086	0.003086	5.24	5.24	185.28	185.28	32.97	32.97	0.39	0.39
Cooper Creek	13598	100 YR	1372.57	1372.57	2043.67	2043.67	2053.95	2053.95			2054.50	2054.50	0.003535	0.003535	5.98	5.98	233.61	233.61	63.72	63.72	0.42	0.42
Cooper Creek	13410	10 YR	330.24	330.24	2043.15	2043.15	2048.71	2048.71			2048.91	2048.91	0.002426	0.002426	3.63	3.63	91.09	91.09	24.04	24.04	0.33	0.33
Cooper Creek	13410	50 YR	970.27	970.27	2043.15	2043.15	2052.12	2052.12			2052.53	2052.53	0.003117	0.003117	5.09	5.09	190.65	190.65	36.22	36.22	0.39	0.39
Cooper Creek	13410	100 YR	1372.57	1372.57	2043.15	2043.15	2053.29	2053.29			2053.81	2053.81	0.003695		5.77	5.77	245.18	245.18	77.66	77.66	0.43	0.43
Cooper Creek	12678	10 YR	330.24	330,24	2040.72	2040.72	2047.17	2047.17			2047,31	2047.31	0.001924	0.001924	2.98	2.98	110.74	110.74	34.45	34.45	0,29	0.29
Cooper Creek	12678	50 YR	970.27	970.27	2040.72	2040.72	2050.75	2050.75			2050.94	2050.94	0.00149	0.00149	3.45	3.45	281.37	281.37	59.14	59.14	0.28	0.28
Cooper Creek	12678	100 YR	1372.57	1372.57	2040.72	2040.72	2051.77	2051.77			2052.00	2052.00	0.00167	0.00167	3.88	3.88	417.18	417.18	299.60	299.60	0.30	0.30
Cooper Creek	11979	10 YR	330.24	330.24	2039.76	2039.76	2045.74	2045.74			2045.91	2045.91			3.31	3.31	99.70	99.70	27.59	27.59	0.31	0.31
Cooper Creek	11979	50 YR	970.27	970.27	2039.76	2039.76	2049.45	2049.45			2049.73	2049.73	0.001998		4.26	4.26	228.16	228.16	57.27	57.27	0.32	0.32
Cooper Creek	11979	100 YR	1372.57	1372.57	2039.76	2039.76	2050.48	2050.48			2050.73	2050.73	0.00206	0.00206	4.34	4.34	500.96	500.96	514.11	514.11	0.33	0.33
Cooper Creek	11776	10 YR	330.24	330.24	2038.76	2038.76	2045.33	2045.33			2045.49	2045.49	0.002019	0.002019	3.23	3.23	102.13	102.13	28.67	28.67	0.30	0.30
Cooper Creek	11776	50 YR	970.27	970.27	2038.76	2038.76	2049.06	2049.06			2049.33	2049.33	0.001896		4.17	4.17	233.61	233.61	57.18	57.18	0.31	0.31
Cooper Creek	11776	100 YR	1372.57	1372.57	2038.76	2038.76	2050.04	2050.04			2050.30	2050.30	0.001959		4.33	4.33	513.55	513.55	541.97	541.97	0.32	0.32
Cooper Creek	11415	10 YR	330.24	330.24	2037.77	2037.77	2044.53	2044.53			2044.72	2044.72	0.00226	0.00226	3.44	3.44	95.89	95.89	25.71	25.71	0.31	0.31
Cooper Creek	11415	50 YR	970.27	970.27	2037.77	2037.77	2048.27	2048.27	2045.29	2045.20	2048.58	2048.58			4.44	5.12	221.03	221.03	57.18	57.18	0.34	0.34
Cooper Creek	11415	100 YR 10 YR	1372.57	1372.57	2037.77	2037.77	2049.07	2049.07	2045.38	2045.38	2049.46	2049.46	0.002941		5.13 3.82	5.13	358.37	358.37	335.67	335.67	0.39	0.39
Cooper Creek	11216		330.24 970.27	330.24	2037.28	2037.28	2043.99	2043.99			2044.21	2044.21				5.05	86.56 201.25	86.56 201.25	22.78 111.85	22.78	0.35	
Cooper Creek	11216	50 YR		970.27	2037.28	2037.28	2047.65	2047.65			2048.05	2048.05	0.003086		5.05	5.05		201.25		111.85		0.39
Cooper Creek	11216	100 YR	1372.57	1372.57	2037.28	2037.28	2048.54	2048.54			2048.85	2048.85	0.002936		4.98	4.98	496.18	496.18	542.73	542.73	0.38	0.38
Cooper Creek	10953	10 YR	330.24	330.24	2037.08	2037.08	2043.40	2043.40			2043.57	2043.57	0.002051	0.002051	3.31	3.31	99.87	99.87	26.81	26.81	0.30	0.30
Cooper Creek	10953	50 YR	970.27	970.27	2037.08	2037.08	2047.04	2047.04			2047.34	2047.34	0.002172		4.42	4.42	234.12	234.12	105.80	105.80	0.33	0.33
Cooper Creek	10953	100 YR	1372.57	1372.57	2037.08	2037.08	2047.95	2047.95			2048.19	2048.19	0.002088		4.37	4.37	604.47	604.47	735,96	735.96	0.33	0.33
Cooper Creek	10537	10 YR	330.24	330.24	2036.01	2036.01	2042.60	2042.60			2042.76	2042.76	0.001846		3.22	3.22	102.59	102.59	26.49	26.49	0.29	0.29
Cooper Creek	10537	50 YR	970.27	970.27	2036.01	2036.01	2046.10	2046.10			2046.40	2046.40	0.002382		4.42	4.42	225.83	225.83	99.92	99.92	0.35	0.35
Cooper Creek	10537	100 YR	1372.57	1372.57	2036.01	2036.01	2046.86	2046.86	1	l	2047.17	2047.17	J U.UU2928	0.002928	4.75	4.75	426.48	426.48	382.24	382.24	0.38	0.38





SCALE:	N/A			SH	HEET	4	OF	6
FHWA DIVISION	PF	ROJECT	НΙ	GHW	ON YA			
6	SEE	TITLE	SH	EET		CR	316	
STATE			SH	IEET 1	١0.			
TEXAS								
DISTRICT	CONTROL	SECTIO	3		42			
ABL	0908	27		000	5			

			QT	otal	Min Ch	Elev	w.s.	Elev	Crit	w.s.	E.G.	Elev	E.G.	Slope	Vel (	Chnl	Flow	Area	Top V	Vidth		
D It	River	D #11-	(c.	fs)	(f	it)	(f	t)	(1	t)	(1	ft)	(ft	/ft)	(ft/	/s)	(sq	ft)	(f	t)	Froude	# Chl
Reach	Sta	Profile		CR316		CR316		CR316		CR316		CR316	CR316	CR316		CR316		CR316		CR316	27242 FY	CR316
			CR316 EX	PROP	CR316 EX	PROP	CR316 EX	PROP	CR316 EX	PROP	CR316 EX	PROP	EX	PROP	CR316 EX	PROP	CR316 EX	PROP	CR316 EX	PROP	CR316 EX	PROP
Cooper Creek	10163	10 YR	330.24	330.24	2035.56	2035.56	2041.85	2041.85			2042.03	2042.03	0.002058	0.002058	3.39	3.39	97.53	97.53	25.23	25.23	0.30	0.30
Cooper Creek	10163	50 YR	970.27	970.27	2035.56	2035.56	2045.12	2045.12			2045.46	2045.46	0.002683	0.002683	4.80	4.80	247.52	247.52	226.22	226.22	0.36	0.36
Cooper Creek	10163	100 YR	1372.57	1372.57	2035.56	2035.56	2045.87	2045.87			2046.15	2046.15	0.002703	0.002703	4.76	4.76	547.21	547.21	558.85	558.85	0.37	0.37
Cooper Creek	9818	10 YR	330.24	330.24	2034.75	2034.75	2041.00	2041.00			2041.22	2041.22	0.002697	0.002697	3.68	3.68	89.68	89.68	25.18	25.18	0.34	0.34
Cooper Creek	9818	50 YR	970.27 1372.57	970.27	2034.75	2034.75	2044.02	2044.02	2042.07	2042.07	2044.41	2044.41	0.003451	0.003451	5.11	5.11	233.59	233.59 636.36	179.86	179.86	0.41	0.41
Cooper Creek Cooper Creek	9818 9526	100 YR 10 YR	330.24	1372.57 330.24	2034.75	2034.75 2034.18	2045.03	2045.03	2042.07	2042.07	2045.23	2045.23	0.002494	0.002494	4.22 4.09	4.22	636.44 80.75	80.75	640.95 26.06	26.06	0.35	0.35
Cooper Creek	9526	50 YR	970.27	970.27	2034.18	2034.18	2042.66	2042.66			2043.19	2043.19	0.003079	0.003073	5.89	5.89	164.79	164.79	37.04	37.04	0.49	0.49
Cooper Creek	9526	100 YR	1372.57	1372.57	2034.18	2034.18	2043.20	2043.20	2041.47	2041.47	2044.01	2044.01	0.007157	0.007158	7.29	7.29	212.41	212.39	176.86	176.82	0.60	0.60
Cooper Creek	9057	10 YR	330.24	330.24	2033.63	2033.63	2039.51	2039.51			2039.56	2039.56	0.000702	0.000702	1.90	1.90	173.82	173.82	52.32	52.32	0.18	0.18
Cooper Creek	9057	50 YR	970.27	970.27	2033.63	2033.63	2042.25	2042.25			2042.32	2042.32	0.000709	0.000709	2.30	2.30	614.76	614.76	443.34	443.34	0.19	0.19
Cooper Creek	9057	100 YR	1372.57	1372.57	2033.63	2033.63	2042.82	2042.82			2042.90	2042.90	0.00079	0.00079	2.51	2.51	1029.85	1029.98	1047.05	1047.12	0.21	0.21
Cooper Creek	8602	10 YR	330.24	330.24	2032.14	2032.14	2038.92	2038.92			2039.07	2039.07	0.001791	0.001791	3.15	3.15	104.70	104.70	27.36	27.36	0.28	0.28
Cooper Creek	8602	50 YR	970.27	970.27	2032.14	2032.14	2041.25	2041.25	2038.40	2038.40	2041.69	2041.69	0.003762	0.003762	5.35	5.35	205.93	205.93	183.50	183.50	0.43	0.43
Cooper Creek	8602	100 YR	1372.57	1372.57	2032.14	2032.14	2041.84	2041.84	2039.43	2039.43	2042.25	2042.25	0.003903	0.003906	5.62	5.63	465.27	464.92	718.41	718.03	0.44	0.44
Cooper Creek	8213	10 YR	330.24	330.24	2031.59	2031.59	2038.22	2038.22			2038.37	2038.37	0.001786	0.001786	3.13	3.13	105.67	105.66	28.27	28.27	0.28	0.28
Cooper Creek	8213	50 YR	970.27	970.27	2031.59	2031.59	2040.07	2040.07			2040.31	2040.31	0.002754	0.002754	4.46	4.46	386.16	386.16	396.70	396.70	0.37	0.37
Cooper Creek	8213	100 YR	1372.57	1372.57	2031.59	2031.59	2040.49	2040.49			2040.72	2040.72	0.003134	0.003124	4.78	4.77	597.03	598.12	596.55	597.22	0.39	0.39
Cooper Creek	7906	10 YR	330.24	330.24	2030.96	2030.96	2037.47	2037.47			2037.68	2037.68	0.002867	0.002868	3.69	3.69	89.55	89.54	26.15 566.99	26.14 566.99	0.35	0.35
Cooper Creek Cooper Creek	7906 7906	50 YR 100 YR	970.27 1372.57	970.27 1372.57	2030.96	2030.96 2030.96	2039.39	2039.39			2039.53	2039.53	0.002257	0.002257	3.83 3.81	3.83 3.81	540.68 810.20	540.68 809.56	754.43	754.14	0.32	0.32
Cooper Creek	7708	10 YR	330.24	330.24	2030.54	2030.54	2039.80	2039.80			2039.91	2039.91	0.002139	0.002143	3.48	3.48	94.80	94.79	28.47	28.47	0.32	0.32
Cooper Creek	7708	50 YR	970.27	970.27	2030.54	2030.54	2039.00	2039.00			2039.12	2039.12	0.001799	0.002343	3.54	3.54	575.16	575.16	540.49	540.49	0.30	0.30
Cooper Creek	7708	100 YR	1372.57	1372.57	2030.54	2030.54	2039.38	2039.37			2039.50	2039.49	0.001993	0.002013	3.78	3.80	825.56	821.59	793.63	791.26	0.31	0.31
Cooper Creek	7417	10 YR	330.24	330.24	2029.87	2029.87	2036.51	2036.51			2036.59	2036.59	0.001314	0.001315	2.24	2.24	147.32	147.29	54.05	54.05	0.24	0.24
Cooper Creek	7417	50 YR	970.27	970.27	2029.87	2029.87	2038.71	2038.71			2038.79	2038.79	0.001061	0.001061	2.62	2.62	627.16	627.16	614.49	614.49	0.23	0.23
Cooper Creek	7417	100 YR	1372.57	1372.57	2029.87	2029.87	2039.04	2039.03			2039.14	2039.13	0.001326	0.001331	3.02	3.03	873.12	867.28	903.72	901.21	0.26	0.26
Cooper Creek	7197	10 YR	330.24	330.24	2029.07	2029.07	2036.03	2036.02			2036.20	2036.20	0.002358	0.00236	3.35	3.35	98.66	98.63	29.26	29.26	0.32	0.32
Cooper Creek	7197	50 YR	970.27	970.27	2029.07	2029.07	2038.13	2038.13	2035.87	2035.87	2038.41	2038.41	0.003667	0.003667	4.64	4.64	370.27	370.27	652.98	652.98	0.42	0.42
Cooper Creek	7197	100 YR	1372.57	1372.57	2029.07	2029.07	2038.49	2038.48			2038.70	2038.69	0.003784	0.003813	4.54	4.55	675.81	667.66	956.64	951.80	0.42	0.42
Cooper Creek	6865	10 YR	330.24	330.24	2028.31	2028.31	2035.47	2035.46			2035.56	2035.55	0.001444	0.001448	2.68	2.68	172.33	172.09	100.51	100.41	0.25	0.25
Cooper Creek	6865	50 YR	970.27	970.27	2028.31	2028.31	2037.69	2037.69			2037.74	2037.74	0.001039		2.35	2.35	771.77	771.67	828.22	828.20	0.22	0.22
Cooper Creek	6865	100 YR	1372.57	1372.57	2028.31	2028.31	2038.11	2038.05			2038.14	2038.09	0.000848		2.28	2.39	1124.35	1076.41	894.99	882.97	0.20	0.21
Cooper Creek	6547	10 YR	330.24	330.24	2027.97	2027.97	2034.89	2034.89	2024 47	2024 47	2035.05	2035.05	0.001551	0.001554	3.22	3.22	102.65	102.55	27.88	27.86	0.30	0.30
Cooper Creek Cooper Creek	6547 6547	50 YR 100 YR	970.27 1372.57	970.27 1372.57	2027.97	2027.97	2036.72	2036.71	2034.47	2034.47	2037.13	2037.12	0.003661	0.003703	5.36 6.92	5.39 6.09	275.31 348.97	272.18 451.46	375.83 528.31	366.74 663.76	0.47 0.60	0.47
Cooper Creek	6080	10 YR	330.24	330.24	2027.97	2027.97	2034.06	2034.06	2033.33	2033.33	2037.32	2034.21	<del> </del>		3.10	3.10	107.39	107.09	41.75	41.29	0.30	0.30
Cooper Creek	6080	50 YR	970.27	970.27	2027.07	2027.07	2035.83	2035.85			2035.93	2035.94	0.001703	0.001653	3.11	3.07	671.38	681.35	807.11	810.88	0.28	0.28
Cooper Creek	6080	100 YR	1372.57	1372.57	2027.07	2027.07	2036.46	2036.15			2036.51	2036.23		0.001676	2.47	3.26	1212.63	936.37	878.30	868.27	0.21	0.29
Cooper Creek	5830	10 YR	330.24	330.24	2026.65	2026.65	2033.43	2033.42			2033.63	2033.62	0.002628	0.00265	3.58	3.59	92.35	92.03	26.52	26.46	0.34	0.34
Cooper Creek	5830	50 YR	970.27	970.27	2026.65	2026.65	2035.40	2035.20		2033.37	2035.52	2035.43	0.002403	0.004151	3.56	4.50	621.56	438.90	966.92	821.64	0.33	0.43
Cooper Creek	5830	100 YR	1372.57	1372.57	2026.65	2026.65	2036.34	2035.83			2036.36	2035.90	0.000524	0.001581	1.95	3.12	1593.44	1051.66	1067.06	1041.19	0.16	0.28
Cooper Creek	5610	10 YR	330.24	330.24	2026.14	2026.14	2033.01	2032.99			2033.13	2033.12	0.00176	0.00179	2.84	2.86	125.42	124.21	65.99	65.24	0.28	0.28
Cooper Creek	5610	50 YR	970.27	970.27	2026.14	2026.14	2035.27	2035.00			2035.30	2035.04	0.000449	0.000757	1.86	2.32	1170.45	948.43	845.38	804.58	0.15	0.20
Cooper Creek	5610	100 YR	1372.57	1372.57	2026.14	2026.14	2036.28	2035.69			2036.29	2035.71	0.000201	0.000453	1.42	1.98	2110.62	1534.96	1018.30	930.12	0.11	0.16
Cooper Creek	5559	10 YR	330.24	330.24	2025.84	2025.84	2032.97	2032.96	2029.91	2029.91	2033.05	2033.03	0.001053	0.001049	2.24	2.07	147.70	159.24	163.00	161.83	0.22	0.22
Cooper Creek	5559	50 YR	970.27	970.27	2025.84	2025.84	2035.00	2034.77	2032.12	2032.20	2035.23	2034.97	0.001813	0.001752	3.90	3.59	252.93	281.21	736.57	721.60	0.31	0.30
Cooper Creek	5559	100 YR	1372.57	1372.57	2025.84	2025.84	2035.90	2035.34	2032.78	2032.75	2036.23	2035.64	0.002041	0.002293	4.63	4.45	302.67	323.59	912.46	810.30	0.34	0.35
Cooper Creek	5521		Bridge	Bridge																		

Standard Table for Cooper Creek





SCALE:	N/A		SH	HEET	5	OF	6
FHWA DIVISION	PF	ROJECT NO	•	ΗI	GHWA	AA NO	
6	SEE	TITLE SH	IEET		CR	316	
STATE		COUNT		SH	EET N	١٥.	
TEXAS		KENT					
DISTRICT	CONTROL	SECTION	ов 43				
ABL	0908	27	000	ô			

