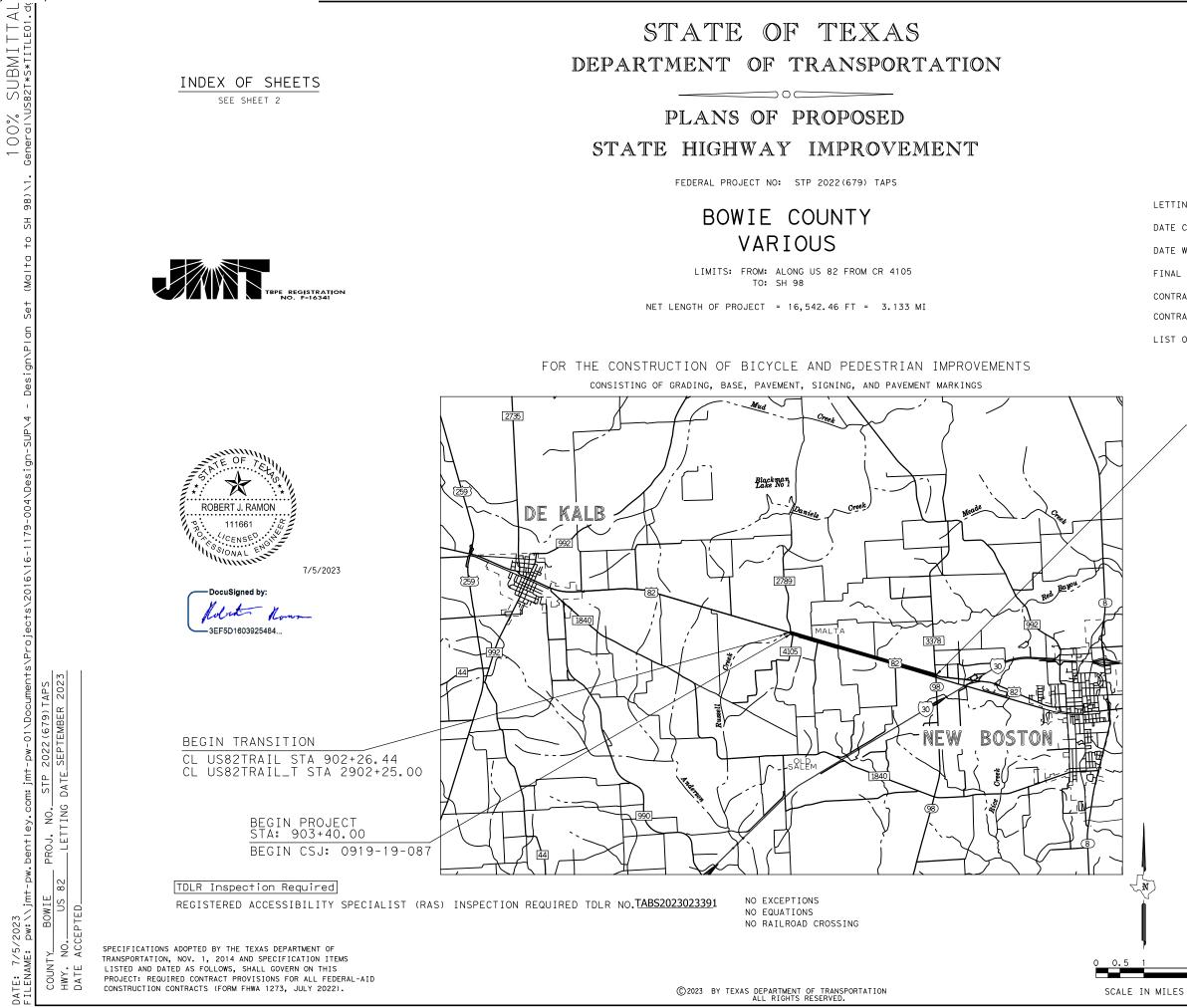
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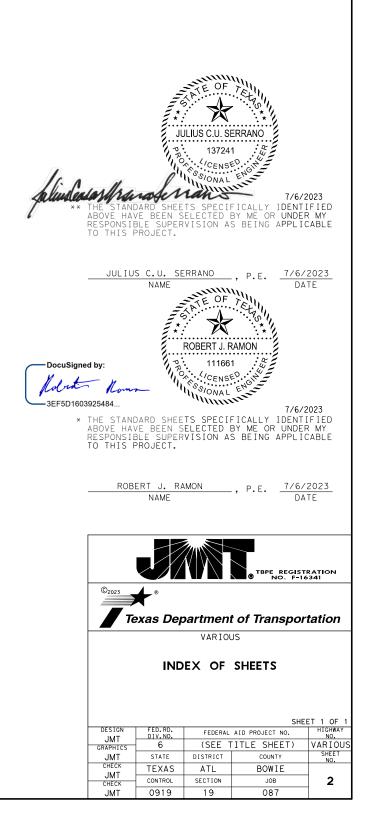
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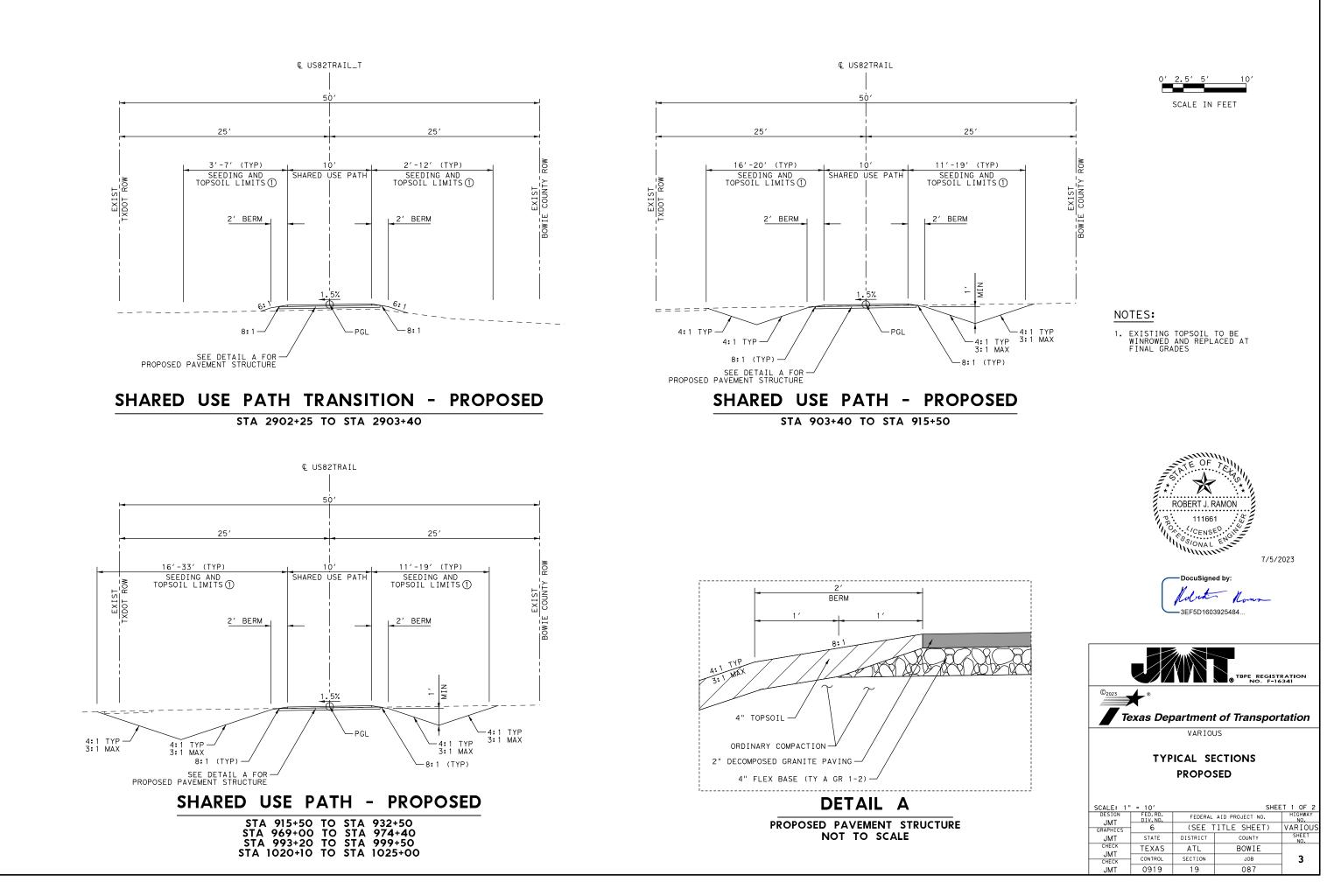


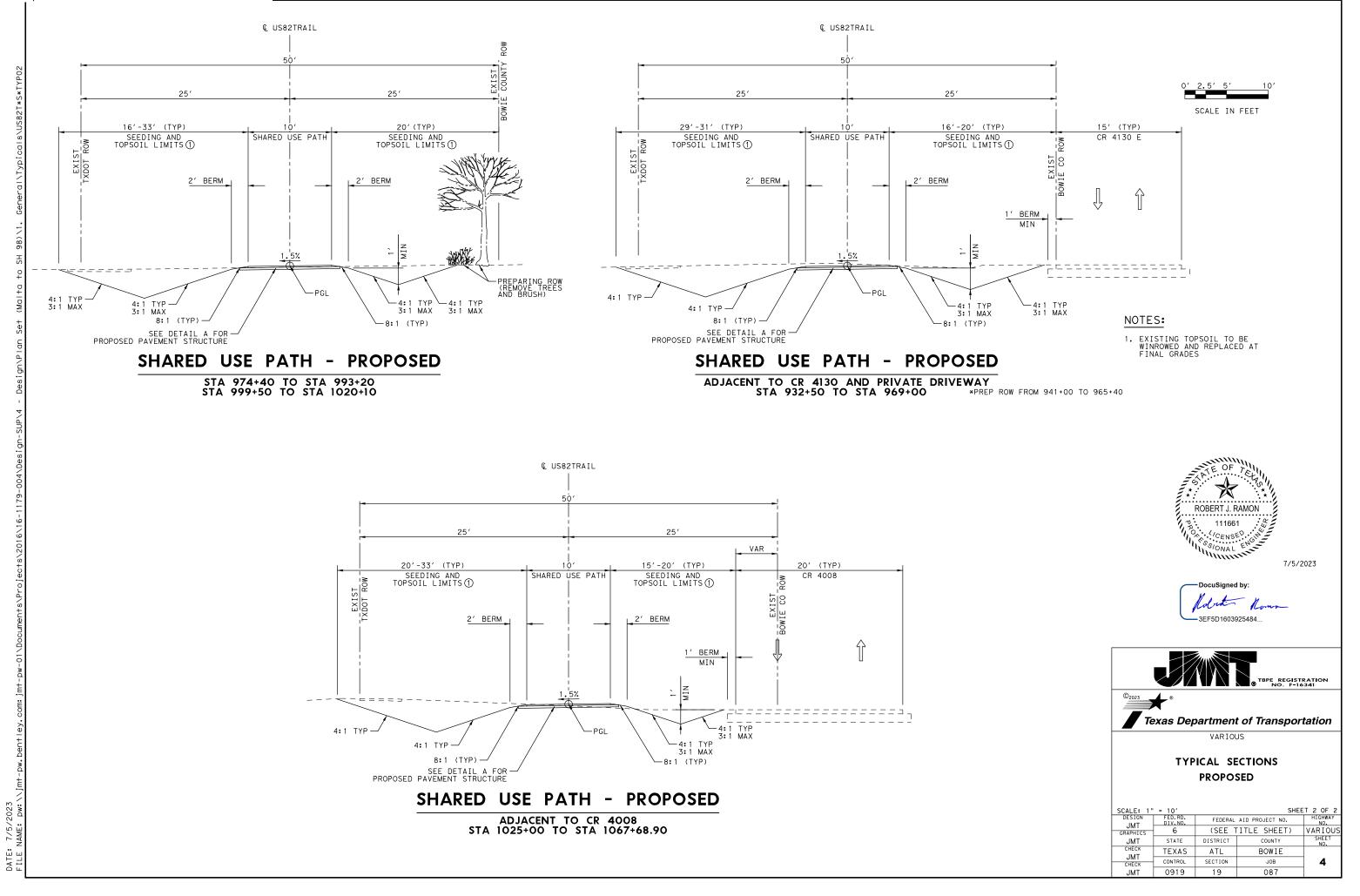
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	FED.RD. DIV.NO.		EDERAL AID PROJECT N		SHEET NO.
	6 state	STP STATE DIST.	2022(679) cour		1
	TEXAS	ATL	BOW		
	CONT.	SECT.	JOB	HIGHWAY	NO.
	0919	19	087	VARIO	SUC
CSJ FACILITY TYP 0919-19-087 SHARED USE PA FINAL PLAN LETTING DATE: DATE CONTRACTOR BEGAN WORK:	ТН	IGN SI 15 MPI			
DATE WORK WAS COMPLETED & ACCEPTED:					
FINAL CONTRACT COST: \$					
CONTRACTOR:					
CONTRACTOR ADDRESS:					
LIST OF APPROVED FIELD CHANGES:					
END PROJECT					
STA: 1067+68.90					
END CSJ: 0919-19-087	-				
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<u>SHEET NO.</u>		DESCRIPTION	SHEET NO.		DESCRIPTION
	١.	GENERAL		VI.	EROSION CONTROL
1		TITLE SHEET	109		ENVIRONMENTAL PERMITS, ISSUES AND COMMITMENTS
2		INDEX OF SHEETS	110-111		STORM WATER POLLUTION PREVENTION PLAN (SWP3)
3-4		TYPICAL SECTIONS PROPOSED	112-127		EROSION CONTROL PLAN SWP3 LAYOUT SHEETS
5, 5A - 5D 6,6A		GENERAL NOTES ESTIMATE & QUANTITY			STANDARDS
7		SUMMARY OF EARTHWORK QUANTITIES			STANDARDS
8		SUMMARY OF SHARED USE PATH QUANTITIES	128		*EC(1)-16
9		SUMMARY OF DRAINAGE QUANTITIES	129		*EC(2)-16
10		SUMMARY OF EROSION CONTROL QUANTITIES	130 131-133		*EC(3)-16 *EC(9)-16
	II.	TRAFFIC CONTROL PLAN			
				VII.	UTILITY RELOCATION
11		TCP CONSTRUCTION SEQUENCE	174		
12 13-17		TCP TYPICAL SECTION TRAFFIC CONTROL PLAN LAYOUTS	1 3 4 135, 135A - 135ZB		TITLE PAGE GENERAL NOTES AND TECHNICAL SPECIFICATIONS
18		TCP MISCELLANEOUS DETAILS PARALLEL CULVERT PHASING	135, 155A - 155ZB 136		SUMMARY OF UTILITY RELOCATION
			137		LINE A
		STANDARDS	138		LINE G & LINE H
19-30		*BC(1)-21 THRU BC(12)-21	1 39 1 40		LINE I LINE L
31		*TCP (1-2) -18	141-143		CONSTRUCTION STANDARDS
32		*TCP(2-1)-18			
33		* TCP (2-2) - 18			
34		*TCP (ATL-21)-14			
	III.	SHARED USE PATH (SUP) DETAILS			
35-37 38		SURVEY CONTROL POINT DATA			
39		HORIZONTAL ALIGNMENT DATA PLAN AND PROFILE SUP TRANSITION			
40-69		PLAN AND PROFILE			
70 71		DITCH SUMMARY TABLE			
72-74		DRIVEWAY GRADING DETAIL MISCELLANEOUS SUP DETAILS			
75		PLUG CULVERT DETAIL			
76		TREE REMOVAL AND TRIMMING DETAILS			
		STANDARDS			
77 78-81		* JS-14 *PED-18			
	IV.	DRAINAGE DETAILS			
82-87		DRAINAGE AREA MAP			
88-89		RUNOFF COMPUTATIONS CROSS CULVERTS			
90		DITCH COMPUTATIONS			
91		CROSS CULVERT SUMMARY TABLE CROSS STREET AND DRIVEWAY CULVERT SUMMARY TABLE			
92 93-99		HYDRAULICS COMPUTATIONS			
		STANDARDS			
100		**PBGC			
101		**PSET-RC			
102		**PSET-RP			
103 104		**PSET-SP **CH-PW-0			
	v .	TRAFFIC ITEMS			
105	۷.	SIGN DETAILS			
105		STANDARDS			
106					
106 107		* SMD (GEN) - 08 * SMD (TWT) - 08			
108		* TSR (4) -13			







GENERAL NOTES:

General Requirements and Covenants:

Contractor questions on this project are to be addressed to the following individuals:

Tommy Bruce – Area Engineer <u>Tommy.Bruce@Txdot.gov</u> Dana Moore – Assistant Area Engineer <u>Dana.Moore@Txdot.gov</u>

Questions may be submitted via the Letting Pre-Bid Q&A web page. This webpage can be accessed from the Notice to Contractors dashboard located at the following Address:

https://tableau.txdot.gov/views/ProjectInformationDashboard/NoticetoContractors?%

All contractor questions will be reviewed by the Engineer. All questions and any corresponding responses that are generated will be posted through the same Letting Pre-Bid Q&A web page.

The Letting Pre-Bid Q&A web page for each project can be accessed by using the dashboard to navigate to the project you are interested in by scrolling or filtering the dashboard using the controls on the left. Hover over the blue hyperlink for the project you want to view the Q&A for and click on the link in the window that pops up.

The project requires TDLR inspection. The design follows PROWAG guidelines for ADA compliance including midblock crossings (R302.6.2) that permits the cross slope to equal the existing cross street and driveway grades.

ITEM 5 – Control of the Work:

Prior to contract letting, bidders may request a free electronic copy of the files that contain the earthwork information from the District Office in Atlanta. If printed copies of the actual cross-sections in addition to, or instead of, the electronic files are requested, prospective bidders may purchase prints of earthwork cross sections from the District Office in Atlanta.

Place construction points, stakes, and marks at intervals of no more than 100 ft., or as directed. Place stakes and marks so as not to interfere with normal maintenance operations.

It is the Contractor's responsibility to verify the accuracy of any department provided control points prior to use.

Control: 0919-09-087 County: Bowie Highway: VARIOUS

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/business/resources/highway/bridge/bridge-publications.html#design

Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

ITEM 6 - Control of Material:

To comply with the latest provisions of Build America, Buy America Act (BABA Act) of the Bipartisan Infrastructure Law, the contractor must submit a notarized original of the TxDOT Construction Material Buy America Certification Form for all items classified as construction materials. This form is not required for materials classified as a manufactured product.

Refer to the Buy America Material Classification Sheet for clarification on material categorization.

The Buy America Material Classification Sheet is located at the below link.

<u>https://www.txdot.gov/business/resources/materials/buy-america-material-classification-sheet.html</u> for clarification on material categorization.

When requesting payments for material on hand, contractor's material storage facility will be within the Atlanta District.

Pre-qualified products can be found at <u>http://www.txdot.gov/business/resources/producer-list.html</u>

ITEM 7 – Legal Relations and Responsibilities:

This project is covered by a U.S. Army Corps of Engineers Nationwide 14 permit with no coordination. Obtain a copy of permit and conditions at the Engineer's office.

The total area disturbed for this project is 21.38 acres. The disturbed area in this project, all project locations in the Contract, and the Contractor project specific locations (PSLs) within 1 mile of the project limits will be used to establish the authorization requirements for storm water discharges. The Department will obtain an authorization to discharge storm water from the Texas Commission on Environmental Quality (TCEQ) for the construction activities shown on the plans. The Contractor is to obtain required authorization from the TCEQ for Contractor PSLs for construction support activities on or off the ROW. When the total area disturbed in the Contract and PSLs

General Notes

Sheet:

within 1 mile of the project limits exceeds 5 acres, provide a copy of the Contractor NOI for PSLs on the ROW to the Engineer and to the local government that operates a separate storm sewer system.

The Contractor will not remove active nests from bridges and other structures during nesting season of the birds associated with the nests.

Transmit copies of correspondence between Contractor and resource agencies as listed in Article 7.7 "Preservation of Cultural and Natural Resources and the Environment".

The following significant traffic generator events have been identified:

• Run the Rail – Marathon and Half-Marathon is held on the second Saturday of October 2023.

Traffic handling for Run the Rail – Marathon and Half-Marathon will be the responsibility of the event organizer and will require coordination with construction activities.

ITEM 8 – Prosecution and Progress:

Working days will be charged in accordance with Section 8.3.1.4.

Provide progress schedules meeting the requirements of Section 8.5.2 in 2014 Standard Specifications for Construction and Maintenance of Highways, Streets, and Bridges.

The Contractor will not have the option of working on the second Saturday of October while the Run the Rail - Marathon and Half-Marathon is underway. No working day will be charged on this day if the principal item of work cannot be started by 11:00 A.M.

ITEM 100 – Preparing Right of Way:

Do not burn trash, debris, etc. within the City limits without prior written city approval.

Preparing Right of Way limits are called out on Shared Use Path Plan & Profile sheets. Preparing Right of Way quantifies length of tree lines to be removed. See Tree Removal and Trimming Details for tree removal requirements. Other removal items are quantified individually.

Remove debris, vegetation, and other landscape features not designated for preservation, whether above or below ground.

Removal of live utility facilities is not included in this Item.

ITEM 110 - Excavation:

Compact subgrade in earth cut sections, in accordance with section 132.3.4.1.

General Notes

Sheet:

Control: 0919-09-087 County: Bowie **Highway:** VARIOUS

As cut slopes are constructed, round off the tops of back slopes to blend into the natural ground.

Excavation of existing stabilized materials will be measured and paid for as excavation (roadway).

Remove abandoned underground utility lines encountered. This work will be subsidiary to the pertinent bid items.

Flare ditches to prevent erosion of the toe of slope in areas of transition from cut to fill.

ITEM 132 – Embankment:

Furnish material with an organic content less than 1.0%. The Engineer will test using UV-VIS equipment and procedure determined by TxDOT. Allow two weeks for testing.

Compact subgrade in earth cut sections, in accordance with section 132.3.4.1.

Test borrow sources and furnish results to the Engineer.

Remove deleterious material, organic matter, and sediment, etc., from all ponds, lakes, sloughs, channels, and existing roadway ditches prior to placement of embankment. This work will be subsidiary to this item.

Drill or dig one or more holes for thickness measurement, refill, and re-compact material at the location and frequency as directed. This work is considered subsidiary to this item.

Beginning with the final lift of embankment, measure the cross slope during pavement structure operations, at the completion of each land, and prior to covering with another course or lift to ensure that the cross slope is uniform and in compliance with the cross slope shown in the plans. Measure the cross slope at a minimum frequency of one measurement every 100 feet. The number of measurements may be reduced by demonstrating consistently acceptable results, with the approval of the Engineer. Furnish a digital measuring device approved by the Engineer for the measurement of cross slope. Make this measuring device available at the jobsite for the Engineer's use. Report the cross slope to the nearest 0.1%. Record all measurements on an approved form signed and dated certifying correct and submit to the Engineer the next working day for documentation. The Engineer will determine the number of verification measurements.

ITEM 160 – Topsoil:

Windrow topsoil from the right of way. Finish slopes with a tracked vehicle running vertically up and down the slope.

Spread topsoil before placement of the surface course. Re-dress topsoil after placement of the surface course.

Control: 0919-09-087 County: Bowie **Highway:** VARIOUS

Mow tall growing vegetation as directed, to provide optimum growing conditions for temporary or permanent seeded areas in accordance with Item 730 "Roadside Mowing" except for measurement and payment. This work will be subsidiary to pertinent bid items.

Repair topsoil, damaged by causes other than the Contractor's operations, as directed using topsoil, seeding, and fertilizer. This work will be measured and paid for in accordance with the applicable bid items of the contract.

ITEM 162 – Sodding for Erosion Control:

Mow tall growing vegetation as directed, to provide optimum growing conditions for temporary or permanent seeded areas in accordance with Item 730 "Roadside Mowing" except for measurement and payment. This work will be subsidiary to pertinent bid items.

Repair mulch sod, damaged by causes other than the Contractor's operations, as directed using mulch sod, seeding, and fertilizer. This work will be measured and paid for in accordance with the applicable bid items of the contract.

ITEM 164 – Seeding for Erosion Control:

PERMANENT PLANTING MIXTURE

Species and Rates (lb. PLS/ac.)

(Season: February 1 to May 15) Green Sprangletop 0.4 Bermudagrass 2.4 Sand Lovegrass 1.0 Lance-Leaf Coreopsis 1.25

(Season: September 1 to November 30) Bermuda (Unhulled) 12 Crimson Clover 10

TEMPORARY SEEDING FOR EROSION CONTROL

Warm Season (Season: May 15 to August 31) Bermudagrass 6 Foxtail Millet 34

Cool Season (Season: September 1 to November 30) Tall Fescue 4.5

Oats	24
Wheat	34

Sheet:

Control: 0919-09-087 County: Bowie **Highway:** VARIOUS

Adjust the seeding mixture and rates if directed.

Inoculate crimson clover seed with a legume inoculant. Sow inoculated seed dry, with either hand operated or mechanical equipment, after the fertilizer is placed.

Do not use Bahia grass.

Use crimper immediately after spreading mulch. Apply ballast to machine to achieve an anchoring depth of 2 to 3 inches to form soil-binding mulch and to prevent loss or bunching of the mulch by wind. Anchor the machine to prevent the formation of ridges and ruts. Use coulters at least ten inches in diameter. Traverse slopes horizontally. The number of passes needed, not to exceed three, will be as directed. In areas where an anchoring machine cannot be used, the Department will require a tacking agent be used in the mulch as directed.

Use broadcast seeding for temporary erosion control, when and as directed. This will not be paid for directly but is subsidiary to the various bid items.

Use additional temporary seeding if permanent seeding is placed outside the optimum growing season shown for this item, if directed.

Finish slopes with a tracked vehicle running vertically up and down the slope.

Mow tall growing vegetation as directed, to provide optimum growing conditions for temporary or permanent seeded areas in accordance with Item 730 "Roadside Mowing" except for measurement and payment. This work will be subsidiary to pertinent bid items.

Repair topsoil, damaged by causes other than the Contractor's operations, as directed using topsoil, seeding, and fertilizer. This work will be measured and paid for in accordance with the applicable bid items of the contract.

ITEM 166 - Fertilizer:

When seeding between September 1 and January 1, place one-half of the amount of fertilizer specified for seeding with the seeds and place the remainder the following spring unless otherwise directed. When seeding is placed between January 1 and June 1, place one-half the amount of fertilizer specified for seeding with the seeds and place the remainder 30 days later unless otherwise directed.

Apply fertilizer (13-13-13) at a rate of 300 lbs. /5000 sq. yds.

ITEM 247 – Flexible Base:

Drill or dig one or more holes for thickness measurement, refill, and re-compact material at the location and frequency as directed. This work is considered subsidiary to this item.

General Notes

Control: 0919-09-087 County: Bowie **Highway:** VARIOUS

Furnish material with an organic content less than 1.0%. The Engineer will test using UV-VIS equipment and procedure determined by TxDOT. Allow two weeks for testing.

The Engineer will test each stockpile. A minimum of 14 days will be required for testing after stockpile has been sampled.

Do not use iron ore.

Measure the cross slope during pavement structure operations, at the completion of each land, and prior to covering with another course or lift to ensure that the cross slope is uniform and in compliance with the cross slope shown in the plans. Measure the cross slope at a minimum frequency of one measurement every 100 feet. The number of measurements may be reduced by demonstrating consistently acceptable results, with the approval of the Engineer. Furnish a digital measuring device approved by the Engineer for the measurement of cross slope. Make this measuring device available at the jobsite for the Engineer's use. Report the cross slope to the nearest 0.1%. Record all measurements on an approved form signed and dated certifying correct and submit to the Engineer the next working day for documentation. The Engineer will determine the number of verification measurements.

Moist cure the layer by sprinkling in accordance with ITEM 204, "Sprinkling" until primed or the next successive course is placed. The Engineer will measure the moisture content in the upper two inches of the layer using Tex-115E Part I, Nuclear Gauge Method. When the moisture content at any location within a land is more than 2 percent points below optimum the Contractor will prime or cover with the next successive course within three days unless approved otherwise.

Furnish clean 5-gallon plastic buckets with lids and wire handles for sampling, transporting, and shipping aggregate and base to the District Lab.

ITEM 251 – Reworking Base Courses:

Stockpile salvaged base material within Bowie County right-of-way.

Drill or dig one or more holes for thickness measurement, refill, and re-compact material at the location and frequency as directed. This work is considered subsidiary to this item.

Moist cure the layer by sprinkling in accordance with ITEM 204, "Sprinkling" until primed or the next successive course is placed. The Engineer will measure the moisture content in the upper two inches of the layer using Tex-115E Part I, Nuclear Gauge Method. When the moisture content at any location within a land is more than 2 percent points below optimum the Contractor will prime or cover with the next successive course within three days unless approved otherwise.

Sheet:

Control: 0919-09-087 County: Bowie **Highway:** VARIOUS

ITEM 421 – Hydraulic Cement Concrete:

The Department will furnish and maintain concrete compressive strength testing equipment.

Elevate curing tanks as directed.

The Engineer will determine flow lines of pipes under private driveways. When a curing tank is provided the following information must be provided. All items must always be clearly legible and visible from all directions.

- Provide a copy of the SDS sheet for the lime in use.
- and Underwriters Laboratories Inc. requirements.

ITEM 464 – Reinforced Concrete Pipe:

Backfill driveway culverts to obtain a minimum cover of 6 inches. Place backfill in accordance with section 132.3.4.1 "Ordinary Compaction" using approved equipment.

ITEM 467 – Safety End Treatments:

Provide precast safety end treatments with a toewall measuring at least 12 inches. Construct toewalls for cast-in-place safety end treatments as shown in the plans.

Remove trees, bushes, and underbrush as directed. This work will be subsidiary to the pertinent bid items.

ITEM 496 – Removing Structures:

All structures removed under this item become the property of the contractor.

Remove culverts, storm sewers, manholes, and inlets in proper sequence to maintain traffic and drainage.

• Post and maintain the message "Caution Lime Solution, Eye and Skin Irritant".

• Provide the personal protective equipment (PPE) listed below for Department use only: Face shield, a pair of chemical gloves at least 18 inches in length and a chemical apron. Store the SDS sheet and PPE in a clean dry location adjacent to the curing tank. • Provide an eye wash station capable of providing a 15-minute flush as required by the United States Occupational Safety and Health Administration (OSHA). The eye wash station shall be located within ten feet of the curing tank. When a tank heater is required ensure that all electrical wiring, receptacles, and devices meet National Electrical Code

Control: 0919-09-087 **County:** Bowie **Highway:** VARIOUS

ITEM 502 – Barricades, Signs, and Traffic Handling:

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

The Contractor's responsible person (CRP) will be responsible for ensuring that the signs and traffic control devices are in place and functioning properly.

The CRP will inspect and ensure any deficiencies are corrected each and every day throughout the duration of this contract. Notify the Engineer in writing of the name, address, and telephone number of this employee or these employees.

Maintenance of driveways and intersections will not be paid for directly but is subsidiary to the pertinent bid items.

Restrict the movement of equipment across traffic lanes to an absolute minimum.

Use strobe lights or rotating beacons on all motorized equipment, operating on or adjacent to the road surface.

No equipment will be left within 30 feet of the travel way. Equipment and/or obstructions within 30 feet of the travel way will be removed.

ITEM 506 – Temporary Erosion, Sedimentation, and Environmental Controls:

Sprinkle water for dust control. Meet the requirements of Item 204, "Sprinkling" except for measurement and payment. Sprinkling will be considered subsidiary to this Item.

ITEM 644 – Sign Identification Decals:

Type A signs will be made of flat aluminum.

Existing sign assemblies will be removed after the proposed sign is installed. Contractor will leave existing sign in place while proposed sign goes up. The existing sign will be removed immediately after the proposed sign is installed.

Control: 0919-09-087 **County:** Bowie **Highway:** VARIOUS

For this project, the standard triangular slip base two bolt casting will be used. This casting must be furnished from an approved manufacturer.

Texas MUTCD, as directed and as shown on the plans.

ITEM 6185 – Truck Mounted Attenuator (TMA) and Trailer Attenuator **(TA):**

The shadow vehicle with truck mounted attenuator (TMA) will not be optional but will be required as shown on the appropriate traffic control plan sheets.

A total of one (1) shadow vehicle with TMA will be required for work. The contractor will be responsible for determining if one or more of these operations will be ongoing at the same time to determine the total number of TMA's needed for the project.

SPECIFICATION DATA TEST TO BE IN ACCORDANCE WITH DEPARTMENT OF **TRANSPORTATION TEST METHODS**

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ITEM DESCRIPTION Embankment (Type C) 132

Sheet:

- Erect the proposed signs an appropriate distance from adjacent signs in accordance with the
- Verify the elevation difference between the edge of the travel lane and bottom of the sign.
- Do not remove existing sign assemblies until signs are ready to be installed on new mounts.

GRAD	ING REO	QUIREN	1ENTS			
ERCEN	NT RETA	AINED -	SIEVES	SOIL C	CONST	ANTS
				L.L		P.I.
-1/2"	1-3/4"	No. 4	No. 40	MAX.	MAX.	MIN.
				50	25	4



CONTROLLING PROJECT ID 0919-19-087

Estimate & Quantity Sheet

DISTRICT Atlanta

HIGHWAY Various

COUNTY Bowie

		CONTROL SECTIO	CONTROL SECTION JOB		-087	0919-1	L9-094		
		PROJ	PROJECT ID		019	R000	L3260		
		C	DUNTY	Bowi	e			TOTAL EST.	TOTAL FINAL
		ніс	HWAY	Vario	us				FINAL
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL	EST.	FINAL		
	100-6002	PREPARING ROW	STA	63.800				63.800	
	100-6004	PREPARING ROW(TREE)(12" TO 24" DIA)	EA	4.000				4.000	
	105-6011	REMOVING STAB BASE AND ASPH PAV (2"-6")	SY	106.000				106.000	
	110-6001	EXCAVATION (ROADWAY)	CY	16,981.000				16,981.000	
	132-6005	EMBANKMENT (FINAL)(ORD COMP)(TY C)	CY	3,364.000				3,364.000	
	160-6001	FURNISHING AND PLACING TOPSOIL (4")	STA	165.500				165.500	
	162-6002	BLOCK SODDING	SY	137.000				137.000	
	164-6054	BOND FBR MTRX SEED (PERM)(RURAL)(SAND)	SY	68,338.000				68,338.000	
	164-6055	BONDED FBR MTRX SEED (TEMP)(WARM)	SY	34,169.000				34,169.000	
	164-6056	BONDED FBR MTRX SEED (TEMP)(COOL)	SY	34,169.000				34,169.000	
	168-6001	VEGETATIVE WATERING	MG	1,093.300				1,093.300	
	169-6004	SOIL RETENTION BLANKETS (CL 1) (TY D)	SY	1,614.000				1,614.000	
	247-6041	FL BS (CMP IN PLC)(TYA GR1-2)(FNAL POS)	CY	2,194.000				2,194.000	
	251-6478	REWORK BS MTL (TY B) (9-18") (ORD COMP)	SY	101.000				101.000	
	400-6006	CUT & RESTORING PAV	SY	325.000				325.000	
	401-6001	FLOWABLE BACKFILL	CY	3.000				3.000	
	402-6001	TRENCH EXCAVATION PROTECTION	LF	86.000				86.000	
	420-6010	CL A CONC (PLUG)	EA	2.000				2.000	
	464-6003	RC PIPE (CL III)(18 IN)	LF	436.000				436.000	
	464-6005	RC PIPE (CL III)(24 IN)	LF	468.000				468.000	
	464-6008	RC PIPE (CL III)(36 IN)	LF	48.000				48.000	
	466-6097	HEADWALL (CH - PW - 0) (DIA= 24 IN)	EA	1.000				1.000	
	467-6358	SET (TY II) (18 IN) (RCP) (4: 1) (C)	EA	10.000				10.000	
	467-6363	SET (TY II) (18 IN) (RCP) (6: 1) (P)	EA	8.000				8.000	
	467-6389	SET (TY II) (24 IN) (RCP) (3: 1) (P)	EA	2.000				2.000	
	467-6390	SET (TY II) (24 IN) (RCP) (4: 1) (C)	EA	4.000				4.000	
	467-6395	SET (TY II) (24 IN) (RCP) (6: 1) (P)	EA	10.000				10.000	
	467-6448	SET (TY II) (36 IN) (RCP) (3: 1) (C)	EA	2.000				2.000	
	467-6450	SET (TY II) (36 IN) (RCP) (4: 1) (C)	EA	2.000				2.000	
	496-6006	REMOV STR (HEADWALL)	EA	4.000				4.000	
	496-6007	REMOV STR (PIPE)	LF	238.000				238.000	
	500-6001	MOBILIZATION	LS	1.000				1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	MO	12.000				12.000	
	506-6002	ROCK FILTER DAMS (INSTALL) (TY 2)	LF	258.000				258.000	
	506-6003	ROCK FILTER DAMS (INSTALL) (TY 3)	LF	18.000				18.000	
	506-6011	ROCK FILTER DAMS (REMOVE)	LF	276.000				276.000	
	506-6020	CONSTRUCTION EXITS (INSTALL) (TY 1)	SY	877.000				877.000	



DISTRICT	COUNTY	CCSJ	SHEET
Atlanta	Bowie	0919-19-087	6



CONTROLLING PROJECT ID 0919-19-087

Estimate & Quantity Sheet

DISTRICT Atlanta

HIGHWAY Various

COUNTY Bowie

		CONTROL SECTIO	ON JOB	0919-19	-087	0919-19	-094		
		PROJ	PROJECT ID		019	R00013	260		
		C	OUNTY	Bowi	e			TOTAL EST.	TOTAL FINAL
		ніс	HIGHWAY Various						FINAL
٩LT	BID CODE	DESCRIPTION		EST.	FINAL	EST.	FINAL		
	506-6024	CONSTRUCTION EXITS (REMOVE)	SY	877.000				877.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	5,669.000				5,669.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	5,669.000				5,669.000	
	506-6041	BIODEG EROSN CONT LOGS (INSTL) (12")	LF	740.000				740.000	
	506-6043	BIODEG EROSN CONT LOGS (REMOVE)	LF	740.000				740.000	
	531-6001	CONC SIDEWALKS (4")	SY	296.000				296.000	
	644-6060	IN SM RD SN SUP&AM TYTWT(1)WS(P)	EA	10.000				10.000	
	3099-6001	DECOMPOSED GRANITE PAVING MATERIALS	SY	17,891.000				17,891.000	
	5084-6002	INSTALL REMOVABLE BOLLARD	EA	28.000				28.000	
	6185-6002	TMA (STATIONARY)	DAY	185.000				185.000	
	7332-6001	MOBILIZATION	LS			1.000		1.000	
	7332-6002	1 1/2" C901 DR 9 POLYETHELENE TUBING	LF			115.000		115.000	
	7332-6008	8" C900 DR 18 PVC	LF			653.000		653.000	
	7332-6009	8" RJ C900 DR 18 PVC	LF			340.000		340.000	
	7332-6014	CONCRETE ENCASEMENT OF PIPE	LF			234.000		234.000	
	7332-6015	6" TAPPING SLEEVE AND VALVE	LF			1.000		1.000	
	7332-6018	5 1/4" FLUSH ASSY WITH VALVE & FITTINGS	LF			1.000		1.000	
	7332-6019	TESTING/FLUSHING LOCATION	EA			0.780		0.780	
	7332-6020	DUCTILE IRON FITTINGS	TON			1.000		1.000	
	7332-6021	CUST SVCS TRANSFER & METER RELO	EA			1.000		1.000	
	7332-6024	ASPHALT DRIVEWAY REPAIR	LF			1.000		1.000	
	7332-6025	RESEEDING DISTURBED AREAS	LF			234.000		234.000	
	7332-6029	3" RJ SDR 21 CL 200 PVC	LF			236.000		236.000	
	7332-6033	8" DIRECTIONAL DRILL	LF			73.000		73.000	
	7332-6036	6" STEEL ENCASEMENT	LF			220.000		220.000	
	7332-6037	6" DRY BORE	LF			220.000		220.000	
	7332-6042	14" STEEL ENCASEMENT	LF			251.000		251.000	
	7332-6043	14" DRY BORE	LF			249.000		249.000	
	7332-6044	SAND EMBEDMENT	LF			35.100		35.100	
	7332-6046	ABAN EX 6" WATERLINE WITH CELLULAR GRT	LF			1,125.000		1,125.000	
	7332-6048	ABAN EX 2" WATERLINE WITH CELLULAR GRT	LF			75.000		75.000	
	18	EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART)	LS	1.000				1.000	
		SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING)	LS	1.000				1.000	

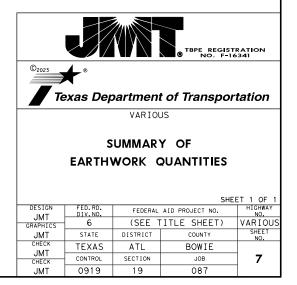


DISTRICT	COUNTY	CCSJ	SHEET
Atlanta	Bowie	0919-19-087	6A

SUMM	ARY OF E		ITEMS
		110	132
		6001	6005
FROM STA	TO STA	EXCAVATION (ROADWAY)	EMBANKMENT (FINAL) (ORD COMP (TY C)
		CY	CY
CSJ 0919	9-19-087		
902+00.00	903+00.00	7	21
903+00.00	904+00.00		88
904+00.00	905+00.00	17	128
905+00.00	906+00.00	33	49
906+00.00	907+00.00	45	11
907+00.00	908+00.00	60	2
908+00.00 909+00.00	<u>909+00.00</u> 910+00.00	67 75	2
910+00.00	911+00.00	71	7
911+00.00	912+00.00	57	7
912+00.00	913+00.00	57	2
913+00.00	914+00.00	59	1
914+00.00	915+00.00	52	1
915+00.00	916+00.00	27	8
916+00.00	917+00.00	29	25
917+00.00	918+00.00	71	18
918+00.00	919+00.00	103	2
919+00.00 920+00.00	<u>920+00.00</u> 921+00.00	99 81	3
920+00.00	922+00.00	81	3
922+00.00	923+00.00	92	1
923+00.00	924+00.00	98	0
924+00.00	925+00.00	91	Ő
925+00.00	926+00.00	80	1
926+00.00	927+00.00	77	1
927+00.00	928+00.00	77	1
928+00.00	929+00.00	78	1
929+00.00	930+00.00	79	1
930+00.00 931+00.00	931+00.00	71	3
932+00.00	<u>932+00.00</u> 933+00.00	66 74	3
933+00.00	934+00.00	55	5
934+00.00	935+00.00	28	7
935+00.00	936+00.00	31	11
936+00.00	937+00.00	36	24
937+00.00	938+00.00	43	18
938+00.00	939+00.00	61	1
939+00.00	940+00.00	68	3
940+00.00 941+00.00	941+00.00 942+00.00	55 47	13
942+00.00	943+00.00	47	8
943+00.00	944+00.00	59	4
944+00.00	945+00.00	125	1
945+00.00	946+00.00	161	1
946+00.00	947+00.00	140	2
947+00.00	948+00.00	131	6
948+00.00	949+00.00	136	6
949+00.00 950+00.00	<u>950+00.00</u> 951+00.00	146	9
951+00.00	952+00.00	132	18
952+00.00	953+00.00	158	11
953+00.00	954+00.00	149	3
954+00.00	955+00.00	131	3
955+00.00	956+00.00	75	1
956+00.00	957+00.00	90	0
957+00.00	958+00.00	161 155	0
958+00.00 959+00.00	959+00.00 960+00.00	174	0
960+00.00	961+00.00	166	3
961+00.00	962+00.00	119	12
962+00.00	963+00.00	101	19
963+00.00	964+00.00	105	19
964+00.00	965+00.00	122	19
965+00.00	966+00.00	146	18
966+00.00	967+00.00	1 <u>49</u> 1 2 4	<u>11</u> 5
967+00.00 968+00.00	968+00.00 969+00.00	131	2
969+00.00	970+00.00	79	3
970+00.00	971+00.00	73	4
971+00.00	972+00.00	153	2
972+00.00	973+00.00	154	2
973+00.00	974+00.00	138	1
974+00.00	975+00.00	96	18
975+00.00	976+00.00	108	18
976+00.00	977+00.00	184	0
977+00.00	978+00.00	208	0
978+00.00	979+00.00	228	0
979+00.00	980+00.00	270 257	0
980+00.00 981+00.00	<u>981+00.00</u> 982+00.00	124	0
982+00.00	983+00.00	78	8
983+00.00	984+00.00	155	17
984+00.00	985+00.00	188	9

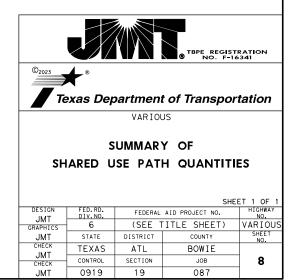
SUMM	ARY OF EA		ITEMS
		110	132
FROM STA	TO STA	6001 EXCAVATION (ROADWAY)	6005 EMBANKMENT (FINAL) (ORD COMP (TY C)
		СҮ	СҮ
CSJ 0919	9-19-087		
987+00.00	988+00.00	190	0
988+00.00 989+00.00	989+00.00 990+00.00	174	0
990+00.00	991+00.00	108	59
991+00.00	992+00.00	120	93
992+00.00	993+00.00	139	59
993+00.00 994+00.00	994+00.00 995+00.00	156 188	44 82
995+00.00	996+00.00	157	132
996+00.00	997+00.00	53	100
997+00.00 998+00.00	998+00.00	<u>6</u> 19	23
999+00.00	999+00.00 1000+00.00	105	26
1000+00.00	1001+00.00	128	5
1001+00.00	1002+00.00	83	2
1002+00.00 1003+00.00	1003+00.00 1004+00.00	<u>92</u> 117	1 4 30
1004+00.00	1005+00.00	127	58
1005+00.00	1006+00.00	131	73
1006+00.00 1007+00.00	1007+00.00 1008+00.00	1 49	61 39
1008+00.00	1009+00.00	263	17
1009+00.00	1010+00.00	274	18
1010+00.00	1011+00.00 1012+00.00	111 82	40
1011+00.00 1012+00.00	1012+00.00	64	29
1013+00.00	1014+00.00	62	17
1014+00.00	1015+00.00	58	18
1015+00.00 1016+00.00	1016+00.00 1017+00.00	65 123	18
1017+00.00	1018+00.00	112	50
1018+00.00	1019+00.00	48	194
1019+00.00 1020+00.00	1020+00.00 1021+00.00	61 90	278
1021+00.00	1022+00.00	55	99
1022+00.00	1023+00.00	52	90
1023+00.00 1024+00.00	1024+00.00 1025+00.00	76 69	1 40 62
1025+00.00	1026+00.00	87	26
1026+00.00	1027+00.00	105	17
1027+00.00 1028+00.00	1028+00.00	94	12
1029+00.00	1029+00.00 1030+00.00	78	18
1030+00.00	1031+00.00	78	19
1031+00.00 1032+00.00	1032+00.00 1033+00.00	93 95	10
1033+00.00	1034+00.00	84	6
1034+00.00	1035+00.00	80	14
1035+00.00	1036+00.00	91	13
1036+00.00 1037+00.00	1037+00.00 1038+00.00	106 150	4
1038+00.00	1039+00.00	177	0
1039+00.00	1040+00.00	173	0
1040+00.00 1041+00.00	1041+00.00	176 161	0
1042+00.00	1043+00.00	164	1
1043+00.00	1044+00.00	159	2
1044+00.00 1045+00.00	1045+00.00 1046+00.00	125 105	3
1046+00.00	1047+00.00	99	1
1047+00.00	1048+00.00	81	1
1048+00.00 1049+00.00	1049+00.00 1050+00.00	74 90	2
1050+00.00	1051+00.00	101	22
1051+00.00	1052+00.00	105	15
1052+00.00	1053+00.00 1054+00.00	95 97	8
1053+00.00 1054+00.00	1054+00.00	110	12
1055+00.00	1056+00.00	115	11
1056+00.00	1057+00.00	118	6
1057+00.00 1058+00.00	1058+00.00 1059+00.00	93 68	5
1059+00.00	1060+00.00	62	18
1060+00.00	1061+00.00	61	19
1061+00.00 1062+00.00	1062+00.00 1063+00.00	56 46	5
1063+00.00	1064+00.00	51	3
1064+00.00	1065+00.00	51	1
1065+00.00 1066+00.00	1066+00.00 1067+00.00	<u>32</u> 16	2
1067+00.00	1068+00.00	11	1
0010	9-19-087	16981	3364

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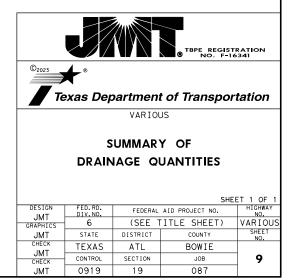
,						SUMMA	ARY OF SHA	RED USE	PATH ITEN	٨S					
		100	100	105	247	251	400	401	420	496	496	531	644	3099	5084
		6002	6004	6011	6041	6478	6006	6001	6010	6006	6007	6001	6060	6001	6002
SHT NO.	STATION LIMITS	PREPARING ROW	PREPARING ROW (TREE) (12" TO 24" DIA)	REMOVING STAB BASE AND ASPH PAV (2"-6")	FL BS (CMP IN PLC) (TYA GR1-2) (FNAL POS)	REWORK BS MTL (TY B) (9-18") (ORD COMP)	CUT & RESTORING PAV	FLOWABLE BACKFILL	CL A CONC (PLUG)	REMOV STR (HEADWALL)	REMOV STR (PIPE)	CONC SIDEWALKS (4")	IN SM RD SN SUP&AM TYTWT (1)WS (P)	DECOMPOSED GRAVITE PAVING MATERIALS	INSTALL REMOVABLE BOLLARD
		STA	EA	SY	CY	SY	SY	CY	EA	EA	LF	SY	EA	SY	EA
CSJ 0919	9-19-087														
	2902+25 TO 2903+40				16									130	
P&P SHT 1 OF 30	903+00 TO 909+00				67			3	2		26	44	2	551	4
P&P SHT 2 OF 30	909+00 TO 914+50				75			-	-				-	611	
P&P SHT 3 OF 30	914+50 TO 920+00				67					1		31	2	552	4
P&P SHT 4 OF 30	920+00 TO 925+50				75					1	1	1	-	611	† '
P&P SHT 5 OF 30	925+50 TO 931+00				75									611	
P&P SHT 6 OF 30	931+00 TO 936+50				65		59					56	2	529	4
P&P SHT 7 OF 30	936+50 TO 942+00	1.0			75		55				33		<u>د</u>	611	
P&P SHT 8 OF 30	942+00 TO 947+50	5.5			75									611	
P&P SHT 9 OF 30	947+50 TO 953+00	5.5			75									611	
	953+00 TO 958+50	5.5			64		70					56	2	527	4
	958+50 TO 964+00	5.5			75		10					50	<u>د</u>	611	
	964+00 TO 969+00	1.4			68									556	
	969+00 TO 975+00	0.6			72		106					39		587	4
	975+00 TO 980+50	5.5			75		100					- 39		611	4
	980+50 TO 986+00	5.5			69		36					32		561	4
	986+00 TO 991+50	5.5			75		50					52		611	4
	991+50 TO 997+00	1.7			75									611	
	997+00 TO 1002+50	3.0			75									611	
					75										
	1002+50 TO 1008+00	5.5												611	
	1008+00 TO 1013+50	5.5		100	75						67			611	
	1013+50 TO 1019+00	5.5		106	75					2	67	70		611	
	1019+00 TO 1024+50	1.1			67		54			2	85	38	2	546	4
	1024+50 TO 1030+00				75		-							611	
	1030+00 TO 1035+50		2		75		-							611	
	1035+50 TO 1041+00				75									611	
	1041+00 TO 1046+50		1		75									611	
	1046+50 TO 1052+00				75									611	
	1052+00 TO 1057+50				75									611	
	1057+50 TO 1063+00				75									611	
P&P SHT 30 OF 30	1063+00 TO END				64									521	
DRIVEWAY GRA	ADING DETAIL		1			101					27				
CSJ 0919-19	-087 TOTALS	63.8	4	106	2194	101	325	3	2	4	238	296	10	17891	28
	TOTALS	63.8	4	106	2194	101	325	3	2	4	238	296	10	17891	28

	SUMMARY OF	
		6185
		6002
SHEET NO.	STATION LIMITS	TMA (STATIONARY)
		DAY
SHARED U	SE PATH TCP	
CSJ 0919-1	185	
PROJEC	185	



					SUMN	MARY OF D	RAINAGE I ⁻	ΓEMS			
	402	464	464	464	466	467	467	467	467	467	467
	6001	6003	6005	6008	6097	6358	6363	6389	6390	6395	6448
	TRENCH EXCAVATION PROTECTION	RC PIPE (CL III) (18 IN)	RC PIPE (CL III) (24 IN)	RC PIPE (CL III) (36 IN)	HEADWALL (CH - PW - O) (DIA= 24 IN)	SET (TY II) (18 IN) (RCP) (4: 1) (C)	SET (TY II) (18 IN) (RCP) (6: 1) (P)	SET (TY II) (24 IN) (RCP) (3: 1) (P)	SET (TY II) (24 IN) (RCP) (4: 1) (C)	SET (TY II) (24 IN) (RCP) (6: 1) (P)	SET (TY II) (36 IN) (RCP) (3: 1) (C)
	LF	LF	LF	LF	EA	EA	EA	EA	EA	EA	EA
CROSS CULVERTS											
CULVERT SUMMARY TABLE											
CSJ 0919-19-087		108	66	48	1	10			4		2
PARALLEL CULVERTS											
CULVERT SUMMARY TABLE											
CSJ 0919-19-087	86	328	402				8	2		10	
CSJ 0919-19-087 TOTALS	86	436	468	48	1	10	8	2	4	10	2
PROJECT TOTALS	86	436	468	48	1	10	8	2	4	10	2

467
6450
SET (TY II) (36 IN) (RCP) (4: 1) (C)
EA
2
2
2



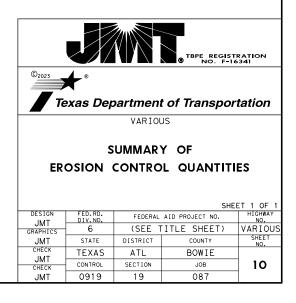
							SL	JMMARY OF	EROSION	CONTROL	ITEMS							
		160	162	164	164	164	166	168	169	506	506	506	506	506	506	506	506	506
		6001	6002	6054	6055	6056		6001	6004	6002	6003	6011	6020	6024	6038	6039	6041	6043
SHEET NO.	STATION LIMITS	FURNISHING AND PLACING TOPSOIL (4")	BLOCK SODDING	BOND FBR MTRX SEED (PERM) (RURAL) (SAND)	BONDED FBR MTRX SEED (TEMP) (WARM)	BONDED FBR MTRX SEED (TEMP) (COOL)	FERTILIZER	VEGETATIVE WATERING	SOIL RETENTION BLANKETS (CL 1) (TY D)	ROCK FILTER DAMS (INSTALL) (TY 2)	ROCK FILTER DAMS (INSTALL) (TY 3)	ROCK FILTER DAMS (REMOVE)	CONSTRUCTION EXITS (INSTALL) (TY 1)	CONSTRUCTION EXITS (REMOVE)	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDMT CONT FENCE (REMOVE)	BIODEG EROSN CONT LOGS (INSTL) (12")	BIODEG ERO CONT LOGS (REMOVE)
		STA	SY	SY	SY	SY	TON	MG	SY	LF	LF	LF	SY	SY	LF	LF	LF	LF
	9-19-087																	
ECP SHT 1 OF 16	902+25 TO 909+00	6.8	6	1852	926	926	0.06	29.6		24		24	111	111	706	706	68	68
ECP SHT 2 OF 16	909+00 TO 920+00	11.0		3052	1526	1526	0.09	48.8							399	399	51	51
ECP SHT 3 OF 16	920+00 TO 931+00	11.0		3246	1623	1623	0.10	51.9							1022	1022		
ECP SHT 4 OF 16	931+00 TO 942+00	11.0	22	3268	1634	1634	0.10	52.3		48		48	222	222	401	401	68	68
ECP SHT 5 OF 16	942+00 TO 953+00	11.0		5430	2715	2715	0.16	86.9										
ECP SHT 6 OF 16	953+00 TO 964+00	11.0	18	5138	2569	2569	0.15	82.2	195	30		30	222	222			34	34
ECP SHT 7 OF 16	964+00 TO 975+00	11.0	18	4842	2421	2421	0.15	77.5		30		30						
ECP SHT 8 OF 16	975+00 TO 986+00	11.0	18	5552	2776	2776	0.17	88.8	420	30		30			167	167	17	17
ECP SHT 9 OF 16	986+00 TO 997+00	11.0		6212	3106	3106	0.19	99.4	236						320	320	81	81
CP SHT 10 OF 16	997+00 TO 1008+00	11.0		5290	2645	2645	0.16	84.6			18	18			583	583	51	51
ECP SHT 11 OF 16	1008+00 TO 1019+00	11.0	26	5716	2858	2858	0.17	91.5	295	36		36			679	679	102	102
	1019+00 TO 1030+00	11.0	16	5184	2592	2592	0.16	82.9	299	30		30	222	222	128	128	64	64
CP SHT 13 OF 16		11.0		4686	2343	2343	0.14	75.0									68	68
	1041+00 TO 1052+00	11.0	6	3830	1915	1915	0.11	61.3		18		18			451	451	68	68
CP SHT 15 OF 16	1052+00 TO 1063+00	11.0	7	3938	1969	1969	0.12	63.0	169	12		12	100	100	483	483	68	68
CP SHT 16 OF 16	1063+00 TO END	4.7		1102	551	551	0.03	17.6							330	330		
	9-087 TOTALS	165.5	137	68338	34169	34169	2.06	1093.3	1614	258	18	276	877	877	5669	5669	740	740
PROJEC	T TOTALS	165.5	137	68338	34169	34169	2,06	1093.3	1614	258	18	276	877	877	5669	5669	740	740

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NOTES:

 13-13-13 FERTILIZER, 300 LB/5,000 SQ YD OF SEED. FOR CONTRACTOR'S INFORMATION ONLY.



PHASE O GENERAL NOTES

CONTRACTOR SHALL PROVIDE A SCHEDULE OF CONSTRUCTION ACTIVITIES INCLUDING STAGING OF LOCATIONS AND ANTICIPATED CONSTRUCTION DURATIONS.

THE CONTRACTOR SHALL PLACE EQUIPMENT AND STOCKPILES OUTSIDE OF US 82, FM 3378 AND COUNTY ROAD CLEAR ZONE. PLACE WORK ZONE SIGNING AND CHANNELIZING DEVICES PER TXDOT STANDARD TCP(2-1)-18 FOR INCIDENTAL WORK WITHIN THE CLEAR ZONE.

CONSTRUCTION/TEMPORARY DRAINAGE: THE CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE IS MAINTAINED THROUGH EACH STEP.

PLACE WORK ZONE SIGNING AND SW3P MEASURES PRIOR TO BEGINNING WORK.

CLOSE NORTHEAST TEXAS TRAIL. PLACE BARRICADE AND "SIDEWALK CLOSED" SIGN (R9-9) 100-FT UPSTREAM AND DOWNSTREAM OF THE PROPOSED WORK. ADVANCE NOTIFICATION OF TRAIL CLOSURES SHALL BE PROVIDED TO THE MAINTAINING AGENCY.

DURING NON-WORK PERIODS EDGE OF PAVEMENT CHANNELIZING DEVICES AND UTILITY WORK AHEAD SIGNS (CW 21-7) SHALL BE IN PLACE.

USE TXDOT STANDARD SHEET TCP (2-1)-18-TCP (2-1A) FOR WORK INSIDE TXDOT ROW.

THE CONTRACTOR SHALL WORK FROM WEST (LINE L) TO EAST (LINE A).

PHASE 1 GENERAL NOTES

CONSTRUCTION PHASING SHALL BE SEGMENTED BEGINNING FROM WEST TO FAST. FACH INDIVIDUAL STEP IS ESTIMATED TO BE COMPLETE WITHIN 90 CALENDAR DAYS AND SHALL TAKE NO LONGER THAN 120 CALENDAR DAYS IN DURATION. CONTACT THE ENGINEER AND AREA OFFICER IF THE CONSTRUCTION TIME FOR ANY SEGMENT IS EXPECTED TO EXCEED 120 CALENDAR DAYS.

ADVANCE NOTIFICATION OF TRAIL CLOSURES SHALL BE PROVIDED TO THE MAINTAINING AGENCY.

THE CONTRACTOR SHALL PLACE EQUIPMENT AND STOCKPILES OUTSIDE OF US 82 CLEAR ZONE (30 FEET FROM EDGE OF TRAVELWAY). PLACE WORK ZONE SIGNING AND CHANNELIZING DEVICES PER TXDOT STANDARD TCP(2-1)-18 FOR INCIDENTAL WORK WITHIN US 82 CLEAR ZONE.

CONSTRUCTION/TEMPORARY DRAINAGE: THE CONTRACTOR SHALL ENSURE POSITIVE DRAINAGE IS MAINTAINED THROUGH EACH STEP.

PHASE 1 STEP 1

OBJECTIVE: CONSTRUCT SHARED USE PATH FROM STA 902+26.44 TO STA 956+36.

1. CLOSE NORTHEAST TEXAS TRAIL THROUGH THESE LIMITS. PLACE BARRICADE AND "SIDEWALK CLOSED" SIGN (R9-9) AS DIRECTED ON TCP LAYOUT SHEETS. ESTABLISH PROJECT LIMITS BY INSTALLING TRAFFIC CONTROL DEVICES AS PROPOSED ON TCP LAYOUT SHEETS.

2. PLACE EROSION CONTROL DEVICES AS NEEDED IN ACCORDANCE WITH SWP3 LAYOUTS.

- 3. PLACE WORK ZONE SIGNING AND CHANNELIZING DEVICES AS PER TXDOT BC AND TCP STANDARDS, THE LATEST EDITION OF TEXAS MUTCD AND AS DIRECTED ON TCP LAYOUT SHEETS.
- 4. MAINTAIN CONTINUOUS ACCESS TO EACH ADJACENT PROPERTY AND SIDE STREETS DURING CONSTRUCTION.
- 5. INSTALL CONSTRUCTION EXITS AT CR 4105 (NEAR STA 904+00, EAST SIDE ONLY), CR 4104, AND CR 4102 (WEST SIDE ONLY).
- 6. CONSTRUCT SHARED USE PATH AND ASSOCIATED GRADING. REMOVE AND REPLACE SMALL DIAMETER CROSS CULVERTS. INSTALL DRIVEWAY PIPES.
 - G. CONDUCT ONE-LANE TWO-WAY TRAFFIC CONTROL ALONG CR 4130 IN AREA OF CONSTRUCTION ZONE DURING WORK OPERATIONS ACCORDING TO STANDARD TCP(2-2B)-18 AND TCP(ATL-21)-14. CR 4130 WILL BE OPEN TO TWO-WAY TRAFFIC DURING NON-WORKING HOURS.
- 7. REMOVE TRAFFIC CONTROL AND EROSION CONTROL DEVICES. MAKE SHARED USE PATH AVAILABLE TO PUBLIC THROUGH THESE LIMITS.

PHASE 1 STEP 2

OBJECTIVE: CONSTRUCT SHARED USE PATH FROM STA 956+36 TO STA 1023+00.

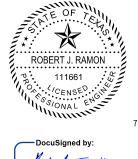
- 1. CLOSE NORTHEAST TEXAS TRAIL THROUGH THESE LIMITS. PLACE BARRICADE AND "SIDEWALK CLOSED" SIGN (R9-9) AS DIRECTED ON TCP LAYOUT SHEETS. ESTABLISH PROJECT LIMITS BY INSTALLING TRAFFIC CONTROL DEVICES AS PROPOSED ON TCP LAYOUT SHEETS.
- 2. PLACE EROSION CONTROL DEVICES AS NEEDED IN ACCORDANCE WITH SWP3 LAYOUTS.
 - a.NOTE PROPOSED PROTECTION AT RED BAYOU WETLAND (NEAR STA 998+00).
- 3. PLACE WORK ZONE SIGNING AND CHANNELIZING DEVICES AS PER TXDOT BC AND TCP STANDARDS, THE LATEST EDITION OF TEXAS MUTCD AND AS DIRECTED ON TCP LAYOUT SHEETS.
- 4. MAINTAIN CONTINUOUS ACCESS TO EACH ADJACENT PROPERTY AND SIDE STREETS DURING CONSTRUCTION.
- 5. INSTALL CONSTRUCTION EXITS AT CR 4102 (EAST SIDE ONLY) AND CR 4007 (WEST SIDE ONLY).
- 6. CONSTRUCT SHARED USE PATH AND ASSOCIATED GRADING. RE-GRADE DRIVEWAY AT STA 969+50. REMOVE AND REPLACE SMALL DIAMETER CROSS CULVERTS. INSTALL DRIVEWAY PIPES.
- 7. REMOVE TRAFFIC CONTROL AND EROSION CONTROL DEVICES. MAKE SHARED USE PATH AVAILABLE TO PUBLIC THROUGH THESE LIMITS.

PHASE 1 STEP 3

OBJECTIVE: CONSTRUCT SHARED USE PATH FROM STA 1023+00 TO END PROJECT.

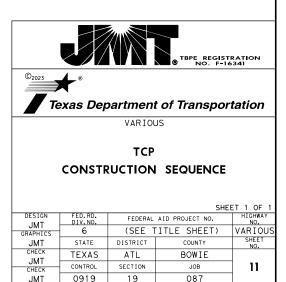
- 1. CLOSE NORTHEAST TEXAS TRAIL THROUGH THESE LIMITS. PLACE BARRICADE AND "SIDEWALK CLOSED" SIGN (R9-9) AS DIRECTED ON TCP LAYOUT SHEETS. ESTABLISH PROJECT LIMITS BY INSTALLING TRAFFIC CONTROL DEVICES AS PROPOSED ON TCP LAYOUT SHEETS.
- 2.PLACE EROSION CONTROL DEVICES AS NEEDED IN ACCORDANCE WITH SWP3 LAYOUTS.
- 3. PLACE WORK ZONE SIGNING AND CHANNELIZING DEVICES AS PER TXDOT BC AND TCP STANDARDS, THE LATEST EDITION OF TEXAS MUTCD AND AS DIRECTED ON TCP LAYOUT SHEETS.
- 4. MAINTAIN CONTINUOUS ACCESS TO EACH ADJACENT PROPERTY AND SIDE STREETS DURING CONSTRUCTION.
- 5. INSTALL CONSTRUCTION EXITS AT CR 4007 (EAST SIDE ONLY) AND US 82 AT STA 1059+00.
 - G. CONSTRUCTION EXIT ALONG US 82 REQUIRES PLACEMENT OF TRUCKS ENTERING ROADWAY SIGN (CW27-1T). USE TXDOT STANDARD SHEET TCP (1-1)-18 FOR INSTALLATION AND REMOVAL OF CONSTRUCTION EXIT.
- 6.CONSTRUCT SHARED USE PATH AND ASSOCIATED GRADING. REMOVE AND REPLACE SMALL DIAMETER CROSS CULVERTS. INSTALL DRIVEWAY PIPES.
 - a. CONDUCT ONE-LANE TWO-WAY TRAFFIC CONTROL ALONG CR 4008 IN AREA OF CONSTRUCTION ZONE DURING WORK OPERATIONS ACCORDING TO STANDARD TCP(2-2B)-18 AND TCP(ATL-21)-14. CR 4008 WILL BE OPEN TO TWO WAY TRAFFIC DURING NON-WORKING HOURS.

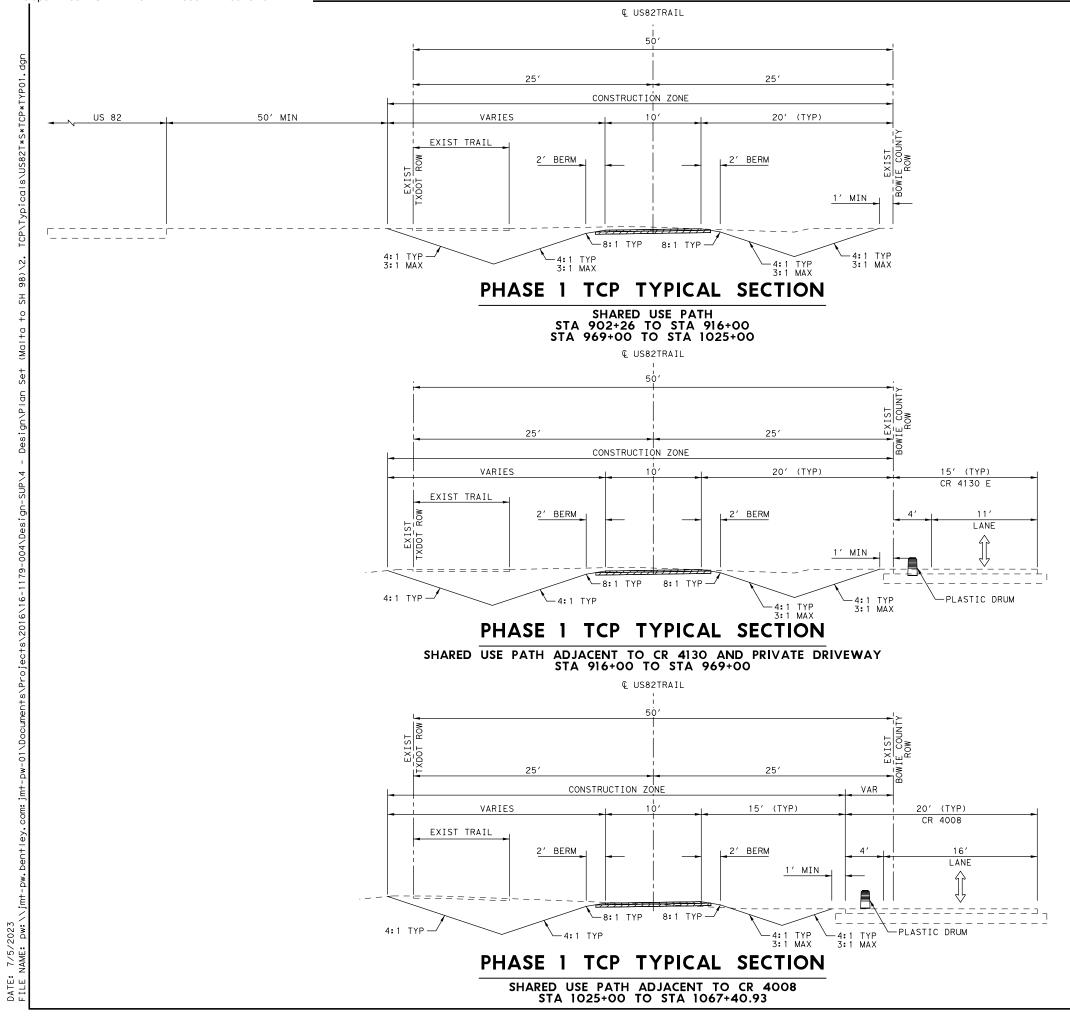
REMOVE TRAFFIC CONTROL AND EROSION CONTROL DEVICES. MAKE SHARED USE PATH AVAILABLE TO PUBLIC THROUGH THESE LIMITS.

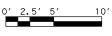


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SCALE IN FEET



CONSTRUCTION THIS STEP

NOTES:

 CONDUCT TWO-WAY ONE-LANE WORK OPERATIONS ACCORDING TO STD TCP (2-2B)-18 THROUGH LIMITS SHOWN ON LAYOUTS. ROADS OPEN TO TWO-WAY TRAFFIC DURING NON-WORKING HOURS.



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Texas Department of Transportation

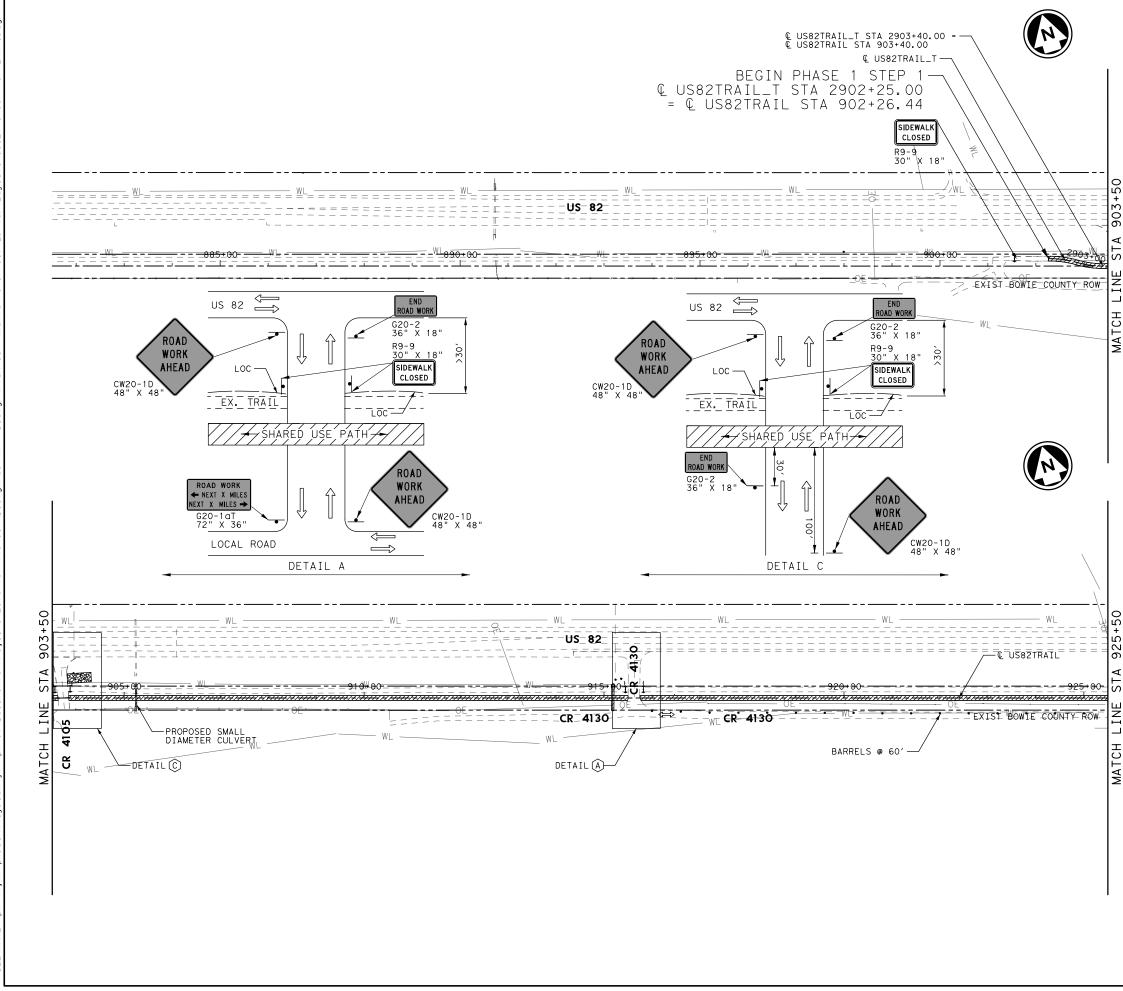
VARIOUS

TCP TYPICAL SECTIONS

SCALE: 1"			SHEE	
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	12
JMT	0919	19	087	

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DATE:



SCALE IN FEET

LEGEND \square CONSTRUCTION THIS STEP 5885 CONSTRUCTION EXITS THIS STEP Ì EXIST DIRECTIONAL ARROW BARRICADE CHANNELIZING DEVICE

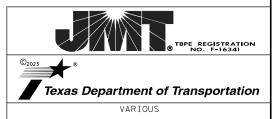
NOTES:

- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- INSTALL SMALL DIAMETER CULVERT, CUT AND RESTORE PAVEMENT. SEE TRAFFIC CONTROL PLAN MISCELLANEOUS DETAILS FOR MORE INFORMATION.
- 3. SEE SWP3 LAYOUT SHEETS FOR PROPOSED TEMPORARY EROSION CONTROL.



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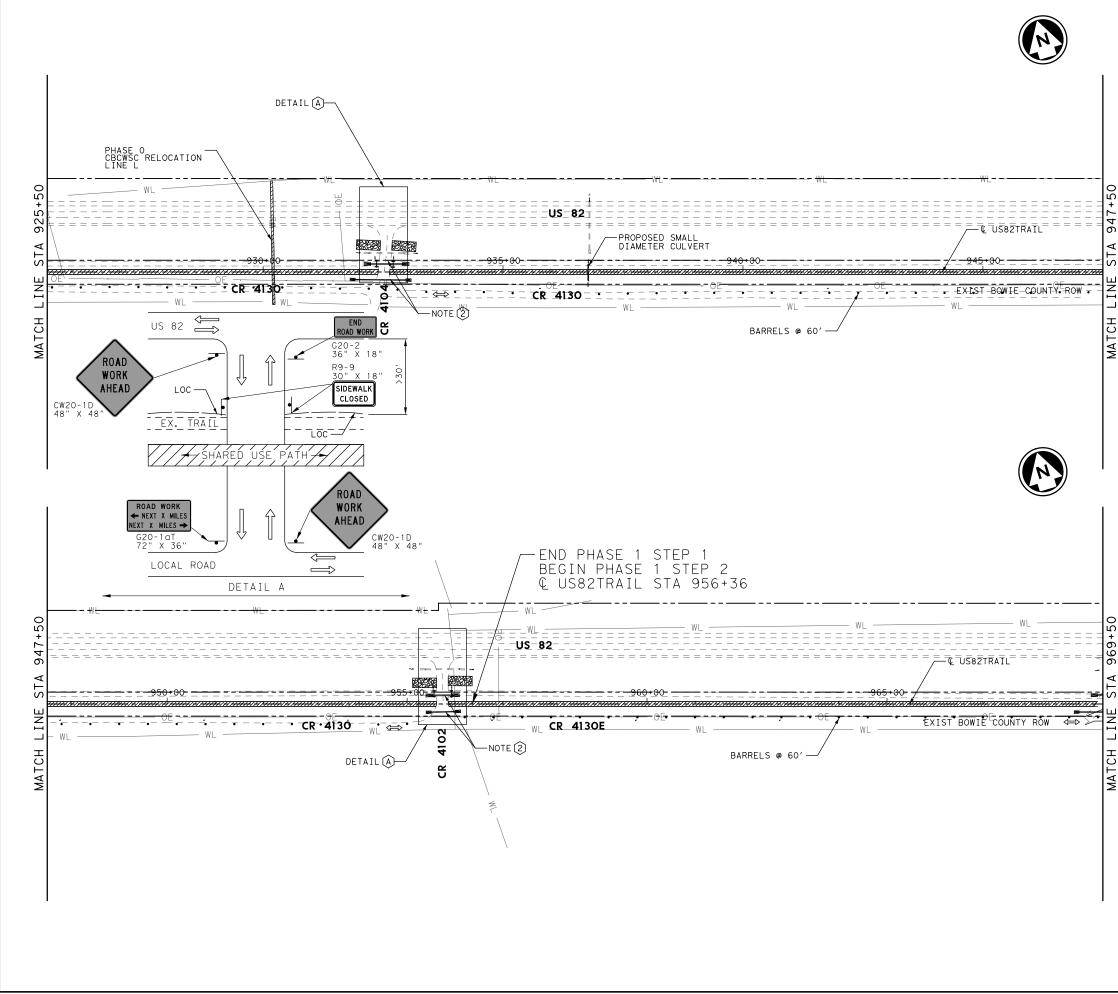


TRAFFIC CONTROL PLAN LAYOUTS BEGIN TO STA 925+50

SCALE: 1"			SHEE	T 1 OF 5
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
JMT CHECK	CONTROL	SECTION	JOB	13
JMT	0919	19	087	

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SCALE IN FEET

LEGEND CONSTRUCTION THIS STEP \square 5885 CONSTRUCTION EXITS THIS STEP ⇒ EXIST DIRECTIONAL ARROW BARRICADE CHANNELIZING DEVICE

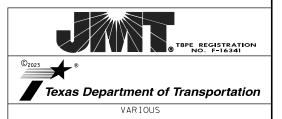
NOTES:

- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- INSTALL SMALL DIAMETER CULVERT, CUT AND RESTORE PAVEMENT. SEE TRAFFIC CONTROL PLAN MISCELLANEOUS DETAILS FOR MORE INFORMATION.
- 3. SEE SWP3 LAYOUT SHEETS FOR PROPOSED TEMPORARY EROSION CONTROL.



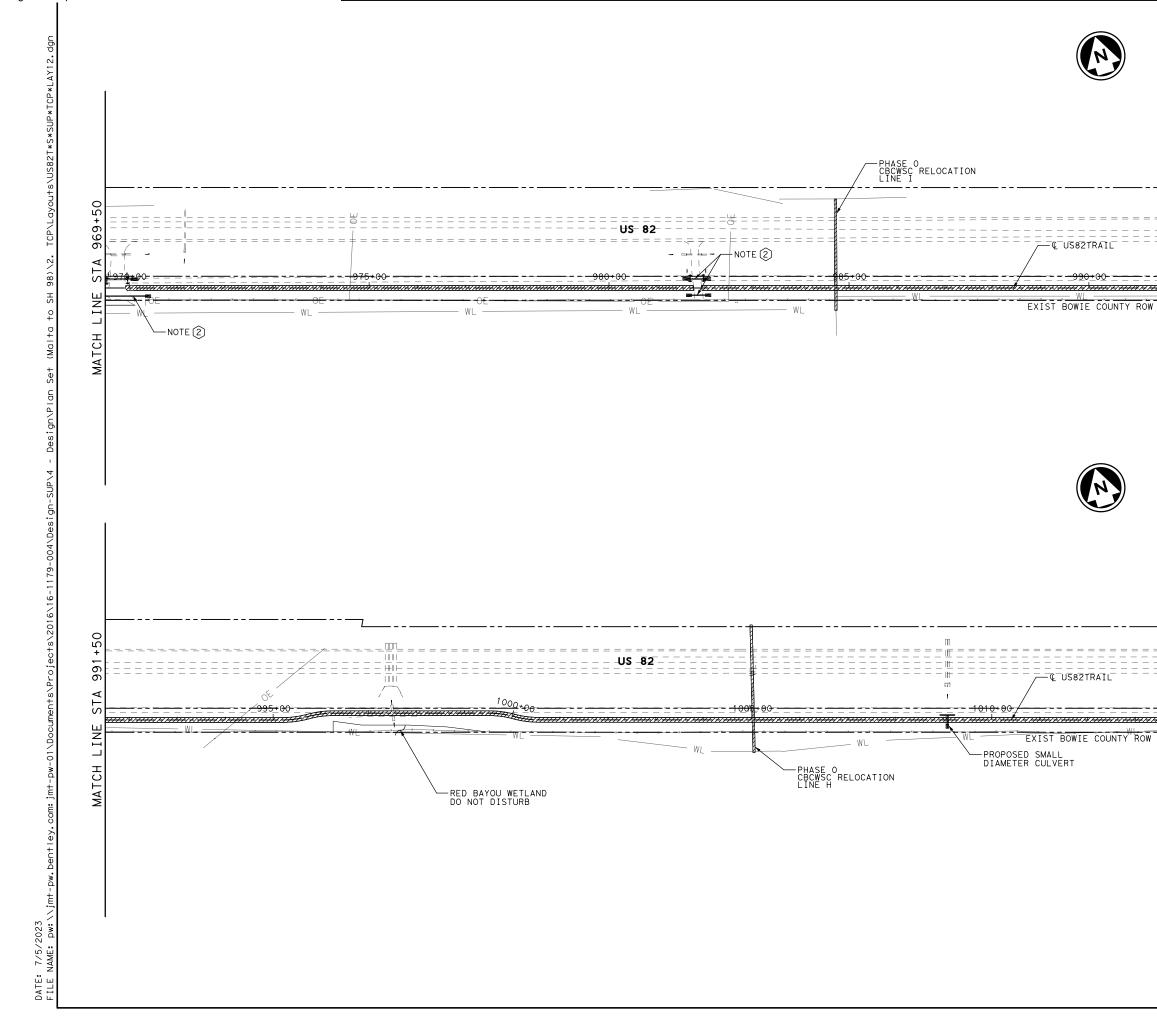
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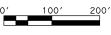
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TRAFFIC CONTROL PLAN LAYOUTS STA 925+50 TO STA 969+50

SCALE: 1"			SHEE	T 2 OF 5
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
JMT CHECK	CONTROL	SECTION	JOB	14
JMT	0919	19	087	





SCALE IN FEET

LEGEND CONSTRUCTION THIS STEP CONSTRUCTION EXITS THIS STEP CONSTRUCTION EXITS THIS STEP CONSTRUCTIONAL ARROW F→F BARRICADE CHANNELIZING DEVICE EXIST NE TEXAS TRAIL

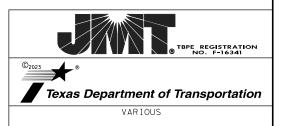
NOTES:

- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- INSTALL SMALL DIAMETER CULVERT, CUT AND RESTORE PAVEMENT. SEE TRAFFIC CONTROL PLAN MISCELLANEOUS DETAILS FOR MORE INFORMATION.
- 3. SEE SWP3 LAYOUT SHEETS FOR PROPOSED TEMPORARY EROSION CONTROL.



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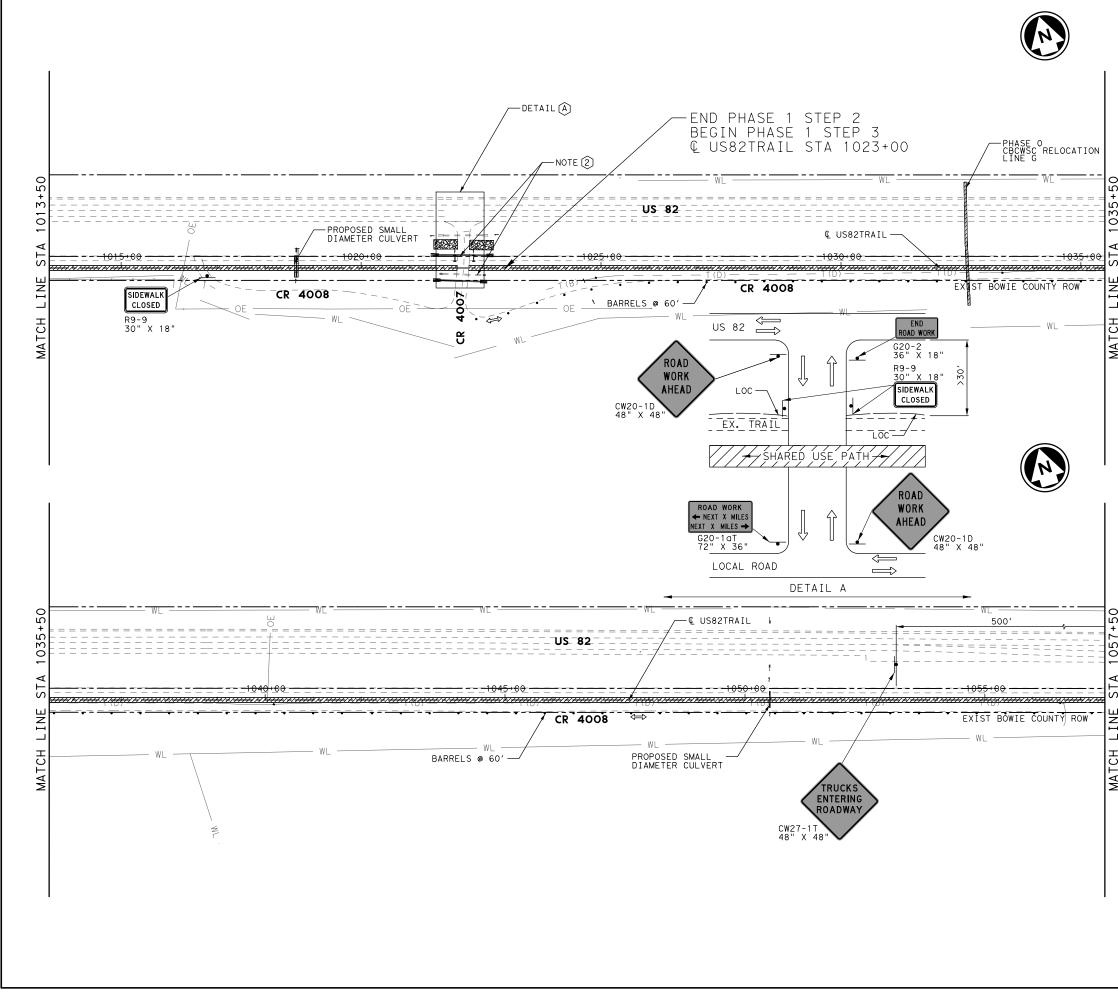


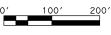
TRAFFIC CONTROL PLAN LAYOUTS STA 969+50 TO STA 1013+50

SCALE: 1"			SHEE	T 3 OF 5
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
JMT CHECK	CONTROL	SECTION	JOB	15
JMT	0919	19	087	

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DATE:





SCALE IN FEET

LEGEND \square CONSTRUCTION THIS STEP 5885 CONSTRUCTION EXITS THIS STEP Ì EXIST DIRECTIONAL ARROW BARRICADE CHANNELIZING DEVICE

NOTES:

- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- INSTALL SMALL DIAMETER CULVERT, CUT AND RESTORE PAVEMENT. SEE TRAFFIC CONTROL PLAN MISCELLANEOUS DETAILS FOR MORE INFORMATION.
- 3. SEE SWP3 LAYOUT SHEETS FOR PROPOSED TEMPORARY EROSION CONTROL.



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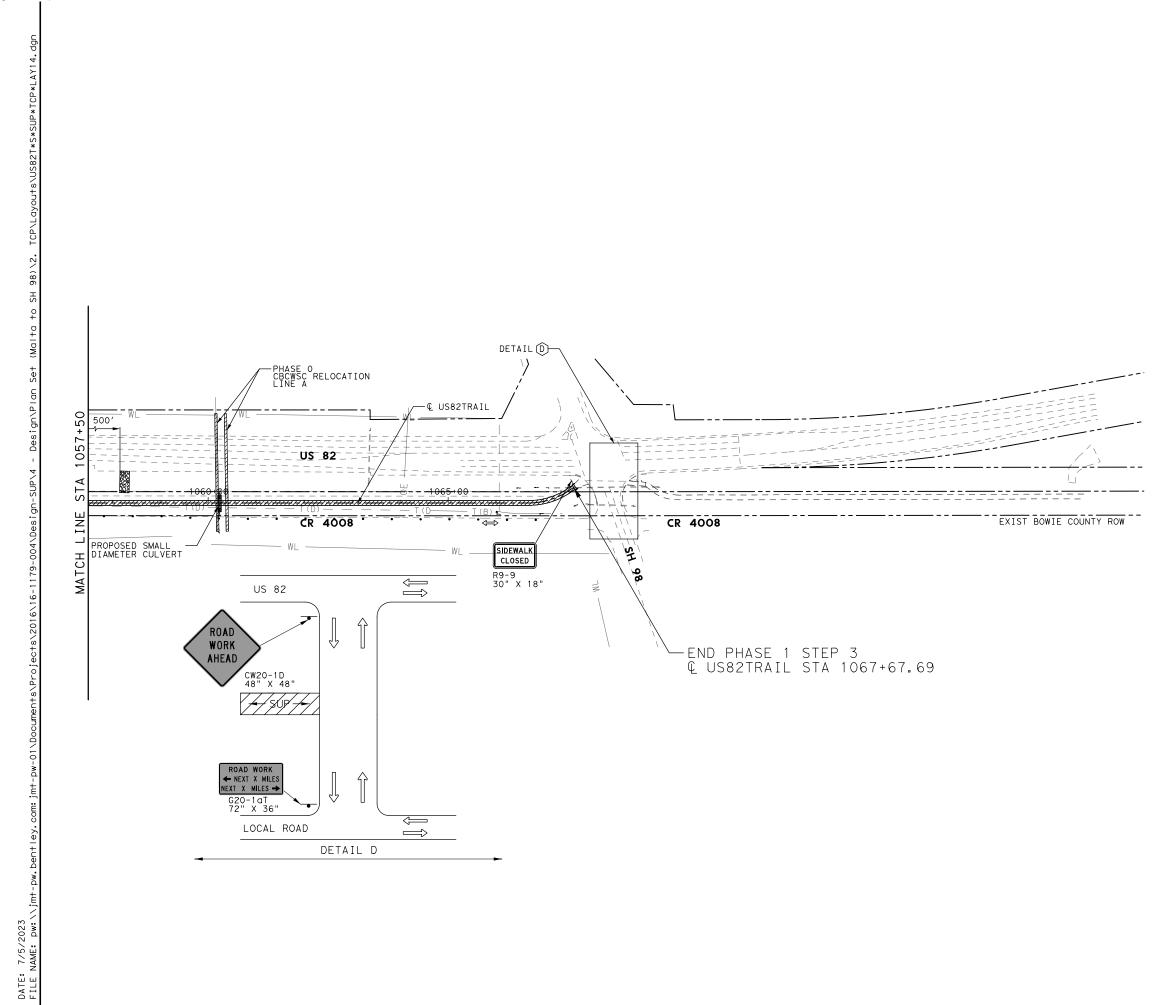




VARIOUS

TRAFFIC CONTROL PLAN LAYOUTS STA 1013+50 TO STA 1057+50

SCALE: 1"			SHEE	
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	16
JMT	0919	19	087	





LEGEND CONSTRUCTION THIS STEP CONSTRUCTION EXITS THIS STEP CONSTRUCTION EXITS THIS STEP CONSTRUCTIONAL ARROW H→H BARRICADE CHANNELIZING DEVICE EXIST NE TEXAS TRAIL

NOTES:

- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- INSTALL SMALL DIAMETER CULVERT, CUT AND RESTORE PAVEMENT. SEE TRAFFIC CONTROL PLAN MISCELLANEOUS DETAILS FOR MORE INFORMATION.
- 3. SEE SWP3 LAYOUT SHEETS FOR PROPOSED TEMPORARY EROSION CONTROL.

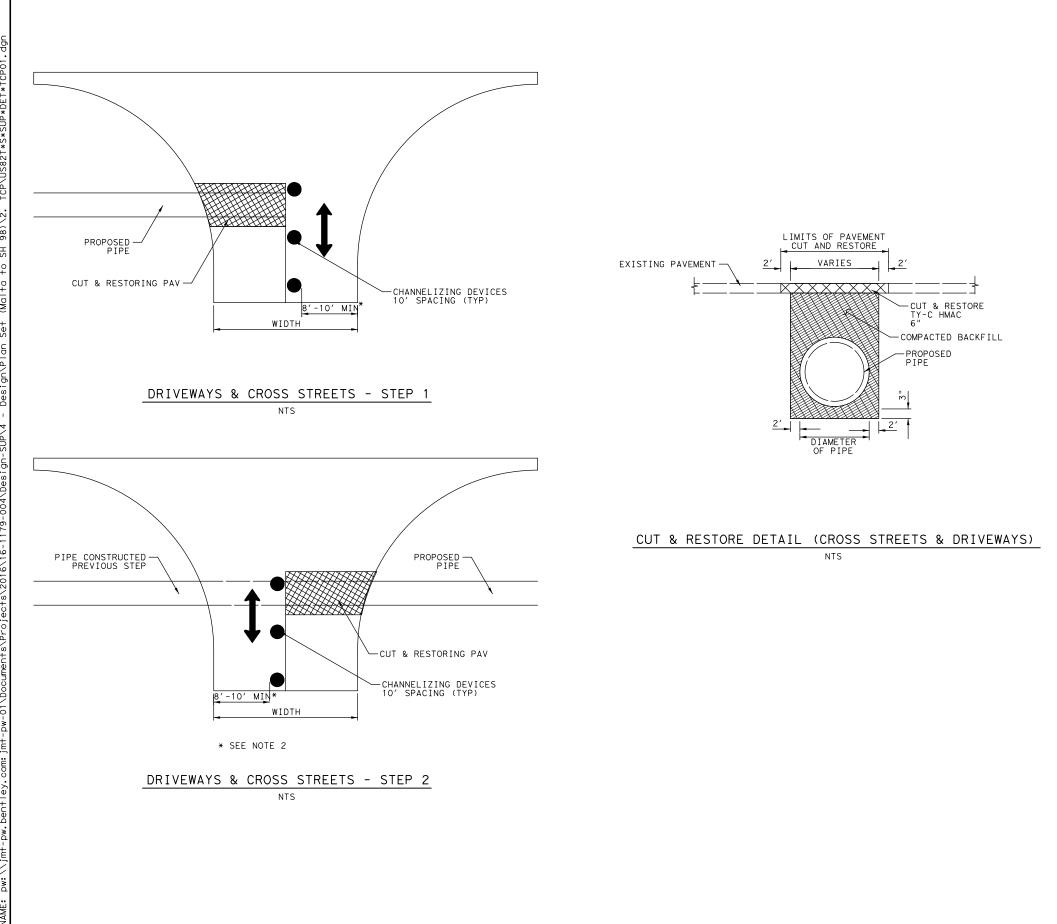


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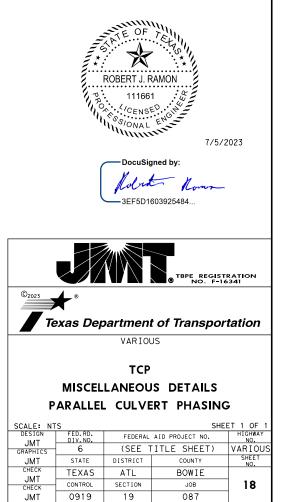


SCALE: 1"			SHEE	T 5 0F 5
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	17
JMT	0919	19	087	



NOTES:

- 1. TY-C HMAC TO BE USED FOR RESTORING PAVEMENT. MATERIAL SUBSIDARY TO CUT & RESTORING PAV ITEM 400.
- 2. MINIMUM TRAVEL WIDTH FOR CROSS STREETS IS 10'. MINIMUM TRAVEL WIDTH FOR DRIVEWAYS IS 8'.IF DRIVEWAYS ARE LESS THAN 16' WIDE, THE PROPERTY OWNER WILL NEED TO NOTIFIED THAT A DAYTIME CLOSURE OF THE DRIVEWAY WILL BE NECESSARY FOR PIPE INSTALLATION.
- CONDUCT TWO-WAY ONE-LANE WORK OPERATIONS ACCORDING TO STD TCP(2-2B)-18 AND TCP(ATL-21)-14. ROADS OPEN TO TWO-WAY TRAFFIC DURING NON-WORKING HOURS.



0919

19

087

BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

- 1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- The development and design of the Traffic Control Plan (TCP) is the 2. responsibility of the Engineer.
- The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop, sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES. CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:

- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel." or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

- 1. Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-aualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT
http://www.txdot.gov
COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD)
DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS)
MATERIAL PRODUCER LIST (MPL)
ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)"
STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD)
TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD)
TRAFFIC ENGINEERING STANDARD SHEETS

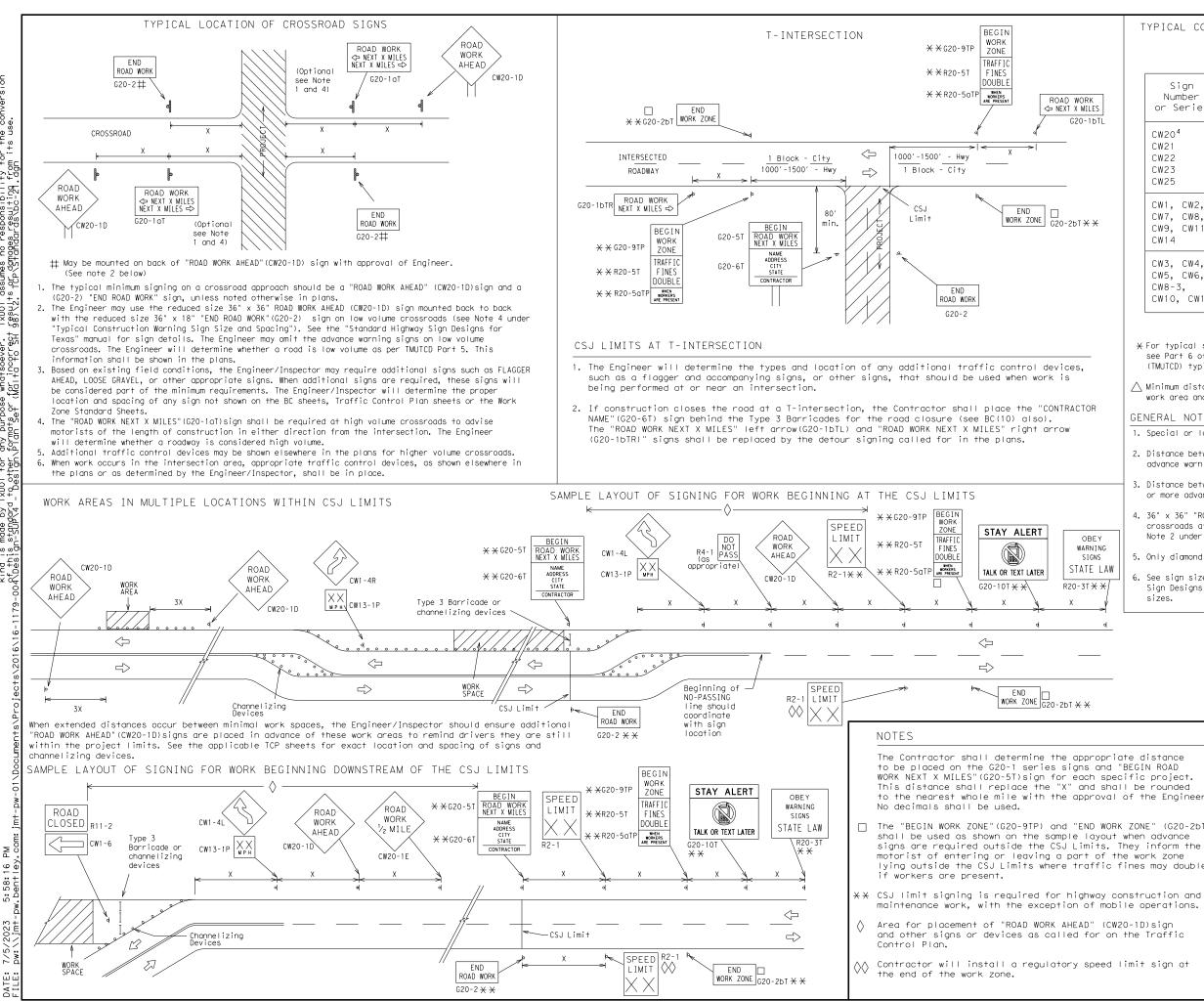
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		/				_
FILE: bc-21.dgn	dn: Tx	DOT	ск: TxDOT	DW:	TxDOT	ск: TxDOT
© TxDOT November 2002	CONT	SECT	JOB		H	IGHWAY
REVISIONS 4-03 7-13	0919	19	087		VA	RIOUS
9-07 8-14	DIST		COUNTY			SHEET NO.
5-10 5-21	ATL		BOWIE			19
95						



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TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING $^{\rm l,5,6}$

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s٢	A	C	L	NG	

Sign Number or Series	Conventional Road	Expressway/ Freeway
CW20 ⁴ CW21 CW22 CW23 CW25	48" x 48"	48" × 48"
CW1, CW2, CW7, CW8, CW9, CW11, CW14	36" × 36"	48" x 48"
CW3, CW4, CW5, CW6, CW8-3, CW10, CW12	48" × 48"	48" × 48"

Posted Speed	Sign∆ Spacing "X"
MPH	Feet (Apprx.)
30	120
35	160
40	240
45	320
50	400
55	500 ²
60	600 ²
65	700 2
70	800 ²
75	900 ²
80	1000 ²
*	* 3

X For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.

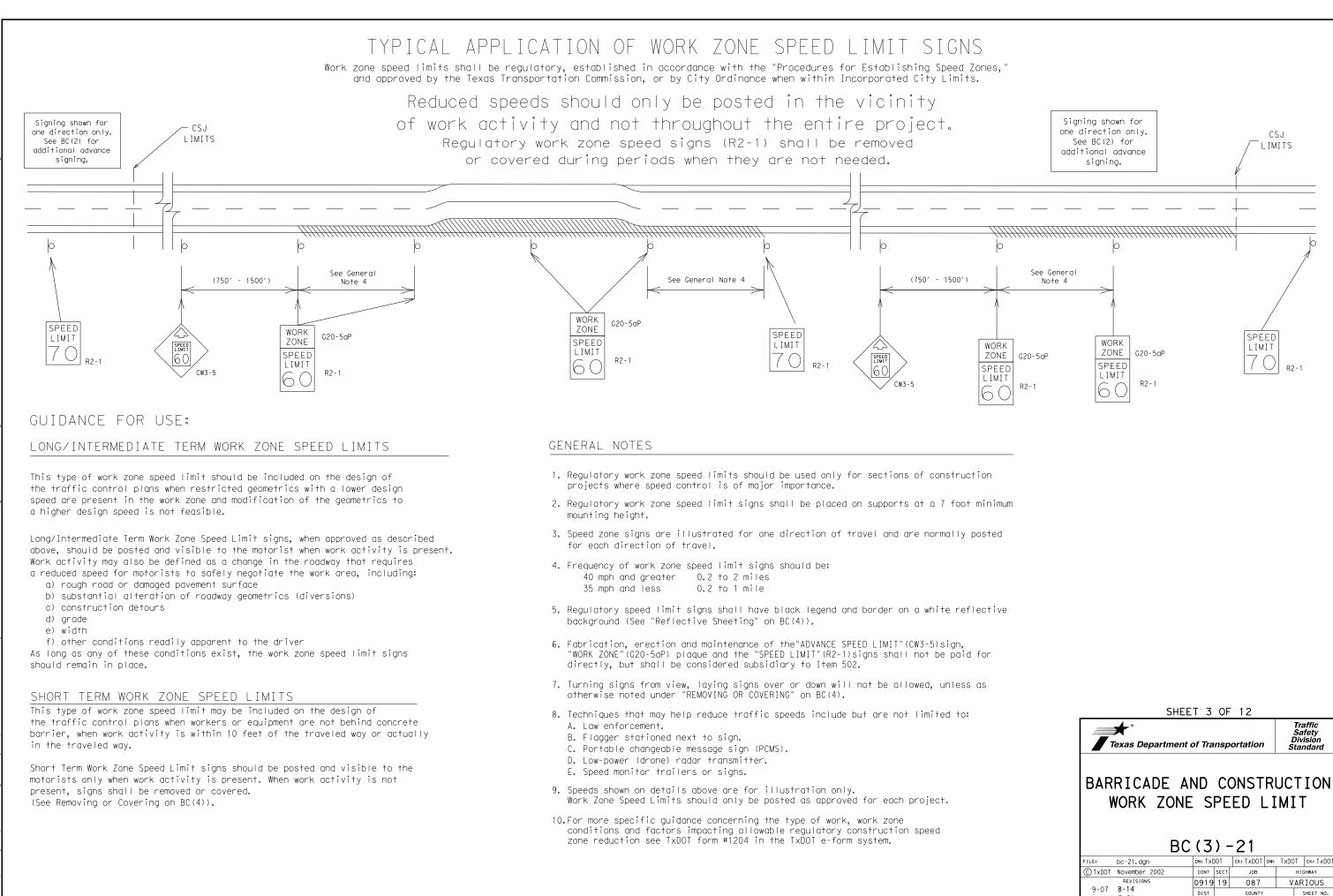
ightarrow Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- 3. Distance between signs should be increased as required to have $\ 1/2 \$ mile or more advance warning,
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D)signs may be used on low volume crossroads at the discretion of the Engineer as per TMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.
- 6. See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design sizes.

	LEGEND						
	⊢⊣ Type 3 Barricade						
	000 Channelizing Devices						
		•	Sign				
	X See Typical Construction Warning Sign Size and Spacing chart or the TMUTCD for sign spacing requirements.						
			SHEET 2 OF 12				
er.	Texas Department of Transportation						
e e							
	BC (2) - 21						

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© TxDOT	November 2002	CONT	SECT	SECT JOB HIGHWAY			
	REVISIONS	0919	19	087	087 VARIOUS		RIOUS
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	ATL		BOWIE			20
96							



DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TROT for any purpose whatsoever. TXDOT assumes no responsibility for the conversion At Design=tSOP4qd - Design_f9rmptSepr (ABT+Bocprest 58FVL+s ACt 94PABABArdStUbc+24.fgm its use.

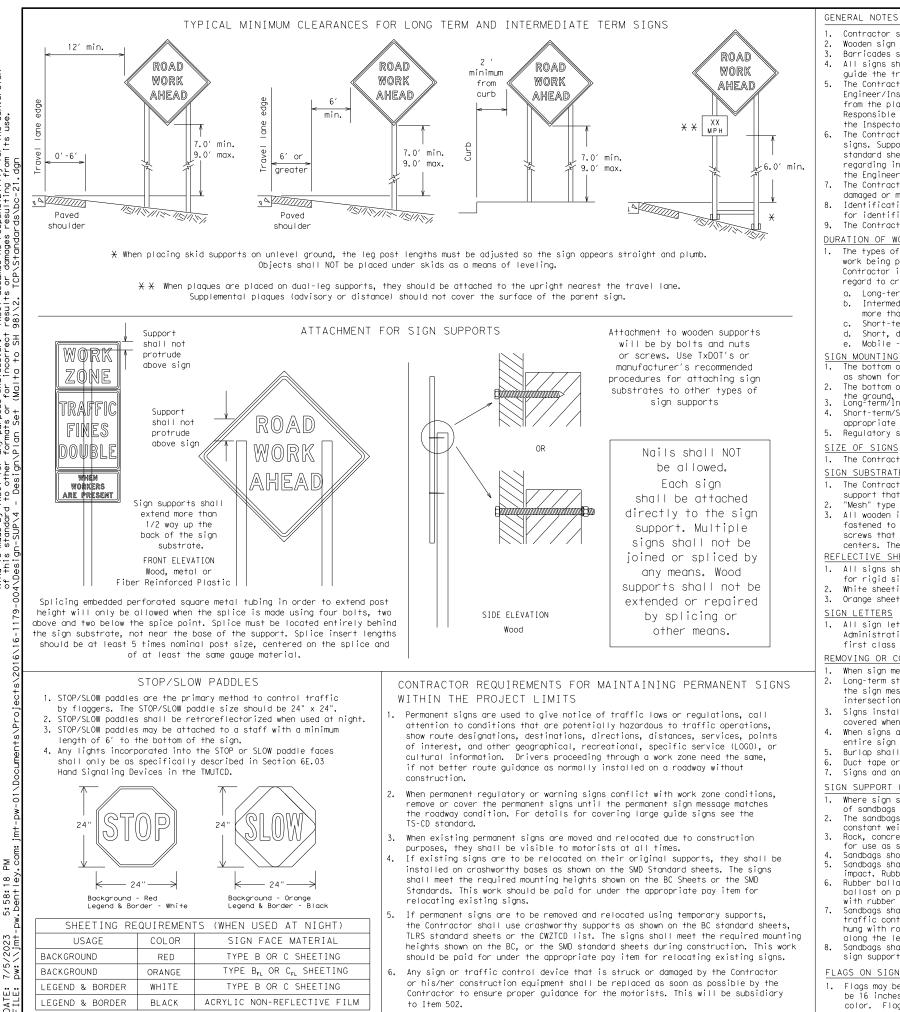
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7-13 5-21

ATL

BOWIE

21



GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports
- guide the traveling public safely through the work zone.

 - the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes. the Engineer can verify the correct procedures are being followed.
 - damaged or marred reflective sheeting as directed by the Engineer/Inspector. Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used
 - for identification shall be 1 inch.

9. The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

- DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6) regard to crashworthiness and duration of work requirements.
 - a. Long-term stationary work that occupies a location more than 3 days. more than one hour.
- Short-term stationary daytime work that occupies a location for more than 1 hour in a single daylight period.
- Short, duration work that occupies a location up to 1 hour. e. Mobile - work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- as shown for supplemental plaques mounted below other signs.
- the ground. Long-term/Intermediate-term Signs may be used in Lieu of Short-term/Short Duration signing.
- 4. Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to
- appropriate Long-term/Intermediate sign height.

1. The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer. SIGN SUBSTRATES

- centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- 1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300
- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).

first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- 1. When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- intersections where the sign may be seen from approaching traffic. 3. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely
- covered when not required.
- Burlap shall NOT be used to cover signs. Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a
- constant weight. Rock, concrete, iron, steel or other solid objects shall not be permitted
- for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used.
- Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list. Sandbags shall only be placed along or laid over the base supports of the
- traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

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All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and

The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in

The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so

The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or

The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in

Intermediate-term stationary - work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting

The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except

The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above

Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports. "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave. 3. All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6"

White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL}, shall be used for rigid signs with orange backgrounds.

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of

2. Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any

4. When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the entire sign face and minitain their opaque properties under automobile headlights at night, without damaging the sign sheeting.

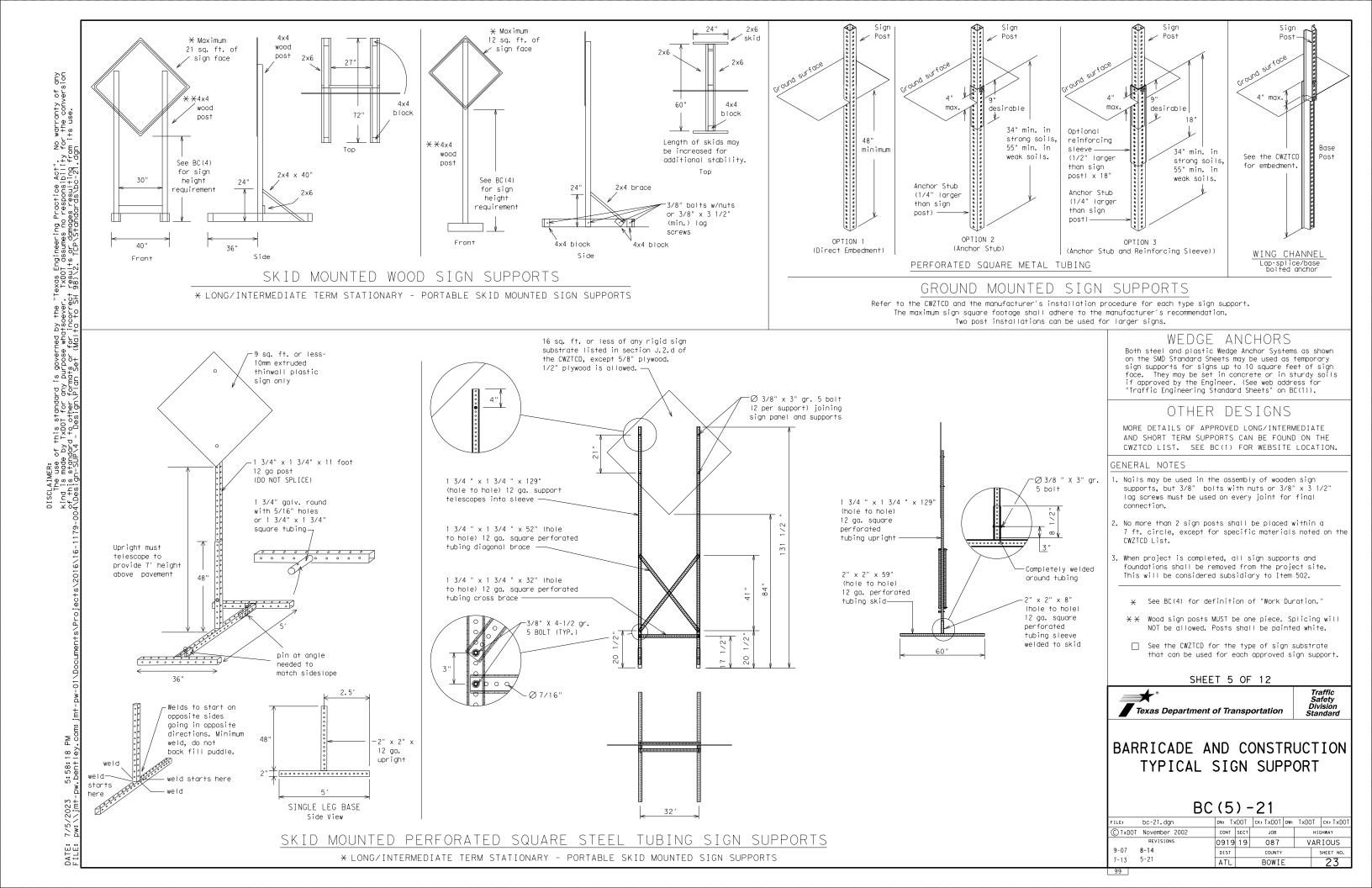
SHEET 4 OF 12

Texas Department of Transportation

Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

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WHEN NOT IN USE. REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

PORTABLE CHANGEABLE MESSAGE SIGNS

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message sians (PCMS).
- 2. Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO, "FOR." "AT." etc.
- 3. Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- 4. Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., "EXIT CLOSED." Do not use the term "RAMP."
- 5. Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be 6. a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to 7. start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are avail-8. able for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message 9. should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line. 11. Do not use the word "Danger" in message.
- 12. Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated. unless shown in the TMUTCD.
- 15 PCMS character beight should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE	ABBREVIATIO
Access Road	ACCS RD	Major	MAJ
Alternate	ALT	Miles	MI
Avenue	AVE	Miles Per Hour	MPH
Best Route	BEST RTE	Minor	MNR
Boulevard	BLVD	Monday	MON
Bridge	BRDG	Normal	NORM
Cannot	CANT	North	N
Center	CTR	Northbound	(route) N
Construction Ahead	CONST AHD	Parking	PKING
CROSSING	XING	Road	RD
Detour Route	DETOUR RTE	Right Lane	RT LN SAT
Do Not	DONT	Saturday	
East	E	Service Road	SERV RD
Eastbound	(route) E	Shoulder	SHLDR
Emergency	FMFR	Slippery	SLIP
	EMER VEH	South	S
Emergency Vehicle Entrance, Enter	EMER VEH	Southbound	(route) S
	EXP LN	Speed	SPD
Express Lane	EXPLN	Street	ST
Expressway	XXXX FT	Sunday	SUN
XXXX Feet	FOG AHD	Telephone	PHONE
Fog Ahead		Temporary	TEMP
Freeway	FRWY, FWY	Thursday	THURS
Freeway Blocked	FWY BLKD	To Downtown	TO DWNTN
Friday	FRI	Traffic	TRAF
Hazardous Driving		Travelers	TRVLRS
Hazardous Material		Tuesday	TUES
High-Occupancy	HOV	Time Minutes	TIME MIN
Vehicle	HWY	Upper Level	UPR LEVEL
Highway		Vehicles (s)	VEH, VEHS
Hour (s)	HR, HRS	Warning	WARN
Information	INFO	Wednesday	WED
It Is	ITS	Weight Limit	WT LIMIT
Junction	JCT	West	W
Left	LFT	Westbound	(route) W
Left Lane	LFT LN	Wet Pavement	WET PVMT
Lane Closed	LN CLOSED	Will Not	WONT
Lower Level	LWR LEVEL		1
Maintenance	MAINT		

designation # IH-number, US-number, SH-number, FM-number

RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES (The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

Road/Lane/Ramp Closure List

FREEWAY CLOSED X MILE	FRONTAGE ROAD CLOSED	F
ROAD CLOSED AT SH XXX	SHOULDER CLOSED XXX FT	
ROAD CLSD AT FM XXXX	RIGHT LN CLOSED XXX FT	F
RIGHT X LANES CLOSED	RIGHT X LANES OPEN	
CENTER LANE CLOSED	DAYTIME LANE CLOSURES	
NIGHT LANE CLOSURES	I-XX SOUTH EXIT CLOSED	
VARIOUS LANES CLOSED	EXIT XXX CLOSED X MILE	F
EXIT CLOSED	RIGHT LN TO BE CLOSED	
MALL DRIVEWAY CLOSED	X LANES CLOSED TUE - FRI	
XXXXXXXX BLVD CLOSED	X LANES SHIFT in Phase	1 mu

Other Cor	ndition List
ROADWORK XXX FT	ROAD REPAIRS XXXX FT
FLAGGER XXXX FT	LANE NARROWS XXXX FT
RIGHT LN	TWO-WAY
NARROWS	TRAFFIC
XXXX FT	XX MILE
MERGING	CONST
TRAFFIC	TRAFFIC
XXXX FT	XXX FT
LOOSE	UNEVEN
GRAVEL	LANES
XXXX FT	XXXX FT
DETOUR X MILE	ROUGH ROAD XXXX FT
ROADWORK	ROADWORK
PAST	NEXT
SH XXXX	FRI-SUN
BUMP XXXX FT	US XXX EXIT X MILES
TRAFFIC	LANES
SIGNAL	SHIFT

Phase 2: Possible Component Lists

Action to Take/Effect on Travel list

offici com				LIS	ST
ROADWORK XXX FT	ROAD REPAIRS XXXX FT		MERGE RIGHT		FORM X LINES RIGHT
FLAGGER XXXX FT	LANE NARROWS XXXX FT		DETOUR NEXT X EXITS		USE XXXXX RD EXIT
RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE		USE EXIT XXX		USE EXIT I-XX NORTH
MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT		STAY ON US XXX SOUTH		USE I-XX E TO I-XX N
LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT		TRUCKS USE US XXX N		WATCH FOR TRUCKS
DETOUR X MILE	ROUGH ROAD XXXX FT		WATCH FOR TRUCKS		EXPECT DELAYS
ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN		EXPECT DELAYS		PREPARE TO STOP
BUMP XXXX FT	US XXX EXIT X MILES		REDUCE SPEED XXX FT		END SHOULDER USE
TRAFFIC SIGNAL XXXX FT	LANES Shift	*	USE OTHER ROUTES		WATCH FOR WORKERS
nust be used wit	h STAY IN LANE in	Phase 2.	STAY IN LANE	*	

APPLICATION GUIDELINES

1. Only 1 or 2 phases are to be used on a PCMS.

- 2. The 1st phase (or both) should be selected from the "Road/Lane/Ramp Closure List" and the "Other Condition List".
- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".

- 4. A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- 5. If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- appropriate.
- 3. EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary. 7. FT and MI. MILE and MILES interchanged as appropriate.
- 8. AT. BEFORE and PAST interchanged as needed.
- 9. Distances or AHEAD can be eliminated from the message if a
- location phase is used.

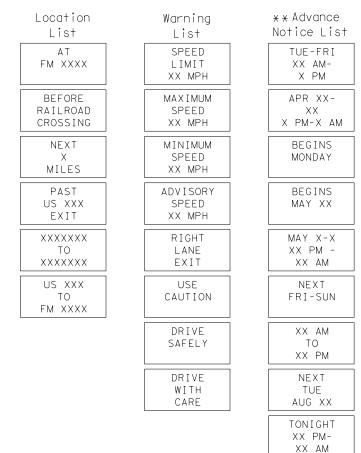
PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

FULL MATRIX PCMS SIGNS

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the shall maintain the legibility/visibility requirement listed above.
- 3. When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and for, or replace that sign.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(same size arrow.

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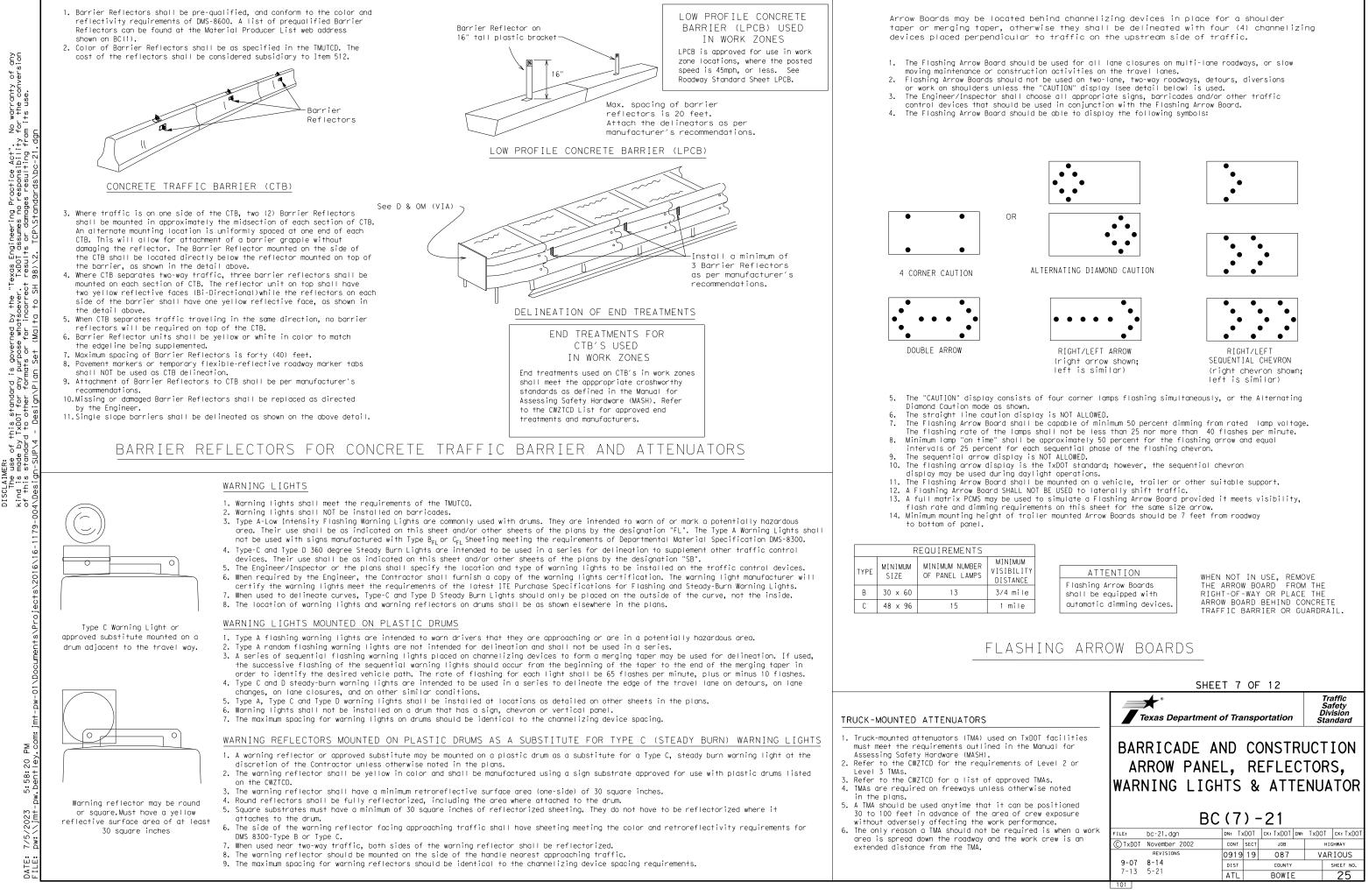
Roadway



X X See Application Guidelines Note 6.

2. Roadway designations IH, US, SH, FM and LP can be interchanged as

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GENERAL NOTES

- 1. For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 5. Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

- Pre-qualified plastic drums shall meet the following requirements:
- Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

BALLAST

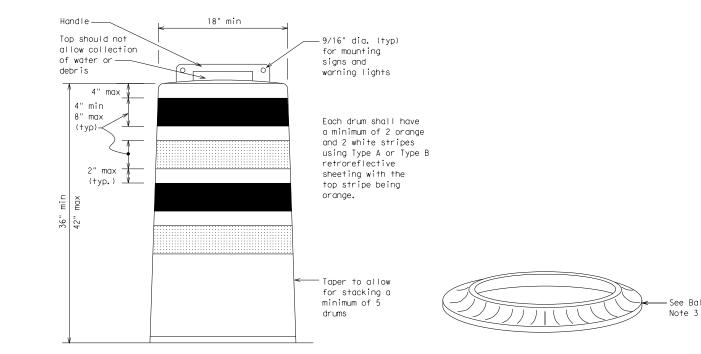
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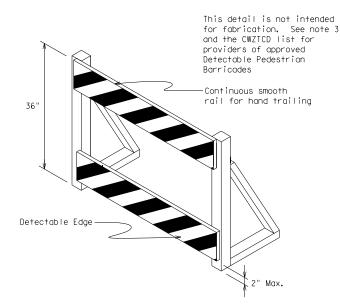
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- Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- 3. Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- 5. When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.

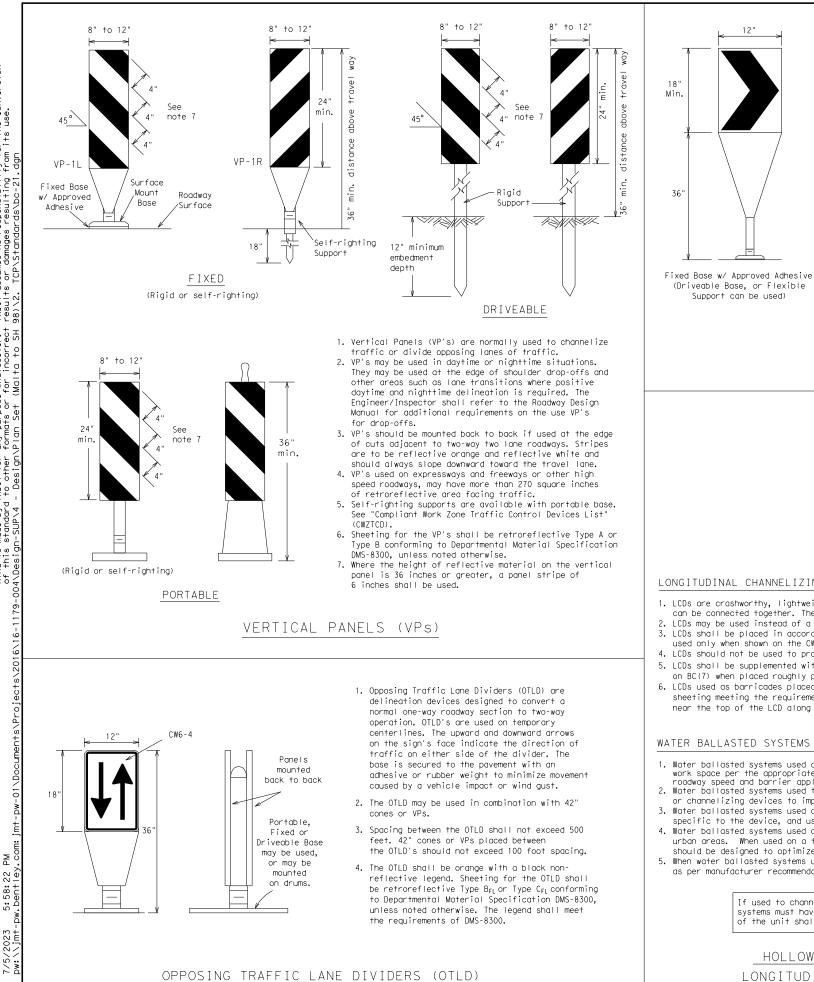




DETECTABLE PEDESTRIAN BARRICADES

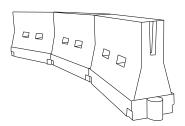
- When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
- Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian path.
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian movements.
- 5. Warning lights shall not be attached to detectable pedestrian barricades.
- Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.

	18" x 24" Sign (Maximum Sign Dimension) Chevron CW1-8, Opposing Traffic Lane Divider, Driveway sign D70a, Keep Right R4 series or other signs as approved by Engineer12" x 24" Vertical Panel mount with diagonals sloping down towards travel way
	Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums
las†	SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS
	 Signs used on plastic drums shall be manufactured using substrates listed on the CWZICD.
	 Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL}Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
	 Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
	4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
	 Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection.
	 Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts.
	7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans.
	 R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.
	SHEET 8 OF 12
	Traffic Safety Division Standard
	BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES
	BC (8) -21
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- 1. The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the outside of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type Br or Type Cr conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- 6. For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS



LONGITUDINAL CHANNELIZING DEVICES (LCD)

12"

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact. 2. LCDs may be used instead of a line of cones or drums.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- 1. Water ballasted systems used as barriers shall not be used solely to channelize road users, but work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness required and barrier application.
- 2. Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation
- or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings. 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements
- specific to the device, and used only when shown on the CWZTCD list. 4. Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH) urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length
- should be designed to optimize road user operations considering the available geometric conditions. 5. When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

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GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- 6. Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.

Posted Speed	Formula	Minimum Desirable Taper Lengths X X			Suggested Maximum Spacing of Channelizing Devices			
		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent		
30	2	150′	165′	180′	30′	60′		
35	$L = \frac{WS^2}{60}$	2051	225′	245′	35′	70′		
40	00	265′	295′	320′	40′	80′		
45		450′	495′	540′	45′	90′		
50		5001	550′	600′	50′	100′		
55	L=WS	550′	605′	660′	55′	110′		
60		600′	660′	720′	60′	120′		
65		650′	715′	780′	65′	130′		
70		700′	770′	840′	70′	140′		
75		750′	825′	900′	75′	150′		
80		800′	880′	960′	80′	160′		

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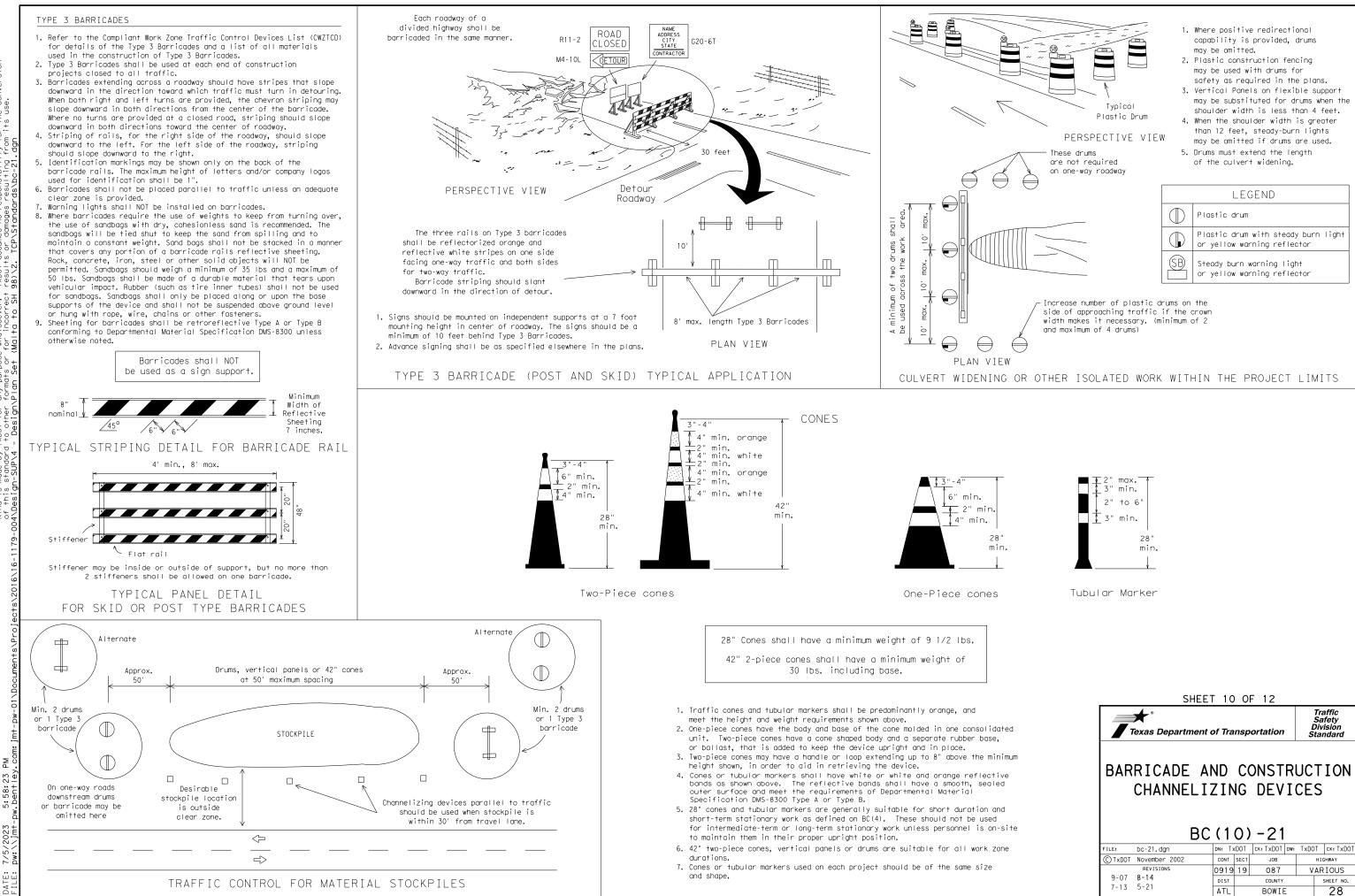
S=Posted Speed (MPH) SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

 \times Taper lengths have been rounded off.

L=Length of Taper (FT.) W=Width of Offset (FT.)

SHEET 9 OF 12	
Texas Department of Transportation	Traffic Safety Division Standard
BARRICADE AND CONSTR CHANNELIZING DEVI	

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© TxDOT	November 2002		CONT	SECT	JOB		HIGHWAY		HWAY
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WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 3. Additional supplemental pavement marking details may be found in the plans or specifications.
- 4. Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ (STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- 1. Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

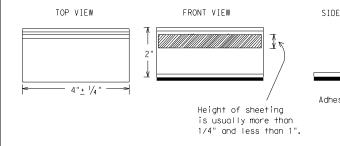
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- 2. The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- 8. Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10.Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECU TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARK TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guiden shall meet the requirements of DMS-8242.
- 2. Tabs detailed on this sheet are to be inspected and accepted by Engineer or designated representative. Sampling and testing is a normally required, however at the option of the Engineer, either or "B" below may be imposed to assure quality before placement of roadway.
 - A. Select five (5) or more tabs at random from each lot or sh and submit to the Construction Division, Materials and Pay Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix (5) tabs at 24 inch intervals on an asphaltic pavement in straight line. Using a medium size passenger vehicle or pi run over the markers with the front and rear tires at a sp of 35 to 40 miles per hour, four (4) times in each directi more than one (1) out of the five (5) reflective surfaces be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARK

- Raised pavement markers used as guidemarks shall be from the approduct list, and meet the requirements of DMS-4200.
- 2. All temporary construction raised pavement markers provided on project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applie butyl rubber pad for all surfaces, or thermoplastic for concresurfaces.

Guidemarks shall be designated as:

YELLOW - (two amber reflective surfaces with yellow body). WHITE - (one silver reflective surface with white body).

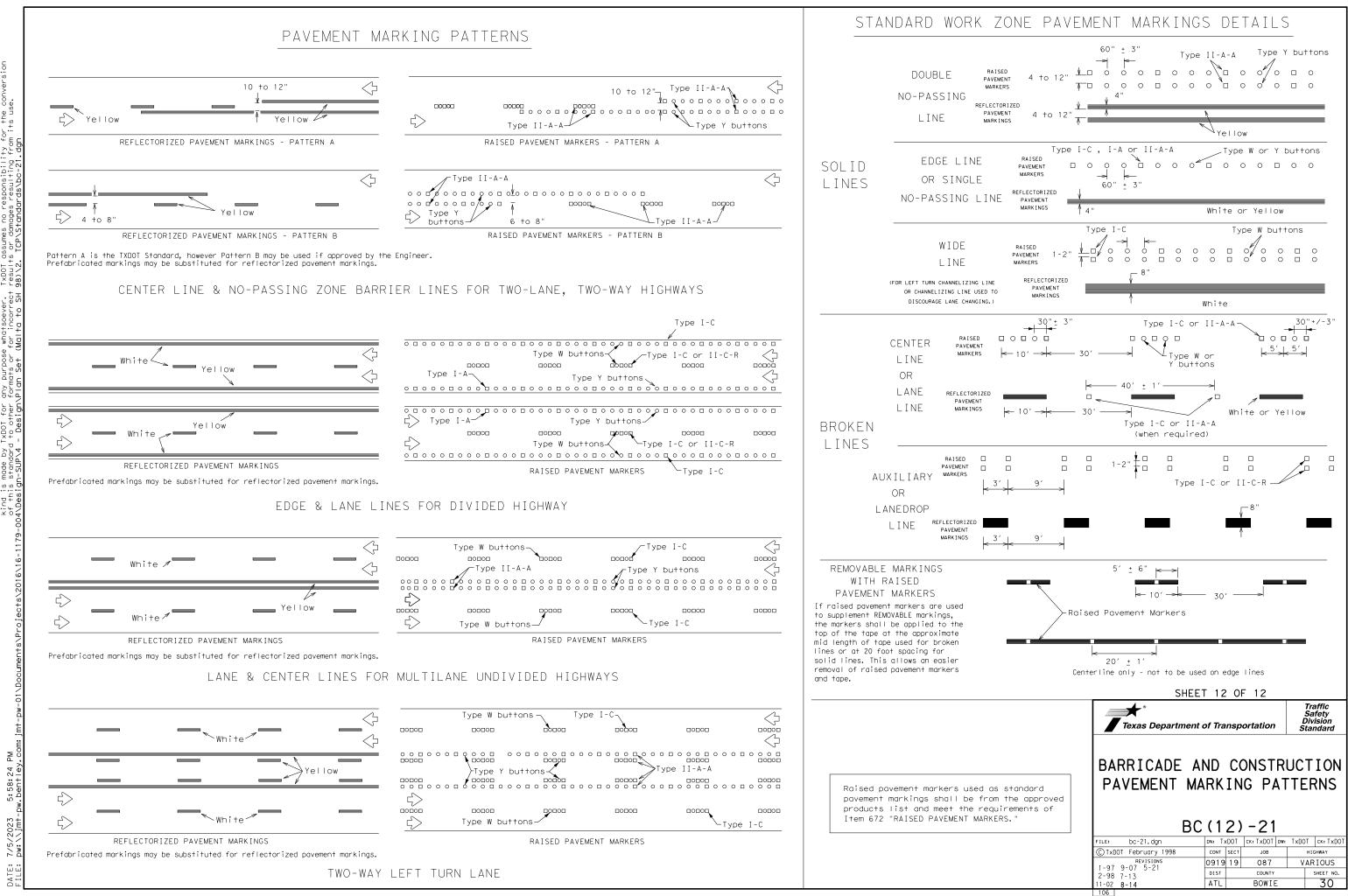
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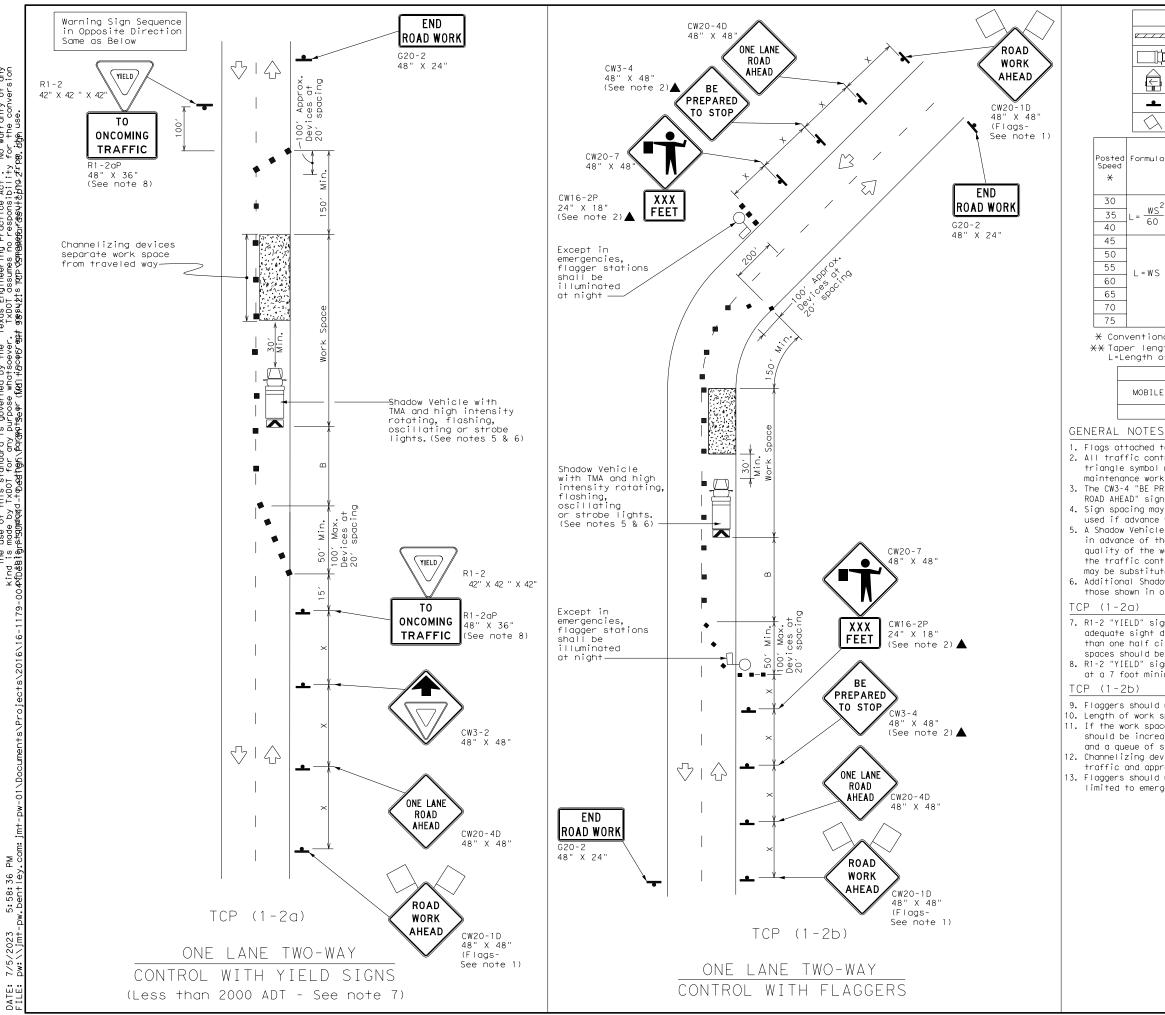
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DEPARTMENTAL MATERIAL SPECIFI	CATIONS
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
TRAFFIC BUTTONS	DMS-4300
/IEW	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS	DMS-8241
TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242
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SHEET 11 OF 1	2
Texas Department of Transporta	Traffic Safety Division
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		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent		^ Distance		
		150′	165′	180′	30′	60′		120′	90'	200′
1	$=\frac{WS^2}{60}$	205′	2251	245′	35′	70′		160′	120′	250′
	00	265′	295′	3201	40′	80′		240′	155′	305′
		450′	495′	540′	45′	90′		320′	1957	360′
		500′	550′	600′	50′	100′		400′	240′	425′
]	L=WS	550′	605′	660′	55′	110′		500′	295′	495′
	L - 11 3	600′	660′	720′	60′	120′		600′	350′	570′
		650′	715′	780′	65′	130′		700′	410′	645′
		700′	770′	840′	70′	140′		800′	475′	730′
		750′	825′	900′	75′	150′		900′	540′	820′

X Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH) TYPICAL LICACE

TYPICAL USAGE							
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY			
	✓	✓					
				· · · · · · · · · · · · · · · · · · ·			

1. Flags attached to signs where shown are REQUIRED.

2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved by the Engineer.

3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4D "ONE LANE ROAD AHEAD" sign, but proper sign spacing shall be maintained.

4. Sign spacing may be increased or an additional CW20-1D "ROAD WORK AHEAD" sign may be used if advance warning ahead of the flagger or R1-2 "YIELD" sign is less than 1500 feet. 5. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.

6. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect wider work spaces.

7. R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work spaces should be no longer than one half city block. In rural areas on roadways with less than 2000 ADT, work spaces should be no longer than 400 feet.

8. R1-2 "YIELD" sign with R1-2aP "TO ONCOMING TRAFFIC" plaque shall be placed on a support at a 7 foot minimum mounting height.

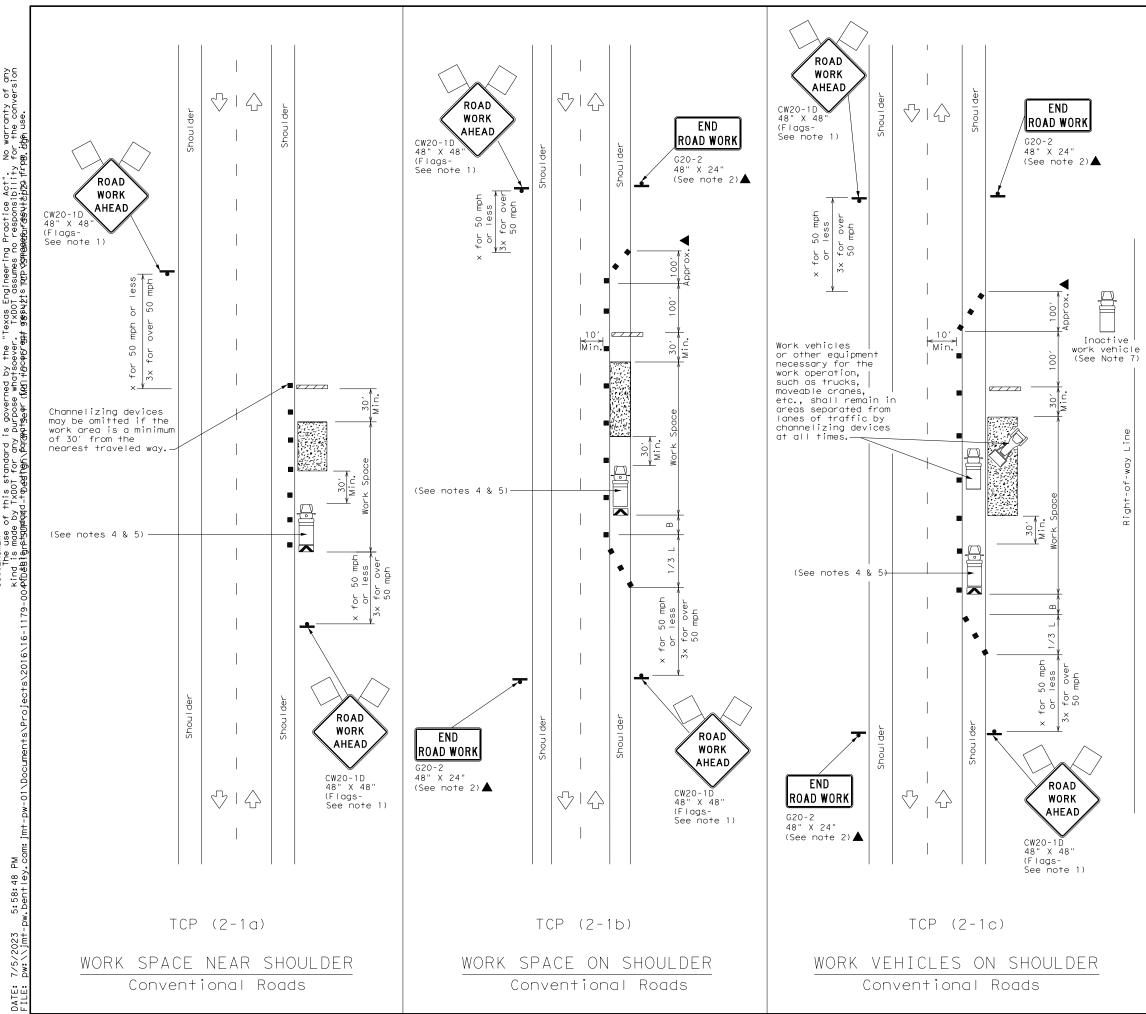
9. Flaggers should use two-way radios or other methods of communication to control traffic. 10. Length of work space should be based on the ability of flaggers to communicate. 11. If the work space is located near a horizontal or vertical curve, the buffer distances

should be increased in order to maintain adequate stopping sight distance to the flagger and a queue of stopped vehicles (see table above).

12. Channelizing devices on the center-line may be omitted when a pilot car is leading traffic and approved by the Engineer.

13. Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to emergency situations.

Texas Department	,	Traffic Operations Division Standard					
TRAFFIC CONTROL PLAN ONE-LANE TWO-WAY TRAFFIC CONTROL TCP(1-2)-18							
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1-97 2-18	ATL		BOWI	E	31		



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~~~~~	Type 3 Barricade		Channelizing Devices
□ ‡	Heavy Work Vehicle		Truck Mounted Attenuator (TMA)
	Trailer Mounted Flashing Arrow Board	(M)	Portable Changeable Message Sign (PCMS)
÷	Sign	$\triangleleft$	Traffic Flow
$\bigtriangleup$	Flag	LO	Flagger

Posted Speed <del>X</del>	Formula	D	Minimur esirab er Leno XX	le gths	Špacir Channe		Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"
30	<u>ws</u> ²	150′	165′	180′	30′	60′	120′	90′
35	$L = \frac{WS}{60}$	205′	225′	245′	35′	70′	160′	120′
40	60	265′	295′	3201	40′	80′	240′	155′
45		450′	495′	540′	45′	90′	320′	195′
50		500′	550′	600′	50′	100′	400′	240'
55	L=WS	550′	605′	660′	55′	110′	500′	295′
60	L 113	600′	660′	720′	60′	120′	600′	350′
65		650′	715′	780′	65′	130′	700′	410′
70		700′	770′	840′	70′	140′	800′	475′
75		750′	825′	900′	75′	150′	900′	540′

X Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

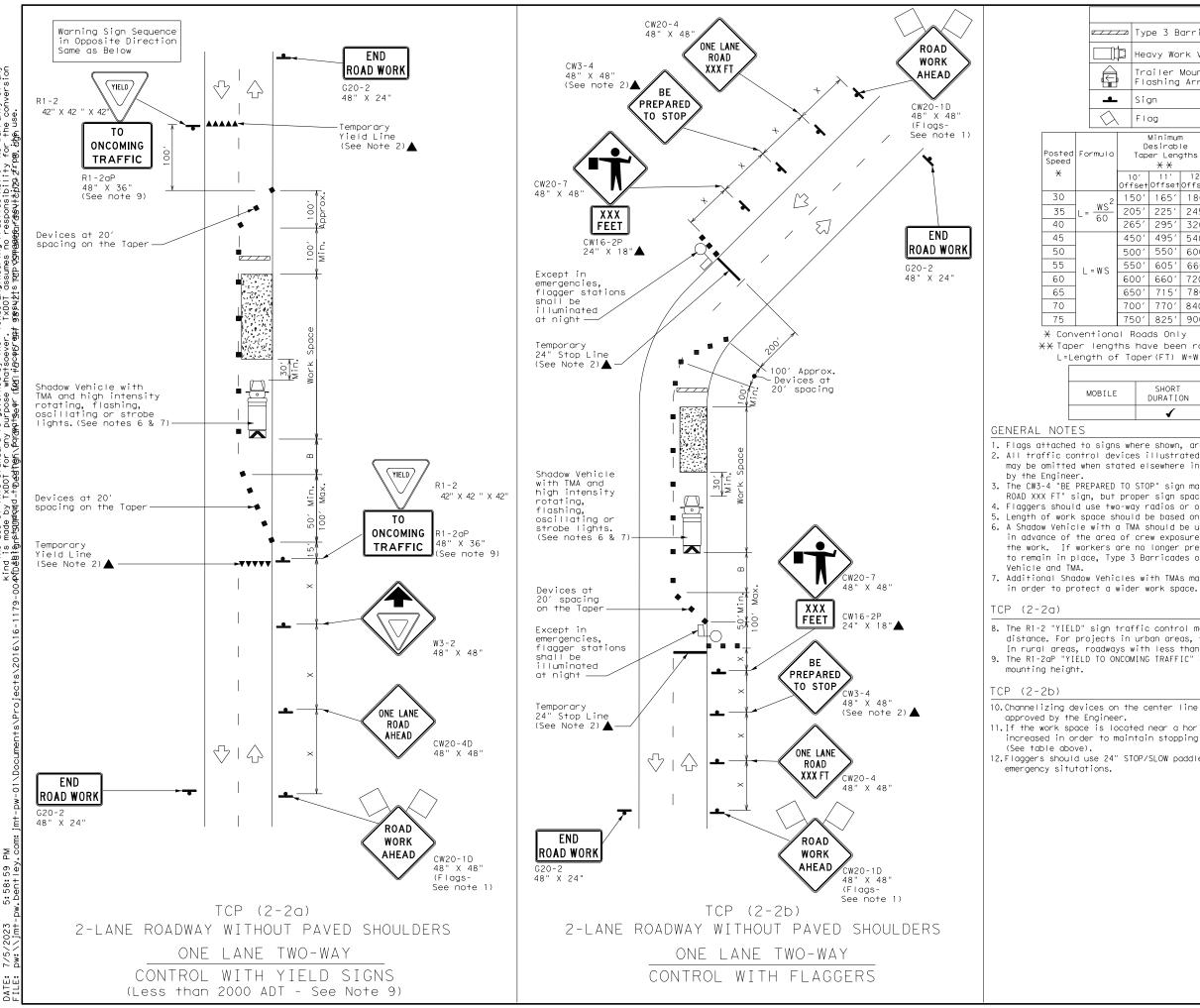
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MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY
	✓	1	1	1

## GENERAL NOTES

1. Flags attached to signs where shown, are REQUIRED.

- 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated in the plans, or for routine maintenance work, when approved by the Engineer.
- 3. Stockpiled material should be placed a minimum of 30 feet from
- 4. Shadow Vehicle with TMA and high intensity rotating, flashing, oscillating or strobe lights. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.
- 5. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect a wider work space. 6. See TCP(5-1) for shoulder work on divided highways, expressways and
- freeways. 7. Inactive work vehicles or other equipment should be parked near the
- right-of-way line and not parked on the paved shoulder. 8. CW21-5 "SHOULDER WORK" signs may be used in place of CW20-1D
- "ROAD WORK AHEAD" signs for shoulder work on conventional roadways.

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	10' Offs	11' etOffset	12' Offset	On a Taper	On a Tangen	t	Distance	"B"	
2	150	)' 165'	180′	30′	60′		1201	90′	200′
_	205	225'	245′	35′	70′		1601	120′	250′
	265	2951	320′	40′	80′		240′	155′	305′
	450	)' 495'	540′	45′	90′		320′	1957	360′
	500	oʻ 550ʻ	600′	50′	100′		400′	240′	425′
	550	6051	660′	55′	110′		500′	295′	495′
	600	660′	720′	60′	120′		600′	350′	570′
	650	)' 715'	780′	65′	130′		700′	410′	645′
	700	° 770′	840′	70′	140′		800′	475′	730′
	75C	° 825′	900′	75′	150′		900′	540′	820′

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

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1. Flags attached to signs where shown, are REQUIRED. 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved

3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4 "ONE LANE ROAD XXX FT" sign, but proper sign spacing shall be maintained. 4. Flaggers should use two-way radios or other methods of communication to control traffic. 5. Length of work space should be based on the ability of flaggers to communicate. 6. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow

7. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown

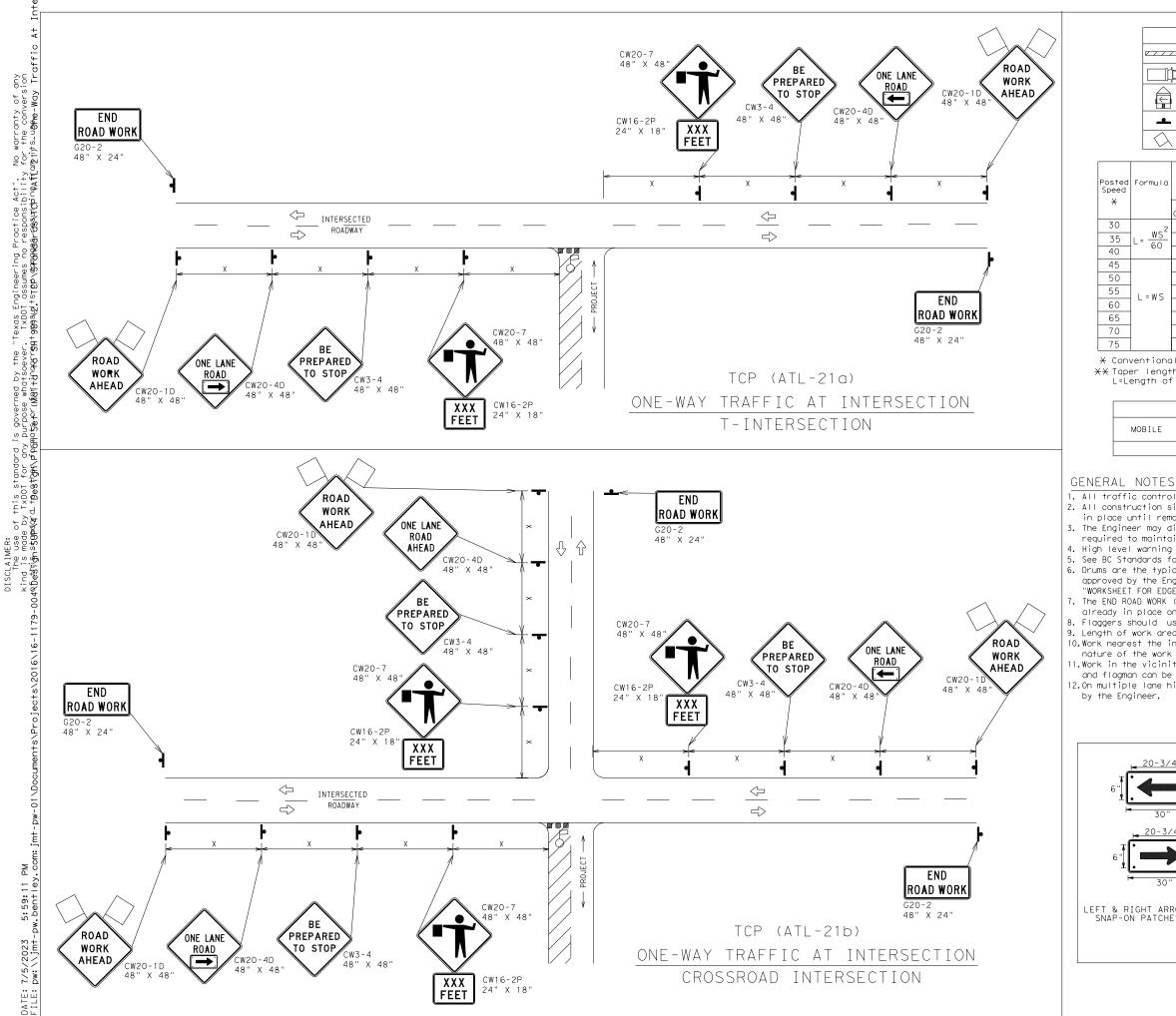
8. The R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work space should be no longer than one half city block. In rural areas, roadways with less than 2000 ADT, work space should be no longer than 400 feet. 9. The R1-2aP "YIELD TO ONCOMING TRAFFIC" sign shall be placed on a support at a 7 foot minimum

10. Channelizing devices on the center line may be omitted when a pilot car is leading traffic and

11. If the work space is located near a horizontal or vertical curve, the buffer distances should be increased in order to maintain stopping sight distance to the flagger and a queue of stopped vehicles.