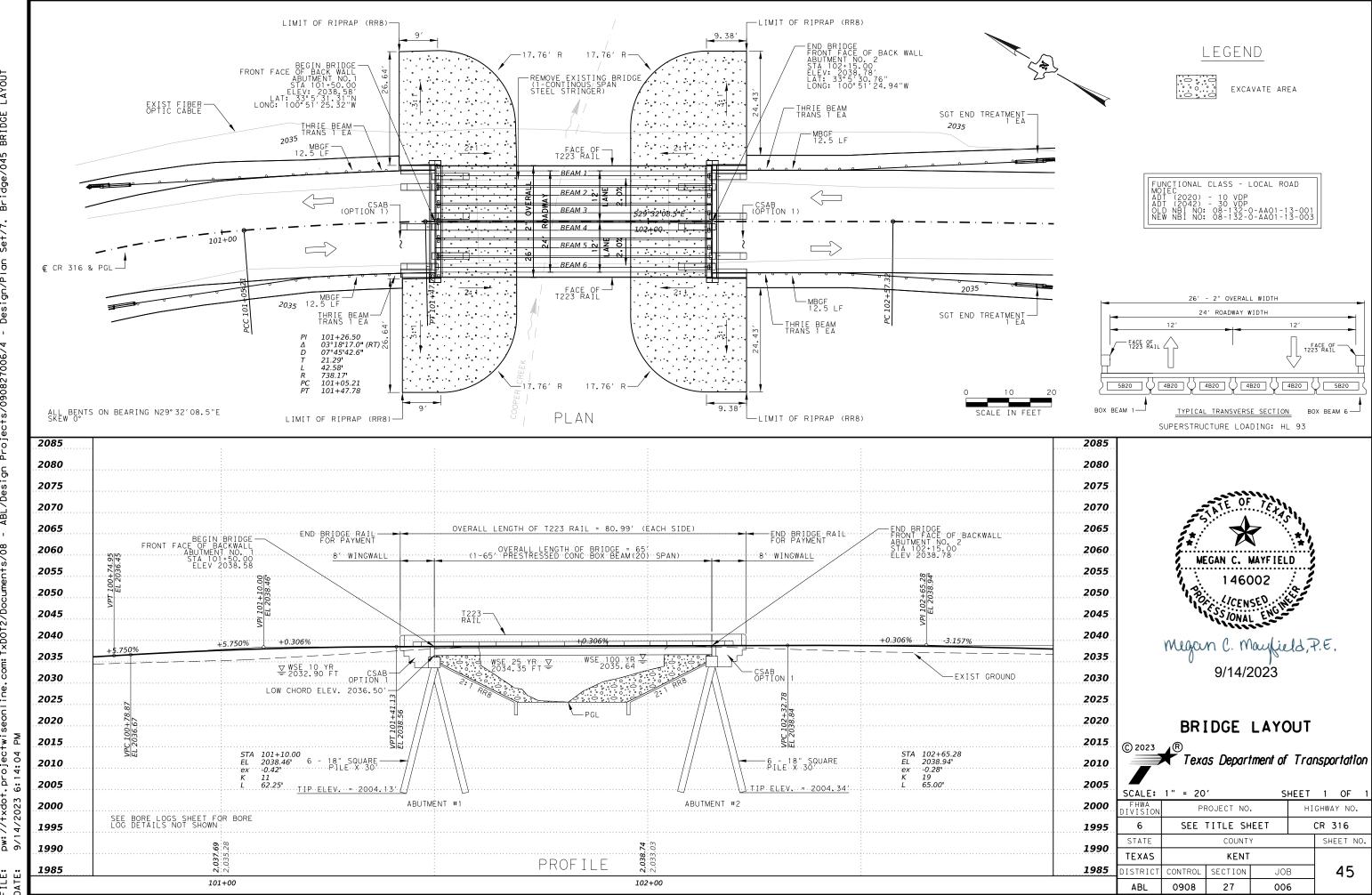
			Q To	otal	Min Ch	Elev	w.s.	Elev	Crit	W.S.	E.G.	Elev	E.G.	Slope	Vel C	hnl	Flow	Area	Top \	Vidth	Froude	e#Chl
Reach	River	Profile	(cf	s)	(f	t)	(f	t)	(f	t)	(f	t)	(ft	/ft)	(ft/	s)	(sq	ft)	(f	t)	11000	
	Sta		CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP	CR316 EX	CR316 PROP
Cooper Creek	5486	10 YR	330.24	330.24	2026.03	2026.03	2032.73	2032.76	2029.91	2029.91	2032.89	2032.90	0.001951	0.002328	3.22	3.03	102.41	108.86	45.50	47.77	0.30	0.32
Cooper Creek	5486	50 YR	970,27	970.27	2026.03	2026.03	2034.08	2034.21	2032.35	2032,54	2034.72	2034.65	0.006724	0.005149	6.46	5.41	157.78	195,38	418.17	451.11	0.57	0.50
Cooper Creek	5486	100 YR	1372.57	1372.57	2026.03	2026.03	2033.91	2034.32	2033.71	2033.58	2035.33	2035.13	0.015714	0.009226	9.60	7.38	148.54	203.60	381.27	499.17	0.87	0.67
Cooper Creek	5460	10 YR	330.24	330.24	2025.84	2025.84	2032.65	2032.65			2032.83	2032.83	0.002868	0.002868	3.39	3.39	99.00	99.00	47.30	47.30	0.35	0.35
Cooper Creek	5460	50 YR	970.27	970.27	2025.84	2025.84	2034.38	2034.38			2034.46	2034.46	0.001819	0.001819	3.12	3.12	626.27	626.27	574.06	574.06	0.29	0.29
Cooper Creek	5460	100 YR	1372.57	1372.57	2025.84	2025.84	2034.73	2034.73			2034.82	2034.82	0.00182	0.00182	3.33	3.33	840.95	840.95	629.25	629.25	0.30	0.30
Cooper Creek	4952	10 YR	330.24	330.24	2023.99	2023.99	2030.45	2030.45			2030.68	2030.68	0.00683	0.00683	3.84	3.84	85.98	85.98	30.11	30.11	0.40	0.40
Cooper Creek	4952	50 YR	970.27	970.27	2023.99	2023.99	2033.18	2033.18			2033.33	2033.33	0.004108	0.004108	3.46	3.46	383.90	383.90	358.18	358.18	0.33	0.33
Cooper Creek	4952	100 YR	1372.57	1372.57	2023.99	2023.99	2033.76	2033.76			2033.88	2033.88	0.003022	0.003022	3.30	3.30	629.53	629.53	485.34	485.34	0.29	0.29
Cooper Creek	4629	10 YR	330.24	330.24	2022.98	2022.98	2029.10	2029.10			2029.29	2029.29	0.0029	0.0029	3.51	3.51	94.10	94.10	31.46	31.46	0.36	0.36
Cooper Creek	4629	50 YR	970.27	970.27	2022.98	2022.98	2031.75	2031.75			2032.09	2032.09	0.003447	0.003447	4.75	4.75	238.36	238.36	123.54	123.54	0.41	0.41
Cooper Creek	4629	100 YR	1372.57	1372.57	2022.98	2022.98	2032.49	2032.49			2032.85	2032.85	0.003464	0.003464	5.14	5.14	377.01	377.01	230.88	230.88	0.42	0.42
Cooper Creek	4424	10 YR	330.24	330.24 970.27	2022.54	2022.54	2028.30	2028.30			2028.54	2028.54	0.004747	0.004747	3.95	3.95	83.66	83.66	33.93	33.93	0.44	0.44
Cooper Creek	4424 4424	50 YR 100 YR	970.27 1372.57	1372.57	2022.54	2022.54	2030.97	2030.97			2031.32	2031.32	0.00404	0.00404	4.82 5.21	4.82 5.21	227.71 361.70	227.71 361.70	114.99 232.33	114.99 232.33	0.44	0.44
Cooper Creek Cooper Creek	3946	100 TK	330.24	330.24	2022.34	2022.54	2026.80	2026.80			2026.95	2026.95	0.003887	0.003887	3.08	3.08	107.13	107.13	36.69	36.69	0.43	0.43
Cooper Creek	3946	50 YR	970.27	970.27	2020.20	2020.20	2020.80	2020.80			2020.95	2020.95	0.002382	0.002382	3.98	3.98	279.98	279.98	117.00	117.00	0.32	0.32
Cooper Creek	3946	100 YR	1372.57	1372.57	2020.20	2020.20	2030.51	2030.51			2030.79	2030.79	0.002000	0.002080	4.47	4.47	378.72	378.72	126.93	126.93	0.34	0.32
Cooper Creek	3659	10 YR	330.24	330.24	2019.75	2019.75	2026.15	2026.15			2026.30	2026.30	0.002181	0.002181	3.11	3.11	106.04	106.04	34.41	34.41	0.34	0.34
Cooper Creek	3659	50 YR	970.27	970.27	2019.75	2019.75	2029.02	2029.02			2029.22	2029.22	0.003048	0.003048	3.61	3.61	269.13	269.13	93.31	93.31	0.37	0.37
Cooper Creek	3659	100 YR	1372.57	1372.57	2019.75	2019.75	2029.83	2029.83			2030.08	2030.08	0.002881	0.002881	3.94	3.94	348.01	348.01	101.13	101.13	0.37	0.37
Cooper Creek	3352	10 YR	330.24	330.24	2019.87	2019.87	2025.10	2025.10			2025.32	2025.32	0.005008	0.005008	3.74	3.74	88.23	88.23	41.63	41.63	0.45	0.45
Cooper Creek	3352	50 YR	970.27	970.27	2019.87	2019.87	2028.17	2028.17			2028.39	2028.39	0.002427	0.002427	3.80	3.80	272.81	272.81	122.08	122.08	0.35	0.35
Cooper Creek	3352	100 YR	1372.57	1372.57	2019.87	2019.87	2029.11	2029.11			2029.35	2029.35	0.002022	0.002022	4.04	4.04	402.03	402.03	149.05	149.05	0.33	0.33
Cooper Creek	3039	10 YR	330.24	330.24	2017.97	2017.97	2023.93	2023.93			2024.14	2024.14	0.002961	0.002961	3.60	3.60	91.83	91.83	29.54	29.54	0.36	0.36
Cooper Creek	3039	50 YR	970.27	970.27	2017.97	2017.97	2027.36	2027.36			2027.62	2027.62	0.002476	0.002476	4.06	4.06	244.78	244.78	87.20	87.20	0.35	0.35
Cooper Creek	3039	100 YR	1372.57	1372.57	2017.97	2017.97	2028.42	2028.42			2028.70	2028.70	0.002084	0.002084	4.33	4.33	368.05	368.05	128.12	128.12	0.33	0.33
Cooper Creek	2633	10 YR	330.24	330.24	2015.84	2015.84	2022.55	2022.55			2022.81	2022.81	0.003622	0.003622	4.03	4.03	82.03	82.03	24.00	24.00	0.38	0.38
Cooper Creek	2633	50 YR	970.27	970.27	2015.84	2015.84	2026.05	2026.05			2026.42	2026.42	0.003492	0.003492	4.93	4.93	196.81	196.81	43.07	43.07	0.41	0.41
Cooper Creek	2633	100 YR	1372.57	1372.57	2015.84	2015.84	2027.39	2027.39			2027.69	2027.69	0.002993	0.002993	4.62	4.62	406.08	406.08	283.69	283.69	0.38	0.38
Cooper Creek	2261	10 YR	330.24	330.24	2014.59	2014.59	2021.56	2021.56			2021.74	2021.74	0.002229	0.002229	3.39	3.39	97.41	97.41	26.45	26.45	0.31	0.31
Cooper Creek	2261	50 YR	970.27	970.27	2014.59	2014.59	2025.04	2025.04			2025.32	2025.32	0.002444	0.002444	4.25	4.25	228.16	228.16	49.29	49.29	0.35	0.35
Cooper Creek	2261	100 YR	1372.57	1372.57	2014.59	2014.59	2026.40	2026.40			2026.72	2026.72	0.002288		4.52	4.52	313.48	313.48	87.51	87.51	0.35	0.35
Cooper Creek	1906	10 YR	330.24	330.24	2013.74	2013.74	2020.81	2020.81			2020.97		0.002119		3.20	3.20	103.24	103.24	30.07	30.07	0.30	0.30
Cooper Creek	1906	50 YR	970.27	970.27	2013.74	2013.74	2024.23	2024.23			2024.48	2024.48	0.002241		3.97	3.97	244.27	244.27	55.56	55.56	0.33	0.33
Cooper Creek	1906	100 YR	1372.57	1372.57	2013.74	2013.74	2025.63	2025.63			2025.89	2025.89	0.002262		4.10	4.10	334.75	334.75	74.38	74.38	0.34	0.34
Cooper Creek	1602	10 YR	330.24	330.24	2013.76	2013.76	2019.98	2019.98			2020.20	2020.20	0.002943		3.78	3.78	87.27	87.27	24.87	24.87	0.36	0.36
Cooper Creek	1602	50 YR	970.27	970.27	2013.76	2013.76	2023.37	2023.37			2023.68	2023.68	0.003055		4.46	4.46	217.49	217.49	51.99	51.99	0.38	0.38
Cooper Creek	1602	100 YR	1372.57	1372.57	2013.76	2013.76	2024.84	2024.84			2025.07	2025.07		0.003201	3.80	3.80	361.23	361.23	119.14	119.14	0.38	0.38
Cooper Creek	1240	10 YR	330.24	330.24	2012.97	2012.97	2019.19	2019.19			2019.35	2019.35	0.001878	0.001878	3.19	3.19	103.52	103.52	27.83	27.83	0.29	0.29
Cooper Creek	1240	50 YR	970.27	970.27	2012.97	2012.97	2022.27	2022.27			2022.59	2022.59		0.002951	4.56	4.56	212.88	212.88	48.12	48.12	0.38	0.38
Cooper Creek	1240 896	100 YR 10 YR	1372.57 330.24	1372.57 330.24	2012.97 2013.34	2012.97	2023.55	2023.55	2016.29	2016.29	2023.92 2018.58	2023.92	0.003058 0.00265	0.003058 0.00265	4.86 3.49	4.86 3.49	282.54 94.57	282.54 94.57	60.38 30.06	60.38 30.06	0.40	0.40
Cooper Creek Cooper Creek	896	50 YR	970.27	970.27	2013.34	2013.34	2018.39	2018.39	2018.43	2018.43	2018.58	2018.58	0.00265		5.09	5.09	190.60	190.60	40.91	40.91	0.35	0.35
Cooper Creek	896	100 YR	1372.57	1372.57	2013.34	2013.34	2021.09	2021.09	2019.34	2019.34	2021.30	2021.30	0.003392	0.003392	5.70	5.70	240.96	240.96	46.33	46.33	0.42	0.42
Cooper Creek	565	100 TK	330.24	330.24	2013.34	2013.34	2022.23	2015.58	2019.54	2015.58	2016.49	2016.49		0.003073	7.65	7.65	43.16	43.16	24.12	24.12	1.01	1.01
Cooper Creek	565	50 YR	970.27	970.27	2013.17	2013.17	2017.54	2017.54	2017.54	2017.54	2010.49	2010.49	0.023838		9.92	9.92	97.86	97.86	32.04	32.04	1.00	1.00
Cooper Creek	565	100 YR	1372.57	1372.57	2013.17	2013.17	2017.34	2018.43	2018.43	2018.43	2020.21	2020.21	0.020889		10.71	10.71	128.11	128.11	35.94	35.94	1.00	1.00
Sooper Greek	000	100 110	1012.01	1012.01	2010.17	2010.17	2010.70	2010.70	2010.70	2010.70	2020.21	2020.21	1 3.020003	3.020003	1 10.71	10.71	120.11	120.11	00.04	00.07	1.00	1.50

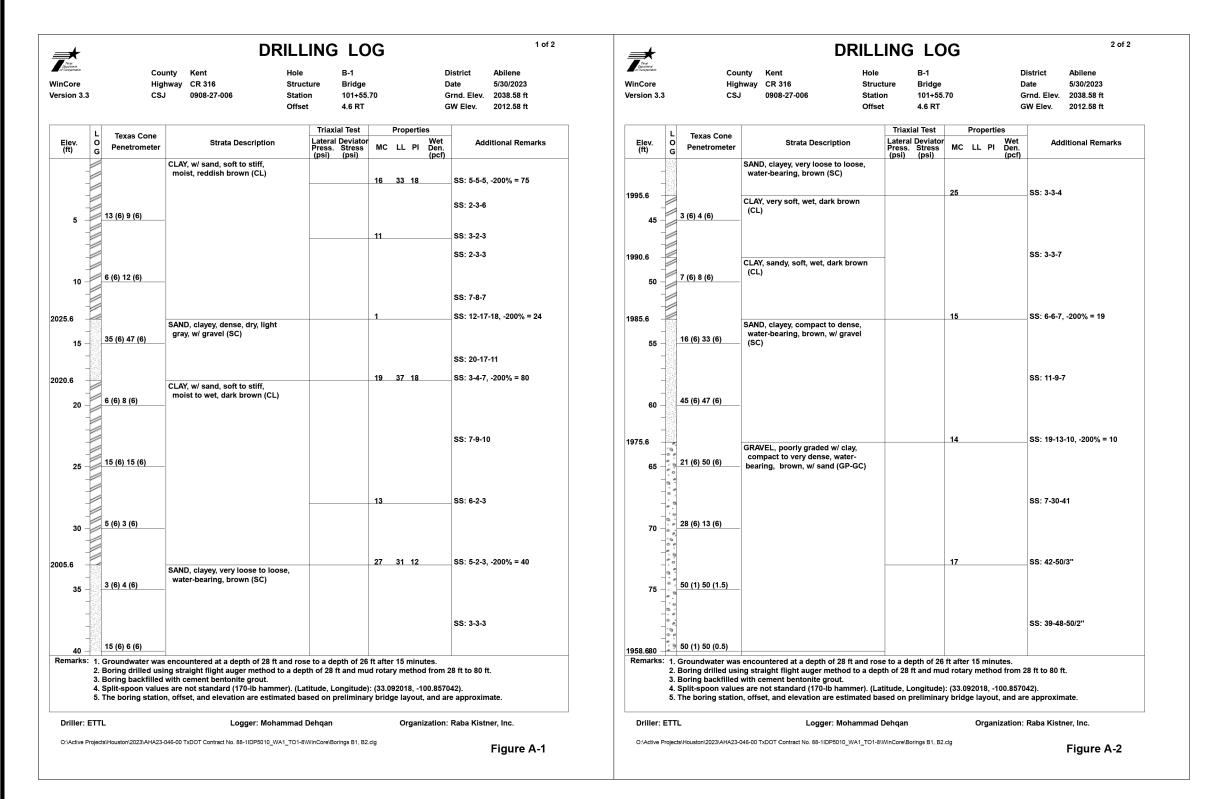
Standard Table for Cooper Creek





SCALE:	N/A			SI	HEET	6	OF	6
FHWA DIVISION	PF	ROJECT	NO.		НΙ	GHW	ON YA	
6	SEE	TITLE		CR	316			
STATE			SH	IEET N	١٥.			
TEXAS								
DISTRICT	CONTROL	3		44				
ABL	0908	27		000	5			



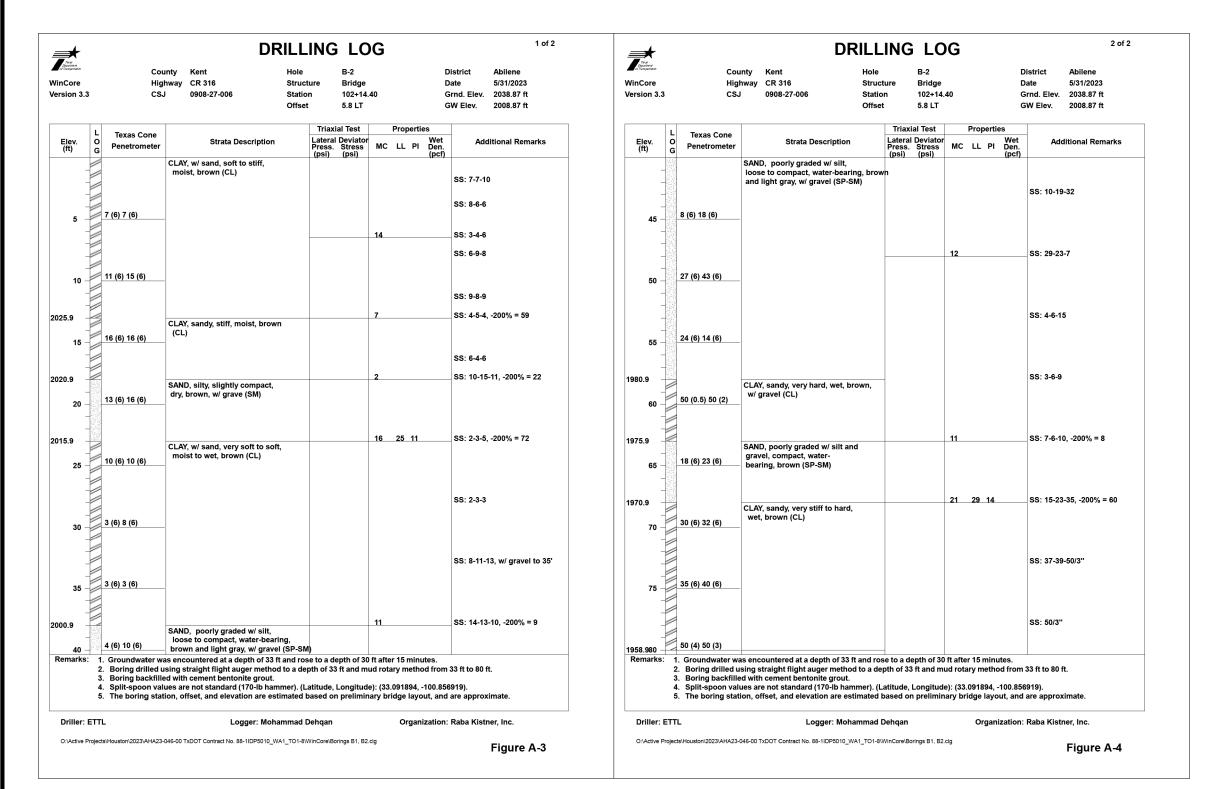




CR 316 SOIL BORING DATA



SCALE:	N/A		SI	HEET	1	OF 2	
FHWA DIVISION	PF	ROJECT NO	•	HIGHWAY NO.			
6	SEE	TITLE SH	IEET	CR 316			
STATE		COUNT		SH	EET NO.		
TEXAS		KENT					
DISTRICT	CONTROL	SECTION	JOI	В	ີ 46		
ABL	0908	27	000	6			





### CR 316 SOIL BORING DATA



_							
SCALE:	N/A		SI	HEET	2	OF	2
FHWA DIVISION	PF	ROJECT N	) <b>.</b>	НΙ	GHWA	AA NO	
6	SEE	TITLE S	CR 316				
STATE		COUN.		SH	EET N	١٥.	
TEXAS		KEN					
DISTRICT	CONTROL	SECTION	JO	В		47	
ABL	0908	27	00	6			

BRIDGE DESCRIPTION

ROADWAY WIDTH: 24 FT SUPERSTRUCTURE TYPE: PRESTRESSED BOX BEAMS BEAM TYPE: B20 ROADWAY TYPE: 5" SLAB RAIL TYPE" TY 223

NUMBER OF SPANS: 1

BENT 1 STA: 101+50.00 BENT 2 STA: 102+15.00 SPAN 1 LENGTH: 65 FT

### Bearing Seats

### Elevation

Elevation				_									
Support Line Name	Bearing Line	Girder1(')	Girder2(')	Girder3(')	Girder4(')	Girder5(')	Girder6(')	Girder7(')	Girder8(')	Girder9(*)	Girder10(')	Girder11(')	Girder12(')
Abutment1	Ahead	#1	#1	#2	#2	#3	#3	#4	#4	#5	#5	#6	#6
		2035.8532	2035.9328	2035.9553	2036.0148	2036.0373	2036.0969	2036.0969	2036.0373	2036.0148	2035.9553	2035.9328	2035.8532
Abutment2	Back	#1	#2	#3	#4	#5	#6				, i		
		2036.1735	2036.2656	2036.3477	2036.3477	2036.2656	2036.1735						

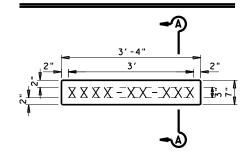


### CR 316 BEARING SEAT ELEVATIONS

© 2023 R
Texas Department of Transportation

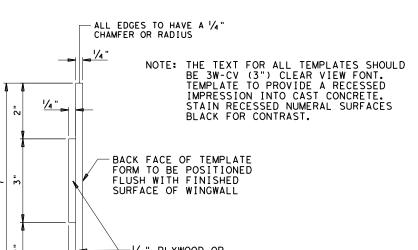
SCALE:	N/A		S	HEET	1 OF 1		
FHWA DIVISION	PF	ROJECT NO	НΙ	GHWAY NO.			
6	SEE	TITLE SH		CR 316			
STATE		SHEET NO.					
TEXAS		KENT					
DISTRICT	CONTROL	SECTION	JOI	3	48		
ABL	0908	27	000	6			

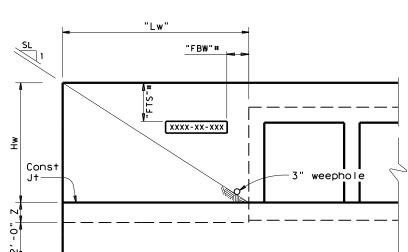
### STRUCTURE ID TEMPLATES



NOTE: THE SYMBOLS XXXX-XX-XXX REPRESENT THE STRUCTURE NUMBER WHICH IS SHOWN IN THE TABLE TO THE RIGHT.

ALL CHARACTERS ARE REQUIRED, AND ARE TO BE FORMATTED EXACTLY AS SHOWN IN THE STRUCTURE NUMBER COLUMN TO



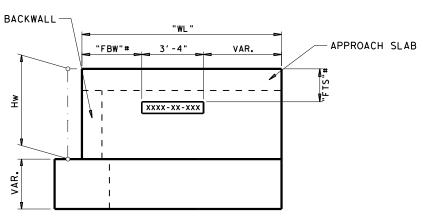


08-132-0-AA01-13-003 | CR 316 OVER COOPER CREEK

LOCATION

OF APPROACHES. THIS PLACES THE ID'S ON DIAGONAL CORNERS. THE STRUCTURE ID'S WILL NOT BE PAID FOR DIRECTLY, BUT WILL BE

CONSIDERED SUBSIDIARY TO THE VARIOUS BRIDGE ITEMS.



"Hw"

2′

VARIOUS

"FTS"#

### WINGWALL ELEVATION

"Lw"

NΑ

STRUCTURE ID TEMPLATE NUMBERS

8′

STRUCTURE NUMBER

AA03-13-003

# FIELD LOCATE TO AVOID CONFLICT WITH REINFORCEMENT AND RIPRAP. THE ENGINEER SHALL APPROVE INSTALLATION LOCATION PRIOR TO PLACEMENT.