12.Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to

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	Type 3 Barricade		Channelizing Devices
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	Trailer Mounted Flashing Arrow Board	Ω,	Portable Changeable Message Sign (PCMS)
•	Sign	Ŷ	Traffic Flow
$\bigtriangleup$	Flag	LO	Flagger

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	10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"	
	150′	165′	180′	30′	60′	120′	90′	200′
$=\frac{WS^2}{60}$	205′	225′	245′	35′	70′	160′	120′	250′
00	265′	295′	320′	40′	80′	240′	155′	305′
	450′	495′	540′	45′	90′	320′	195′	360′
	500′	550′	600′	50′	100′	400′	240′	425′
L=WS	550′	605′	660′	55′	110′	500′	295′	495′
	600′	660′	720′	60′	120′	600′	350′	570′
	650′	715′	780′	65′	130′	700′	410′	645′
	700′	770'	840′	70′	140′	800′	475′	730′
	750′	825′	900′	75′	150′	900′	540′	820′

* Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

		TYPICAL L	ISAGE	
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I. All traffic control devices illustrated are REQUIRED unless approved by the Engineer. 2. All construction signs and barricades placed during any phase of work shall remain in place until removal is approved by the Engineer.

3. The Engineer may direct the Contractor to furnish additional signs and barricades as required to maintain traffic flow, detours and motorist safety during construction. 4. High level warning flags should be used on advance warning signs during daytime operations. See BC Standards for additional sign details.

6. Drums are the typical channelizing device. Cones or other devices may be used if approved by the Engineer. Channelizing devices shall also be in accordance with "WORKSHEET FOR EDGE CONDITION TREATMENT TYPES."

7. The END ROAD WORK (G20-2) sign may be omitted when it conflicts with G20-2 signs already in place on the project.

8. Flaggers should use two-way radios or other methods of communication to control traffic.

9. Length of work area should be based on the ability of flaggers to communicate.

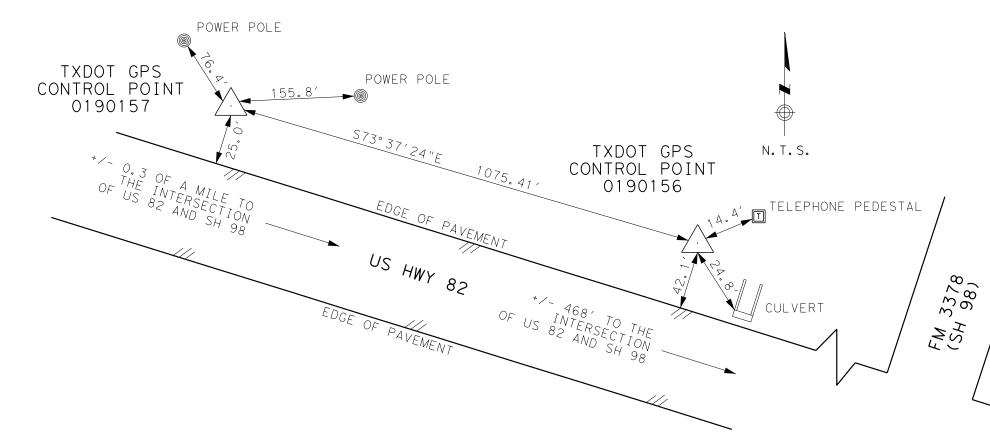
10.Work nearest the intersection should be done during the lowest traffic volume hours, when nature of the work allows.

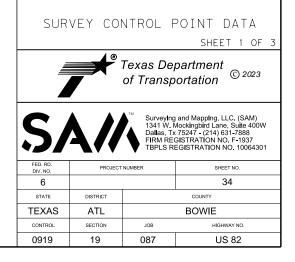
11. Work in the vicinity of the intersection should be prioritized through completion so signage and flagman can be moved from the intersection as work progresses away from the intersection. 12.0n multiple lane highways, an additional flagman may be needed on each approach as directed

8"						
30"	Texas Depo Atlanta				porta	ition
	TRAFFIC					N
30"	ONE-WA	١	IR	AFFIC	,	
GHT ARROWS ON VINYL PATCHES 30" X 8"	AT INT	ERS	SE	CTION	1	
	TCP (A	TL		$\frac{21}{1}$ - 1		
	FILE: atl-21.dgn	dn: Tx		CK: TxDOT DW:		
	© T×DOT January 2014		SECT	JOB		IGHWAY
	REVISIONS		19	087		RIOUS
		DIST		COUNTY		SHEET NO.
		ATL		BOWIE		34
	021					

		ΤΥΡΟΤ	<b>GPS CONTRO</b>		0190156	
Local Name (if	any): CON				Date:	11/29/22
Project Name:	US 8	32	Project CSJ:	0046-04-057	County:	BOWIE
Project Limits:	FM 1	840 TO SH 98				
Monument Typ	e: 3 1/4	" TxDOT Aluminun	n Disk on a 5/8" Iron	Rod found in co	oncrete flus	n with the ground.
Intervisible Poir	nt:	0190157	Surveyed a) T	XDOT VRS NE	TWORK	
Grid Bearing:		N 73°37'24" W	From: b)			
Grid Distance (	(FT):	1075.41	c)			
		Directions from	Major Highway Inte	rsection to Contr	rol Point	
From the inters	section of l		Northwest along US			ne monument on
the Northeast s	ide of US	82. Located 14.4' S	outhwest of a teleph	one pedestal, 24	.8' Northwe	st of a culvert, and 42.1'
Northeast of th	e Northeas	st edge of pavement	t of US 82.			
Northeast of th	e Northeas	st edge of pavement	t of US 82.			
Northeast of th	e Northeas	st edge of pavement	t of US 82.			
Northeast of th	e Northeas		t of US 82. djusted Horizontal C	ontrol Data		
	e Northeas				AV	
Order:		Ad Survey Method:	djusted Horizontal C GPS ☑	TR	AV 🗌 🛛 Mapping Ang	and the second sec
Order:	0.999899	Ac Survey Method: 6507 Combine	djusted Horizontal C GPS ☑	TR	Mapping Ang	and the second sec
Order: Scale Factor: Texas State	0.999899 9 Plane	Ac Survey Method: 6507 Combine Grid Co	djusted Horizontal C GPS ☑ ed Factor: 0	TR .9998849374	Vapping Ang face	le: 2°11'49"
Order: Scale Factor: Texas State Grid Data NAD	0.999899 9 Plane	Ad Survey Method: 6507 Combine Grid Co (U.S. Su	djusted Horizontal C GPS ☑ ed Factor: 0 ordinates	TR .9998849374 M TXDOT Sur	Vapping Ang face	le: 2°11'49" Surface Coordinates
Order: Scale Factor: Texas State Grid Data NAD State:	0.999899 Plane 83 (1993)	Ad Survey Method: 6507 Combine Grid Co (U.S. Su	djusted Horizontal C GPS ed Factor: 0 ordinates rvey Feet)	TR .9998849374 M TXDOT Sur	Vapping Ang face actor N:	le: 2°11'49" Surface Coordinates (U.S. Survey Feet)
Order: Scale Factor:	0.999899 Plane 83 (1993) Texas	Ac Survey Method: 6507 Combine Grid Co (U.S. Su N: 7242	djusted Horizontal C GPS ed Factor: 0 ordinates rvey Feet)	.9998849374 M TXDOT Surf Adjustment F	Vapping Ang face actor N:	le: 2°11'49" Surface Coordinates (U.S. Survey Feet)
Order: Scale Factor: Texas State Grid Data NAD State: Zone:	0.9998999 Plane 83 (1993) Texas TXNC	Ac Survey Method: 6507 Combine Grid Co (U.S. Su N: 7242	djusted Horizontal C GPS ☑ ed Factor: 0 ordinates rvey Feet) 2585.9670 6528.1825	.9998849374 M TXDOT Surf Adjustment F	Mapping Ang face actor N:	le: 2°11'49" Surface Coordinates (U.S. Survey Feet) 7243455.0773
Order: Scale Factor: Texas State Grid Data NAD State: Zone:	0.9998999 Plane 83 (1993) Texas TXNC	Ac Survey Method: 3507 Combine Grid Co (U.S. Su N: 7242 E: 3196	djusted Horizontal C GPS ☑ ed Factor: 0 ordinates rvey Feet) 2585.9670 6528.1825 NAD	TR           .9998849374         M           TXDOT Surf         Adjustment F           Adjustment F         1.000120	Mapping Ang face actor N:	le: 2°11'49" Surface Coordinates (U.S. Survey Feet) 7243455.0773 3196911.7659

Local Name (if a							Date:	11/29	
Project Name	US			Project CSJ:	0046-04-0	57 Co	ounty:	BOV	VIE
Project Limits:			O SH 98		D al faun al	·	<b>f</b> l		
Monument Type				m Disk on a 5/8" I				vith the gro	una.
Intervisible Point	t:		0190156	Surveyed	a) TxDOT VRS	SNETWOR	K		
Grid Bearing:			3°37'24" E		) \				
Grid Distance (F	-1):		1075.41	n Major Highway	<u>;)</u>	<u> </u>			
From the interse				o Northwest along					
				Level - Caller MI	orthoast odgo (	of pavement	of US 8	32 76 4' Soi	utheast of a
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the Northeast si					Ji li least euge (	, parement		, , , , , , , , , , , , , , , , , , , ,	
the Northeast si					o meast edge t	, parement		2, 10.1 00.	
the Northeast si power pole, and					Jineast euge (	, per em en		2, 10.1 00.	
the Northeast si		Vest of	another pow	er pole. djusted Horizonta	al Control Data			·	
the Northeast si power pole, and Order:	155.8' \	Vest of	another pow A rey Method:	er pole. djusted Horizonta GPS	al Control Data	TRAV		OTHER	
the Northeast si power pole, and	155.8' \	Vest of	another pow A rey Method:	er pole. djusted Horizonta	al Control Data	TRAV 4 Mappir	ig Angle	OTHER : 2°	11'43"
the Northeast si power pole, and Order: Scale Factor: ( Texas State	1 155.8' \ 0.99989 Plane	Vest of Surv 97715	another pow A rey Method: Combine Grid Co	er pole. djusted Horizonta GPS ed Factor: pordinates	al Control Data       Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data	TRAV 34 Mappir	ng Angle	OTHER : 2° urface Coor	11'43" dinates
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the Northeast si power pole, and Order: Scale Factor: ( Texas State	1 155.8' \ 0.99989 Plane	Vest of Surv 97715	Another power Another power Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another Another An	er pole. djusted Horizonta GPS ed Factor: pordinates	al Control Data       Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data      Image: Control Data	TRAV 34 Mappir	ng Angle	OTHER : 2° urface Coor	tl'43" dinates Feet)
the Northeast si power pole, and Order: Scale Factor: Texas State Grid Data NAD8	0.99989 Plane 33 (1993	Vest of Surv 97715 N:	Another pow Arey Method: Combine Grid Co (U.S. Su 724	er pole. djusted Horizonta GPS ed Factor: pordinates irvey Feet) 2889.1401	al Control Data	TRAV 34 Mappir	ng Angle Si ( N:	OTHER 2° urface Coor U.S. Survey 724375	11'43" dinates / Feet) 8.2868
the Northeast si power pole, and Order: Scale Factor: Cexas State Grid Data NAD8 State:	0.99989 Plane 33 (1993 Texas	Vest of Surv 97715	Another pow Arey Method: Combine Grid Co (U.S. Su 724	er pole. djusted Horizonta GPS ed Factor: pordinates urvey Feet)	al Control Data	TRAV 34 Mappir 7 Surface ent Factor	ng Angle Si	OTHER 2° urface Coor U.S. Survey	11'43" dinates / Feet) 8.2868
the Northeast si power pole, and Order: Scale Factor: ( Texas State Grid Data NAD8 State: Zone: Code:	0.99989 Plane 33 (1993 Texas TXNC	Vest of Surv 97715 N: E:	Another pow Arey Method: Combine Grid Co (U.S. Su 724	er pole. djusted Horizonta GPS ed Factor: pordinates urvey Feet) 2889.1401 5496.5289 N	al Control Data 0.999885043 TXDO Adjustm 1.00 AD83 (1983)	TRAV 34 Mappir 7 Surface ent Factor	ng Angle Si ( N:	OTHER 2° urface Coor U.S. Survey 724375	2    11'43" dinates / Feet) 8.2868 9.9885
the Northeast si power pole, and Order: Scale Factor: Cexas State Grid Data NAD8 State: Zone:	0.99989 Plane 33 (1993 Texas TXNC	Vest of Surv 97715 N: E: Po	Another power Another power (Another another a Another a Another Another Another a Another a Ano	er pole. djusted Horizonta GPS ed Factor: pordinates rvey Feet) 2889.1401 5496.5289 N 33	al Control Data          0.999885043         TXDO         Adjustm         1.00	TRAV 34 Mappir 7 Surface ent Factor	ng Angle Si ( N:	OTHER 2° urface Coor U.S. Survey 724375 319587	2

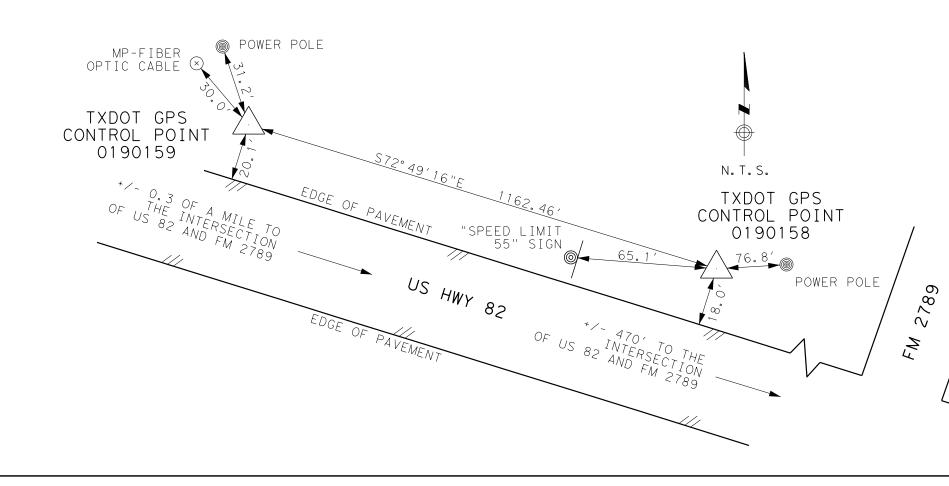




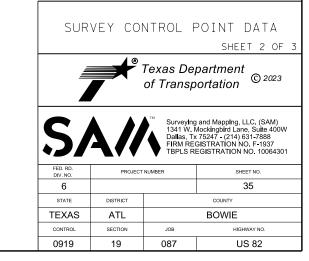
KREINEF

			TYDOT				0400	1 50		
				GPS CON	IRU	L POINT	0190			
Local Name (i	f any): CC	ONTROL :	3					Date:	11/29	9/22
Project Name	: US	82		Project CSJ:		0046-04-057	Co	unty:	BOV	VIE
Project Limits	: FM	1840 TO	SH 98							
Monument Ty	pe: 3 1/	4" TxDO1	r Aluminum	Disk on a 5/8" I	Iron F	Rod found in a	concrete	lush wit	h the grou	und.
Intervisible Po	int:	019	90159	Surveyed	a) Tx	OOT VRS N	ETWOR	<		
Grid Bearing:		N 72°	49'16" W	From:	b)					
Grid Distance	(FT):	11	62.46		c)					
		Dire	ctions from	Major Highway	Inters	section to Cor	ntrol Point			
From the inter	section of	US 82 an	d FM 2789	go Northwest al	long l	JS 82 approxi	imately 47	'0' to the	e monume	ent on
the Northeast	side of US	6 82. Loca	ted 18.0' No	ortheast of the N	Vorthe	east edge of p	avement	of US 82	2, 76.8' We	est of a
power pole, ar	ACE 11 E									
poner poie, ai	10 05.1 Ea	ast of a "S	SPEED LIMI	T 55" sign.						
	10 65.1 E	ast of a "S	SPEED LIMI	T 55" sign.						
power pole, di	na 65.1 Ea	ast of a "S	PEED LIMI	T 55" sign.						
	10 05.1 E	ast of a "S		T 55" sign. Ijusted Horizonta	al Co	ntrol Data				
Order:							TRAV		OTHER	2
		Survey	Ac Method:	ljusted Horizonta	•			g Angle:		R 🔲 10'07"
Order:	0.99990	Survey	Ac Method: Combine	ljusted Horizonta	•		Mappin	<u> </u>		10'07"
Order: Scale Factor:	0.99990 te Plane	Survey	Ac Method: Combine Grid Co	ljusted Horizonta GPS ed Factor:	•	9998865464	Mappin urface	Sur	2°	10'07" dinates
Order: Scale Factor: Texas Stat	0.99990 te Plane	Survey	Ac Method: Combine Grid Co (U.S. Su	ljusted Horizonta GPS ed Factor: ordinates	•	9998865464 TXDOT S	Mappin urface	Sur	2° face Coor	10'07" dinates 7 Feet)
Order: Scale Factor: Texas Stat Grid Data NAI	0.99990 te Plane 283 (1993	Survey 16246	Ac Method: Combine Grid Co (U.S. Su	ljusted Horizonta GPS ed Factor: ordinates rvey Feet)	•	9998865464 TXDOT S	Mappin urface t Factor	Sur (U	2° face Coor .S. Survey	10'07" dinates 7 Feet)
Order: Scale Factor: Texas Stat Grid Data NAI State:	0.99990 te Plane D83 (1993 Texas	Survey 16246	Ac Method: Combine Grid Co (U.S. Su 7247	ljusted Horizonta GPS ed Factor: ordinates rvey Feet)	•	9998865464 TXDOT S Adjustment	Mappin urface t Factor	Sur (U	2° face Coor .S. Survey	10'07" dinates 7 Feet) 1.6498
Order: Scale Factor: Texas Stat Grid Data NAI State: Zone:	0.99990 te Plane D83 (1993 Texas TXNC	Survey 16246 ) N:	Ac Method: Combine Grid Co (U.S. Su 7247 3180	ljusted Horizonta GPS ed Factor: ordinates rvey Feet) 7481.9522	0.	9998865464 TXDOT S Adjustment	Mappin urface t Factor	Sur (U N:	2° face Coor .S. Survey 724835	10'07" dinates Feet) 1.6498 2.0444
Order: Scale Factor: Texas Stat Grid Data NAI State: Zone:	0.99990 te Plane D83 (1993 Texas TXNC	Survey 16246 ) N: E:	Ac Method: Combine Grid Co (U.S. Su 7247 3180 ion	ljusted Horizonta GPS ed Factor: ordinates rvey Feet) 7481.9522 0520.3816	0.	9998865464 TXDOT S Adjustment 1.0001	Mappin urface t Factor	Sur (U N:	2° face Coor .S. Survey 724835 318090	10'07" dinates <u>Feet)</u> 1.6498 2.0444 T)

			TXDOT	GPS CON	IRU		0190	)159	
Local Name (if	f any): (	CONTRO	L 4				[	Date:	11/29/22
Project Name:		US 82		Project CSJ:		0046-04-057	Co	unty:	BOWIE
Project Limits:		FM 1840 1	O SH 98						
Monument Typ	be: 3	3 1/4" TxD	OT Aluminum	n Disk on a 5/8"	Iron F	Rod found in c	oncrete	flush witl	h the ground.
Intervisible Poi	int:		0190158	Curries and	a) T	XDOT VRS N	ETWOR	<	
Grid Bearing:		S 7	′2°49'16" E	Surveyed From:	b)				
Grid Distance	(FT):		1162.46	FIOIII.	C)				
		C	irections from	n Major Highway	/ Inter	section to Con	trol Point		
From the inters	sectior	n of US 82	and FM 2789	go Northwest a	along I	US 82 approxir	mately 0.3	3 of a mil	le to the monument o
the Northeast	cido of	110 00 1	acted 20 11 M	orthogat of the	North	anat adap of p		af LIC 00	20 0' Southoost of
				Southeast of a			avement	01 05 82	, SOLO SOUTHEAST O
			st, and 31.2' S	Southeast of a	powe	r pole.	avement	01 05 82	, 30.0 Southeast o
		harker pos	st, and 31.2' S	Southeast of a djusted Horizon	powe tal Co	r pole.			
a fiber optic c	able m	narker pos	st, and 31.2' S Ac vey Method:	Southeast of a	powe tal Co ; ☑	r pole.	RAV	g Angle:	
a fiber optic c Order:	able m	narker pos Sur 99017653	st, and 31.2' S Ad vey Method: Combine	Southeast of a djusted Horizon GPS	powe tal Co ; ☑	n pole.	RAV 🗆 Mappin	g Angle:	OTHER 🗌
a fiber optic c Order: Scale Factor:	able m 0.999 e Plane	Sur 99017653	st, and 31.2' S Ac vey Method: Combine Grid Co	Southeast of a djusted Horizon GPS ed Factor:	powe tal Co ; ☑	r pole. ontrol Data 9998868666	RAV □ Mappin	g Angle: Surf	OTHER 2°10'00"
a fiber optic c Order: Scale Factor: Texas State	able m 0.999 e Plane	99017653 993)	st, and 31.2' S Ac vey Method: Combine Grid Co (U.S. Su	Southeast of a djusted Horizon GPS ed Factor: pordinates	powe tal Co ; ☑	r pole. Introl Data 9998868666 TXDOT Su	RAV □ Mappin	g Angle: Surf	OTHER 2°10'00" face Coordinates
a fiber optic c Order: Scale Factor: Texas State Grid Data NAE	able m 0.999 e Plane 083 (19	Sur 99017653 e 993) as N:	st, and 31.2' S Ac vey Method: Combine Grid Co (U.S. Su	Southeast of a djusted Horizon GPS ed Factor: oordinates irvey Feet)	powe tal Co ; ☑	r pole. Introl Data 9998868666 TXDOT Su	RAV Mapping Irface Factor	g Angle: Surl	OTHER 2°10'00" face Coordinates S. Survey Feet)
a fiber optic c Order: Scale Factor: Texas State Grid Data NAE State:	able m 0.999 e Plano 083 (19 Tex	Sur       99017653       e       993)       as     N:	st, and 31.2' S Advey Method: Combine Grid Co (U.S. Su 724	Southeast of a djusted Horizon GPS ed Factor: oordinates irvey Feet)	powe tal Co ; ☑	r pole. ontrol Data 1 99988686666 TXDOT Su Adjustment	RAV Mapping Irface Factor	g Angle: Surl	OTHER 2°10'00" face Coordinates S. Survey Feet)
a fiber optic c Order: Scale Factor: Texas State Grid Data NAE State: Zone:	able m 0.999 e Plane 083 (19 Tex TXN	Sur           99017653           e           993)           as         N:           NC           02         E:	st, and 31.2' S Advey Method: Combine Grid Co (U.S. Su 724	Southeast of a djusted Horizon GPS ed Factor: oordinates irvey Feet) 7825.2510 9409.9168	powe tal Co S ♥ 0.	r pole. ontrol Data 1 99988686666 TXDOT Su Adjustment	RAV Mapping Irface Factor	g Angle: Surf (U. N:	OTHER 2°10'00" face Coordinates S. Survey Feet) 7248694.9900
a fiber optic c Order: Scale Factor: Texas State Grid Data NAE State: Zone:	able m 0.999 e Plane 083 (19 Tex TXN	Sur       99017653       e       993)       as       N:       VC       02       E:	st, and 31.2' S Advey Method: Combine Grid Co (U.S. Su 724 3179	Southeast of a djusted Horizon GPS ed Factor: oordinates irvey Feet) 7825.2510 9409.9168	tal Cc D 0.	ntrol Data 1 99988686666 TXDOT Su Adjustment 1.00012	RAV Mapping Irface Factor	g Angle: Surf (U. N:	OTHER 2°10'00" face Coordinates S. Survey Feet) 7248694.9900 3179791.4462



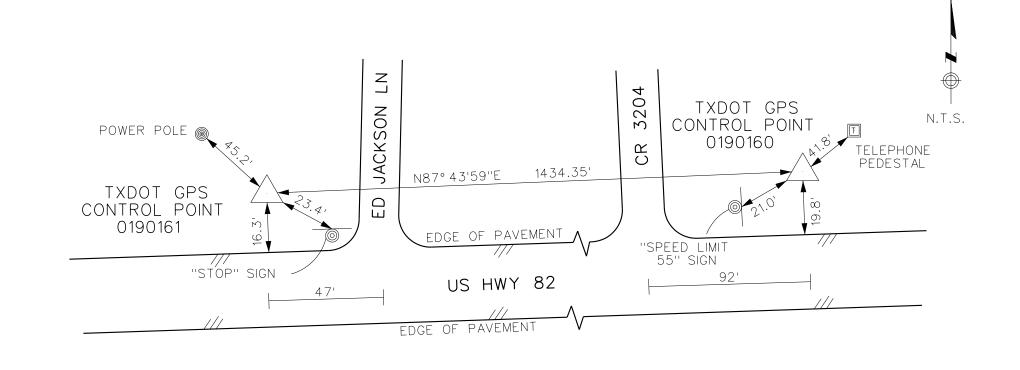




			TVDOT			0404	4.0.4	
				GPS CONT	ROL POINT	0190	0161	
Local Name (	(if any): Co	ONTRO	L 6			0	Date:	11/29/22
Project Name	e: US	S 82		Project CSJ:	0046-04-057	Co	unty:	BOWIE
Project Limits	s: FN	vi 1840 T	O SH 98					
Monument Ty	ype: 3	1/4" TxD	OT Aluminum	Disk on a 5/8" Ir	on Rod set in cor	ncrete flus	h with th	ne ground.
Intervisible Po	oint:		0190160	Surveyed	a) WDS NETWO	RK		
Grid Bearing:		N 8	37°43'59" E	From:	)			
Grid Distance	e (FT):		1434.35	c	;)			
		C	irections from	Major Highway li	ntersection to Co	ntrol Point		
From the inte	ersection of	of US 82	and Ed Jacks	on Ln go East al	ong US 82 appro	ximately 92	2' to the	monument on the
side of US 82	2. Located	l 16.3' N	orth of the Nor	th edge of pavem	nent of US 82, 23,	4' Northwe	est of a	"STOP" sign, and
					, -			0 /
Southeast of	a power p							0,
Southeast of	a power p							
Southeast of	a power p							
Southeast of	a power p							
	a power p	oole.		ljusted Horizonta	l Control Data	TRAV		OTHER 🗌
Order:		oole.	Ac vey Method:	ljusted Horizonta	l Control Data			OTHER 🗌
Southeast of Order: Scale Factor: Texas Sta	: 0.9999	oole.	Ac vey Method: Combine	ljusted Horizonta GPS	l Control Data ☑	TRAV D	g Angle:	OTHER 🗌
Order: Scale Factor:	: 0.9999 ate Plane	50le. Sur 036953	Ac vey Method: Combine Grid Co	djusted Horizonta GPS ed Factor:	I Control Data	TRAV Mappin Surface	g Angle: Sui	OTHER 2°07'34"
Order: Scale Factor: Texas Sta	: 0.9999 ate Plane	Sur 036953 3)	Ac vey Method: Combine Grid Co (U.S. Su	djusted Horizonta GPS ed Factor: ordinates	I Control Data	TRAV Mappin Surface	g Angle: Sui	OTHER 2°07'34" rface Coordinates
Order: Scale Factor: Texas Sta Grid Data NA State:	: 0.9999 ate Plane D83 (199	Sur 036953 3) s N:	Ac vey Method: Combine Grid Co (U.S. Su	djusted Horizonta GPS ed Factor: ordinates rvey Feet)	I Control Data	TRAV Mapping Surface t Factor	g Angle: Sui (L	OTHER 2°07'34" rface Coordinates J.S. Survey Feet)
Order: Scale Factor: Texas Sta Grid Data NA	0.9999 ate Plane D83 (199 Texas	Sur 036953 3) s N:	Ac vey Method: Combine Grid Co (U.S. Su 725;	djusted Horizonta GPS ed Factor: ordinates rvey Feet)	I Control Data	TRAV Mapping Surface t Factor	g Angle: Sui (L	OTHER 2°07'34" rface Coordinates J.S. Survey Feet)
Order: Scale Factor: Texas Sta Grid Data NA State: Zone:	: 0.9999 ate Plane D83 (199 Texas TXNC	Sur 036953 3) 3) 3 5 N: 5 N: 5	Ac vey Method: Combine Grid Co (U.S. Su 725;	djusted Horizonta GPS ad Factor: ordinates rvey Feet) 2159.8508 6514.5833	I Control Data	TRAV Mapping Surface t Factor	g Angle: Sui (L N:	OTHER 2°07'34" rface Coordinates J.S. Survey Feet) 7253030.1102
Order: Scale Factor: Texas Sta Grid Data NA State: Zone:	: 0.9999 ate Plane D83 (199 Texas TXNC	Sur       036953       3)       3       N:       E:	Ac vey Method: Combine Grid Co (U.S. Su 7252 3156	djusted Horizonta GPS ad Factor: ordinates rvey Feet) 2159.8508 3514.5833	I Control Data       0.9998886412       TXDOT S       Adjustmen       1.000	TRAV Mapping Surface t Factor	g Angle: Sui (L N:	OTHER 2°07'34" rface Coordinates J.S. Survey Feet) 7253030.1102 3156893.3647

			TXDO	r gps c	ONTF	ROL POINT	0190	0160	
Local Name	e (if any):	CONTRO	DL 5					Date:	11/29/22
Project Nam	ne:	US 82		Project	CSJ:	0046-04-057	Co	unty:	BOWIE
Project Limi	its:	FM 1840	TO SH 98						
Monument 7	Туре:	3 1/4" Txl	DOT Aluminum	ו Disk on a	a 5/8" Iroi	n Rod set in co	ncrete flus	h with the	e ground.
Intervisible F	Point:		0190161	Cum,	reyed a)	WDS NETWO	RK		
Grid Bearing	g:	S	87°43'59" W		b)				
Grid Distand	ce (FT):		1434.35		лп. с)				
			Directions from	n Major Hig	hway Int	ersection to Co	ntrol Point		
From the inf	tersectio	on of US 8	2 and CR 3204	l go East a	long US	82 approximate	ely 92' to th	ne monun	nent on the North
of US 82. Lo	ocated 1	9.8' North	of the North ed	dge of pave	ement of	US 82, 21.0' N	ortheast of	a "SPEE	ED LIMIT 55" sign,
of US 82. Lo 41.8' South				dge of pave	ement of	US 82, 21.0' N	ortheast of	a "SPEE	D LIMIT 55" sign,
41.8' South		a telephon	e pedestal. A	djusted Ho	rizontal	Control Data		a "SPEE	
41.8' South	west of a	a telephon	e pedestal. A rvey Method:	djusted Ho		Control Data	TRAV		
41.8' Southv Order: Scale Facto	west of a	a telephon Su 99036966	e pedestal. A rvey Method: Combin	djusted Ho	rizontal	Control Data ] 0.9998882731	TRAV 🗌 Mappin	g Angle:	OTHER 2°07'43"
41.8' Southv Order: Scale Facto Texas S	west of a pr: 0.99 itate Plar	a telephon Su 99036966 ne	e pedestal. A rvey Method: Combin Grid Cc	djusted Ho di Factor: pordinates	rizontal GPS ⊻	Control Data ] 0.9998882731 TXDOT \$	TRAV 🗆 Mappin Surface	g Angle: Surf	OTHER 2°07'43" ace Coordinates
41.8' South Order: Scale Facto Texas S Grid Data N	west of a pr: 0.99 itate Plar	a telephon Su 99036966 ne 993)	e pedestal. Av rvey Method: Combin Grid Co (U.S. Su	djusted Ho d Factor: pordinates urvey Feet)	rizontal GPS ☑	Control Data ] 0.9998882731	TRAV 🗆 Mappin Surface	g Angle: Surf	OTHER 2°07'43" ace Coordinates S. Survey Feet)
41.8' Southv Order: Scale Facto Texas S	west of a br: 0.99 tate Plar VAD83 (1 Te:	a telephon Su 99036966 ne 993) xas N:	e pedestal. Av rvey Method: Combin Grid Co (U.S. Su	djusted Ho di Factor: pordinates	rizontal GPS ☑	Control Data 0.9998882731 TXDOT S Adjustmer	TRAV Mappin Surface It Factor	g Angle: Surf	OTHER 2°07'43" ace Coordinates
41.8' South Order: Scale Facto Texas S Grid Data N	west of a br: 0.99 tate Plar VAD83 (1 Te:	a telephon Su 99036966 ne 993)	e pedestal. Av rvey Method: Combin Grid Co (U.S. Su	djusted Ho d Factor: pordinates urvey Feet)	rizontal GPS ☑	Control Data ] 0.9998882731 TXDOT \$	TRAV Mappin Surface It Factor	g Angle: Surf (U. N:	OTHER 2°07'43" ace Coordinates S. Survey Feet)
41.8' South Order: Scale Facto Texas S Grid Data N State:	west of a or: 0.99 state Plar JAD83 (1 Te: TX	a telephon Su 99036966 ne 993) xas N:	e pedestal. Av rvey Method: Combin Grid Co (U.S. Su 725	djusted Ho d Factor: pordinates urvey Feet)	rizontal GPS	Control Data 0.9998882731 TXDOT S Adjustmer	TRAV Mappin Surface It Factor	g Angle: Surf	OTHER 2°07'43" ace Coordinates S. Survey Feet)
41.8' South Order: Scale Facto Texas Si Grid Data N State: Zone:	west of a or: 0.99 state Plar JAD83 (1 Te: TX	a telephon 99036966 ne 993) xas N: NC 902 E:	e pedestal. Av rvey Method: Combin Grid Co (U.S. Su 725	djusted Ho ed Factor: pordinates <u>urvey Feet)</u> 2216.5800	rizontal ( GPS 0 3	Control Data 0.9998882731 TXDOT S Adjustmer	TRAV Mappin Surface It Factor	g Angle: Surf (U. N:	OTHER 2°07'43" ace Coordinates S. Survey Feet) 7253086.8460
41.8' South Order: Scale Facto Texas Si Grid Data N State: Zone:	west of a pr: 0.99 itate Plar VAD83 (1 Te: TX 42	a telephon 99036966 ne 993) xas N: NC 202 E:	e pedestal. Arvey Method: Combin Grid Cc (U.S. Su (U.S. Su 725 315	djusted Ho ed Factor: pordinates <u>urvey Feet)</u> 2216.5800	rizontal GPS GPS 0 3 NAI	Control Data 0.9998882731 TXDOT \$ Adjustmer 1.000	TRAV Mappin Surface It Factor	g Angle: Surf (U. N:	OTHER 2°07'43" ace Coordinates S. Survey Feet) 7253086.8460 3158326.5947

	Adjusted Horizontal Control Data											
Order:				Survey Method: GPS 🗹								TRAV 🗆
Scale Fac	ctor:	0.999	99036	6966 Combined Factor: 0.9						0.999	8882731	Mappin
Texas	State	e Plan	е		Grid Co	ordin	ates				TXDOT S	Surface
Grid Data	NAC	83 (1	993)		(U.S. Su	rvey	Feet)			A	djustmer	t Factor
State:		Te>	as	N:	7252	2216	.5800	)				
Zone:		TXI	٧C								1.000	120
Code:		42	02	E:	3157	7947	.6413	}				
				Pc	osition				NAE	083 (1	983)	
GEODET	IC			La	titude:			3	3°3	0'14.4	0341"	
DATA				Lon	igitude:			9	4°3	5'49.0	9512"	





DESIGN MM	FED.RD. DIV.NO.	FEDERAL	HIGHWAY NO.	
GRAPHICS	6	(SEE T	VARIOUS	
MM	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
S AM CHECK	CONTROL	SECTION JOB		37
JMT	0919	19	087	

# US 82 SUP TRANSITION

	US82TRAIL_T descri				
Point 207			E 3,181,470.75		
Course from 207	to PC US82TRAIL_T_	.3 S 72	° 10′ 29.80" E D	ist 6.	1291
		Curve	Data		
Curve US82TRAIL_ P.I. Station Delta = Degree = Tangent = Length = Radius = External = Long Chord =	.T_3 2902+57.84 15° 12′ 42.35" 28° 38′ 52.40" 26.7066 53.0991 200.0000 1.7752 52.9433	* N (RT)	7,248,010.0194	E	3,181,502.0160
Mid. Ord. = P.C. Station P.T. Station C.C. Back = S	1.7596 2902+31.13 2902+84.23 72° 10′ 29 81″ F	N N N	7,248,018.1946 7,247,995.4595 7,247,827.7954	E E E	3,181,476.5915 3,181,524.4047 3,181,415.3692
Ahead = S Chord Bear = S	56° 57′ 47.45" E 64° 34′ 08.63" E				
			Data		
Curve US82TRAIL_ P.I. Station Delta = Degree = Tangent = Length = Radius = External =	.T_4 2903+12.30 15° 58′ 38.87″ 28° 38′ 52.40″ 28.0681 55.7719 200.0000 1.9599	* N (LT)	7,247,980.1574	E	3,181,547.9347
Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S Chord Bear = S	55.5913 1.9409 2902+84.23 2903+40.00 56° 57′ 47.45″ E 72° 56′ 26.32″ E 64° 57′ 06.88″ E	N N N	7,247,995.4595 7,247,971.9233 7,248,163.1236	E E E	3,181,524.4047 3,181,574.7678 3,181,633.4403
3	32TRAIL_T descripti	on			
Beginning chain	US82TRAIL descript	<b>US 82</b>			
Beginning chain	US82TRAIL descript	US 82			
Beginning chain Point 320	US82TRAIL descript N 7,250,45	US 82	E 3,173,378.64	192 S+	
Beginning chain Point 320	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4	US 82 -ion 91.4778 41.41"	E 3,173,378.64	192 S+ )	a 817+65 <b>.</b> 36
Beginning chain Point 320 Course from 320 Point 321	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4	US 82	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.6 56′26.32″E Dis	192 S+ ) 716 S+	a 817+65.36 a 901+52.66
Beginning chain Point 320 Course from 320 Point 321	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4 N 7,248,02 to PC US82TRAIL_56 56 995+46.79 15° 12′ 12.97" 28° 38′ 52.40" 26.6921 53.0706 200.0000 1.7733	US 82 ion 01.4778 01.41" 26.8815 5 S 72° Curve *	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.6 56′26.32″E Dis	192 S+ ) 716 S+ s+ 9,3	a 817+65.36 a 901+52.66
Beginning chain Point 320 Course from 320 Point 321 Course from 321 Course from 321 Curve US82TRAIL P.I. Station Delta = Degree = Tangent = Length = Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C.	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4 N 7,248,02 to PC US82TRAIL_56 56 995+46.79 15° 12′ 12.97″ 28° 38′ 52.40″ 26.6921 53.0706 200.0000 1.7733 52.9150 1.7577 995+20.10 995+73.17	US 82 ion 01.4778 01.41" 26.8815 5 S 72° Curve *	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.6 56′26.32″E Dis Data	192 S+ ) 716 S+ s+ 9,3	a 817+65.36 a 901+52.66 67.4390
Beginning chain Point 320 Course from 320 Point 321 Course from 321 Curve US82TRAIL- P.I. Station Degree = Tangent = Length = Radius = External = Long Chord = P.C. Station P.T. Station	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4 N 7,248,02 to PC US82TRAIL_56 56 995+46.79 15° 12′ 12.97" 28° 38′ 52.40" 26.6921 53.0706 200.0000 1.7733 52.9150 1.7777 995+20.10	US 82 ion 01.4778 11.41" 26.8815 5 5 72° Curve * N (LT)	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.6 56' 26.32" E Dis Data * 7,245,270.9982 7,245,278.8286 7,245,270.1338	492 S+ 9 716 S+ s+ 9,3 E E	a 817+65.36 a 901+52.66 67.4390 3,190,376.4736 3,190,350.9560 3,190,403.1518
Beginning chain Point 320 Course from 320 Point 321 Course from 321 Course from 321 Curve US82TRAIL- P.I. Station Degree = Tangent = Length = Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. = Back = S Ahead = S	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4 N 7,248,02 to PC US82TRAIL_56 56 995+46.79 15° 12′ 12.97" 28° 38′ 52.40" 26.6921 53.0706 200.0000 1.7733 52.9150 1.7577 995+20.10 95+73.17 72° 56′ 26.32" E 88° 08′ 39.29" E	US 82 ion 1.4778 11.41" 26.8815 5 S 72° Curve * N (LT)	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.67 56′26.32″E Dis 2 Data 7,245,270.9982 7,245,270.9982 7,245,270.1338 7,245,470.0289	492 S+ 9 716 S+ s+ 9,3 E E	a 817+65.36 a 901+52.66 67.4390 3,190,376.4736 3,190,350.9560 3,190,403.1518
Beginning chain Point 320 Course from 320 Point 321 Course from 321 Course from 321 Curve US82TRAIL- P.I. Station Degree = Tangent = Length = Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. = Back = S Ahead = S	US82TRAIL descript N 7,250,49 to 321 S 72° 54′ 4 N 7,248,02 to PC US82TRAIL_56 56 995+46.79 15° 12′ 12.97" 28° 38′ 52.40" 26.6921 53.0706 200.0000 1.7733 52.9150 1.7757 995+20.10 995+73.17 72° 56′ 26.32" E 88° 08′ 39.29" E 80° 32′ 32.81" E	US 82 ion 1.4778 11.41" 26.8815 5 S 72° Curve * N (LT)	E 3,173,378.64 E Dist 8,387.3049 E 3,181,395.6 56′26.32″E Dis Data * 7,245,270.9982 7,245,270.9982 7,245,270.1338 7,245,470.0289	492 S+ 9 716 S+ s+ 9,3 E E E E	a 817+65.36 a 901+52.66 67.4390 3,190,376.4736 3,190,350.9560 3,190,403.1518 3,190,409.6284

Course from PT US82TRAIL_57 to PC US82TRAIL_60 S 72° 56′ 26.32" E Dist 325.0000

		Data	
Curve US82TRAIL_60 P.I. Station 999+77.89 Delta = 15° 10′ 44.60" Degree = 28° 38′ 52.40" Tangent = 26.6485 Length = 52.9849 Radius = 200.0000	N (RT)	7,245,158.2786	E 3,190,
External = 1,7675 Long Chord = 52.8301 Mid. Ord. = 1.7521 P.C. Station 999+51.24 P.T. Station 1000+04.23 C.C. Back = S 72° 56′ 26.32″ E Ahead = S 57° 45′ 41.72″ E Chord Bear = S 65° 21′ 04.02″ E	N N N	7,245,166.0962 7,245,144.0631 7,244,974.8960	E 3,190, E 3,190, E 3,190,
		Data	
Curve US82TRAIL_61 P.I. Station 1000+30.87 Delta = 15° 10′ 42.86 Degree = 28° 38′ 52.40" Tangent = 26.6474 Length = 52.9827	N (LT)	,245,129.8482	E 3,190,
Radius       =       200,0000         External       =       1.7674         Long Chord       =       52.8279         Mid. Ord.       =       1.7519         P.C. Station       1000+04.23         P.T. Station       1000+57.21         C.C.       Back       =       5.7°       45′       41.72″ E	N N N	7, 245, 144. 0631 7, 245, 122. 0306 7, 245, 313. 2303	E 3,190, E 3,190, E 3,190,
Ahead = S 72° 56′ 24.08" E Chord Bear = S 65° 21′ 02.90" E			
Course from PT US82TRAIL_61 to 322	2 S 72°	56' 24.08" E Dis	+ 4,135.5554
Point 322 N 7,243,90	08.7719	E 3,194,815.66	18 Sta 1041
Course from 322 to PC US82TRAIL_66	6 S 72°	57′ 32.07" E Dis	+ 2,474.9767
		Data	
Curve US82TRAIL_66 P.I. Station 1067+13.48 Delta = 33° 54′ 56.27" Degree = 38° 11′ 49.87" Tangent = 45.7389 Length = 88.7909 Radius = 150.0000 External = 6.8185	N (LT)	7,243,170.0573	E 3,197,
Long Chord = 87.5003 Mid. Ord. = 6.5220 P.C. Station 1066+67.74 P.T. Station 1067+56.53 C.C. Back = S 72° 57′ 32.07" E Ahead = N 73° 07′ 31.67" E	N N N	7,243,183.4614 7,243,183.3343 7,243,326.8757	E 3,197, E 3,197, E 3,197,
Chord Bear = S 89° 55′ 00.20" E Course from PT US82TRAIL_66 to 323	3 N 77º	07/ 31 67" E D:~	+ 10 3717
	CT N C	UI DI.UI E DIS	1 12.3111

Point 323

0,791.5240

0,766.0480 0,814.0642 0,707.3755

0,836.6035

0,814.0642 0,862.0784 0,920.7529

41+92.76

7,225.7050

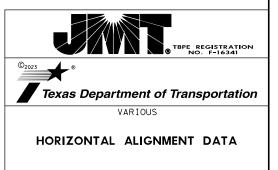
7,181.9743 7,269.4745 7,225.9329

N 7,243,186.9255 E 3,197,281.3135 Sta 1067+68.90

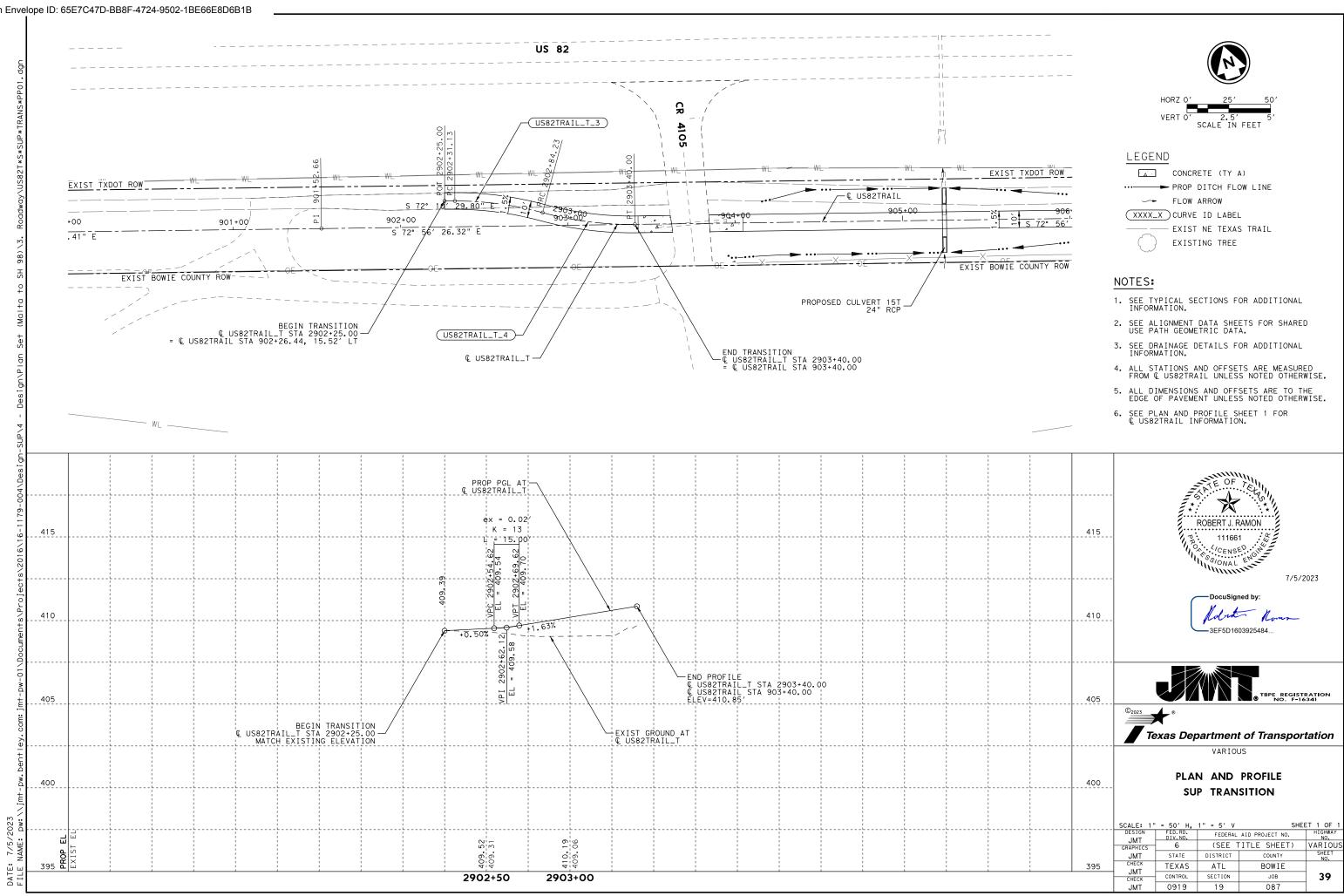
Ending chain US82TRAIL description

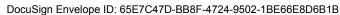


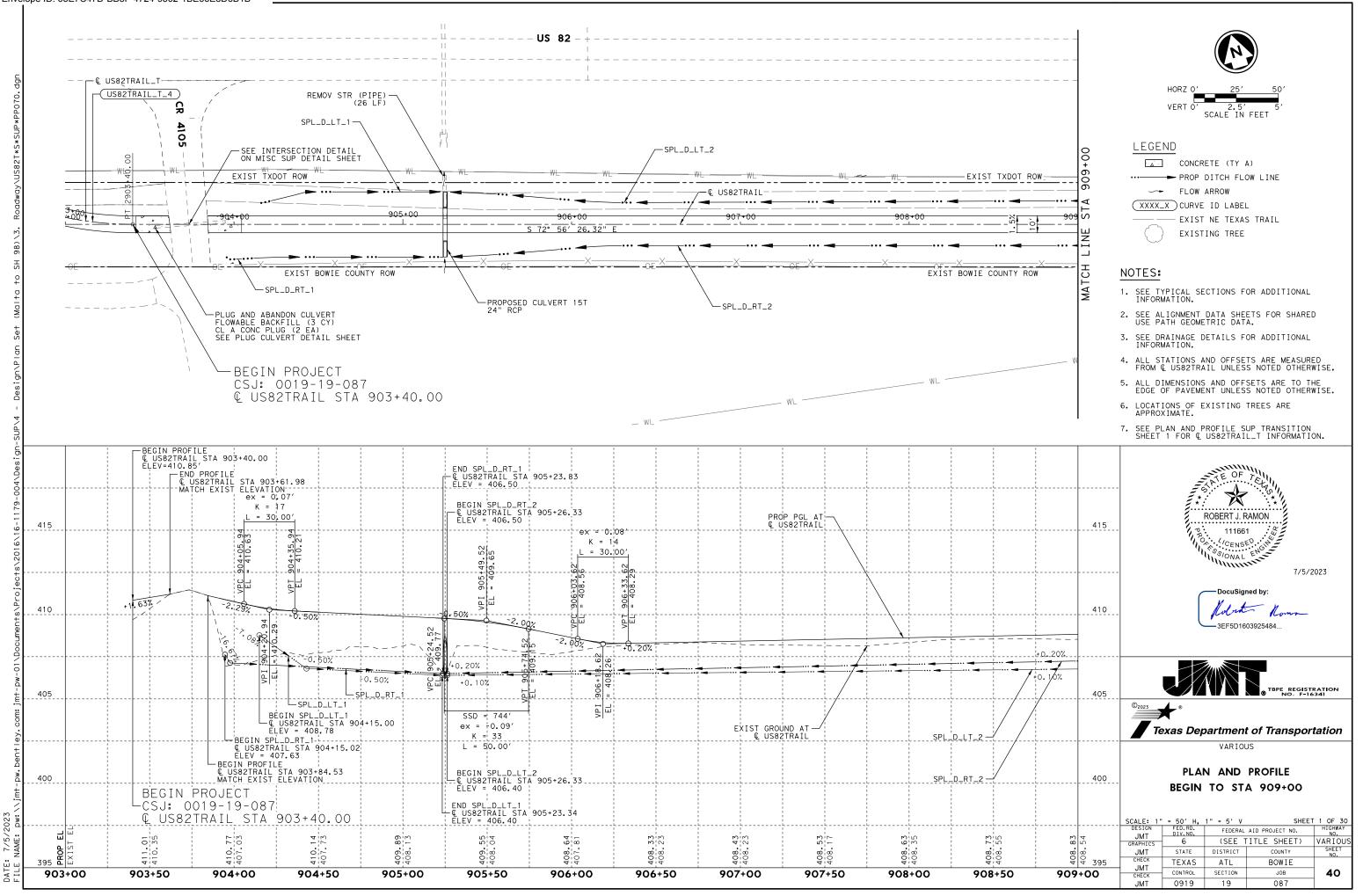
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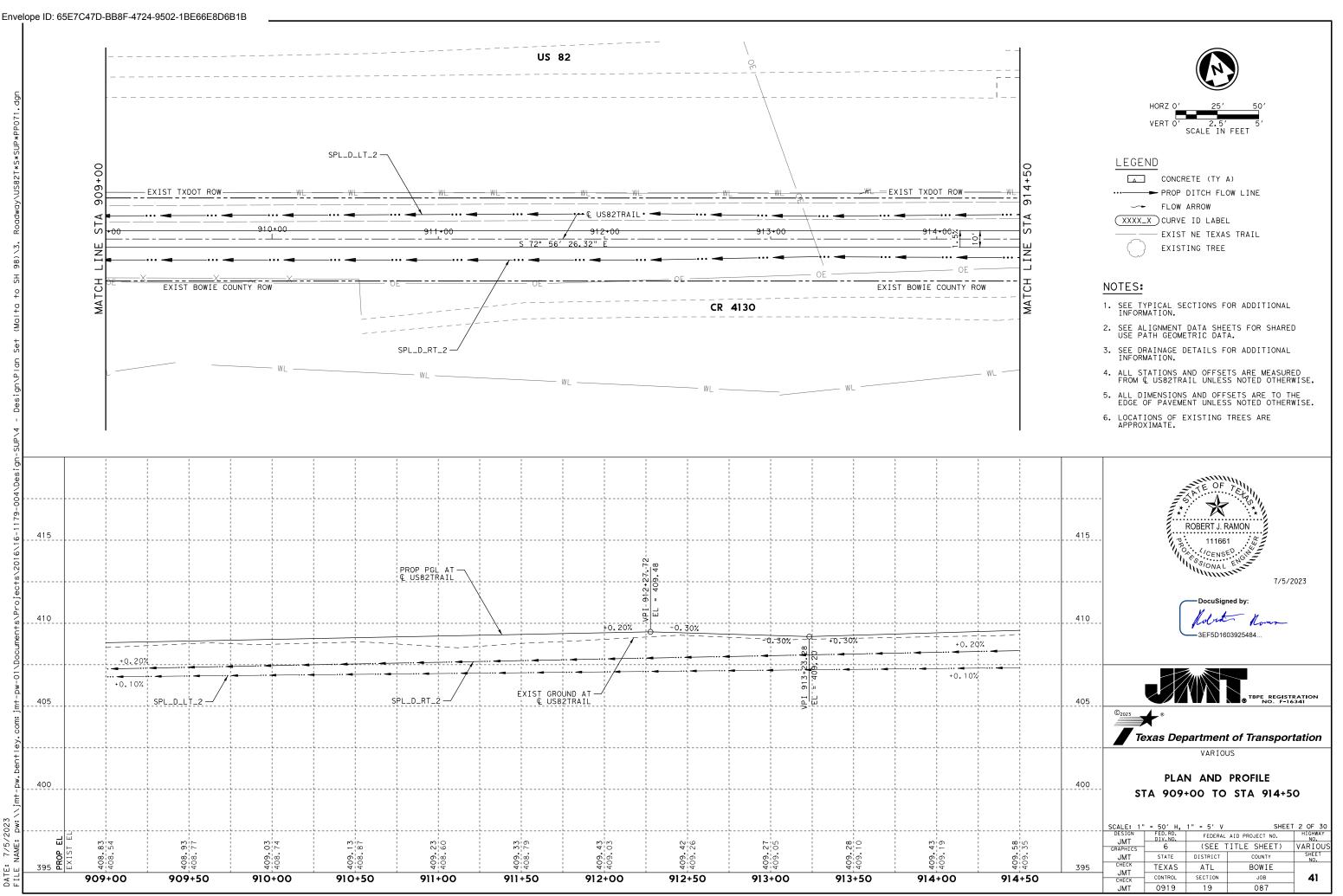


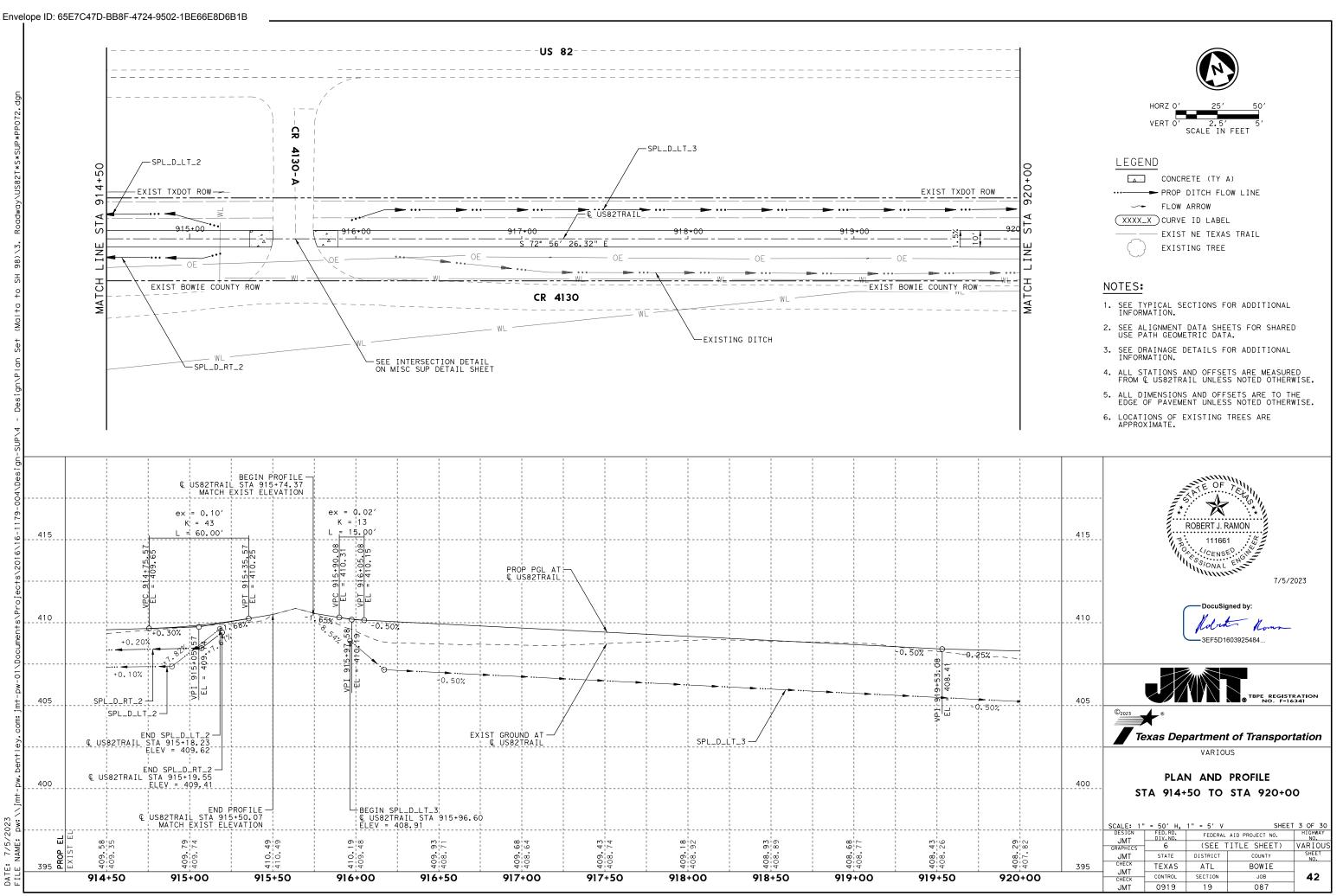
			SHE	ET 1 OF 1
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
JMT CHECK	CONTROL	SECTION	JOB	38
JMT	0919	19	087	



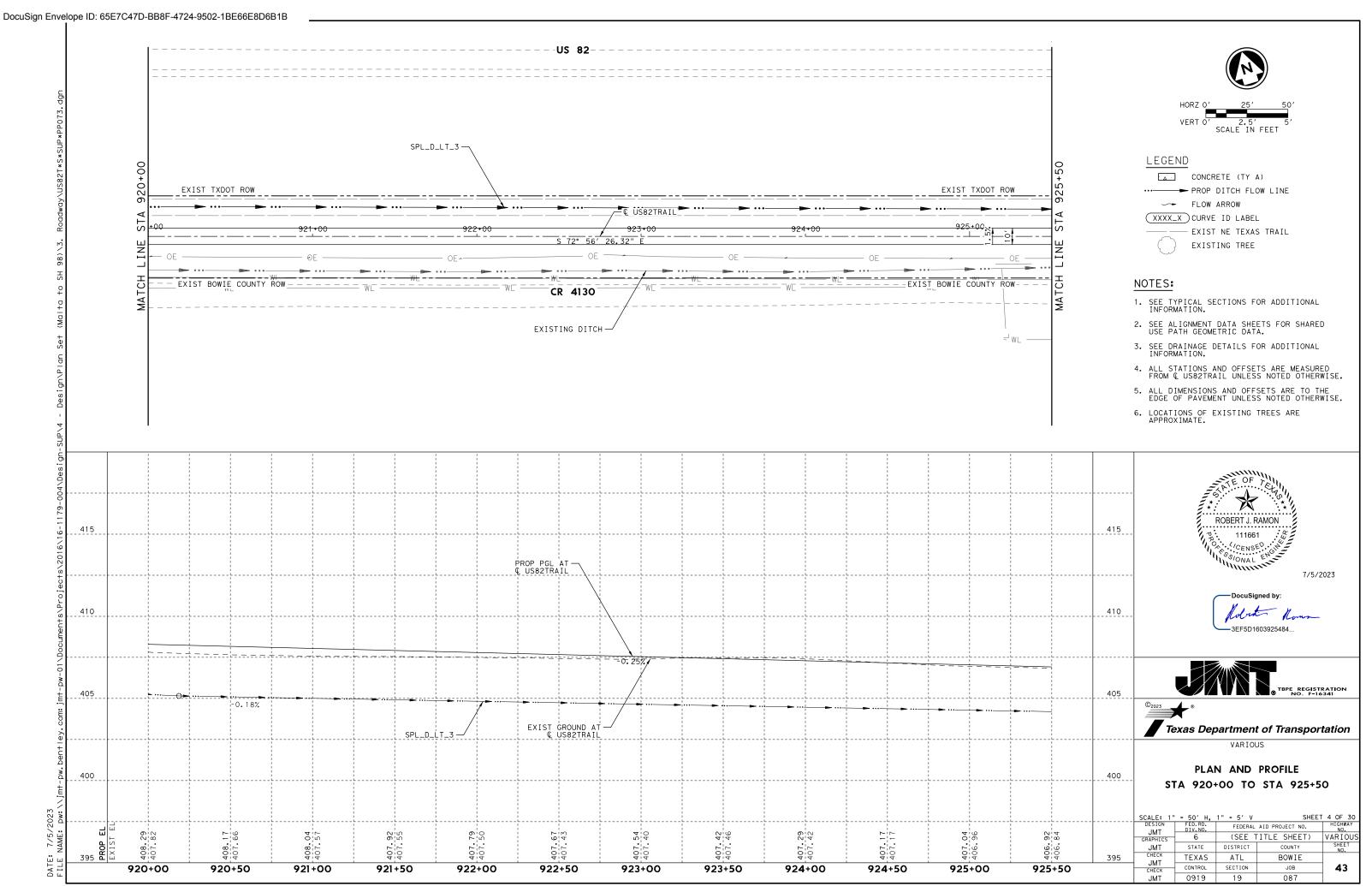


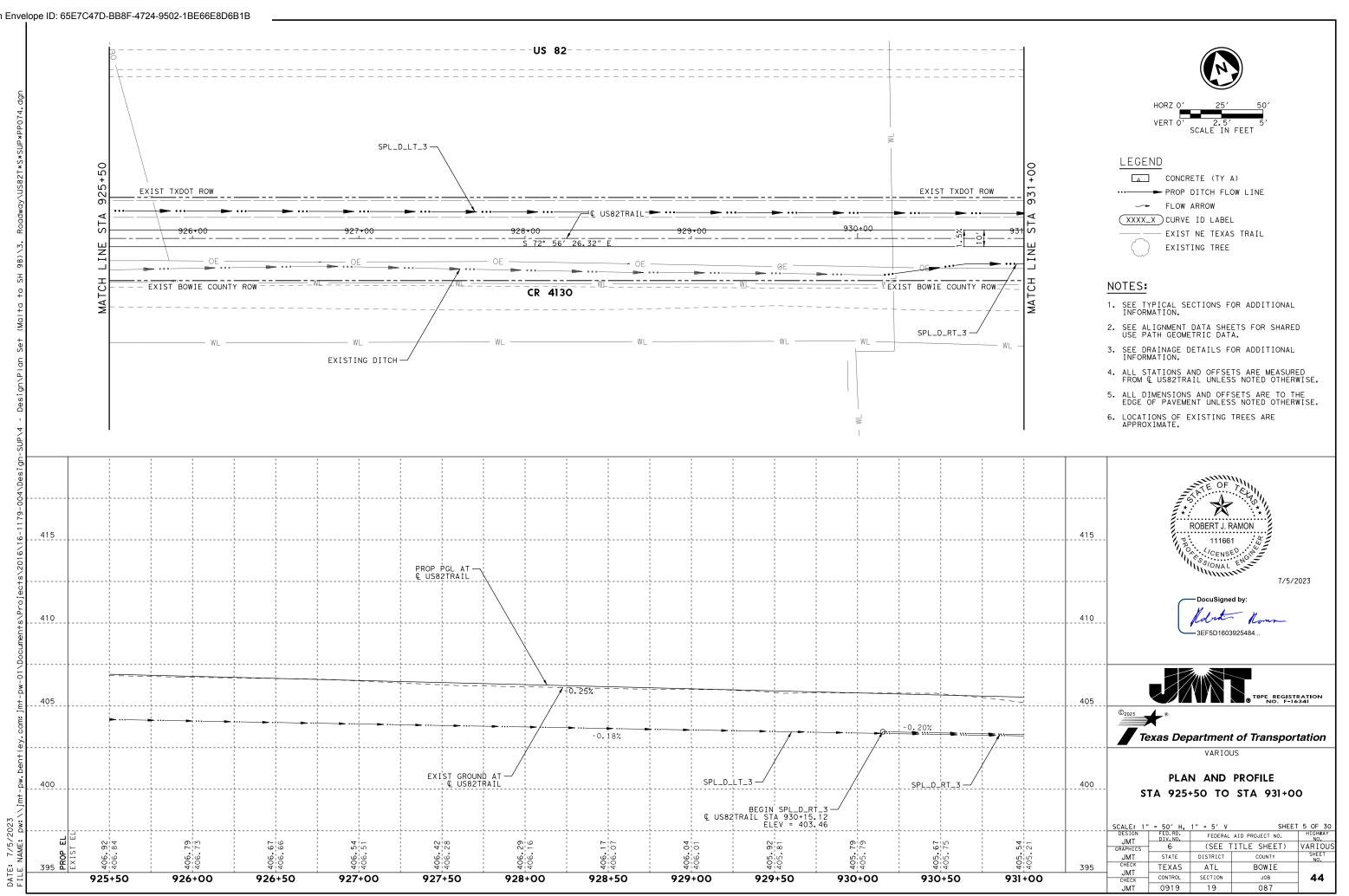


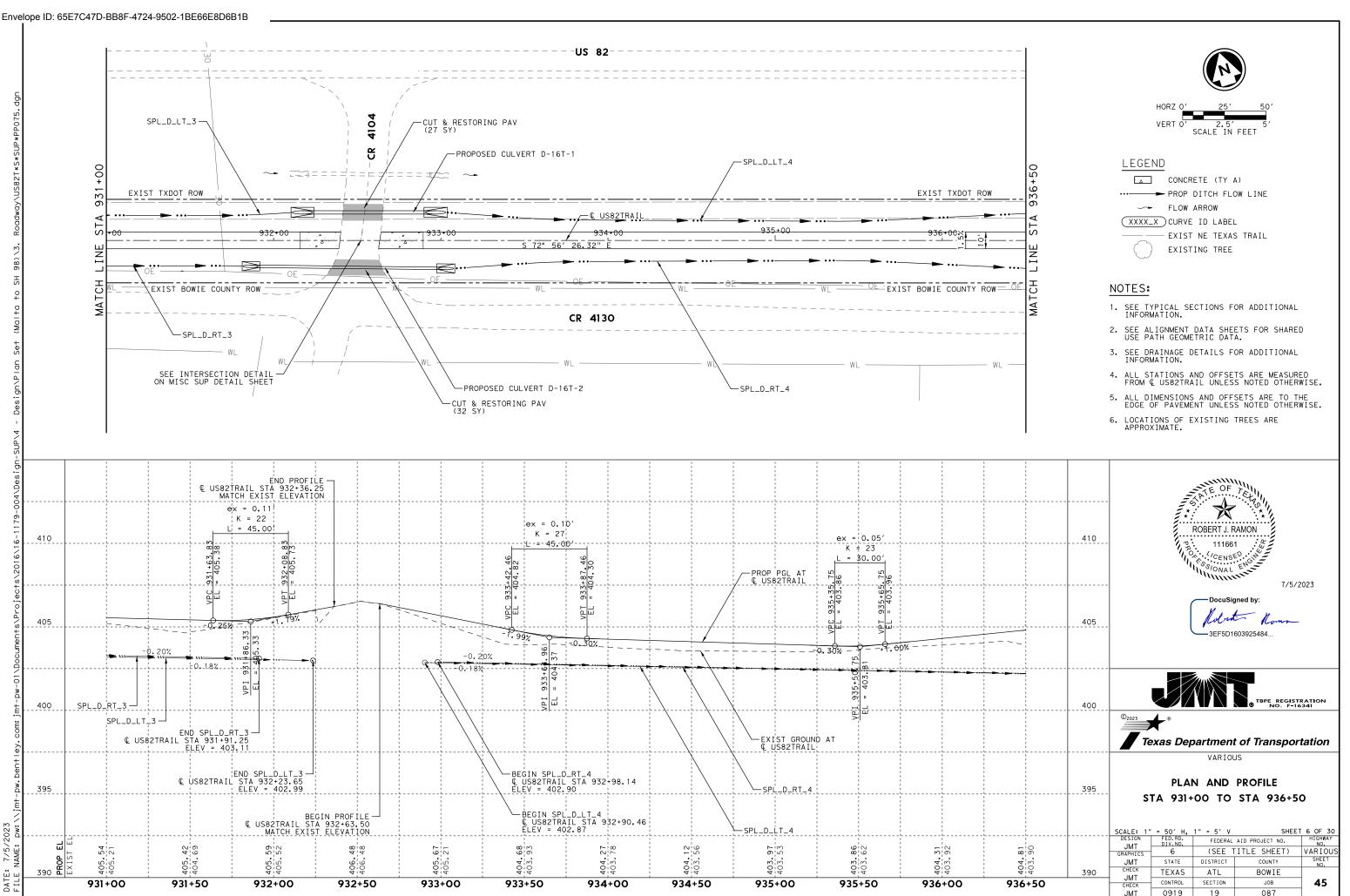




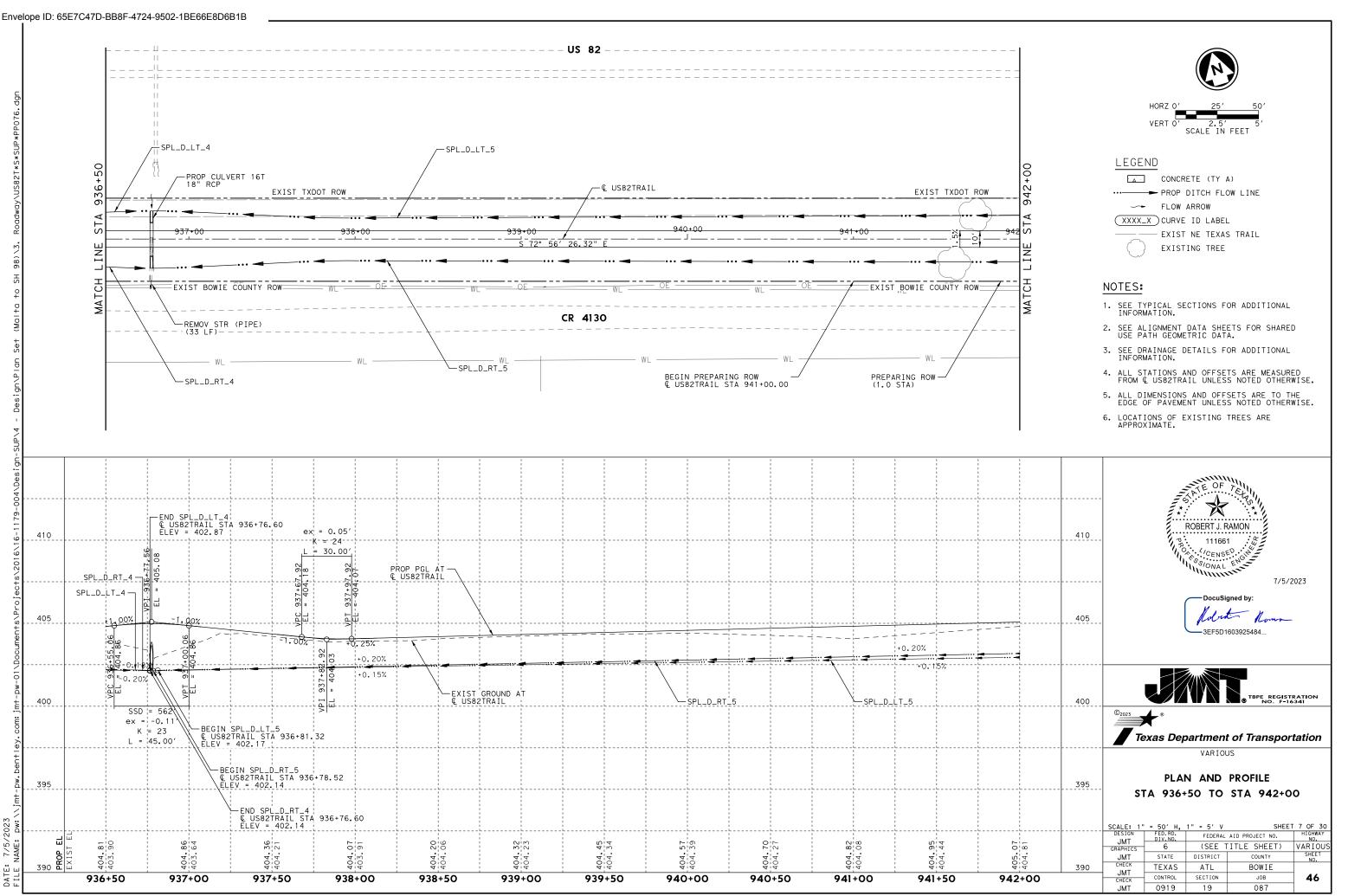
1		SCALE: 1"	= 50′ H,	1" = 5' V	SHEET	3 OF 30
1	[	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO,
82'30		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
.2.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
40	395	снеск ЈМТ	TEXAS	ATL	BOWIE	
920+00		CHECK	CONTROL	SECTION	JOB	42
		JMT	0919	19	087	

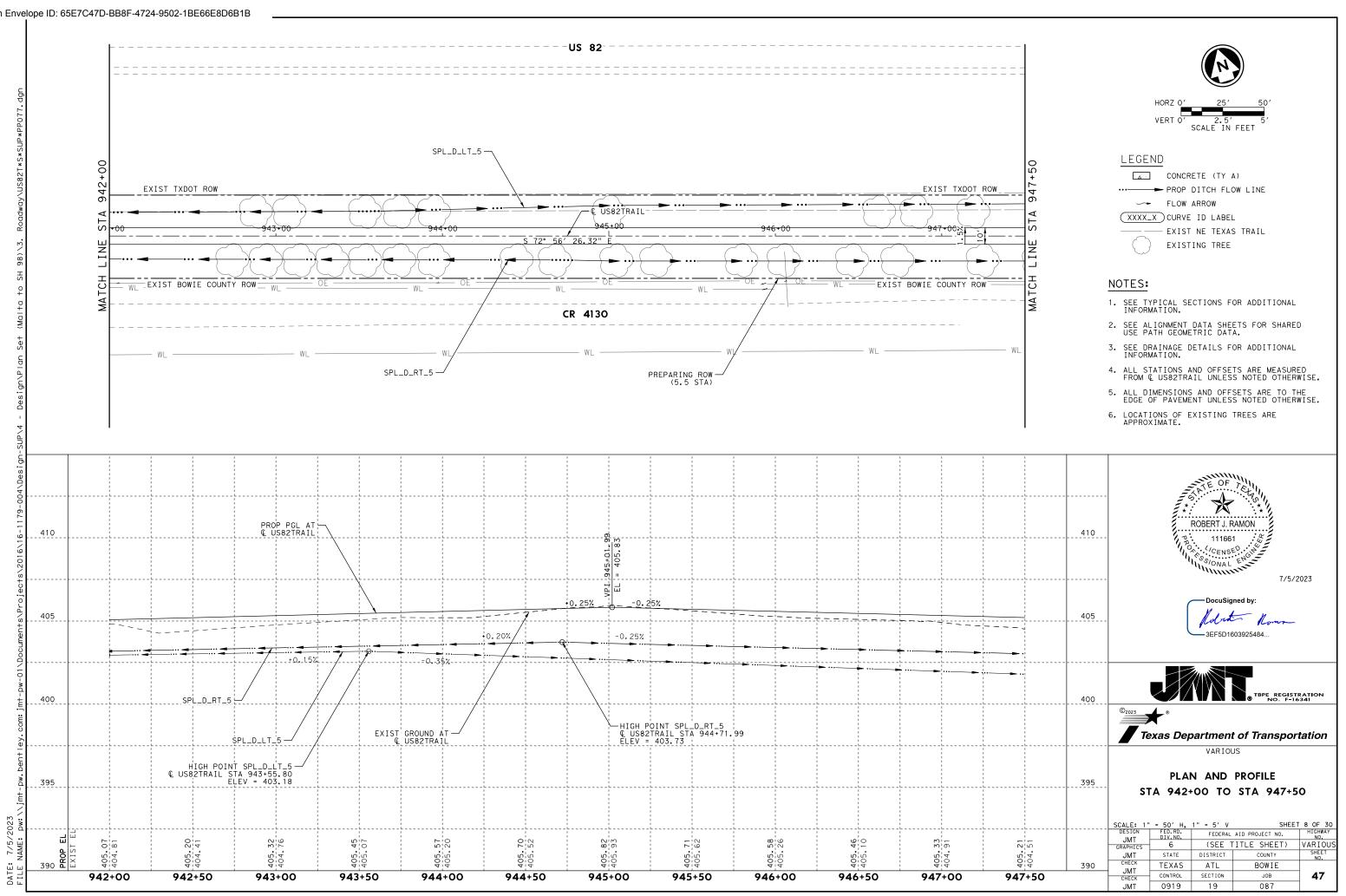


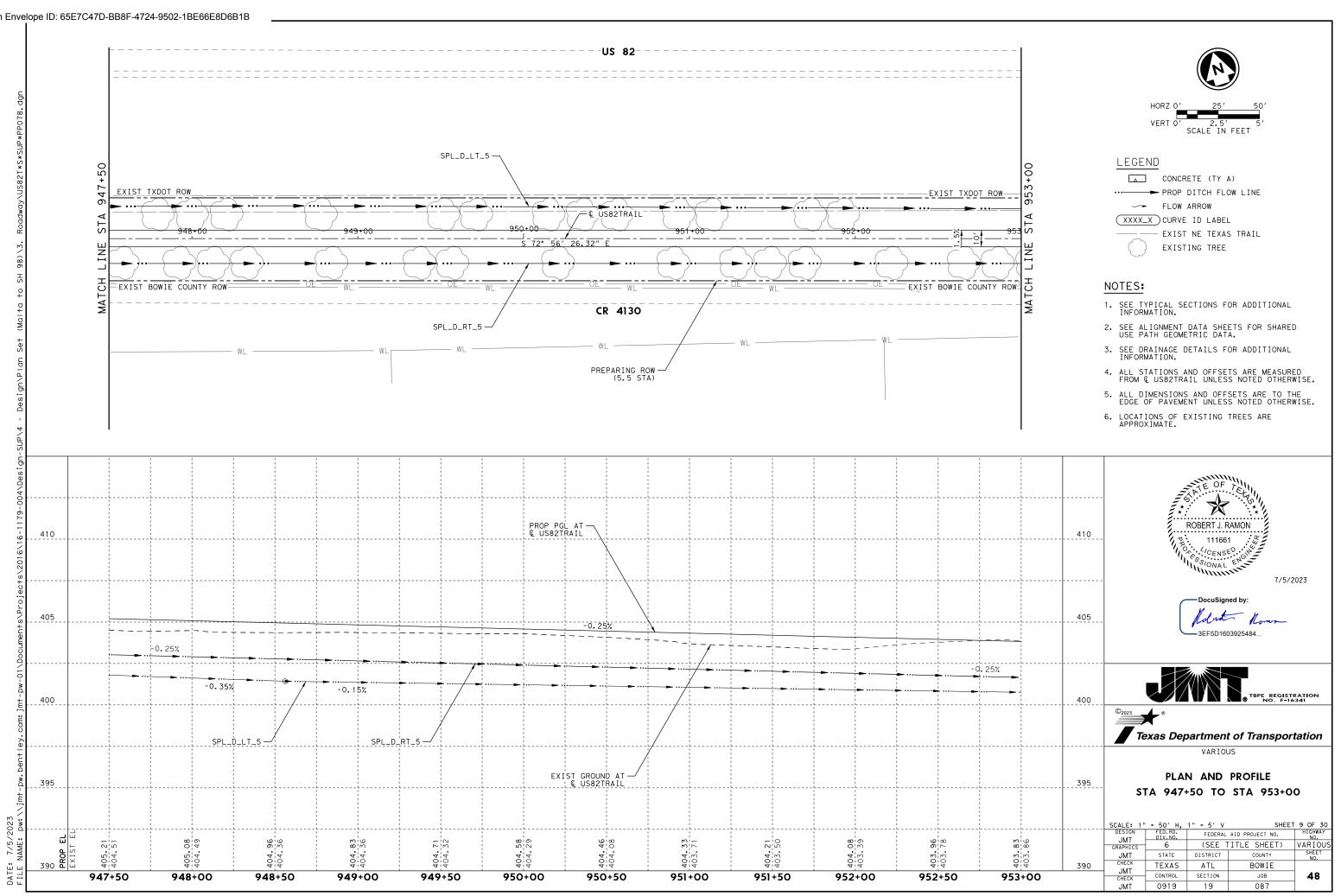




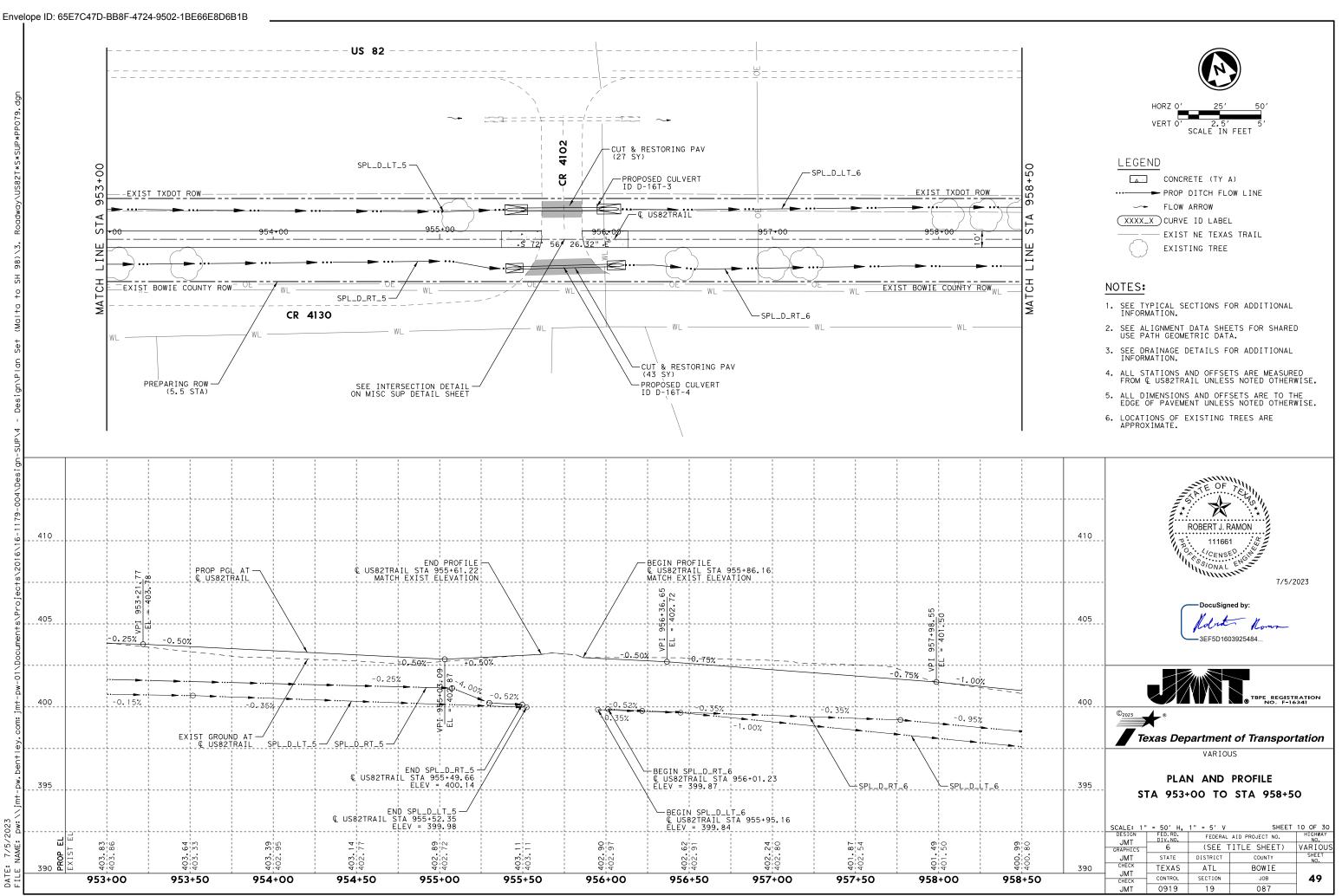
		SCALE: 1"		1" = 5' V	SHEET	6 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
90		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
<u>-</u>		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
40	390	снеск ЈМТ	TEXAS	ATL	BOWIE	
936+50		CHECK	CONTROL	SECTION	JOB	45
		JMT	0919	19	087	