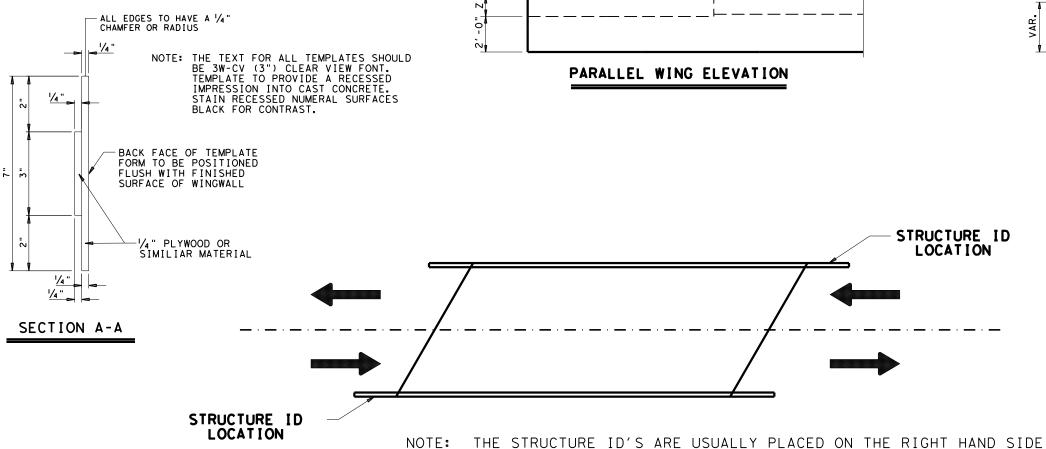


9/14/2023

### STRUCTURE ID DETAILS SIDD-14



NO SCAL	.E		HEET	1	OF	1		
FHWA DIVISION	PF	ROJECT NO	НΙ	GHWA	Y NO.			
6	SEE	-	CR :	316				
STATE			SHEET NO.					
TEXAS	·	KENT		·		•		
DISTRICT	CONTROL SECTION JOB					□ 49		
ABL	0908	27	000	O)				



**NBI NUMBER** 

2'-0" (Typ)

1'-0" (Typ)_

(Typ:

<u>½" (Typ)</u>

1.000'

Beam Spa

Dr Shaft Spa

Parallel to Roadway Surface

72

1'-0" (Typ)

Top of Cap Elev(1)

Outside Edge

5.292

of Beam (Typ)

(Typ)

·C Beam 1

4.604'

14.292

SHOWING DRILLED SHAFTS

Bars V

Spa at 12" Max

Cap Elev 1

Parallel to

Roadway

Surface

4.105

9.000

Face of Backwall, & Cap and & Drilled Shaft or Piles

4.104

28.584

PL AN

See Layout for Slope

€ Beam 6 or 1 —

(Typ)

14.292

4.604

SHOWING BATTERED PILES

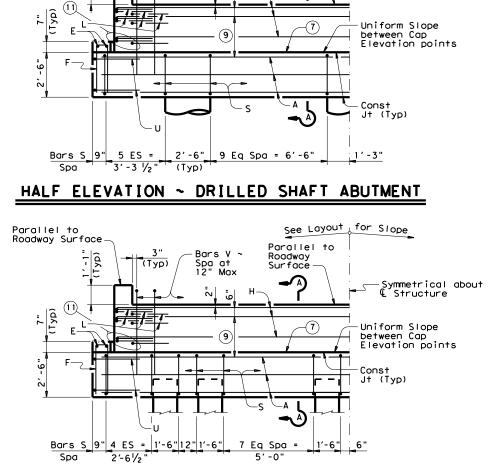
-Begin or End Bridge

ِـا ﴿ Brgs۔

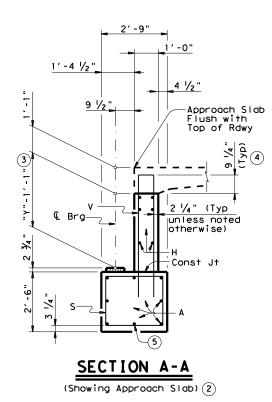
4.105

9.000

- Symmetrical about & Structure



HALF ELEVATION ~ PILE ABUTMENT (Showing 16" Piles ~ for Piles larger than 16" adjust Bars S spacing as required to avoid Pilina)



Earwall (Typ)(10)

Cap Elev (1)

1.000'

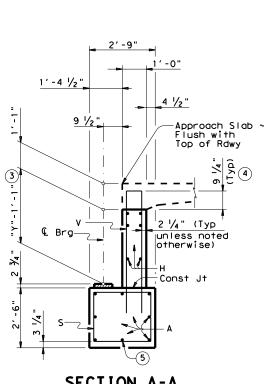
Pile Spa

1'-3"

(Typ)

3.531'

5.292





"WL "								
Beam Type	"WL"							
B20	8.000′							
B28	10.000′							
B34	11.000							

### TABLE OF FOUNDATION LOADS ®

Drilled Shaft Load	Battered Pile Load
Tons/DS	Tons/Pile
50	38
55	41
60	43
64	45
68	47
73	50
77	52
81	54
85	56
89	58
93	60
97	62
101	64
105	66
	Shaft Load Tons/DS 50 55 60 64 68 73 77 81 85 89 93 97

EARWALL ELEVATION DETAIL 10

Backwal

- (Slope top of earwall away from beams)
  - (1) Top of Cap Elevations are based on section depths shown on Span Details.
  - (2) See Bridge Layout for Joint type and to determine if Approach Slab is present.
  - 3 See Span details for "Y" value.

Const Jt

- $\stackrel{ ext{\scriptsize 4}}{}$  Increase as required to maintain 3  $rac{ ext{\scriptsize 3}}{ ext{\scriptsize 4}}$ " from Finished Grade.
- $^{\left(5\right)}$  With pile foundations, replace Bar A, located at bottom centerline of cap with 2  $\sim$  #11 x 5'-0" bars placed between pile groups. Deduct 93 Lbs from reinforcing steel total.
- $^{(6)}$ ½" Preformed Bituminous Fiber material between beam and earwall. Bond to beam with an approved adhesive. Inside face of earwall to be cast with
- $\ensuremath{\bigcirc{7}}$  Surface finish for the top of Cap will be a textured wood float finish. The surface must be level in the direction of the centerline of Beams.
- ${\color{red} 8}$  Foundation loads are based on B34 beams.
- $^{igg(9)}$  Use 2 Eq Spa for B28 and B34 beams. Use 1 space for B20 beams.
- $\stackrel{\textstyle \textcircled{\scriptsize 10}}{\textstyle }$  Do not cast earwalls until beams are erected in their final position.
- (1) This set of Bars L only required for B28 and B34 beams.

### GENERAL NOTES:

Designed according to AASHTO LRFD Specifications.
Concrete strength f'c = 3,600 psi.
All reinforcing must be Grade 60.
Designed for normal embankment header slope of 3:1 or 2:1.

See Bridge Layout for beam type and foundation type, size and length.

See standard FD for all foundation details and notes. See applicable rail details for rail anchorage cast in

HL93 LOADING

wingwalls.
See standard CRR for riprap attachment details, if applicable.
These abutment details may be used only with the following

SBS-B20-24 or SBB0-B20-24 SBBS-B28-24 or SBB0-B28-24 SBBS-B34-24 or SBB0-B34-24

SHEET 1 OF 2



Bridge Division Standard

**ABUTMENTS** PRESTR CONC BOX BEAMS 24' RDWY

**ABB-24** 

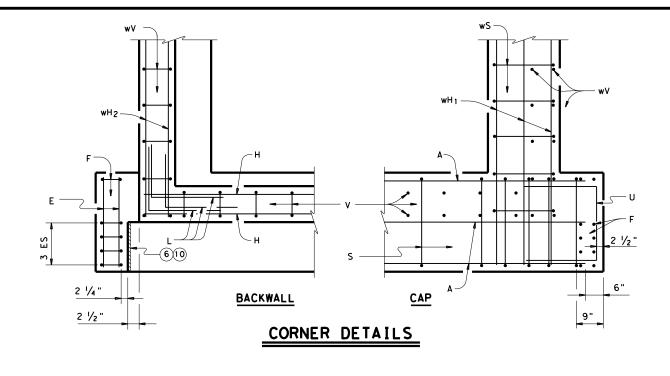
				-			
FILE: bbstde17.dgn	DN: TXL	DOT	CK: TXDOT	DW:	TxD0T	ck: TxD0T	
◯TxDOT December, 2006	CONT	SECT	JOB		HIG	iHWAY	
REVISIONS	0908	27	006	CR	CR 316		
04-11: Span length.	DIST	ST COUNTY			SHEET NO.		
	ABL		KENT			50	

Surface -Const Jt

Roadway

BACKWALL DETAIL

(Without Approach Slab)(2)



	QUANTITIES (TYPE B20 BEAMS)12							JANT	ITIES 8 BE	
BAR	NO.	SIZE	LENGTH	WEIGHT		BAR	NO.	SIZE	LENGT	
A (5)	8	#11	27' - 7"	1,172	A	5	8	#11	27' - 7	
E	4	# 5	2′ - 5"	10	E		4	# 5	2' - 5	
F	10	# 5	6′ - 1"	63	F		10	<b>#</b> 5	6′ - 1	
Н	4	# 6	25′-10"	155	Н		6	# 6	25′-10	
L	12	# 6	4'- 0"	72	L		18	# 6	4' - C	
S	32	# 4	9′ - 8"	207	S		32	# 4	9′ - 8	
U	4	# 6	7′ - 3"	44	U	1	4	# 6	7' - 3	
٧	25	# 5	7′ - 6"	191	٧		25	# 5	8' - 9	
wH 1	14	# 6	9′- 0"	189	wH	1	14	# 6	11'- C	
wH 2	12	# 6	7′ - 8"	138	wH	2	16	# 6	9′-8	
wS	18	# 4	7′ - 9"	93	wS		22	# 4	7'- 9	
wV	18	# 5	7′ - 9"	145	w۷		22	# 5	9′- 0	
Reinforc	Reinforcing Steel Lb					Reinforcing Steel				
Class "C	" Conc	rete	(w/Slab) CY	12.6	С	Class "C" Concrete (w/Slab)				
Class "C	" Conc	rete	(w/ACP) CY	12.3	С	lass "C	" Cond	crete	(w/ACP)	

TABLE OF ESTIMATED

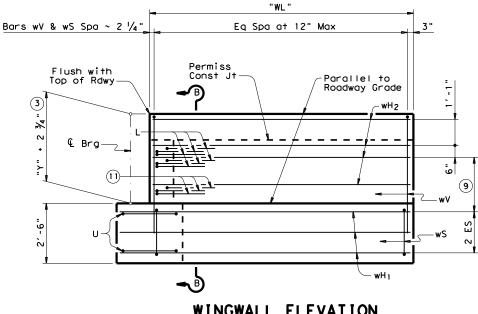
_	QL	JANT	STIM ITIES 8 BE	S			QU	ANT	STIM/ ITIES 4 BEA		_
BAR	NO.	SIZE	LENG	ГН	WEIGHT	BAR	NO.	SIZE	LENGT	Ή	WEIGHT
A (5)	8	#11	27' - 7	7 "	1,172	A (5)	8	#11	27' - 7		1,172
Ε	4	# 5	2' - 5	5"	10	Ε	4	# 5	2' - 5	."	10
F	10	# 5	6' - 1	ı "	63	F	10	# 5	6' - 1	"	63
Н	6	# 6	25′-10	)"	233	Н	6	# 6	25′-10	)"	233
L	18	# 6	4' - (	)"	108	L	18	# 6	4' - 0	)"	108
S	32	# 4	9′ - 8	3"	207	S	32	# 4	9′ - 8	3"	207
U	4	<b>#</b> 6	7' - 3	3"	44	U	4	# 6	7' - 3	5"	44
٧	25	# 5	8' - 9	9"	226	٧	25	# 5	9′-10	)"	254
wH 1	14	<b>#</b> 6	11'- (	)"	231	wH 1	14	# 6	12'- 0	)"	252
wH 2	16	# 6	9′ - 8	3"	232	wH 2	16	# 6	10' - 8	3"	256
wS	22	# 4	7' - 9	9"	114	wS	24	# 4	7'- 9	)"	124
wV	22	# 5	9'- (	)"	207	wV	24	# 5	10' - 1		252
Reinforc	ing St	eel		Lb	2,847	Reinforc	ing St	ee I		Lb	2,975
Class "C	" Cond	rete	(w/Slab)	CY	14.7	Class "C	" Cond	crete	(w/Slab)	CY	16.2

- 3 See Span details for "Y" value.
- $\stackrel{\textstyle (5)}{}$  With pile foundations, replace Bar A, located at bottom centerline of cap, with 2  $\sim$  #11 x 5'-0" bars placed between pile groups. Deduct 93 Lbs from reinforcing steel total.
- $^{\Large{(6)}}$   $/\!\!\!/_2$  " Preformed Bituminous Fiber material between beam and earwall. Bond to beam with an approved adhesive. Inside face of earwall to be cast with vertical side of beam.
- $^{igg(9)}$  Use 2 Eq Spa for B28 and B34 beams and 1 space for B20 beams.

14.4

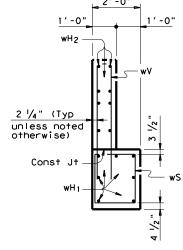
CY

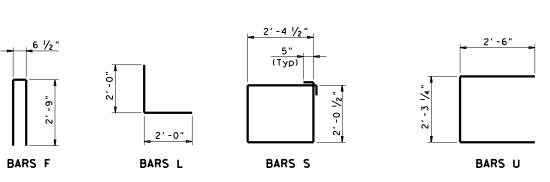
- $\stackrel{\textstyle \bigcirc}{\textstyle \bigcirc}$  Do not cast earwalls until beams are erected in their final position.
- 1) This set of Bars L only required for B28 and B34 beams.
- Quantities shown are for one Abutment only (with Approach Slab). With no Approach Slab, add 1.0 CY Class "C" concrete and 78 Lb reinforcing steel for 2 additional Bars H.

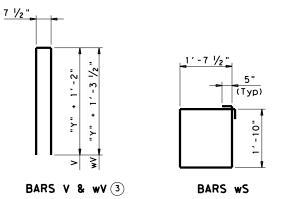


## WINGWALL ELEVATION

(Earwall omitted for clarity)







Bridge Division Standard Texas Department of Transportation **ABUTMENTS** PRESTR CONC BOX BEAMS 24' RDWY **ABB-24** bbstde17.dgn

SECTION B-B

Class "C" Concrete (w/ACP)

HL93 LOADING

SHEET 2 OF 2

DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO OTxDOT December, 2006 CR 316 0908 27 006 04-11: Span length. KENT

26'-2" Overall Top Width 13'-1" 13'-1" 1'-0" 24'-0" Roadway Width (Nominal) 1'-0" _— Face of Raiι Face of Rail " at Brg & **_**_—⊈ Structure (2) Y" a+ Brg ( Bars T at 1 ½" End See Layout for Slope (4) 12" Max Spa (Typ) Cover (Typ) Permissible Shear Key Const Joint Conc (Typ) 4B20 4B20 4B20 4B20 5B20 5B20 3/4" (6) Box Beam 1 Box Beam 6 ¾" 1 ½" 1 1/2" 1 1/2" 1 1/2 4'-11 3/4" 3'-11 3/4' 3'-11 3/4" 3'-11 3/4" 3'-11 3/4" 4'-11 3/4"

30.000' thru 65.000' Spans

Outside top edge of beam and slab

- T (1)

–⊈ Structure

Outside bottom edge of beam —

-C Box Beam 1

-**©** Bo× Beam 6

PLAN

Outside bottom edge of beam

Face of Bkwl or & Bent

2" End Cover $^{(1)}$ 

(Typ)

End Diaphragm ~ See TYPICAL END DIAPHRAGM SECTIONS for

Bars DT and H placement

Rai

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. E þ

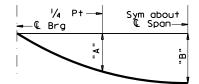
Bars T

بر: مد بر: مد

Layout for a

## TYPICAL TRANSVERSE SECTION

26'-6" Overall Bottom Width



Face of Bkwl or & Bent —

End Diaphragm ~ See TYPICAL END DIAPHRAGM SECTIONS for_

-Outside top edge of beam and slab

Bars DT and H placement

See Layout for Joint type and location (g)

2 3/4"

Note: Deflections shown are due to shear key and concrete slab only, (Ec =  $5 \times 10^3$  ksi). Calculated deflections shown are theoretical and actual dimension may be less. Deflections may be adjusted based on field observation.

### DEAD LOAD DEFLECTION DIAGRAM

	TABLE OF DEFLECTIONS AND SECTION DEPTHS									
SPAN			DEAD LOA	D DEFLECT	IONS (FT)	SECTION	DEPTHS			
LENGTH (FT)	BE AM NO.	POINT	SHEAR KEY	SLAB	TOTAL	"X" AT © BRG	"Y" AT & BRG			
30	ALL	"A"	0.000	0.002	0.002	5 1/4"	2'-1 1/4"			
		"B"	0.001	0.002	0.003	- ,,				
35	ALL	"A"	0.001	0.003	0.004	5 1/4"	2'-1 1/4"			
		"B"	0.001	0.004	0.005	J 74	/4			
40	ALL	"A"	0.002	0.005	0.007	5 1/4"	2'-1 1/4"			
L	~	"B"	0.003	0.007	0.010	5 /4	2 1 /4			
45	ALL	" A "	0.003	0.009	0.012	5 1/2"	2'-1 1/2"			
"	ALL	"B"	0.004	0.012	0.016	3 /2	2 1 /2			
50	ALL	"A"	0.005	0.013	0.018	5 ¾"	2'-1 3/4"			
] 30	~LL	"B"	0.006	0.019	0.025	J /4	2 -1 /4			
55	ALL	"A"	0.007	0.019	0.026	6 1/4"	2'-2 1/4"			
33	^	"B"	0.010	0.027	0.037	0 /4	c -c 74			
60	ALL	"A"	0.010	0.028	0.038	6 ¾"	2'-2 3/4"			
00	^	"B"	0.014	0.039	0.053	0 74	2 -2 74			
65	ALL	"A"	0.013	0.039	0.052	7"	2′-3"			
65	ALL	"B"	0.019	0.054	0.073	ı	2 -3			

- (1) If multi-span units (with slab continuous over Interior Bents) are indicated on the Bridge Layout, Bars T must be continuous through joint. See Continuous Slab Detail.
- 2 Based on theoretical beam camber, dead load deflections of 5" Cast-in-place slab, shear key dead load and a constant grade. The contractor must adjust these values for any vertical
- $\stackrel{\textstyle \bigcirc}{3}$  Slab thickness at midspan of Beams may not exceed 7 inches.
- $\stackrel{ullet}{ ext{4}}$  This standard does not provide for changes in roadway cross slopes within the structure.
- $^{(5)}$  If using Type A expansion joints, the maximum distance between joints is 100 feet.
- $^{(6)}$ Form bottom of shear keys with foam backer rod or other material acceptable to the Engineer.

BAR	TABLE
BAR	SIZE
Α	#4
DT	#4
Н	#5
T	#4

**GENERAL NOTES:** 

Designed according to AASHTO LRFD Specifications.

Provide Class S concrete (f'c = 4,000 psi) for slab and shear key.

Provide Class S (HPC) concrete if shown elsewhere in the plans. All reinforcing must be Grade 60.

Two-span or three-span units, with the slab continuous over Interior Bents, may be formed with the details on this standard. Unit Length cannot exceed 3.5 times length of the shortest end span.

Bar laps, where required, will be as follows:

Uncoated ~ #4 = 1'-5"

Epoxy coated ~ #4 = 2'-1"

It is recommended, with crown cross-slope, to erect beams adjacent to crown point first. For structures without a crown point, it is recommended to erect beams on the high side of cross-slope first and progress to the low side.

This sheet does not support the use of Transition Bents. See railing details and standard BBRAS for rail anchorage.

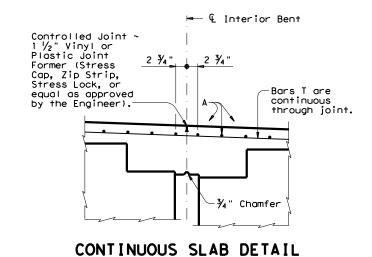


PRESTRESSED CONCRETE BOX BEAM SPANS TYPE B20 24' RDWY (WITH SLAB)

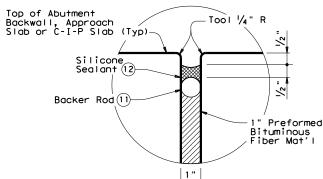
SBBS-B20-24

ILE: bbstds19.dgn	DN: TXE	OT	ck: TxD0T	DW:	TxD0T	ck: TxD0T
C)TxD0T December, 2006	CONT	SECT	JOB		ніс	HWAY
REVISIONS 01-12: Cover.	0908	27	006		CR	316
10-15: Table of Est Quantities, Notes.	DIST		COUNTY		SHEET NO.	
	ABI		KENT			52

(along centerline of Box Beam)



(Diaphragm reinforcing not shown for clarity)



TYPE A JOINT DETAIL 5

TABLE OF ESTIMATED QUANTITIES PRESTR CONCRETE BOX BEAMS (TY 4B20) PRESTR CONCRETE BOX BEAMS (TY 5B20) TOTAL REINF STEEL REINF CONC SLAB (BOX BEAM) SPAN LENGTH SHEAR (14) (13) (13) FΤ SF LF LF CY Lb 30 4.0 785 118.00 59.00 1,570 35 4.6 916 138.00 69.00 1,832 5.3 1,047 40 158.00 79.00 2,094 45 6.0 1,177 178.00 89.00 2,354 2,616 50 6.6 1,308 198.00 99.00 55 7.3 1,439 218.00 109.00 2,878 60 8.0 1,570 238.00 119.00 3,140

258.00

129.00

3,402

1,701

- $^{f{5}}$  If using Type A expansion joints, the maximum distance between joints is 100 ft.
- ${\overline{\mathcal{O}}}$  Slab reinforcing omitted for clarity.
- 8 See Bridge Layout for Joint type.
- $^{\textcircled{9}}$  Provide 1  $\slash\!\!/_2$  " end cover to Bars H. After all beams have been placed, weld one Bar H to two Bars D at each end of all beams.

65

8.6

- (10) Lap Bars DT 9" Min with each Beam Bar D at Interior Bents without Expansion Joints. Bars DT shown bent for clarity only.
- (1) Backer rod must be 25% larger than joint opening and must be compatible with the sealant.
- ② Use Class 7 silicone sealant. Prepare joint and seal in accordance with Item 438 "Cleaning and Sealing Joints".
- ${}^{\scriptsize{\textcircled{\scriptsize{1}}}}$  Fabricator must adjust beam lengths for beam slopes as required.
- $^{oxed{(4)}}$  Reinforcing steel weight is based on an approximate factor of 2.0 lbs per square foot of slab.

HL93 LOADING

SHEET 2 OF 2

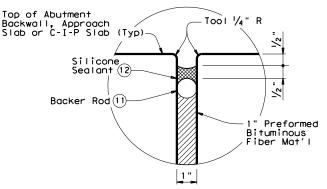
Texas Department of Transportation

Bridge Division Standard

PRESTRESSED CONCRETE BOX BEAM SPANS TYPE B20 24' RDWY (WITH SLAB)

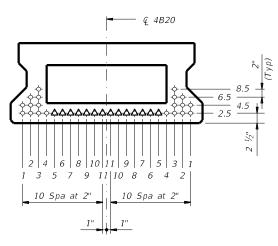
SBBS-B20-24

FILE: bbstds19.dgn	DN: TXE	DOT .	ck: TxD0T	DW:	TxD0T	ck: TxD0T
	CONT	SECT	JOB	DB HIGHWAY		SHWAY
REVISIONS 01-12: Cover.	0908	27	006		CR	316
10-15: Table of Est Quantities, Notes.	DIST COUNTY				SHEET NO.	
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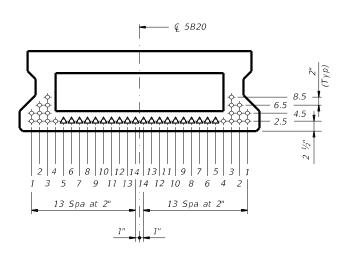


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		9/14/2023 6:15:37 PM	pw://txdot.