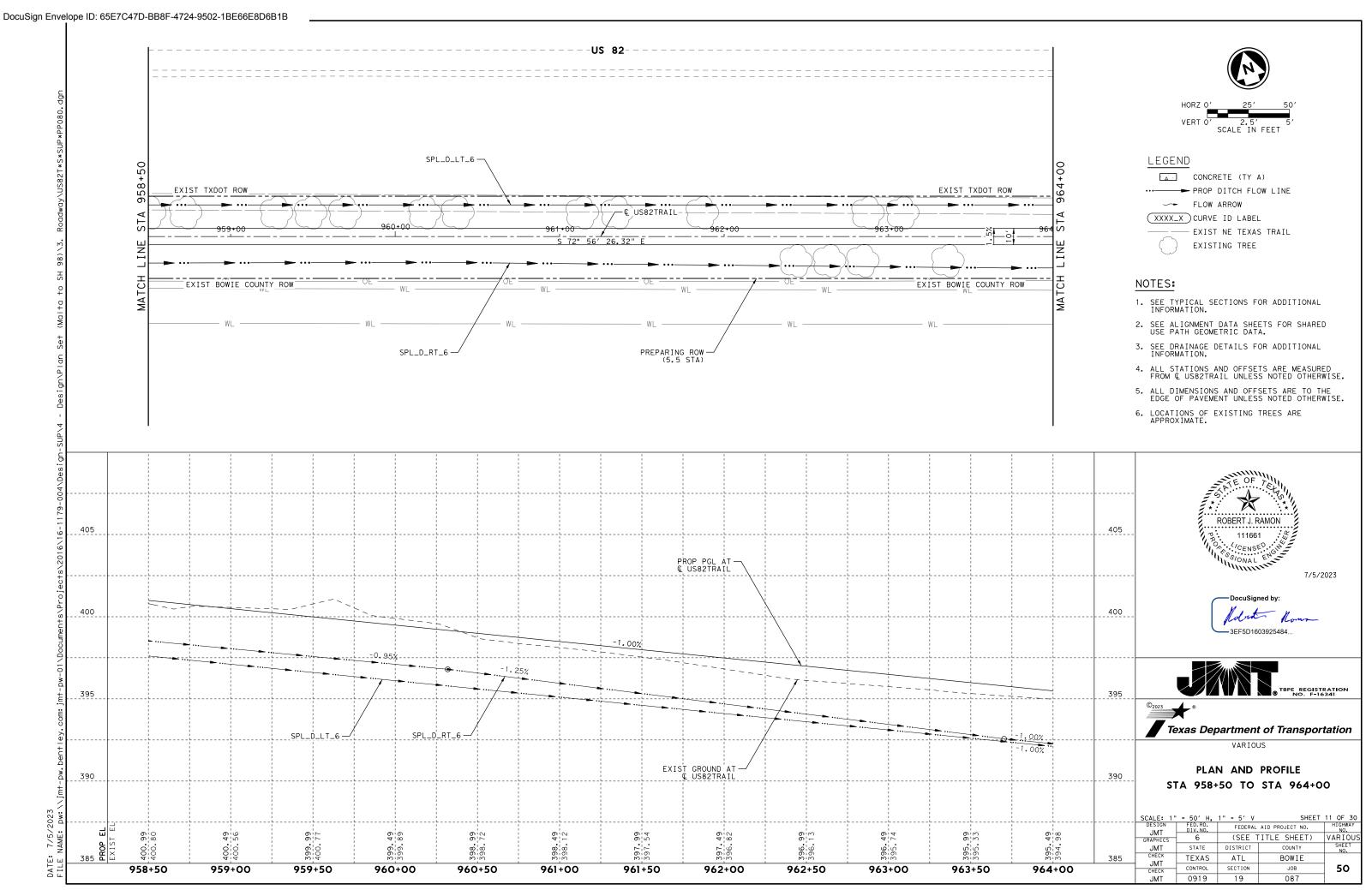




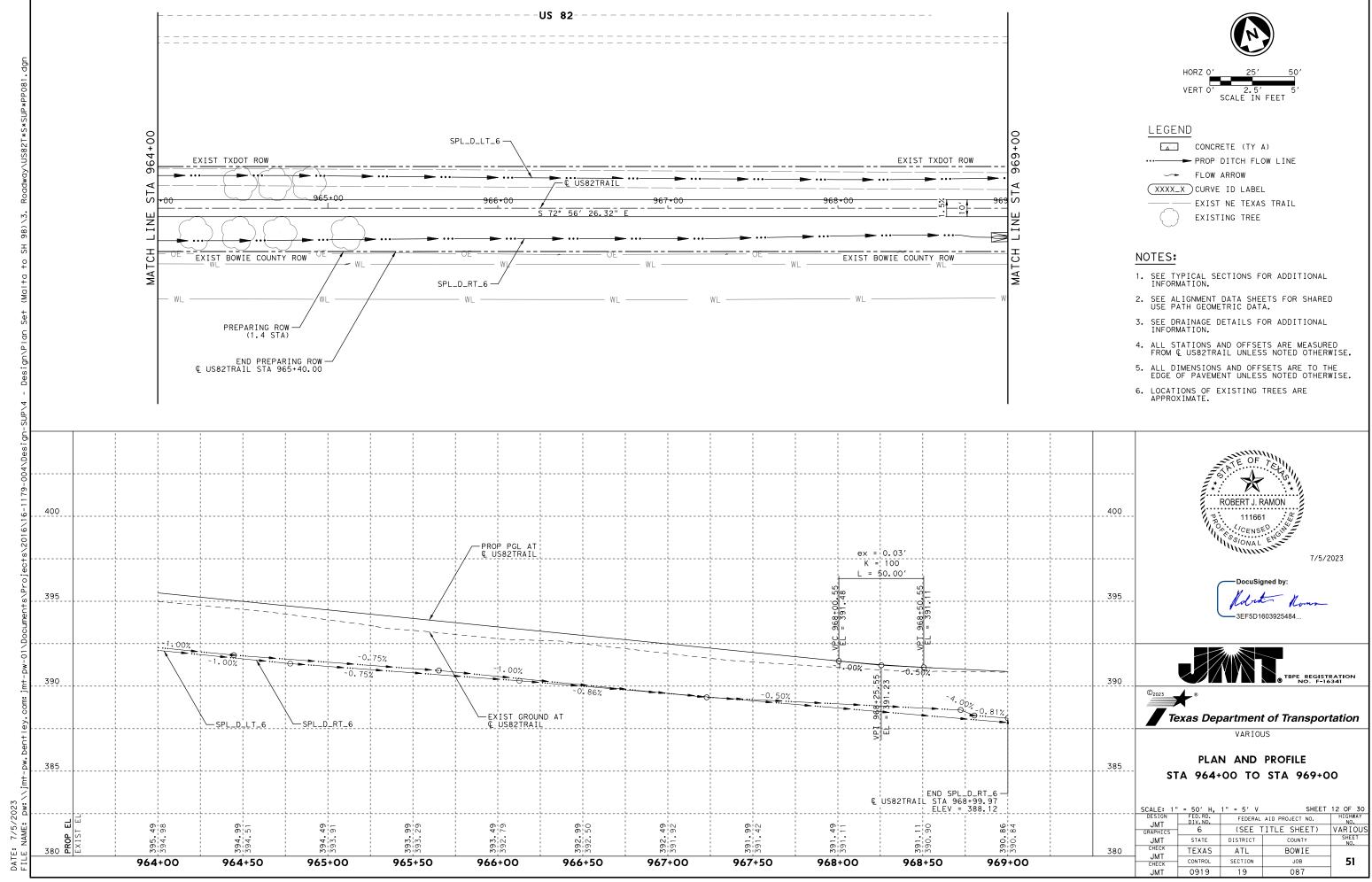
			= 50′ H,	1" = 5' V	SHEET	9 OF 30
	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.	
86 86 80		GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
03.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
404	390	снеск ЈМТ	TEXAS	ATL	BOWIE	
953+00		CHECK	CONTROL	SECTION	JOB	48
		JMT	0919	19	087	

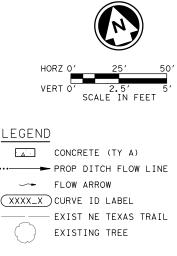


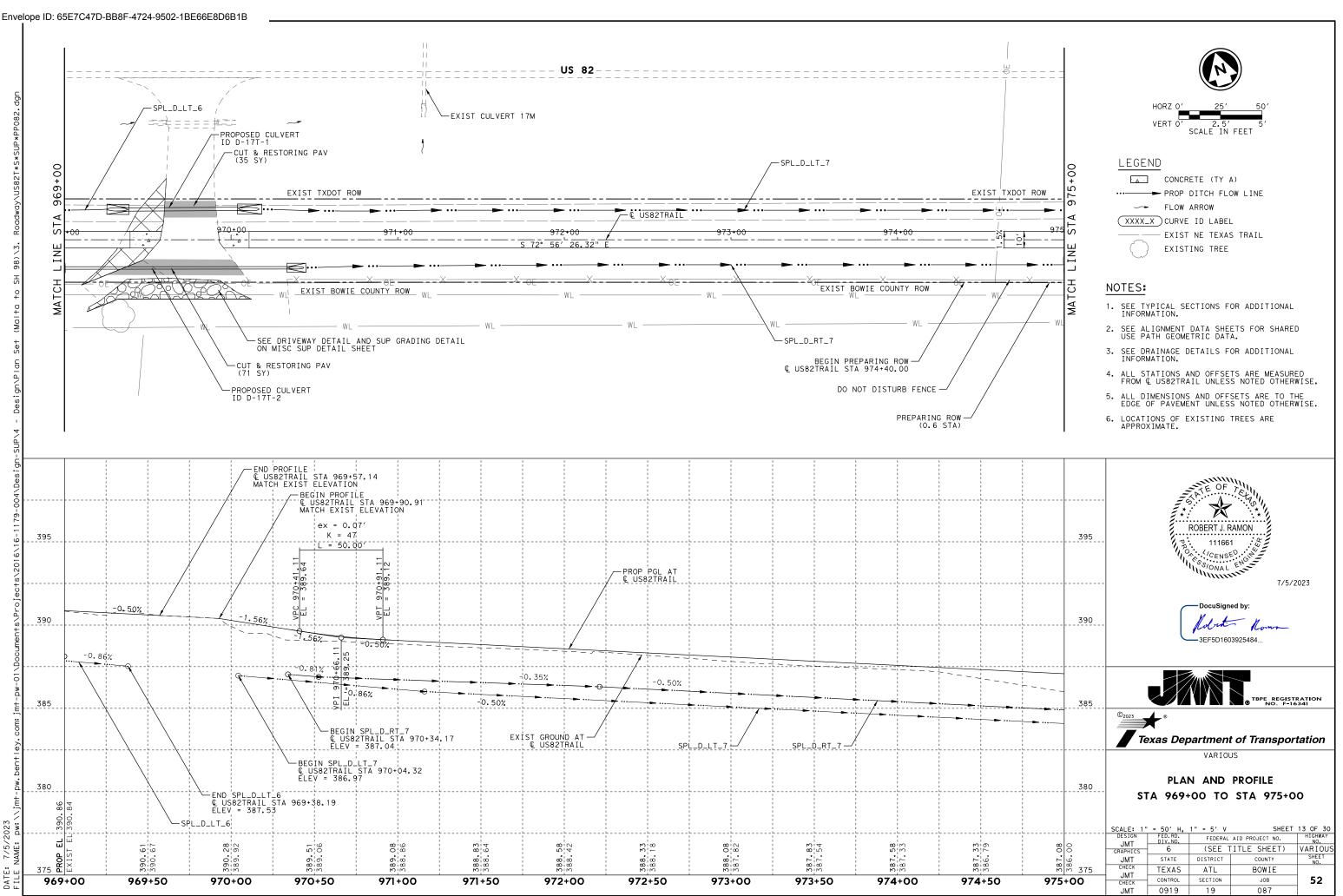
		SCALE: 1"		1" = 5' V	SHEET	10 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
66-08		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
000		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
400	390	CHECK	TEXAS	ATL	BOWIE	
958+50		ЈМТ СНЕСК	CONTROL	SECTION	JOB	49
		JMT	0919	19	087	



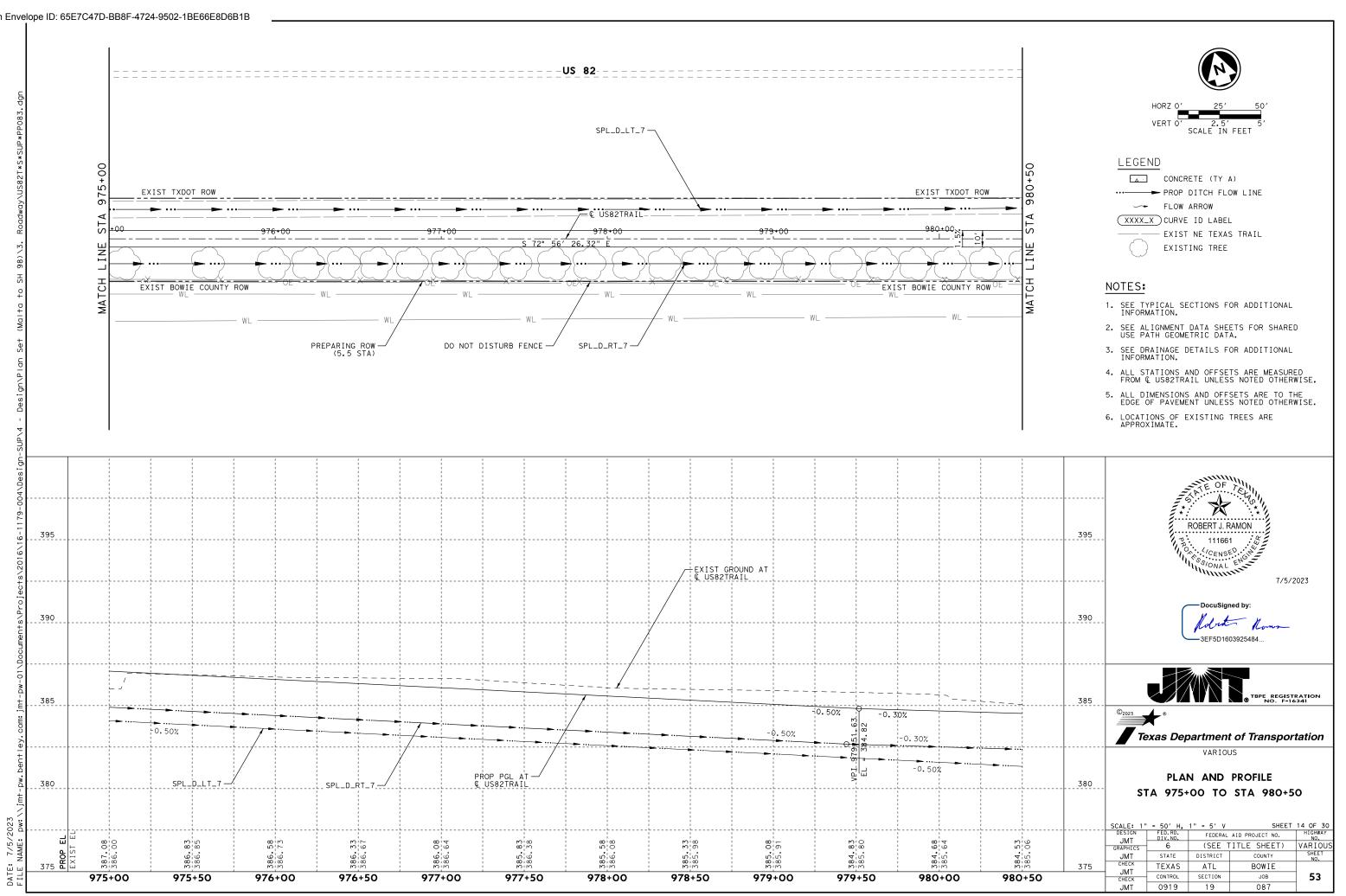
             		SCALE: 1"		1" = 5' V	SHEET	
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
98		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
<u>5</u> .		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
30.0	385	снеск ЈМТ	TEXAS	ATL	BOWIE	
964+00		CHECK	CONTROL	SECTION	JOB	50
		JMT	0919	19	087	

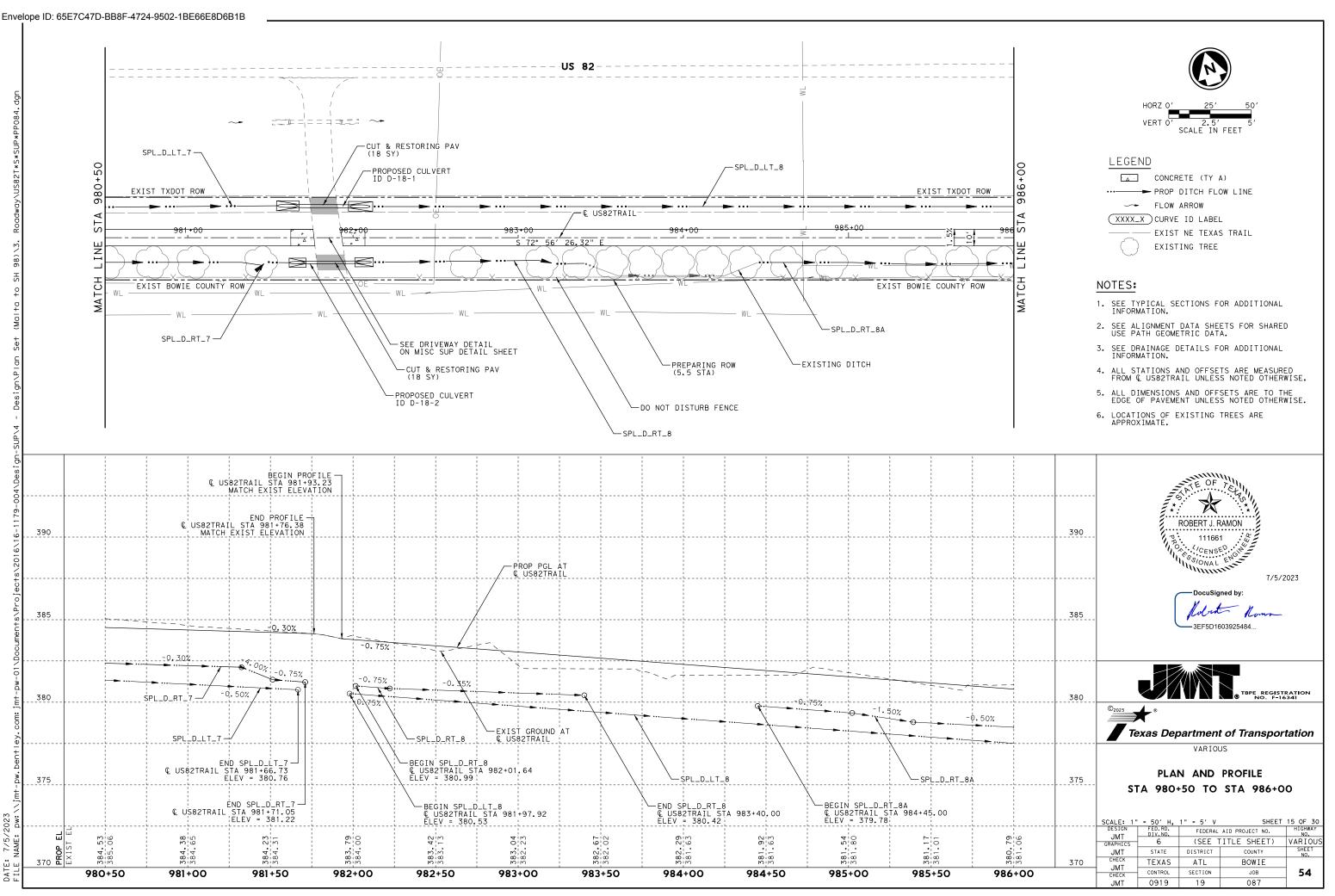




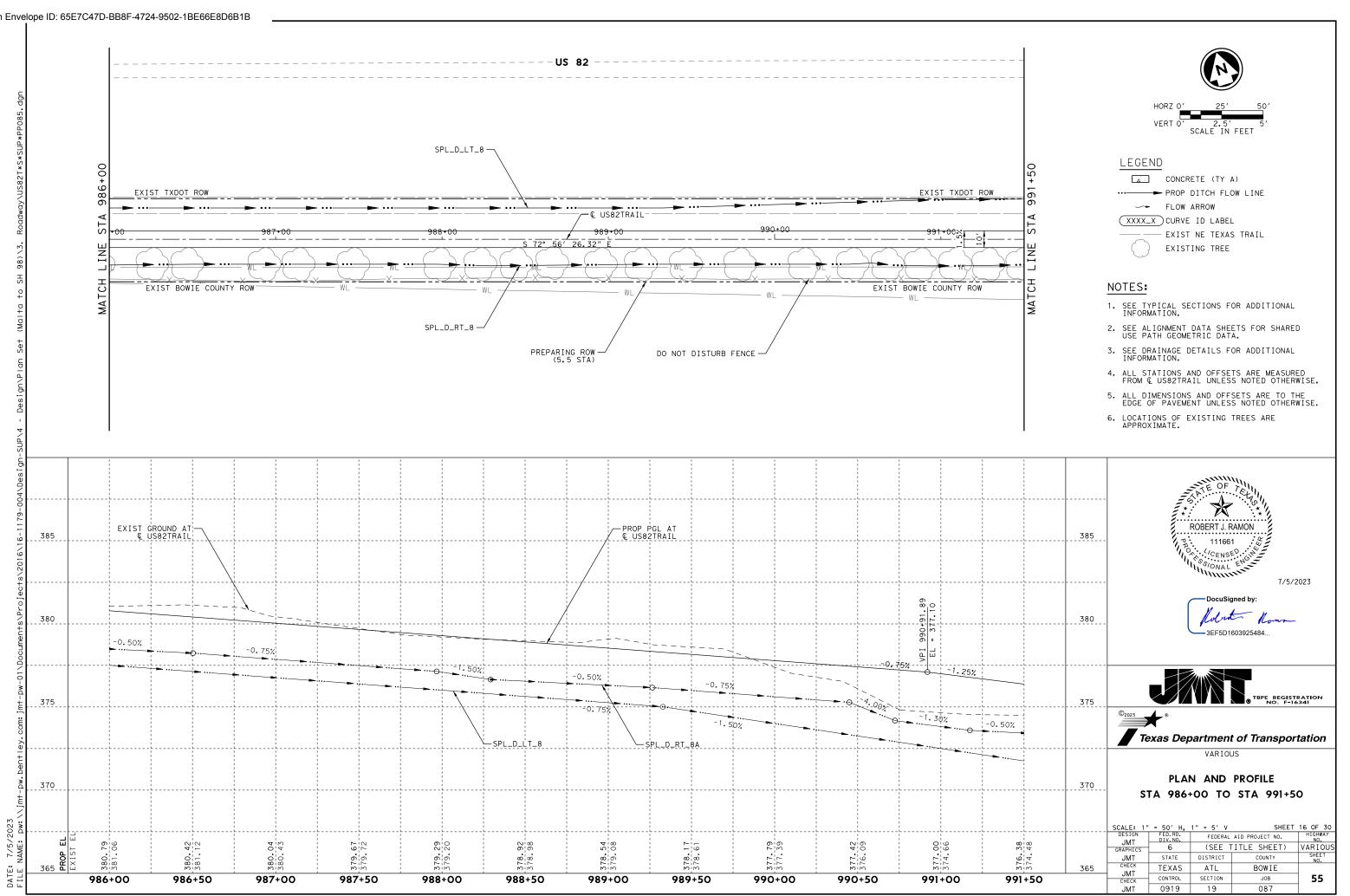


		SCALE: 1"		1" = 5' V	SHEET	13 OF 30
		JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
20	00	GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
80.	87.	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
n m	- m m 375	снеск ЈМТ	TEXAS	ATL	BOWIE	
4+50	975+00	CHECK	CONTROL	SECTION	JOB	52
		JMT	0919	19	087	

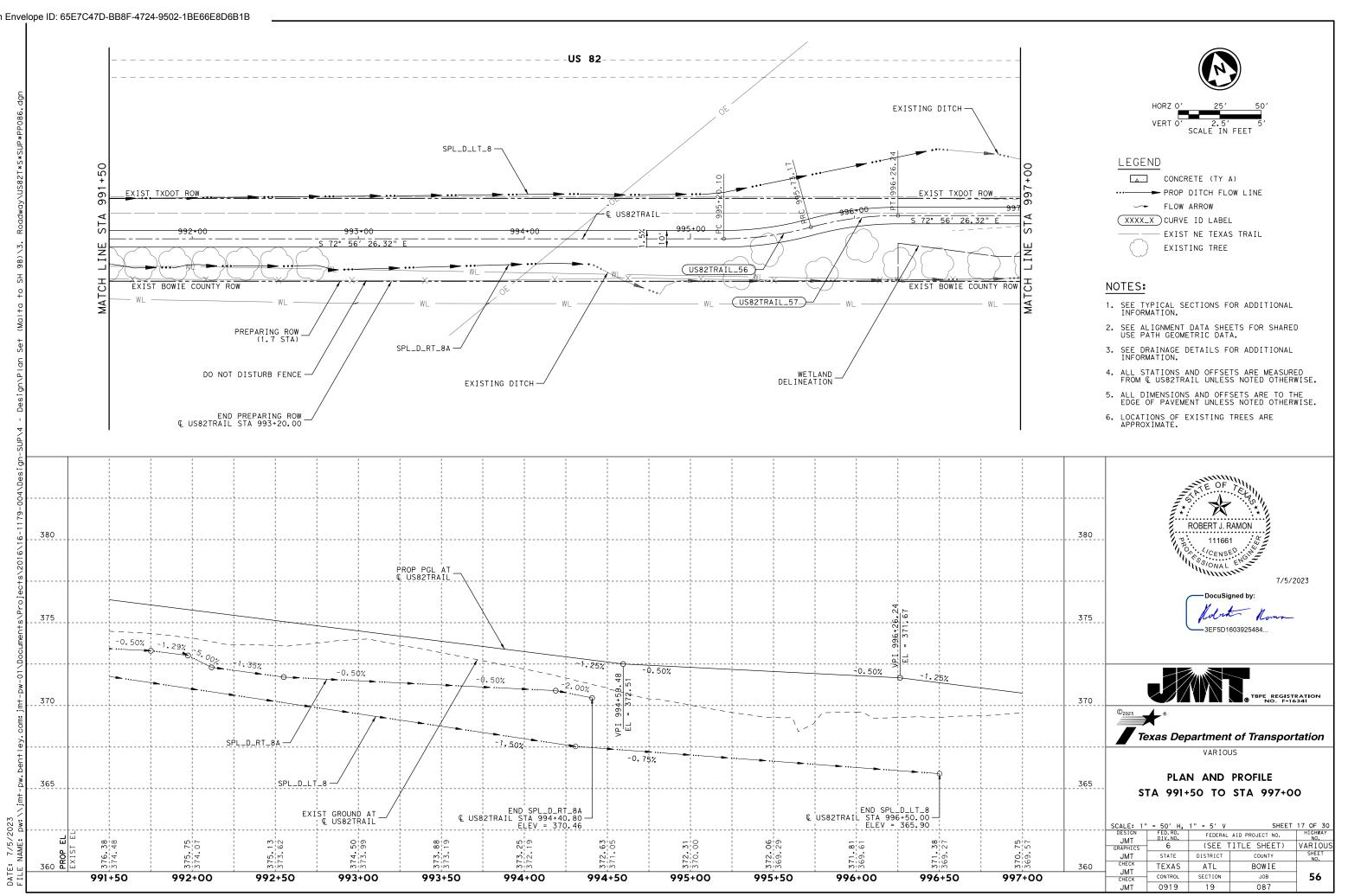




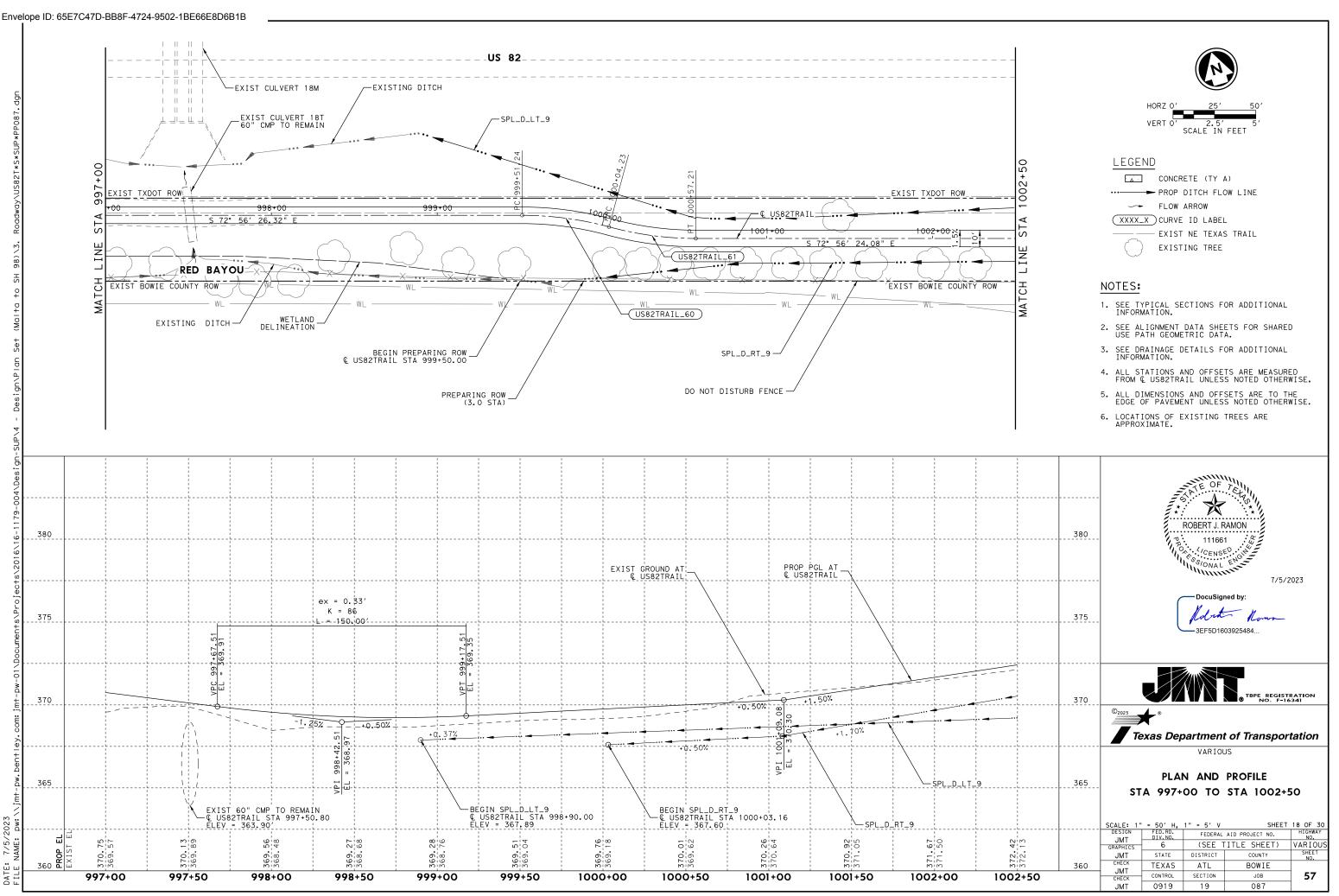
		SCALE: 1" DESIGN	= 50' H, FED.RD. DIV.NO.	1" = 5' V FEDERAL	SHEET AID PROJECT NO.	15 OF 30 HIGHWAY NO.
1.06		JMT graphics JMT	6 STATE	(SEE T DISTRICT	ITLE SHEET) COUNTY	VARIOUS
1 00100 1 10110	370	снеск ЈМТ	TEXAS	ATL	BOWIE	
986+00		CHECK	CONTROL	SECTION	JOB	54
		JMT	0919	19	087	



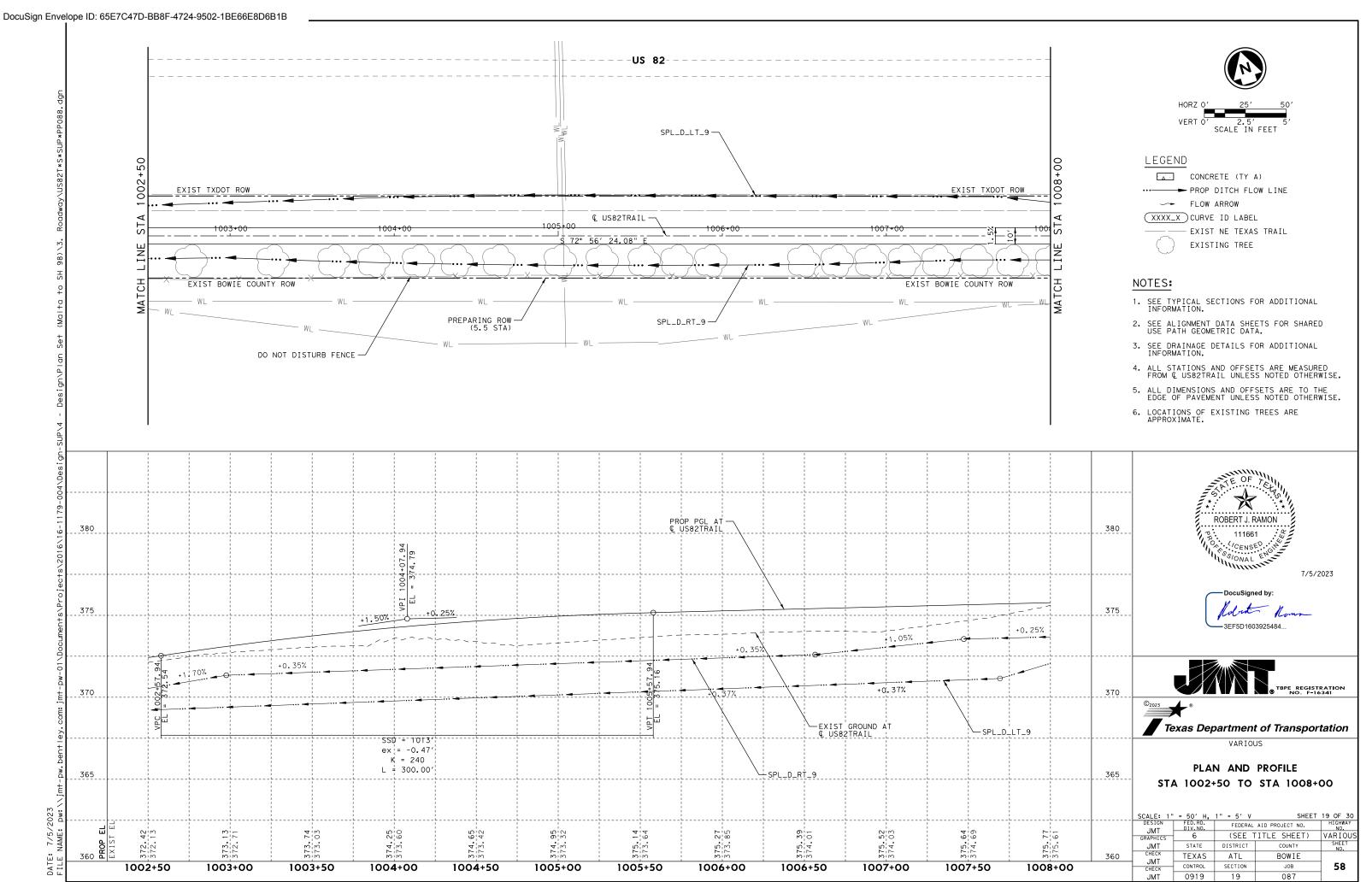
   				1" = 5' V	SHEET	
1		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
40.38		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
74.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
n'n	365	снеск ЈМТ	TEXAS	ATL	BOWIE	
991+50		CHECK	CONTROL	SECTION	JOB	55
		JMT	0919	19	087	



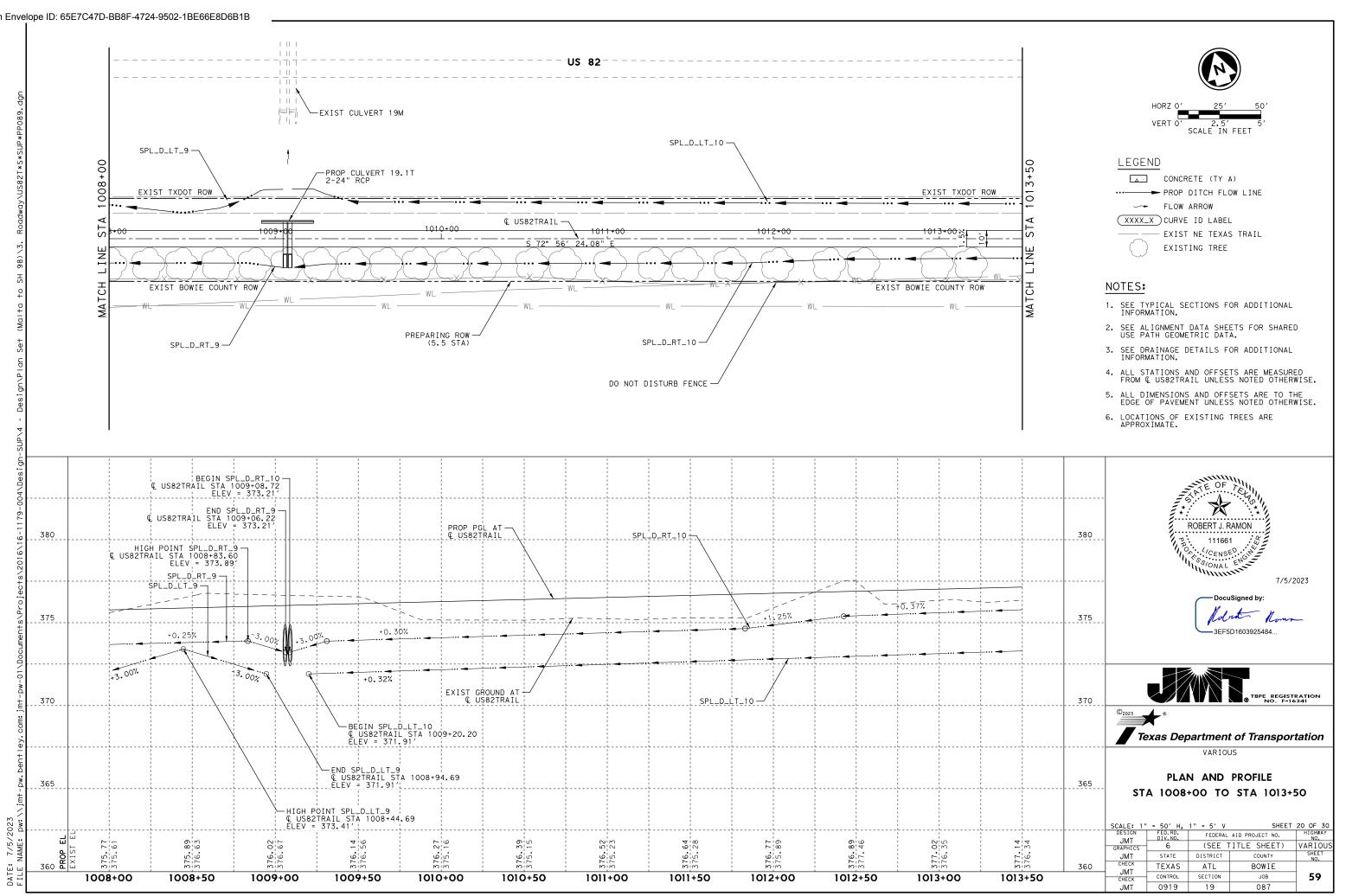
997+00		JMT CHECK JMT	CONTROL 0919	SECTION	јов 087	56
36	360	CHECK	TEXAS	ATL	BOWIE	
0 6		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
75		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
		SCALE: 1"		1" = 5' V	SHEET	
i i						

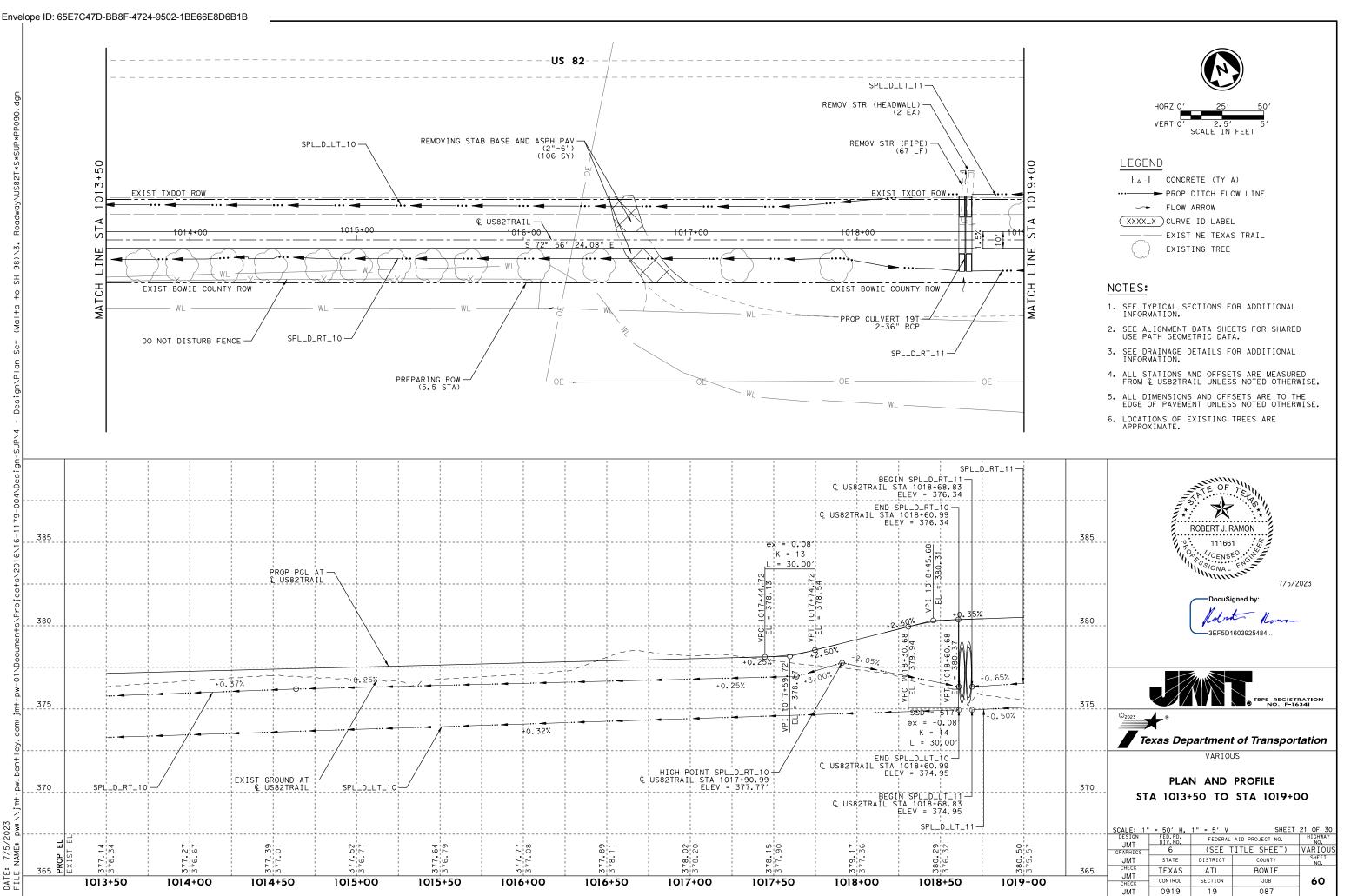


				1" = 5' V	SHEET	18 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.
4-1-22		GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
72.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
M M	360	CHECK	TEXAS	ATL	BOWIE	
1002+50		ЈМТ СНЕСК	CONTROL	SECTION	JOB	57
		JMT	0919	19	087	

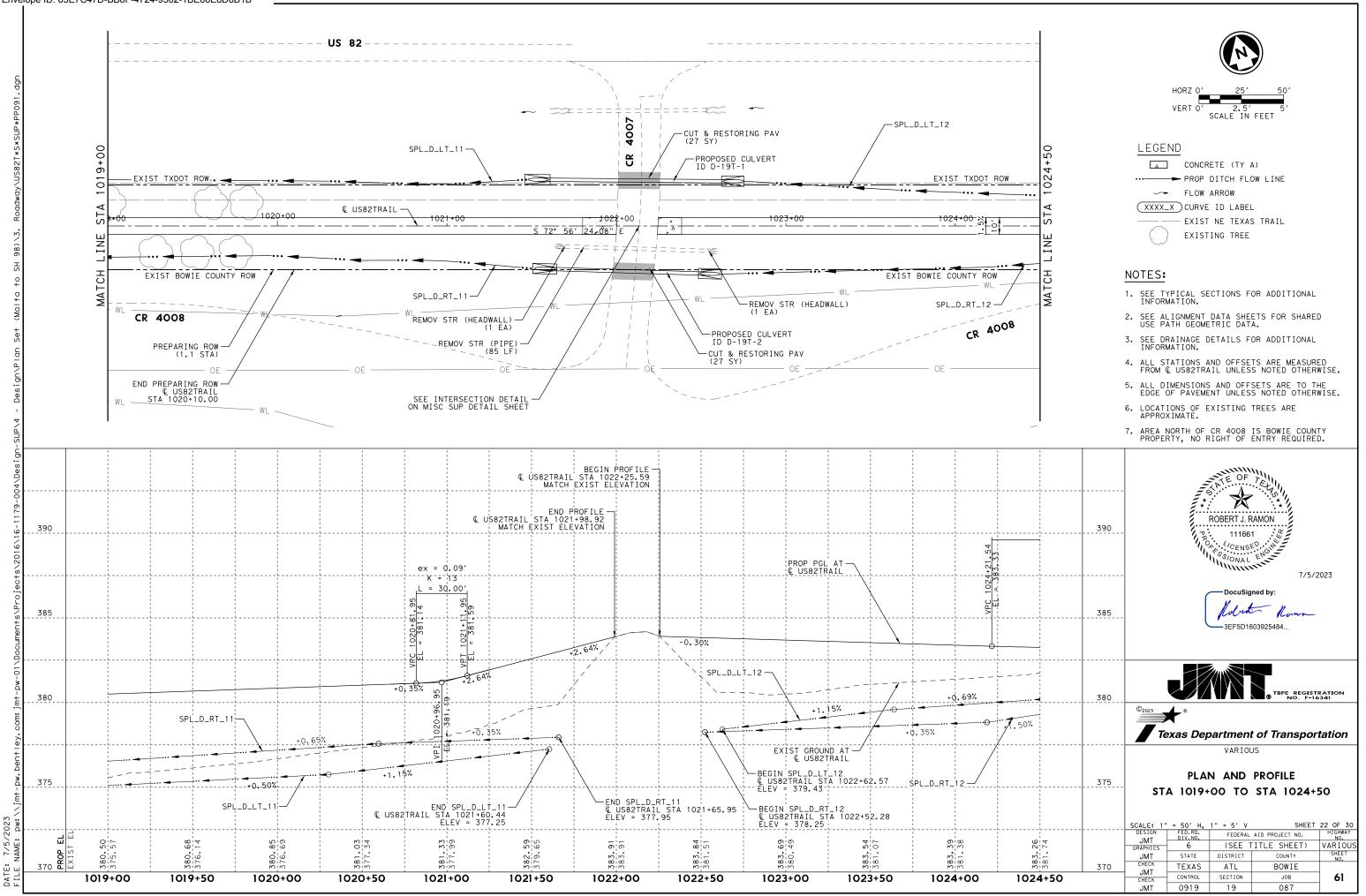


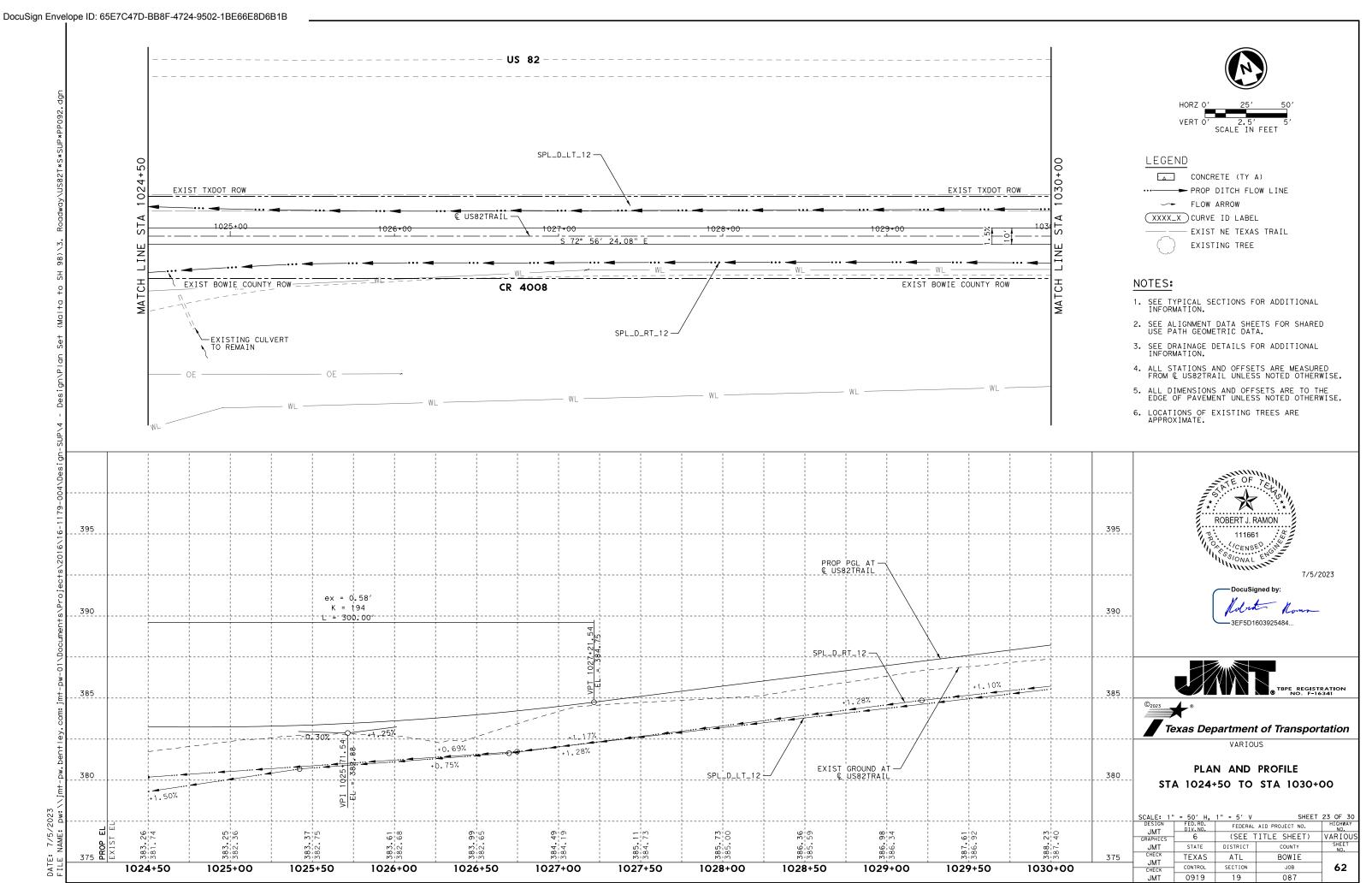
			= 50′ H,	1" = 5' V	SHEET	19 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO,
61		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
75.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
mm	360	снеск ЈМТ	TEXAS	ATL	BOWIE	
1008+00		CHECK	CONTROL	SECTION	JOB	58
		JMT	0919	19	087	



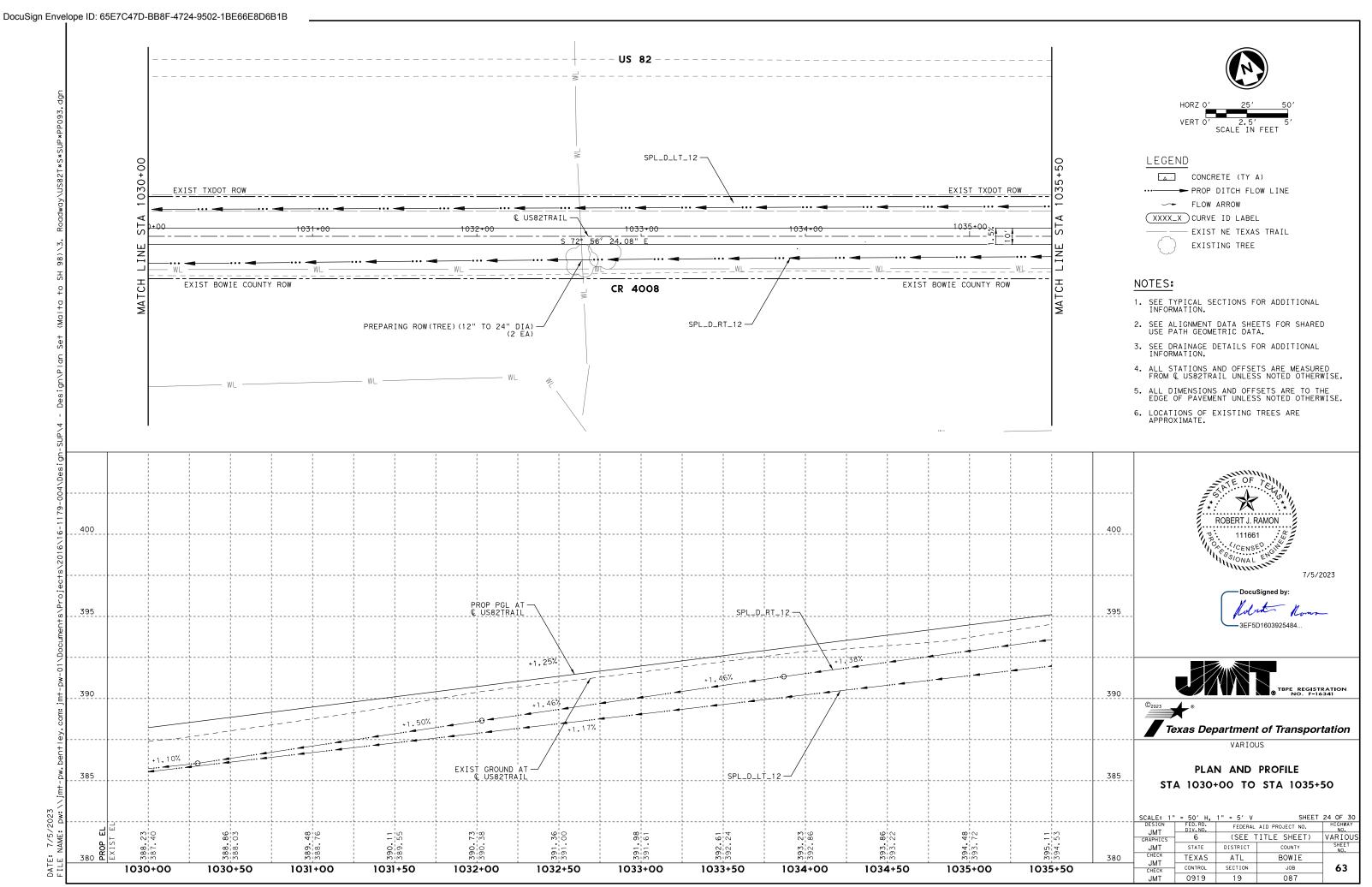


		SCALE: 1"	- 50 11,	1" = 5′ V	SHEET	
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
57		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
80. 75.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
m m	365	снеск ЈМТ	TEXAS	ATL	BOWIE	
1019+00		CHECK	CONTROL	SECTION	JOB	60
		JMT	0919	19	087	

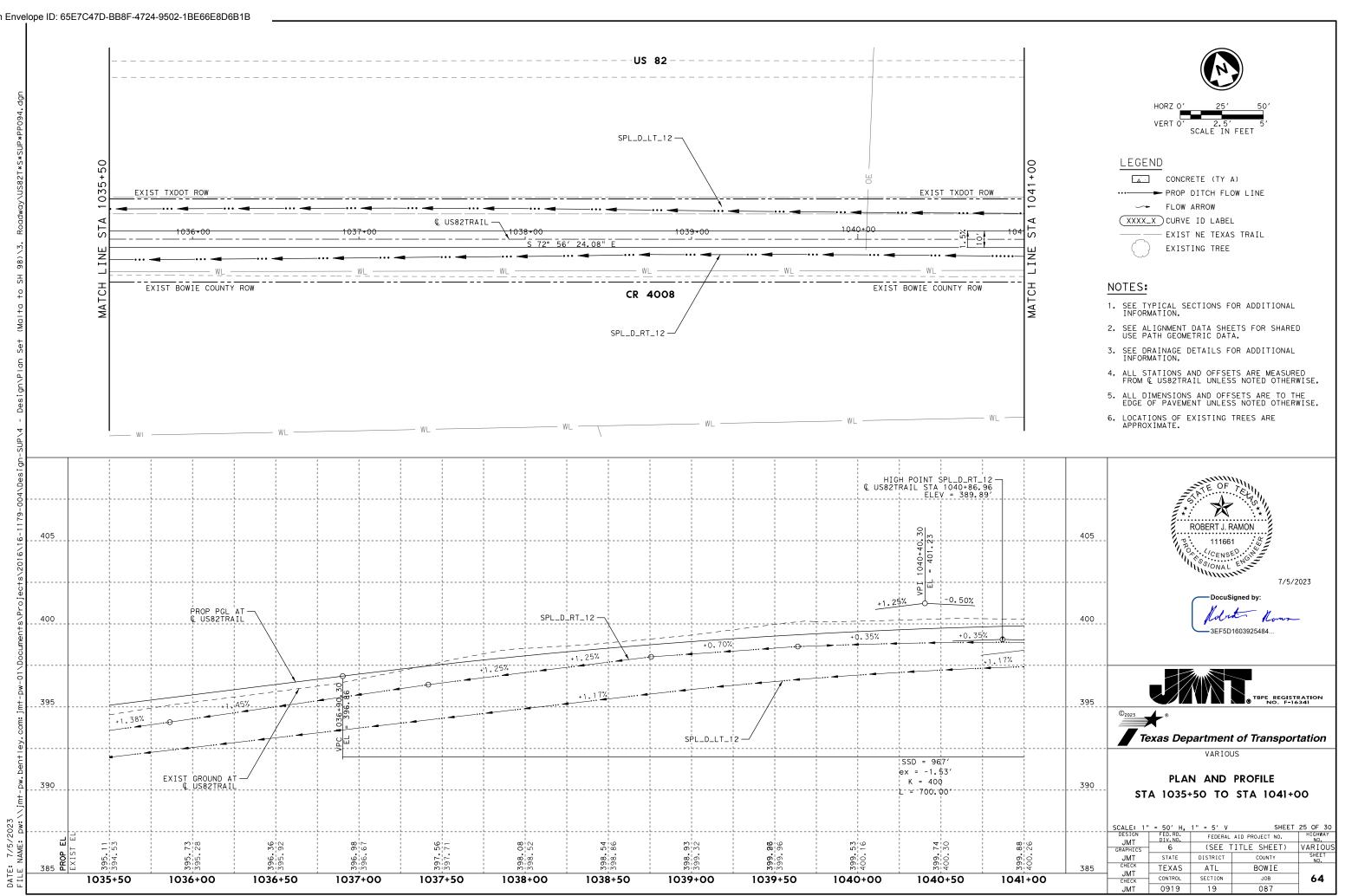


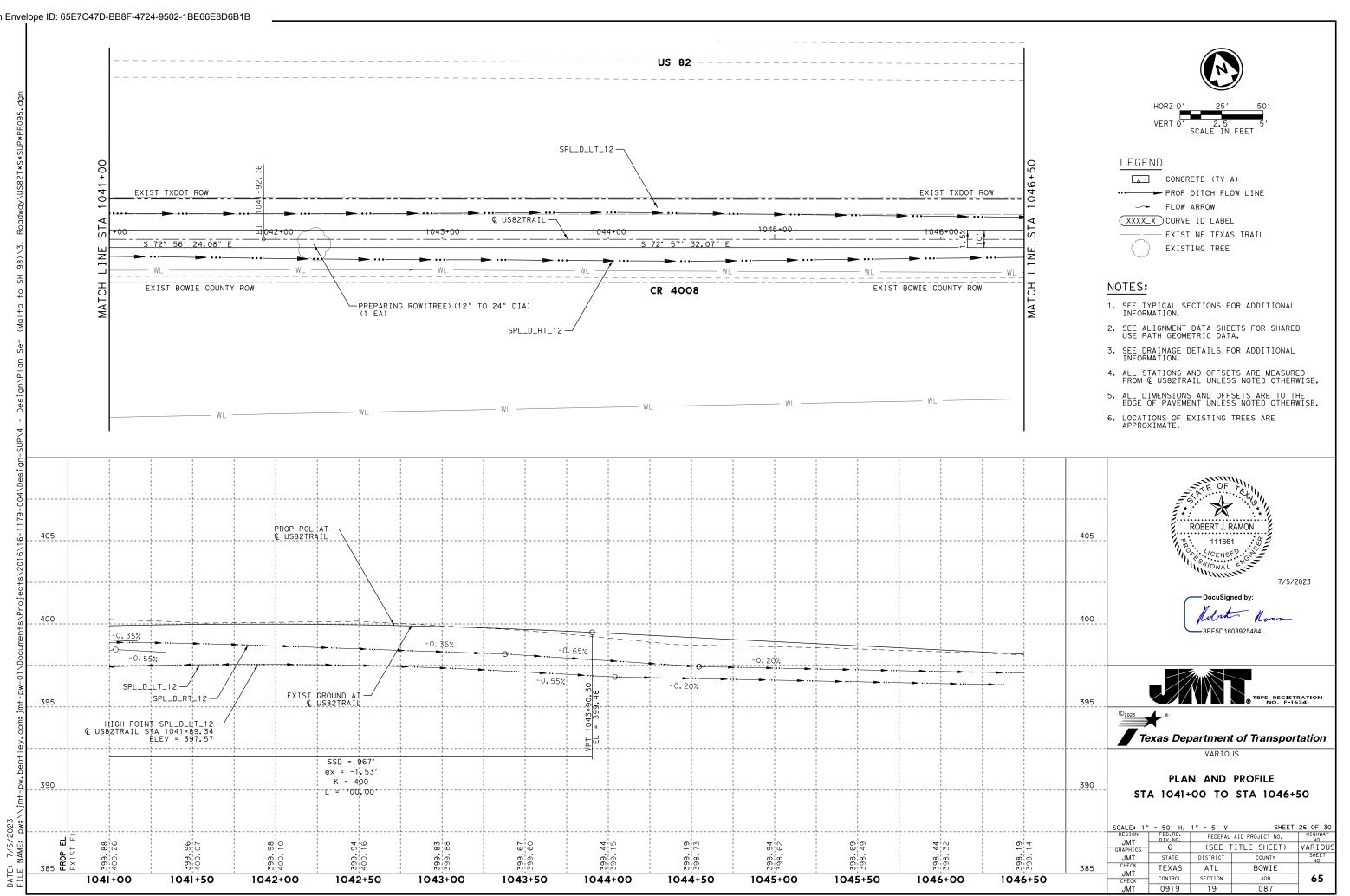


1		SCALE: 1"		1" = 5' V	SHEET	23 OF 30
	[	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
40		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
888.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
m'm	375	снеск ЈМТ	TEXAS	ATL	BOWIE	
1030+00		CHECK	CONTROL	SECTION	JOB	62
		JMT	0919	19	087	

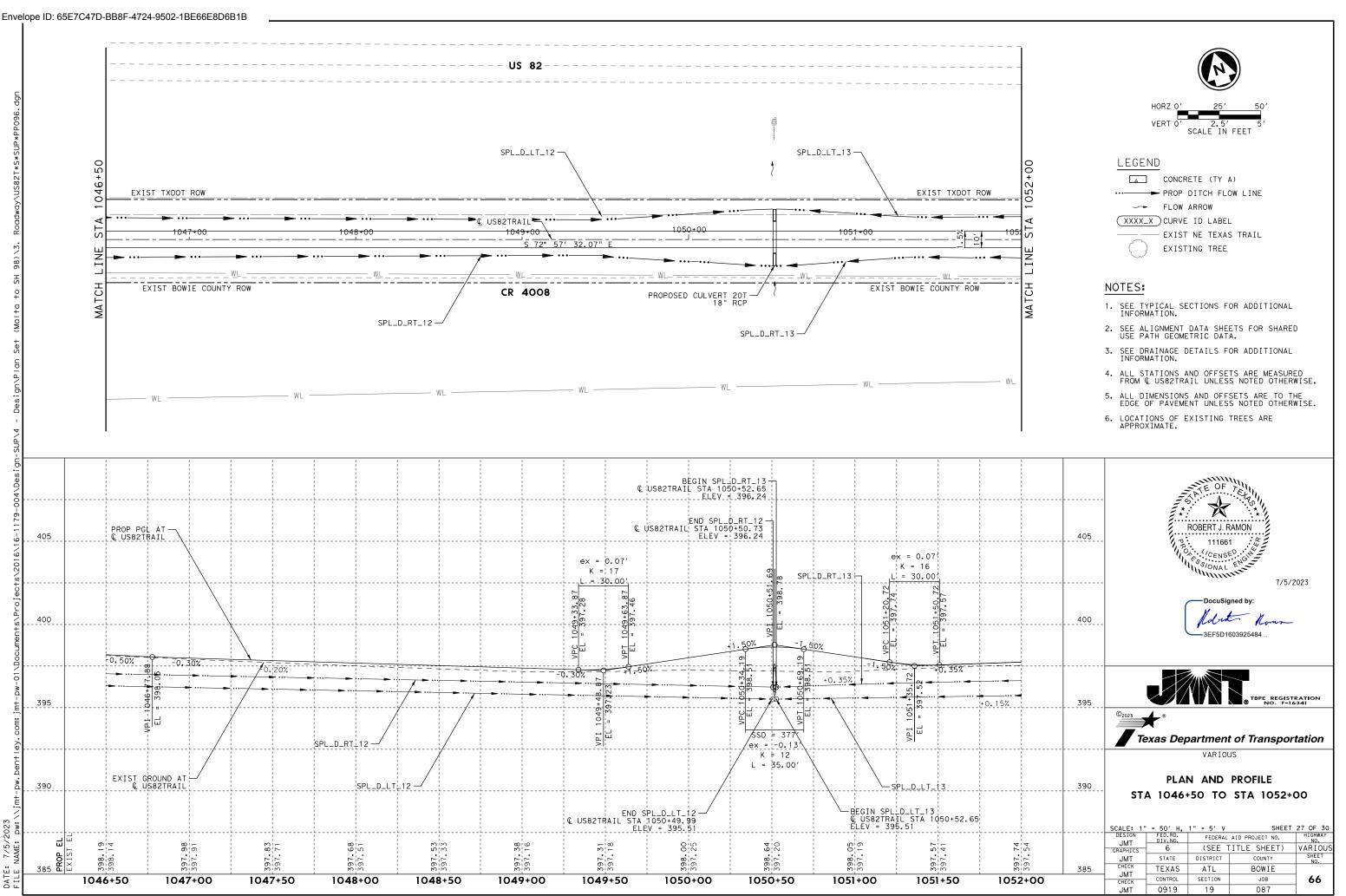


1		SCALE: 1"		1" = 5' V	SHEET	24 OF 30
1		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
231-1		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOL
5- 2- 2-		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
0 0 0	380	снеск ЈМТ	TEXAS	ATL	BOWIE	
1035+50		CHECK	CONTROL	SECTION	JOB	63
		JMT	0919	19	087	]

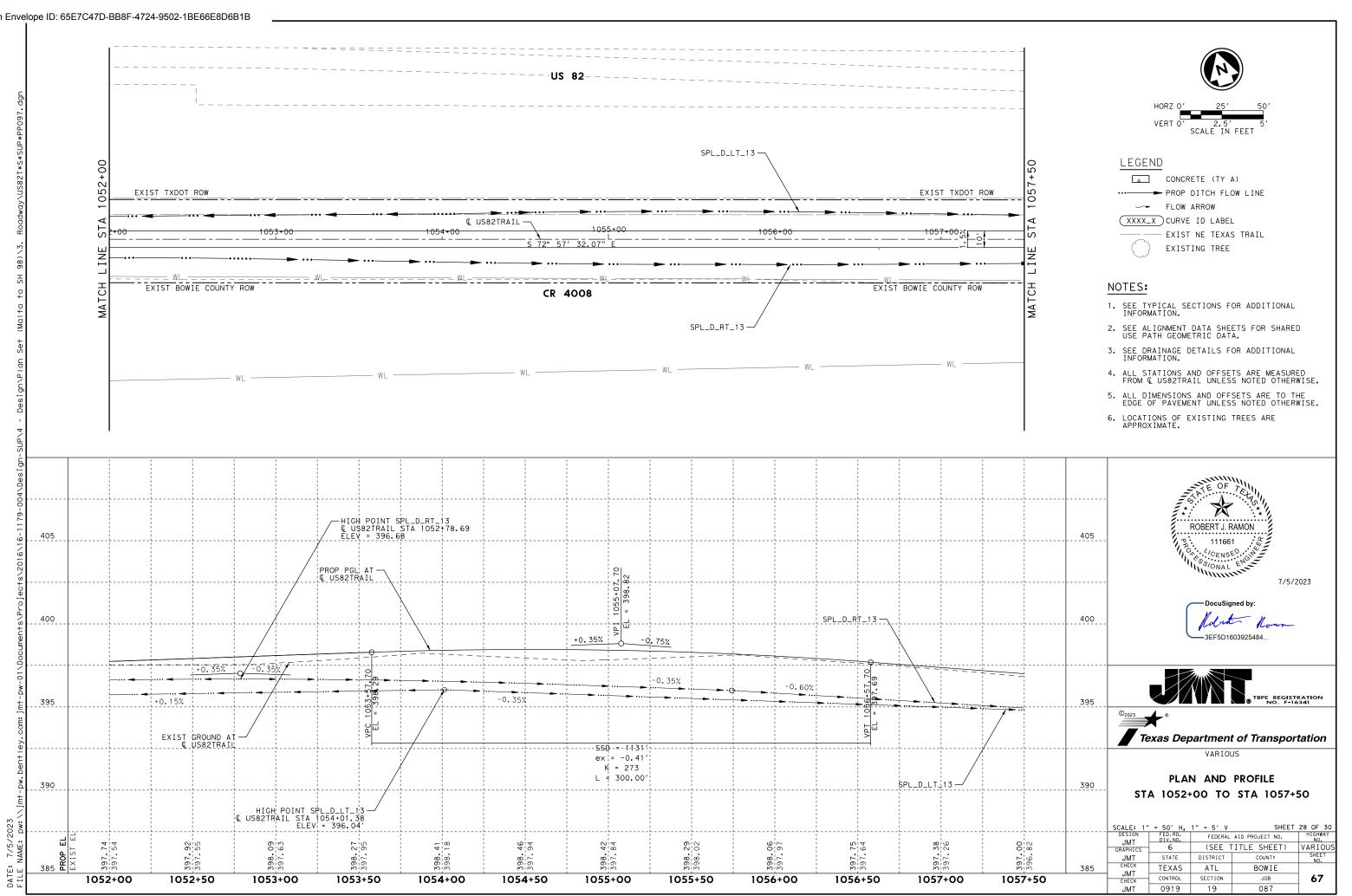




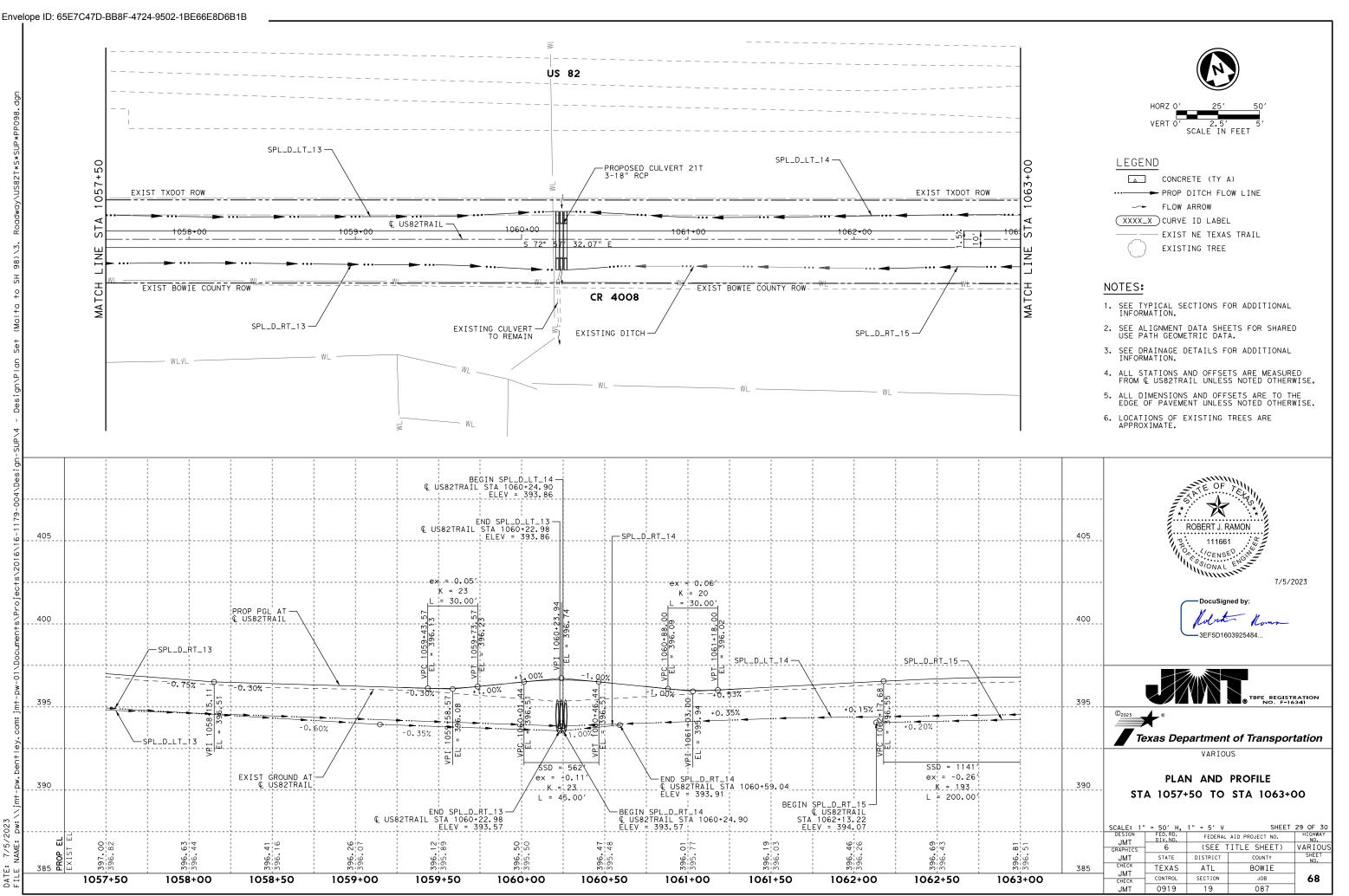
		SCALE: 1"		1" = 5' V	SHEET	26 OF 30
_		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.
19		GRAPHICS	6	(SEE TITLE SHEET)		VARIOUS
8.8		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
0 0 0	385	снеск ЈМТ	TEXAS	ATL	BOWIE	
1046+50		CHECK	CONTROL	SECTION	JOB	65
		JMT	0919	19	087	



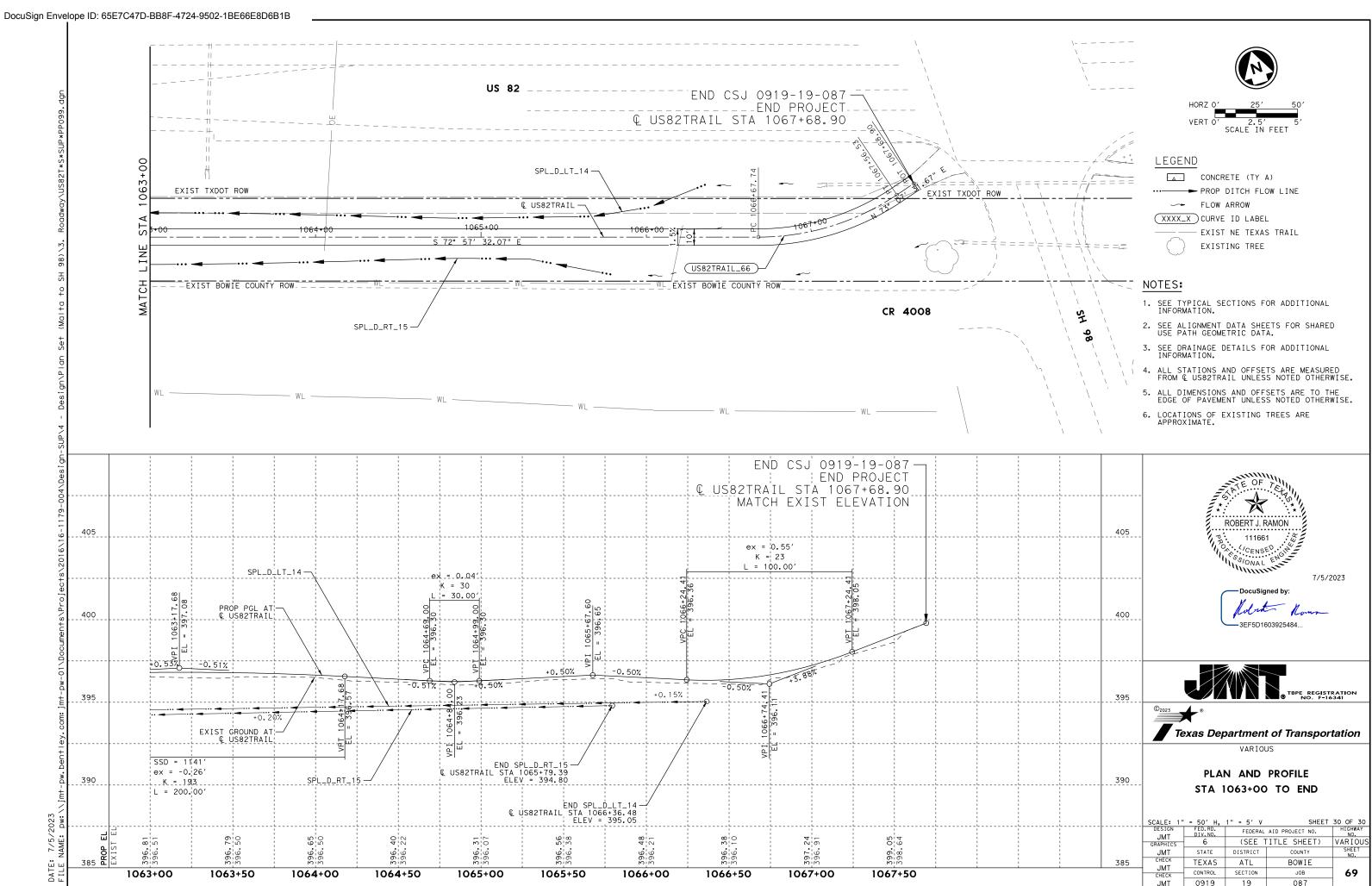
			SCALE: 1"		1" = 5' V	SHEET	27 OF 30
 			DESIGN JMT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.
54		GRAPHICS	6	(SEE TITLE SHEET)		VARIOUS	
97. 97.			JMT	STATE	DISTRICT	COUNTY	SHEET NO.
m m		385	снеск ЈМТ	TEXAS	ATL	BOWIE	
1052+	00		CHECK	CONTROL	SECTION	JOB	66
_			JMT	0919	19	087	



		SCALE: 1"		1" = 5' V	SHEET	28 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO,
82		GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
97. 96.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
mim	385	снеск ЈМТ	TEXAS	ATL	BOWIE	
1057+50		CHECK	CONTROL	SECTION	JOB	67
		JMT	0919	19	087	



1			= 50′ H,	1" = 5' V	SHEET	29 OF 30
		DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO,
51.0		GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
96. 96.		JMT	STATE	DISTRICT	COUNTY	SHEET NO.
mim	385	снеск ЈМТ	TEXAS	ATL	BOWIE	
1063+00		CHECK	CONTROL	SECTION	JOB	68
		JMT	0919	19	087	



		" = 50′ H,	1" = 5' V	SHEET	30 OF 30
	JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
	GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
38	5 CHECK JMT	TEXAS	ATL	BOWIE	
	CHECK	CONTROL	SECTION	JOB	69
	JMT	0919	19	087	

F	RIGHT DITCHES			RIGHT DITCHES	
DITCH NAME	STATION	ELEVATION	DITCH NAME	STATION	ELEVA
SPL_D_RT_01	903+94.66	407.63′	SPL_D_RT_09	1000+03.16	367.
	903+97.65	407.13′		1001+08.22	368.
	905+23.83	406.50′		1002+97.67	371
SPL_D_RT_02	905+26.33	406.50′		1006+56.57	372.
	915+07.21	408.46′		1007+47.21	373
-	915+19,55	409.41′		1008+83.60	373.
SPL_D_RT_03	930+15.12	403.46′		1009+06.22	373.
	931+91.25	403.11′	SPL_D_RT_10	1009+08.72	373
SPL_D_RT_04	932+98.14	402.90′		1009+31.10	373
	936+76.60	402.14′		1011+83.05	374
SPL_D_RT_05	936+78.52	402.14′		1012+42.42	375.
	944+71.99	403.73'		1014+63,79	376
-	955+07.54	401.14'		1017+63.69	376
-	955+29.83	400.25′		1017+90.99	377
-	955+49,66	400,14′		1018+60,99	376
SPL_D_RT_06	956+01.23	399.87′	SPL_D_RT_11	1018+68.83	376
-	956+21.70	399.77'		1020+59.72	377
-	957+76.96	399.22'		1021+65.95	377
-	960+31.97	396.80'	SPL_D_RT_12	1022+52.28	378
-	963+70,23	392.57'		1024+18,66	378
-	964+44.66	391.83′		1025+42.20	380
-	965+65.33	390.92'		1026+69.81	381
	967+22.89	389.35'		1029+21,19	384
-	968+72,25	388.60'		1030+30.05	386
-	968+80.30	388.28'		1032+03.03	388
-	968+99.97	388.12'		1033+86.88	391
SPL_D_RT_07	970+34.17	387.04'		1035+86.35	394
-	970+52.55	386.89'		1037+41.75	396
-	972+21.12	386.30'		1038+75.72	398
-	979+44.08	382.68'		1039+63,88	398
-	981+32.51	382.12'		1039+86.96	398
-	981+51.36	381.37'		1040+86,96	399
-	981+71.05	381.22'		1041+86.96	398
SPL_D_RT_08	982+01.64	380.99′		1040+86,96	398
-	982+22.27	380.83'		1043+37,96	398
-	983+40.00	380.42'		1044+54,52	397
SPL_D_RT_08A	984+45.00	379.78'		1050+50.73	396
-	985+02.14	379.35'	SPL_D_RT_13	1050+52.65	396
-	985+39,18	378.80'		1050+78,69	396.
	986+50.28	378.24'		1052+78.69	397
-	987+96,81	377.14'		1054+78,69	396
-	988+29.24	376.65'		1052+78.69	396
-	989+26.53	376.17'		1055+74,22	396
-	990+44.92	375.28'		1059+14.78	393
-	990+72.35	374.18'		1060+22.98	393
-	991+17.45	373.60'	SPL_D_RT_14	1060+24.90	393
-	991+75.08	373.31'		1060+59.04	393
-	991+97.42	373.02'	SPL_D_RT_15	1062+13.22	394
ŀ	992+11.68	372.31'		1065+79.39	394
-	992+55.11	371.72'	L		554
ŀ	994+18.99	370.90'			
-	994+40.80	370,46'			

## NOTES:

1. SEE DITCH COMPUTATIONS SHEET FOR VERTICAL POINT OF INTERSECTION FOR LEFT DITCH.



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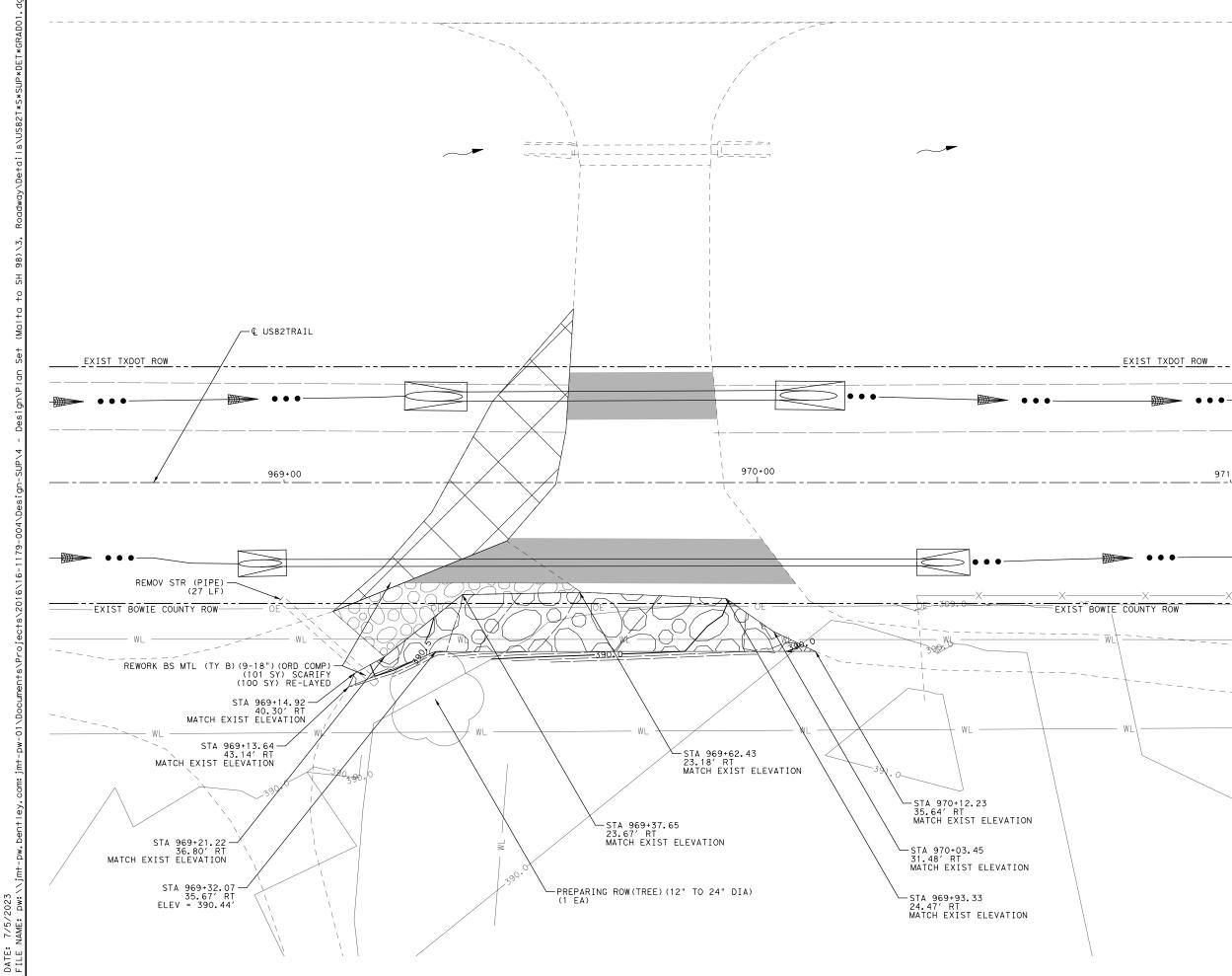
7/6/2023



VARIOUS

## DITCH SUMMARY TABLE

l				SHEE	ET 1 OF 1
ſ	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
$\left  \right $	GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
l	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
ſ	снеск ЈМТ	TEXAS	ATL	BOWIE	
$\left  \right $	CHECK	CONTROL	SECTION	JOB	70
	JMT	0919	19	087	





## LEGEND

<u>    400    </u>	PROPOSED	CONTOURS	(0.5′)
	PROPOSED	CONTOURS	(0.1′)
	EXISTING	CONTOURS	(1.0')
$\longrightarrow$	FLOW ARRO	W	
$\ge$	RE-LAYED	GRAVEL	
77 X 3	EXISTING	GRAVEL TO	REMAIN
XX	SCARIFIED	GRAVEL	
	CUT & RES	STORE PAVE	MENT

## NOTES:

- 1. REFER TO HORIZONTAL ALIGNMENT DATA FOR CURVE DATA AND ALIGNMENT INFORMATION.
- 2. ALL DIMENSIONS ARE TO THE EDGE OF PAVEMENT UNLESS NOTED OTHERWISE.
- 3. POINTS ARE SHOWN AT CRITICAL POINTS (PC, PT, ETC.) UNLESS NOTED OTHERWISE.
- 4. SEE PLAN AND PROFILE SHEETS FOR PAYMENT AND ADDITIONAL INFORMATION.
- ALL STATIONS AND OFFSETS ARE MEASURED FROM € US82TRAIL UNLESS NOTED OTHERWISE.
- 971 6. EXISTING GROUND ELEVATIONS OUTSIDE OF BOWIE COUNTY ROW LINES ARE APPROXIMATE AND BASED ON 2017 TNRIS CONTOURS.



7/5/2023

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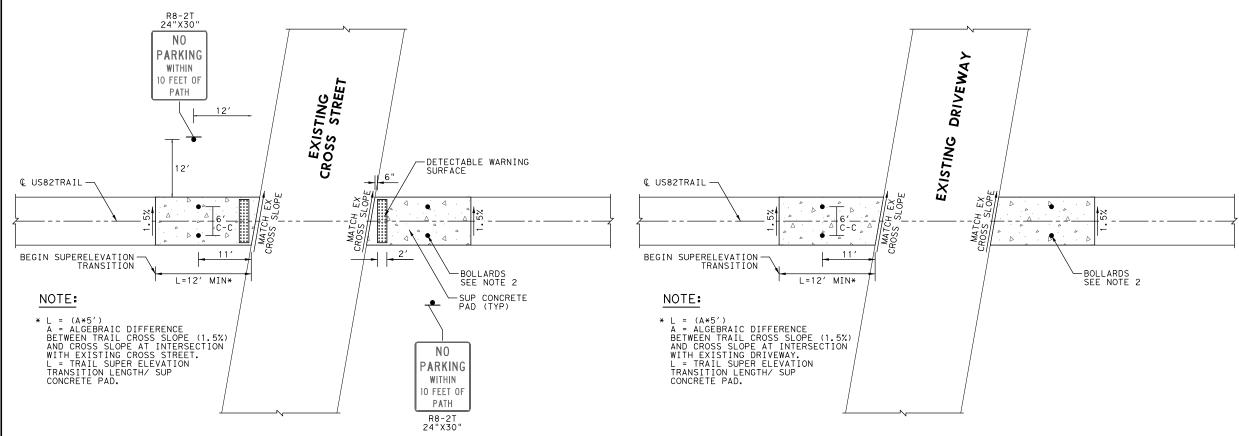




VARIOUS

## DRIVEWAY GRADING DETAIL

		= 20'		SHEE	ET 1 OF 1
	sign MT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.
	NU I PHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
	MT	STATE	DISTRICT	COUNTY	SHEET NO.
	іеск МТ	TEXAS	ATL	BOWIE	
	IVI I IECK	CONTROL	SECTION	JOB	71
J	MT	0919	19	087	



INTERSECTION DETAIL

DRIVEWAY DETAIL

EXIST STREET/ DRIVEWAY	BEGIN TRANSITION AT & US82TRAIL	BEGIN CROSS SLOPE	END TRANSITION AT ⊈ US82TRAIL	END CROSS SLOPE	TRANSITION LENGTH
CR 4105	903+43.94	1.50%	903+61.94	-2.29%	20
	903+84.53	-2.32%	904+05.53	1.50%	20
CR 4130-A	915+38.11	1.50%	915+50.11	0.66%	14
CR 4130-A	915+75.23	-0.33%	915+87.23	1.50%	14
CR 4104	932+15.69	1.50%	932+39.69	-2.99%	24
CR 4104	932+63.50	-3.43%	932+89.50	1.50%	26
CR 4102	955+37.27	1.50%	955+61.27	-2.69%	24
CR 4102	955+86.15	-3.39%	956+13.15	1.50%	27
PRIVATE ROAD	969+39.45	1.50%	969+57.45	-1.06%	18
FRIVATE ROAD	969+92.85	-1.28%	970+10.85	1.50%	18
	981+64.38	1.50%	981+76.38	-0.48%	14
DRIVEWAY	981+93.23	-0.77%	982+07.23	1.50%	14
CR 4007	1021+80.13	1.50%	1022+00.13	-2.38%	20
CK 4007	1022+24.52	-0.97%	1022+38.52	1.50%	14

## SUP SUPER ELEVATION CROSS SLOPE TRANSITIONS

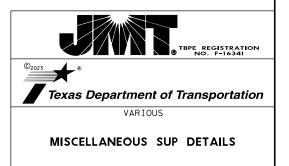
## NOTES:

- 1. SEE "MISCELLANEOUS SUP DETAILS -CONCRETE SUP" SHEET FOR MORE INFORMATION ON CONCRETE PAD.
- 2. SEE "MISCELLANEOUS SUP DETAILS -BOLLARDS" SHEET FOR MORE INFORMATION ON BOLLARDS.
- 3. SEE TYPICAL SECTIONS FOR SHARED USE PATH PAVEMENT SECTION.
- 4. PAYMENT FOR DETECTABLE WARNING SURFACE IS SUBSIDIARY TO CONCRETE SUP PAD.
- 5. R8-2T SMALL SIGN SUPPORTS SHALL BE PAID FOR USING ITEM 644 6060. SEE SUMMARY OF QUANTITIES FOR PAYMENT AND LOCATIONS.



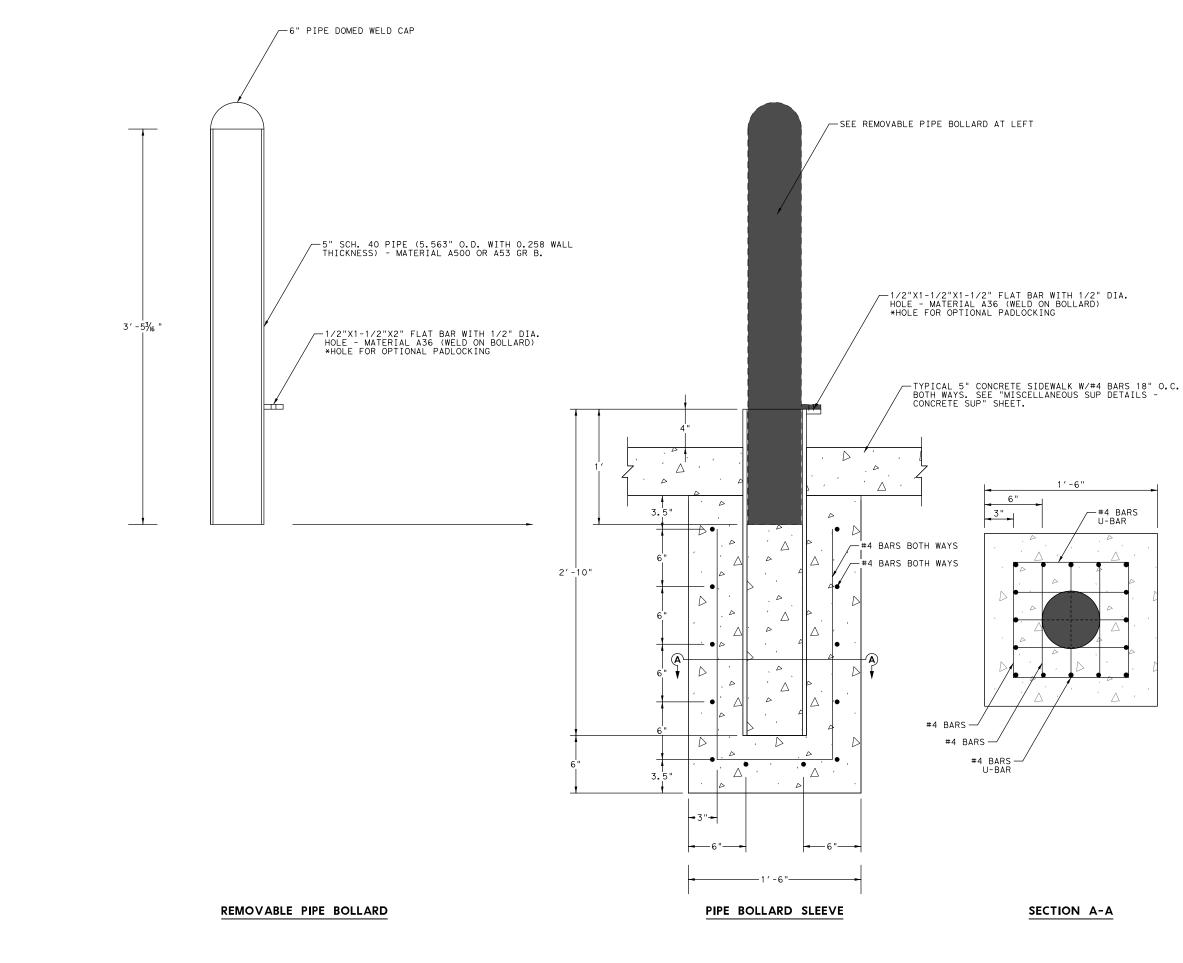
7/5/2023

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## INTERSECTIONS AND DRIVEWAYS

SCALE: NT			SHEE	
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL AID PROJECT NO.		HIGHWAY NO.
GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	72
JMT	0919	19	087	



FILE NAME: pw:

## NOTES:

- 1. CONCRETE SHALL BE CLASS A UNLESS NOTED OTHERWISE.
- 2. DEFORMED REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60. ALL REINFORCING BAR DIMENSIONS ON THE DETAILED DRAWINGS ARE TO CENTER OF BAR EXCEPT WHERE OTHERWISE NOTED AND ARE SUBJECT TO FABRICATION AND CONSTRUCTION TOLERANCES.



7/5/2023

-DocuSigned by: Robert Roma - 3EF5D1603925484...



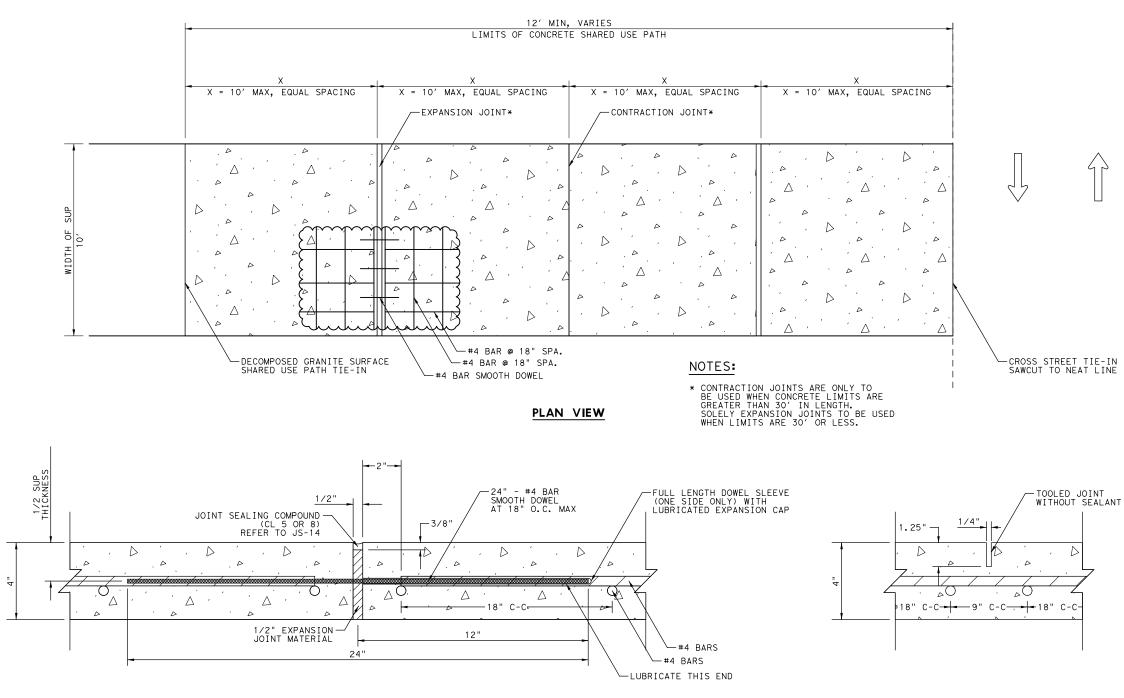
Texas Department of Transportation

VARIOUS

## MISCELLANEOUS SUP DETAILS

## BOLLARDS

SCALE: N1	s		SHE	ET 2 OF 3
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	HIGHWAY NO.	
GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	73
JMT	0919	19	087	]



**EXPANSION JOINT** 

CONTRACTION JOINT

(IST STREET/ DRIVEWAY	BEGIN TRANSITION AT ℚ US82TRAIL	END TRANSITION AT € US82TRAIL	CONCRETE PAD (SY)
00.4105	903+43.94	903+61.94	22
CR 4105	903+84.53	904+05.53	22
CR 4130-A	915+38.11	915+50.11	16
CR 4130-A	915+75.23	915+87.23	15
CR 4104	932+15.69	932+39.69	27
	932+63.50	932+89.50	29
00.4100	955+37.27	955+61.27	26
CR 4102	955+86.15	956+13.15	30
	969+39.45	969+57.45	19
PRIVATE ROAD	969+92.85	970+10.85	20
DDIVEWAY	981+64.38	981+76.38	16
DRIVEWAY	981+93.23	982+07.23	16
00.4007	1021+80.13	1022+00.13	22
CR 4007	1022+24.52	1022+38.52	16

## SUP CONCRETE PAD QUANTITIES



## NOTES:

1. CONCRETE SHARED USE PATH SHALL BE PAID FOR AS CONCRETE SIDEWALKS (4"). ALL MATERIALS AND CONSTRUCTION SHALL BE IN ACCORDANCE WITH ITEM 531 "SIDEWALKS."



7/5/2023

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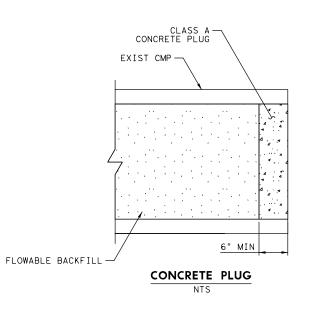


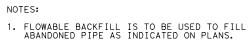
VARIOUS

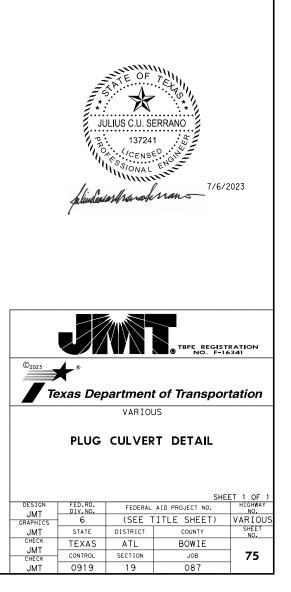
## MISCELLANEOUS SUP DETAILS

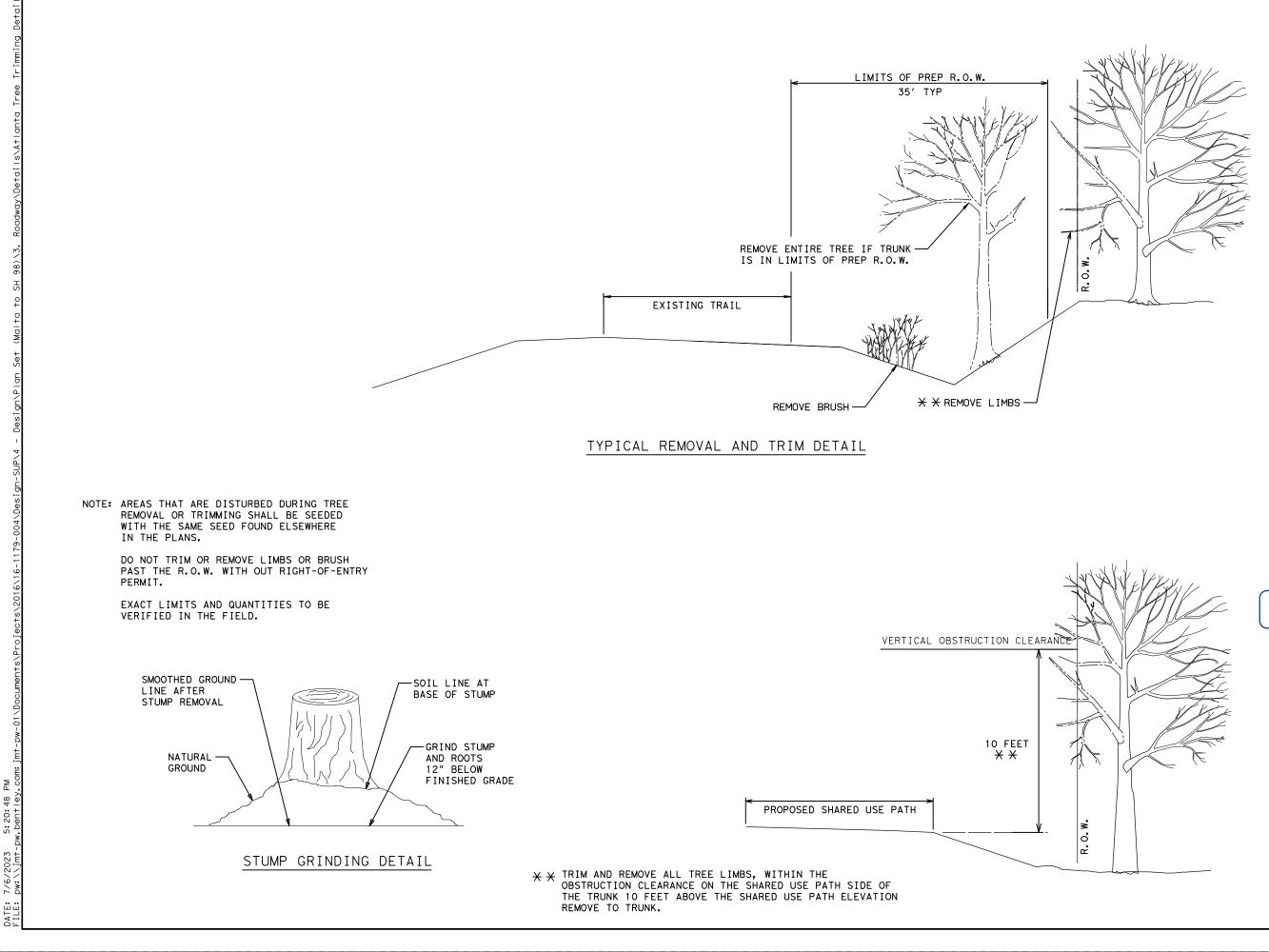
## CONCRETE SUP

SCALE: NT			SHEE	T 3 OF 3
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	HIGHWAY NO.	
GRAPHICS	6	(SEE ]	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	74
JMT	0919	19	087	









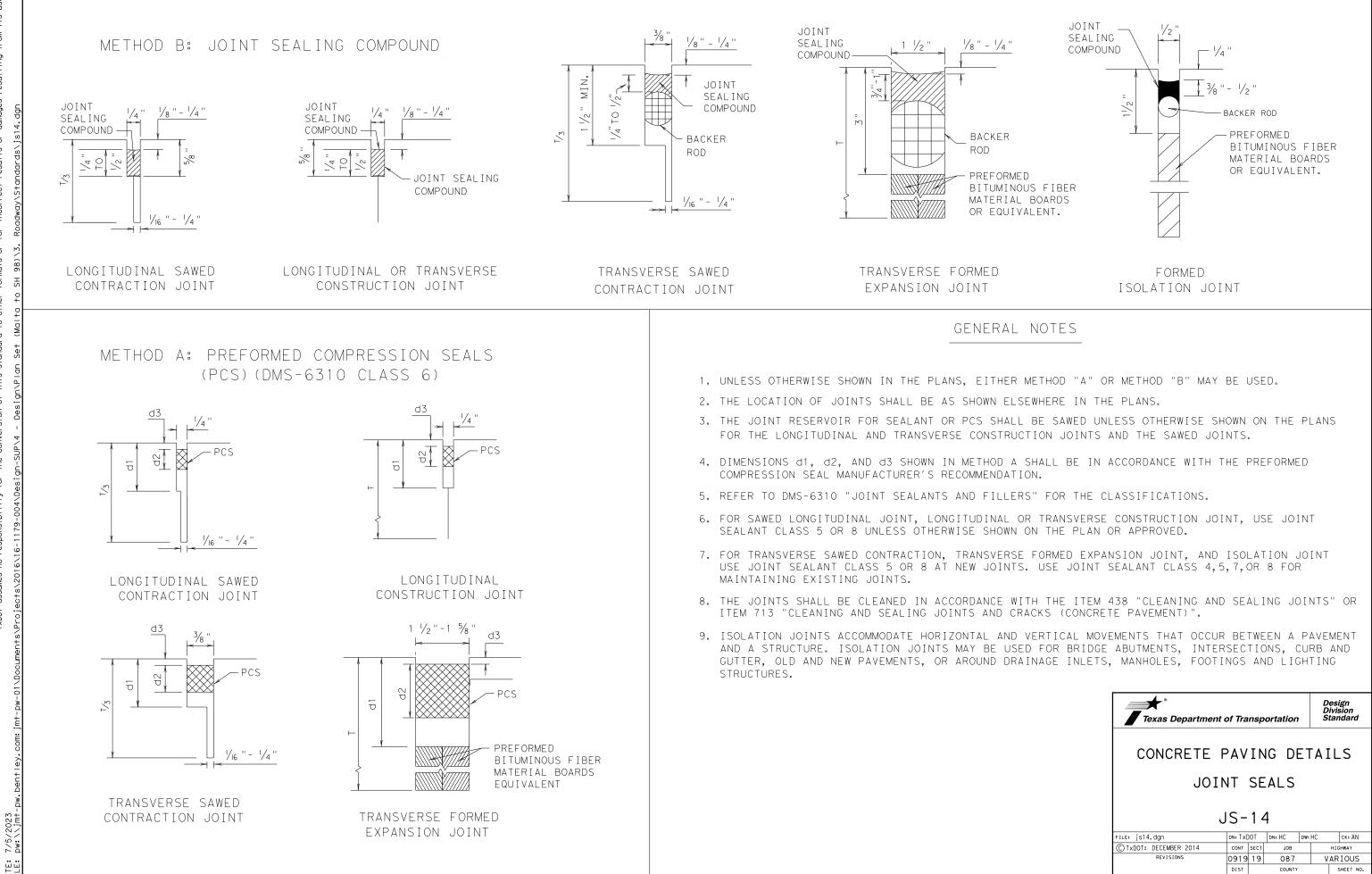


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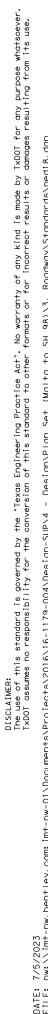
NOT TO SCALE

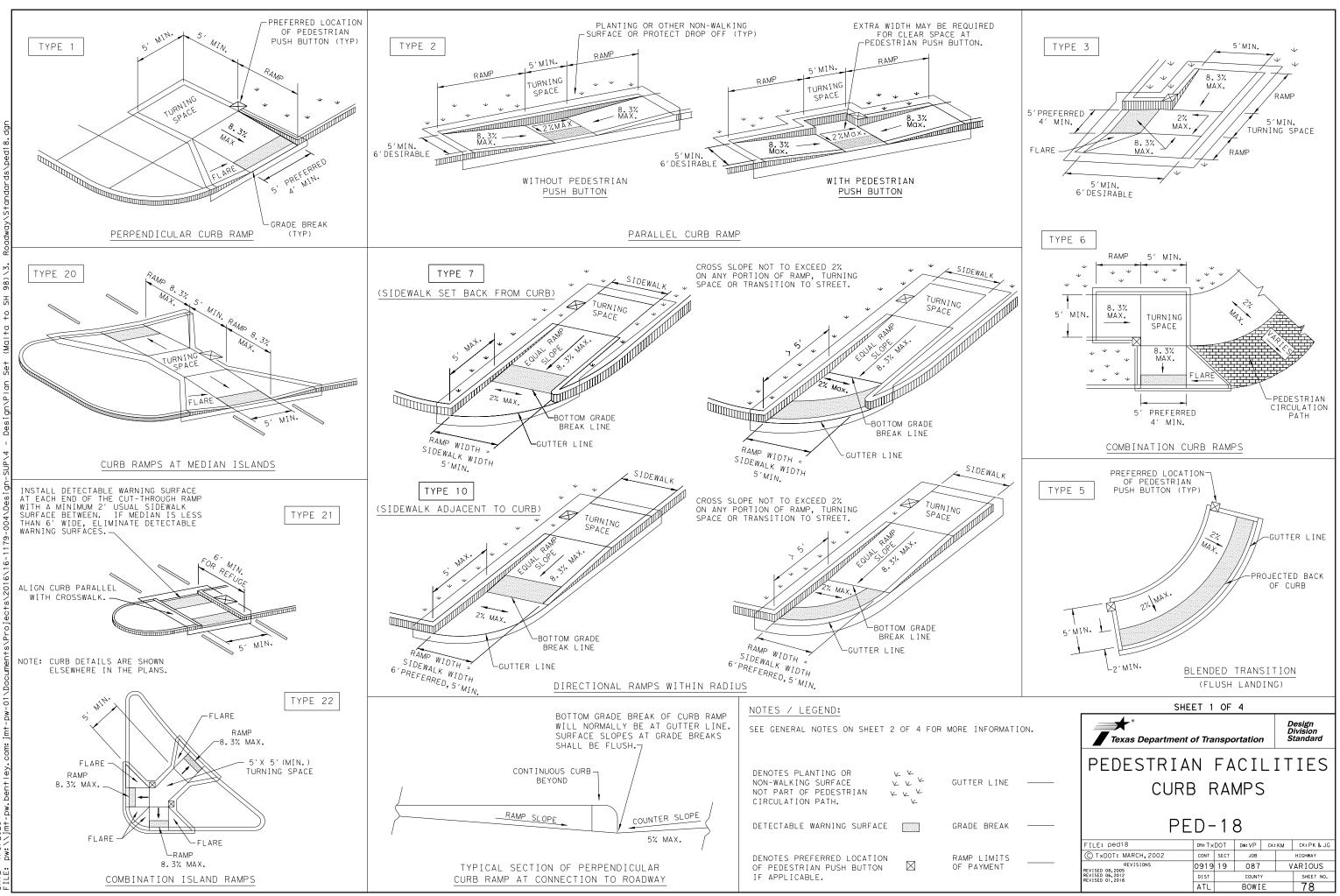
Texas Department of Transportation						
CONT	SECT	JOB		HIGHWAY		
0919	19	087	V	ARIOUS		
DIST		COUNTY	SHEET NO.			
ATL		BOWIE		76		



DATE:

Texas Department	of Tra	nsp	ortation		D	esign ivision andard
CONCRETE F JOI1				ΞT	ĀI	LS
L. L	IS-	14	1			
FILE: js14.dgn	dn: Tx[	)0T	dn: HC	DW:	нс	ск: АЛ
C TxDOT: DECEMBER 2014	CONT	SECT	JOB			HIGHWAY
REVISIONS	0919	19	087		V.	ARIOUS
	DIST		COUNTY			SHEET NO.
	ATL		BOWIE			77





## GENERAL NOTES

## CURB RAMPS

- 1. Install a curb ramp or blended transition at each pedestrian street crossing.
- 2. All slopes shown are maximum allowable. Cross slopes of 1.5% and lesser running should be used. Adjust curb ramp length or grade of approach sidewalks as directed.
- 3. Maximum allowable cross slope on sidewalk and curb ramp surfaces is 2%.
- 4. The minimum sidewalk width is 5'. Where the sidewalk is adjacent to the back of curb, a 6' sidewalk width is desirable. Where a 5' sidewalk cannot be provided due to site constraints, sidewalk width may be reduced to 4' for short distances. 5'x 5' passing areas at intervals not to exceed 200' are required.
- 5. Turning Spaces shall be 5'x 5' minimum. Cross slope shall be maximum 2%.
- 6. Clear space at the bottom of curb ramps shall be a minimum of 4'x 4' wholly contained within the crosswalk and wholly outside the parallel vehicular travel path.
- 7. Provide flared sides where the pedestrian circulation path crosses the curb ramp. Flared sides shall be sloped at 10% maximum, measured parallel to the curb. Returned curbs may be used only where pedestrians would not normally walk across the ramp, either because the adjacent surface is planted, substantially obstructed, or otherwise protected.
- 8. Additional information on curb ramp location, design, light reflective value and texture may be found in the latest draft of the Proposed Guidelines for Pedestrian Facilities in the Public Right of Way (PROWAG) as published by the U.S. Architectural and Transportation Barriers Compliance Board (Access Board).
- 9. To serve as a pedestrian refuge area, the median should be a minimum of 6' wide, measured from back of curbs. Medians should be designed to provide accessible passage over or through them.
- 10. Small channelization islands, which do not provide a minimum 5'x 5' landing at the top of curb ramps, shall be cut through level with the surface of the street.
- 11. Crosswalk dimensions, crosswalk markings and stop bar locations shall be as shown elsewhere in the plans. At intersections where crosswalk markings are not required, curb ramps shall align with theoretical crosswalks unless otherwise directed.
- 12. Provide curb ramps to connect the pedestrian access route at each pedestrian street crossing. Handrails are not required on curb ramps.
- 13. Curb ramps and landings shall be constructed and paid for in accordance with Item 531 "Sidewalks".
- 14. Place concrete at a minimum depth of 5" for ramps, flares and landings, unless otherwise directed.
- 15. Furnish and install No. 3 reinforcing steel bars at 18" o.c. both ways, unless otherwise directed.
- 16. Provide a smooth transition where the curb ramps connect to the street.
- 17. Curbs shown on sheet 1 within the limits of payment are considered part of the curb ramp for payment, whether it is concrete curb, gutter, or combined curb and gutter.
- 18. Existing features that comply with applicable standards may remain in place unless otherwise shown on the plans.

## DETECTABLE WARNING MATERIAL

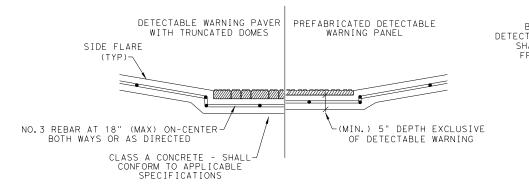
- 19. Curb ramps must contain a detectable warning surface that consists of raised truncated domes complying with PROWAG. The surface must contrast visually with adjoining surfaces, including side flares. Furnish and install an approved cast-in-place dark brown or dark red detectable warning surface material adjacent to uncolored concrete, unless specified elsewhere in the plans.
- Detectable Warning Materials must meet TxDOT Departmental Materials Specification DMS 4350 and be listed on the Material Producer List. Install products in accordance with manufacturer's specifications.
- 21. Detectable warning surfaces must be firm, stable and slip resistant.
- 22. Detectable warning surfaces shall be a minimum of 24 inches in depth in the direction of pedestrian travel, and extend the full width of the curb ramp or landing where the pedestrian access route enters the street.
- 23. Detectable warning surfaces shall be located so that the edge nearest the curb line is at the back of curb and neither end of that edge is greater than 5 feet from the back of curb. Detectable warning surfaces may be curved along the corner radius.
- 24. Shaded areas on Sheet 1 of 4 indicate the approximate location for the detectable warning surface for each curb ramp type.

## DETECTABLE WARNING PAVERS (IF USED)

- 25. Furnish detectable warning paver units meeting all requirements of ASTM C-936, C-33. Lay in a two by two unit basket weave pattern or as directed.
- 26. Lay full-size units first followed by closure units consisting of at least 25 percent (25%) of a full unit. Cut detectable warning paver units using a power saw.

## SIDEWALKS

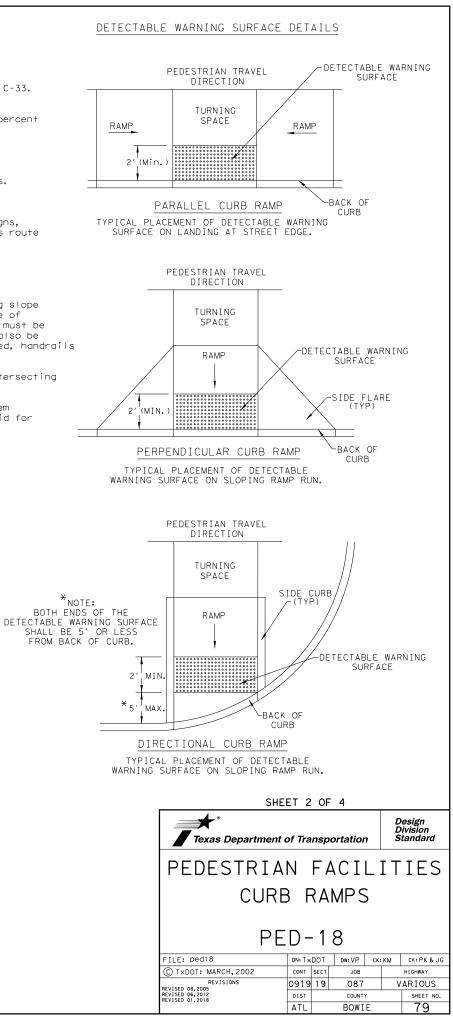
- 27. Provide clear ground space at operable parts, including pedestrian push buttons. Operable parts shall be placed within unobstructed reach range specified in PROWAG section R406.
- 28. Place traffic signal or illumination poles, ground boxes, controller boxes, signs, drainage facilities and other items so as not to obstruct the pedestrian access route or clear ground space.
- 29. Street grades and cross slopes shall be as shown elsewhere in the plans.
- 30. Changes in level greater than 1/4 inch are not permitted.
- 31. The least possible grade should be used to maximize accessibility. The running slope of sidewalks and crosswalks within the public right of way may follow the grade of the parallel roadway. Where a continuous grade greater than five percent (5%) must be provided, handrails may be desirable to improve accessibility. Handrails may also be needed to protect pedestrians from potentially hazardous conditions. If provided, handrails shall comply with PROWAG R409.
- 32. Handrail extensions shall not protrude into the usable landing area or into intersecting pedestrian routes.
- 33. Driveways and turnouts shall be constructed and paid for in accordance with Item "Intersections, Driveways and Turnouts". Sidewalks shall be constructed and paid for in accordance with Item, "Sidewalks".
- 34. Sidewalk details are shown elsewhere in the plans.



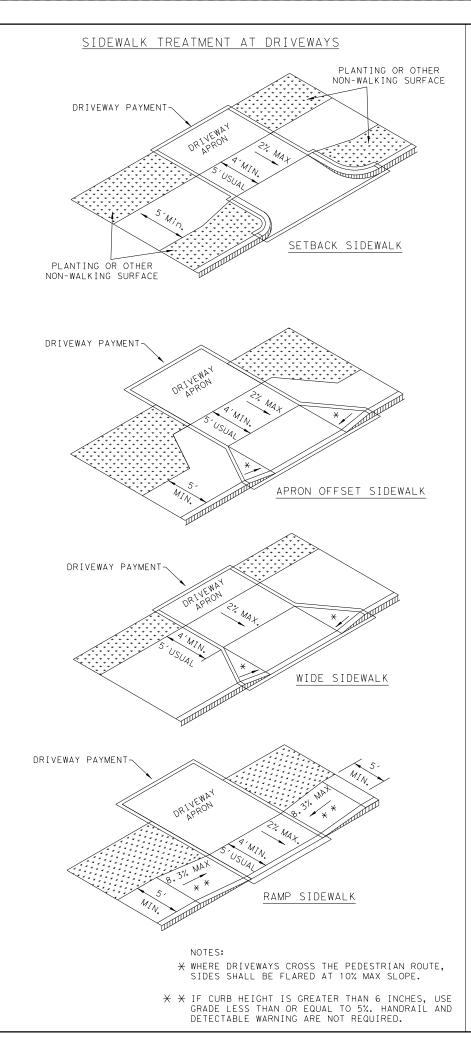
SECTION VIEW DETAIL CURB RAMP AT DETECTIBLE WARNINGS

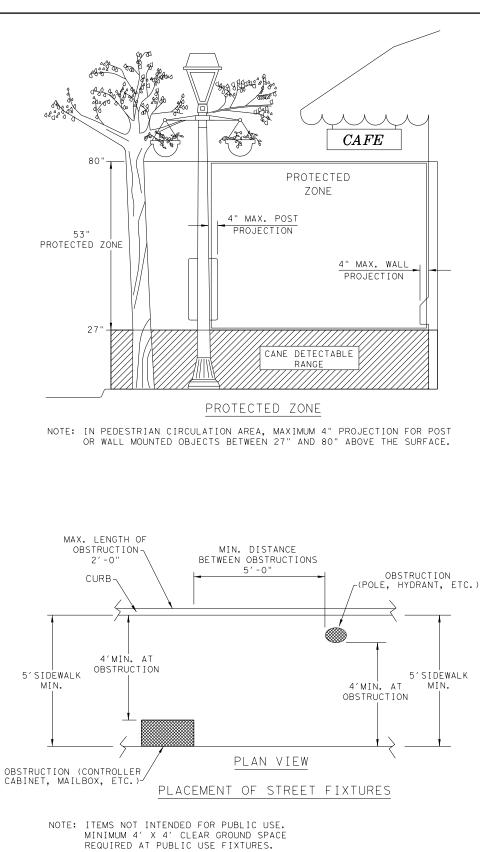
2023

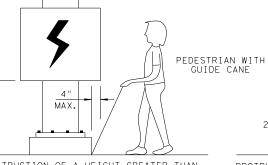
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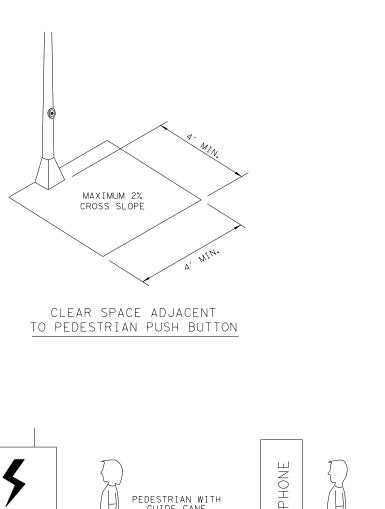








> 27"



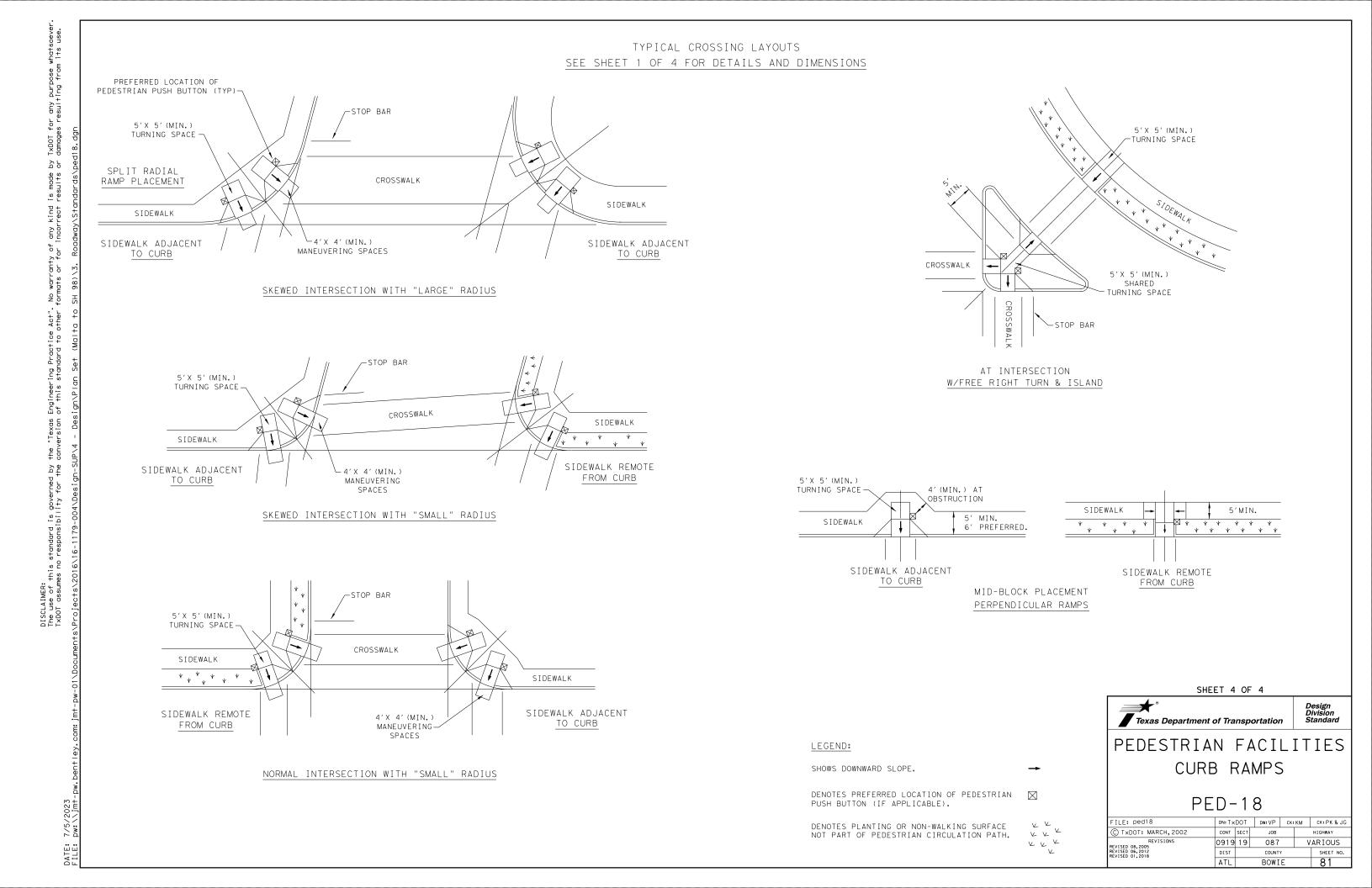
27 "MAX.

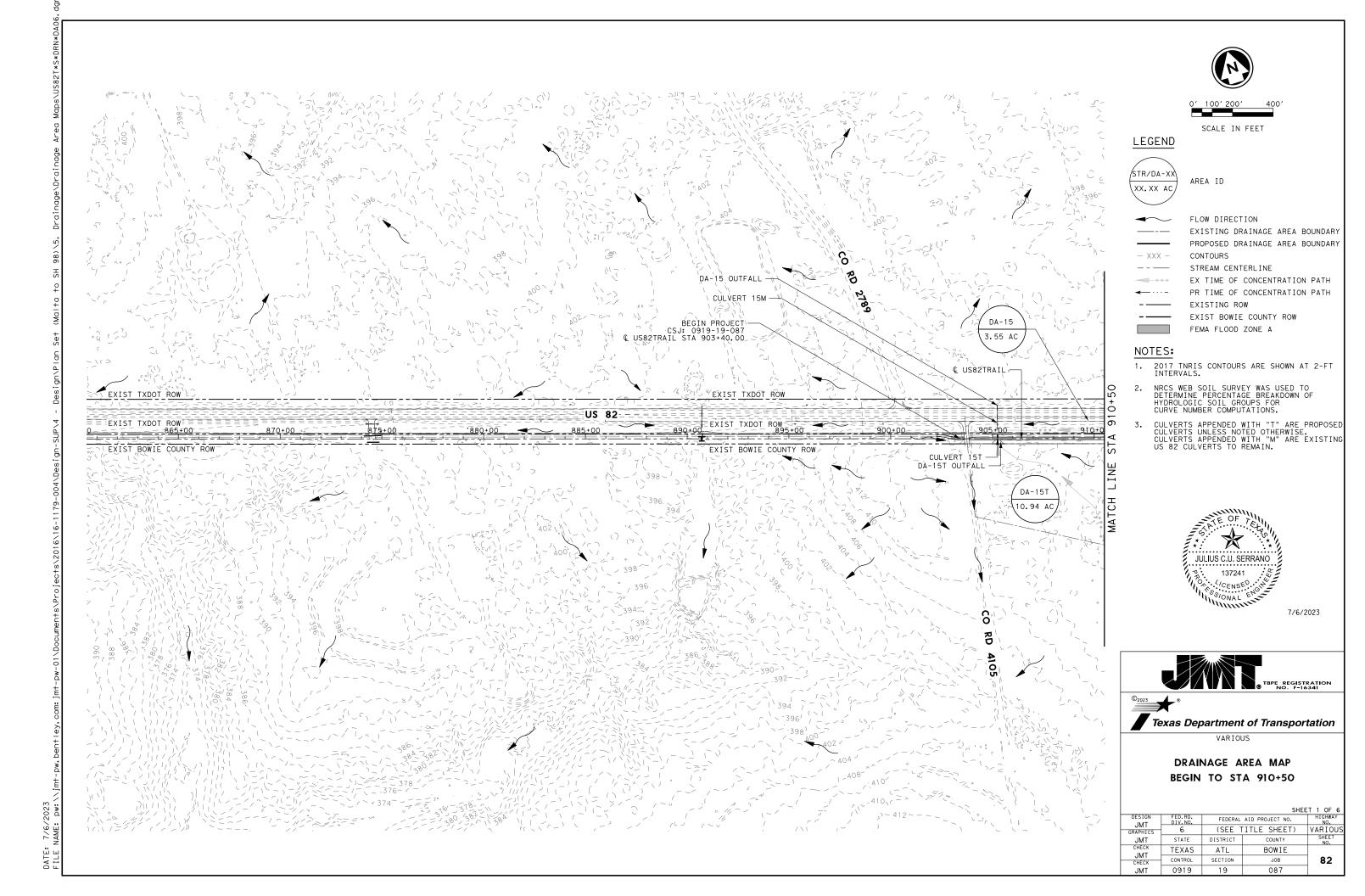
WHEN AN OBSTRUCTION OF A HEIGHT GREATER THAN 27" FROM THE SURFACE WOULD CREATE A PROTRUSION OF MORE THAN 4" INTO THE PEDESTRIAN CIRCULATION AREA, CONSTRUCT ADDITIONAL CURB OR FOUNDATION AT THE BOTTOM TO PROVIDE A MAXIMUM 4" OVERHANG.

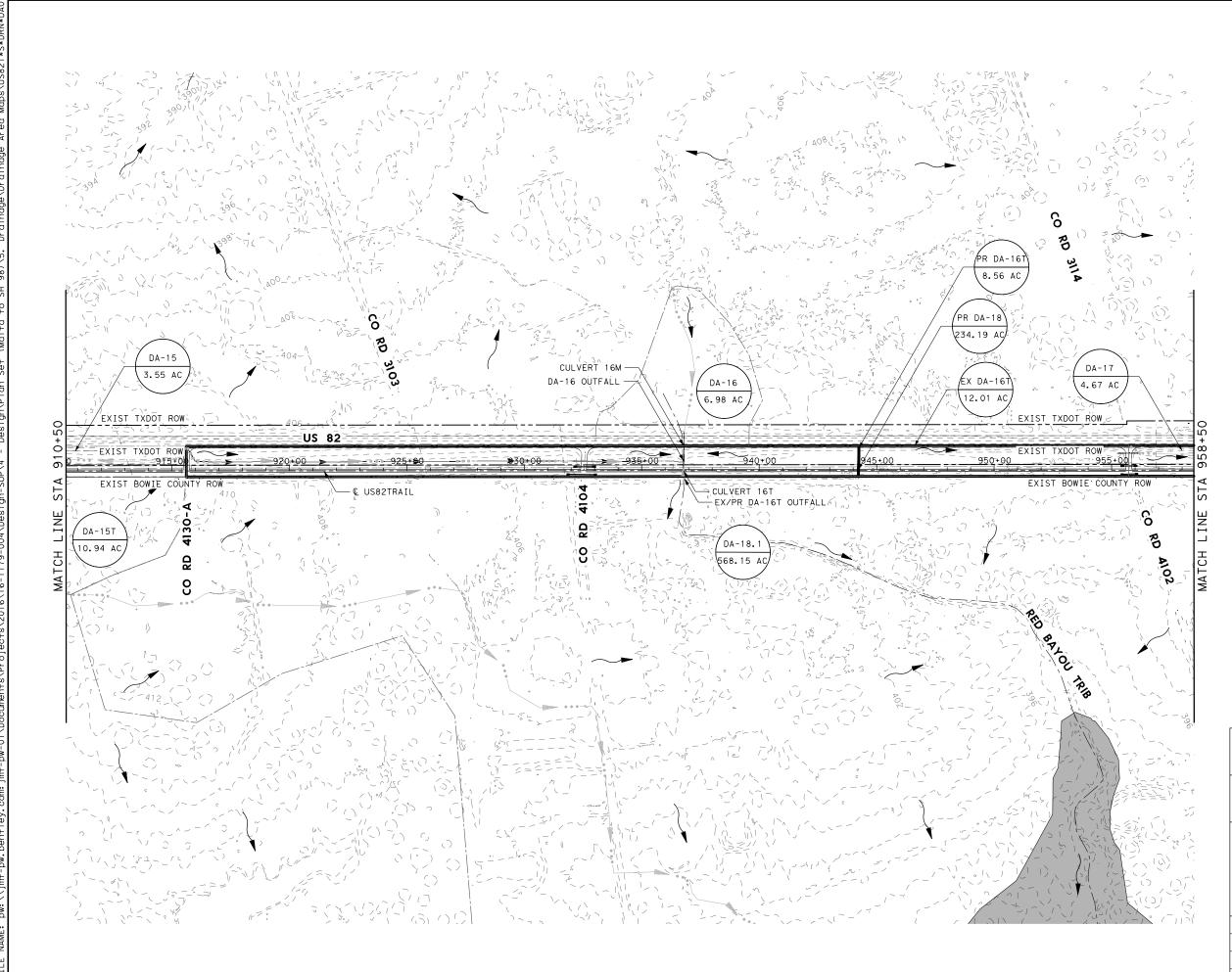
PROTRUDING OBJECTS OF A HEIGHT  $\leq$  27" ARE DETECTABLE BY CANE AND DO NOT REQUIRE ADDITIONAL TREATMENT.

DETECTION BARRIER FOR VERTICAL CLEARANCE < 80"

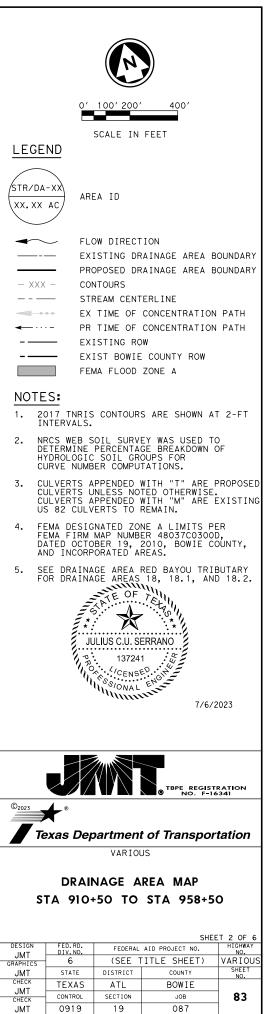
SHEET 3 OF 4									
Texas Department of	of Tra	nspo	ortation	1	D	esign ivision tandard			
PEDESTRIA					ΙT	IES			
PE		•	8						
FILE: ped18	DN: T X	DOT	DW:VP	CK:	КМ	CK: PK & JG			
© T×DOT: MARCH,2002	CONT	SECT	JOB			HIGHWAY			
REVISIONS REVISED 08,2005	0919	19	087		V	ARIOUS			
REVISED 06,2012 REVISED 01,2018	DIST		COUNTY	r .		SHEET NO.			
	ATL		BOWI	E		80			

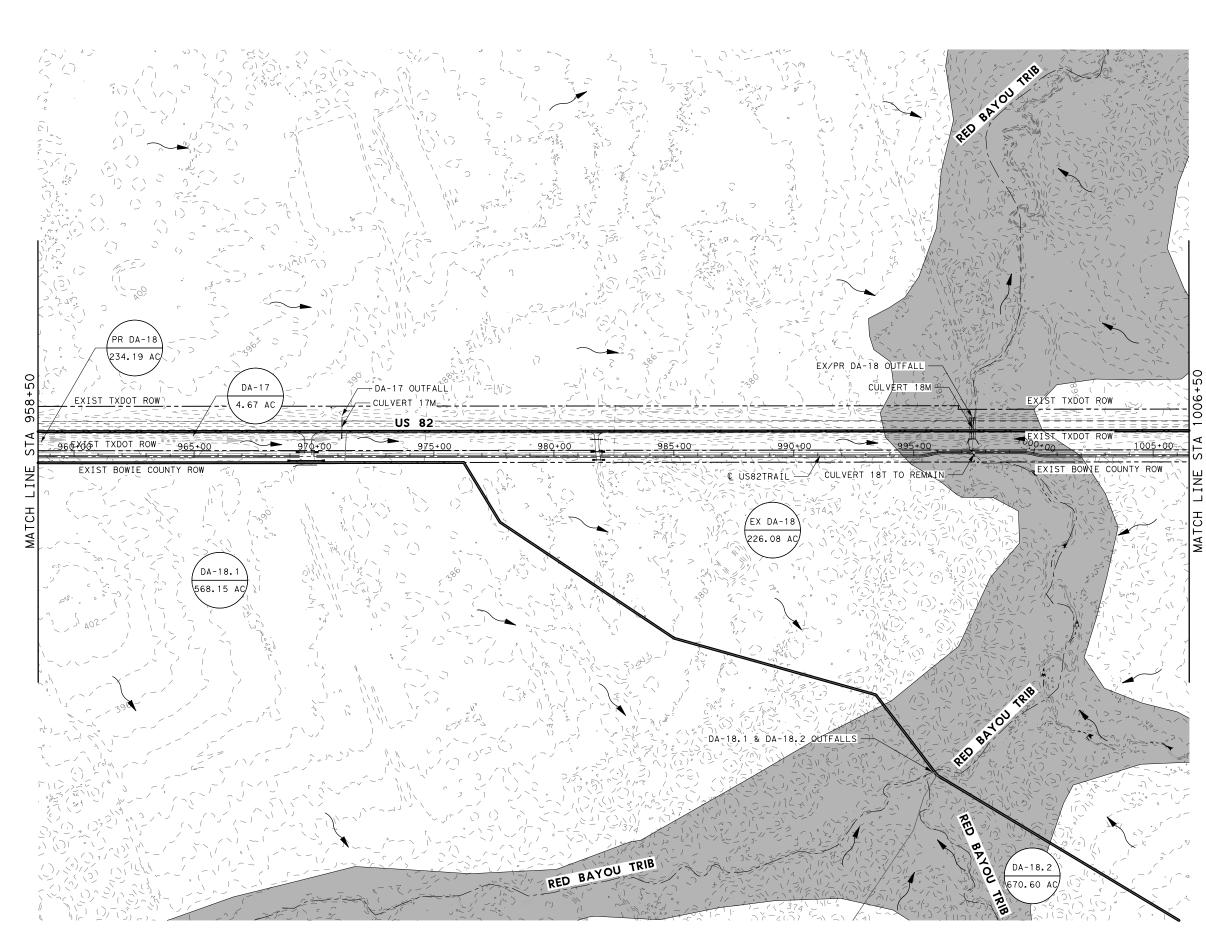




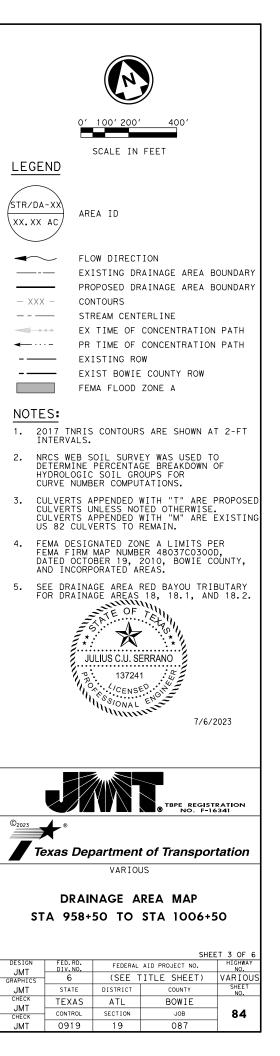


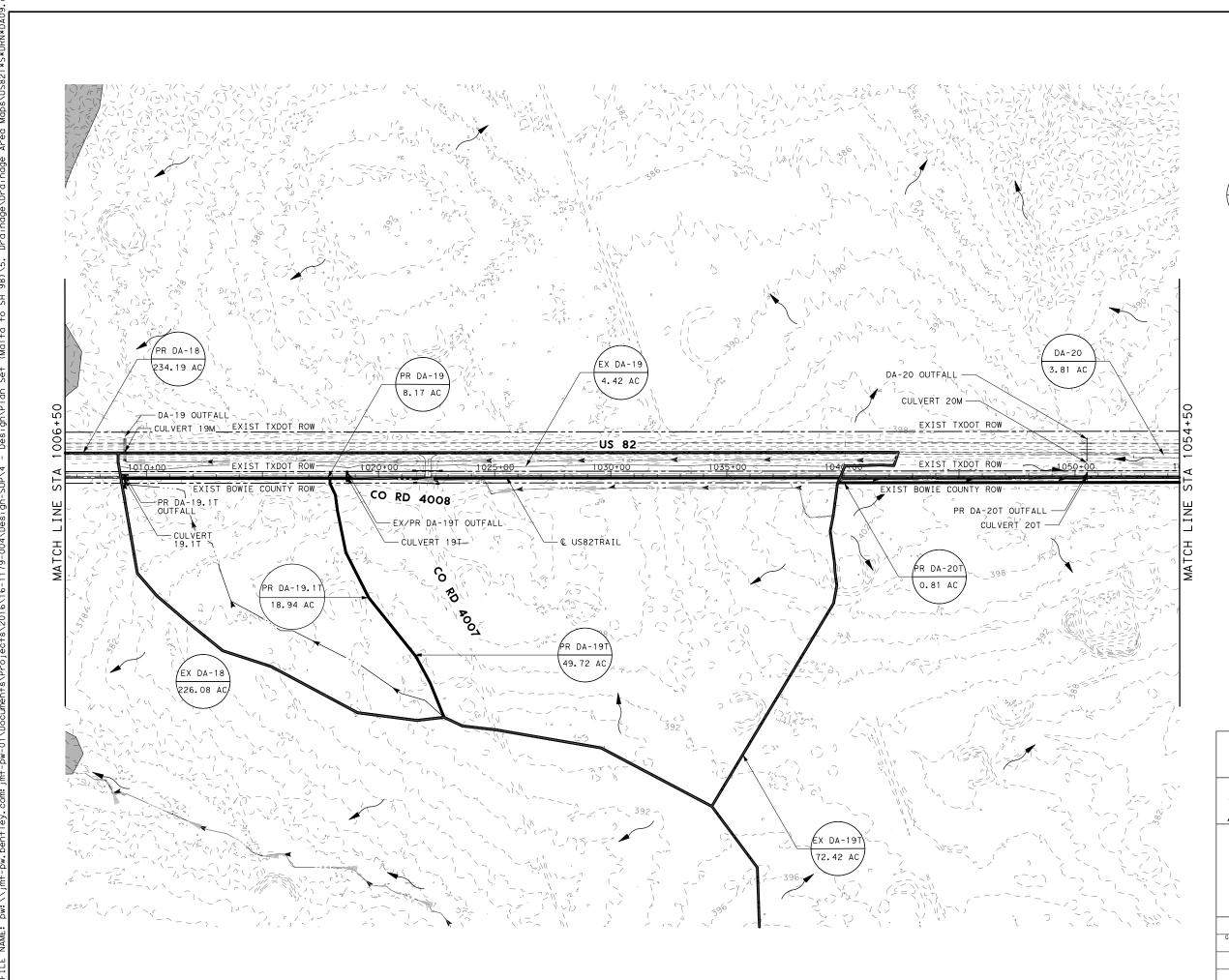
DATE: 7/6/2023



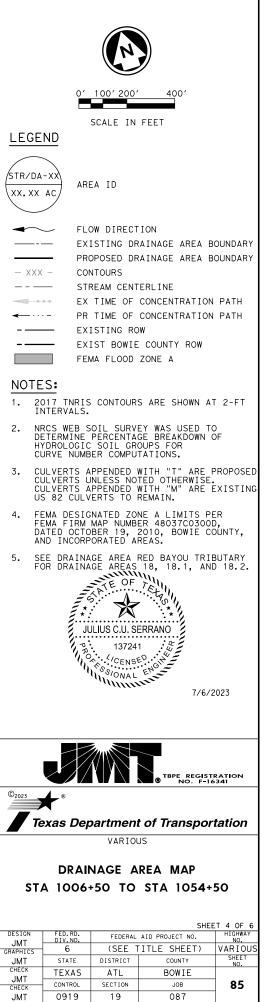








7/6/ DATE:



85

JOB

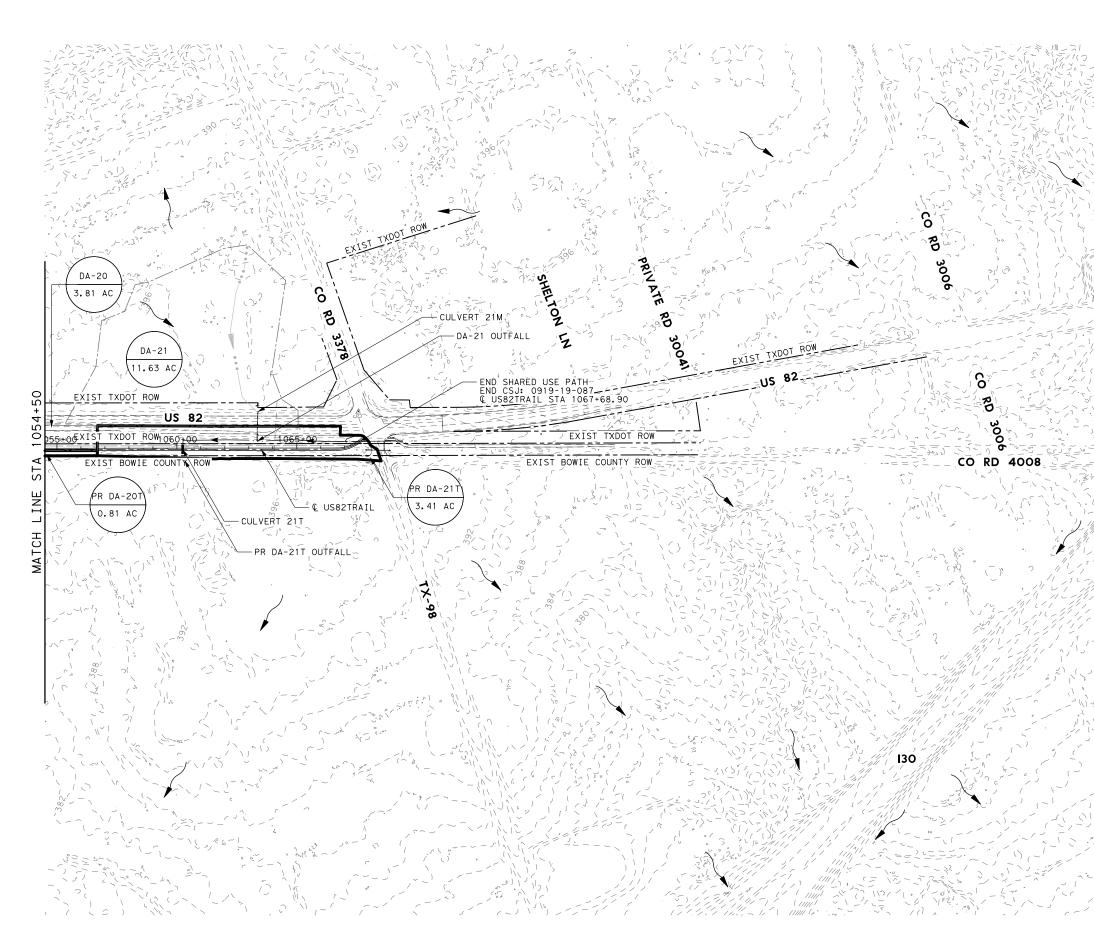
087

CONTROL

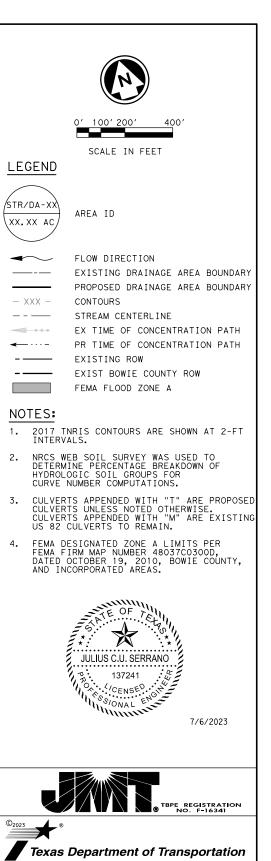
0919

SECTION

19



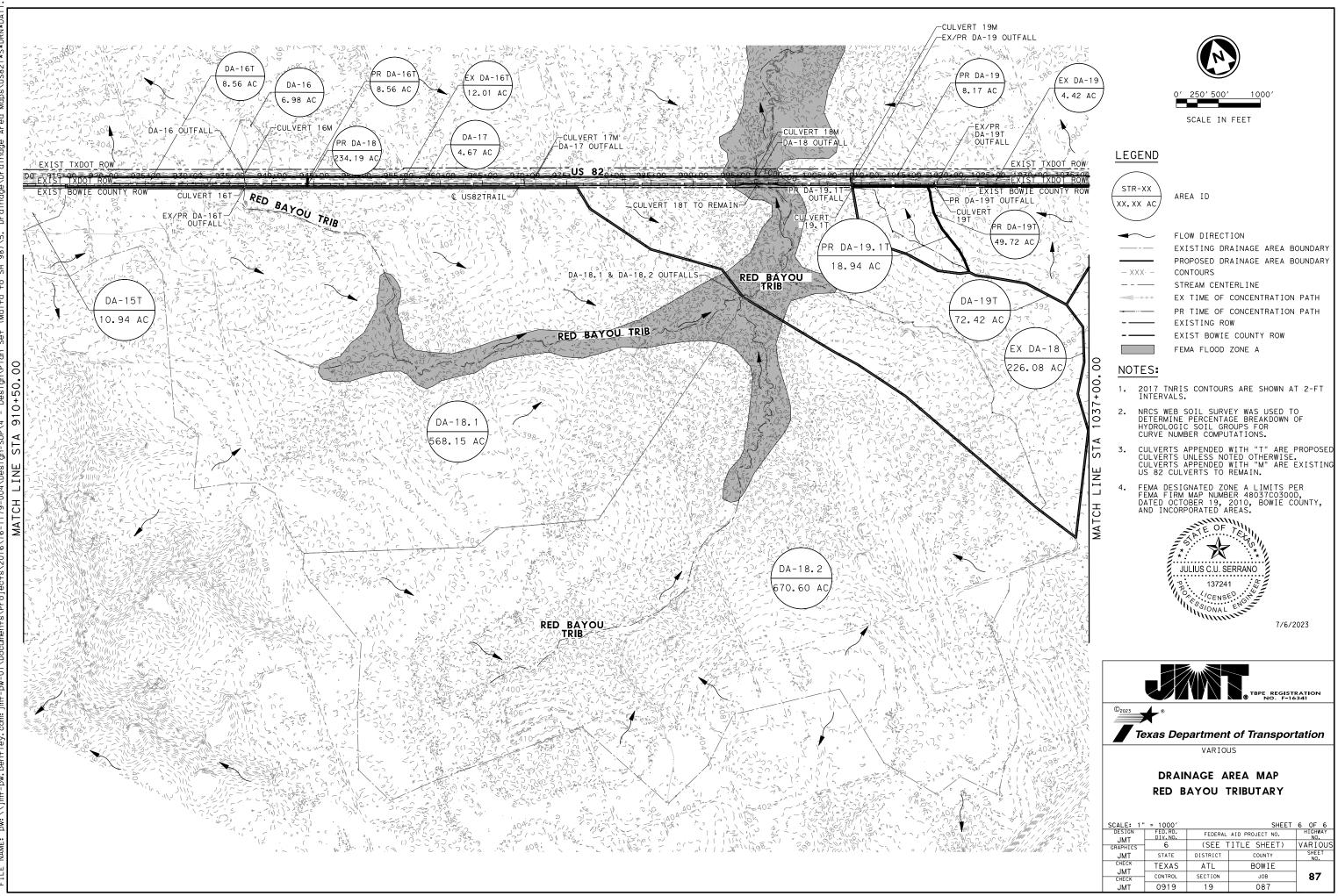




VARIOUS

## DRAINAGE AREA MAP STA 1054+50 TO END

			SHEE	T 5 OF 6
DESIGN	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
JMT GRAPHICS	6	(SEE 1	(ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	86
JMT	0919	19	087	



7/6/ DATE:

## CSJ 0919-19-087 ( US82TRAIL STA 903+40 TO END)

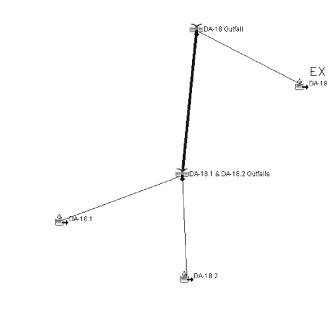
	EXISTING RATIONAL METHOD RESULTS (DA < 200 ACRES)														
DRAINAGE ID	DRAINAGE AREA (ACRES)	RUNOFF COEFFICIENT	TC (MINUTES )	2-YR INTENSITY (IN/HR)	2-YR DISCHARGE (CFS)	5-YR INTENSITY (IN/HR)	5-YR DISCHARGE (CFS)	10-YR INTENSITY (IN/HR)	10-YR DISCHARGE (CFS)	25-YR INTENSITY (IN/HR)	25-YR DISCHARGE (CFS)	50-YR INTENSITY (IN/HR)	50-YR DISCHARGE (CFS)	100-YR INTENSITY (IN/HR)	100-YR DISCHARGE (CFS)
DA-15	3.55	0.58	12.6	4.16	8	5.25	11	6.05	12	7.10	14	7.86	16	8.61	18
DA-15T	10.94	0.38	31.5	2.60	11	3.27	14	3.77	16	4.41	19	4.88	20	5.33	22
DA-16	6.98	0.42	33.3	2.54	7	3.21	9	3.70	11	4.32	13	4.78	14	5.22	15
EX DA-16T	12.01	0.46	22.0	3.25	18	4.10	23	4.73	26	5.54	30	6.13	34	6.70	37
DA-17	4.67	0.49	10.0	4.55	10	5.75	13	6.64	15	7.79	18	8.63	20	9.46	22
EX DA-19	4.42	0.44	22.7	3.20	6	4.04	8	4.65	9	5.45	10	6.03	12	6.59	13
EX DA-19T	72.42	0.38	43.8	2.22	62	2.81	78	3.24	90	3.79	105	4.19	116	4.58	127
DA-20	3.81	0.49	17.8	3.53	7	4.44	8	5.12	10	6.00	11	6.64	12	7.28	14
DA-21	11.63	0.43	34.2	2.46	12	3.11	16	3.59	18	4.23	21	4.69	24	5.16	26

## CSJ 0919-19-087 ( @ US82TRAIL STA 903+40 TO END)

	EXISTING HEC-HMS RESULTS (DA > 200 ACRES)*												
DRA I NAGE I D	DRAINAGE AREA (SQ. MI.)	DRAINAGE AREA (ACRES)	COMPOSITE CN VALUE	TC (MINUTES)	LAG TIME (MIN)	INITIAL ABSTRACTION (IN)	IMPERVIOUS COVER (%)	2-YEAR PEAK DISCHARGE (CFS)	5-YEAR PEAK DISCHARGE (CFS)	10-YEAR PEAK DISCHARGE (CFS)	25-YEAR PEAK DISCHARGE (CFS)	50-YEAR PEAK DISCHARGE (CFS)	
EX DA-18	0.35	226.08	65	115	68.7	1.1	2.21%	72	131	181	253	312	
DA-18.1	0.89	568.15	67	80	48.1	1.0	2.43%	259	458	623	857	1044	
DA-18.2	1.05	670.60	66	139	83.6	1.0	1.36%	202	361	494	684	836	
DA-18.1 & DA-18.2 Outfalls	1.94	1239.04	-		-	-	-	414	737	1008	1 3 9 5	1706	
DA-18 Outfall	2.29	1464.96	-		-	-	-	484	865	1186	1643	2009	

* SEE HEC-HMS DIAGRAM

HEC-HMS DIAGRAM

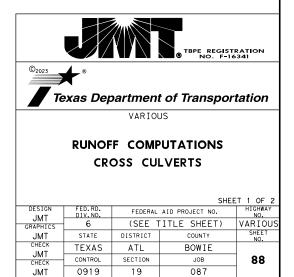


## NOTES:

- FLOWS WERE DETERMINED USING RATIONAL METHOD AND NRCS HYDROGRAPH METHOD. COMPUTATIONS CALCULATED USING BENTLEY POWER GEOPAK (DRAINAGE) V81 SS10 AND HEC-HMS 4.4.
- TIME OF CONCENTRATION (TC) WAS COMPUTED USING NRCS METHOD.
- 3. MINIMUM TIME OF CONCENTRATION USED IS 10 MINUTES.
- EXISTING TRAIL CULVERTS WERE REPLACED WITH SAME SIZE PIPE IN PROPOSED CONDITIONS. PROPOSED CULVERTS WERE ANALYZED TO ASSURE NO ADVERSE IMPACTS TO ADJACENT PROPERTIES AND HYDRAULIC PERFORMANCE OF US 82 ROADWAY CULVERTS.
- REFER TO CULVERT NOTES ON HYDRAULIC DATA SHEETS FOR COMBINED RUNOFF DESCRIPTION AT STRUCTURES.