					L	DESIG	NED E	BEAMS (	STRAIG	HT S	STRANDS	5)										OPTION	AL DESIGI	V	
STANDARD SBBS-B20-24	SPAN LENGTH	BEAM NO.	BEAM TYPE	NON- STD	TOTAL		STRGTH	STRANDS "e" (i	"e" END	TOT NO.	DIST FROM	NO	D STRAN D.OF RANDS		UMBER DEE		TRANE D TO	)S	RELEASE STRGTH	RETE MINIMUM 28 DAY COMP	DESIGN LOAD COMP STRESS	DESIGN LOAD TENSILE STRESS	REQUIRED MINIMUM ULTIMATE MOMENT	DISTR FAC	LOAD IBUTION CTOR
	(ft)			STRAND PATTERN	NO.	(in)	f pu (ksi)	(in)	(in)	DEB	BOTTOM (in)	TOTAL	DE- BONDED	3	6	9	12	15	(1) f'ci (ksi)	STRGTH f'c (ksi)	(TOP ©) (SERVICE I) fct(ksi)	(BOTT Q) (SERVICE III) fcb(ksi)	CAPACITY (STRENGTH I) (ft-kips)	Moment	2) Shear
	30 30	1&6 2 - 5	5B20 4B20		8 6	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	8 6	0	0	0 0	0 0	0 0	0	4.000 4.000	5.000 5.000	0.640 0.693	-0.808 -0.860	704 601	0.454 0.379	0.691 0.511
	35 35	1&6 2 - 5	5B20 4B20		8 6	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	8 6	0	0 0	0 0	0 0	0 0	0	4.000 4.000	5.000 5.000	0.838 0.911	-1.041 -1.111	795 615	0.440 0.367	0.680 0.498
24' Roadway	40 40	1&6 2 - 5	5B20 4B20		10 8	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	10 8	0	0 0	0 0	0 0	0 0	0	4.000 4.000	5.000 5.000	1.061 1.156	-1.297 -1.388	889 712	0.427 0.356	0.671 0.488
5" Slab	45 45	1&6 2 - 5	5B20 4B20		10 10	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	10 10	0	0 0	0 0	0 0	0 0	0	4.000 4.000	5.000 5.000	1.316 1.437	-1.590 -1.706	960 824		0.663 0.481
	50 50	1&6 2 - 5	5B20 4B20		12 12	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	12 12	0	0 0	0 0	0 0	0 0	0	4.000 4.000	5.000 5.000	1.606 1.755	-1.927 -2.070	1147 985		0.655 0.476
	55 55	1&6 2 - 5	5B20 4B20		16 14	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0	2.50 2.50	16 14	0	0 0	0 0	0 0	0 0	0	4.000	5.000 5.000	1.921 2.104	- 2 . 289 - 2 . 464	1344 1157	0.400 0.334	0.649 0.471
	60 60	1&6 2 - 5	5B20 4B20		18 18	0.6 0.6	270 270	7.38 7.31	7.38 7.31	0 2	2.50 2.50	18 18	0 2	0 0	0 2	0 0	0 0	0	4.000	5.000 5.000	2.262 2.487	-2.677 -2.899	1551 1347	0.393 0.333	0.643 0.467
	65 65	1&6 2 - 5	5B20 4B20		24 20	0.6 0.6	270 270	7.38 7.31	7.38 7.31	6 4	2.50 2.50	24 20	6 4	2 0	2 2	0 0	2 2	0	4.000 4.000	5.000 5.800	2.627 2.903	- 3 . 091 - 3 . 368	1769 1551	0.387 0.333	0.638 0.463



TXDOT 4B20 BOX BEAM



TXDOT 5B20 BOX BEAM

#### DESIGN NOTES:

Designed in accordance with AASHTO LRFD Bridge Design Specifications. Prestress losses for the designed beams have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform.

Beam designs are applicable for 5" concrete slabs without overlay and 0 degree

#### FABRICATION NOTES:

Provide Class H concrete.

Provide Grade 60 reinforcing steel bars.
Use low relaxation strands, each pretensioned to 75 percent of fpu.
When shown on this sheet, the Fabricator has the option of furnishing either the designed beam or an approved optional beam design. All optional design submittals and shop drawings must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.

Locate strands for the designed beam as low as possible on the 2" grid system unless a non-standard stand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc. Place strands within a row as follows:

1) Locate a strand in each "!" position.

2) Place strand symmetrically about vertical centerline of box.

3) Space strands as equally as possible across the entire width.
Strand debonding must comply with Item 424.4.2.2.2.4.
Do not debond strands in position "1". Distribute debonded strands equally about the vertical centerline. Decrease debonded lengths working inward, with debonding staggered in each row. Full-length debonded strands are only permitted in positions marked  $\Delta$  .

1 Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

Tension =  $0.24\sqrt{f'ci}$ 

Optional designs must likewise conform.

2 Portion of full HL93.

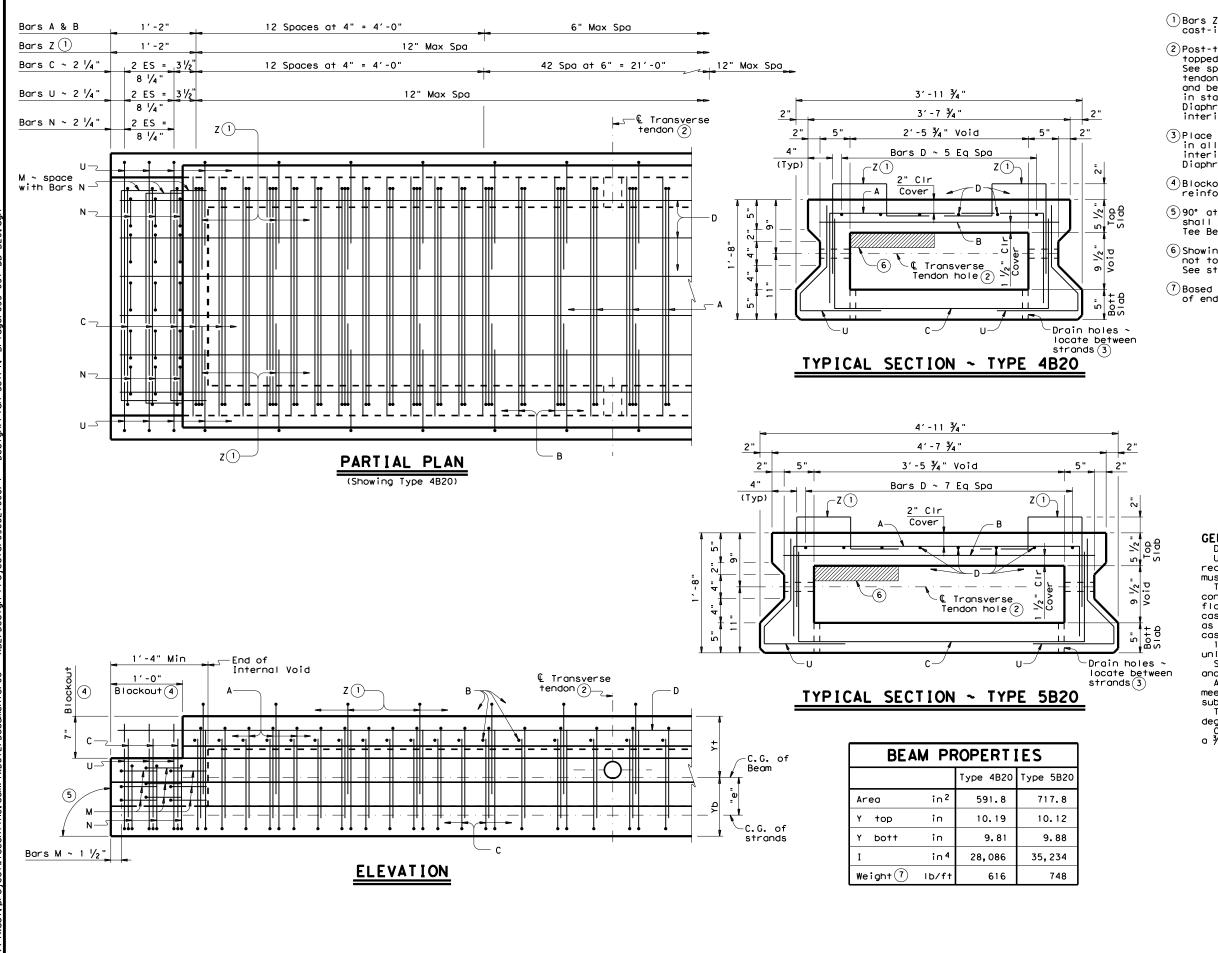
HL93 LOADING



PRESTR CONC BOX BEAM STANDARD DESIGNS 24' RDWY TYPE B20 (WITH SLAB)

BBSDS-B20-24

FILE:	bbstds11.dgn	DN: SF	RW	ск: ВМР	DW:	SFS	ck: SDB
©T x D OT	December 2006	CONT	SECT	JOB		ніс	SHWAY
REVISIONS  04-11: f'ci and LLDF. 01-16: Notes. 0.6" strand designs.		0908	27	006		CR	316
		DIST		COUNTY			SHEET NO.
		ABL	L KENT			54	



 $\ensuremath{\bigcirc}$  Bars Z are required for beams topped with a cast-in-place concrete slab only.

2) Post-tensioning tendons are required for beams not topped with a Min 5" cast-in-place concrete slab. See span details for number and spacing of transverse tendons. Cast interior diaphragms in exterior beams and beams that serve temporarily as exterior beams in staged constructed bridges. See "Blockout, Interior Diaphragm, and Drain Details". Form 3" Dia holes in interior beams. See standard BBPT for details.

(3) Place drain holes (1" Dia PVC Sch 40 Pipe) as shown in all beam void corners including each side of interior diaphragms. See "Blockout, Interior Diaphragm, and Drain Details".

(4) Blockouts required at ends of all beams. Extend beam reinforcement into blockouts.

(5)90° at conventional Interior Bents. Ends of beams shall be vertical at Abutment backwall and Inverted Tee Bent Stems.

6 Showing void modification required in exterior beams not topped with a Min 5" cast-in-place concrete slab. See standard BBRAO for void modification dimensions.

? Based on 150 pcf weight density of concrete. Weight of end blocks and interior diaphragms is not included.

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications.
Use Class H concrete. Use Class H (HPC) if
required elsewhere in plans. All reinforcing steel

must be Grade 60.

Two-stage monolithic casting is required. The concrete in the first stage cast (bottom beam flange) must remain plastic until the second stage cast (webs and top beam flange) is placed. Vibrate as required to ensure consolidation between the two

casts.

1 1/4" clear cover to reinforcement is required unless noted otherwise. See standard BBRAS or BBRAO for railing

anchorage at bridge edges to be cast in beams. An equal area of welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be substituted for Bars A, B, C, and D.

These details are applicable for skews up to 30

degrees only. Chamfer bottom beam corners  $\frac{7}{4}$ " or round to  $a \frac{3}{4}$ " radius.

HL93 LOADING

SHEET 1 OF 3

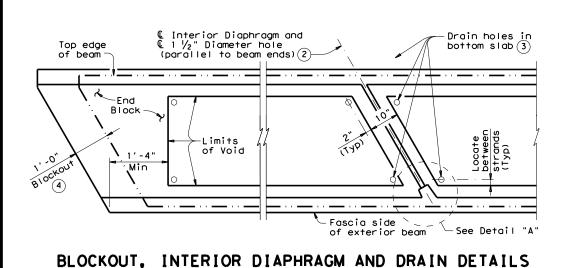
Texas Department of Transportation

Bridge Division Standard

PRESTRESSED CONCRETE BOX BEAM DETAILS (TYPE B20)

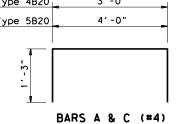
BB-B20

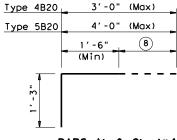
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E: bbstds01.dgn	DN: TxL	TXDOT CK: TXDOT DW: TXDOT		TxD0T	ck: TxD0T	
TxDOT December, 2006	CONT	SECT	JOB	HIGHWA		SHWAY
REVISIONS	0908	27	006	CR	316	
01-12: Bars Z.	DIST		COUNTY			SHEET NO.
	ABL		KENT	•		55

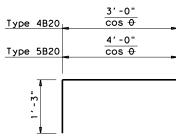


(Showing 30° skew)

3'-0" Type 4B20 <u>Type 5</u>B20 4'-0"

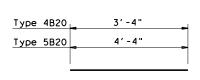






BARS AL & CL (#4)

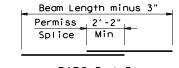
BARS AA & CC (#4)

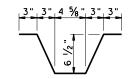


_6 ½"__6 ½"__3"

BARS F (#4)

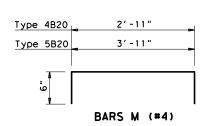
BARS B (#4)

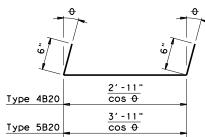




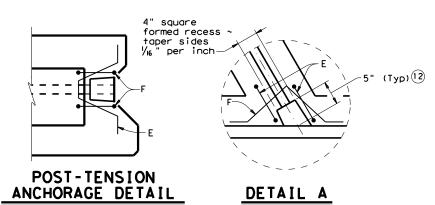
BARS D (#5) Permissible splices to be placed in middle third of span

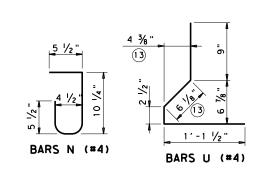
**BARS E (#4)** 

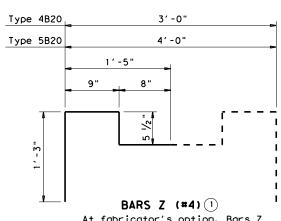




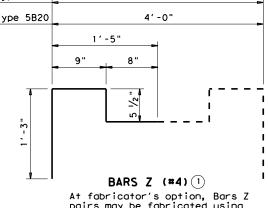
BARS MM (#4)







- (1) Bars Z are required for beams topped with a cast-in-place concrete slab only.
- (2) Post-tensioning tendons are required for beams not topped with a Min 5" cast-in-place concrete slab. See span details for number and spacing of transverse tendons. Cast interior diaphragms in exterior beams and beams that serve temporarily as exterior beams in staged constructed bridges. Form 3" Dia holes in interior beams. See "Blockout, Interior Diaphragm, and Drain Details". See standard BBPT for details.
- (3) Place drain holes (1" Dia PVC Sch 40 Pipe) as shown in all beam void corners including each side of interior diaphragms. See "Blockout, Interior Diaphragm, and Drain Details".
- (4) Blockouts required at ends of all beams. Extend beam reinforcement into blockouts.
- (8) Cut as required to maintain one inch clear between bars.
- (2) 5" (Typ) or sufficient depth to provide 1" Cover on cut-off tendon. See BBPT for
- (13) Dimension will vary slightly with skew. Adjust as necessary.



At fabricator's option, Bars Z pairs may be fabricated using one continuous bar. If this option is used, Bars B at Bar Z locations (only) may be omitted.

HL93 LOADING

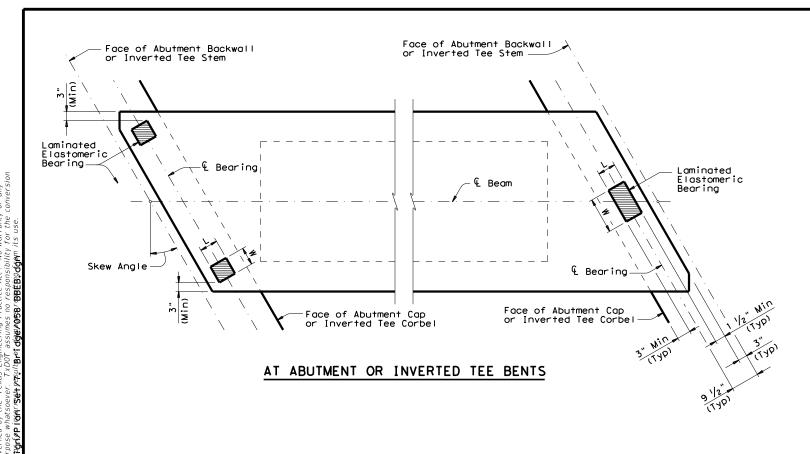
SHEET 3 OF 3

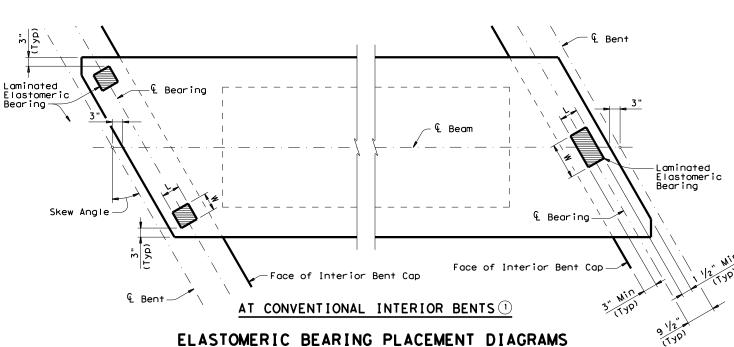
Texas Department of Transportation

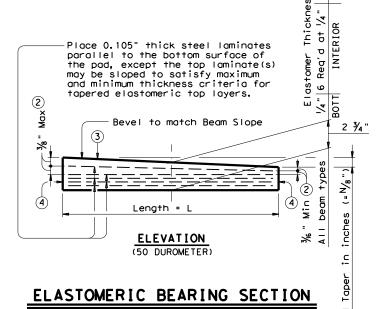
PRESTRESSED CONCRETE BOX BEAM DETAILS (TYPE B20)

BB-B20

01-1	2: Bars Z.	DIST		COUNTY			SHEET NO.	
REVISIONS		0908	27	006	CR	CR 316		
TxD0T	December, 2006	CONT	SECT	JOB		HIGHWAY		
e: bl	ostds01.dgn	DN: TXE	OOT	ck: TxD0T	OT DW: TxDO		ck: TxD0T	







1) For Transition Bents with backwall, beams and elastomeric bearings will receive the same treatment as shown for Abutment Bents.

(50 DUROMETER)
The use of Polyisoprene (natural rubber), for the manufacture of bearing pads, is not permitted.

- ② Maximum and minimum layer thicknesses shown are for elastomer only, on tapered layers.
- 3 Indicate BEARING TYPE on all pads.
  For tapered pads, BEARING TYPE will be located on the high side. The Fabricator will include the value of "N" (amount of taper in ½" increments) in this mark.

  Examples: N=0, (for 0" taper)
  N=1, (for ½" taper)
  N=2, (for ¼" taper)

Fabricated pad top surface slope must not vary from plan beam slope by more than  $\left(\begin{array}{c} 0.0625"\\ \text{Length} \end{array}\right)$  IN/IN.

4 Locate Permanent Mark here.

### ELASTOMETRIC BEARING DIMENSIONS

BEARING	BEAM	ONE BI	EARING	TWO BEARINGS		
TYPE	TYPE	L	W	L	W	
B20-"N"	4B20	6"	12"	6"	6"	
	5B20	6"	12"	6"	6"	
DO0    N.	4B28	6"	14"	6"	7"	
B28-"N"	5B28	6"	14"	6"	7"	
B34-"N"	4B34	6"	16"	6"	8"	
D34- IN	5B34	6"	16"	6"	8"	
D.40	4B40	6"	20"	6"	10"	
B40-"N"	5B40	6"	20"	6"	10"	

GENERAL NOTES:

Set beams on elastomeric bearings of the dimensions shown. Center bearings as near nominal \( \frac{1}{2} \) bearing as possible within limits shown.

Constant thickness bearings may be used for moderate beam slopes up to 0.0113 ft/ft. For skewed supports, Bearings beveled for beam slope may not provide uniform contact. However, predicted contact is considered within allowable tolerances.

Shop drawings for approval are required.
A bearing layout which identifies location and orientation of all bearings will be developed by the bearing fabricator. Permanently mark each bearing in accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer.

Cost of furnishing and installing elastomeric bearings is to be included in unit price bid for "Prestressed Concrete Box Beams".

Details are drawn showing right forward skew.
See Bridge Layout for actual direction.

These details are applicable for skews up to 30 degrees only.

HL93 LOADING



Texas Department of Transportation

Bridge Division Standard

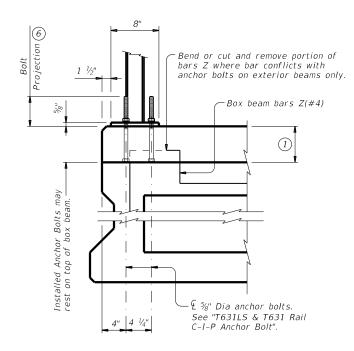
ELASTOMERIC BEARING DETAILS PRESTR CONC BOX BEAMS

**BBEB** 

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xD0T	December,	2006	CONT	SECT	JOB		HIG	HWAY
	REVISIONS		0908	27	006		CR	316
			DIST		COUNTY			SHEET NO.
			ABL		KENT			58

ACEMENT DIAGRAMS

The Forward Station Beam End will have one bearing and the Back Station Beam End will have two bearings.



1 4 3/4" 5%" Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut (ASTM A563). See "Material Notes" for installation.

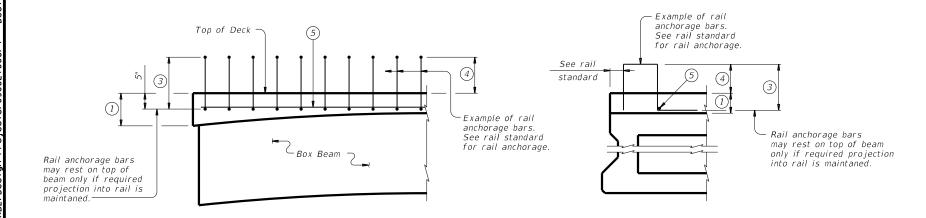
CAST-IN-PLACE ANCHORAGE OPTION

PART SPAN ELEVATION

ADHESIVE ANCHORAGE OPTION

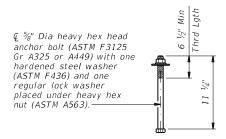
SECTION

### T631LS & T631 RAIL ANCHORAGE PLACEMENT (2)(7)

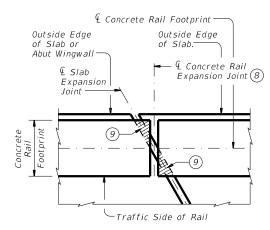


### TYPICAL CONCRETE RAIL ANCHORAGE

(Showing typical concrete rail anchorage)



T631LS & T631 RAIL C-I-P ANCHOR BOLT



PLAN OF CONCRETE RAILS AT EXPANSION JOINTS

- 1 Cast-in-place slab thickness varies due to beam camber (5" minimum)
- (2) Replace cast-in-place anchor bolts shown on T631LS or T631 Rail standard with an adhesive anchor system or cast-in-place anchor bolts shown on
- (3) Bar length shown on rail standard, minus 1 14". Adjust bar length for a raised sidewalk
- 4 See Rail standard for projection from finished grade or top of sidewalk.
- 5 Place additional (#5) longitudinal bar.
- 6 Excess bolt length has been provided to accommodate a variable slab thickness due to beam camber. If slab thickness on span details exceed 10", bolt length must be increased accordingly. After posts have been set and bolts tightened, bolt projection above nuts of more than  $\frac{1}{2}$ " must be cut off and painted with two coats of zinc-rich paint conforming to the Item 445 "Galvanizing".
- Distance from end of top outside edge of slab to center of first bolt group can not be less than 9", except: 15° Skew: 1'-0" (acute corner only) 30° Skew: 1'-3" (acute corner only)
- 8 Location of Rail Expansion Joint must be at the intersection of Q Slab Expansion Joint, & Rail Footprint and perpendicular to slab outside edge.
- ${rac{9}{9}}$ Cross-hatched area must have  ${rac{1}{2}}$ " Preformed Bitumuminous Fiber Material under concrete rail, as shown.

#### CONSTRUCTION NOTES:

Rail anchorage bars may be field bent as required to clear rail reinforcing or provide minimum cover shown on standard rail detail sheets.

Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

#### **MATERIAL NOTES:**

Galvanize all steel components of steel rail system. Provide Grade 60 reinforcing steel.

Cast-in-place anchorage system for T631LS and T631 Rail must be 5%" Dia heavy hex head anchor bolts (ASTM F3125 Gr 325 or A449) with one hardened steel washer (ASTM F436) and one regular lock washer placed under heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed anchor bolts 4 1/2" minimum.

Adhesive anchors for T631LS and T631 Rail must be 5%" Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 3/4". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole

size, drilling, and clean out, must be in accordance with Item 450, "Railing." Epoxy coat or galvanize reinforcing steel shown on this standard if rail reinforcement is epoxy coated or galvanized.

#### GENERAL NOTES:

Designed in accordance with AASHTO LRFD Bridge Design Specifications. This standard is for use with structures with a 5" minimum cast-in-place concrete slab.

This standard may require modification for interior rails. This standard does not apply to median barriers.

This standard does not provide details for Type T221P, T224, T80HT, T80SS, C412, PR11, PR22 and PR3 rails on box beam bridges. See rail standards for approved speed restrictions, notes and details not shown.

Cover dimensions are clear dimensions, unless noted otherwise.