7/6/2023



19

087

0919

## CSJ 0919-19-087 ( C US82TRAIL STA 903+40 TO END)

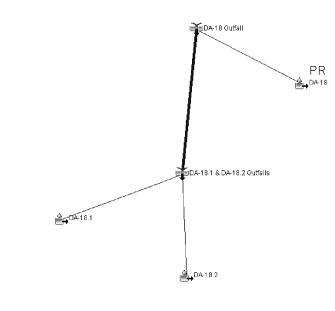
					PI	ROPOSED RAI	FIONAL METH	OD RESULTS	(DA < 200	ACRES)				
DRAINAGE ID	DRAINAGE AREA (ACRES)	RUNOFF COEFFICIENT	TC (MINUTES)	2-YR INTENSITY (IN/HR)	2-YR DISCHARGE (CFS)	5-YR INTENSITY (IN/HR)	5-YR DISCHARGE (CFS)	10-YR INTENSITY (IN/HR)	10-YR DISCHARGE (CFS)	25-YR INTENSITY (IN/HR)	25-YR DISCHARGE (CFS)	50-YR INTENSITY (IN/HR)	50-YR DISCHARGE (CFS)	100-YR INTENSITY (IN/HR)
PR DA-16T	8.56	0.46	22.0	3.16	12	3.98	16	4.59	18	5.39	21	5.97	23	6.55
PR DA-19	8.17	0.38	22.7	3.11	10	3.92	12	4.52	14	5.30	16	5.88	18	6.45
PR DA-19.1T	18.94	0.36	13.9	3.97	27	4.99	34	5.74	39	6.72	46	7.44	51	8.14
PR DA-19T	49.72	0.38	43.8	2.12	40	2.68	50	3.10	58	3.65	69	4.05	76	4.46
PR DA-20T	0.81	0.40	10.0	4.55	1	5.71	2	6.57	2	7.68	2	8.48	3	9.27
PR DA-21T	3.41	0.58	11.9	4.24	8	5.33	11	6.13	12	7.17	14	7.93	16	8.67

## CSJ 0919-19-087 ( C US82TRAIL STA 903+40 TO END)

				PROPOSE	D HEC-HMS	RESULTS (D	A > 200 AC	RES) *				
DRAINAGE ID	DRAINAGE AREA (SQ. MI.)	DRAINAGE AREA (ACRES)	COMPOSITE CN VALUE	TC (MINUTES)	LAG TIME (MIN)	INITIAL ABSTRACTION (IN)	IMPERVIOUS COVER (%)	2-YEAR PEAK DISCHARGE (CFS)	5-YEAR PEAK DISCHARGE (CFS)	10-YEAR PEAK DISCHARGE (CFS)	25-YEAR PEAK DISCHARGE (CFS)	50-YEAR PEAK DISCHARGE (CFS)
PR DA-18	0.37	234.19	65	115	68.7	1.1	2.85%	75	137	189	264	325
DA-18.1	0.89	568.15	67	78	46.8	1.0	2.43%	259	458	623	857	1044
DA-18.2	1.05	670.60	66	139	83.6	1.0	1.36%	202	361	494	684	836
DA-18.1 & DA-18.2 Outfalls	1.94	1239.04	-		-	-	-	414	737	1008	1395	1706
DA-18 Outfall	2.30	1473.28	-		-	-	-	487	871	1194	1654	2022

* SEE HEC-HMS DIAGRAM

HEC-HMS DIAGRAM



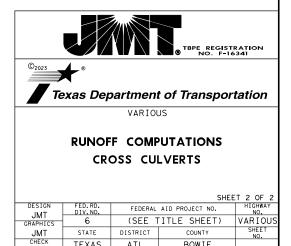
Y	100-YR DISCHARGE (CFS)
	26
	20
	56
	84
	3
	17

## NOTES:

- FLOWS WERE DETERMINED USING RATIONAL METHOD AND NRCS HYDROGRAPH METHOD. COMPUTATIONS CALCULATED USING BENTLEY POWER GEOPAK (DRAINAGE) V81 SS10 AND HEC-HMS 4.4.
- TIME OF CONCENTRATION (TC) WAS COMPUTED USING NRCS METHOD.
- 3. MINIMUM TIME OF CONCENTRATION USED IS 10 MINUTES.
- EXISTING TRAIL CULVERTS WERE REPLACED WITH SAME SIZE PIPE IN PROPOSED CONDITIONS. PROPOSED CULVERTS WERE ANALYZED TO ASSURE NO ADVERSE IMPACTS TO ADJACENT PROPERTIES AND HYDRAULIC PERFORMANCE OF US 82 ROADWAY CULVERTS.
- REFER TO CULVERT NOTES ON HYDRAULIC DATA SHEETS FOR COMBINED RUNOFF DESCRIPTION AT STRUCTURES.



7/6/2023



ATL

SECTION

19

BOWIE

JOB

087

89

TEXAS

0919

CONTROL

JMT CHECK JMT

## CSJ 0919-19-087

DITCH ID	FROM US82TRAIL STATION	FROM FLOWLINE ELEVATION (FT)	TO € US82TRAIL STATION	TO FLOWLINE ELEVATION (FT)	DITCH GRADE (FT/FT)	LINING	BOTTOM WIDTH (FT)	DITCH DEPTH (FT)	FRONT SLOPE (H:1)	BACK SLOPE (H:1)	MANNING'S N	DITCH CAPACITY (CFS)	VELOCITY (FPS)	DESIGN SHEAR STRESS (PSF)	ALLOWABLE SHEAR STRESS (PSF)	DESIGN 5-YR FLOW (CFS)	CONTRIBUTING DRAINAGE AREA TO DITCH
SPL_D_LT_1	904+15.00	408.78	904+42.93	406.80	-0.071	CLASS B	0	0.33	4	4	0.035	1	3.30	1.44	2.10	1	DA-15T
JFL_D_LI_I	904+42.93	406.80	905+23.34	406.40	-0.005	CLASS B	0	1.29	4	4	0.035	15	2.19	0.40	2.10	14	DA-15T
SPL_D_LT_2	905+26.33	406.40	914+89.34	407.36	-0.001	CLASS B	0	1.52	4	4	0.035	10.2	1.10	0.10	2.10	10.2	DA-15T
JI L_D_L I_Z	914+89.34	407.36	915+18.23	409.62	-0.078	CLASS B	0	0.30	4	4	0.035	1	3.28	1.46	2.10	1	DA-15T
	915+96.60	408.91	916+17.08	407.16	-0.085	CLASS B	0	0.30	4	4	0.035	1	3.43	1.60	2.10	1	DA-16 & PR DA-16T
SPL_D_LT_3	916+17.08	407.16	920+18.70	405.15	-0.005	CLASS B	0	2.02	4	4	0.035	48	2.96	0.63	2.10	25	DA-16 & PR DA-16T
	920+18.70	405.15	932+10.42	402.99	-0.002	CLASS B	0	5.44	4	4	0.035	408	3.44	0.61	2.10	25	DA-16 & PR DA-16T
SPL_D_LT_4	932+90.46	402.87	936+76.60	402.17	-0.002	CLASS B	0	2.34	4	4	0.035	43	1.96	0.26	2.10	25	DA-16 & PR DA-16T
	936+81.32	402.17	943+55.80	403.18	0.002	CLASS B	0	1.50	4	4	0.035	12	1.33	0.14	2.10	8	DA-16 & PR DA-16T
SPL_D_LT_5	943+55.80	403.18	948+56.20	401.43	-0.004	CLASS B	0	2.98	4	4	0.035	114	3.21	0.65	2.10	13	DA - 1 7
SPL_U_LI_S	948+56.20	401.43	953+51.76	400.69	-0.002	CLASS B	0	2.62	4	4	0.035	53	1.93	0.24	2.10	13	DA - 1 7
	953+51.76	400.69	955+52.35	399.98	-0.004	CLASS B	0	2.71	4	4	0.035	88	3.01	0.59	2.10	13	DA-17
	955+95.16	399.84	956+44.83	399.66	-0.004	CLASS B	0	2.67	4	4	0.035	85	2.99	0.58	2.10	13	DA-17
	956+44.83	399.66	964+77.88	391.33	-0.010	CLASS B	0	2.95	4	4	0.035	188	5.39	1.84	2.10	13	DA-17
SPL_D_LT_6	964+77.88	391.33	966+12.68	390.32	-0.008	CLASS B	0	2.72	4	4	0.035	131	4.42	1.27	2.10	13	DA - 17
	966+12.68	390.32	969+38.19	387.53	-0.009	CLASS B	0	2.91	4	4	0.035	168	4.95	1.56	2.10	13	DA-17
SPL_D_LT_7	970+04.32	386.97	971+16.08	386.01	-0.009	CLASS B	0	2.66	4	4	0.035	131	4.65	1.42	2.10	13	DA-17
SPL_U_LI_I	971+16.08	386.01	981+66.73	380.76	-0.005	CLASS B	0	3.11	4	4	0.035	153	3.95	0.97	2.10	19	DA-18 (UP TO ROW)
	981+97.92	380.53	989+32.83	375.01	-0.008	CLASS B	0	2.53	4	4	0.035	107	4.21	1.18	2.10	19	DA-18 (UP TO ROW)
SPL_D_LT_8	989+32.83	375.01	994+30.80	367.55	-0.015	ROCK RIPRAP	0	2.98	4	4	0.025	329	9.30	2.79	5.00	19	DA-18 (UP TO ROW)
	994+30.80	367.55	996+50.00	365.90	-0.008	CLASS B	0	2.93	4	4	0.035	163	4.65	1.37	2.10	19	DA-18 (UP TO ROW)
	998+90.00	367.89	1007+69.21	371.15	0.004	CLASS B	0	1.26	31	4	0.035	53	1.90	0.29	2.10	34	PR DA-19.1T
SPL_D_LT_9	1007+69.21	371.15	1008+44.69	373.41	-0.004	CLASS B	0	3.49	4	4	0.035	179	3.67	0.81	2.10	34	PR DA-19.1T
	1008+44.69	373.41	1008+94.69	371.91	-0.004	CLASS B	0	2.70	4	4	0.035	90	3.09	0.62	2.10	34	PR DA-19.1T
SPL_D_LT_10	1009+20.03	371.61	1018+60.99	374.95	0.004	CLASS B	0	3.05	4	4	0.035	122	3.29	0.68	2.10	34	PR DA-19.1T
	1018+68.83	374,95	1020+30.21	375.76	0.005	CLASS B	35	2.11	6	6	0.035	422	4.20	0.66	2.10	50	PR DA-19T
SPL_D_LT_11	1020+30.21	375.76	1021+60.44	377.25	0.012	ROCK RIPRAP	0	3.01	4	4	0.025	297	8.20	2.16	5.00	50	PR DA-19T
	1022+62.57	378.43	1023+63.92	379.59	0.015	ROCK RIPRAP	0	2.97	4	4	0.025	329	9.30	2.78	5.00	50	PR DA-19T
	1023+63.92	379.59	1026+74.76	381.74	0.007	CLASS B	0	2.13	4	4	0.035	65	3.60	0.92	2.10	50	PR DA-19T
	1026+74.76	381.74	1038+66.25	395.68	0.012	CLASS B	0	2.65	4	4	0.035	153	5.43	1.94	2.10	50	PR DA-19T
SPL_D_LT_12	1038+66.25	395.68	1043+41.25	397.15	0.003	CLASS B	0	2.67	4	4	0.035	80	2.81	0.52	2.10	2	PR DA-20T
	1043+41.25	397.15	1044+04.07	396.81	-0.005	CLASS B	0	2.29	4	4	0.035	70	3.37	0.78	2.10	2	PR DA-20T
	1044+04.07	396.81	1050+50.73	395.51	-0.002	CLASS B	0	2.71	4	4	0.035	67	2.28	0.34	2.10	2	PR DA-20T
	1050+52.65	395.51	1054+01.38	396.04	0.001	CLASS B	0	2.71	4	4	0.035	58	1.97	0.25	2.10	2	PR DA-20T
SPL_D_LT_13	1054+01.38	396.04	1060+22.98	393,86	-0.003	CLASS B	0	1.89	4	4	0.035	34	2.37	0.41	2.10	26	DA-21 & PR DA-21T
	1060+24.90	393.86	1061+62.32	394.34	0.004	CLASS B	0	2.07	4	4	0.035	43	2.52	0.45	2.10	16	DA-21 & PR DA-21T
SPL_D_LT_14	1061+62.32		1066+36.49	395.05	0.002	CLASS B	0	1.59	4	4	0.035	14	1.38	0.15	2.10	13	DA-21 & PR DA-21T

## NOTES:

- 1. THE DESIGN 5-YEAR FLOW USED FOR DITCH ANALYSIS IS BASED ON PEAK FLOW CALCULATIONS OF CONTRIBUTING DRAINAGE AREAS. REFER TO THE RUNOFF COMPUTATION SHEET FOR ADDITIONAL INFORMATION.

- DITCH COMPUTATIONS ARE BASED ON PROPOSED DITCH GEOMETRY NEEDED FOR SUP GRADING WITHIN ROW. DITCH GEOMETRIES WERE NOT OPTIMIZED FOR A 5-YR DESIGN FLOW.

3. SEE DITCH SUMMARY TABLE FOR VERTICAL POINT OF INTERSECTION FOR RIGHT DITCH. 4. DITCHES LINED WITH RIPRAP WILL BE CONSTRUCTED IN CSJ 0046-04-062.



TBPE REGISTRATION NO. F-16341 ©2023 Texas Department of Transportation

VARIOUS

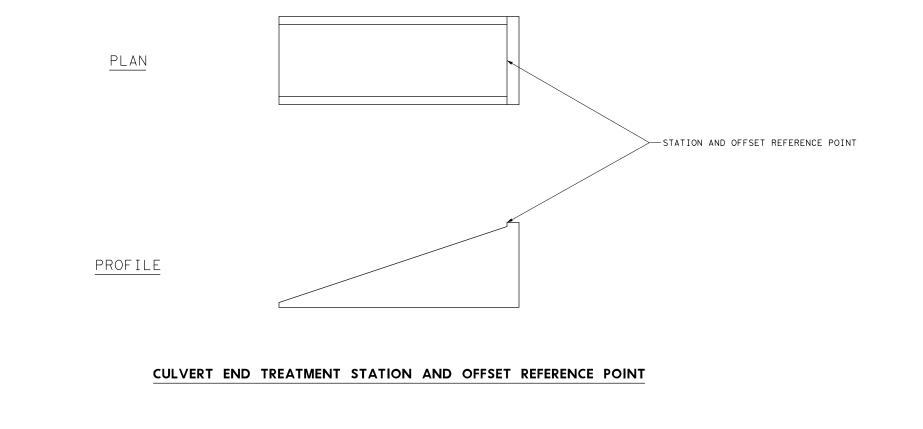
## DITCH COMPUTATIONS

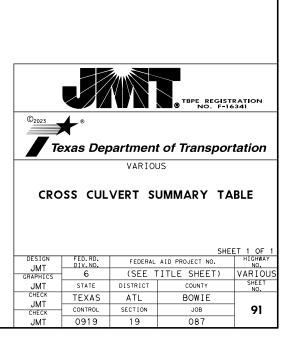
			SHEE	T 1 OF 1
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	90
JMT	0919	19	087	

## CSJ 0919-19-087

		L	JPSTREAM			BREAK		DO	WNSTREAM					
€ US82TRAIL ALIGNMENT STATION	CROSS CULVERT ID	CUS82TRAIL STATION	OFFSET	FL	CUS82TRAIL STATION	OFFSET	FL	C US82TRAIL STATION	OFFSET	FL	CULVERT SIZE	RC PIPE (CL III) (18 IN)	RC PIPE (CL III) (24 IN)	RC PIPE (CL III) (36 IN)
905+25.08	15T	905+25.08	10.31 RT	406.50	-	-	-	905+25.08	10.14 LT	406.40	1-24 IN RCP		22	
936+77.56	16T	936+77.56	10.04 LT	402.17	-	-	-	936+77.56	10.74 RT	402.14	1-18 IN RCP	22		
1009+07.47	19.1T	1009+07.47	9.59 RT	372.94	-	-	-	1009+07.47	10.75 LT	372.31	2-24 IN RCP		44	
1018+64.91	19T	1018+64.91	8.46 RT	376.23	1018+64.91	01.48 RT	374.95	1018+64.91	14.11 LT	374.94	2-36 IN RCP			48
1050+51.69	20T	1050+51.69	8.59 RT	396.24	1050+51.69	3.61 RT	395.60	1050+51.69	11.25 LT	395.51	1-18 IN RCP	20		
1060+23.94	21 T	1060+23.94	9.87 LT	393.86	-	-	-	1060+23.94	11.64 RT	393.57	3-18 IN RCP	66		
PROJECT TOT	AL.											108	66	48

US 82	SHARED	USE PATH -	PROPOSED C	ROSS CULVER	RT SUMMARY	TABLE
© US82TRAIL ALIGNMENT STATION	CROSS CULVERT ID	HEADWALL (CH - PW - O) (DIA= 24 IN)	(18 IN)	SET (TY II) (24 IN) (RCP) (4:1) (C)	(36 IN)	(36 IN)
905+25.08	15T	-	-	2	-	-
936+77.56	16T	-	2	-	-	-
1009+07.47	19.1T	1	-	2	-	-
1018+64.91	19T	-	-	-	2	2
1050+51.69	20T	-	2	-	-	-
1060+23.94	21 T	-	6	-	_	-
PROJECT TOT.	AL	1	10	4	2	2





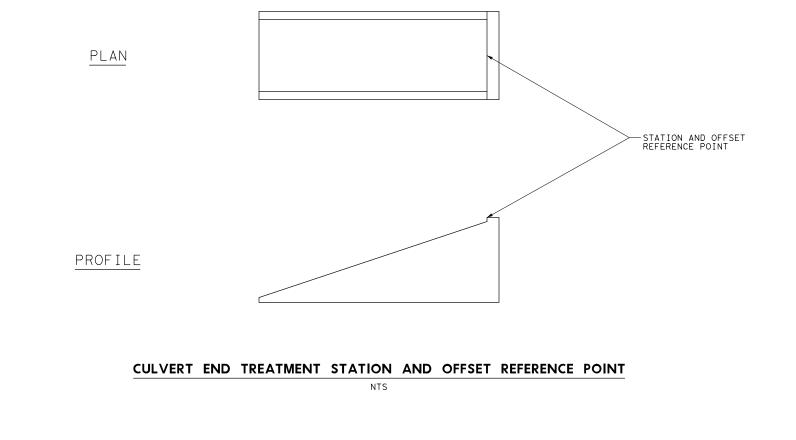


7/6/2023

## CSJ 0919-19-087

DATE: 7/6/2023 FILE NAME: pw:`

(													
			US	82 SHARED	USE PATH -	PROPOSED CR	OSS STREET	AND DRIVEWA	Y CULVERTS	SUMMARY TAB	LE		
© US82TRAIL			UPSTREAM			DOWNSTREAM			TRENCH	RC PIPE	RC PIPE	SET TY(II)	SET TY(II
AL IGNMENT STATION		CUS82TRAIL STATION	OFFSET (FT)	FL (FT)	€ US82TRAIL STATION	OFFSET (FT)	FL (FT)	CULVERT SIZE	EXCAVATION PROTECTION (FT)	(CL III) (18 IN)	(CL III) (24 IN)	(18 IN) (RCP) (6:1) (P)	
932+57	D-16T-1	932+23.69	16.76 LT	402.99	932+90.46	16.77 LT	402.87	1-24 IN RCP			68		
932+45	D-16T-2	931+91.25	15.53 RT	403.11	932+98.14	16.75 RT	402.90	1-18 IN RCP		108		2	
955+74	D-16T-3	955+52.36	17.81 LT	399.98	955+95.16	18.14 LT	399.84	1-24 IN RCP			44		
955+75	D-16T-4	955+49.69	17.26 RT	400.14	956+01.24	15.81 RT	399.87	1-18 IN RCP		52		2	
969+71	D-17T-1	969+38.19	18.15 LT	387.53	970+04.32	18.46 LT	386.97	1-24 IN RCP			68		
969+67	D-17T-2	968+99.97	16.96 RT	388.12	970+34.14	16.86 RT	387.04	1-18 IN RCP		136		2	
981+82	D-18-1	981+66.84	19.18 LT	380.75	981+97.79	18.92 LT	380.52	1-24 IN RCP			32		
981+86	D-18-2	981+71.07	15.12 RT	381.22	982+01.66	14.91 RT	380.99	1-18 IN RCP		32		2	
1022+12	D-19T-1	1021+60.45	27.31 LT	378.43	1022+62.61	26.39 LT	377.25	1-24 IN RCP			104		
1022+07	D-19T-2	1022+48.89	27.99 RT	378.26	1021+64.36	25.42 RT	377.94	1-24 IN RCP	86		86		2
PROJECT TOTA	AL.					· · · · · · · · · · · · · · · · · · ·			86	328	402	8	2



SET TY(II) (24 IN)(RCP) (6:1)(P)
2
2
2
2
2
10



7/6/2023



VARIOUS

## CROSS STREET AND DRIVEWAY CULVERT SUMMARY TABLE

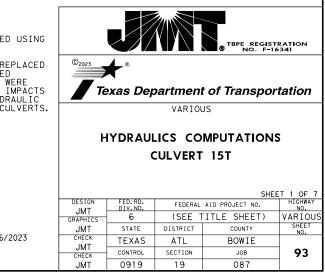
				SHEE	
Γ	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
┢	GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
ſ	снеск ЈМТ	TEXAS	ATL	BOWIE	
┢	CHECK	CONTROL	SECTION	JOB	92
	JMT	0919	19	087	

				HY	-8 CULVERT AN EXISTING C												HY	-8 CULVERT A PROPOSED (			Г				
	SUMMARY OF	CULVER	T FLOWS	AT CROS	SING				CULVE	RT NOT	ES			SUMMARY OF	CULVER	T FLOWS	AT CROS	SING				CULVE	RT NOTES	S	
H₩ Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations								HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations							
408.15	5 YR	14	2	12	9	_					T flows. een on ru		408.93	5 YR	14	14	0	1			ulvert 1 alculat				
408.17 408.19	25 YR 100 YR	19 22	3	16 18	2	_			omputat				409.38	25 YR 100 YR	19 22	19 22	0	25	-				ons she		
	Overtopping	0	0	0	Overtopping									)vertopping	22	22	0	Overtopping	,						
					CULVERT SUM	MARY T	ABLE											CULVERT SUN	- MARY T	ABLE					
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	HW Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)		Depth	Outlet Velocity (ft/s)	y Velocity	Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	- H₩ Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Den+b	Critica Depth (ft)	Outlet Depth (ft)	Depth V	Outlet 'elocity (ft/s)	T Velc (f†
5 YR	14	2	408.15		1.61		0.51	0.53		2.00	0.75	0.00	5 YR	14	14	408.93		2.43		1.38	1.34	2.00		4.46	0.
25 YR 100 YR	19 22	3	408.17		1.63		0.60	0.63		2.00	1.04	0.00	25 YR 100	<u>19</u> 22	19 22	409.38		2.88 3.17	_	2.00	1.57	2.00		6.05 6.87	0. 0.
	Crest Lengt Crest Elevat Roadway Sur Coadway Top W	ion: 4 face:	08.02 f Gravel	°+	Site Data Op	SITE C	gth: 25 ATA Culver		Culv Bar	rrel St rrel St rel Dic I Mate	lope: 0.0 VERT DAT nape: Ci ameter: C steel Steel	A	Ci	Crest Leng rest Eleva Roadway Su adway Top	tion: 4 urface:	109.84 f Paved		Site Data O	SITE C	gth: 22 NATA Culver		Barr	ert Slo CULVE rel Sha el Diam	pe: 0.00 ERT DATA	
	TAILWA er Channel O Enter Consta	•			Inlet S Inlet Ele Outlet S Outlet El Number	evatior Statior evatio	406. 25.6	54 ft 3 ft 12 ft	Barre Cul Inle	el Manr Ivert 1 t Conf Edge	Type: St	: 0.0240 traight n: Thin ing		TAILW r Channel nter Const	•			Inlet El Outlet Outlet El	evatior Statior evatio	: 22.(	.50 ft )0 ft .40 ft	Barre Cul Inlet	l Manni vert Ty Configu Edge wid	t: 0.00 ng's n: pe: Str uration: th Headw ession:	0.0 aigh Squ all

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DATE: 7/6/2023 FILE NAME: pw:`





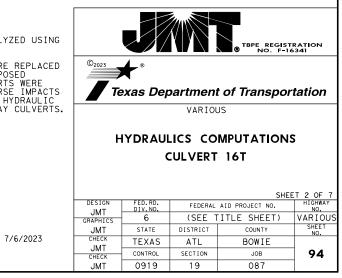
CULVERT HYDRAULICS WAS ANALYZED USING FHWA'S HY-8 VERSION 7.50.

7/6/2023

				HY	-8 CULVERT A EXISTING C			Г									HY	-8 CULVERT AN PROPOSED C			Т				
	SUMMARY OF	CULVER	FLOWS	AT CROS	SING				CULV	ERT NOT	ES			SUMMARY OF	CULVER	T FLOWS	AT CROS	SING	]			CULVER	T NOTE:	5	
HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations								HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations							
04.22	5 YR	32	5	27	6	_						rom DA-16 tions may	405.01	5 YR	25	12	13	7	-			uses comb T flows.			
04.28	25 YR 100 YR	43 52	5	38 46	3	_					tations		405.06	25 YR 100 YR	<u>34</u> 41	12 12	22 29	3	-			runoff			
	Overtopping	4	4	0	Overtopping	3							405.10         100 YR         41         12         29         3           404.89         Overtopping         12         12         0         Overtopping												
					CULVERT SUN	 MMARY ⁻	TABLE											CULVERT SUM	MARY TA	ABLE					
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	HW Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type		Critica Depth (ft)	Depth	t TW Depth (ft)		TW y Velocity (ft/s)	Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	HW Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)			Depth V	Outlet /elocity (ft/s)	T\ Veloo (ft
5 YR	32	5	404.22		1.67		f 1.01	0.84		1.50		0.00	5 YR	25	12	405.01		2.80		1.50	1.32			6.83	0.
25 YR 00 YR	43 52	5	404.28		1.73		f 1.05 f 1.14	0.86		1.50		0.00	25 YR 100 YR	<u> </u>	12	405.06		2.84		1.50	1.32			6.92 6.99	0.0
	Crest Lengt Crest Elevat Roadway Sur oadway Top V	ion: 4 face:	04.00 ft Gravel		Culve	ert Ler SITE	ngth: 28	17 ft,		vert S	Ope: 0.0			Crest Leng Crest Elevat Roadway Su Roadway Top '	tion: 4	05.04 f [.] Gravel			rt Leng <u>SITE D</u>	)th: 22	.00 ft,	402.14 ´f- Culve	rt Slo	pe: 0.00 ERT DATA	
					Site Data O; Inlet S Inlet Ele Outlet S Outlet El Number	Dat Statio evatio Statio levatio	ta n: 0.00 n: 402. n: 28.1	) f+ 55 f+ 7 f+ .26 f+	+ Barre Barre Barr Cu Inle	rel Di el Mate Embedma el Manu ilvert et Conf Edge	Steel ent: 0.0 ning's n: Type: St	1.50 ft orrugated 00 in 0.0240 traight n: Thin						Site Data Op Inlet S Inlet Ele Outlet S Outlet Ele Number	Dato tation vation tation	0.00 402. 22.0	) f+ 17 f+ 0 f+ .14 f+	t Barre Barre Barrel Culv Inlet	el Diam I Mater Mannin vert Typ Configu dge wit	pe: Cir heter: 1 rial: C ng's n: pe: Str uration: th Headw ession:	.50 oncre ) in 0.0 ⁻ aigh Squ all
ailwa+		TER DAT	Α		-								Tailwat		ATER DAT	A		-							
	er Channel C Enter Consta		water											er Channel ( Enter Consta		water									

NOTES:

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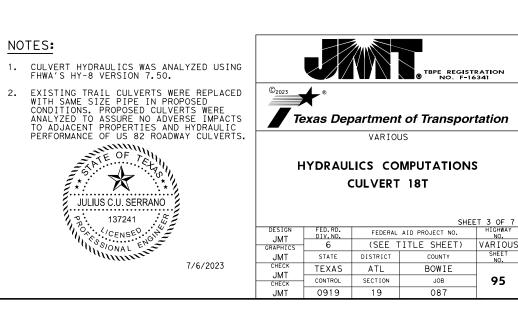
CULVERT HYDRAULICS WAS ANALYZED USING FHWA'S HY-8 VERSION 7.50.



				HY	-8 CULVERT AN EXISTING CO			-				
	SUMMARY OF		TELOWS	AT CROS		1				RT NOT	EC	
HW Elev (ft)	Storm Freq.		Culvert Q (cfs)		Iterations	-					<u>L3</u>	
370.97	5 YR	891	171	720	10	-					flows fro flows. F	
372.05	25 YR	1681	193	1488	4	]	c	alculati	ons may	/ be se	een on ru	noff
372.94	100 YR	2444	209	2235	3			cc	mputat	ions st	neets.	
369.16	Overtopping	107	107	0	Overtopping							
					CULVERT SUM	MARY TA	BLE					
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	HW Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)	Outlet Depth (ft)		Outlet Velocity (ft/s)	TW Veloci (ft/s
5 YR	891	171	370.97	7.34	7.23	7-JA2c	-1.00	3.74	3.74	0.00	10.55	0.00
25 YR	1681	193	372.05	8.42	7.84	7-JA2c	-1.00	3.97	3.97	0.00	11.26	0.00
	ROADM Profile Shc Crest Lengt Crest Elevat	h: 100	onstant 0.00 ft	5	Inlet Ele Culver	evation t Leng	i (inve ir)	traight ( rt): 363, vert): 3 14 ft,	63 ft, 64.32	OL ft	utlet Elev ope: -0.0	
	Roadway Sur		Gravel	1		SITE D				CUIL	VERT DATA	
F	Roadway Top W	/idth:	26.71 f	Ŧ	Site Data Op Inlet S Inlet Ele Outlet S Outlet El	Datc tation vation tation	0.00 363. 33.1		Barre Barre Barre Cul	el Dic I Mate Mbedme I Manr vert 1	meter: : rial: Cc Steel	orrugate 0 in 0.024 raiaht

## **EXISTING STRUCTURE TO REMAIN**

NOTES:



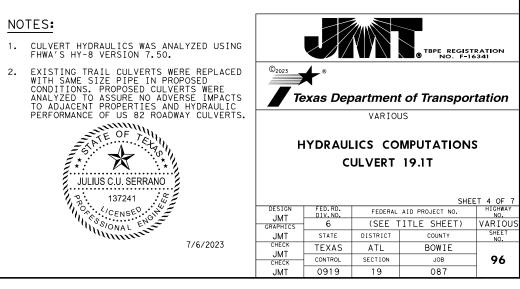
FILE NAME: pw:

				HY-	8 CULVERT AN	ALYSIS	REPORT
					PROPOSED CO	NDITIC	DNS
	SUMMARY OF	CULVER	F FLOWS	AT CROS	SING		CULVERT NOTES
HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations		
375.43	5 YR	34	34	0	1		Culvert 19.1T uses PR DA-19.1T flows. Flow
376.02	25 YR	46	42	4	14		calculations may be seen on runoff computations sheets.
376.09	100 YR	56	43	13	4		
375.96	Overtopping	41	41	0	Overtopping		
					CULVERT SUMM	IARY TA	BLE

					CULVERI SUM		ADLE					
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	H₩ Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)			Outlet Velocity (ft/s)	TW Velocity (ft/s)
5 YR	34	34	375.43	2.48	2.16	5-S2n	0.87	1.48	1.08	2.00	9.52	0.00
25 YR	46	42	376.02	3.07	2.57	5-S2n	0.98	1.64	1.23	2.00	10.06	0.00
100 YR	56	43	376.09	3.14	2.61	5-S2n	0.99	1.65	1.24	2.00	10.12	0.00

## ROADWAY DATA Roadway Profile Shape: Constant Roadway Crest Length: 100.00 ft Crest Elevation: 375.96 ft Roadway Surface: Paved Roadway Top Width: 10.00 ft Site Do ١r Inle 0u1 0u†l TAILWATER DATA Tailwater Channel Option: Enter Constant Tailwater Constant Tailwater Elevation: 374.31 ft

NOTES:





## NO EXISTING STRUCTURE

DATE: 7/6/2023 FIIE NAME: pw: \

Straight Culvert Inlet Elevation (invert): 372.94 ft, (invert): 372.31 ft Outlet Elevation Culvert Length: 22.00 ft, Culvert Slope: 0.0287

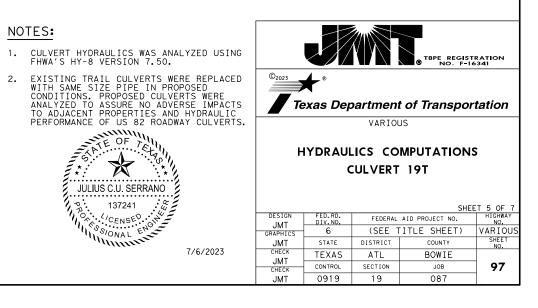
SITE DATA CULVERT DATA
Barrel Shape: Circular Barrel Diameter: 2.00 ft Barrel Material: Concrete Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight Inlet Configuration: Square Edge with Headwall Inlet Depression: None

				HY	-8 CULVERT AN EXISTING C			-									HY	-8 CULVERT AN PROPOSED C			Т				
	SUMMARY OF	CULVER	F FLOWS	AT CROS	SSING				CULV	ERT NOT	ES			SUMMARY OF	CULVER	T FLOWS	AT CROS	SSING				CULVE	RT NOT	ES	
HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations								HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations							
377.10	5 YR	78	78	0	1			alculati	ions mo	iv be s	een on r	vs. Flow Tunoff	378.50		50	50	0	1	-		alculat	ions ma	v be se	9T flows en on ru	
577.39 577.49	25 YR 100 YR	105 127	90 93	<u>15</u> 33	7 4	-		C	omputat	fions s	heets.	dite i i	379.1 379.58		69 84	69 84	0	1		·	c	omputat	ions sr	neets.	
	Overtopping	84	84	0	Overtopping									Overtopping		103	0	Overtopping							
					CULVERT SUM	MARY T	ABLE											CULVERT SUM	JLVERT SUMMARY TABLE						
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	H₩ Elev (ft)	Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Deeth	Critica Depth (ft)	Depth	n∣Depth	Outlet Velocit (ft/s)	ty Veloci-	, Storm Freq.	Total Q (cfs)	Culver t Q (cfs)	- HW Elev (ft)	Inlet Contro Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critica Depth (ft)	Depth	- TW Depth (ft)	Outlet Velocity (ft/s)	y Velo (ft
5 YR	78	78	377.10		<b>3.</b> 86 <b>4.</b> 15		3.00	2.03		3.00		0.00	5 YR	50	50	378.56		2.02	1-JS1f		1.13		3.00	3.54	0.
25 YR	5 YR10590377.393.86DO YR12793377.494.03						3.00	2.18 2.22		3.00		0.00	25 YR 100 YF		69 84	379 <b>.</b> 11 379 <b>.</b> 58		2.29 2.57	1-JS1f 5-S2n		1.13		3.00	4.88	0.0
oadway Profile Shape: Constant Roadway Crest Length: 100.00 ft Crest Elevation: 377.24 ft Roadway Surface: Gravel Roadway Top Width: 26.00 ft				Culver		(ir gth: 33	45 ft,	373.20	ft vert S	lope: 0. VERT DA		Nonstant BradwayRoadway Profile Shape: Constant RoadwaySingle Broken-back CulvertCrest Length: 100.00 ftInlet Elevation (invert): 376.23ft,Crest Elevation: 380.11 ftBreak Elevation (invert): 374.95 ft,Crest Elevation: 380.11 ftOutlet Elevation (invert): 374.94 ftRoadway Surface: PavedCulvert Length: 24.00 ft,Roadway Top Width: 10.00 ftSteep Culvert Section Slope: 0.1832							832						
I.															10.00 1	•		Steep SITE DA		ert Sect	ion Slo	·	/ERT DAT		
TAILWATER DATA Tailwater Channel Option: Enter Constant Tailwater Constant Tailwater Elevation: 376.20 ft			-	Date Station evation Station evation	a n: 0.00 n: 373. n: 33.4	) f+ 24 f+ 5 f+ 20 f+	t Bar Barı Barr Cu Inle	rel Di rel Mat Embedm el Man lvert t Confi Edge w	erial: ent: 0 ning's r Type: S guration vith Head	3.00 ft Concrete 00 in 1: 0.0120 Straight n: Sauar	Tailwa	TAILWA ter Channel C Enter Consta nt Tailwater	int Tail	lwater		Site Data Op Inlet S Inlet Ele Break S Break Ele Outlet S Outlet El Number	otion: Data Station: evation: Station: evation: Station:	Culver 0.00 376. 6.98 374. 24.0 : 374	0 ft 23 ft 8 ft 95 ft 00 ft 94 ft	Barr Up t Lov E Uppe Lowe Cu Inlet	rrel Sh rel Dia per Sec co wer Sect cr Sect ( ulvert Bro Confic Edge wi	ape: Ci meter: tion Mat oncrete nt: 0.0 ion Mann 0.0120 Type: S ken-back guration: th Heady ression:	rcular 3.00 t terial: terial: ing's ing's ing's ingle : Squ wall		

NOTES:

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SED CONDITIONS	RT	ANALYSIS	REPORT
	SED	CONDITIO	NS



	SUMMARY OF	CULVER	FLOWS	AT CROS	SING
HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterations
396.96	5 YR	2	2	0	1
397.16	25 YR	3	3	0	1
397.16	100 YR	3	3	0	1
398.62	Overtopping	11	11	0	Overtopping
					,
					CULVERT SUMM

					CULVERT SUM	MARY TA	BLE					
Storm Freq.	Total Q (cfs)	Culver t Q (cfs)		Inlet Control Depth (ft)	Outlet Control Depth (ft)	Flow Type	Normal Depth (ft)	Critical Depth (ft)			Outlet Velocity (ft/s)	TW Velocity (ft/s)
5 YR	2	2	396.96	0.72	0.0*	1-S2n	0.00	0.53	0.00	0.00	6.08	0.00
25 YR	3	3	397.16	0.92	0.03	1-S2n	0.00	0.53	0.00	0.00	6.82	0.00
100 YR	3	3	397.16	0.92	0.03	1-S2n	0.00	0.55	0.00	0.00	6.82	0.00

## ROADWAY DATA Roadway Profile Shape: Constant Roadway Crest Length: 100.00 ft Crest Elevation: 398.62 ft Roadway Surface: Paved Roadway Top Width: 10.00 ft Site Da In Inle Br Brea Out Out I TAILWATER DATA Tailwater Channel Option: Enter Constant Tailwater Constant Tailwater Elevation: 395.51 ft

NO EXISTING STRUCTURE

NOTES:



DATE: 7/6/2023 FILE NAME: DW:

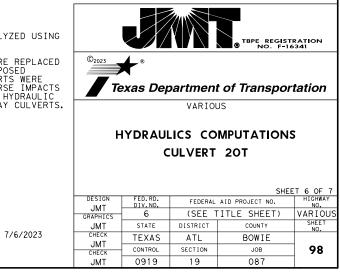
## HY-8 CULVERT ANALYSIS REPORT PROPOSED CONDITIONS

Culvert 20T uses combined flows from PR DA-20T flows. Flow calculations may be seen on runoff computations sheets.

CULVERT NOTES

Single Broken-back Culvert Inlet Elevation (invert): 396.24 ft, Break Elevation (invert): 395.60 ft, Outlet Elevation (invert): 395.51 ft Culvert Length: 20.00 ft, Upper Culvert Section Slope: 0.1286 Steep Culvert Section Slope: 0.0060

SITE DATA	CULVERT DATA
ata Option: Culvert Invert Data nlet Station: 0.00 ft et Elevation: 396.24 ft reak Station: 4.98 ft ak Elevation: 395.60 ft tlet Station: 20.00 ft et Elevation: 395.51 ft Number of Barrels: 1	Barrel Shape: Circular Barrel Diameter: 1.50 ft Upper Section Material: Concrete Lower Section Material: Embedment: 0.00 in Upper Section Manning's n: 0.0120 Lower Section Manning's n: 0.0120 Culvert Type: Single Broken-back Inlet Configuration: Square Edge with Headwall Inlet Depression: None



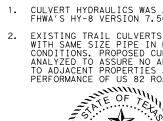
- 1. CULVERT HYDRAULICS WAS ANALYZED USING FHWA'S HY-8 VERSION 7.50.
- EXISTING TRAIL CULVERTS WERE REPLACED WITH SAME SIZE PIPE IN PROPOSED CONDITIONS. PROPOSED CULVERTS WERE ANALYZED TO ASSURE NO ADVERSE IMPACTS TO ADJACENT PROPERTIES AND HYDRAULIC PERFORMANCE OF US 82 ROADWAY CULVERTS. OF

	SUMMARY OF	CULVER1	F FLOWS	AT CROS	SING
HW Elev (ft)	Storm Freq.	Total Q (cfs)	Culvert Q (cfs)	Rdwy Q (cfs)	Iterat
395.89	5 YR	27	27	0	1
396.55	25 YR	35	35	0	37
396.63	100 YR	43	36	7	5
396.54	Overtopping	35	35	0	Overtop
					CUL VER

				COLVENT JONNANT IF				
5 YP 27 27 305 99 2 03 1 05 5-151 0 99 1 16 1 50 1 50 5 09 0			t Elev Control Q (ft) Depth	Control Flow Depth Type	Depth Depth	Depth Depth	Velocity	TW Velocity (ft/s)
5 TK   21   21   595.69 2.05   1.95   5-0311 0.69   1.16   1.50   1.50   5.09   0	5 YR	27 2	27 395.89 2.03	1.95 5-JS1f	0.89 1.16	1.50 1.50	5.09	0.00
25 YR         35         396.55         2.69         2.44         5-JS1f         1.07         1.30         1.50         1.50         6.55         00	25 YR	35 35	35 396.55 2.69	2.44 5-JS1f	1.07 1.30	1.50 1.50	6.55	0.00
100 YR 43 36 396.63 2.77 2.49 5-JS1f 1.09 1.31 1.50 1.50 6.70 0	100 YR	R 43 30	36 396.63 2.77	2.49 5-JS1f	1.09 1.31	1.50 1.50	6.70	0.00

## ROADWAY DATA Roadway Profile Shape: Constant Roadway Crest Length: 100.00 ft Crest Elevation: 396.54 ft Roadway Surface: Paved Roadway Top Width: 10.00 ft Site Da ١r Inle 0u1 Outle TAILWATER DATA Tailwater Channel Option: Enter Constant Tailwater Constant Tailwater Elevation: 395.07 ft

NOTES:



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## NO EXISTING STRUCTURE

DATE: 7/6/2023 FILE NAME: pw:/

## HY-8 CULVERT ANALYSIS REPORT PROPOSED CONDITIONS

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## CULVERT NOTES

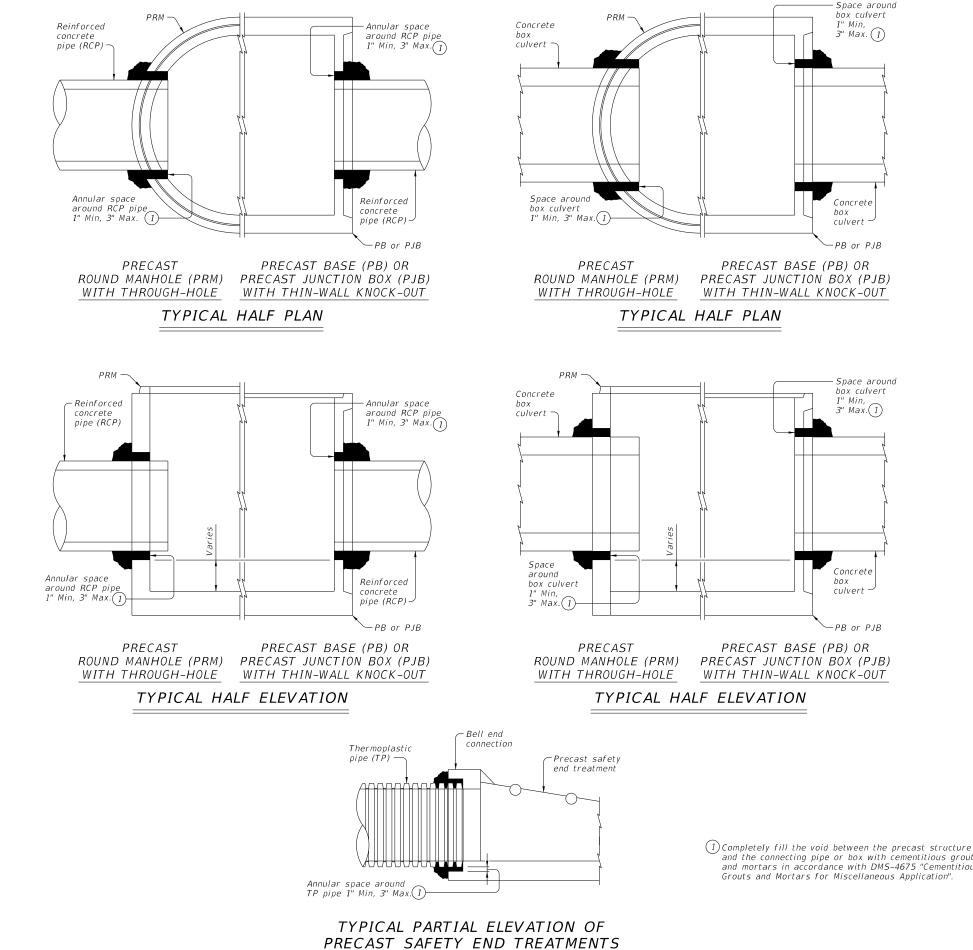
Culvert 21T uses combined flows from DA-21 and PR DA-21T flows. Flow calculations may be seen on runoff computations sheets.

## CULVERT SUMMARY TABLE

Straight Culvert Inlet Elevation (invert): 393.86 ft, (invert): 393.57 ft Outlet Elevation Culvert Length: 22.00 ft, Culvert Slope: 0.0132

SITE DATA	CULVERT DATA
ata Option: Culvert Invert Data nlet Station: 0.00 ft et Elevation: 393.86 ft tlet Station: 22.00 ft let Elevation: 393.57 ft Number of Barrels: 3	Barrel Shape: Circular Barrel Diameter: 1.50 ft Barrel Material: Concrete Embedment: 0.00 in Barrel Manning's n: 0.0120 Culvert Type: Straight Inlet Configuration: Square Edge with Headwall Inlet Depression: None

- HYDRAULICS WAS ANALYZED USING HY-8 VERSION 7.50.				B TBPE REGIST	IRATION 6341
IG TRAIL CULVERTS WERE REPLACED ME SIZE PIPE IN PROPOSED ONS. PROPOSED CULVERTS WERE D TO ASSURE NO ADVERSE IMPACTS VENT PROPERTIES AND HYDRAULIC		🗲 ® exas Dep		t of Transpor	rtation
JULIUS C.U. SERRANO	H		VARIOL LICS CO CULVERT	MPUTATION 21T	-
0	DESIGN JMT	FED. RD. DIV. NO.		SHE AID PROJECT NO. FITLE SHEET)	ET 7 OF 7 HIGHWAY NO. VARIOUS
MUNICIPAL CONTRACT	GRAPHICS JMT	STATE	DISTRICT		SHEET NO.
7/6/2023	CHECK	TEXAS	ATL	BOWIE	
	JMT CHECK	CONTROL	SECTION	JOB	99
	JMT	0919	19	087	7



Showing square PSET for parallel drainage, cross drainage shown similar.

and the connecting pipe or box with cementitious grouts and mortars in accordance with DMS-4675 "Cementitious Grouts and Mortars for Miscellaneous Application"

## CONSTRUCTION NOTES:

Do not grout rubber gasket joints without Manufacturer's recommendations.

Do not use bricks, masonry blocks, native stone, or similar materials in conjunction with grouted connections when filling void spaces around pipes or box culverts.

### MATERIAL NOTES:

Provide grouted connections in accordance with DMS-4675 "Cementitious Grouts and Mortars for Miscellaneous Application".

GENERAL NOTES: See applicable standards for notes and details not shown: Precast Base (PB)

Precast Junction Box (PJB) Precast Round Manhole (PRM)

Precast Safety End Treatments C/D Square (PSET-SC)

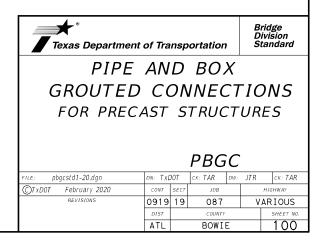
Precast Safety End Treatments P/D Square (PSET-SP)

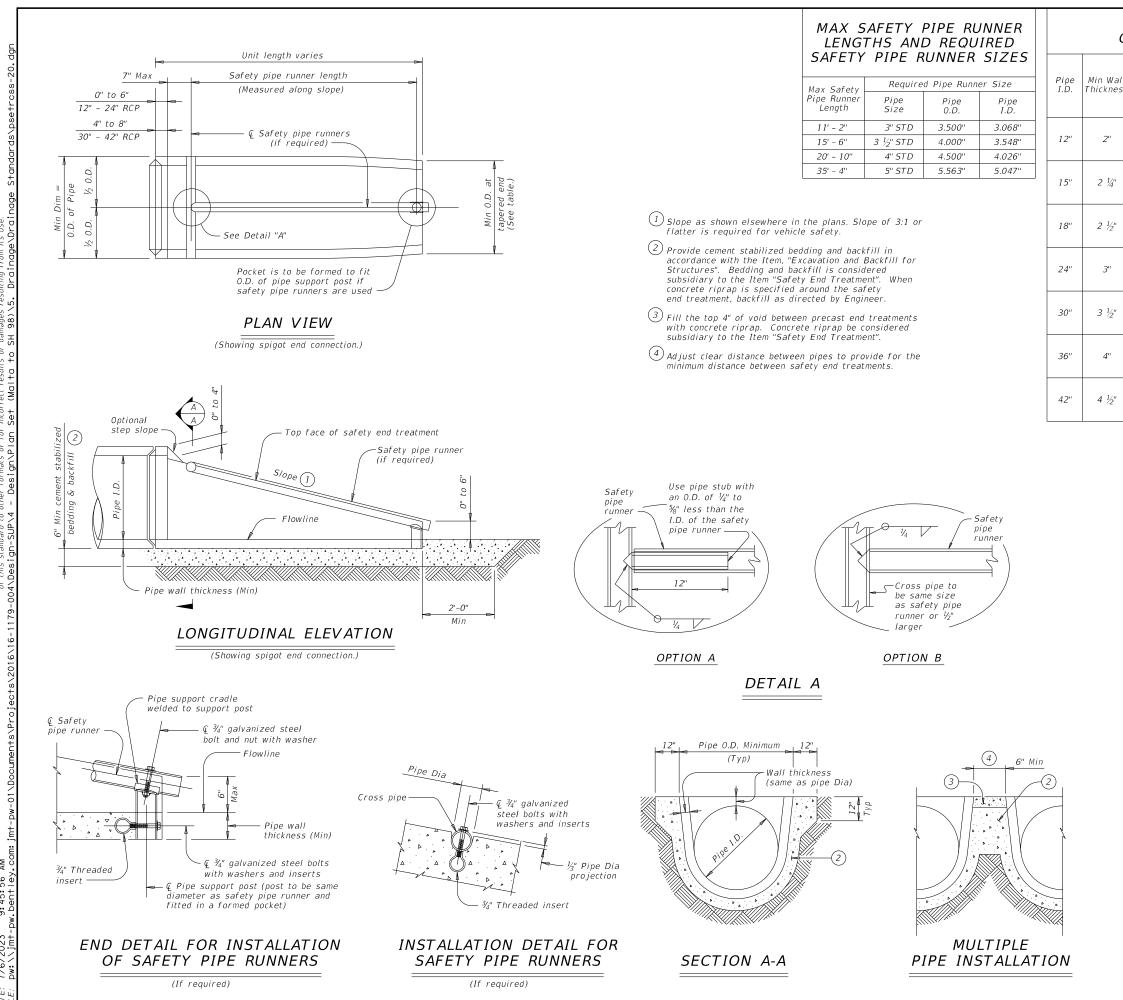
Provide Concrete Box Culverts in accordance with Item 462 "Concrete Box Culverts and Drains".

Provide Reinforced Concrete Pipe (RCP) in accordance with Item 464 "Reinforced Concrete Pipe"

Provide Thermoplastic Pipe (TP) in accordance with Special Specification Thermoplastic Pipe.

Payment for grouted connections is considered subsidiary to other bid Items.





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## REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

-									
						Single	e Pipe	Multip	le Pipe
all ess	Min 0.D.	Min O.D. at Tapered End	Min Reinf Requirements (sq. in. / ft. of pipe)	Slope	Minimum Length of Unit	Skew	Pipe Runners Required	Skew	Pipe Runners Required
				3:1	2' - 0''				
	16"	16"	0.07 Circ.	4:1	2' - 8''	$\leq 45^{\circ}$	No	$\leq 45^{\circ}$	No
				6:1	4' - 0''				
				3:1	2' - 10''				
	19 ½"	19"	0.07 Circ.	4:1	3' - 9''	$\leq 45^{\circ}$	No	≤ 45°	No
				6:1	5' - 8''				
			-	3:1	3' - 8''				
'	23"	21 ½"	0.07 Circ.	4:1	4' - 10''	<i>≤</i> 45°	No	$\leq 45^{\circ}$	No
				6:1	7' - 3''				
				3:1	5' - 3''			≤ 30°	No
	30"	27"	0.07 Circ.	4:1	7' - 0''	<u>≤</u> 45°	No	> 30°	Yes
				6:1	10' - 6''				
,	וודכ	2.1//	0.10.0	3:1	6' - 3''	<u>≤</u> 15°	No	$\leq 15^{\circ}$	No
	37"	31"	0.18 Circ.	4:1 6:1	8' - 2'' 12' - 1''	> 15°	Yes	> 15°	Yes
				3:1	7' - 10''				
	44"	36"	0.19 Ellip.	4:1	10' - 4''	$= 0^{\circ}$	No	$\geq 0^{\circ}$	Yes
	44	50	0.19 Emp.	6:1	10 - 4 15' - 4''	> 0°	Yes	20	res
				3:1	15 - 4 9' - 6''				
,	51"	41 ½"	0.23 Ellip.	4:1	9 - 0 12' - 6''	$\geq 0^{\circ}$	Yes	$\geq 0^{\circ}$	Yes
	51	41 72	0.25 Linp.	6:1	12 - 0	_0	185	_0	185
				0.1	10 - 7				

## MATERIAL NOTES:

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Provide safety pipe runners, cross pipes, pipe support posts, and pipe stubs meeting the requirements of ASTM A53 (Type E or S, Gr B), ASTM A500 Gr B, or API 5LX52.

Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

## GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (CRP) may be used for TYPE II end treatment as specified in Item 467, "Safety End Treatment".

When precast safety end treatment is used as a Contractor's alternate to mitered RCP, riprap will not be required unless noted otherwise on the plans.

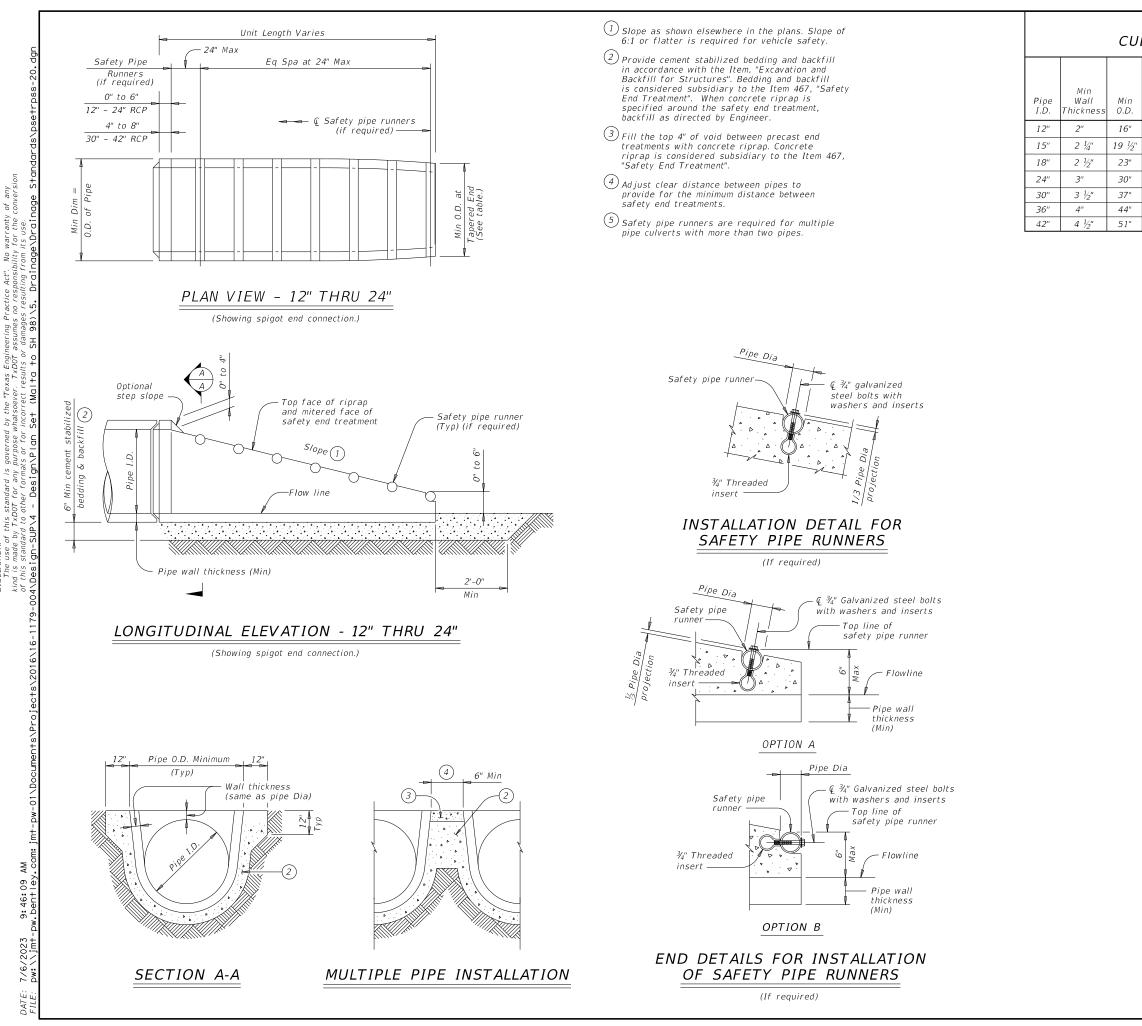
Manufacture precast concrete end sections in accordance with Item 464, "Reinforced Concrete Pipe" and in accordance with ASTM Specification C-76, Class III, Wall B for circular pipe.

Provide precast concrete end sections with a spigot or bell end for compatibility to upstream or downstream end conditions with sufficient annular space to allow for grout, mortar, cold applied asphalt joint compound or pre-formed plastic gasket material.

Methods of lifting shall be provided by the manufacturer for ease of

loading, unloading, and installation. Pipe runners are designed for a traversing load of 1,800 Lbs at yield as recommended by Research Report 280-1, "Safety Treatment of Roadside Cross-Drainage Structures", Texas Transportation Institute, March 1981.

Texas Departme	nt of Tran	sportation	D	ridge ivision tandard
PRECAST	T SA	FETY I	ΕN	D
	REAT	MENT		
TYPE II ~	CROS	'S DRAII	٧AC	ΞE
TYPE II ~	CROS	'S DRAII	VAC	ΞE
TYPE II ~		s draii SET-RC		θE
TYPE II ~				<b>БЕ</b> ск: GAF
	PS DN: RLW	SET-RC	• •	
FILE: psetrcss-20.dgn	DN: RLW	SET-RC	JTR	ск: GAF
FILE: psetrcss-20.dgn ©TxD0T February 2020	DN: RLW	SET-RC	JTR	CK: GAF HIGHWAY



## REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

Min O.D.	Min Reinf Requirements		Min		Runner ements	Required	Pipe Run	ner Sizes
at Tapered End	(sq. in. per ft. of Pipe)	Max Slope	Length of Unit	Single Pipe	Multiple Pipe	Nominal Dia	0.D.	I.D.
16"	0.07 Circ.	6:1	4' - 0''	No	5	3" STD	3.500"	3.068''
19"	0.07 Circ.	6:1	5' - 8''	No	5	3" STD	3.500"	3.068''
21 ½"	0.07 Circ.	6:1	7' - 3''	No	5	3" STD	3.500"	3.068''
27"	0.07 Circ.	6:1	10' - 6''	No	5	3" STD	3.500"	3.068"
31"	0.18 Circ.	6:1	12' - 1''	No	Yes	4" STD	4.500"	4.026"
36"	0.19 Ellip.	6:1	15' - 4''	Yes	Yes	4" STD	4.500"	4.026"
41 ½"	0.23 Ellip.	6:1	18' - 7''	Yes	Yes	4" STD	4.500"	4.026"

MATERIAL NOTES: Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Provide pipe runners meeting the requirements of ASTM A53 (Type E or S, Gr B), ASTM A500 Gr B, or API 5LX52.

Galvanize steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

### GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (RCP) may be used for TYPE II end treatment as specified in Item 467, "Safety End Treatment"

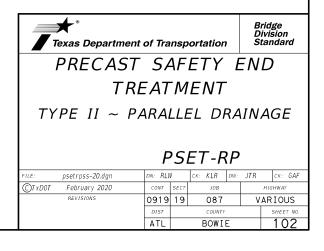
When precast safety end treatment is used as a Contractor's alternate to mitered RCP, riprap will not be required unless noted otherwise on the plans.

Manufacture precast concrete end sections in accordance with Item 464, "Reinforced Concrete Pipe" and in accordance with ASTM Specification C-76, Class III, Wall B for circular pipe. Provide precast concrete end sections with a spigot or bell end for

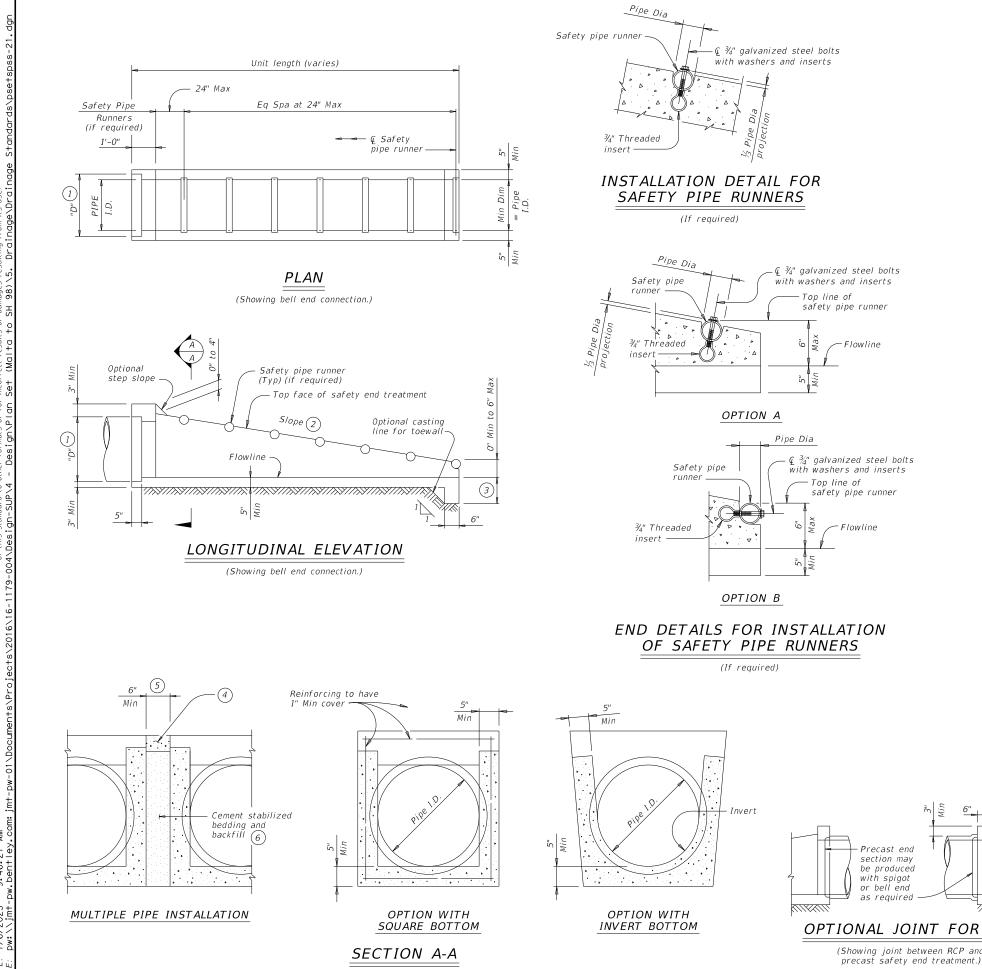
compatibility to upstream or downstream end conditions with sufficient annular space to allow for grout, mortar, cold applied asphalt joint compound or pre-formed plastic gasket material. Methods of lifting shall be provided by the manufacturer for ease of

loading, unloading and installation.

Pipe runners are designed for a traversing load of 10,000 Lbs at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981.



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Pipe	RCP Wall	TP Wall			Min		lunners uired	Required	Pipe Run	ner Size
I.D.	Thickness	Thickness	"D" 1	Slope	Length	Single Pipe	Multiple Pipe	Nominal Dia.	0.D.	I.D.
12"	2''	1.15"	17.00"	6:1	4' - 9''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
15"	2 ¼"	1.30"	20.50"	6:1	6' - 5''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068''
18''	2 ½"	1.60"	24.00"	6:1	8' - 0''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068''
24"	3"	1.95"	31.00"	6:1	11' - 3''	No	Yes, for > 2 pipes	3" STD	3.500"	3.068"
30"	3 ½"	2.65"	38.50"	6:1	14' - 8''	No	Yes	4" STD	4.500"	4.026"
36"	4"	2.75"	45.50"	6:1	17' - 11''	Yes	Yes	4" STD	4.500"	4.026"
42"	4 ½"	2.7"	52.50"	6:1	21' - 2"	Yes	Yes	4" STD	4.500"	4.026"

Precast end section may be produceo 5" Min with spiaot or bell end as required OPTIONAL JOINT FOR RCP (Showing joint between RCP and

## REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

(1) Dimension "D" is based on reinforced concrete pipe (RCP) meeting the requirements of ASTM C-76, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness used. For thermoplastic pipe (TP) take into account the annular space requirements for grouted connections.

 $^{(2)}$  Slope as shown elsewhere in the plans. Slope of 6:1 or flatter is required for vehicle safety.

3 Toewall to be used only when dimension is shown elsewhere in the plans.

(4) Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is considered subsidiary to the Item 467, "Safety End Treatment".

 $^{(5)}$  Adjust clear distance between pipes to provide for the minimum distance between safety end treatments.

(6) Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment". When concrete riprap is specified around the safety end treatment, backfill as directed by Engineer.

(7) Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

### GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment".

When precast safety end treatment is used as a Contractor's alternate to mitered RCP, riprap will not be required unless noted otherwise on the plans

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End Treatment" except as noted below

A. Provide minimum reinforcing of #4 at 6" (Grade 40) or #4 at 9" (Grade 60) each way or 6"x6" - D12 x D12 or 5"x5" - D10 x D10 welded wire reinforcement (WWR).

B. For precast (steel formed) sections, provide Class "C" concrete (f'c = 3,600 psi).

At the option and expense of the Contractor the next larger size of safety end treatment may be furnished; as long as the "D" dimension

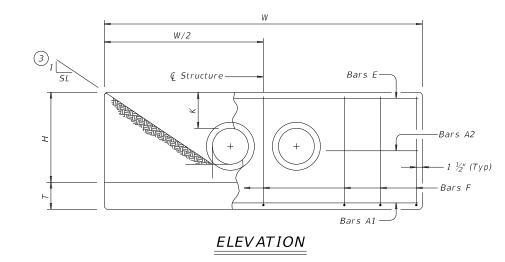
cast is that of the required size of pipe. Pipe runners are designed for a traversing load of 10,000 Lbs at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981. Provide pipe runners meeting the requirements of ASTM A53 (Type E or S, Grade B), ASTM A500 (Grade B), or API 5LX52.

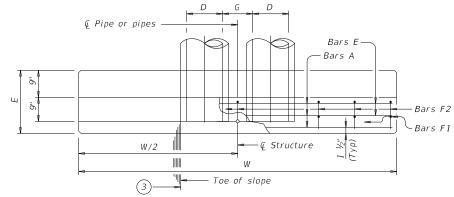
Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications. Connect RCP using the Optional Joint for RCP detail shown or in

accordance with Item 464, "Reinforced Concrete Pipe". Connect TP by grouting. See Pipe and Box Grouted Connections (PBGC) standard for grouted connections with TP and precast safety end treatment.

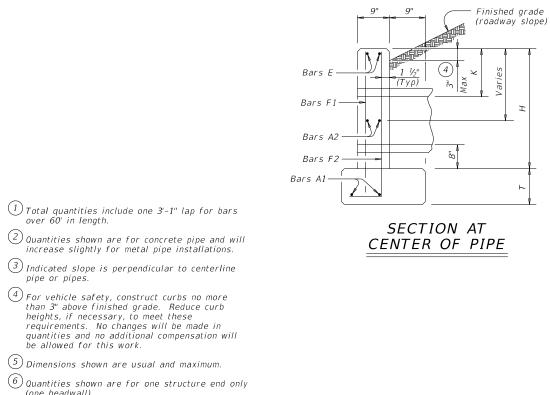
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PRECAST	SA	٩F	ETY	E	ΞN	D
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TYPE II ~ PA	ARAL	.LE	L DI	₹A	1117	AGE
TYPE II ~ PA			ET-S			AGE
TYPE II ~ PA		25				<b>А<i>GE</i></b> ск: GAF
	F	25	ET-S	5P	1	
FILE: psetspss-21.dgn	P	PS V SECT	ET - S	5P	JTR	ск: GAF
FILE: psetspss-21.dgn ©TxDOT February 2020 REVISIONS	P DN: RLW CONT	PS V SECT	<b>ЕТ-S</b> ск: КLR _{JOB}	5 <b>P</b>	JTR	ck: GAF HIGHWAY

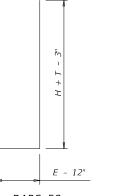
0,	Pipe )	Values	for One F	Pipe	Values for Eacl	Fo Be Ad n Addt'l F	ded Pipe
Slope	Dia of (D)	W	Reinf (Lbs)	Conc (CY)	W	Reinf (Lbs)	Cor (CY
		01 011	(1)	2	11 01	(1)	2
	12"	9' - 0''	122	1.1	1' - 9''	15	0.2
	15" 18"	10' - 3'' 11' - 6''	136 163	1.3 1.5	2' - 2'' 2' - 8''	16 19	0.2
	21"	12' - 9''	200	1.5	2 - 0	31	0.2
	24"	12 - 0"	200	2.1	3' - 7''	34	0.4
	27"	15' - 3''	254	2.4	3' - 11''	37	0.5
	30"	16' - 6''	272	2.7	4' - 4''	40	0.6
2:1	33"	17' - 9''	314	3.1	4' - 8''	43	0.6
	36"	19' - 0''	371	3.9	5' - 1''	46	0.8
	42"	21' - 6''	442	4.9	5' - 10''	52	1.0
	48''	25' - 0''	569	6.4	6' - 7''	59	1
	54"	27' - 6''	701	7.5	7' - 6''	82	1.6
	60"	30' - 0''	794	8.8	8' - 3''	90	1.8
	66"	32' - 6''	894	10.2	8' - 9''	96	2.0
	72"	35' - 0''	1,055	11.7	9' - 4''	103	2
	12"	13' - 0''	175	1.6	1' - 9''	14	0.2
	15"	14' - 9''	193	1.9	2' - 2''	17	0.2
	18''	16' - 6''	228	2.2	2' - 8''	19	0
	21"	18' - 3''	299	2.6	3' - 1''	31	0.4
	24"	20' - 0''	323	3.0	3' - 7''	33	0
	27"	21' - 9''	371	3.5	3' - 11''	37	0
l	30"	23' - 6"	415	4.0	4' - 4''	40	0
3:1	33"	25' - 3"	469	4.6	4' - 8''	43	0.0
	36"	27' - 0"	556	5.7	5' - 1''	46	0.8
	42" 48"	30' - 6'' 35' - 6''	675 837	7.1 9.2	5' - 10'' 6' - 7''	52 59	1.
	40 54''	39' - 0''	1,015	9.2	0 - 7 7' - 6''	84	1.
	60"	42' - 6''	1,015	12.9	8' - 3''	91	1.
	66"	46' - 0''	1,298	14.9	8' - 9''	98	2.
	72"	49' - 6''	1,561	17.1	9' - 4''	103	2.
	12"	17' - 0''	229	2.0	1' - 9''	15	0
	15"	19' - 3''	266	2.4	2' - 2''	17	0
	18"	21' - 6''	308	2.9	2' - 8''	19	0
	21"	23' - 9''	382	3.5	3' - 1''	31	0.
	24"	26' - 0''	430	3.9	3' - 7''	34	0.
	27"	28' - 3''	486	4.7	3' - 11''	37	0.
	30"	30' - 6''	539	5.2	4' - 4''	40	0.
4:1	33"	32' - 9''	603	6.0	4' - 8''	42	0.
	36"	35' - 0''	738	7.5	5' - 1''	47	0.
	42"	39' - 6''	881	9.3	5' - 10''	52	1.
	48''	46' - 0''	1,102	12.1	6' - 7''	61	1.
	54"	50' - 6"	1,364	14.4	7' - 6''	84	1.
	60"	55' - 0"	1,547	16.9	8' - 3''	91	1.0
	66" 7.2"	59' - 6"	1,741	19.5	8' - 9''	98	2.
	72" 12"	64' - 0'' 25' - 0''	2,077 336	22.4 3.0	9' - 4'' 1' - 9''	102 14	2 0
	12"	28' - 3''	336	3.0	2' - 2''	14	0
	15	31' - 6''	452	4.2	2 - 2 2' - 8''	17	0
	21"	34' - 9''	581	5.1	3' - 1''	31	0.4
	24"	38' - 0''	644	5.8	3' - 7''	34	0.
	27"	41' - 3''	737	6.9	3' - 11''	37	0
	30"	44' - 6''	807	7.7	4' - 4''	39	0.0
6:1	33"	47' - 9''	912	8.9	4' - 8''	44	0.
2	36"	51' - 0''	1,108	11.0	5' - 1''	48	0.
	42"	57' - 6''	1,318	13.7	5' - 10''	54	1.
	48''	67' - 0''	1,682	17.9	6' - 7''	59	1
	54"	73' - 6''	2,072	21.3	7' - 6''	83	1.
	60"	80' - 0''	2,351	24.9	8' - 3''	89	1.
	66"	86' - 6''	2,643	28.9	8' - 9''	96	2.
	72"	93' - 0''	3,121	33.1	9' - 4''	101	2.





PLAN OF NON-SKEWED PIPES





(one headwall).

BARS F2

## TABLE OF CONSTANT DIMENSIONS

Dia of Pipe (D)	G	к (5)	Н	Т	E
12"	0' - 9''	1' - O''	2' - 8''	0' - 9"	1' - 9"
15"	0' - 11''	1' - O''	2' - 11"	0' - 9"	1' - 9"
18''	1' - 2''	1' - O''	3' - 2"	0' - 9"	1' - 9"
21"	1' - 4''	1' - O''	3' - 5"	0' - 9"	2' - 0''
24"	1' - 7''	1' - O''	3' - 8''	0' - 9''	2' - 0''
27"	1' - 8''	1' - O''	3' - 11"	0' - 9"	2' - 3"
30"	1' - 10''	1' - O''	4' - 2''	0' - 9"	2' - 3"
33"	1' - 11''	1' - O''	4' - 5"	0' - 9"	2' - 6"
36"	2' - 1''	1' - 0''	4' - 8''	1' - 0''	2' - 6"
42"	2' - 4''	1' - 0''	5' - 2"	1' - 0''	2' - 9"
48''	2' - 7''	1' - 3''	5' - 11"	1' - 0"	3' - 0"
54''	3' - 0''	1' - 3''	6' - 5"	1' - 0''	3' - 3"
60"	3' - 3''	1' - 3''	6' - 11''	1' - 0"	3' - 6"
66"	3' - 3''	1' - 3''	7' - 5"	1' - 0"	3' - 9"
7 <i>2</i> "	3' - 4''	1' - 3''	7' - 11"	1' - 0"	4' - 0''

# TABLE OF6REINFORCING STEEL

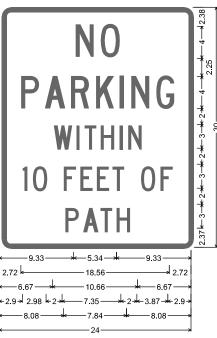
Bar	Size	Spa	No.
A1	#5	~	2
A2	#5	1' - 6"	~
Е	#5	~	2
F	#5	1' - 0''	~

MATERIAL NOTES: Provide Grade 60 reinforcing steel. Provide Class C concrete (f'c = 3,600 psi).

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications. Do not mount bridge rails of any type directly to these culvert headwalls. This standard may not be used for wall heights, H, exceeding the values shown.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing dimensions are out-to-out of bars.

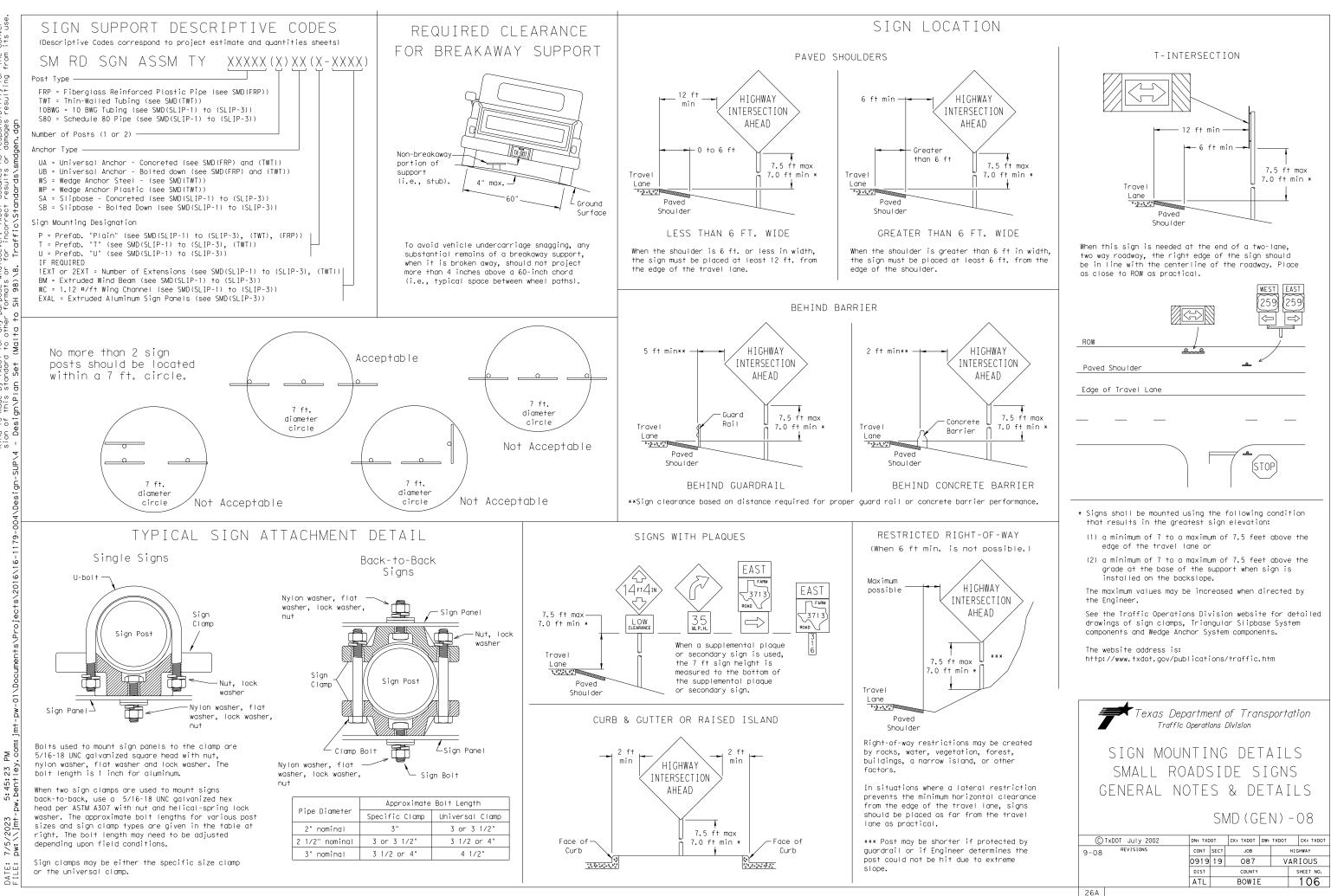
Image: Texas Department of Transportation         Bridge Division Standard							
CONCRETE HEADWALLS							
WITH PARALLEL WINGS FOR							
NON-SKEWEL	NON-SKEWED PIPE CULVERTS						
		~ / .		h			
	C	- П	'-PW-0	)			
FILE: chpw0ste-20.dgn	DN: TXL	D0T	CK: TXDOT DW:	T x D 0T	ск: ТхДОТ		
CTxDOT February 2020	CTXDOT February 2020 CONT SECT JOB HIGHWAY						
REVISIONS	0919 19 087 VARIOUS						
	DIST		COUNTY		SHEET NO.		
	ATL		BOWIE		104		



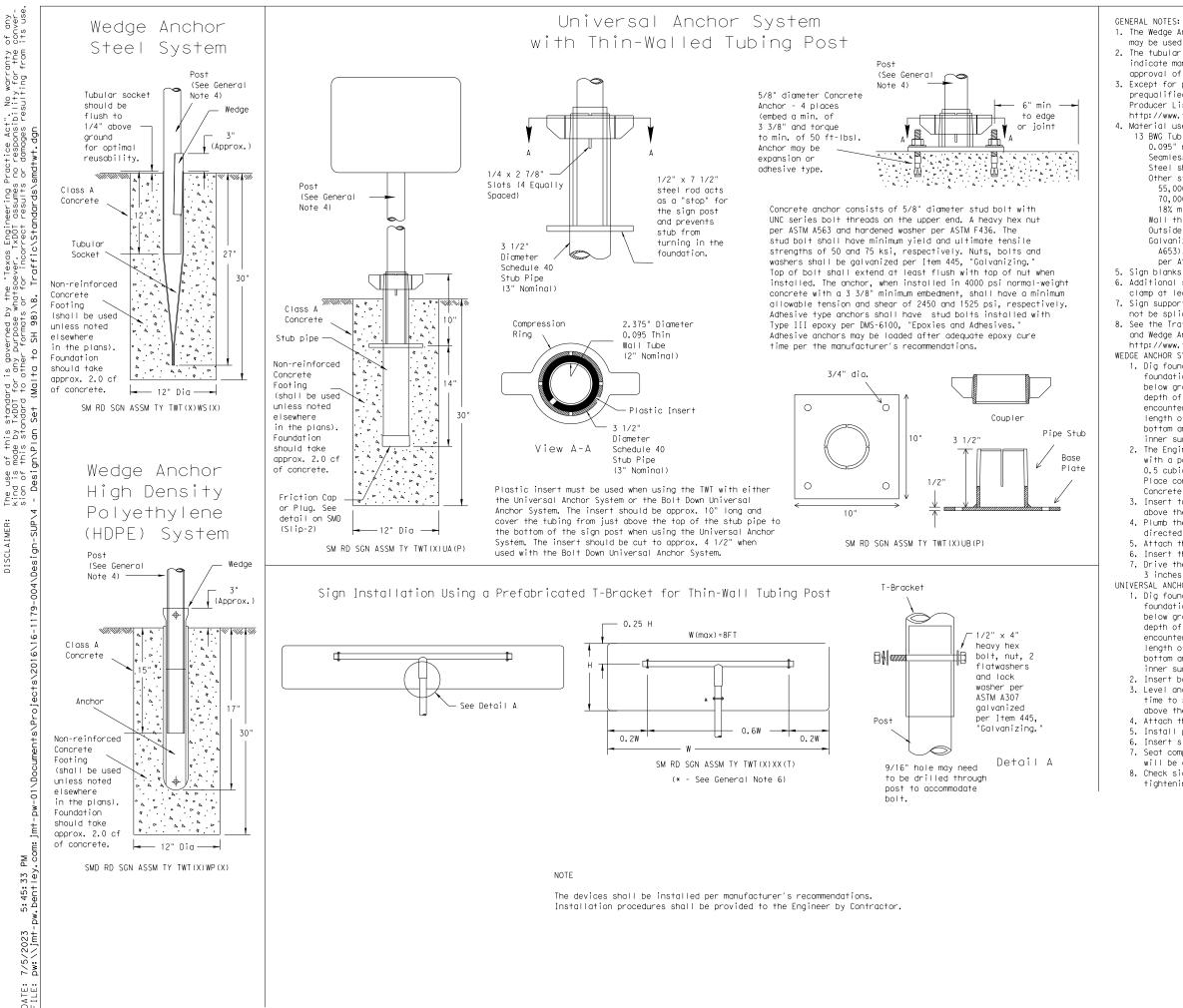
R8-2T_24x30;

1.50" Radius, 0.63" Border, 0.38" Indent, Red on, White; "NO", C; "PARKING", C; "WITHIN", C; "10", C; "FEET", C; "OF", C; "PATH", C;





of con i+s anty the from ctice Act". No warra responsibility for damages resulting f oo ro oo ro eering P assumes results Engir xDOT Soe purpose formats s gov any p other Mal Fot by TxDOT standard standard Ian Set (N of th made this gn/Pl he use ind is ion of - Desig DISCL



1. The Wedge Anchor System and the Universal Anchor System with thin wall tubing post may be used to support up to 10 square feet of sign area. 2. The tubular socket, wedge and prefabricated T-bracket shall be permanently marked to indicate manufacturer. Method, design, and location of marking are subject to the approval of the TxDOT Traffic Standards Engineer. 3. Except for posts (13 BWG Tubing), clamps, nuts and bolts, all components shall be prequalified. A list of prequalified vendors may be obtained from the Material Producer List web page. The website address is: http://www.txdot.gov/business/producer list.htm 4. Material used as post with this system shall conform to the following specifications: 13 BWG Tubing (2.375" outside diameter) (TWT) 0.095" nominal wall thickness Seamless or electric-resistance welded steel tubing Steel shall be HSLAS Gr 55 per ASTM A1011 or ASTM A1008 Other steels may be used if they meet the following: 55,000 PSI minimum yield strength 70,000 PSI minimum tensile strength 18% minimum elongation in 2" Wall thickness (uncoated) shall be within the range of .083" to .099" Outside diameter (uncoated) shall be within the range of 2.369" to 2.381" Galvanization per ASTM 123 or ASTM A653 G210. For precoated steel tubing (ASTM A653), recoat tube outside diameter weld seam by metallizing with zinc wire per ASTM B833. 5. Sign blanks shall be the sizes and shapes shown on the plans. 6. Additional sign clamp required on the "T-bracket" post for 24" high signs. Place clamp at least 3" above bottom of sign when possible. 7. Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced. 8. See the Traffic Operations Division website for detailed drawings of sign clamps and Wedge Anchor System components. The website address is: http://www.txdot.gov/publications/traffic.htm WEDGE ANCHOR SYSTEM INSTALLATION PROCEDURE 1. Dig foundation hole. Where solid rock is encountered at ground level, the foundation shall be a minimum depth of 18". When solid rock is encountered below ground level, the foundation shall extend in the solid rock a minimum depth of 18" or provide a minimum foundation depth of 30". If solid rock is encountered, the socket/stub may be reduced in length as required to a minimum length of 18". Any material removed from the socket/stub shall be from the bottom and the clearance requirements given on SMD(GEN) must be followed. The inner surfaces of the socket/stub must remain free of concrete or other debris. 2. The Engineer may permit batches of concrete less than 2 cubic yards to be mixed with a portable, motor driven concrete mixer. For small placements less than 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Place concrete into hole until it is approximately flush with the ground. Concrete shall be Class A. 3. Insert tubular socket into concrete until top of socket is approximaely 1/4 " above the concrete footing. 4. Plumb the socket. Allow a minimum 4 days for concrete to set, unless otherwise directed by Engineer.. 5. Attach the sign to the sign post. 6. Insert the sign post into socket and align sign face with roadway. 7. Drive the wedge into the socket to secure post. This will leave approximately 3 inches of the wedge exposed. UNIVERSAL ANCHOR SYSTEM INSTALLATION PROCEDURE 1. Dig foundation hole. Where solid rock is encountered at ground level, the foundation shall be a minimum depth of 18". When solid rock is encountered below ground level, the foundation shall extend in the solid rock a minimum depth of 18" or provide a minimum foundation depth of 30". If solid rock is encountered, the socket/stub may be reduced in length as required to a minimum length of 18". Any material removed from the socket/stub shall be from the bottom and the clearance requirements given on SMD(GEN) must be followed. The inner surfaces of the socket/stub must remain free of concrete or other debris. 2. Insert base post in hole to depths shown and backfill hole with concrete. 3. Level and plumb the base post using a torpedo level and allow concrete adequate time to set. The bottom of the slots provided in the stub pipe shall remain above the top of the concrete foundation. 4. Attach the sign to the sign post. 5. Install plastic insert around bottom of post. 6. Insert sign post into base post. Lower until the post comes to rest on steel rod. Seat compression ring using a hammer. Typically, the top of compression ring will be approximately level with top of stub post when optimally installed. 8. Check sign post by hand to ensure it is unable to turn. If loose, increase the tightening of the compression ring. Texas Department of Transportation Traffic Operations Division SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS WEDGE & UNIVERSAL ANCHOR WITH THIN WALL TUBING POST SMD (TWT) - 08 DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDOT ①TxDOT July 2002 REVISION CONT SECT JOB HIGHWAY 9-08

0919 19

ATL

087

BOWIE

VARIOUS

107

REG (stop, y	GULATORY	OT ENTER AND	F	REGULATO	WHITE BACKGROUND RY SIGNS _d, do not enter and y signs)
ST0	P	YIELD			
		WRONG WAY		TYPICAL	EXAMPLES
	QUIREMENTS				
SI	PECIFIC SIG	NS UNLY		SHEETING RE	QUIREMENTS
	SHEETING REQU	JIREMENTS	USAGE	COLOR	SIGN FACE MATERIAL
USAGE	COLOR	SIGN FACE MATERIAL	BACKGROUND	WHITE	TYPE A SHEETING
BACKGROUND	RED	TYPE B OR C SHEETING	BACKGROUND	ALL OTHERS	TYPE B OR C SHEETING
BACKGROUND LEGEND & BORDERS	WHITE	TYPE B OR C SHEETING TYPE B OR C SHEETING	LEGEND, BORDERS AND SYMBOLS	BLACK	ACRYLIC NON-REFLECTIVE FILM
LEGEND	RED	TYPE B OR C SHEETING	LEGEND, BORDERS AND SYMBOLS	ALL OTHER	TYPE B OR C SHEETING
			1110 01110020		
REQUIREME	INTS FOR	WARNING SIGNS		IENTS FO	R SCHOOL SIGNS
	ENTS FOR	Ś	REQUIREN	CHOOL SPEED LIMIT 20 WHEN FLASHING	R SCHOOL SIGNS
		PLES	REQUIREN	CHOOL SPEED LIMIT 20 WHEN FLASHING	EXAMPLES
	YPICAL EXAMP	PLES	REQUIREN	CHOOL PEED IMIT 20 WHEN LASHING	EXAMPLES
T V SH USAGE	YPICAL EXAMP	PLES		CHOOL PEED DIMIT 20 WHEN TASHING TYPICAL	EXAMPLES
T N SI USAGE ACKGROUND	YPICAL EXAMP	PLES REMENTS SIGN FACE MATERIAL TYPE B _{FL} OR C _{FL} SHEETING		CHOOL PEED IMIT 200 WHEN LASHING TYPICAL SHEETING REC COLOR WHITE FLOURESCENT	EXAMPLES SIGN FACE MATERIAL
T N SH USAGE ACKGROUND END & BORDERS	YPICAL EXAMP	PLES REMENTS SIGN FACE MATERIAL	REQUIREN	CHOOL PEED JMIT 200 WHEN SHEETING REC COLOR WHITE	EXAMPLES DUIREMENTS SIGN FACE MATERIAL TYPE A SHEETING

#### NOTES

be furnished shall be as detailed elsewhere in the plans and/or as n sign tabulation sheet. Standard sign designs and arrow dimensions found in the "Standard Highway Sign Designs for Texas" (SHSD).

end shall use the Federal Highway Administration (FHWA) Highway Alphabets (B, C, D, E, Emod or F).

spacing between letters and numerals shall conform with the SHSD, approved changes thereto. Lateral spacing of legend shall provide ced appearance when spacing is not shown.

gend and borders shall be applied by screening process or cut-out non-reflective black film to background sheeting, or combination

egend and borders shall be applied by screening process with transparent ink, transparent colored overlay film to white background sheeting or white sheeting to colored background sheeting, or combination thereof.

legend shall be applied by screening process with transparent colored nsparent colored overlay film or colored sheeting to background or combination thereof.

bstrate shall be any material that meets the Departmental Material cation requirements of DMS-7110 or approved alternative.

details for roadside mounted signs are shown in the "SMD series" Plan Sheets.

ALUMINUM SIGN	BLANKS THICKNESS
Square Feet	Minimum Thickness
Less than 7.5	0.080
7.5 to 15	0.100
Greater than 15	0.125

DEPARTMENTAL MATERIAL SPEC	IFICATIONS
ALUMINUM SIGN BLANKS	DMS-7110
SIGN FACE MATERIALS	DMS-8300

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website. http://www.txdot.gov/



1									
	Ι.	STORMWATER POLLUTION P	PREVENTION-CLEAN WATER	ACT SECTION 402	III.	CULTURAL RESOURCES			VI. <u>HAZARDOUS</u>
		TPDES TXR 150000: Stormwater required for projects with disturbed soil must protect Item 506. List MS4 Operator(s) that m	1 or more acres disturbed so for erosion and sedimentati	bil. Projects with any ion in accordance with		archeological artifacts are fou archeological artifacts (bones,	cations in the event historical iss and during construction. Upon discov burnt rock, flint, pottery, etc.) contact the Engineer immediately.	ery of	General (app Comply with the Ha hazardous materia making workers awa provided with pers
		They may need to be notifie				🗙 No Action Required	Required Action		Obtain and keep of used on the projec
		1.				Action No.			Paints, acids, so compounds or addi
c		2.							products which may
c. dgn		No Action Required	Required Action			1.			Maintain an adequ In the event of a
epi		Action No.				2.			in accordance with immediately. The (
<1a1		1. Prevent stormwater pollu accordance with TPDES Pe		and sedimentation in		3.			of all product sp
onmer		2. Comply with the SW3P and	I revise when necessary to co	ontrol pollution or		4.			Contact the Engin * Dead or dis
viro		required by the Engineer	•		IV.	VEGETATION RESOURCES			* Trash piles. * Undesirable
Ē.		3. Post Construction Site N	lotice (CSN) with SW3P inform the public and TCEQ, EPA or			Preserve native vegetation to t	the extent practical.		* Evidence of
SH 98)\9.		4. When Contractor project		increase disturbed soil		164, 192, 193, 506, 730, 751, 7	rruction Specification Requirements 752 in order to comply with requirem andscaping, and tree/brush removal c	ents for	Does the proje replacements ( Yes
t0	ΙI	. WORK IN OR NEAR STREA		ETLANDS CLEAN WATER		🗙 No Action Required	Required Action		If "No", then If "Yes", then
(Malta		ACT SECTIONS 401 AND				Action No.			Are the result
			filling, dredging, excavati eks, streams, wetlands or we			1.			If "Yes", the
n Set		The Contractor must adhere the following permit(s):	e to all of the terms and co	nditions associated with		1.			the notificati activities as
Design∖Plan		the forfowing perint (3).				2.			15 working day
ign∕		🗌 No Permit Required				3.			If "No", then
I.		Nationwide Permit 14 - wetlands affected)	PCN not Required (less than	1/10th acre waters or		4.			scheduled demo In either case activities and
P∖4		🗌 Nationwide Permit 14 -	PCN Required (1/10 to <1/2 (	acre, 1/3 in tidal waters)					asbestos consu
IN-SI		🗌 Individual 404 Permit R			۷.		THREATENED, ENDANGERED SPECI		Any other evide on site. Haza
04\Design-SUP\4		Other Nationwide Permit	Required: NWP#			CRITICAL HABITAT, STATE L AND MIGRATORY BIRDS.	ISTED SPECIES, CANDIDATE SPE	CIES	No Actio
04\C		Required Actions: List wate	ers of the US permit applies	s to, location in project					Action No.
179-0		and check Best Management F and post-project TSS.	Practices planned to control	erosion, sedimentation		🗙 No Action Required	Required Action		1.
16-1		1.				Action No.			2.
2016		2.				1.			3.
ts/2		3.				2.			VII. <u>Other env</u>
ojec		4.				3.			(includes r
s\Pr			ary high water marks of any	areas requiring work					No Actic
jmt-pw-01\Documents\Projects\2016\16-1			ers of the US requiring the	-		4.			Action No.
∖Dot		Best Management Practic	ces:			-	bserved, cease work in the immediat and contact the Engineer immediate।		1.
W-01		Erosion	Sedimentation	Post-Construction TSS	wc	ork may not remove active nests f	rom bridges and other structures du	ring	2.
nt-p		🗙 Temporary Vegetation	🗙 Silt Fence	Vegetative Filter Strips		re discovered, cease work in the	ated with the nests. If caves or si immediate area, and contact the	nknoies	3.
com: jr		Blankets/Matting	📉 Rock Berm		En	ngineer immediately.			
y. cc		Mulch	🗌 Triangular Filter Dike	Extended Detention Basin					-
-pw.bentley.		Sodding	Sand Bag Berm	Constructed Wetlands		LIST OF AE	BBREVIATIONS		
. ber		Interceptor Swale Diversion Dike	🔄 Straw Bale Dike 🗌 Brush Berms	Wet Basin Erosion Control Compost		Best Management Practice Construction General Permit	SPCC: Spill Prevention Control and Co SW3P: Storm Water Pollution Preventio		
1-₽w		Erosion Control Compost	Erosion Control Compost	Mulch Filter Berm and Socks	DSHS:	Texas Department of State Health Servic Federal Highway Administration			
/202 //jm			Mulch Filter Berm and Socks	Compost Filter Berm and Socks	MOA:	Memorandum of Agreement Memorandum of Understanding	TCEQ: Texas Commission on Environment TPDES: Texas Pollutant Discharge Elimi		
7/5/2023 pw:\\jmt-		Compost Filter Berm and Socks	s 🗌 Compost Filter Berm and Socks		MS4:		stem TPWD: Texas Parks and Wildlife Depart TxDOT: Texas Department of Transportat	ment	
DATE: T FILE: 1			Stone Outlet Sediment Traps	Sand Filter Systems	NOT: NWP:	Notice of Termination Nationwide Permit	T&E: Threatened and Endangered Speci USACE: U.S. Army Corps of Engineers		
ΡA					NOI:	Notice of Intent	USFWS: U.S. Fish and Wildlife Service		

TxDOT for any purpose whatsoever damages resulting from its use.

ζ'n

is made results

"Texas Engineering Practice Act". No warranty of any kind ersion of this standard to other formats or for incorrect

DISCLAIMER: The use of this standard is governed by the TXDDT assumes no responsibility for the conv

#### MATERIALS OR CONTAMINATION ISSUES

lies to all projects):

Azard Communication Act (the Act) for personnel who will be working with als by conducting safety meetings prior to beginning construction and ware of potential hazards in the workplace. Ensure that all workers are sonal protective equipment appropriate for any hazardous materials used. on-site Material Safety Data Sheets (MSDS) for all hazardous products bot, which may include, but are not limited to the following categories: blvents, asphalt products, chemical additives, fuels and concrete curing tives. Provide protected storage, off bare ground and covered, for by be hazardous. Maintain product labelling as required by the Act.

uate supply of on-site spill response materials, as indicated in the MSDS. a spill, take actions to mitigate the spill as indicated in the MSDS, th safe work practices, and contact the District Spill Coordinator Contractor shall be responsible for the proper containment and cleanup bills.

eer if any of the following are detected: tressed vegetation (not identified as normal) , drums, canister, barrels, etc. smells or odors

leaching or seepage of substances

ect involve any bridge class structure rehabilitation or (bridge class structures not including box culverts)?

No No

no further action is required. TxDOT is responsible for completing asbestos assessment/inspection.

ts of the asbestos inspection positive (is asbestos present)?

No No

en TxDOT must retain a DSHS licensed asbestos consultant to assist with ion, develop abatement/mitigation procedures, and perform management necessary. The notification form to DSHS must be postmarked at least ys prior to scheduled demolition.

n TxDOT is still required to notify DSHS 15 working days prior to any plition.

e, the Contractor is responsible for providing the date(s) for abatement d/or demolition with careful coordination between the Engineer and ultant in order to minimize construction delays and subsequent claims.

ence indicating possible hazardous materials or contamination discovered rdous Materials or Contamination Issues Specific to this Project:

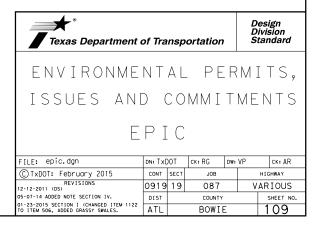
on Required Required Action

#### IRONMENTAL ISSUES

egional issues such as Edwards Aquifer District, etc.)

on Required

Required Action



#### STORMWATER POLLUTION PREVENTION PLAN (SWP3):

This SWP3 has been developed in accordance with the TPDES Construction General Permit TXR150000 (CGP). The Texas Department of Transportation (TxDOT) ensures that project specifications include adequate best management practices (BMPs) for this project.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept in the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans and the projects environmental permits, issues, and commitments (EPICs). A copy of the CGP is included in Attachment 2.12 of the SWP3 binder.

#### **1.0 SITE/PROJECT DESCRIPTION**

## **1.1 PROJECT CONTROL SECTION JOB (CSJ):**

0919-19-087

#### 1.2 PROJECT LIMITS:

From: CR 4105

#### To: SH 98

#### **1.3 PROJECT COORDINATES:**

BEGIN: (Lat) 33° 29' 15. 11 "N ,(Long) 94° 31' 16. 70 "W

END: (Lat) 33° 28′ 22.01 "N ,(Long) 94° 28′ 13.90 "W

1.4 TOTAL PROJECT AREA (Acres): 21.38

1.5 TOTAL AREA TO BE DISTURBED (Acres): 21.38

#### **1.6 NATURE OF CONSTRUCTION ACTIVITY:**

CONSTRUCTION OF SHARED USE PATH CONSISTING OF

GRADING, BASE, PAVEMENT, AND SIGNING.

#### **1.7 MAJOR SOIL TYPES:**

Soil Type	Description
Sawyer silt loam	0 to 3 percent slopes
Eylau very fine sandy loam	0 to 3 percent slopes
Blevins silt loam	1 to 3 percent slopes
Adaton-Muskogee complex	
Sardis silt loam	0 to 1 percent slopes, frequently flooded

#### **1.8 PROJECT SPECIFIC LOCATIONS (PSLs):**

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

- PSLs determined during preconstruction meeting
- PSLs determined during construction
- No PSLs planned for construction

Туре	Sheet #s
All off-ROW PSLs required by th	e Contractor are the Contractor's

responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

#### **1.9 CONSTRUCTION ACTIVITIES:**

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.5.)

- X Mobilization
- Install sediment and erosion controls
- Blade existing topsoil into windrows, prep ROW, clear and grut
- Remove existing pavement
- X Grading operations, excavation, and embankment
- Excavate and prepare subgrade for proposed pavement widenina
- Remove existing culverts, safety end treatments (SETs)
- Remove existing metal beam guard fence (MBGF), bridge rail
- Install proposed pavement per plans
- Install culverts, culvert extensions, SETs Install mow strip, MBGF, bridge rail
- I Place flex base
- ☑ Rework slopes, grade ditches
- Blade windrowed material back across slopes
- Revegetation of unpaved areas
- X Achieve site stabilization and remove sediment and erosion control measures

0ther: ____

Other:

- Other:

#### **1.10 POTENTIAL POLLUTANTS AND SOURCES:**

- Sediment laden stormwater from stormwater conveyance over disturbed area
- I Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- Solvents, paints, adhesives, etc. from various construction activities
- I Transported soils from offsite vehicle tracking
- I Construction debris and waste from various construction activities
- Contaminated water from excavation or dewatering pump-out water
- □ Sanitary waste from onsite restroom facilities
- X Trash from various construction activities/receptacles
- I Long-term stockpiles of material and waste
- □ Other: _____

□ Other:

Other:

**1.11 RECEIVING WATERS:** Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for

Tributaries	Classified Waterbody
Red Bayou	Lower Red River (0201)
Anderson Creek, Peters Branch, Austin Chapel Branch	Wright Patman Lake (0302
* Add (*) for impaired waterbodie	s with pollutant in ().
* Add (*) for impaired waterbodie 1.12 ROLES AND RESPONSI	BILITIES: TxDOT
1.12 ROLES AND RESPONSI X Development of plans and spe	BILITIES: TxDOT
1.12 ROLES AND RESPONSI X Development of plans and spe X Submit Notice of Intent (NOI) t	BILITIES: TxDOT
<b>1.12 ROLES AND RESPONSI</b> X Development of plans and spe	BILITIES: TxDOT
1.12 ROLES AND RESPONSI X Development of plans and spe X Submit Notice of Intent (NOI) t X Post Construction Site Notice	BILITIES: TxDOT
1.12 ROLES AND RESPONSI X Development of plans and specific terms of the second structure of the seco	BILITIES: TxDOT ecifications o TCEQ (≥5 acres) pdate to reflect daily operations
1.12 ROLES AND RESPONSI X Development of plans and specific terms X Submit Notice of Intent (NOI) to X Post Construction Site Notice X Submit NOI/CSN to local MS4 X Perform SWP3 inspections X Maintain SWP3 records and u X Complete and submit Notice of	BILITIES: TxDOT ecifications o TCEQ (≥5 acres) pdate to reflect daily operations f Termination to TCEQ
1.12 ROLES AND RESPONSI X Development of plans and specific terms X Submit Notice of Intent (NOI) to X Post Construction Site Notice X Submit NOI/CSN to local MS4 X Perform SWP3 inspections X Maintain SWP3 records and u X Complete and submit Notice of X Maintain SWP3 records for 3 y	BILITIES: TxDOT ecifications o TCEQ (≥5 acres) pdate to reflect daily operations f Termination to TCEQ /ears
1.12 ROLES AND RESPONSI X Development of plans and specific terms of the second structure of the seco	BILITIES: TxDOT ecifications o TCEQ (≥5 acres) pdate to reflect daily operations f Termination to TCEQ /ears
1.12 ROLES AND RESPONSI X Development of plans and specific terms of the second structure of the seco	BILITIES: TxDOT ecifications to TCEQ (≥5 acres) pdate to reflect daily operations f Termination to TCEQ /ears
1.12 ROLES AND RESPONSI X Development of plans and specific X Submit Notice of Intent (NOI) to X Post Construction Site Notice X Submit NOI/CSN to local MS4 X Perform SWP3 inspections X Maintain SWP3 records and u X Complete and submit Notice of X Maintain SWP3 records for 3 y Other:	BILITIES: TxDOT ecifications to TCEQ (≥5 acres) pdate to reflect daily operations f Termination to TCEQ /ears

#### 1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

X Submit Notice of Intent (NOI) to TCEQ (≥5 acres)

X Post Construction Site Notice

X Submit NOI/CSN to local MS4

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

X Complete and submit Notice of Termination to TCEQ

X Maintain SWP3 records for 3 years

Other:

Other:

Other:

#### 1.14 LOCAL MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) OPERATOR COORDINATION:

MS4 Entity

## **STORMWATER POLLUTION PREVENTION PLAN (SWP3)**



Sheet 1 of 2

Texas Department of Transportation

FED. RD. DIV. NO.		PROJECT NO.					
6		(SEE TITLE SHEET) 11(					
STATE		STATE DIST. COUNTY					
TEXAS	S ATL BOWIE						
CONT.		SECT. JOB HIGHWAY NO.			۷٥.		
0919 19			087	VARIO	JS		

#### STORMWATER POLLUTION PREVENTION PLAN (SWP3):

### 2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

#### 2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:

#### T / P

98) \9.

H

- $\boxtimes$   $\Box$  Protection of Existing Vegetation
- □ □ Vegetated Buffer Zones
- □ ⊠ Soil Retention Blankets
- Geotextiles
- Mulching/ Hydromulching
- Soil Surface Treatments
- 🛛 🗆 Temporary Seeding
- □ ⊠ Permanent Planting, Sodding or Seeding
- 🛛 🗆 Biodegradable Erosion Control Logs
- 🛛 🗆 Rock Filter Dams/ Rock Check Dams
- 🛛 🗆 Vertical Tracking
- Interceptor Swale
- 🗆 🛛 Riprap
- Diversion Dike
- □ □ Temporary Pipe Slope Drain
- Embankment for Erosion Control
- □ □ Paved Flumes
- Other:______
- Other:
- 2.2 SEDIMENT CONTROL BMPs:

#### T / P

- Other: ______

7/5/2023

DATE:

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

Sediment control BMPs requiring design capacity calculations (See SWP3 Attachment 1.3.):

#### T / P

- Sediment Trap
  - Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
  - □ 3,600 cubic feet of storage per acre drained
- □ □ Sedimentation Basin
  - □ Not required (<10 acres disturbed)
  - □ Required (>10 acres) and implemented.
    - □ Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area

□ Other:

- □ 3,600 cubic feet of storage per acre drained
- $\boxtimes$  Required (>10 acres), but not feasible due to:
- Available area/Site geometry
- □ Site slope/Drainage patterns
- □ Site soils/Geotechnical factors
- Public safety

#### 2.3 PERMANENT CONTROLS:

- (Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)
- BMPs To Be Left In Place Post Construction:

Turne	Stat	tioning
Туре	From	То
efer to the Environmental La	ayout Sheets/ SWP	3 Layout Sheets
cated in Attachment 1.2 of t	his SWP3	

#### 2.4 OFFSITE VEHICLE TRACKING CONTROLS:

□ Other:

- Excess dirt/mud on road removed daily
- □ Haul roads dampened for dust control

- _____
- □ Other:_____
- □ Other:

#### 2.5 POLLUTION PREVENTION MEASURES:

- Chemical Management
- $\boxtimes$  Concrete and Materials Waste Management

□ Other:_____

- Debris and Trash Management
- I Dust Control
- Sanitary Facilities
- Other: ______

Other:

□ Other:

#### 2.6 VEGETATED BUFFER ZONES:

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Туре	St	ationing
Туре	From	То
Wetland	996+25	999+45
Refer to the Environmental La	ayout Sheets/ SWP	'3 Layout Sheets

Refer to the Environmental Layout Sheets/ SWP3 Layout Shee located in Attachment 1.2 of this SWP3

#### 2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

#### 2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3 .

#### 2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.

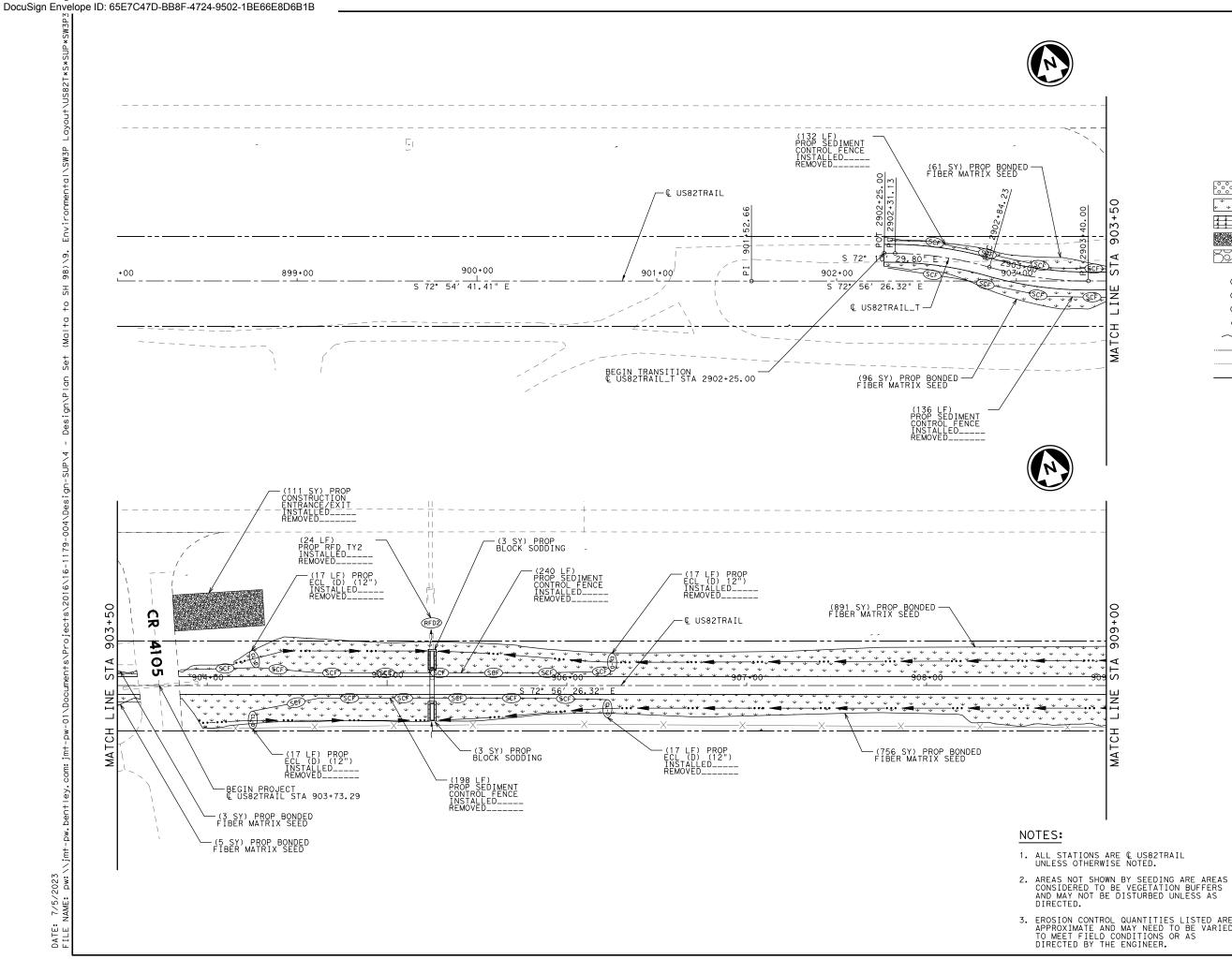
## STORMWATER POLLUTION PREVENTION PLAN (SWP3)



Sheet 2 of 2

Texas Department of Transportation

	FED. RD. DIV. NO.		PROJECT NO.				
	6		(SEE TITLE SHEET)				
	STATE	TATE STATE DIST. COUN			OUNTY		
	TEXAS	5	ATL	BOWIE			
	CONT.		SECT.	JOB HIGHWAY NO.		۱٥.	
0919 19 087 VARIOUS		JS					



SCALE IN FEET

LEGEND				
	SOIL RETENTION BLANKET			
* * * * * * * * * * *	BONDED FIBER MATRIX SEED			
	BLOCK SODDING			
	CONSTRUCTION ENTRANCE/EXIT			
Dinie	STONE RIPRAP			
SCF	SEDIMENT CONTROL FENCE			
(RFD1)	ROCK FILTER DAM (TYP 1)			
(RFD2)	ROCK FILTER DAM (TYP 2)			
(RFD3)	ROCK FILTER DAM (TYP 3)			
CL-D	EROSION CONTROL LOG DAM			
∕~►	FLOW LINE			
····	PROP DITCH FLOW LINE			
·····•	EXISTING DITCH FLOW LINE			
	ROW			



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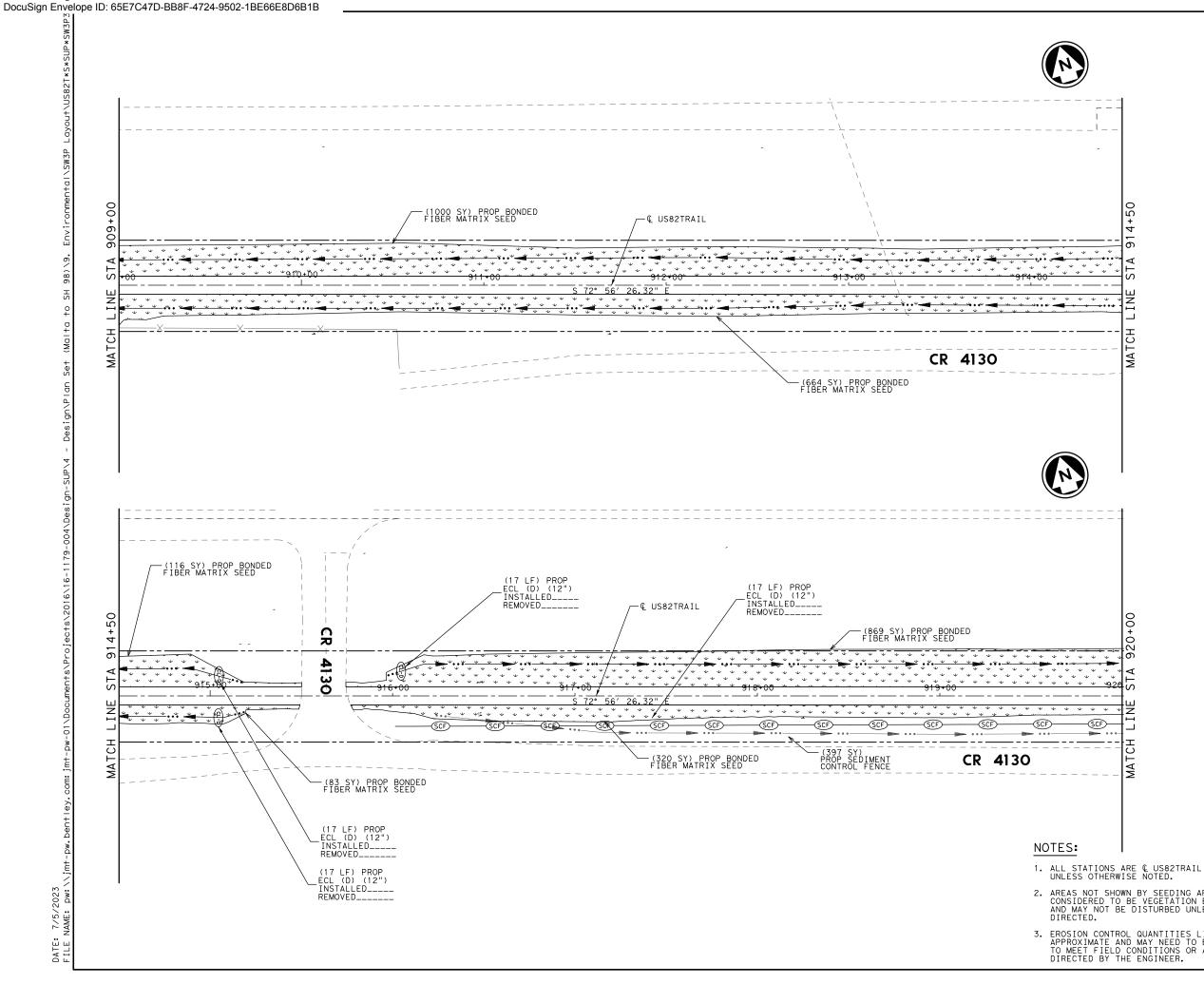


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#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS BEGIN TO STA 909+00

	SCALE: 1"			SHEET	
ĺ	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
ł	GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
	CHECK	TEXAS	ATL	BOWIE	
ł	JMT CHECK	CONTROL	SECTION	JOB	112
	JMT	0919	19	087	

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SCALE IN FEET

	SOIL RETENTION BLANKET
$\begin{array}{c} \psi \\ \psi $	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
59000	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
∕~►	FLOW LINE
······	PROP DITCH FLOW LINE
	EXISTING DITCH FLOW LINE
	ROW



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#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 909+00 TO STA 920+00

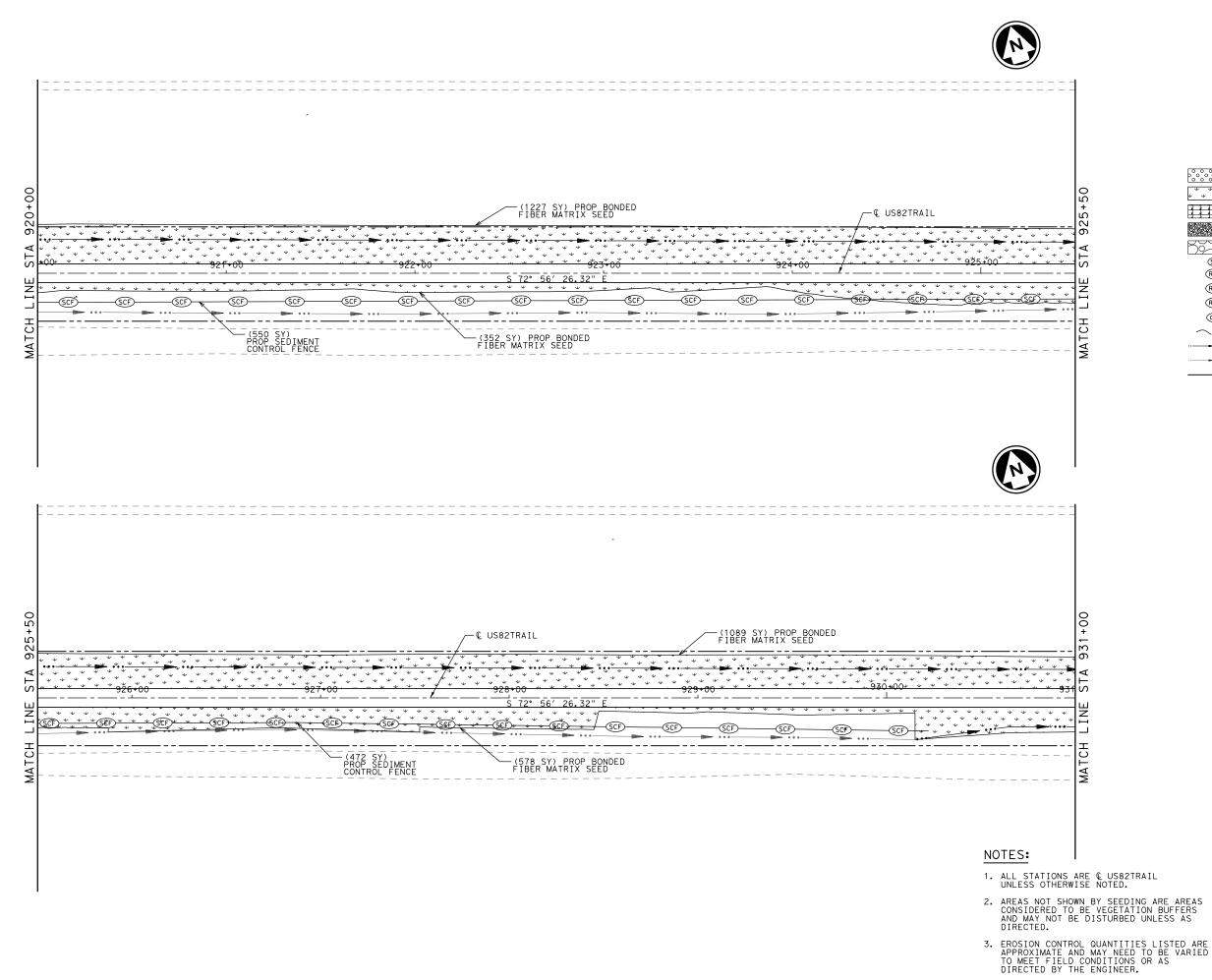
SCALE: 1"			SHEET	2 OF 16
JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	113
JMT	0919	19	087	

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AREAS NOT SHOWN BY SEEDING ARE AREAS CONSIDERED TO BE VEGETATION BUFFERS AND MAY NOT BE DISTURBED UNLESS AS DIRECTED.





DATE:

SCALE IN FEET

#### LEGEND $\circ$ SOIL RETENTION BLANKET * * * * * BONDED FIBER MATRIX SEED BLOCK SODDING CONSTRUCTION ENTRANCE/EXIT STONE RIPRAP SCF SEDIMENT CONTROL FENCE (RFD1) ROCK FILTER DAM (TYP 1) (RFD2) ROCK FILTER DAM (TYP 2) RFD3 ROCK FILTER DAM (TYP 3) CL-D EROSION CONTROL LOG DAM FLOW LINE $\frown$ PROP DITCH FLOW LINE ···---EXISTING DITCH FLOW LINE



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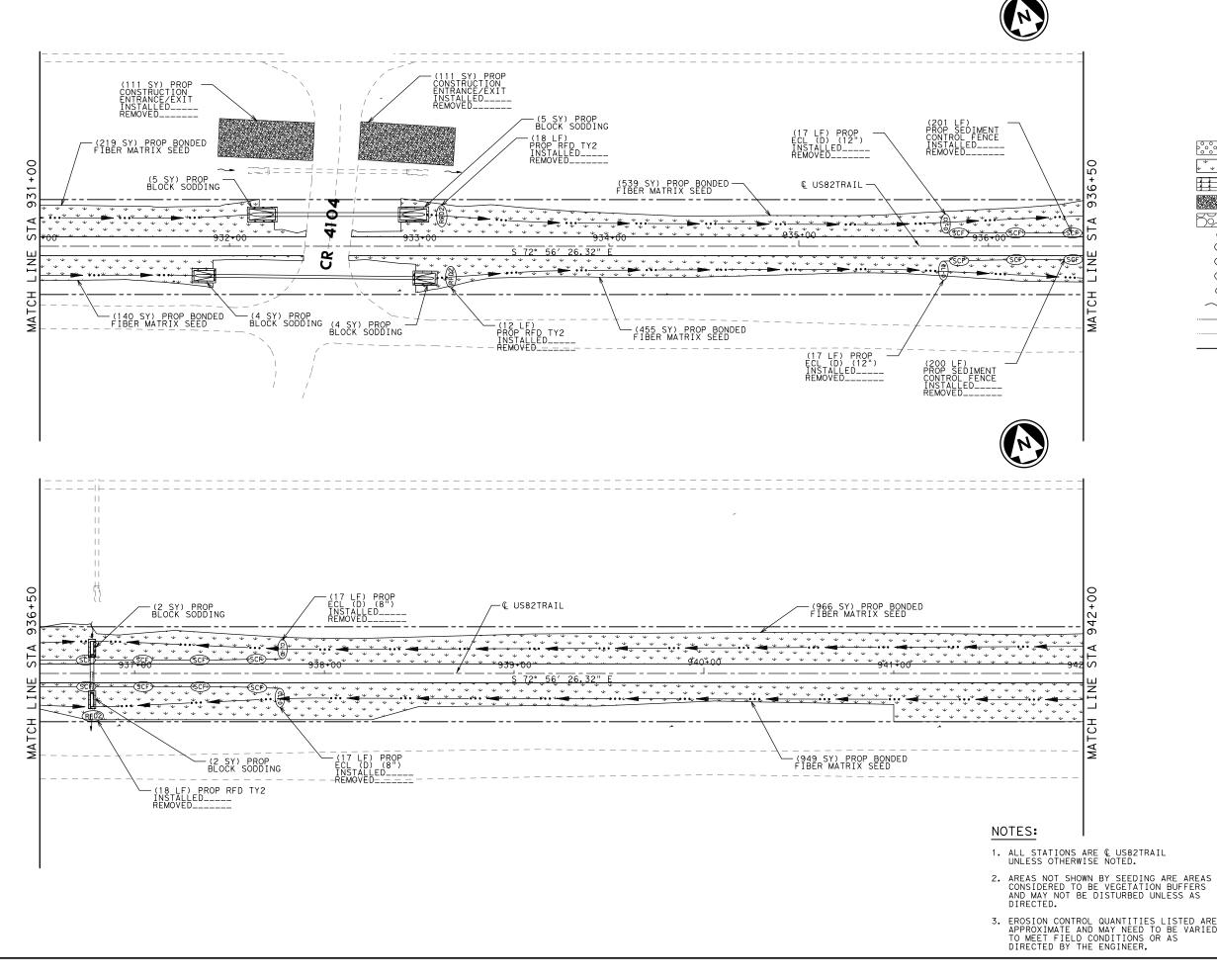
#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 920+00 TO STA 931+00

SCALE: 1"			SHEET	3 OF 16
JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	114
JMT	0919	19	087	

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DATE:



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#### LEGEND

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	BLOCK SODDING			
	CONSTRUCTION ENTRANCE/EXIT			
Dinge	STONE RIPRAP			
SCF	SEDIMENT CONTROL FENCE			
(RFD1)	ROCK FILTER DAM (TYP 1)			
RFD2	ROCK FILTER DAM (TYP 2)			
(RFD3)	ROCK FILTER DAM (TYP 3)			
CL-D	EROSION CONTROL LOG DAM			
∕~>	FLOW LINE			
····	PROP DITCH FLOW LINE			
·········	EXISTING DITCH FLOW LINE			
	ROW			



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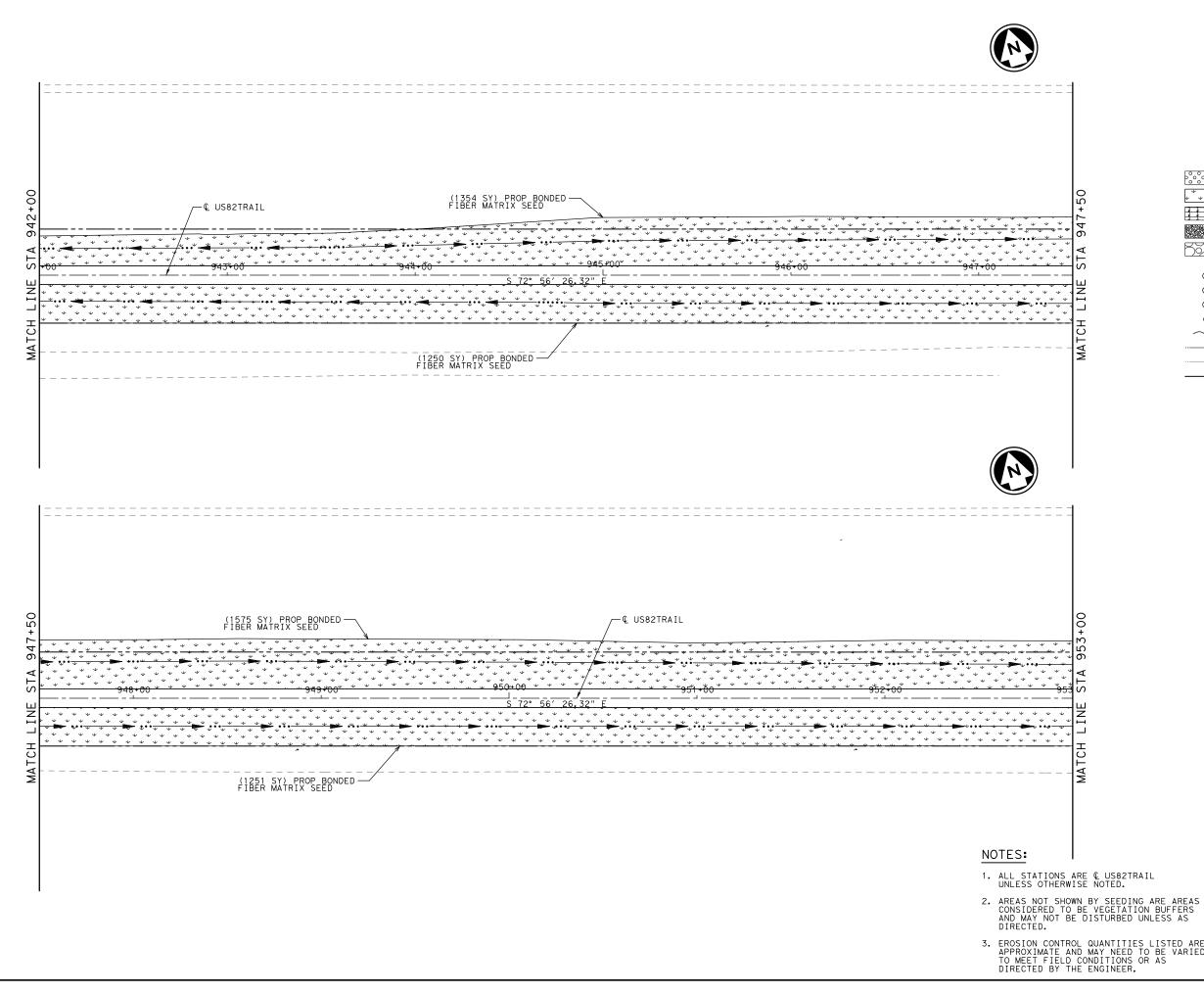
#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 931+00 TO STA 942+00

SCALE: 1"			SHEET	4 OF 16
JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	115
JMT	0919	19	087	



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DATE:



SCALE IN FEET

#### LEGEND

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	BLOCK SODDING			
	CONSTRUCTION ENTRANCE/EXIT			
Den de	STONE RIPRAP			
SCF	SEDIMENT CONTROL FENCE			
(RFD1)	ROCK FILTER DAM (TYP 1)			
(RFD2)	ROCK FILTER DAM (TYP 2)			
(RFD3)	ROCK FILTER DAM (TYP 3)			
CL-D	EROSION CONTROL LOG DAM			
∕~>	FLOW LINE			
······•	PROP DITCH FLOW LINE			
······································	EXISTING DITCH FLOW LINE			
	ROW			



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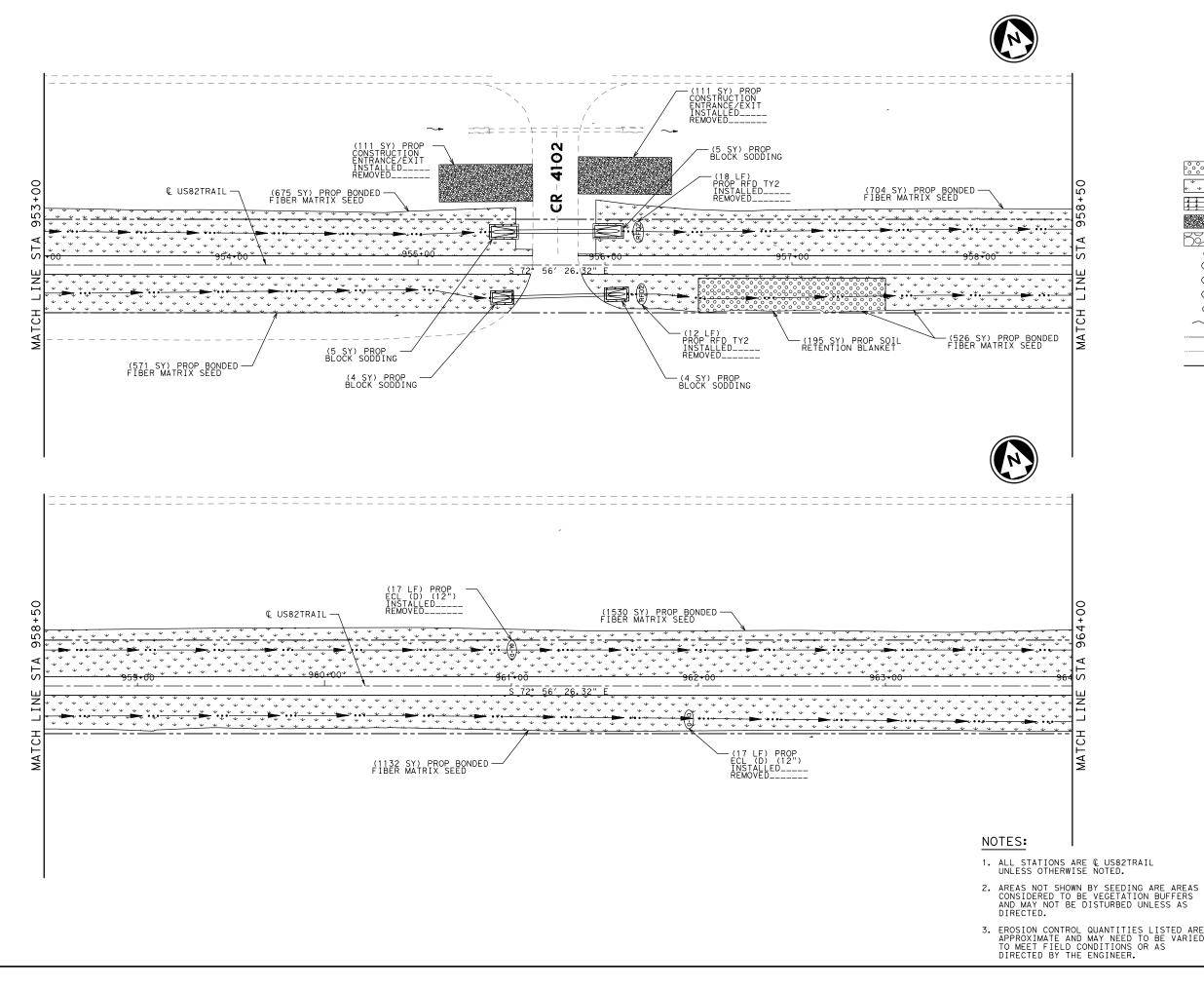
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#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 942+00 TO STA 953+00

SCALE: 1"			SHEET	5 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	116
JMT	0919	19	087	

2023 7/5/

DATE:



SCALE IN FEET

#### LEGEND

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$\begin{array}{c} \psi & \psi & \psi & \psi \\ \psi & \psi & \psi & \psi & \psi \end{array}$	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
<u>Bénai</u>	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
∕~>	FLOW LINE
······•	PROP DITCH FLOW LINE
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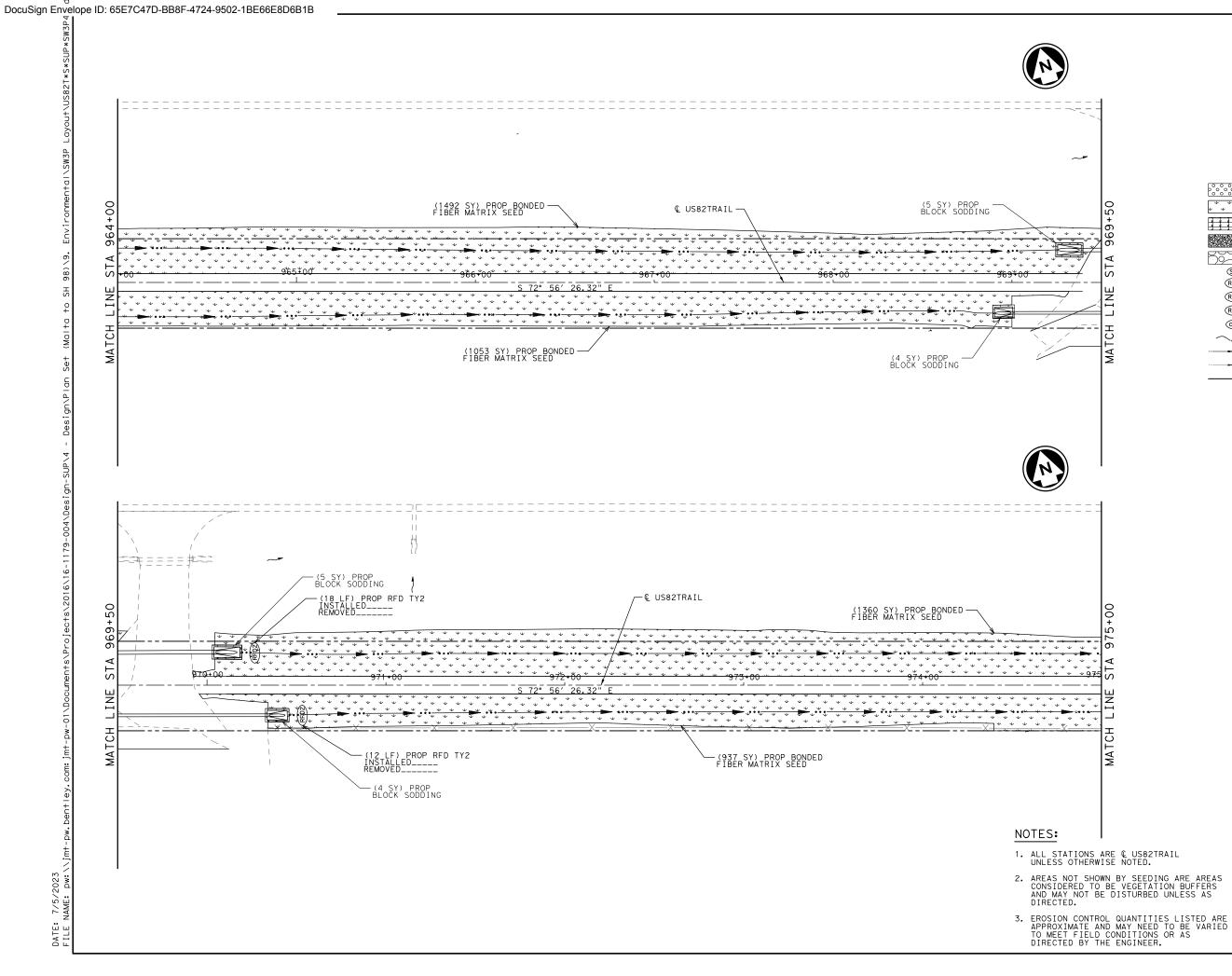


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#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 953+00 TO STA 964+00

	SCALE: 1"			SHEET	6 OF 16
Γ	DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
┢	GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
	JMT	STATE	DISTRICT	COUNTY	SHEET NO.
Γ	снеск ЈМТ	TEXAS	ATL	BOWIE	
┢	CHECK	CONTROL	SECTION	JOB	117
	JMT	0919	19	087	



SCALE IN FEET

	SOIL RETENTION BLANKET
* * * * * * * * *	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
Denie	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ $\rightarrow$	FLOW LINE
····	PROP DITCH FLOW LINE
·······•	EXISTING DITCH FLOW LINE
	ROW



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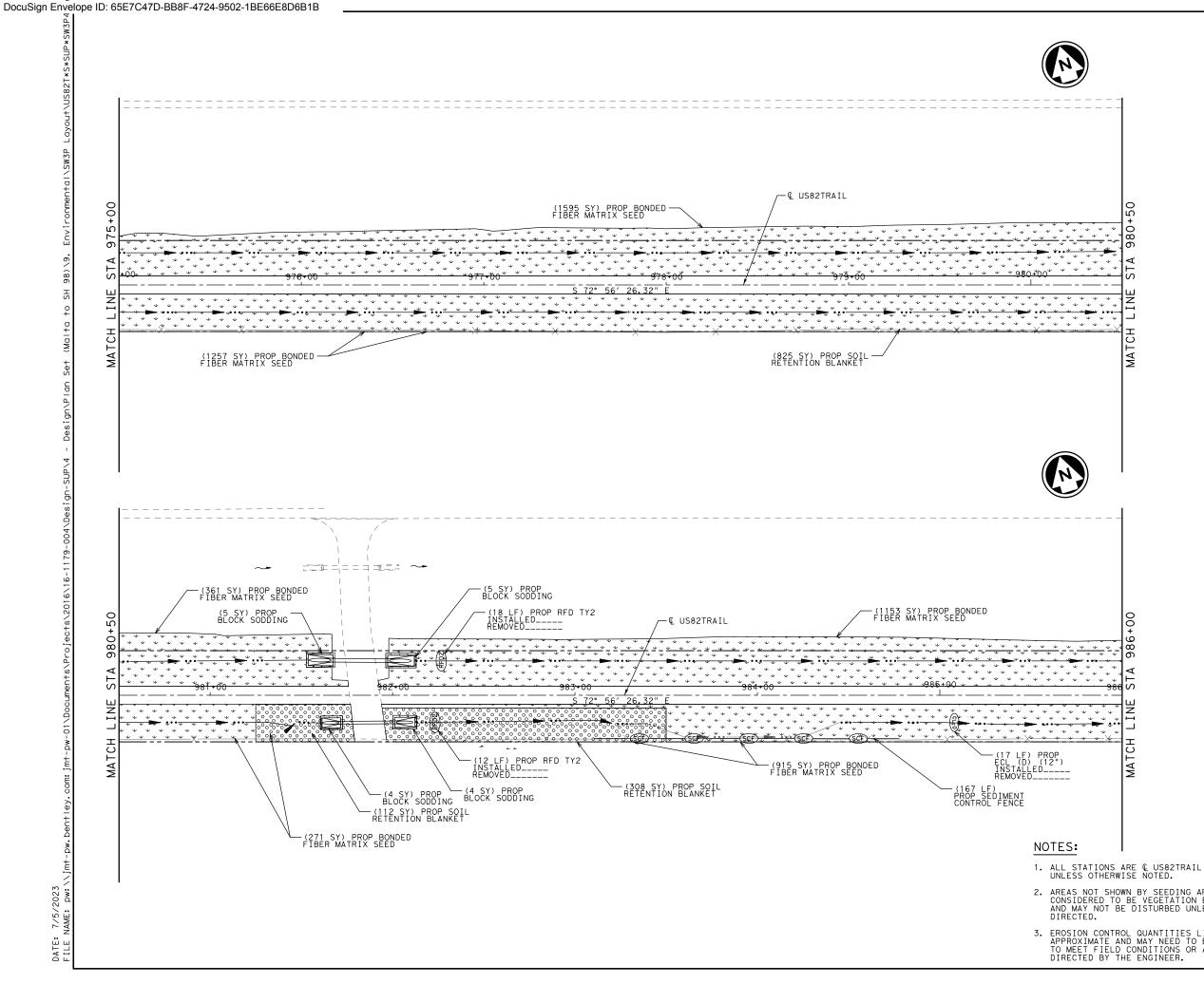


VARIOUS

#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 964+00 TO STA 975+00

SCALE: 1"			SHEET	7 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	118
JMT	0919	19	087	

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SCALE IN FEET

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	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ >	FLOW LINE
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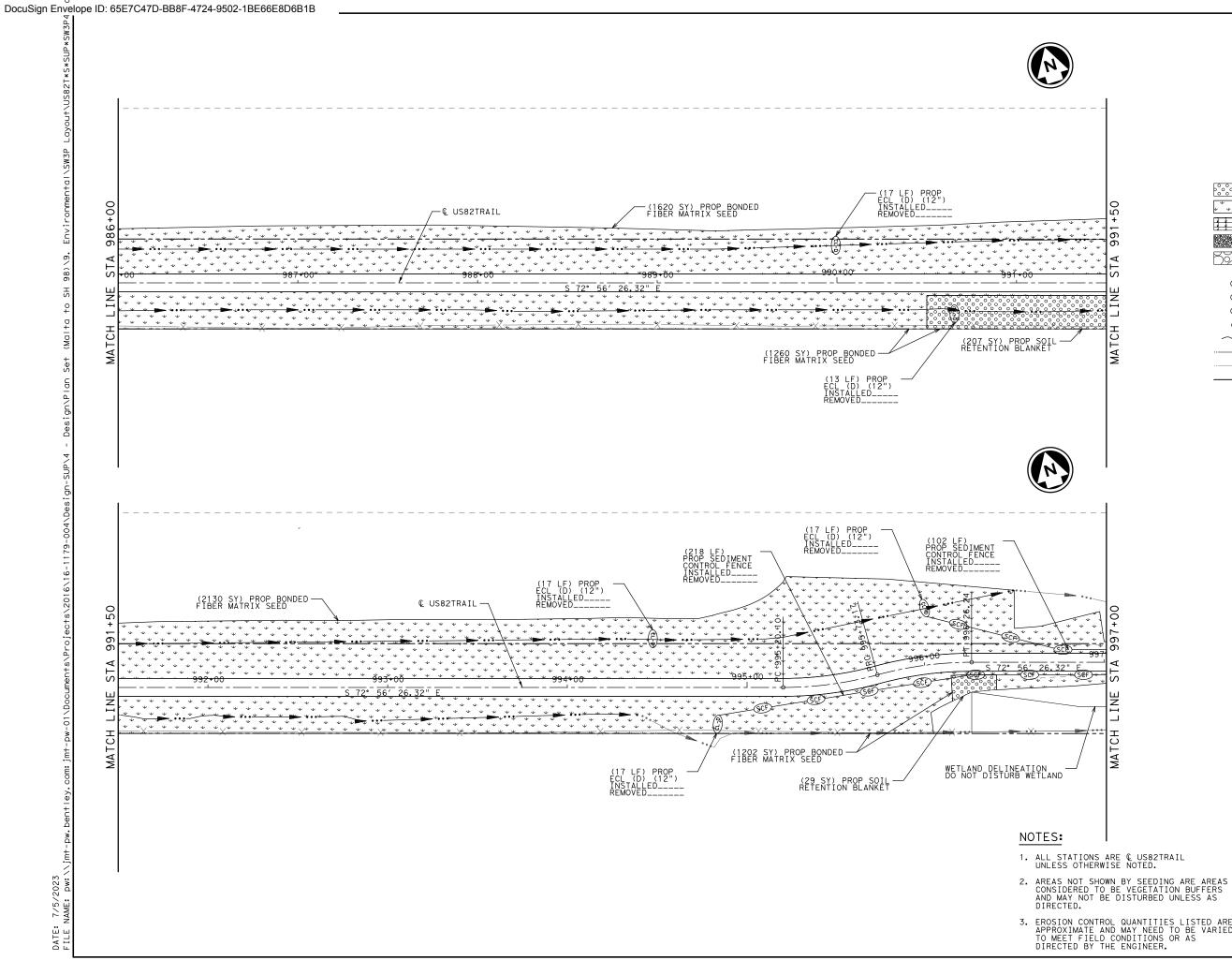
#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 975+00 TO STA 986+00

SCALE: 1"			SHEET	8 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE T	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	119
JMT	0919	19	087	

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	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
Dénie	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ >	FLOW LINE
····	PROP DITCH FLOW LINE
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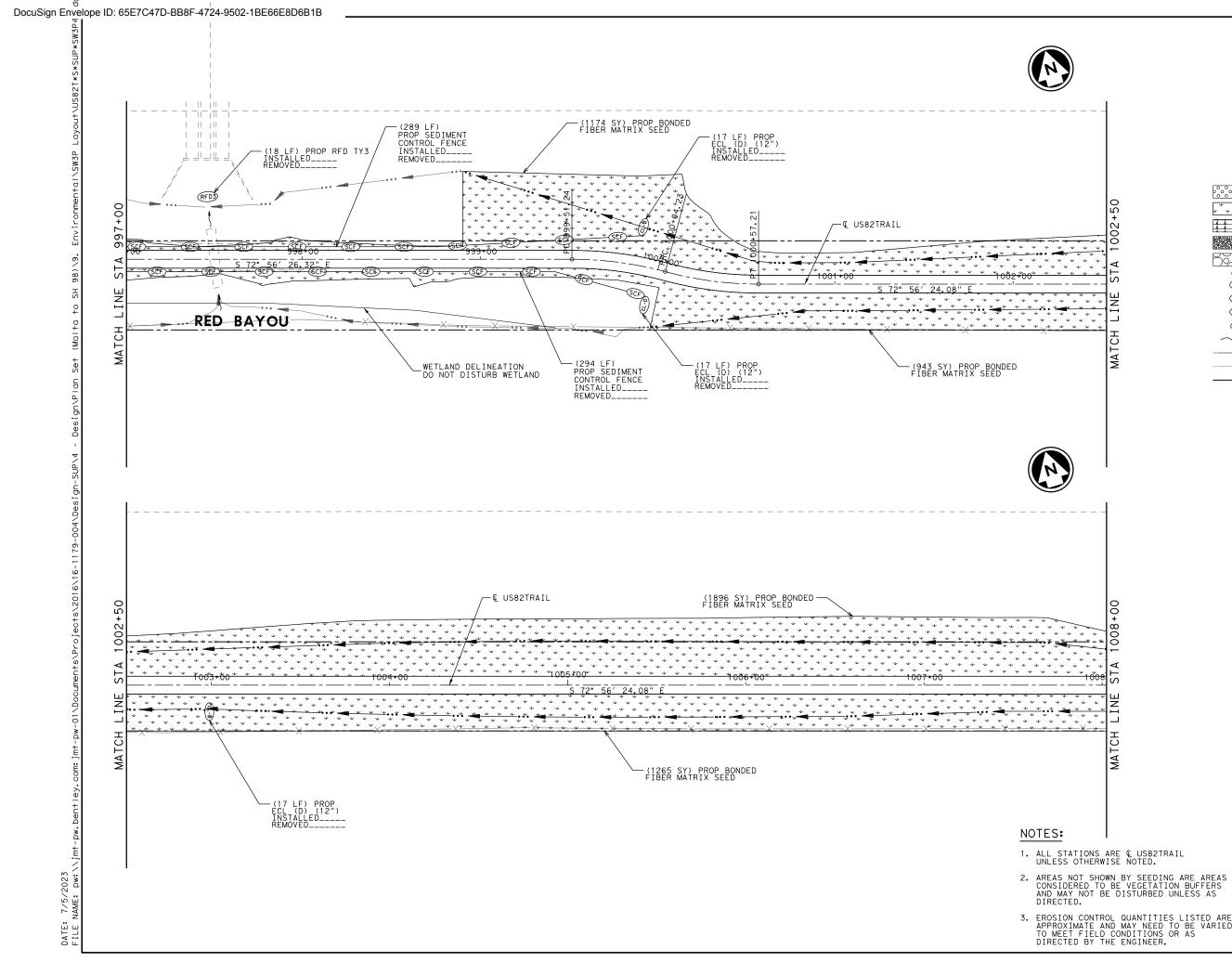
VARIOUS

#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 986+00 TO STA 997+00

SCALE: 1"			SHEET	9 OF 16
JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	120
JMT	0919	19	087	

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	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
Dénie	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
RFD1	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ $\rightarrow$	FLOW LINE
···	PROP DITCH FLOW LINE
······································	EXISTING DITCH FLOW LINE
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#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 997+00 TO STA 1008+00

SCALE: 1"			SHEET	10 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	121
JMT	0919	19	087	

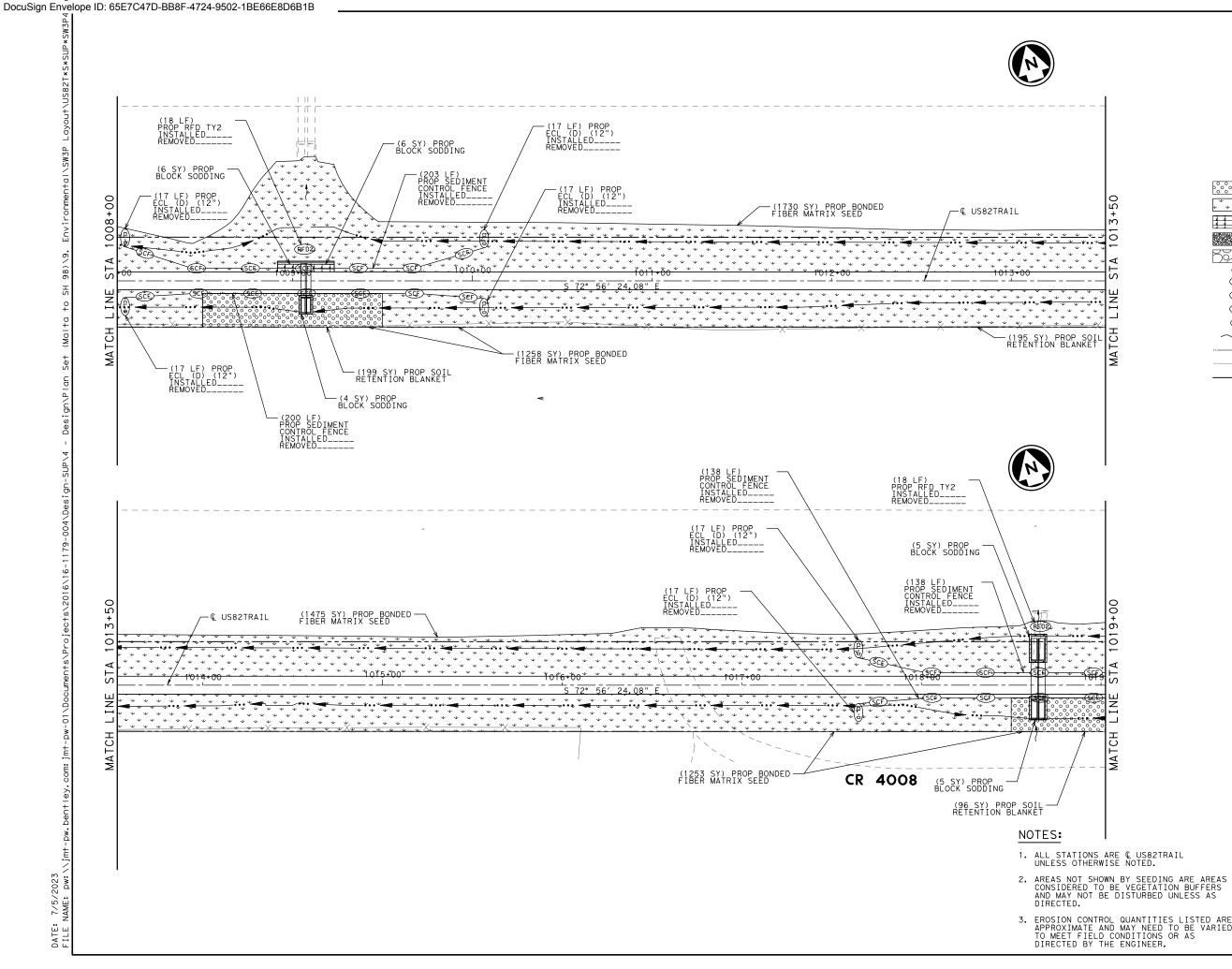
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EROSION CONTROL QUANTITIES LISTED ARE APPROXIMATE AND MAY NEED TO BE VARIED TO MEET FIELD CONDITIONS OR AS DIRECTED BY THE ENGINEER.