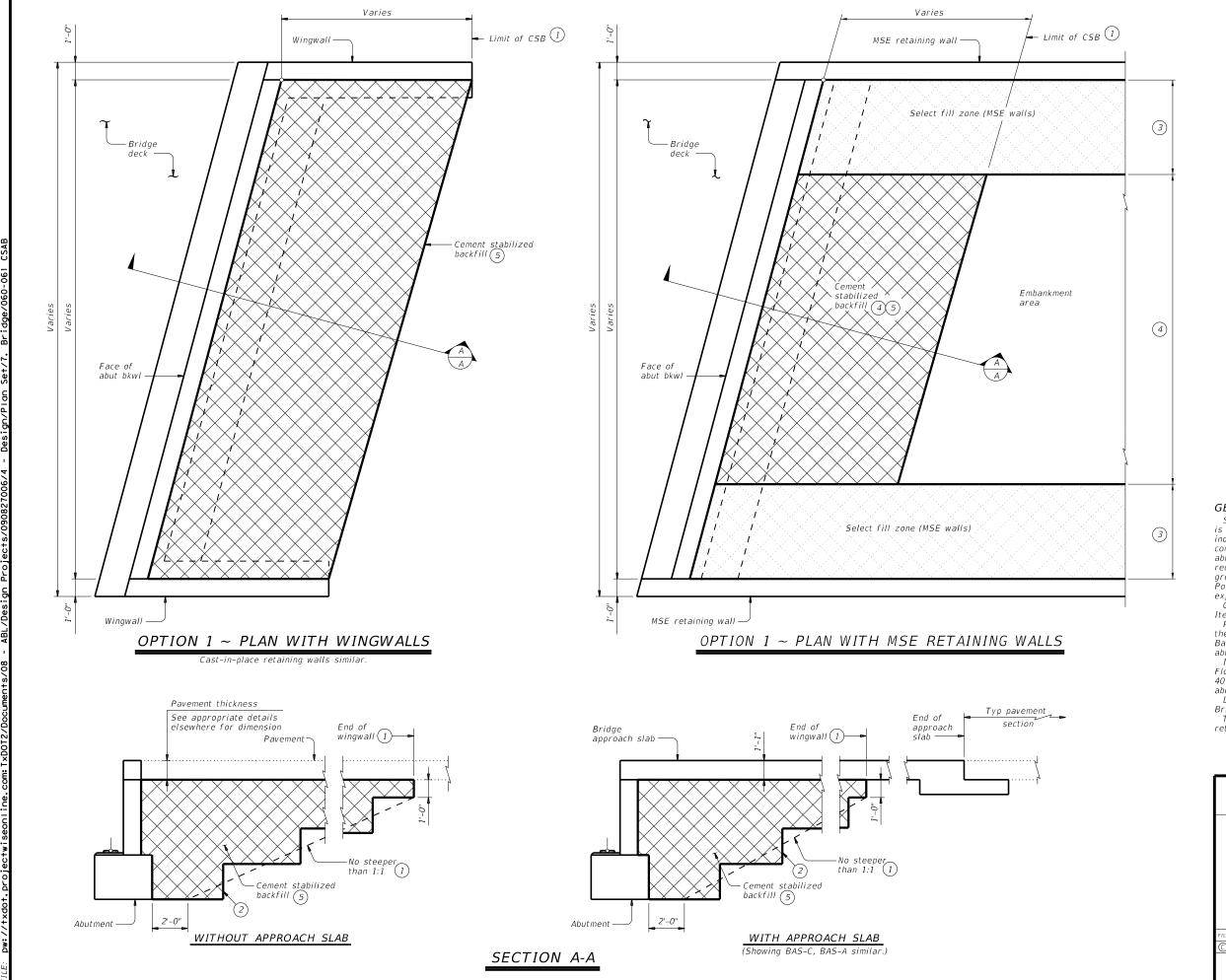
Texas Department of Transportation

Bridge Division Standard

RAIL ANCHORAGE **DETAILS** PRESTR CONC BOX BEAMS (WITH SLAB)

**BBRAS** 

FILE: bbstde09-18.dgn	DN: TxE	(DOT CK: TXDOT DW:		JTR CK: JMH			
CTxDOT December 2006	CONT	SECT	JOB		HIG	HIGHWAY	
REVISIONS 04-90: Updated for new rails.	0908	27	006		CR	316	
01-12: rails anchor bars. 07-14: Removed T101 & T6. Added T631. 03-16: Class D. E. or F epoxy in material	DIST	DIST COUNTY		SHEET NO			
notes, T221P & T224 in general notes.	ΔRI		KFNT			59	



1 Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.

2) Bench backfill as shown with 12" (approximate) bench depths.

(3) Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.

4 When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.

(5) If shown in the plans, flowable backfill can be used as a substitute for cement stabilized backfill with the following constraints:

constraints:
a). If flowable backfill is to be placed over MSE backfill, then a filter fabric will be placed over the MSE backfill prior to placement of the flowable fill; and b). Place flowable fill in lifts not

b). Place flowable fill in lifts not exceeding 2 feet in height. Place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowability).

### GENERAL NOTES:

See the Bridge Layout for selected Option. Option 1 is intended for construction only requiring plasticity index (PI) controlled embankment fill or excavation in competent soils/rocks in order to construct the abutment. Option 2 is intended for new construction requiring high plasticity embankment fill with a PI greater than 30 or pavement built in poor native soil. Poor soils are defined as high plasticity clays or expansive clays.

Construct abutment backfill in accordance with Item 400, "Excavation and Backfill for Structures".

Provide Cement Stabilized Backfill (CSB) meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments.

If required elsewhere in the plans, provide Flowable Backfill meeting the requirements of Item 401, "Flowable Backfill", to the limits shown at bridge abutments.

Details are drawn showing left forward skew. See Bridge Layout for actual skew direction. These details do not apply when Concrete Block

These details do not apply when Concrete Blo retaining walls are used in lieu of wingwalls.

SHEET 1 OF 2

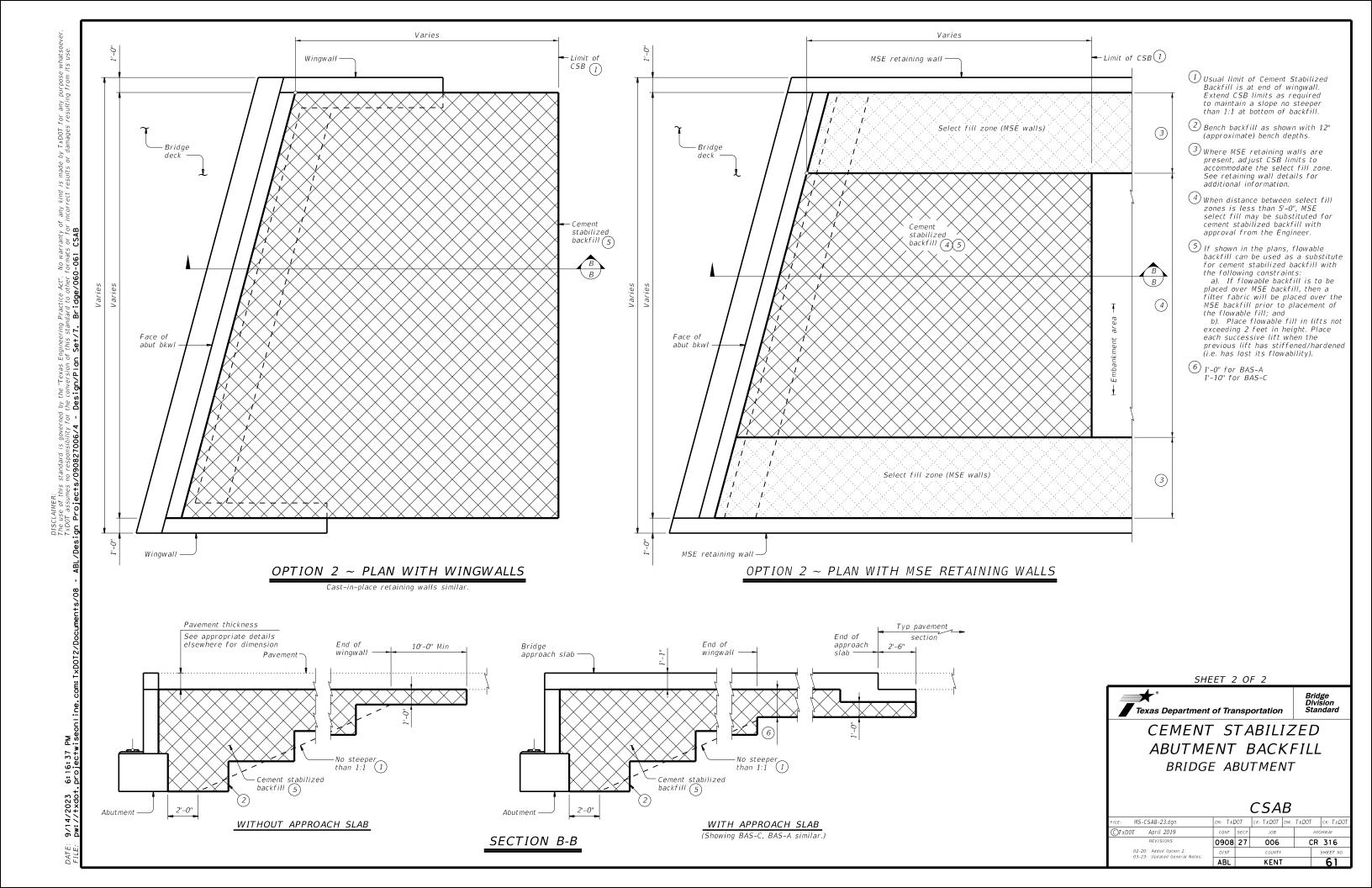


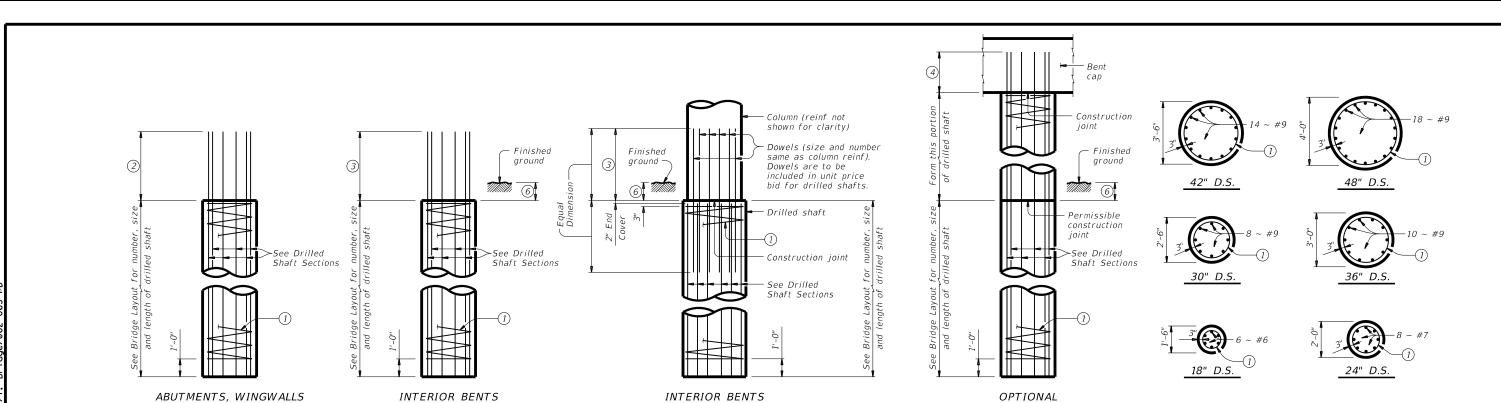
ZED

CEMENT STABILIZED
ABUTMENT BACKFILL
BRIDGE ABUTMENT

CSAB

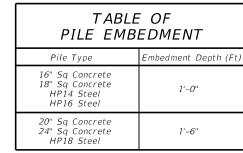
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02-20: Added Option 2. 03-23: Updated General Notes.	DIST		COUNTY			SHEET NO.
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DRILLED SHAFT DIA

EQUAL TO COLUMN DIA

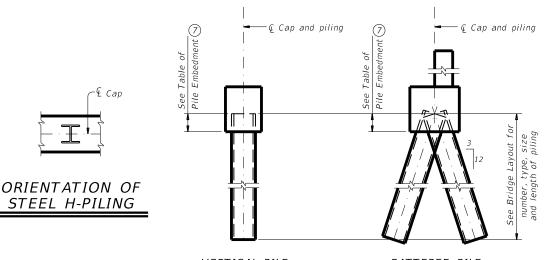


AND MULTI-DRILLED

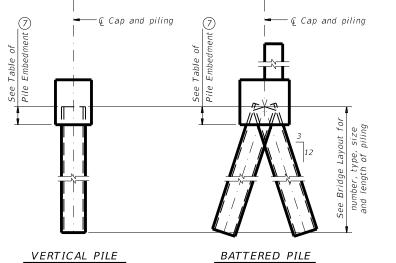
SHAFT FOOTINGS

See Prestressed Concrete Piling (CP) standard for additional details on concrete pile embedment.

ELEVATION

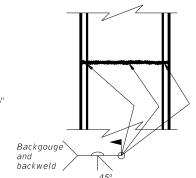


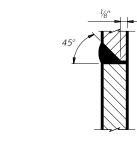




DRILLED SHAFT DIA

GREATER THAN COLUMN DIA

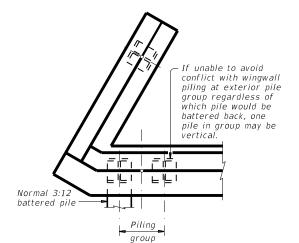




SECTION THRU FLANGE OR WEB

STEEL H-PILE SPLICE DETAIL

### DRILLED SHAFT DETAILS



INTERIOR BENT

DRILLED SHAFT DETAIL 5

### DETAIL "A' (Showing plan view of a 30° skewed abutment)

OTXDOT April 2019 01-20: Added #11 bars to the FD bars

DRILLED SHAFT SECTIONS

top and bottom).

#6 Bars = 1'-11" #7 Bars = 2'-0" #9 Bars = 2'-3"

#7 Bars = 2'-11" #9 Bars = 3'-9"

#11 Bars = 4'-8"

#6 Bars = 1'-11" #7 Bars = 2'-3"

 $#9 \ Bars = 2'-9''$ 

7 Or as shown on plans.

Min lap with column reinf:

SHEET 1 OF 2

#3 spiral at 6" pitch (one and a half flat turns

2 Min extension into supported element:

4 Min extension into supported element:

5 Drilled shafts may extend to the bottom of bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the

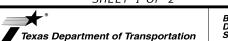
drilled shaft diameter equals the column

diameter. Obtain approval of the forming method above the ground line prior to

construction. No adjustments in payment

6 1'-0" Min, unless shown otherwise on plans.

will be made if this option is used.



## COMMON FOUNDATION **DETAILS**

FD006

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## SECTION B-B

Cut flange 45°

STEEL H-PILE TIP REINFORCEMENT See Item 407 "Steel Piling" to determine when tip reinforcement is required and for options to the details shown.

SECTION A-A

Bevel ¾" PL

45 degrees (Typ) -

Fill flush with

weld metal (Typ), shop or field weld.

field weld

Use when required.

### TABLE OF FOOTING QUANTITIES FOR 30" COLUMNS

50 00207775										
ONE 3 PILE FOOTING										
Bar	No.	Size	h	Weight						
F 1	11	#4	3'- 2	"	23					
F2	6	#4	8'- 2	"	33					
F3	6	#4	6'- 11	1"	28					
F 4	8	#9	3'- 2	"	86					
F5	4	#9	6'- 11	1"	94					
F6	4	#9	8'- 2	"	111					
FC	12	#4	3'- 6	"	28					
FD (10)	8	#9	8'- 1	"	220					
Reinf	orcing	Steel		623						
Class	"C" Cc	ncrete	4.8							
ONE 4 PILE FOOTING										
Bar	No.	Size	Weight							
F 1	20	#4	96							
F2	16	#8	7'- 2	306						
FC	16	#4	3'- 6	*	37					
FD [10]	8	#9	8'- 1	"	220					
Reinf	orcing	Steel		Lb	659					
Class	"C" Cc	ncrete		CY	6.3					
		ONE 5	PILE FOOT	TING						
Bar	No.	Size	Lengti	h	Weight					
F 1	20	#4	#4 8'- 2"							
F2	16	#9	8'- 2	444						
FC	24	#4	3'- 6	"	56					
FD 10	8	#9	8'- 1	"	220					
Reinf	orcing	Steel		Lb	829					
Class	"C" Cc	ncrete		CY	8.0					

### CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise.

Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile unless shown otherwise.

Provide Class C Concrete (f'c = 3,600 psi), unless shown otherwise. Provide Grade 60 reinforcing steel. Galvanize reinforcing if shown elsewhere in the plans.

Provide bar laps for drilled shaft reinforcing, where required, as follows:

Uncoated or galvanized (#6) ~ 2'-6" Uncoated or galvanized (#7) ~ 2'-11" Uncoated or galvanized (#9) ~ 3'-9"

GENERAL NOTES:
Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

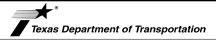
DESIGNER NOTES:
Do not use the drilled shaft details shown on this standard for retaining wall,

noise wall, barrier, or sign foundations without structural evaluation.

Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.

Maximum allowable pile loads for the footings shown are:
72 Tons/Pile with 24" Dia Columns
80 Tons/Pile with 30" Dia Columns
100 Tons/Pile with 36" Dia Columns 120 Tons/Pile with 42" Dia Columns

SHEET 2 OF 2

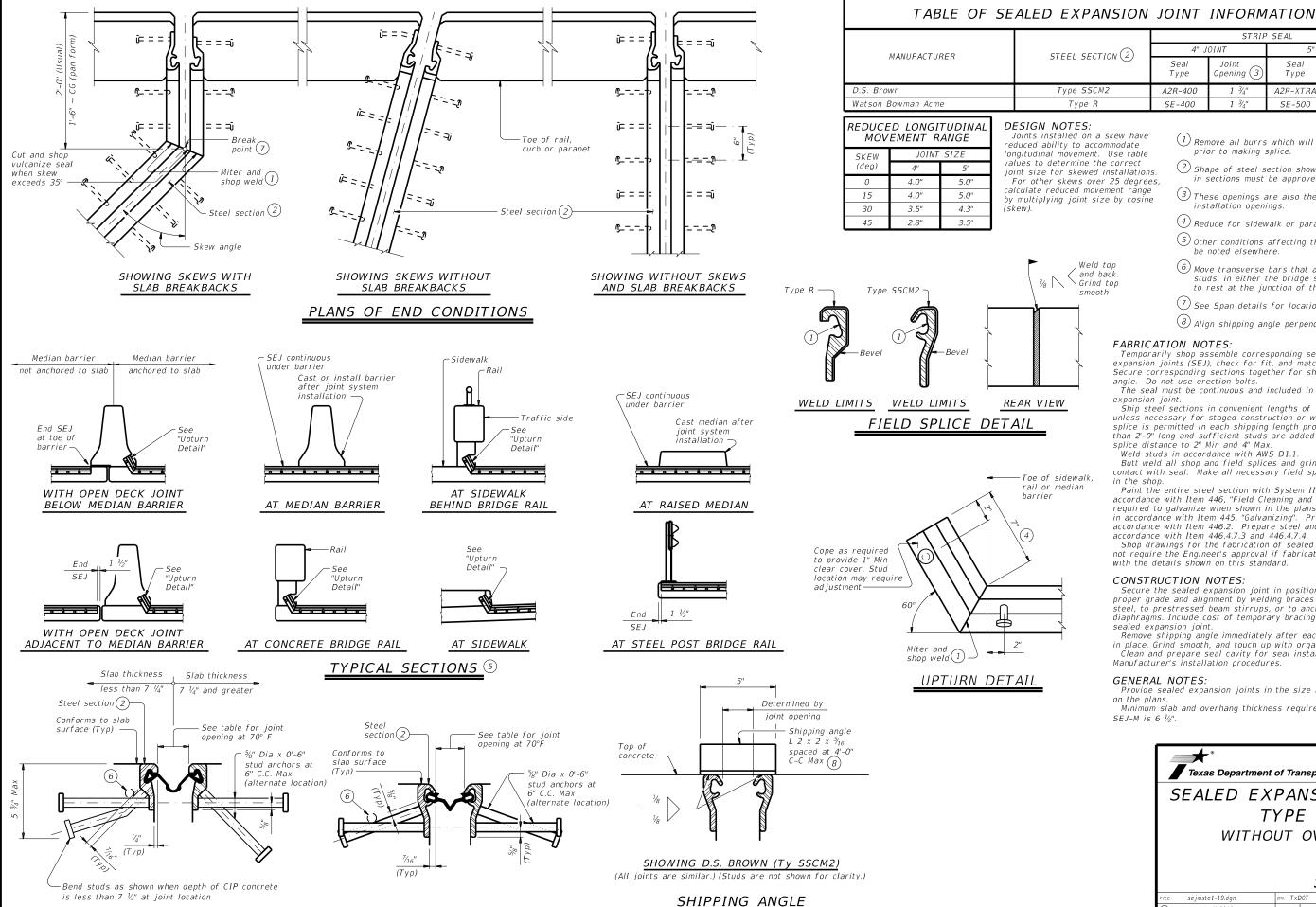


### COMMON FOUNDATION **DETAILS**

FD

Bridge Division Standard

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01-20: Added #11 bars to the FD bars.	DIST	COUNTY				SHEET NO.	
	ABL		KENT			63	



An alternate method of securing joint sections may

be used if approved by the Bridge Division.

Erection bolts are not allowed.

SECTION THRU D.S. BROWN

(A2R-400 OR A2R-XTRA) JOINTS

6:17:08

SECTION THRU WATSON BOWMAN

ACME (SE-400 OR SE-500) JOINTS

Join Joint Opening (3 Type Opening (3 Type A2R-400 A2R-XTRA SE-400 SF-500

Joints installed on a skew have longitudinal movement. Use table

For other skews over 25 degrees, calculate reduced movement range by multiplying joint size by cosine

- (1) Remove all burrs which will be in contact with seal prior to making splice.
- $^{igl(2igr)}$  Shape of steel section shown is typical. Variations in sections must be approved by the Engineer.
- $\stackrel{ ext{ }}{ ext{ }}$  These openings are also the recommended minimum installation openings.
- $\stackrel{ ext{$(4)$}}{}$  Reduce for sidewalk or parapet heights less than 6". (5) Other conditions affecting the joint profile should
- (6) Move transverse bars that are in conflict with SEJ studs, in either the bridge slab or approach slab, to rest at the junction of the studs.
- 7 See Span details for location of break point.
- (8) Align shipping angle perpendicular to joint.

#### FABRICATION NOTES:

Temporarily shop assemble corresponding sections of sealed expansion joints (SEJ), check for fit, and match mark for shipment Secure corresponding sections together for shipment with shipping angle. Do not use erection bolts.

be noted elsewhere.

The seal must be continuous and included in the price bid for sealed expansion joint.

Ship steel sections in convenient lengths of 10'-0" Min and 24'-0" Max unles's necessary for staged construction or widenings. One shop splice is permitted in each shipping length provided no piece is less than 2'-0" long and sufficient studs are added to limit the stud to shop splice distance to 2" Min and 4" Max.

Weld studs in accordance with AWS D1.1.

Butt weld all shop and field splices and grind smooth areas in contact with seal. Make all necessary field splice joint preparations in the shop.

Paint the entire steel section with System II or IV primer in accordance with Item 446, "Field Cleaning and Painting Steel", unless required to galvanize when shown in the plans. Provide galvanizing in accordance with Item 445, "Galvanizing". Provide paints in accordance with Item 446.2. Prepare steel and apply paint in accordance with Item 446.4.7.3 and 446.4.7.4.

Shop drawings for the fabrication of sealed expansion joints will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.

#### CONSTRUCTION NOTES:

Secure the sealed expansion joint in position and place to the proper grade and alignment by welding braces to adjacent reinforcing steel, to prestressed beam stirrups, or to anchors cast in concrete diaphragms. Include cost of temporary bracing in the price bid for sealed expansion joint.

Remove shipping angle immediately after each joint half is secured in place. Grind smooth, and touch up with organic zinc-rich paint. Clean and prepare seal cavity for seal installation as per the Manufacturer's installation procedures.

#### GENERAL NOTES:

Provide sealed expansion joints in the size and at locations shown

Minimum slab and overhang thickness required for the use of SEJ-M is 6 1/2".

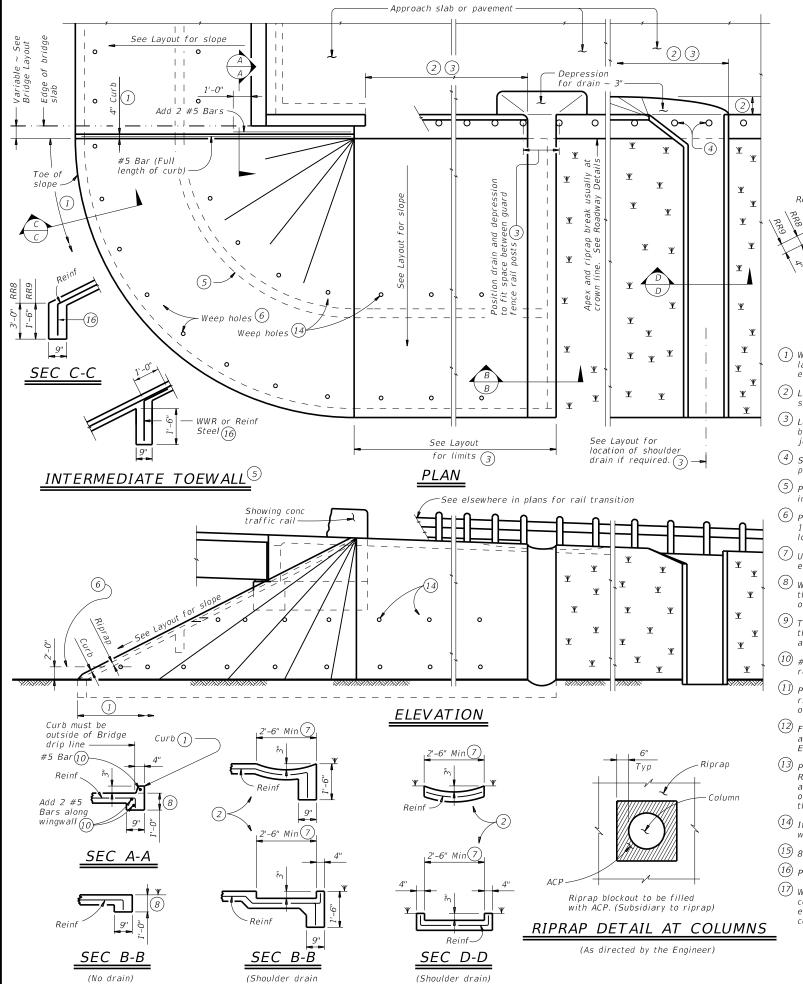


Bridge Division Standard

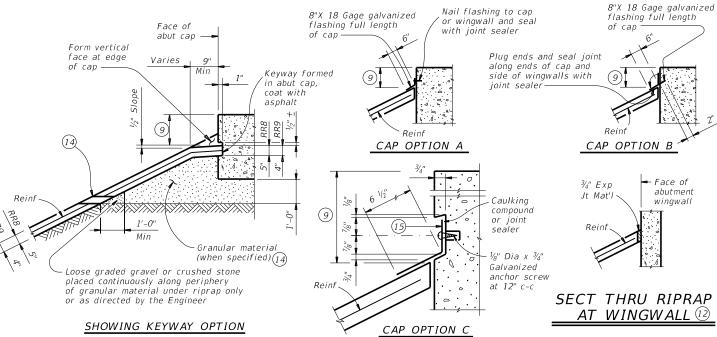
SEALED EXPANSION JOINT TYPEMWITHOUT OVERLAY

SEJ-M

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REVISIONS	0908	27	006		CR 316		
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integral with riprap)



(1) When riprap is shown extended around header on layout, extend slab and toewall as shown and eliminate 4" curb.

## SECTIONS THRU RIPRAP AT CAP (1)

(2) Limits and configuration of drains and depressions are as shown elsewhere in plans or as directed by the Engineer.

 $\stackrel{ ext{ }}{ ext{ }}$  Location of shoulder drain must consider limitations imposed by rail transition. Do not locate shoulder drains at expansion joints between approach slab and concrete pavement.

4 See details elsewhere in plans for installation of guard fence posts through concrete riprap.

(5) Provide intermediate toewall only when designated elsewhere in the plans or included in the specifications.

6 Provide lower level of 2" Dia weep holes at 10' c-c backed by 1 CF packet of gravel and galvanized hardware cloth at all locations unless directed by the Engineer to eliminate.

(7) Use wider or other drain configurations if shown elsewhere in plans or if directed by the Engineer

 $^{ig(8)}$  Wall extension may be reduced or modified if approved by the Engineer. Increase wall extension to 1'-6" whenever the optional intermediate toewall is called for in the plans.

Top of cap to top of riprap dimension varies as directed by the Engineer. Should be 9" Min for beam/slab type bridges and 1'-6" for slab span, box beam, or slab beam bridges.

 $\stackrel{ ext{$(1)}}{ ext{$(2)}}$  #5 bars shown are required even when synthetic fiber reinforcing option is selected.

(11) Provide sealing option for joint between the face of cap and riprap as designated by the Engineer or as shown elsewhere

12) Flashing (shown in Cap Option A) may be used at wingwall in addition to Exp Jt Mat'l if shown on plans or directed by the

Provide #3 reinforcing bars at 18" Spa c-c. Provide Welded Wire Reinforcement (WWR) as 6x6-D2.9xD2.9 or D3xD3. Combinations of WWR and reinforcing bars may be used if both are permitted. Use lap splices of a minimum 6 inches, measured from the transverse wire of WWR, and the ends of reinforcing bars.

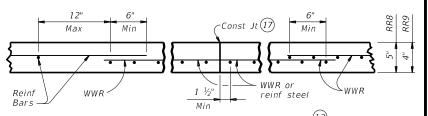
(14) If granular material is specified, provide upper level of 2" Dia weep holes at 10' c-c backed by galvanized hardware cloth.

15 8" x 18 Gage Galv Sheet Metal

(16) Provide WWR or #3 bars, with 1'-0" extension into slope.

(17) WWR or reinforcing steel is continuous through riprap construction joints. Provide WWR or reinforcing steel that extends 1'-1" minimum into adjacent riprap on each side of construction joint even if synthetic reinforcing fiber is utilized.

> FOR CONTRACTOR'S INFORMATION ONLY: 5" of RR8 = 0.015 CY/SF4" of RR9 = 0.012 CY/SF#3 Reinf at 18'' c-c = 0.501 Lbs/SF6x6-D3xD3 = 0.408 Lbs/SF



REINFORCEMENT DETAILS (13)

#### GENERAL NOTES:

Provide Class "B" concrete (f'c = 2,000 psi) unless noted elsewhere

n plans. Provide Grade 60 reinforcing steel. Provide deformed welded wire reinforcement (WWR) meeting

ASTM A1064, unless otherwise shown.

Provide reinforcing bars, deformed WWR, or any suitable combination of both types for riprap reinforcing, unless specified elsewhere in the Optionally synthetic fibers may be used if approved by the Engineer

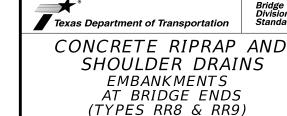
Provide synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) in lieu of steel reinforcing in riprap concrete. Install construction joints or grooved joints extending the full slant

slope height at intervals of approximately 20 feet unless otherwise directed by the Engineer.

Hardware cloth, loose grade stone behind weep holes, flashing, or

other sealing material are subsidiary to the bid item "Riprap". See Layout for limits of riprap.

RR8 is to be used on stream crossings. RR9 is to be used on other embankments.



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1) Use wider or other drain configurations if shown elsewhere in plans or if directed by the Engineer. Location of shoulder drain must consider limitation imposed by rail transition. Do not locate shoulder drains at expansion joints between approach slab and concrete pavement.

-Slope break

-Edge of shoulder drain

reinforced concrete.

2) Fill leave-outs with no more than a 2-sack grout mixture (1 part cement, 5 parts water, and 14 parts sand by volume) with a 28-day compressive strength of approximately 120 psi or less. Provide grout of a consistency that will flow into and completely fill all voids. Due to auger size, larger leave-out dimensions are acceptable from both an impact performance and maintenance repair standpoint (20" Max leave-out).

3 For other slope and flow directions drain configuration may be mirrored wider or tapered wider if shown elsewhere in the plans or directed by the Engineer.

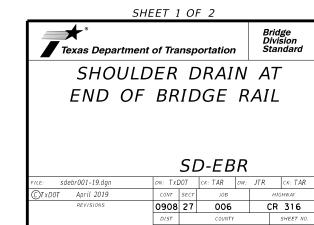
4 Form depression into concrete, asphalt pavement, or approach

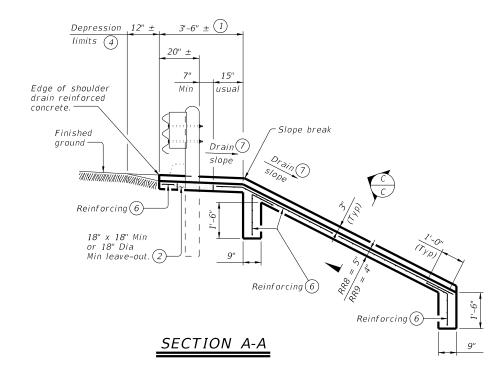
5 See Bridge Layout for rail type.

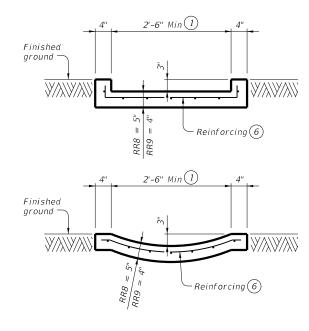
2'-6" (1)

Slope break

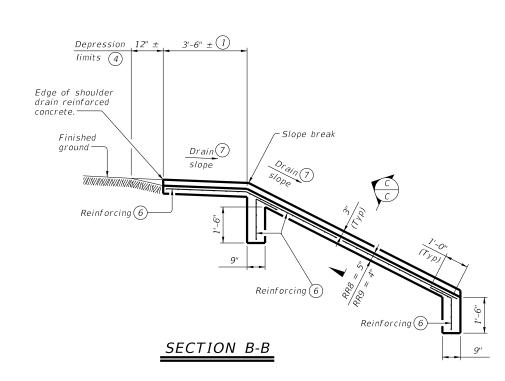
4'-3"(1)

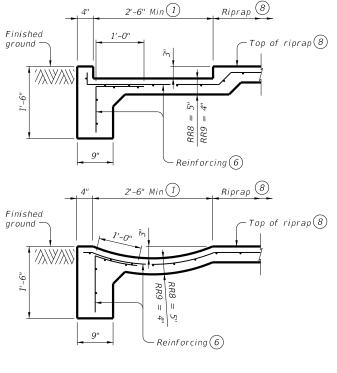




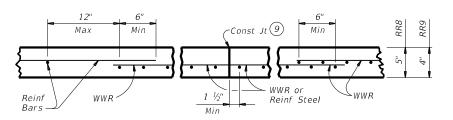


SECTION C-C





SECTION C-C Sections shown with integrated riprap.



## REINFORCEMENT DETAILS 6

- ① Use wider or other drain configurations if shown elsewhere in plans or if directed by the Engineer. Location of shoulder drain must consider limitation imposed by rail transition. Do not locate shoulder drains at expansion joints between approach slab and concrete pavement.
- Fill leave-outs with no more than a 2-sack grout mixture (1 part cement, 5 parts water, and 14 parts sand by volume) with a 28-day compressive strength of approximately 120 psi or less. Provide grout of a consistency that will flow into and completely fill all voids. Due to auger size, larger leave-out dimensions are acceptable from both an impact performance and maintenance repair standpoint (20" Max leave-out).
- 4 Form depression into concrete, asphalt pavement, or approach slab.
- 6 Provide (#3) reinforcing bar at 18" spacing c-c or welded wire reinforcement (WWR) as 6x6-D2.9xD2.9 or D3xD3. Combinations of WWR and reinforcing bars may be used if both are permitted. Use lap splices of a minimum 6 inches, measured from the transverse wire of WWR, and the ends of reinforcing bars, unless shown otherwise.
- 7 See elsewhere in plans or as directed by the Engineer.
- 8 See CRR standard for details and notes not shown.
- 9 WWR or reinforcing steel is continuous through riprap construction joints. Provide WWR or reinforcing steel that extends 1'-1" minimum into adjacent riprap on each side of construction joint even if synthetic fiber is utilized.

#### GENERAL NOTES:

Provide Class "B" concrete with a minimum compressive strength of 2,000 psi unless noted elsewhere in plans. Provide Grade 60 reinforcing steel.

Provide deformed welded wire reinforcement (WWR) meeting

ASTM A1064, unless otherwise shown.

Provide reinforcing bars, deformed WWR, or any suitable combination of both types for riprap reinforcing, unless specified elsewhere in the

Optionally synthetic fibers may be used if approved by the Engineer. Provide synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) in lieu of steel reinforcing in riprap concrete. See Metal Beam Guard Fence (Mow Strip) standard for details and

notes not shown. Payment for furnishing and placing 2-sack grout mixture will be subsidiary to shoulder drain.

Payment for shoulder drain will be as per Item 420, "CI B Conc (Flume)". All details shown herein are subsidiary to shoulder drain.

See Layout for limits of shoulder drain. RR8 is to be used on stream crossings.

RR9 is to be used on other embankments

#### SHEET 2 OF 2



### SHOULDER DRAIN AT END OF BRIDGE RAIL

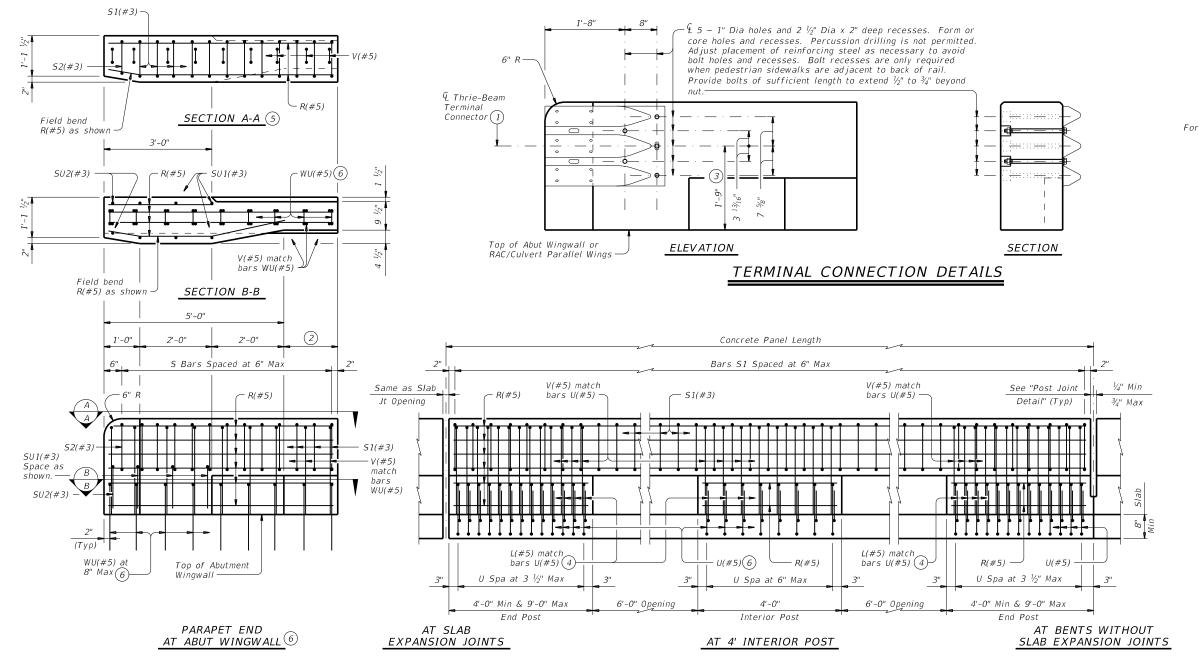
SD-EBR

Bridge Division Standard

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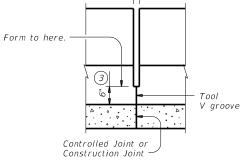
68



### ELEVATION SHOWING TYPICAL REINFORCING PLACEMENT

Showing rail on slab. Rail on box culvert similar

- 1 Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.
- ② Wingwall Length minus 5'-0" (Varies)
- ③ Increase 2" for structures with overlay.
- 4 Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.
- Bars SU1(#3), SU2(#3) and WU(#5) not shown for clarity.
- 6 Substitute Bars U(#5) for Bars WU(#5) when parapet end is located on achorage curb over culvert top slab. Use Bars WU(#5) in culvert parallel wings.



1/4" Min

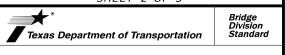
*¾*" Max

Opening

### POST JOINT DETAIL

Provide at all interior bents without slab expansion joints.

SHEET 2 OF 3

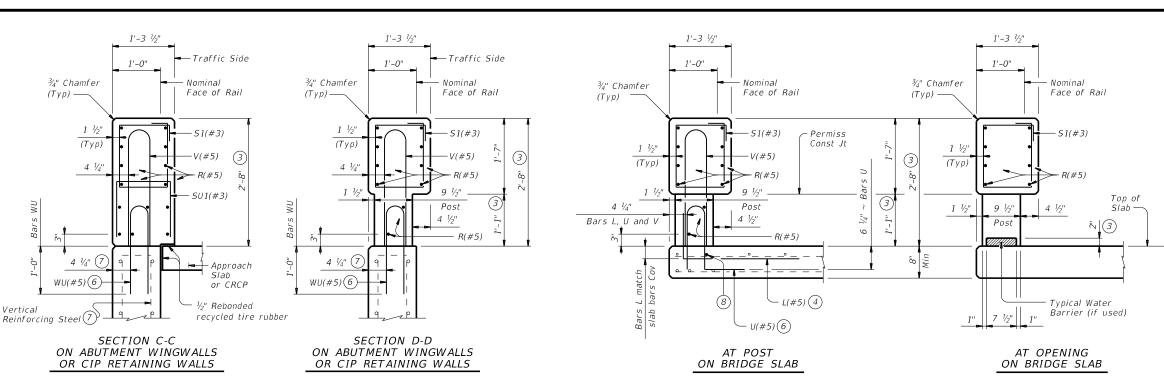


TRAFFIC RAIL

TYPE T223

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### SECTIONS THRU RAIL

Sections on box culverts similar



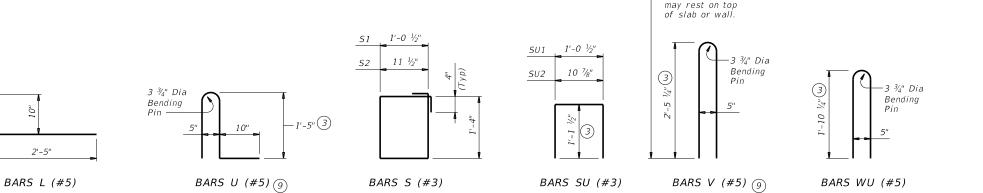
- 3 Increase 2" for structures with overlay.
- 4 Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.
- 6 Substitute Bars U(#5) for Bars WU(#5) when parapet end is located on anchorage curb over culvert top slab. Use Bar's WU(#5) in culvert parallel wings.
- (7) When vertical reinforcing has closer clear cover over horizontal reinforcing in abutment wingwalls on traffic side of wall, move the horizontal wingwall/retaining wall reinforcing to the inside of Bars WU where bars conflict.
- $\fbox{8}$  Top longitudinal slab bar may be adjusted laterally 3" plus or minus to tie reinforcing.
- (9) At the Contractor's option, Bars V may be replaced by extending Bars U to 2'-5 1/4" above the roadway surface without overlay.

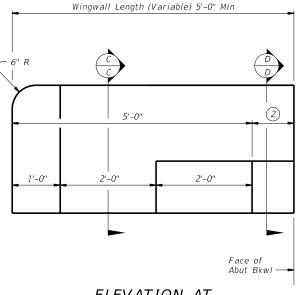
### € Concrete Rail Footprint Outside Edge Outside Edge Abut Wingwall ► ¶ Concrete Rail Expansion Joint. Location of Rail Expansion € Slab Joint must be at the intersection of & Slab Expansion Joint, Expansion 4 Rail Footprint and perpendicular to slab outside edge. Joint Cross-hatched area must have 1/2" Preformed Bitumuminous Fiber Material under concrete rail, as shown -Traffic Side of Rail

### PLAN OF RAIL AT EXPANSION JOINTS

-Installed bar

Example showing Slab Expansion Joints without breakbacks.





### ELEVATION AT ABUTMENT WINGWALL

CONSTRUCTION NOTES:
Face of rail and parapet must be vertical transversely unless otherwise shown in the plans or approved by the Engineer.

Provide water barriers at openings draining onto undercrossing roadways and sidewalks. They may be cast-in-place or precast in convenient lengths and bonded to the bridge deck with an approved

Chamfer all exposed corners.

### MATERIAL NOTES:

Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere.

Provide Grade 60 reinforcing steel.

Epoxy coat or galvanize all reinforcing steel if slab bars are epoxy coated or galvanized.

Deformed Welded Wire Reinforcing (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars U, V, and WU unless noted otherwise. Provide the same laps as required for reinforcing

Provide bar laps, where required, as follows:

Uncoated or galvanized ~ #5 = 2'-0" Epoxy coated  $\sim #5 = 3'-0''$ 

#### GENERAL NOTES:

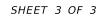
This rail has been evaluated by full-scale crash test to meet MASH TL-3 criteria. This rail can be used for speeds of 50 mph and greater when a TL-3 rated guard fence transition is used. When a TL-2 rated guard fence transition is used, this rail can only be used for speeds of 45 mph and less.

Do not use this railing on bridges with expansion joints providing more than 5" movement.

Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details elsewhere in plans for these modifications. Shop drawings are not required for this rail

Average weight of railing with no overlay is 358 plf

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.





### TRAFFIC RAIL

### TYPE T223

					_			
FILE: rlstd005-19.dgn		DN: TXI	DOT	CK: TXDOT DW:		JTR	CK: AES	
©T x D0T	CONT	SECT	JOB			HIGHWAY		
REVISIONS		0908	27 006			CR 316		
		DIST	COUNTY			SHEET NO.		
		ABL		KENT	•		70	

SECTION OF TRAFFIC RAIL ON MOMENT SLAB (TRF-MS)

(Showing SSTR rail other rails are similar.)

CONSTRUCTION NOTES: Align moment slab (TRF-MS) or grade beam (TRF-GB) open joints with rail open joints maintaining no less than minimum rail length Provide moment slab (TRF-MS) or grade beam (TRF-GB) with open joints at no greater than 100' spacing unless otherwise shown on the plans or approved by the Engineer.

### MATERIAL NOTES:

Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere.

Provide Grade 60 reinforcing steel.

Epoxy coat or galvanize all reinforcing steel if required elsewhere.

Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for bars S1(#4), S2(#4) and H(#5) unless noted otherwise. Provide the same laps as required for reinforcing bars.

Provide bar laps, where required, as follows:

Uncoated or galvanized ~ #5 = 2'-4" Epoxy coated  $\sim #5 = 3'-6''$ 

#### GENERAL NOTES:

Use of these details will result in a moment slab (TRF-MS) or grade beam (TRF-GB) foundation that is acceptable for traffic rails which are MASH TL-2, TL-3, or TL-4 compliant.

See elsewhere in the plans for selected options between moment slab (TRF-MS) and/or grade beam (TRF-GB).
The foundation design resistance is based on the current

AASHTO bridge railing requirements with the assumption of fair to good soil support conditions. Poor soil conditions will require

suitably deeper and/or wider foundations. See appropriate rail standard for details and notes not shown. This detail is intended for use as a guide to unusual railing anchorage situations but may be included in the plans, modified as necessary to apply to specific installations required on the

project. Payment for moment slab (TRF-MS) and/or grade beam (TRF-GB) will be by Class "C" concrete or Class "C" (HPC) concrete for rail foundations.

The associated bridge railing will be paid for by the linear foot which includes the concrete and reinforcement. Excavation will be subsidiary to other Items.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.



Bridge Division Standard

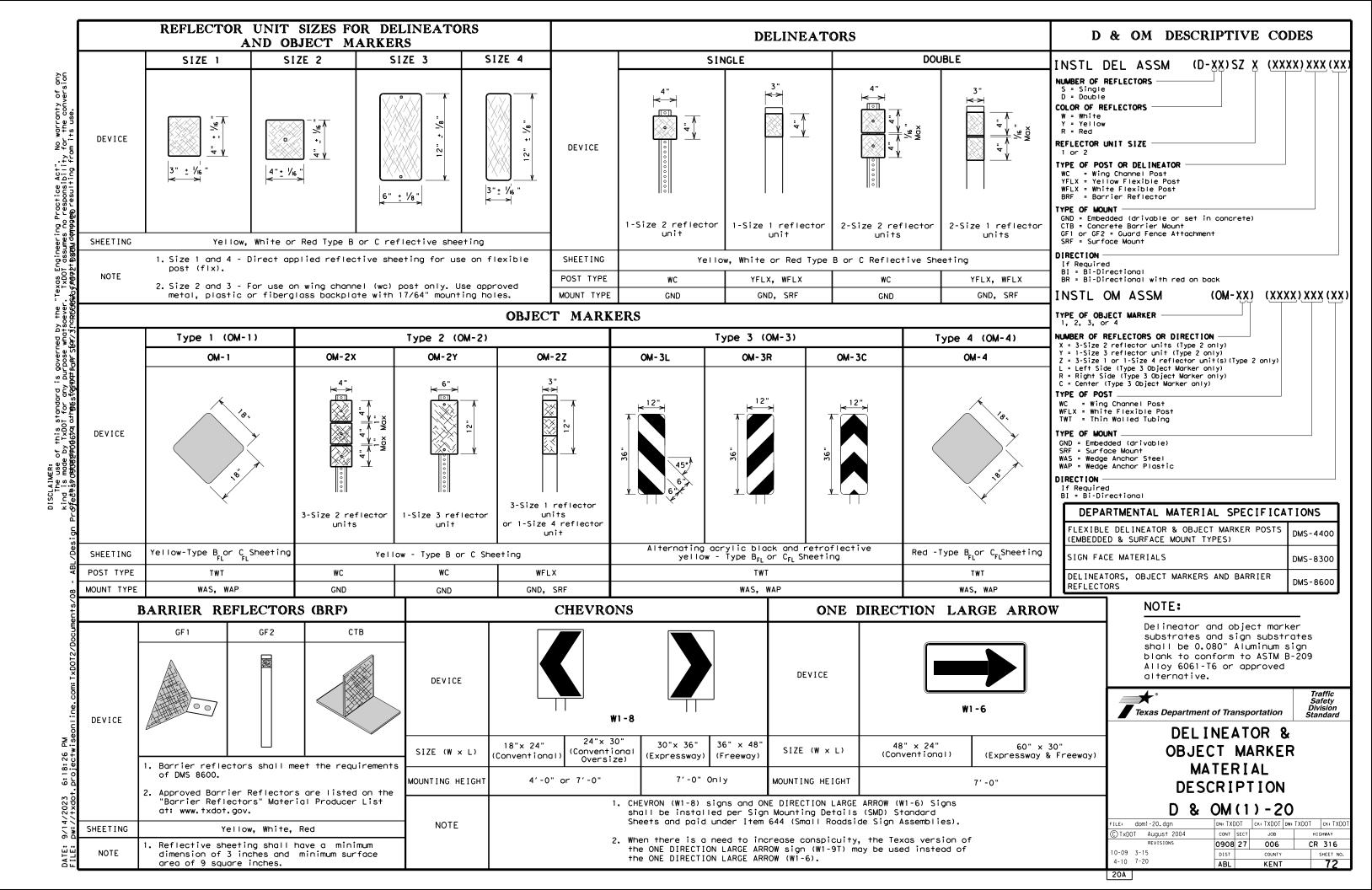
**FOUNDATIONS** FOR MASH TL-2, TL-3 & TL-4 BRIDGE RAILS

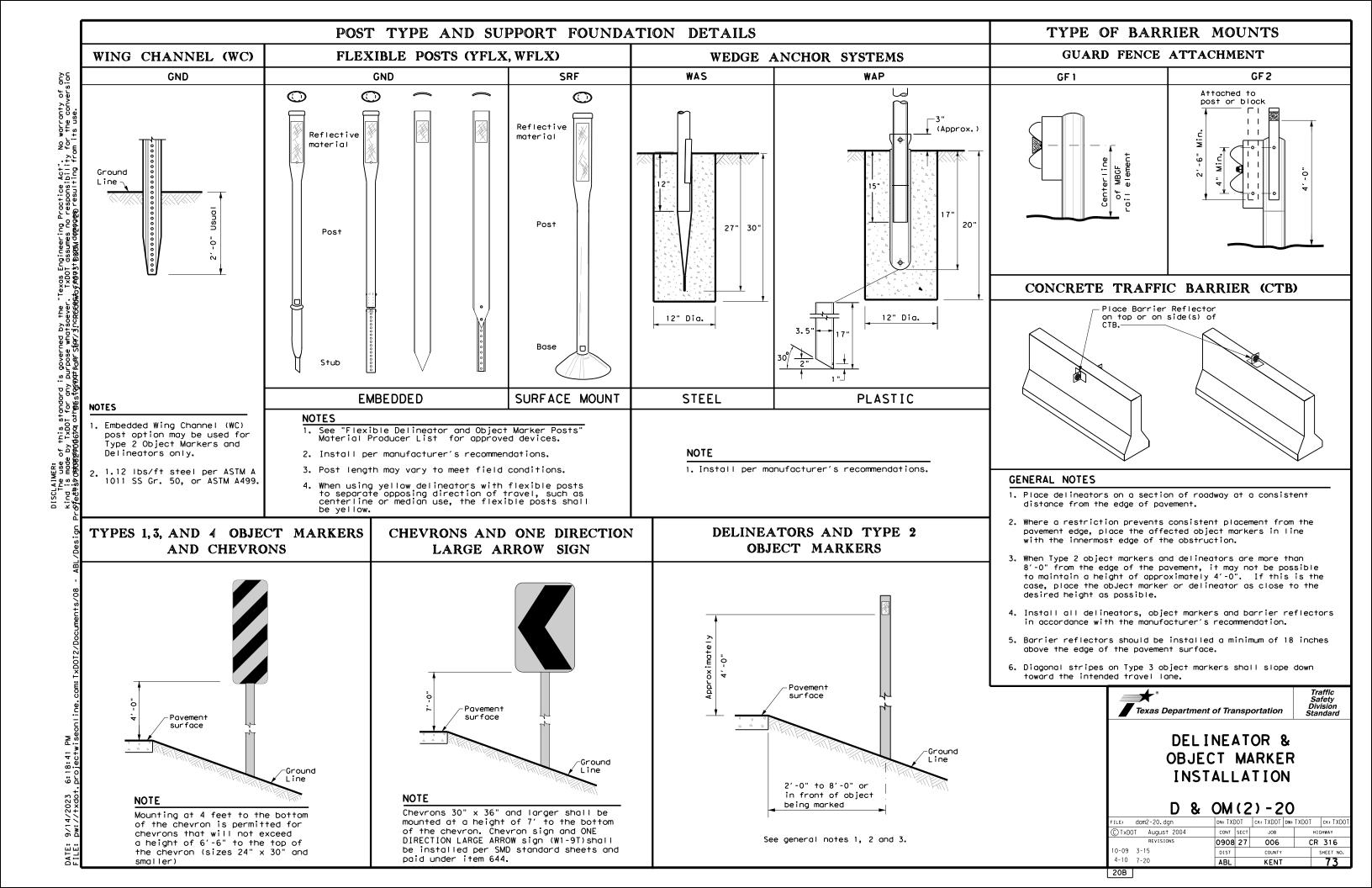
TRF

rlstd027-20.dgn	DN: TXE	70T	CK: TAR	DW:	JTR	(	ck: TAR
TxDOT September 2019	CONT	SECT	JOB		HIGHWAY		₩AY
REVISIONS	0908	27	7 006		CR 316		
07-20: Added moment slab with rall foundation lengths.	DIST	DIST COUNTY			SHEET NO.		
	ABL		KENT				71

SECTION OF TRAFFIC RAIL ON GRADE BEAM (TRF-GB)

(Showing SSTR rail other rails are similar.)



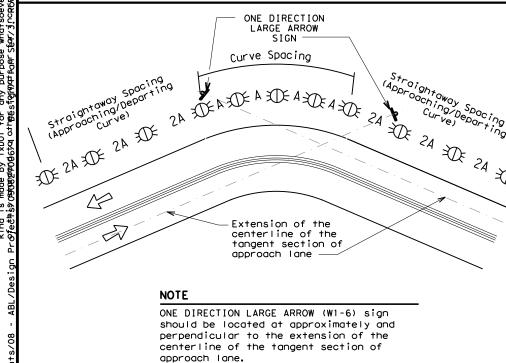


## MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

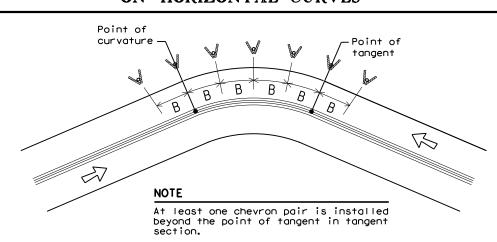
Amount by which Advisory Speed	Curve Advisory Speed			
is less than Posted Speed	Turn (30 MPH or less)	Curve (35 MPH or more)		
5 MPH & 10 MPH	• RPMs	• RPMs		
15 MPH & 20 MPH	<ul> <li>RPMs and One Direction Large Arrow sign</li> </ul>	<ul> <li>RPMs and Chevrons; or</li> <li>RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons.</li> </ul>		
25 MPH & more	<ul> <li>RPMs and Chevrons; or</li> <li>RPMs and One Direction         Large Arrow sign where         geometric conditions or         roadside obstacles prevent         the installation of</li> </ul>	• RPMs and Chevrons		

#### SUGGESTED SPACING FOR DELINEATORS ON HORIZONTAL CURVES

chevrons



#### SUGGESTED SPACING FOR CHEVRONS ON HORIZONTAL CURVES



#### DELINEATOR AND CHEVRON **SPACING**

WHEN DEGREE OF CURVE OR RADIUS IS KNOWN

	FEET					
Degree of Curve	Radius of Curve	Spacing in Curve	Spacing in Straightaway	Chevron Spacing in Curve		
		Α	2A	В		
1	5730	225	450			
2	2865	160	320			
3	1910	130	260	200		
4	1433	110	220	160		
5	1146	100	200	160		
6	955	90	180	160		
7	819	85	170	160		
8	716	75	150	160		
9	637	75	150	120		
10	573	70	140	120		
11	521	65	130	120		
12	478	60	120	120		
13	441	60	120	120		
14	409	55	110	80		
15	382	55	110	80		
16	358	55	110	80		
19	302	50	100	80		
23	249	40	80	80		
29	198	35	70	40		
38	151	30	60	40		
57	101	20	40	40		

Curve delineator approach and departure spacing should include 3 delineators spaced at 2A. This spacing should be used during design preparation or when the degree of curve is known.

#### DELINEATOR AND CHEVRON **SPACING**

WHEN DEGREE OF CURVE OR RADIUS IS NOT KNOWN

Advisory Speed (MPH)	Spacing in Curve	Spacing in Straightaway	Chevron Spacing in Curve
	Α	2×A	В
65	130	260	200
60	110	220	160
55	100	200	160
50	85	170	160
45	75	150	120
40	70	140	120
35	60	120	120
30	55	110	80
25	50	100	80
20	40	80	80
15	35	70	40

If the degree of curve is not known, delineator spacing may be determined based on the Advisory Speed of the curve. Use the delineator curve spacing for each Advisory Speed (MPH).

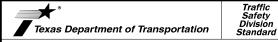
# DELINEATOR AND OBJECT MARKER APPLICATION AND SPACING

CONDITION	REQUIRED TREATMENT	MINIMUM SPACING
Frwy./Exp. Tangent	RPMs	See PM-series and FPM-series standard sheets
Frwy./Exp. Curve	Single delineators on right side	See delineator spacing table
Frwy/Exp.Ramp	Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4))	100 feet on ramp tangents Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves)
Acceleration/Deceleration Lane	Double delineators (see Detail 3 on D&OM(4))	100 feet (See Detail 3 on D & OM (4))
Truck Escape Ramp	Single red delineators on both sides	50 feet
Bridge Rail (steel or concrete)and Metal Beam Guard Fence	Bi-Directional Delineators when undivided with one lane each direction  Single Delineators when multiple lanes each direction	Equal spacing (100'max) but not less than 3 delineators
Concrete Traffic Barrier (CTB) or Steel Traffic Barrier	Barrier reflectors matching the color of the edge line	Equal spacing 100' max
Cable Barrier	Reflectors matching the color of the edge line	Every 5th cable barrier post (up to 100'max)
Guard Rai∣ Terminus/Impact Head	Divided highway - Object marker on approach end  Undivided 2-lane highways - Object marker on approach and departure end	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6)
Bridges with no Approach Rail	Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail	See D & OM(5)
Reduced Width Approaches to Bridge Rail	Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end
		See D & OM (5)
Culverts without MBGF	Type 2 Object Markers	See Detail 2 on D & OM(4)
Crossovers	Double yellow delineators and RPMs	See Detail 1 on D & OM (4)
Pavement Narrowing (lane merge) on Freeways/Expressway	Single delineators adjacent to affected lane for full length of transition	100 feet

#### MO1F2

- 1. Unless indicated otherwise, the delineator or barrier reflector color shall conform to the color of the pavement edge line on the side of the road where the delineators or barrier reflectors are placed.
- 2. Barrier reflectors may be used to replace required delineators.
- 3. Single red delineators may be mounted on the back side of delineator posts for wrong way driver applications

LEGEND				
<b>XX</b>	Bi-directional Delineator			
K	Delineator			
4	Sign			



**DELINEATOR &** OBJECT MARKER PLACEMENT DETAILS

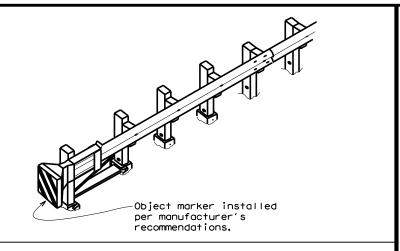
D & OM(3) - 20

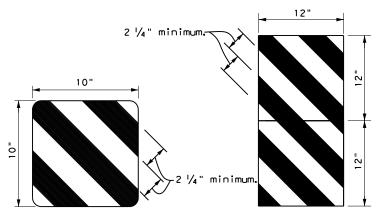
	-	_				
ILE: dom3-20.dgn	DN: TX[	)OT	ck: TXDOT	DW:	TXDOT	ck: TXDOT
TxDOT August 2004	CONT	SECT	JOB		HIG	HWAY
	0908	27	006		CR	316
15 8-15	DIST		COUNTY		9	SHEET NO.
-15 7-20	ABL		KENT			74

#### TWO-WAY, TWO LANE ROADWAY TWO-WAY, TWO LANE ROADWAY TWO-WAY, TWO LANE ROADWAY BRIDGE WITH NO APPROACH RAIL WITH REDUCED WIDTH APPROACH RAIL WITH METAL BEAM GUARD FENCE (MBGF) See Note 1 See Note 1 See Note 1 See Note 出 出 25 ft. 25 ft. 3- Type D-SW 3- Type D-SW /₩ 25 ft. delineators delineators spaced 25' spaced 25' $\stackrel{\wedge}{\mathbb{A}}$ apart apart 出 出 **MBGF** Type D-SW Type D-SW delineators delineators $\stackrel{\wedge}{\mathbb{A}}$ bidirectional bidirectional One barrier $\stackrel{\star}{\bowtie}$ One barrier reflector shall reflector shall be placed $\stackrel{\ \ \, }{\bowtie}$ Steel or concrete-П be placed directly behind Bridge rail directly behind each OM-3. each OM-3. The others The others $\stackrel{*}{\bowtie}$ will have -Steel or concrete will have equal spacing Bridge rail equal spacing (100' max), but (100' max), but not less than 3 Bidirectional white barrier not less than 3 bidirectional Bidirectional bidirectional white barrier white barrier reflectors or white barrier Equal spacing (100' max), but reflectors reflectors or delineators $\stackrel{\wedge}{\bowtie}$ reflectors Equal spacing delineators not less than (100' max), but 3 bidirectional not less than 3 bidirectional white barrier reflectors or white barrier Equal $\stackrel{\wedge}{\mathbb{A}}$ $\stackrel{\wedge}{\mathbb{A}}$ delineators Equal reflectors or spacina spacing delineators (100' max), (100' max), but not but not less than less than 3 total. 3- Type $\mathbf{x}$ $\mathbf{x}$ $\stackrel{\mathsf{H}}{\bowtie}$ $\stackrel{*}{\bowtie}$ 3 total. 3- Type $\stackrel{*}{\bowtie}$ D-SW D-SW delineators MBGF delineators spaced 25' spaced 25' apart $\mathbf{R}$ $\mathbf{x}$ apart $\stackrel{\mathsf{H}}{\bowtie}$ Type D-SW <u>↓</u> ѫ ヌ 土 Edge Line Shoulder Type D-SW delineators delineators bidirectional Edge bidirectional $\stackrel{\wedge}{\mathbb{A}}$ $\Re$ **MBGF** $\stackrel{*}{\bowtie}$ $\stackrel{\wedge}{\mathbb{A}}$ Traffic Safety Division Standard **LEGEND** 25 ft. 25 ft. 25 ft. Texas Department of Transportation $\stackrel{\wedge}{\mathbb{A}}$ Shoul Bidirectional Delineator DELINEATOR & $\mathbf{x}$ Delineator See Note See Note 1 **OBJECT MARKER** PLACEMENT DETAILS NOTE: NOTE: OM-2 D & OM(5) - 201. Terminal ends require reflective 1. Terminal ends require reflective sheeting provided by manufacturer sheeting provided by manufacturer DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO dom5-20.dgn per D & OM (VIA) or a Type 3 per D & OM (VIA) or a Type 3 Terminal End © TxDOT August 2015 JOB Object Marker (OM-3) in front of Object Marker (OM-3) in front CR 316 0908 27 006 the terminal end. of the terminal end. raffic Flow 75

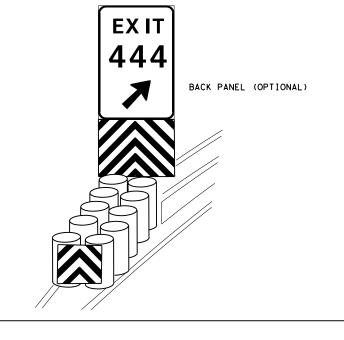
20E

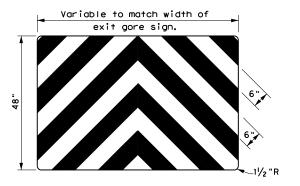
SCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any nd is made by ixDOI for any purpose whatsoever. IxDOI assumes no responsibility for the conversion echès gajagagaqqiqtya athgesfajayatbaet SEar/jinaBGGGAtaypegattagam daggagag resulting from its use.





OBJECT MARKERS SMALLER THAN 3 FT 2





#### NOTES

- Object Markers shall conform to the Texas MUTCD and meet the color and reflectivity requirement of Department Material Specification DMS 8300. Background shall be yellow reflective sheeting (Type B or C) and Chevron shall be black.
- 2. Object Markers may be fabricated from adhesive backed reflective sheeting applied directly to guardrail end treatment, or applied directly to an "end cap" as per the manufacturer's recommendation. Direct applied sheeting shall provide a smooth surface and have no wrinkles, air bubbles, cuts or tears. A radius at the corners is not required for direct applied sheeting.
- 3. Object Marker size may be reduced to fit smaller devices. Width of alternating black and yellow stripes are typically 6". Object Markers smaller than 3ft may have reduced width stripes of a minimum of 2  $\frac{1}{4}$ ".
- 4. Pop rivets, screws, or nuts and bolts may be used to attach object markers and reflectors. Holes, slots or other openings may be cut or drilled through object markers to allow cable or other attachments.
- 5. Object Marker at nose of attenuator is subsidiary to the attenuator.
- 6. See D & OM (1-4) for required barrier reflectors.



Traffic Safety Division Standard

DELINEATOR &
OBJECT MARKER
FOR VEHICLE IMPACT
ATTENUATORS

D & OM(VIA)-20

ILE: domvia20.dgn	DN: TX[	)OT	ck: TXDOT	DW: TXDOT	CK: TXDOT
C)TxDOT December 1989	CONT	SECT	JOB		H I GHWAY
REVISIONS	0908	27	006 CR :		CR 316
4-92 8-04 8-95 3-15	DIST	COUNTY			SHEET NO.
4-98 7-20	ABL	KENT			76
000					•

#### STORMWATER POLLUTION PRVENTION PLAN (SWP3):

This SWP3 has been developed in accordance with TxDOT policy for projects disturbing less than 1 acre of soil, and not part of a larger common plan of development.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept at the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans, and the project's environmental permits, issues, and commitments (EPICs).

#### 1.0 SITE/PROJECT DESCRIPTION

#### 1.1 PROJECT CONTROL SECTION JOB (CSJ):

0908-27-006

#### **1.2 PROJECT LIMITS:**

From: CR 316 AT COOPER CREEK

To: CR 316 AT COOPER CREEK

#### 1.3 PROJECT COORDINATES:

BEGIN: (Lat) 33° 5'32.38"N ,(Long) 100°51'26.52"W

END: (Lat) 33° 5'29.47"N ,(Long) 100°51'24.06"W

1.4 TOTAL PROJECT AREA (Acres): 0.50

1.5 TOTAL AREA TO BE DISTURBED (Acres): 0.32

#### 1.6 NATURE OF CONSTRUCTION ACTIVITY:

REPLACE BRIDGE AND WIDEN APPROACHES

#### 1.7 MAJOR SOIL TYPES:

Soil Type	Description
Clairemont Slit Loam 0 - 1% Slopes	11% Sand, 68% Silt, 21% Clay, Well Drained, Negligible Runoff None - Deposition Erosion

#### 1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

PSLs determined during preconstruction meeting

PSLs determined during preconstruction m
 PSLs determined during construction

- N. Dol

☐ No PSLs	planned for	construction
-----------	-------------	--------------

Туре	Sheet #s
Concrete Washout	80, Enviromental Layout

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

#### 1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.3.)

- Install sediment and erosion controls
- ☑ Blade existing topsoil into windrows, prep ROW, clear and grub
- ⊠ Remove existing pavement
- ☒ Grading operations, excavation, and embankment
- X Excavate and prepare subgrade for proposed pavement widening
- ☐ Remove existing culverts, safety end treatments (SETs)
- X Remove existing metal beam guard fence (MBGF), bridge rail
- ☐ Install proposed pavement per plans
- ☐ Install culverts, culvert extensions, SETs
- ☒ Install mow strip, MBGF, bridge rail

- ☑ Blade windrowed material back across slopes
- Revegetation of unpaved areas
- X Achieve site stabilization and remove sediment and erosion control measures

Other:			

Other:			

#### 1.10 POTENTIAL POLLUTANTS AND SOURCES:

- X Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- □ Solvents, paints, adhesives, etc. from various construction activities
- ☐ Transported soils from offsite vehicle tracking
- Construction debris and waste from various construction activities
- ☐ Contaminated water from excavation or dewatering pump-out water
- X Sanitary waste from onsite restroom facilities
- ☐ Long-term stockpiles of material and waste

O		
ther:		

#### 1.11 RECEIVING WATERS:

**Tributaries** 

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

**Classified Waterbody** 

	_
Cooper Creek	Double Mountain Fork Brazos River (1241); Bacteria in Water (Recreation Use)
No TMDLS Or I-PLAN	S Were Identified
* Add (*) for impaired waterhodies	with pollutant in ()

* Add (*) for impaired waterbodies with pollutant in ().

#### 1.12 ROLES AND RESPONSIBILITIES: TxDOT

- X Development of plans and specifications
- X Perform SWP3 inspections
- $\ensuremath{\mathtt{X}}$  Maintain SWP3 records and update to reflect daily operations

Other	Uner.						
Other:							
Outer.	Other:						-

#### 1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

☐ Other:

Othor.			



STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 1 of 2

Texas Department of Transportation

FED. RD. DIV. NO.		PROJECT NO.				
6		SEE TITLE SHEET			77	
STATE	•	STATE DIST.	COUNTY			
TEXAS	S	ABL	KENT			
CONT.		SECT.	JOB HIGHWAY NO.		NO.	
0908	3	27	006 CR 316		16	

#### STORMWATER POLLUTION PRVENTION PLAN (SWP3):

#### 2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:
T/P
□ □ Temporary Seeding     □ □ Permanent Planting, Sodding or Seeding
<ul> <li>□ Biodegradable Erosion Control Logs</li> <li>□ Rock Filter Dams/ Rock Check Dams</li> </ul>
□ □ Vertical Tracking □ □ Interceptor Swale
<ul> <li>□ X Riprap</li> <li>□ Diversion Dike</li> <li>□ Temporary Pipe Slope Drain</li> <li>□ Embankment for Erosion Control</li> <li>□ Paved Flumes</li> </ul>
□ □ Other: □ □ Other: □
□ □ Other:
2.2 SEDIMENT CONTROL BMPs:
<ul><li>⋈ □ Biodegradable Erosion Control Logs</li><li>□ □ Dewatering Controls</li></ul>
<ul> <li>□ Inlet Protection</li> <li>□ Rock Filter Dams/ Rock Check Dams</li> <li>□ Sandbag Berms</li> </ul>
□ □ Floating Turbidity Barrier □ □ Vegetated Buffer Zones
□ □ Vegetated Filter Strips □ □ Other:
□ □ Other:
□ □ Other:
Refer to the Environmental Layout Sheets/ SWP3 Layout She

located in Attachment 1.2 of this SWP3

#### 2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Type	Stati	oning
Туре	From	То
1		
Refer to the Environmental Layo	ut Sheets/ SWP3	Layout Sheets

located in Attachment 1.2 of this SWP3

#### 2.4 OFFSITE VEHICLE TRACKING CONTROLS:

Excess dirt/mud on road removed daily

<ul> <li>☐ Haul roads dampened for dust control</li> <li>☐ Loaded haul trucks to be covered with tarpaulin</li> <li>☐ Stabilized construction exit</li> </ul>
□ Other:
□ Other:
□ Other:

#### 2.5 POLLUTION PREVENTION MEASURES:

- □ Chemical Management
- X Concrete and Materials Waste Management
- X Debris and Trash Management
- □ Dust Control

☐ Other: _		
□ Other:		
 □ Other: _		

#### **2.6 VEGETATED BUFFER ZONES:**

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Type	Statio	oning
Туре	From	То

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

#### 2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- ★ Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

#### 2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.

#### 2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.



STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 2 of 2

Texas Department of Transportation

FED. RD. DIV. NO.		PROJECT NO.				
6		SEE TITLE SHEET			78	
STATE		STATE DIST.	COUNTY			
TEXA:	S	ABL	KENT			
CONT.		SECT.	JOB HIGHWAY NO.		٧0.	
0908	3	27	ØØ6 CR:		16	

USFWS: U.S. Fish and Wildlife Service

79

ABL

0908

27

006

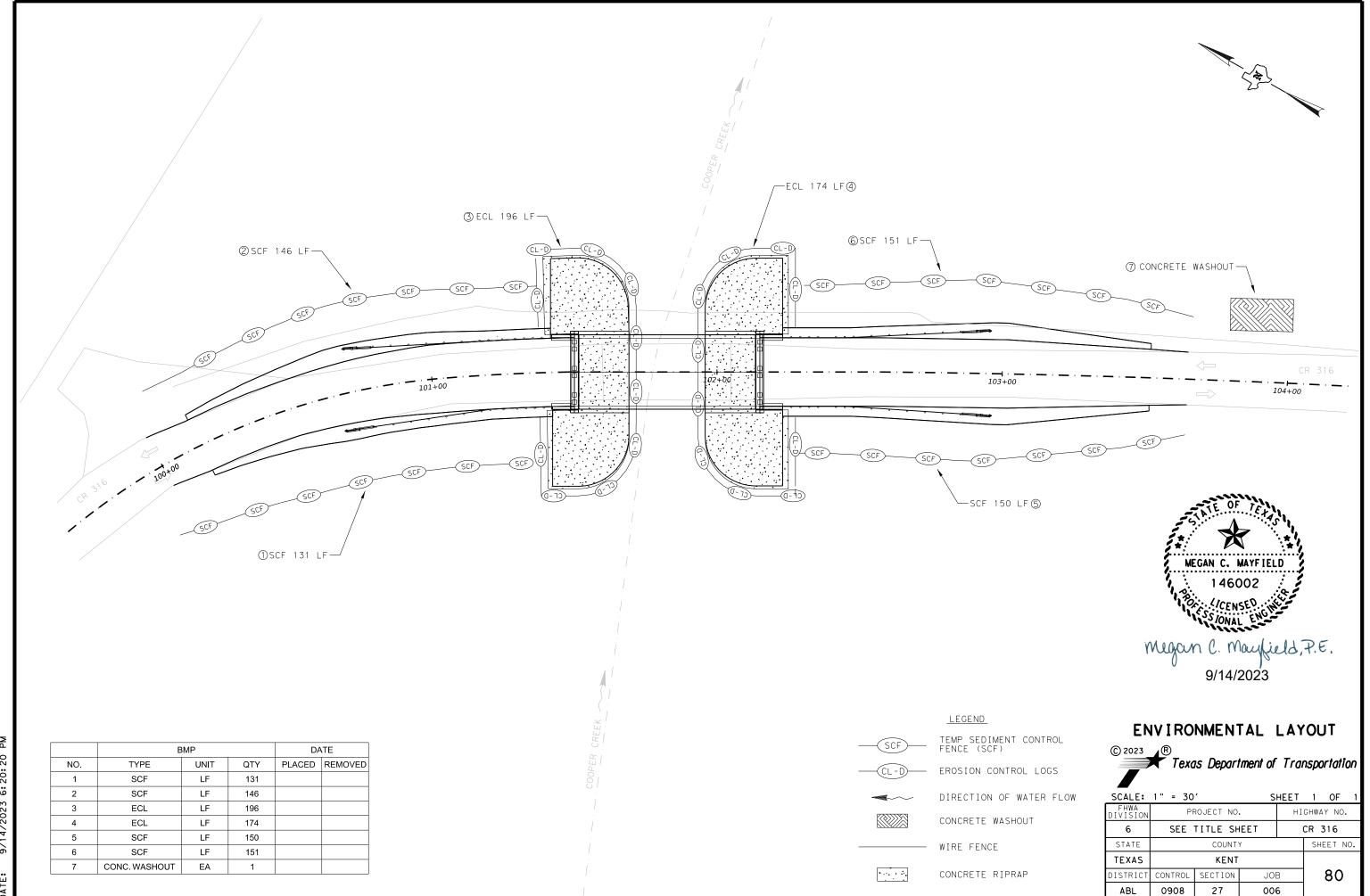
REV. DATE: 02/2015

Construction Exits

Sediment Basins

☐ Grassy Swales

NOI: Notice of Intent



# Embed posts 18" min. or Anchor if in rock. SECTION A-A

#### HINGE JOINT KNOT WOVEN MESH (OPTION) DETAIL

Galvanized hinge joint knot woven mesh (12.5 GA.SWG Min.) requires a minimum of five horizontal wires spaced at a maximum of 12 inches apart and all vertical wires spaced at a maximum of 12 inches apart.

#### SEDIMENT CONTROL FENCE USAGE GUIDELINES

A sediment control fence may be constructed near the downstream perimeter of a disturbed area along a contour to intercept sediment from overland runoff. A 2 year storm frequency may be used to calculate the flow rate to be filtered.

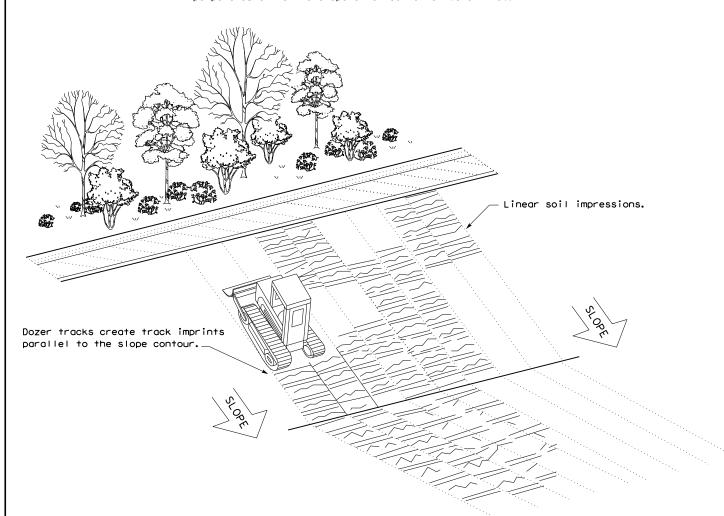
Sediment control fence should be sized to filter a maximum flow through rate of 100 GPM/FT². Sediment control fence is not recommended to control erosion from a drainage area larger than 2 acres.

#### **LEGEND**

Sediment Control Fence —(SCF)—

#### **GENERAL NOTES**

- 1. Vertical tracking is required on projects where soil distributing activities have occurred unless otherwise approved.
- 2. Perform vertical tracking on slopes to temporarily stabilize soil.
- 3. Provide equipment with a track undercarriage capable of producing linear soil impressions measuring a minimum of 12" in length by 2" to 4" in width by 1/2" to 2" in depth.
- 4. Do not exceed 12" between track impressions.
- 5. Install continous linear track impressions where the minimum 12" length impressions are perpendicular to the slope or direction of water flow.



VERTICAL TRACKING



TEMPORARY EROSION. SEDIMENT AND WATER POLLUTION CONTROL MEASURES FENCE & VERTICAL TRACKING

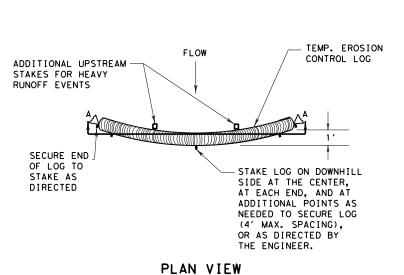
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STAKE LOG ON DOWNHILL

SIDE AT THE CENTER,

AT EACH END, AND AT

AS DIRECTED BY THE

ENGINEER.

ADDITIONAL POINTS AS

NEEDED TO SECURE LOG

(4' MAX. SPACING), OR

ADDITIONAL UPSTREAM

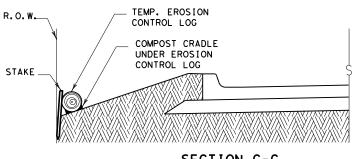
STAKES FOR HEAVY

RUNOFF EVENTS

#### FLOW ADDITIONAL UPSTREAM STAKES FOR HEAVY RUNOFF EVENTS SECURE END OF LOG TO STAKE AS DISTURBED AREA DIRECTED BACK OF CURB LIP OF GUTTER STAKE ON DOWNHILL SIDE OF TEMP. EROSION LOG AT 8' (ON CENTER) MAX. AS NEEDED TO SECURE LOG, CONTROL LOG OR AS DIRECTED BY THE ENGINEER.

#### STAKE ON DOWNHILL SIDE OF LOG AT 8' (ON CENTER) MAX. AS NEEDED TO SECURE LOG, (TYP.) OR AS DIRECTED BY THE ENGINEER. **TEMPORARY** EROSION CONTROL LOG FLOW -DISTURBED AREA SECURE END BACK OF CURB OF LOG TO STAKE AS DIRECTED LIP OF GUTTER ADDITIONAL UPSTREAM STAKES FOR HEAVY RUNOFF EVENTS

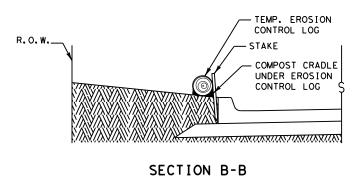
#### PLAN VIEW



SECTION C-C



#### PLAN VIEW



EROSION CONTROL LOG AT BACK OF CURB



### EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY



# SECTION A-A EROSION CONTROL LOG DAM

NIN



#### LEGEND

CL-D EROSION CONTROL LOG DAM

TEMP. EROSION-

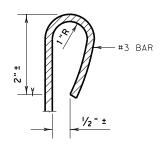
CONTROL LOG

(TYP.)

COMPOST CRADLE UNDER EROSION

CONTROL LOG

- -(cl-boc)- EROSION CONTROL LOG AT BACK OF CURB
- EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY (CL-ROW
- EROSION CONTROL LOGS ON SLOPES STAKE AND TRENCHING ANCHORING -(CL-SST
- EROSION CONTROL LOGS ON SLOPES STAKE AND LASHING ANCHORING -(CL-SSL)
- -(cL-DI)→ EROSION CONTROL LOG AT DROP INLET
- (CL-CI) EROSION CONTROL LOG AT CURB INLET
- (cl-gi)— EROSION CONTROL LOG AT CURB & GRATE INLET



REBAR STAKE DETAIL

Control logs should be placed in the following locations:

- 1. Within drainage ditches spaced as needed or min. 500' on center

- 4. Just before the drainage leaves the right of way
- limits where drainage flows away from the project.

The logs should be cleaned when the sediment has accumulated to a depth of 1/2 the log diameter.

Cleaning and removal of accumulated sediment deposits is incidental and will not be paid for separately.

#### SHEET 1 OF 3

DIAMETER MEASUREMENTS OF EROSION

CONTROL LOGS SPECIFIED IN PLANS

**GENERAL NOTES:** 

1. EROSION CONTROL LOGS SHALL BE INSTALLED IN ACCORDANCE WITH MANFACTURER'S

2. LENGTHS OF EROSION CONTROL LOGS SHALL

BIODEGRADABLE OR PHOTODEGRADABLE

USE RECYCLABLE CONTAINMENT MESH.

STAKES SHALL BE 2" X 2" WOOD OR

THE PURPOSE INTENDED.

3. UNLESS OTHERWISE DIRECTED, USE

ENGINEER.

DEFORMATION.

THE ENGINEER.

MESH.

LOG.

MINIMUM

COMPACTED

DIAMETER

RECOMMENDATIONS, OR AS DIRECTED BY THE

BE IN ACCORDANCE WITH MANUFACTURER'S

RECOMMENDATIONS AND AS REQUIRED FOR

CONTAINMENT MESH ONLY WHERE LOG WILL

SYSTEM. FOR TEMPORARY INSTALLATIONS,

REMAIN IN PLACE AS PART OF A VEGETATIVE

FILL LOGS WITH SUFFICIENT FILTER MATERIAL

TO ACHIEVE THE MINIMUM COMPACTED DIAMETER

SPECIFIED IN THE PLANS WITHOUT EXCESSIVE

#3 REBAR, 2'-4' LONG, EMBEDDED SUCH THAT

2" PROTRUDES ABOVE LOG, OR AS DIRECTED BY

SANDBAGS USED AS ANCHORS SHALL BE PLACED

ON TOP OF LOGS & SHALL BE OF SUFFICIENT

TURN THE ENDS OF EACH ROW OF LOGS UPSLOPE

TO PREVENT RUNOFF FROM FLOWING AROUND THE

UPSTREAM STAKES MAY BE NECESSARY TO KEEP

6. DO NOT PLACE STAKES THROUGH CONTAINMENT

7. COMPOST CRADLE MATERIAL IS INCIDENTAL &

WILL NOT BE PAID FOR SEPARATELY.

10. FOR HEAVY RUNOFF EVENTS, ADDITIONAL

LOG FROM FOLDING IN ON ITSELF.

SIZE TO HOLD LOGS IN PLACE.



MINIMUM

COMPACTED DIAMETER

TEMPORARY EROSION. SEDIMENT AND WATER POLLUTION CONTROL MEASURES

**EROSION CONTROL LOG** 

EC(9) - 16

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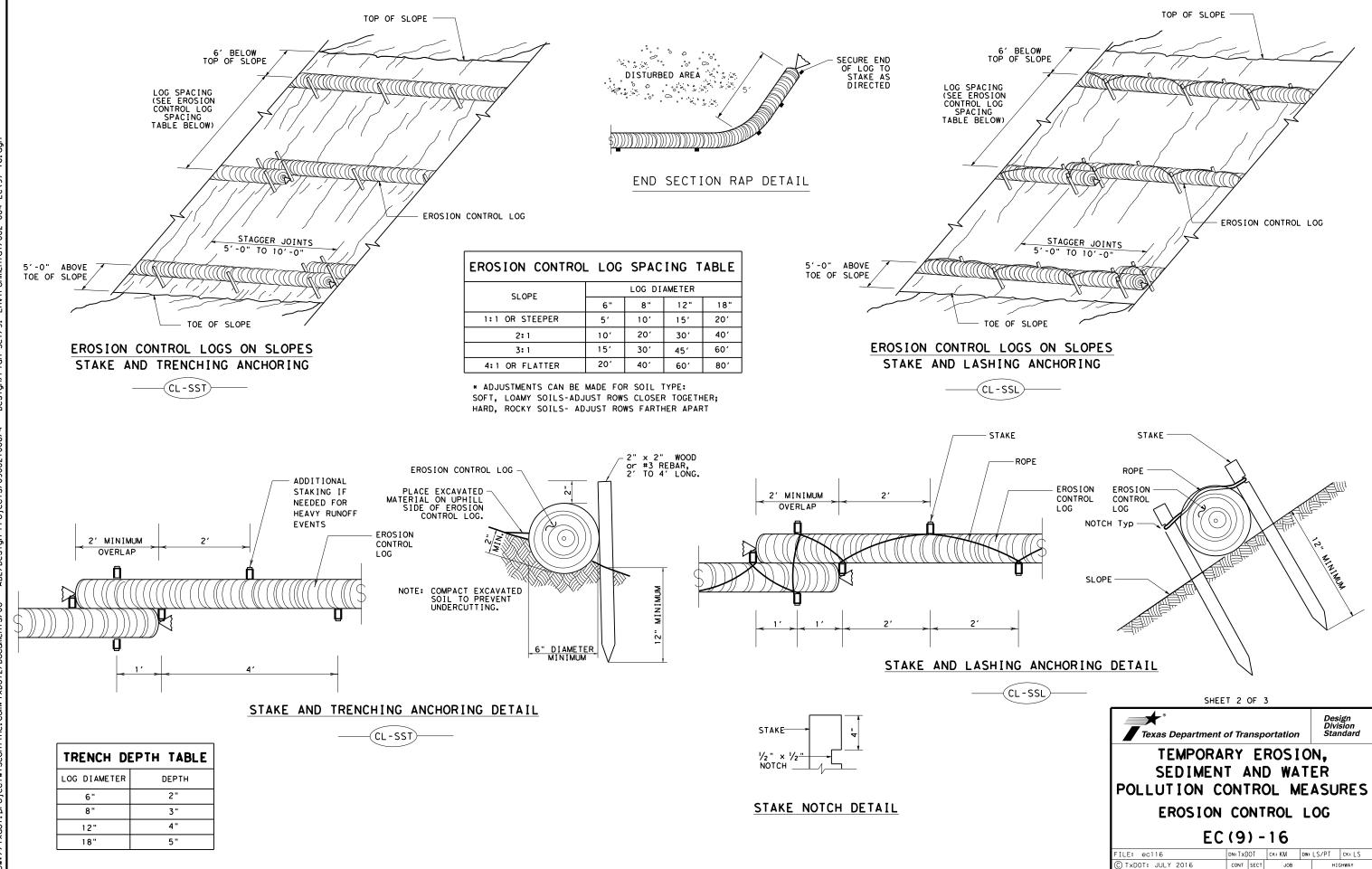
#### SEDIMENT BASIN & TRAP USAGE GUIDELINES

An erosion control log sediment trap may be used to filter sediment out of runoff draining from an unstabilized area.

The drainage area for a sediment trap should not exceed Log Traps: 5 acres. The trap capacity should be 1800 CF/Acre (0.5" over the drainage area).

- 2. Immediately preceding ditch inlets or drain inlets
- 3. Just before the drainage enters a water course
- 5. Just before the drainage leaves the construction





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SECURE END OF LOG TO STAKE AS DIRECTED

TEMP. EROSION-CONTROL LOG

FLOW

(CL - GI)

# CURB AND GRATE INLET

EROSION CONTROL LOG AT DROP INLET

(CL-DÌ

TEMPORARY EROSION CONTROL LOG USE STAKES ON DOWNSTREAM SIDE OF LOGS, AT ENDS, MIDPOINT, & AS NEEDED OR SANDBAGS TO HOLD IN PLACE. SANDBAG

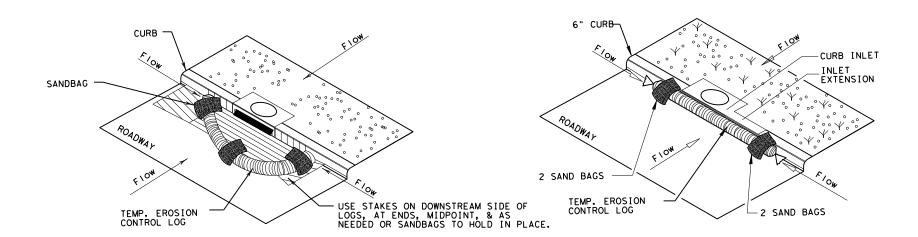
OVERLAP ENDS TIGHTLY 24" MINIMUM

COMPLETELY SURROUND
DRAINAGE ACCESS TO
AREA DRAIN INLETS WITH
EROSION CONTROL LOG

- FLOW

-STAKE OR USE SANDBAGS ON DOWNHILL SIDE OF LOG AS NEEDED TO HOLD IN PLACE (TYPICAL)

#### EROSION CONTROL LOG AT CURB & GRADE INLET



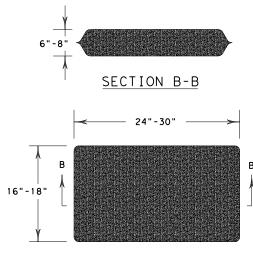
#### EROSION CONTROL LOG AT CURB INLET

#### EROSION CONTROL LOG AT CURB INLET





NOTE: EROSION CONTROL LOGS USED AT CURB INLETS SHOULD ONLY BE USED IF THEY WILL NOT IMPEDE TRAFFIC OR FLOOD THE ROADWAY OR WHEN THE STORM SEWER SYSTEM IS NOT FULLY FUNCTIONAL.



SANDBAG DETAIL

SHEET 3 OF 3



TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES **EROSION CONTROL LOG** 

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