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SCALE IN FEET

	SOIL RETENTION BLANKET
$\begin{array}{c} \psi  \psi  \psi  \psi  \psi \\ \psi  \psi  \psi  \psi  \psi $	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
RFD1	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ $\rightarrow$	FLOW LINE
······································	PROP DITCH FLOW LINE
······································	EXISTING DITCH FLOW LINE
	ROW



7/5/2023

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Texas Department of Transportation

VARIOUS

#### **EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 1008+00 TO STA 1019+00

SCALE: 1			SHEET	11 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	- 6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
CHECK	TEXAS	ATL	BOWIE	
JMT CHECK	CONTROL	SECTION	JOB	122
JMT	0919	19	087	

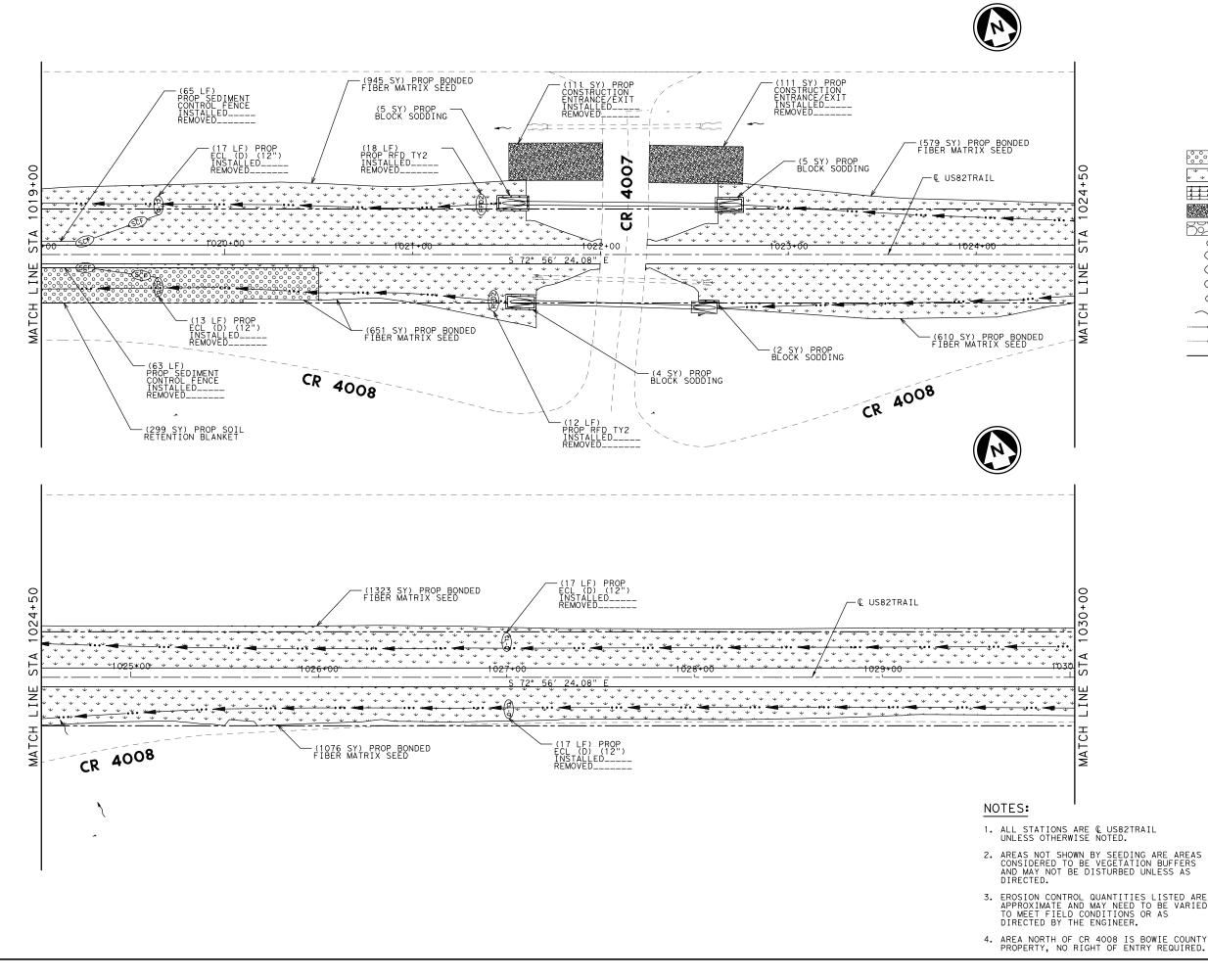
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DATE:



SCALE IN FEET

#### LEGEND

	SOIL RETENTION BLANKET
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	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
59	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
∕≁►	FLOW LINE
···· <b>···</b> ·····························	PROP DITCH FLOW LINE
·····	EXISTING DITCH FLOW LINE
	ROW



7/5/2023

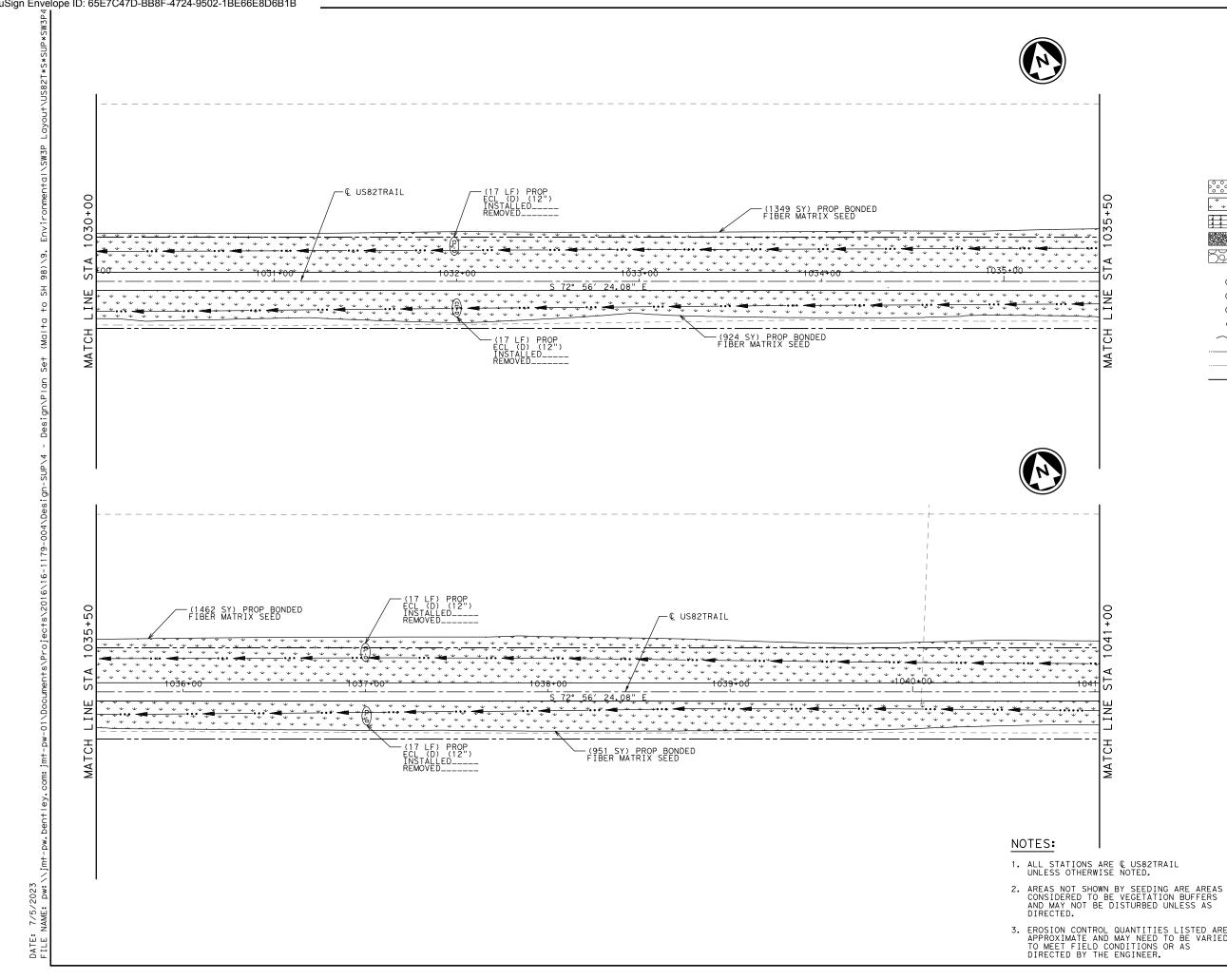
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VARIOUS

**EROSION CONTROL PLAN** SWP3 LAYOUT SHEETS STA 1019+00 TO STA 1030+00

SCALE: 1"			SHEET	12 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	123
JMT	0919	19	087	



SCALE IN FEET

	SOIL RETENTION BLANKET
$\begin{array}{c} \psi & \psi & \psi & \psi \\ \psi & \psi & \psi & \psi & \psi \end{array}$	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
<u>Bénai</u>	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
∕~>	FLOW LINE
······•	PROP DITCH FLOW LINE
······································	EXISTING DITCH FLOW LINE
	ROW



7/5/2023





Texas Department of Transportation

VARIOUS

#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 1030+00 TO STA 1041+00

SCALE: 1"			SHEET	13 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	124
JMT	0919	19	087	

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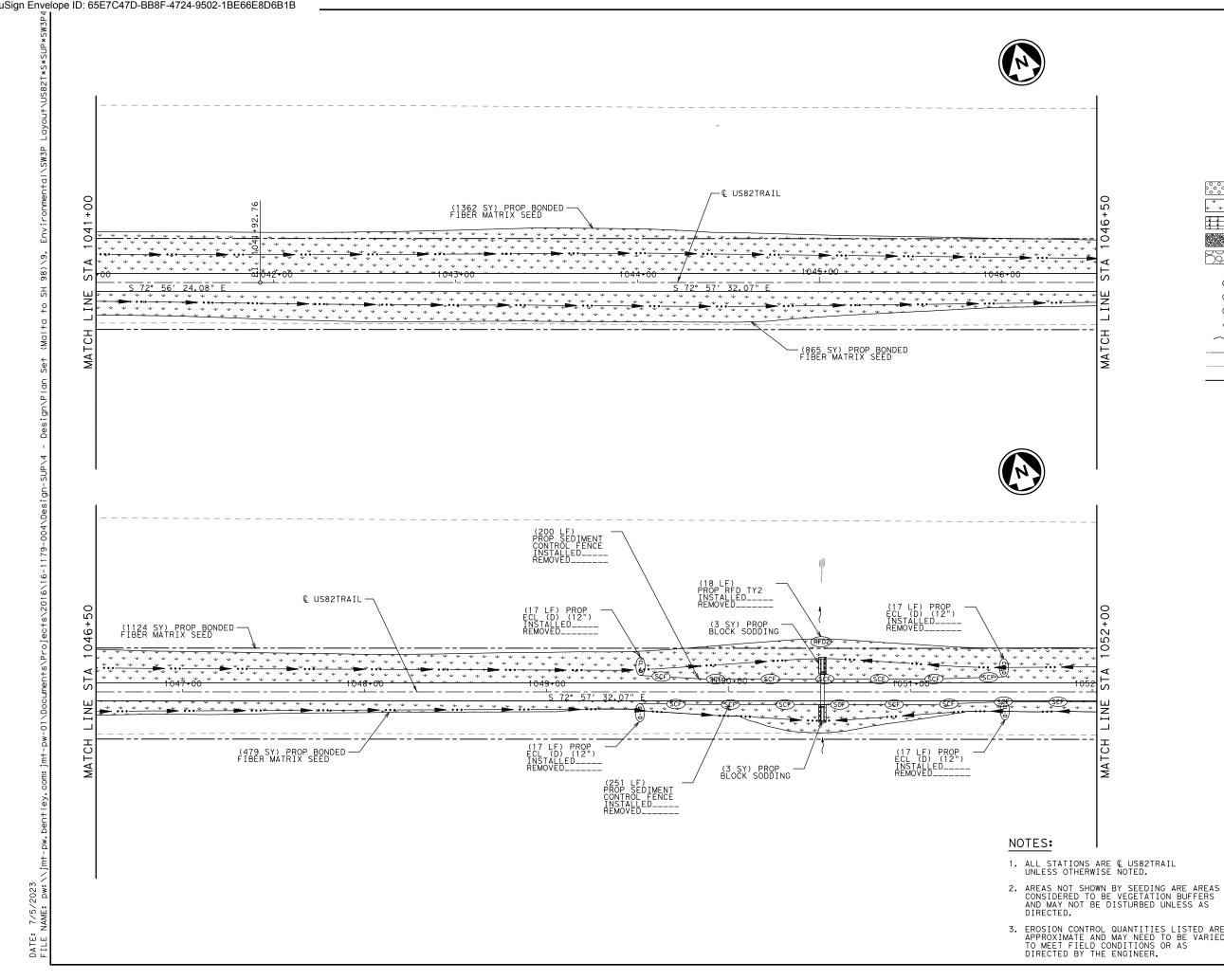
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EROSION CONTROL QUANTITIES LISTED ARE APPROXIMATE AND MAY NEED TO BE VARIED TO MEET FIELD CONDITIONS OR AS DIRECTED BY THE ENGINEER.

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SCALE IN FEET

	SOIL RETENTION BLANKET
$\begin{array}{c} \psi \\ \psi $	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
<u>bénne</u>	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ $\rightarrow$	FLOW LINE
····	PROP DITCH FLOW LINE
······	EXISTING DITCH FLOW LINE
	ROW



7/5/2023

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Texas Department of Transportation

VARIOUS

#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 1041+00 TO STA STA 1052+00

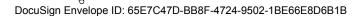
SCALE: 1"			SHEET	14 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	ITLE SHEET)	VARIOUS
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
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CHECK	CONTROL	SECTION	JOB	125
JMT	0919	19	087	

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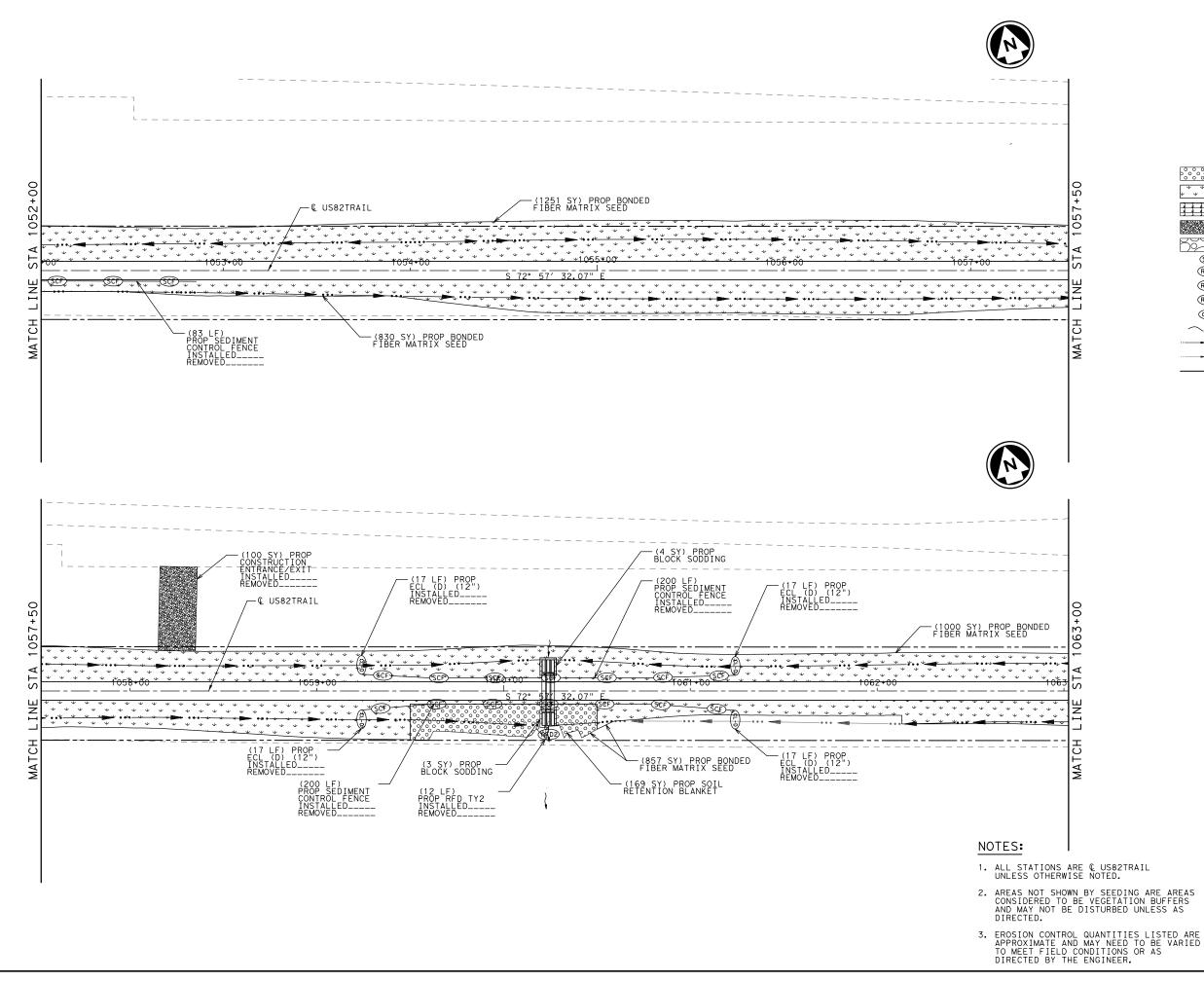
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#### LEGEND

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	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
Denie	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
$\sim$ $\rightarrow$	FLOW LINE
····	PROP DITCH FLOW LINE
······•	EXISTING DITCH FLOW LINE
	ROW

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Texas Department of Transportation

VARIOUS

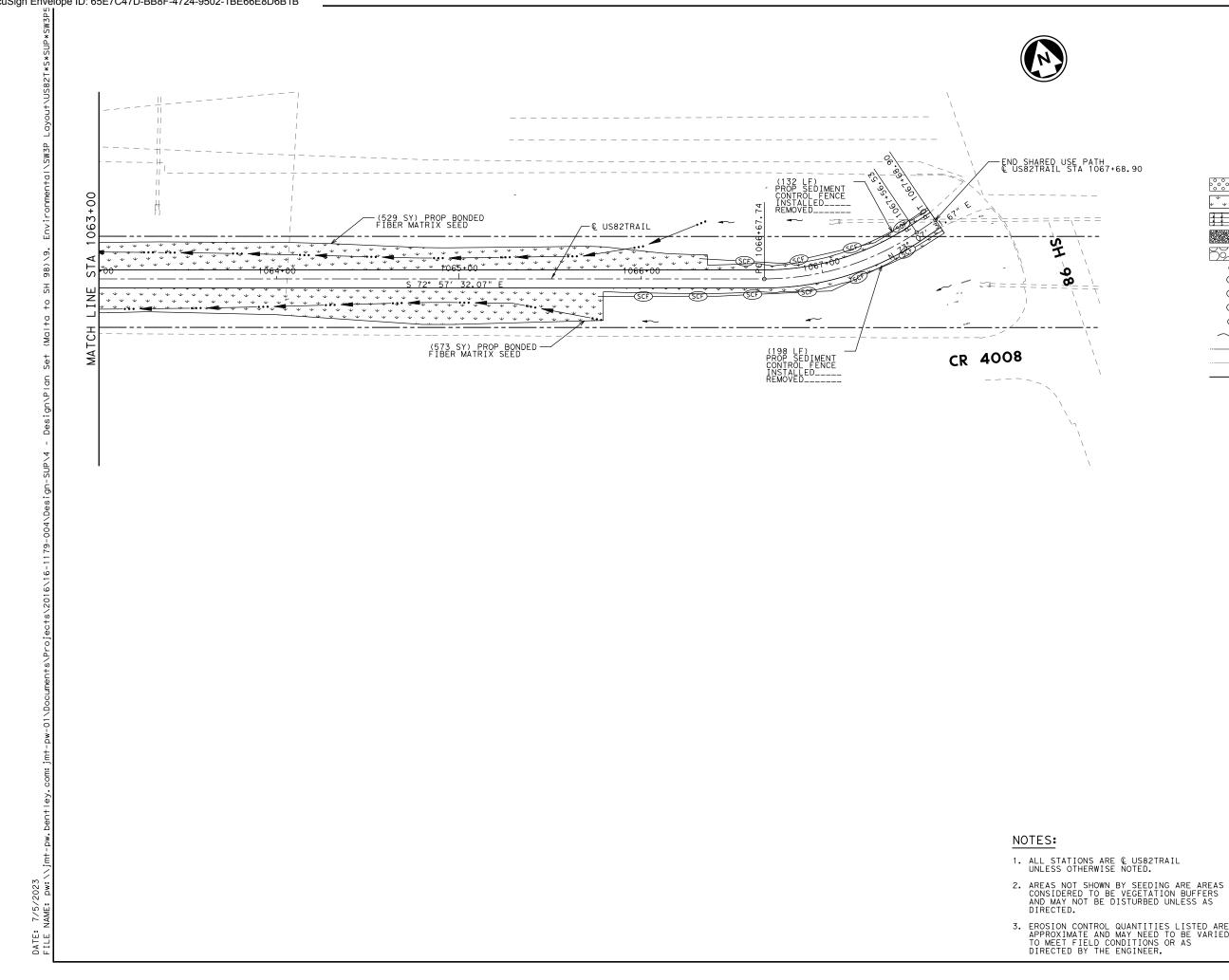
#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 1052+00 TO STA 1063+00

SCALE: 1"			SHEET	15 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	AID PROJECT NO.	HIGHWAY NO.
GRAPHICS	6	(SEE 1	VARIOUS	
JMT	STATE	DISTRICT	COUNTY	SHEET NO.
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	126
JMT	0919	19	087	

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SCALE IN FEET

	SOIL RETENTION BLANKET
$\begin{array}{c} \downarrow & \downarrow & \downarrow & \downarrow \\ \downarrow & \downarrow & \downarrow & \downarrow & \downarrow \\ \downarrow & \downarrow &$	BONDED FIBER MATRIX SEED
	BLOCK SODDING
	CONSTRUCTION ENTRANCE/EXIT
<u>Denne</u>	STONE RIPRAP
SCF	SEDIMENT CONTROL FENCE
(RFD1)	ROCK FILTER DAM (TYP 1)
(RFD2)	ROCK FILTER DAM (TYP 2)
(RFD3)	ROCK FILTER DAM (TYP 3)
CL-D	EROSION CONTROL LOG DAM
<u>∼</u> .~►	FLOW LINE
····	PROP DITCH FLOW LINE
·····•	EXISTING DITCH FLOW LINE
	ROW



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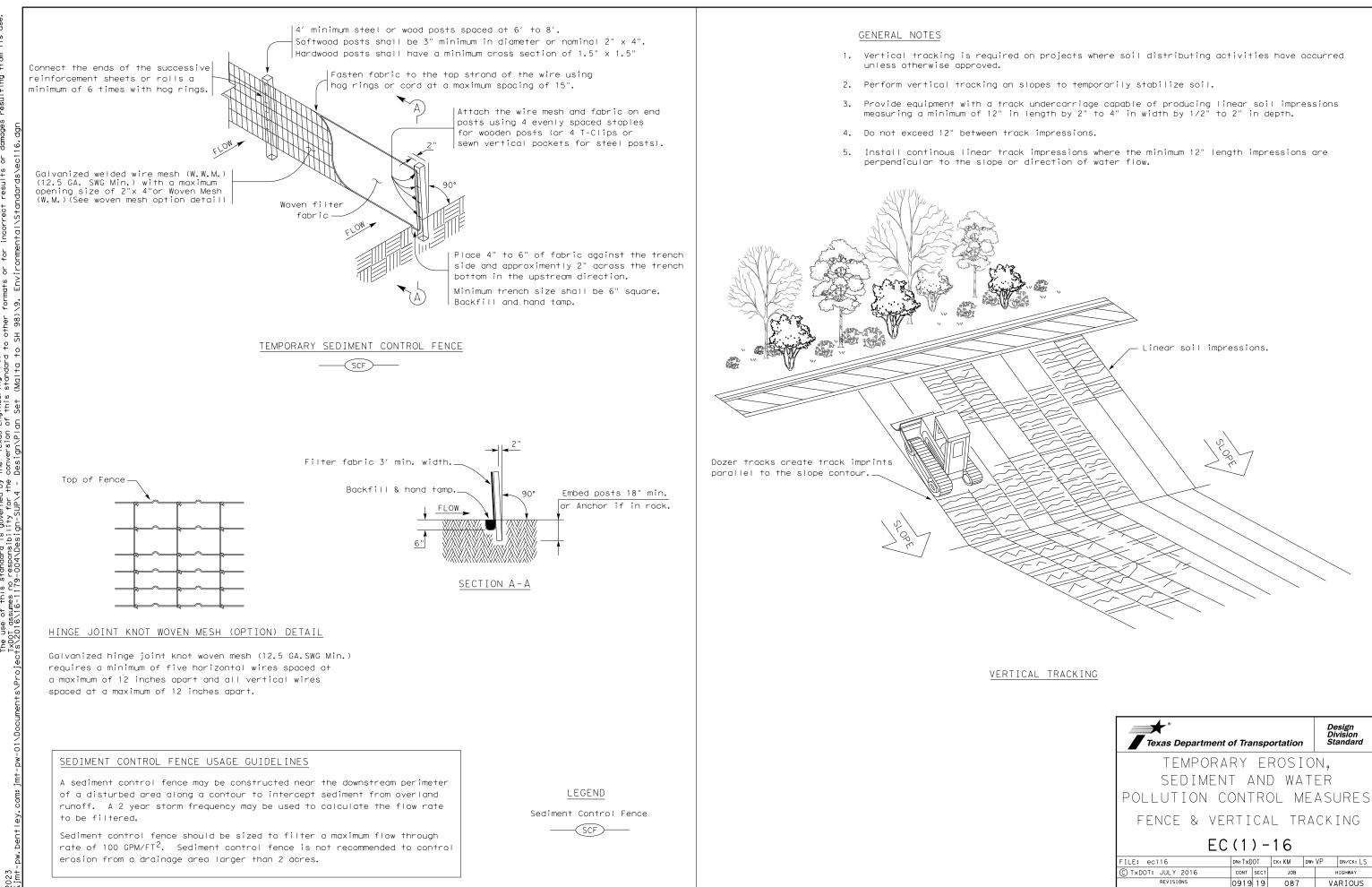




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#### EROSION CONTROL PLAN SWP3 LAYOUT SHEETS STA 1063+00 TO STA 1068+25

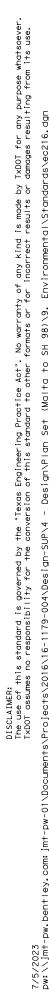
SCALE: 1"			SHEET	16 OF 16
DESIGN JMT	FED.RD. DIV.NO.	FEDERAL	HIGHWAY NO.	
GRAPHICS	6	(SEE 1	VARIOUS	
JMT	STATE	DISTRICT	SHEET NO.	
снеск ЈМТ	TEXAS	ATL	BOWIE	
CHECK	CONTROL	SECTION	JOB	127
JMT	0919	19	087	



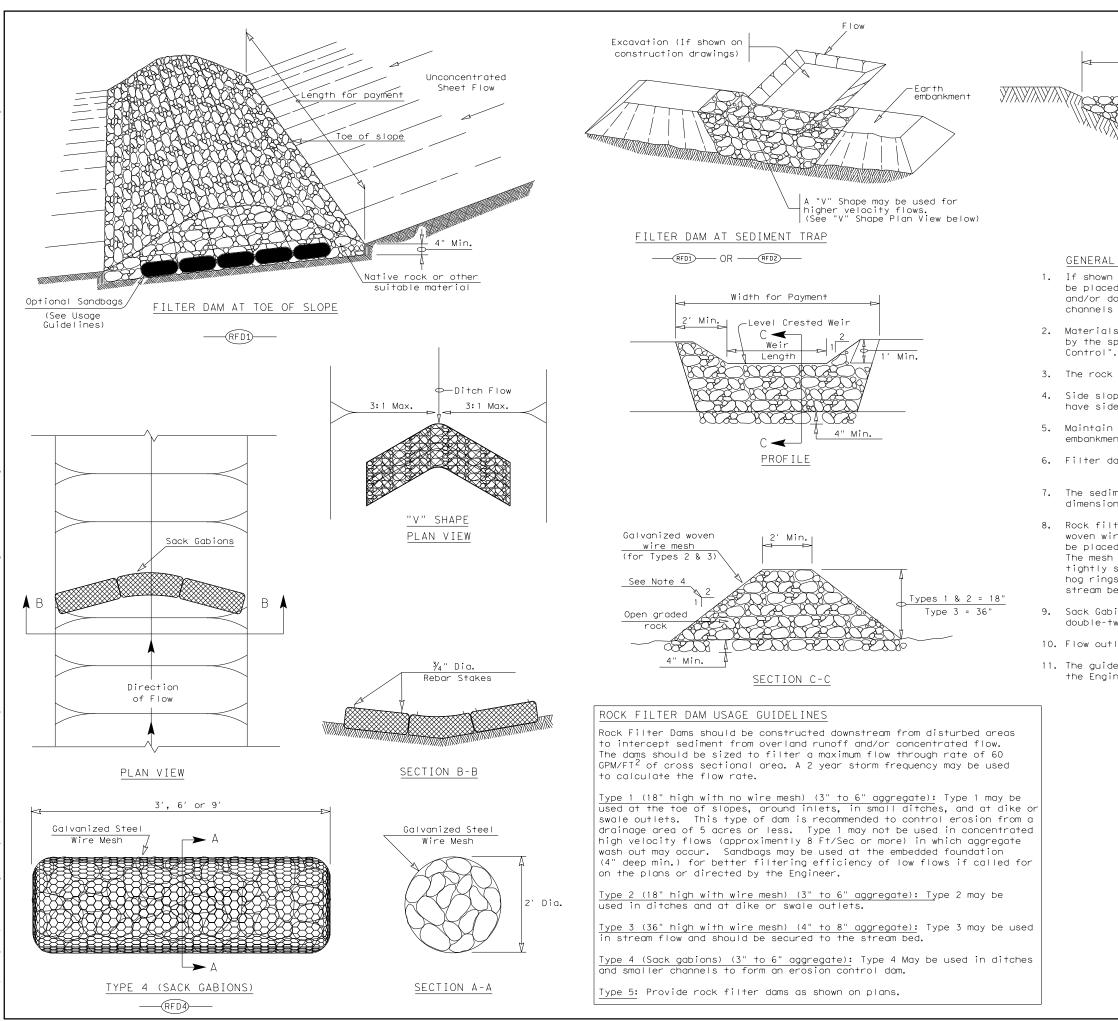
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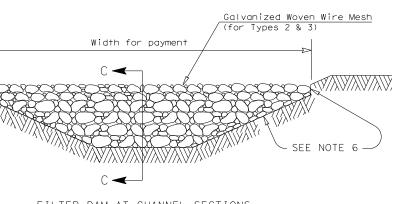
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FILE: ec116	FILE: ec116 DN:TxDOT CK:KM DW:VP DN/CK:LS							
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	ATL BOWIE 128							



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FILIER	DAM	ΑI	CHANNEL	SECTIONS
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#### GENERAL NOTES

1. If shown on the plans or directed by the Engineer, filter dams should be placed near the toe of slopes where erosion is anticipated, upstream and/or downstream at drainage structures, and in roadway ditches and channels to collect sediment.

2. Materials (aggregate, wire mesh, sandbags, etc.) shall be as indicated by the specification for "Rock Filter Dams for Erosion and Sedimentation

3. The rock filter dam dimensions shall be as indicated on the SW3P plans.

4. Side slopes should be 2:1 or flatter. Dams within the safety zone shall have sideslopes of 6:1 or flatter.

5. Maintain a minimum of 1' between top of rock filter dam weir and top of embankment for filter dams at sediment traps.

6. Filter dams should be embedded a minimum of 4" into existing ground.

7. The sediment trap for ponding of sediment laden runoff shall be of the dimensions shown on the plans.

8. Rock filter dam types 2 & 3 shall be secured with 20 gauge galvanized woven wire mesh with 1" diameter hexagonal openings. The aggregate shall be placed on the mesh to the height & slopes specified. The mesh shall be folded at the upstream side over the aggregate and tightly secured to itself on the downstream side using wire ties or hog rings. For in stream use, the mesh should be secured or staked to the stream bed prior to aggregate placement.

9. Sack Gabions should be staked down with  $\frac{3}{4}$ " dia. rebar stakes, and have a double-twisted hexagonal weave with a nominal mesh opening of 2  $\frac{1}{2}$  x 3  $\frac{1}{4}$ 

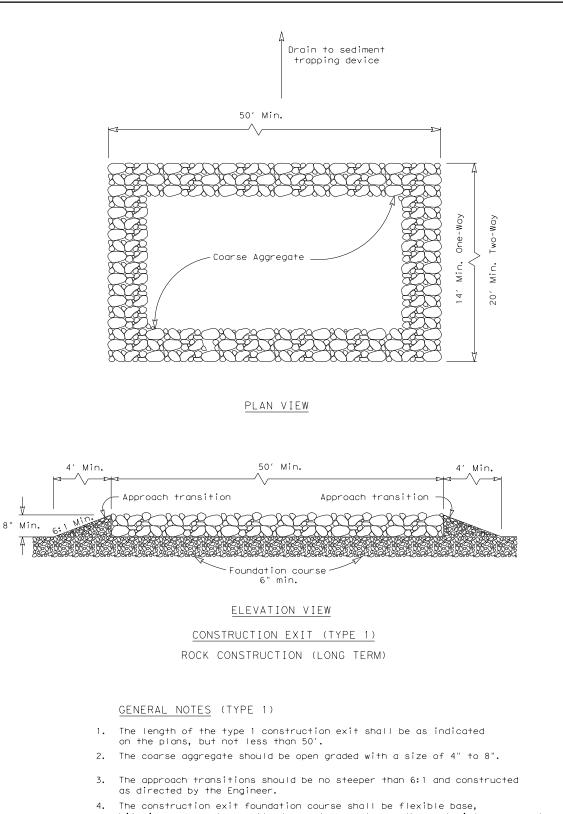
10. Flow outlet should be onto a stabilized area (vegetation, rock, etc.).

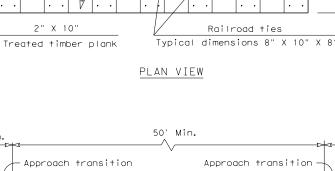
11. The guidelines shown hereon are suggestions only and may be modified by the Engineer.

#### PLAN SHEET LEGEND

Туре	1	Rock	Filter	Dam	
Туре	2	Rock	Filter	Dam	
Туре	3	Rock	Filter	Dam	
Туре	4	Rock	Filter	Dam	

Texas Department of	of Tra	nsp	ortation		Di	esign vision andard	
TEMPORARY EROSION,							
	SEDIMENT AND WATER POLLUTION CONTROL MEASURES						
ROCK F	ROCK FILTER DAMS						
EC	EC(2)-16						
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Drain to sediment

trapping device

2" X 6"

Treated timber plank

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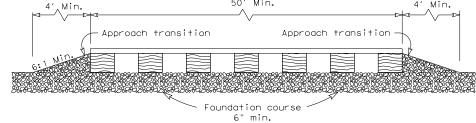
4′ Min.

50′ Min.

10" Min.

15

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#### ELEVATION VIEW

CONSTRUCTION EXIT (TYPE 2)

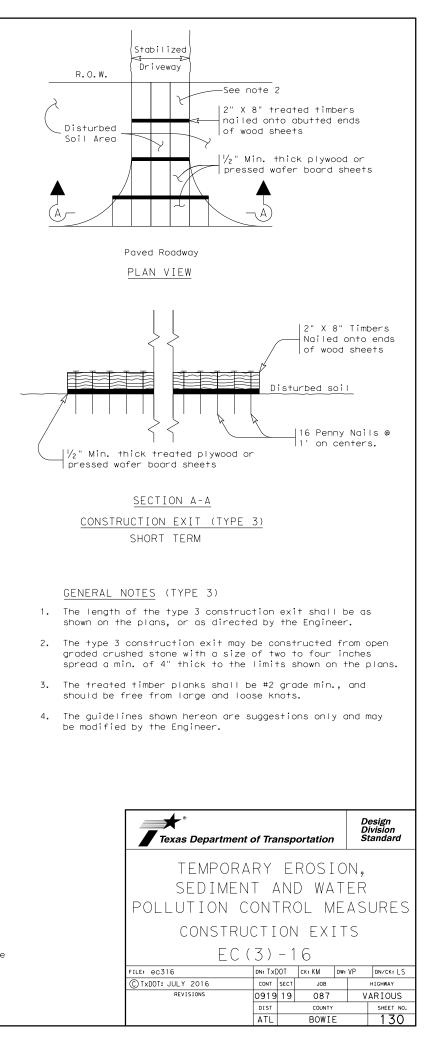
TIMBER CONSTRUCTION (LONG TERM)

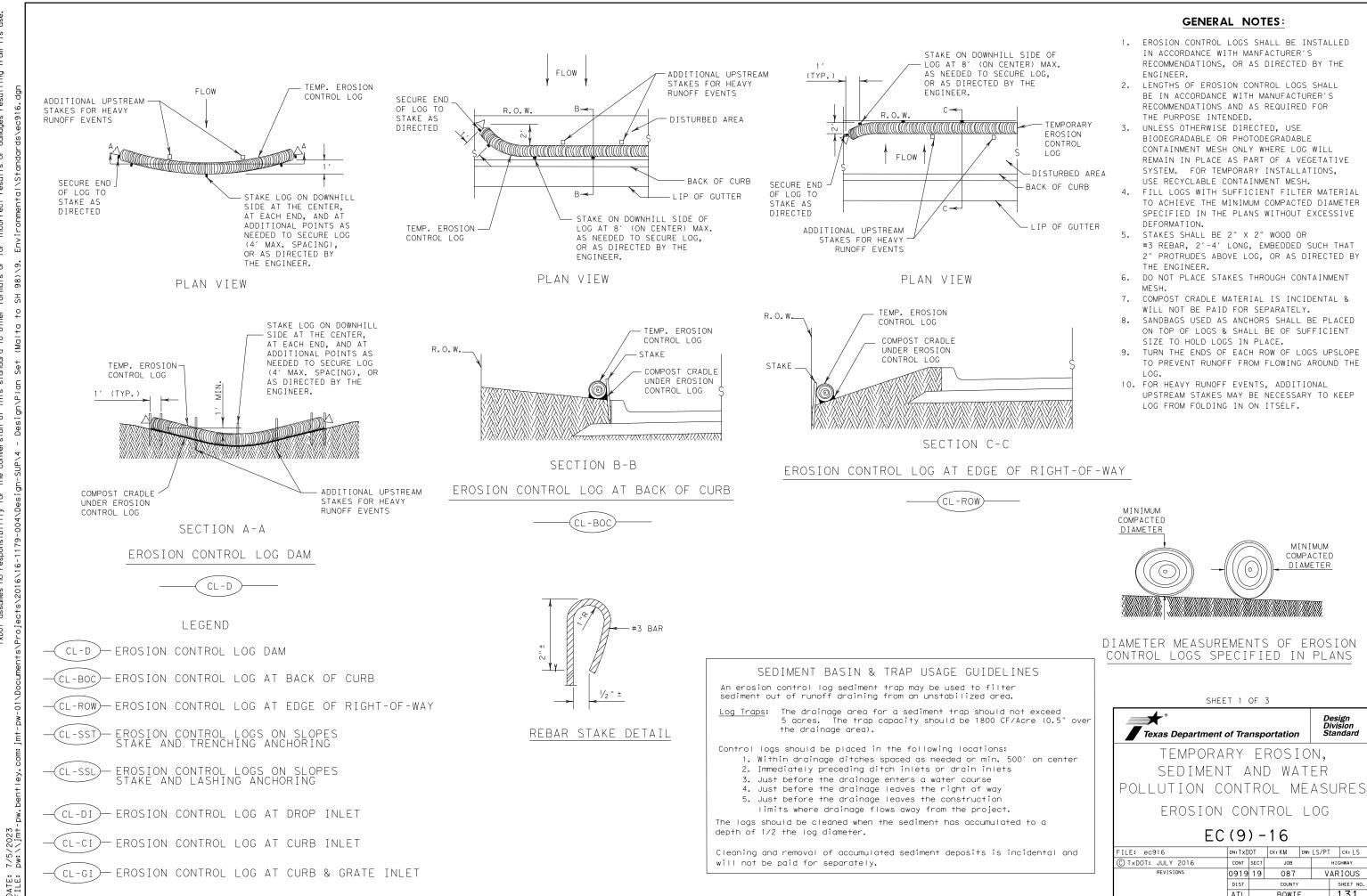
#### GENERAL NOTES (TYPE 2)

- 1. The length of the type 2 construction exit shall be as indicated on the plans, but not less than 50'.
- The treated timber planks shall be attached to the railroad 2. ties with  $\frac{1}{2}$  "x 6" min. Lag bolts. Other fasteners may be used as approved by the Engineer.
- The treated timber planks shall be #2 grade min., and should 3. be free from large and loose knots.
- 4. The approach transitions shall be no steeper than 6:1 and constructed as directed by the Engineer.
- 5. The construction exit foundation course shall be flexible base, bituminous concrete, portland cement concrete or other material as approved by the Engineer.
- The construction exit should be graded to allow drainage to a 6. sediment trapping device.
- The guidelines shown hereon are suggestions only and may 7. be modified by the Engineer.
- 8. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.

- bituminous concrete, portland cement concrete or other materialas approved by the Engineer.
- 5. The construction exit shall be graded to allow drainage to a sediment trapping device.
- 6. The guidelines shown hereon are suggestions only and may be modified by the Engineer.
- 7. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.

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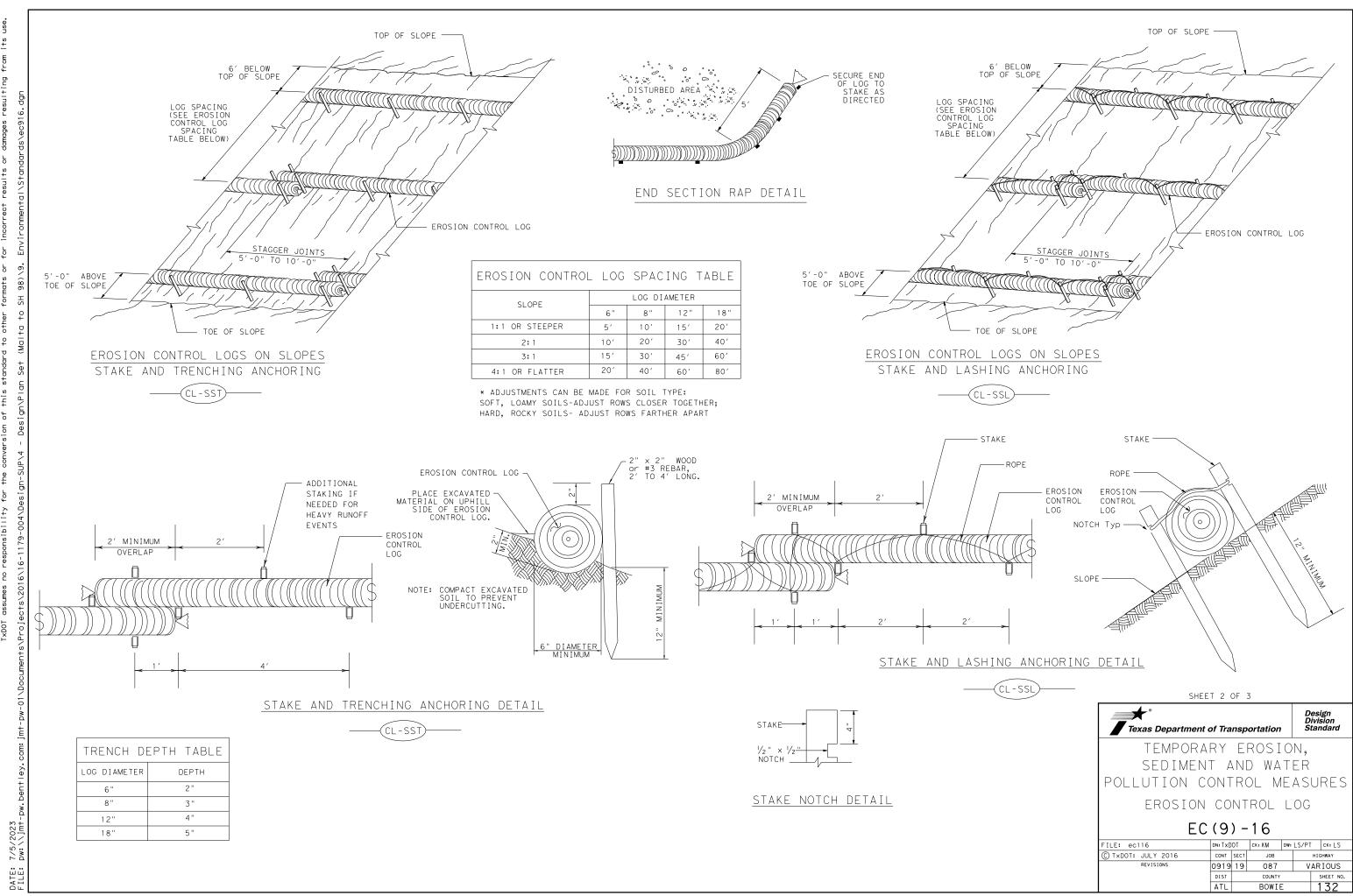




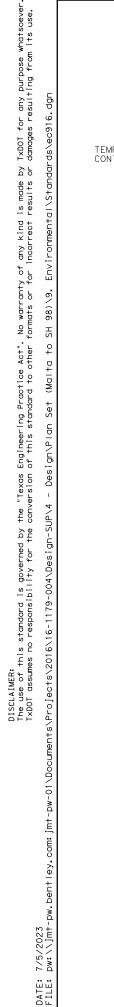
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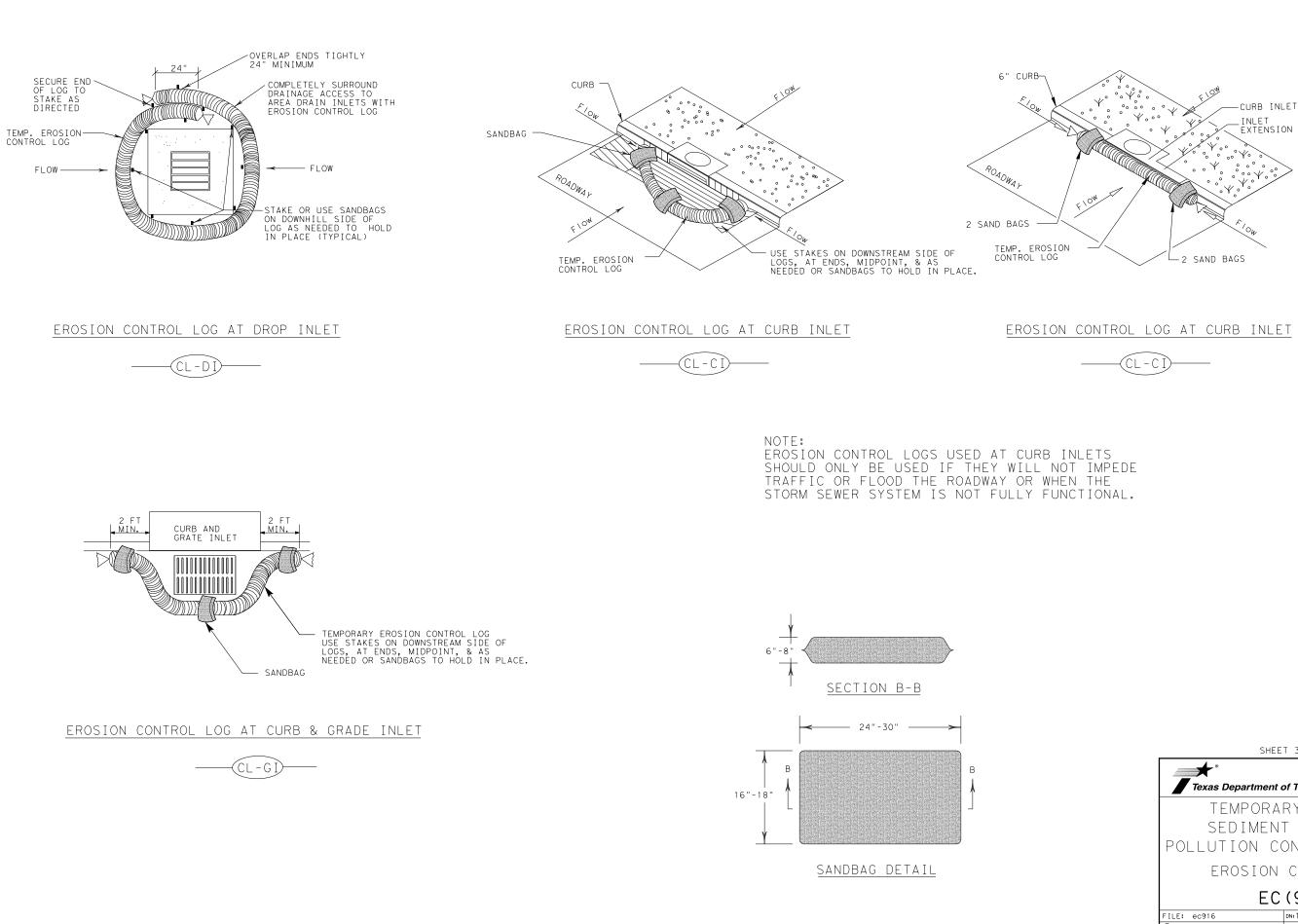
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	C TXDOT: JULY 2016	CONT	SECT JOB			HIGHWAY		
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soever use. by TxDOT for any purpose whats or damages resulting from its anty of any kind is made or for incorrect results "Texas Engineering Practice Act". No warr ersion of this standard to other formats DISCLAIMER: The use of this standard is governed by the TXDOT assumes no responsibility for the conv





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	EC (9) -16						
FILE: ec916	dn:TxD	OT	ск:КМ	DW:	LS/PT	CK: LS	
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REVISIONS	0919	19	087		VA	RIOUS	
	DIST COUNTY SHEET NO			SHEET NO.			
	ATL		BOWIE			133	

# **TXDOT US 82:** MALTA TO 98 UTILITY RELOCATION & HIGHWAY BORE **ROW CSJ#0919-19-094**

# **CENTRAL BOWIE COUNTY WSC**

**BOWIE COUNTY, TEXAS JUNE 2023** 

MAILBOX

SEWER CLEANOUT

SEWER MANHOLE

TREE WITH SIZE

SHRUBS OR BRUSH

WATER METER

FIRE HYDRANT

WATER VALVE

GAS METER

GAS TESTER

GAS LINE

WIRE FENCE

WOOD FENCE

CHAIN LINK FENCE

BURIED FLECTRIC

FIBER OPTIC PEDESTAL

WASTEWATER MAIN & SIZE

CATV/TELEPHONE PEDESTA

TELEPHONE CABLE

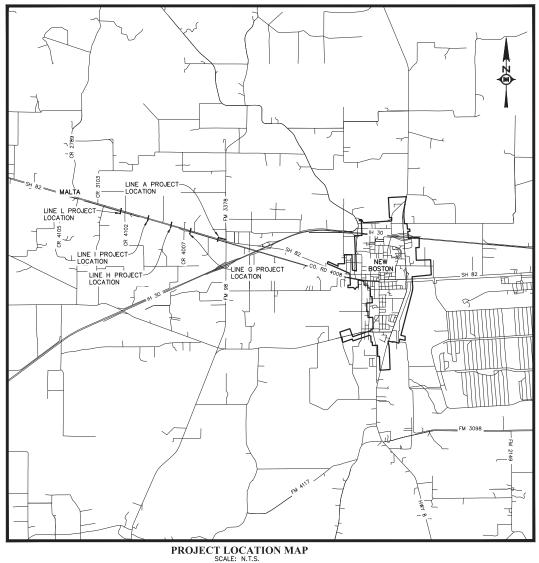
WATER MAIN & SIZE

POWER POLE WITH GUY WIRI

TRAFFIC SIGN

LEGEND

THIS PLAN SHEET HAS BEEN REDUCED TO HALF SCALE FOR INCLUSION IN THE PLAN SET. A COPY OF THE FULL SIZE PAGES ARE AVAILABLE AT THE **ENGINEERS OFFICE FOR USE.** 



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PROPOSED W	АТЕР	137	LINE
6"W	WATER LINE & SIZE	138	LINE
	WATER VALVE	139	LINE
10.0	5 1/4" FLUSH POINT ASSEMBLY	140	LINE
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	5 1/4" FLUSH POINT		
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•	1" FLUSH POINT		GENEI
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**ISSUED FOR BIDDING AND** CONSTRUCTION PURPOSES



## **PAGE INDEX**

## DESCRIPTION

#### LE PAGE

#### **VERAL NOTES & TECHNICAL SPECIFICATIONS**

#### **IMARY OF UTILITY RELOCATIONS**

#### ΕA

#### E G & LINE H

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#### **NSTRUCTION STANDARDS**

ERAL NOTES

CATE ALL TIE-IN POINTS AND VERIFY THE O.D.'S AND TYPES OF ALL LINES TO BE ECTED PRIOR TO LAYING ANY LINE SEGMENTS

4E CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION OF EXISTING ICES. A NEW SERVICE IS REQUIRED FROM THE NEW WATER MAIN TO THE EXISTIN

ELIVER ALL ABANDONED VALVES AND FIRE HYDRANTS TO THE CBCWSC BARN

ONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL EXISTING IED UTILITIES IN PROJECT AREA

5. CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING OVERHEAD UTILITY OWNER TO ENSURE PROTECTION OF EXISTING OVERHEAD UTILITIES. ANY COST INVOLVED IN SAID PROTECTION SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT.

6. REMOVE AND REINSTALL MAILBOXES, STREET SIGNS, AND CULVERTS AS NECESSARY FOR CONSTRUCTION. THESE ARE NOT SEPARATE PAY ITEMS

7. IN LIEU OF TRENCHING AND OPEN CUTTING, DRIVEWAY, AND STREETS, CAN BE DIRECTIONAL DRILLED, NO PAYMENT MODIFICATIONS IF THIS OPTION IS SELECTED

8. NO TREES SHOULD BE REMOVED UNLESS NOTED ON PLANS OR UPON SPECIFIC APPROVAL FROM OWNER/ENGINEER

9. ALL WORK INVOLVING SERVICE LINES SHALL BE PERFORMED BY, OR UNDER THE SUPERVISION OF A LICENSED PLUMBER

10. VERIFY SERVICE TRANSFER WITH CRCWSC REFORE PERFORMING TRANSFER

11. CONTRACTOR SHALL IMPLEMENT TRAFFIC CONTROL PLAN TO MEET WITH TXDO REQUIREMENTS

#### **GENERAL NOTES:**

TO BE CONNECTED PRIOR TO LAYING ANY LINE SEGMENTS.

TO THE EXISTING METER.

3. DELIVER ALL ABANDONED VALVES AND FIRE HYDRANTS TO THE CBCWSC BARN.

EXISTING BURIED UTILITIES IN PROJECT AREA.

INVOLVED IN SAID PROTECTION SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT.

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APPROVAL FROM OWNER/ENGINEER.

THE SUPERVISION OF A LICENSED PLUMBER.

11. CONTRACTOR SHALL IMPLEMENT TRAFFIC CONTROL PLAN TO MEET WITH **TXDOT REQUIREMENTS.** 



Practical Infrastructure Solutions TxEng F-315 | TxSurv F-10028600 | OSBPE/LS #603 | ASBPE #2521 | LA #EF6529 Texas I Oklahoma I Arkansas I Louisiana

**TECHNICAL SPECIFICATIONS** 

FOR

Central Bowie County Water Supply Corporation

FOR

Malta to SH 98 Utility Relocation & Highway Bore ROW CSJ#0919-19-094

June 2023

HEI #065006

EVIN R. VANHOOZIE

127925

IONAL EN

6-21-23

185824

4445 SE LOOP 286 PARIS, TEXAS 75460 (903) 785-0303



# 1. LOCATE ALL TIE-IN POINTS AND VERIFY THE O.D.'S AND TYPES OF ALL LINES

#### 2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING THE LOCATION OF EXISTING SERVICES. A NEW SERVICE IS REQUIRED FROM THE NEW WATER MAIN

#### 4. CONTRACTOR SHALL BE RESPONSIBLE FOR LOCATING AND PROTECTING ALL

## 5. CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING OVERHEAD UTILITY OWNER TO ENSURE PROTECTION OF EXISTING OVERHEAD UTILITIES. ANY COST

# 7. IN LIEU OF TRENCHING AND OPEN CUTTING, DRIVEWAY, AND STREETS, CAN BE

#### 8. NO TREES SHOULD BE REMOVED UNLESS NOTED ON PLANS OR UPON SPECIFIC

# 9. ALL WORK INVOLVING SERVICE LINES SHALL BE PERFORMED BY, OR UNDER

#### 10. VERIFY SERVICE TRANSFER WITH CBCWSC BEFORE PERFORMING TRANSFER.

4445 SE LOOP 286 PARIS, TEXAS 75460 (903) 785-0303

1.4 lbs.
1.5 lbs.

From September 15 to November 1, add 3 pounds per 1000 square feet of winter rye with a PLS = 0.83 to above mixture, and 1/2 lb. unhulled Bermuda. Preferably the seeding should be accomplished in early spring - no seeding will be allowed from November 1, to February 28, without specific approval of the ENGINEER.

Straw mulch shall be spread uniformly over the area indicated or as designated by the ENGINEER at the rate of 2 to 2-1/2 tons of straw per acre. The actual rate of application will be designated by the ENGINEER. Straw may be hand or machine placed and shall be adequately secured.

Cellulose and wood fiber mulch shall be spread uniformly over the area indicated or as designated by the ENGINEER at the rate of 45 to 80 lbs. per 1000 square feet.

It shall be the CONTRACTOR's responsibility to maintain the initial shape and contour of the entire seeded/sprigged area, irrespective of the project's 12-month maintenance period, until such time as a 75% average grass coverage has been obtained.

#### Cellular Grout

#### I – SCOPE OF WORK

This work consists of providing a non-pervious, Low Density Cellular Concrete (LDCC) to fill abandoned pipes or other structures at the location shown in the plans in accordance with the details in the plans and these specifications.

#### II – MATERIALS

- A. Portland cement shall comply with ASTM C150 (Type I, II or III).
- B. Fly ash shall be Class C or Class F and compatible with foaming agent.
- C. Water shall be free from deleterious substances.
- D. Foaming agent shall conform to ASTM C796.

E. Admixtures for water reducing, retarding, accelerating, and other specific properties may be used when recommended by the manufacturer of the foaming agent.

F. LDCC shall have the following properties:

Range of Cast Density, PCF 24-30 Minimum Compressive Strength (28 Days), PSI 40 Flow Consistency per ASTM D6107 Greater than 7"

#### III – SUBMITTALS

A. Mix design for LDCC, including materials to be used and their sources.

B. Resume of CONTRACTOR showing experience as specified below, including qualifications of CONTRACTOR's superintendent and/or foreman.

C. Description of equipment and placement methods to verify compliance with specifications.

#### **IV – PRODUCTION**

produce consistent foam with stable, uniform cellular structure. producing equipment, which is capable of meeting the specified properties. material shall be conveyed promptly in its final location. D. All equipment used must be approved by the foam manufacturer.

#### V – INSTALLATION

A. CONTRACTOR shall fill the existing abandonment as indicated in the plans or as directed by project engineer.

completely filled.

PSI or the allowable maximum pressure, whichever is greater and shall be water-tight. 3" male threaded NPT.

the next at pressures below maximum allowable pressure. procedure. If required, dewatering shall be continuous during installation. multiple lifts depending upon length and diameter of pipe being abandoned.

#### VI – QUALITY CONTROL AND QUALITY ASSURANCE

that is satisfactory to the Engineer. Including the following:

LDCC.

abandoned pipes or other structures. of LDCC under similar conditions.

#### **B. TESTING**

1. Testing to be performed by the OWNER or approved agency.

- A. Foam generating equipment shall be used to produce a predetermined quantity of pre-formed foam which shall be mixed and blended with cementitious slurry. Equipment shall be calibrated to
- B. LDCC shall be produced utilizing specialized automated proportioning, mixing, and foam
- C. Avoid excessive handling of the material. After sufficient mixing of the foam with slurry, the
- B. LDCC shall be pumped through injection ports located at bulkhead to allow abandonment to be
- C. All bulkheads and injection ports installed shall be capable of withstanding a minimum of 30
- D. Injection and vent ports and pipes must be securely installed and be able to receive a minimum
- E. Space injection points at intervals that allow material to be forced from one injection point to
- F. All water and other residual materials must be removed from pipe prior to initiating filling
- G. LDCC shall be placed in a manner so as not to cause collapsing of material, which may require
- A. CONTRACTOR shall have a record of experience and quality of work placing foam concrete
  - 1. Shall be capable of developing a mix design, batching, mixing, handling, and placing
  - 2. Shall be regularly engaged in the production and pumping of LDCC for filling
  - 3. Workers included the CONTRACTOR's superintendent and /or foreman, shall be fully qualified to perform the work and have had previous experience in production and pumping
  - 2. A minimum of four (4) 3"x 6" cylinders shall be molded for each shift of operation.



3. LDCC to be tested at any age after three (3) days for compressive strength. At least two (2) specimens from each set should be tested at 28 days in accordance with ASTM C-495 unless otherwise approved by engineer.

4. CONTRACTOR shall record and measure wet cast densities at the point of placement regularly. Mix shall be adjusted as required to obtain the specified cast density at the point of placement.

5. CONTRACTOR shall record and measure flow consistency regularly in accordance with ASTM D6107.

#### Coordination of Testing and Sterilization with Installation

The practice of "installing only", and delaying testing, sterilization, operation, and appurtenances until after completion will <u>not</u> be permitted and will cause items which are installed only to be disallowed on monthly partial payment estimates.

#### Preconstruction Video Documentation

A video recording of the site shall be done by the CONTRACTOR <u>before</u> beginning any construction. This requirement will be done to document pre-existing conditions prior to construction. One "thumb-drive" copy of the recording will be given to the ENGINEER prior to the beginning of construction.

#### **Buy American Accommodations**

Iron and steel products used on this project shall comply with TxDOT's requirements for BUY AMERICA. Information is provided on the next page that outlines the BUY AMERICA requirements.

## 135AB

## **TECHNICAL SPECIFICATIONS**

GENERAL
Intent1
Standard Reference Specifications1
Mobilization1
Preconstruction Conference
Schedule and Sequence of Construction
Shop Drawings and Submittals
Construction Water
Construction Electricity
Material and workmanship
Payment for Incidental Items
Coordination with Existing Utilities
Appurtenances
Facilities On and/or Across Highway Rights – of – Way
Easements and Sites
Public and Private Access
Storm Water Pollution Prevention (SWP3)7
Construction Stakes – Water Lines
Reuse of Material
Barricades, Signs, and Lighting
Sanitation8
Clean – Up
Record Drawings
Lead – Free Requirements
Construction Office
WATER PIPING
Reinforcement of Adjacent Existing Utilities10
Trench Excavation for Sanitary Sewer and Potable Water Mains10
Installation and Backfill of Buried Pipe13

Mechanically Tamped Backfill......17

Polyvinyl Chloride (PVC) Pipe for Potable Water Mains and Wastewater Force Mains	17
High Density Polyethylene Pipe	
Fittings and Blocking	19
Hydrostatic Testing – Water and Wastewater Pressure Mains	
Sterilization of Potable Water Mains	
Mechanical Joint Thrust Restraint	
Polyethylene Encasement of Ductile – Iron Piping and Appurtenances	
Corrosion Protection of Buried Threaded Components	
Exiting Water Service Transfers (New Main to Meter)	
Floatation Structures or Pipelines	
Existing Line Abandonment	
Brass Goods	
Galvanized Iron Pipe	
Solid Sleeves	
BORES	
Encasement Pipe	24
Bores	24
Carrier Pipe in Bores	25
Casing Spacers	25
VALVES	
Gate Valves	26
Valve Boxes	27
Gray Iron Castings	27
Tapping Sleeves and Valves	27
FLOW METERS	
Water Meter Boxes	
CONCRETE	
Concrete	
Expansion Joints	40
MISCELLANEOUS	
Clearing and Grubbing	41

Tracer Wire	42
Pipeline, Valve, and Appurtenances	42
Fencing Repairs	42
Pavement Repairs	43
Fine Grading	44
Project Guarantee	45
Silt Fence	46
Re-grassing	46
Cellular Grout	48
Coordination of Testing and Sterilization	50
Preconstruction Video Documentation	50
Buy American Accommodations	50

## **TECHNICAL SPECIFICATIONS**

#### **GENERAL**

#### Intent

It is the intent of these specifications to ensure that this CONTRACTOR will provide the OWNER with all material, labor, equipment, supervision, and administration of construction to complete an operative project as shown on the drawings and/or described herein.

All work required to affect these improvements is to be provided by this CONTRACTOR and no item of work or material will be furnished by the OWNER unless specifically set forth herein.

#### Standard Reference Specifications

These contract documents contain references to standard specifications as adopted by the American Waterworks Association (AWWA), the American Society of Testing Material (ASTM), the American Association of State Highway and Transportation Officials (AASHTO), the Texas Department of Transportation (TxDOT), and the American National Standards Institute (ANSI). All such references shall be construed to refer to the designated standard and the latest revision thereof, regardless of the specification date shown herein.

#### **Mobilization**

"Mobilization" shall consist of the mobilization of personnel, equipment and supplies at the project site and preparation for beginning work on other contract items. Mobilization shall include, but is not limited to, the movement of equipment, personnel, materials, supplies, etc. to the project site, the establishment of office and other facilities necessary prior to beginning the work, bonds, insurance, and similar preparatory costs.

Mobilization shall be considered a lump sum item. Partial payments for mobilization will be as follows: The "adjusted contract amount" for construction items as used below is defined as the total contract amount less the lump sum bid for mobilization.

- 1. case, less the retainage specified elsewhere in the contract) may be paid.
- 2. contract) may be paid.
- 3.

When 5% of the adjusted contract amount for construction items is earned, 75% of the mobilization lump sum bid or 5% of the total contract amount, whichever is less, (in either

When 15% of the adjusted contract amount for construction items is earned, the remainder of the mobilization lump sum bid not paid under (1) above or 10% of the total contract amount, whichever is less, (in either case less the retainage specified elsewhere in the

Any remainder of this bid item not paid under (1) or (2) will be paid upon completion.

The intention of allowing a bid item for mobilization is to give the CONTRACTOR the opportunity to recoup his "up front" costs in the event the OWNER decides to reduce or increase the scope of the project.

# Preconstruction Conference

A preconstruction conference will be held prior to the beginning of any construction. This meeting will be attended by the CONTRACTOR, the OWNER, the ENGINEER, all subcontractors, and any governmental agencies involved in administration of the project. The meeting date will be established by the ENGINEER after the project has been awarded. The successful bidder shall furnish to the ENGINEER a project schedule at this meeting, showing the major items or work to be constructed and the anticipated completion dates.

# Schedule and Sequence of Construction

Prior to beginning work, the CONTRACTOR shall prepare and submit a proposed schedule and sequence of construction to the ENGINEER for his review.

The CONTRACTOR shall construct the segments in a general order of West to East, starting with Line L, continuing with Line I, etc. until Line A. Depending on material acquisition, the CONTRACTOR may start multiple lines at once or start on eastern lines ahead of others, but priority shall be given to western lines to keep ahead of the trail construction.

In the case of pipelines, any segment or system, once started, shall be worked on continuously until it is completed and placed in service. The practice of installing all or most of the pipe before placing any of the system into service, shall be avoided. The CONTRACTOR'S compliance with this condition shall be a prime consideration in determining unit quantities allowed on partial payment estimates.

Where the OWNER provides water and sewer service, it is imperative to continue the operation of the existing facilities. It will be mandatory that this CONTRACTOR so arrange his work as to cause the absolute minimum inconvenience to the existing users.

# Shop Drawings and Submittals

Submit catalog data on the following (5 copies):

No.	Description	Date Submitted	Date Approved	Log No.
1	PVC Pipe			
2	Ductile Iron Fittings			
3	Mechanical Joint Thrust Restraints			
4	Gate Valves			
6	Tapping Sleeve & Valve			
7	Valve Boxes			
8	Tracer Wire			
9	Encasement Pipe (Steel & PVC)			

10	Casing Spacers	
11 Concrete Mix Design		
12	Cellular Grout	
13	HDPE Tubing	
14	Service Saddle	
15	Curb Stop	
16	Corp Stop	
17	Foster Adaptor	

Review of shop drawings and submittals will be performed only for general conformance with the design concept of the project and general compliance with the information given in the Contract Documents. Modifications or comments made on the shop drawings during the review do not relieve the CONTRACTOR from compliance with the plans and specifications. CONTRACTOR is responsible for confirming and correlation of dimensions; information that pertains solely to the fabrication process or to the means, methods, techniques, sequences, and procedures of construction; coordination of the work of all trades; and for performing all work in a safe and satisfactory manner.

Shop drawings and submittals provided that are not list above are not required and will not be reviewed or returned. The CONTRACTOR's level of responsibility is the same for all materials and equipment, irrespective of shop drawing submittal or review.

# Construction Water

This CONTRACTOR shall pay for all construction and testing water. He will pay the bill directly to the supplier.

# Construction Electricity

This CONTRACTOR shall pay for all construction and testing electricity. The construction meter shall be in the CONTRACTOR'S name, and he shall pay the power company directly.

# Material and Workmanship

All equipment, materials, and articles incorporated into the work shall be new, and of a quality equal to, or better than, specified herein. If not specified herein, the quality shall be satisfactory to the ENGINEER. All workmanship shall be of acceptable quality as determined by the ENGINEER, and the OWNER may require the CONTRACTOR to dismiss from the work any employee or employees the OWNER may deem incompetent, unqualified, unreliable, careless, or insubordinate.

# Payment for Incidental Items

For unit price contracts, the payment for all labor, materials, equipment, supervision, overhead, and associated costs to complete the project contemplated by the drawings and/or specifications shall be according to the unit items set forth in the Bid Schedule. Payment for items required, but

not set forth in the Bid Schedule as separate pay items, shall be considered as incidental and shall be included in the various items provided for in the Bid Schedule.

For lump sum contracts, the lump sum specified in the Bid Schedule shall include all necessary labor, materials, equipment, supervision, overhead and associated costs to complete the project contemplated by the drawings and/or these specifications, whether specifically listed on the Bid Schedule, or not.

#### Coordination with Existing Utilities

The location of existing buried utilities, where shown, are approximate only. There may exist utilities, which are not shown. This CONTRACTOR shall be responsible for locating and protecting all buried utilities. The existence of utilities not shown shall not be grounds for additional compensation.

The CONTRACTOR shall contact each utility company ahead of performing the work and request the utility to locate its lines. Failure of the utility to locate, or improper location by the utility, shall not result in additional cost to the OWNER.

All OWNER'S utilities damaged by the CONTRACTOR shall be repaired by the CONTRACTOR at his expense. Materials used in repair of utilities shall conform to these specifications. The CONTRACTOR shall notify any utility customer in advance of shutting-off his utility service. Existing water and sewer lines damaged shall be repaired within 4 hours on the day damaged. OWNER shall operate his existing water valves in his system. However, he does not guarantee 100% shut-off of existing water lines.

Where existing facilities of the OWNER or its customers are specifically designated to be connected to the facilities constructed herein, this CONTRACTOR shall do so under the OWNER'S supervision. If required by the OWNER, these connections will have to be made during the period from midnight to 6:00 a.m.

#### Appurtenances

Appurtenances shall be constructed as soon as the line of which they are a part is constructed to their locations. The construction of appurtenances in advance of the construction of the line will not be permitted, except:

- Tapping sleeves and valves 1.
- 2. Boring and encasement

Should the CONTRACTOR elect to install the bore and encasement prior to installing the adjacent pipeline, adjustments to grade or location of the bore/encasement required to properly accommodate the pipeline shall be at the CONTRACTOR'S expense.

# Facilities On and/or Across Highway Rights-of-Way

The OWNER has obtained permits from the District Engineer of the Texas Department of Transportation authorizing the installation of the facilities covered by the project plans. The CONTRACTOR may obtain a copy of such permits from either the OWNER or the ENGINEER. No facilities, other than the specifically authorized facilities, shall be placed on or across any highway right-of-way or any part thereof.

The CONTRACTOR shall hold harmless the Texas Department of Transportation and its duly appointed agents and employees, against any action for personal injury or damage to property sustained by reason of the exercise of the privileges granted by these permits.

During the construction of the authorized work the CONTRACTOR shall exercise all diligence in observing the following rules and safety precautions:

- 1. way.
- 2. to the highway roadway.
- 3. bore pits shall be approved by TxDOT.
- 4. by the appropriate permit.
- 5. be continuous with not splices.
- 6. of-way.

TxDOT reserves the right to have a representative present during installation of the authorized work and to inspect such installation during construction and upon completion thereof. Therefore, the CONTRACTOR shall notify the appropriate area Engineer at least forty-eight (48) hours prior to commencing any construction work on highway rights-of-

The authorized facilities shall be installed in accordance with the project plans. The location of the authorized facilities shall be changed only with the approval of TxDOT. Longitudinal pipelines shall be located on a uniform alignment and as near as practicable to the right-of-way line. Pipelines crossing a highway shall be approximately perpendicular

Authorized pipelines crossing the paved portion of the highways shall be put in place only by TxDOT approved methods. Water jetting methods shall not be allowed. Location of

Authorized pipelines crossing the paved portion of the highways shall be encased as shown

Authorized pipelines laid in open cut trenches on highway rights-of-way shall have a #14 bare, solid copper wire installed concurrently with the pipe for pipe detection purposes. The wire shall terminate where an authorized line crosses a highway right-of-way line. At this point, the wire shall be fastened to the  $1" \times 1" \times 1/8"$  galvanized angle iron that supports the "Pipeline" marker described below as shown by the project plans. The tracer wire shall

The CONTRACTOR shall place a "Pipeline" marker at every point where an authorized line crosses a highway right-of-way line. Where new lines parallel the edge of the rightof-way line, the marker shall be placed as close as possible to the fence or edge of right-

- 7. The CONTRACTOR shall be responsible for locating and protecting any and all existing buried facilities along, across or near the locations of the authorized facilities and for arranging with TxDOT for any adjustment of the plan location of the authorized facilities necessary to protect such existing facilities. The decision of the TxDOT representative, as to the location of the authorized facilities, shall be final.
- 8. The CONTRACTOR shall adequately mark all points of potential hazard to traffic, resulting in any manner from the construction of the authorized facilities, with barricades and signs.
- 9. The CONTRACTOR shall not, at any time, block a roadway traffic lane without first having the proper barricades and signs in place and providing a flagman or police protection or both if considered necessary by TxDOT.
- The CONTRACTOR shall take all precautions necessary to require all workers to park 10. vehicles, including CONTRACTOR'S equipment, well off the paved portion of the highways, including shoulders, and shall always keep all excavated material off the pavements.
- The CONTRACTOR shall compact the backfill, in all open cut trenches on highway rights-11. of-way, to densities equal to the densities of surrounding undisturbed areas.
- All above-ground appurtenances (valve boxes, manholes, etc.) on highway rights-of-way, 12. shall be located flush with the finished grade.
- From time to time, during construction of the authorized facilities the CONTRACTOR 13. shall remove, and keep removed, surplus materials and debris from highway rights-of-way, to the satisfaction of the TxDOT representative. Upon completion of the authorized lines, all highway rights-of-way shall be promptly and thoroughly cleaned up and left in a condition at least equal to their condition prior to commencement of construction of the authorized lines.

# Easements and Sites

The CONTRACTOR shall be solely liable for any damage to property or improvements because of the construction of this work. This CONTRACTOR will cause to be protected all improvements on or off the right-of-way, such as fences, buildings, signs, poles, utilities, trees, shrubs, etc., unless designated for removal, and should such improvements be damaged, he shall make restitution to the satisfaction of their OWNER. The CONTRACTOR shall be responsible for such improvements that must be relocated or temporarily moved and shall do such work at his cost.

# Public and Private Access

This CONTRACTOR shall maintain vehicular access to all private property.

All private driveways not located within state right-of-way will be open-cut and repaired as set forth herein. No driveway will be closed overnight.

All county roads or streets will be open-cut and repaired as set forth herein. No street shall be closed for a period of more than four hours.

No federal or state highways, or railroads shall be closed. All such crossings shall be bored, jacked, pushed, or otherwise crossed in strict accordance with the OWNER'S permits.

CONTRACTOR shall notify appropriate law enforcement and emergency medical services agencies, and local school district, of all road closings.

# Storm Water Pollution Prevention Plan: (SWP3)

The CONTRACTOR shall be responsible for preparing and implementing the Storm Water Pollution Prevention Plan (SWP3). Construction projects located in the State of Texas that disturb one acre or more of land and are subject to storm water discharges require a Storm Water Pollution Prevention Plan (SWP3) under the conditions of TPDES General Permit No. TXR150000, "General Permit to Discharge Waste under provisions of Section 402 of the Clean Water Act and Chapter 26 of the Texas Water Code".

If the disturbed site area is greater than 5 acres, the CONTRACTOR shall file a Notice of Intent (NOI) using the online e-permitting system (STEERS - https://www3.tceq.texas.gov/steers/) before starting construction work and a Notice of Termination (NOT) using the same online epermitting system (STEERS - https://www3.tceq.texas.gov/steers/) upon completion of the project and final stabilization of the construction site. Filing the NOI will require the payment of a \$100 fee by the CONTRACTOR. The NOI shall be posted on site. If the disturbed site is larger than 1 acre, but less than 5 acres, the CONTRACTOR shall post the required notice on site, in accordance with the conditions of TPDES General Permit TXR 150000.

As part of the SWP3 the CONTRACTOR shall be named as the "Operator with Responsibility" for the SWP3. The "Operator with Responsibility" has day-to-day operational control over the activities at the construction site necessary to ensure compliance with the SWP3 and other conditions of TPDES General Permit TXR 150000. As the defined "Operator with Responsibility", the CONTRACTOR shall comply with all conditions of TPDES General Permit No. TXR150000.

The CONTRACTOR shall begin re-grassing or site stabilization within fourteen (14) days after the construction activity in that portion of the site has permanently ceased.

The CONTRACTOR shall begin re-grassing or site stabilization within fourteen (14) days after the construction activity in that portion of the site has temporarily ceased, unless construction activity on a portion of the site is temporarily ceased and earth disturbing activities will be resumed within twenty-one (21) days.

The silt fences shown on the drawings are a method the CONTRACTOR may use to comply with the requirements of TPDES Permit No. TXR 150000. The CONTRACTOR shall amend the SWP3 as necessary to comply with the requirements of TPDES Permit No. TXR 150000 as they pertain

to this construction site. All amendments to the SWP3 must be consistent with the requirements of TPDES General Permit TXR 150000.

#### Construction Stakes - Water Lines

The alignment is shown on the drawings.

The CONTRACTOR shall cause construction alignment stakes to be set, at his expense, at intervals not greater than 300 feet. These are to be set after all clearing, grubbing and rough grading is complete. The CONTRACTOR shall preserve these marks and if they are lost or damaged, he shall reset them at his own expense. All other construction stakes, both line and grade of every nature, shall be furnished by the CONTRACTOR at his expense. The CONTRACTOR shall furnish, at any time, to the OWNER'S representative a surveyor's transit and one laborer, should the OWNER'S representative desire to check line or grade. Such checking shall not relieve the CONTRACTOR of his responsibility to construct every item in accordance with the drawings and specifications.

# Reuse of Material

No used material, or material which has been used by the CONTRACTOR for any temporary purpose whatever is to be incorporated in the permanent structure.

# Barricades, Signs, and Lighting

The CONTRACTOR shall furnish, erect, and maintain adequate warning devices and lighting to protect the public from all construction hazards arising from the building of this project. He shall be totally responsible and solely liable for their adequacy and for any claims or accidents resulting from his failure to provide and maintain them properly.

All traffic control signs or devices used for protection of construction workers shall conform to Part VI of the Manual of Uniform Traffic Control Devices, 1988 Edition, Revision 3, September 3, 1993, FHWA-SA-94-027 or Part VI of the manual on Uniform Traffic Control Devices, Millennium Edition, December 2000, FHWA, which are incorporated by reference (OSHA 29 CFR Part 1926).

# Sanitation

Necessary sanitation conveniences for the use of laborers on the work site shall be furnished and maintained by the CONTRACTOR in acceptable locations properly secluded from the public, and their use shall be enforced.

# Clean-Up

At the conclusion of the work, and prior to final payment, all tools, temporary structures, and materials belonging to the CONTRACTOR shall be promptly removed. All rubbish and other foreign substances shall be removed from the project site and satisfactorily disposed of. All job

sites including streets, easements, rights-of-way, roads, and areas used by the CONTRACTOR shall be left in a clean, neat condition satisfactory to their owner. The OWNER'S representative may also require clean-up of the job site or easements and rights-of-way periodically during the progress of the work.

Burning of brush on the work sites will not be permitted without the landowner's permission, and then only in strict compliance with local, state, and federal laws. Any surplus excavated earth not designated for reuse shall be disposed of at approved locations secured by the CONTRACTOR at his expense, and rough leveled to the landowner's satisfaction.

Any regressing, sodding, or reseeding required shall be in accordance with landscape requirements elsewhere herein.

# Record Drawings

During progress of the job, the CONTRACTOR shall mark up a set of the drawings, including dimensions, to indicate any deviations from the contract drawings. After completion of the job, this set of record drawings is to be furnished to the ENGINEER for review, including sufficiency of information, clarity, and readability.

The ENGINEER will periodically check with the CONTRACTOR to verify compliance with this requirement. Failure to comply will result in delay of monthly progress payments.

# Lead-Free Requirements

All metallic components in contact with raw or potable water shall conform to NSF-61 and NSF/ANSI 372 ("lead free" requirement of the Reduction of Lead in Drinking Water Act).

# Construction Office

The CONTRACTOR shall furnish and maintain, during construction of the improvements embraced in this contract, an adequate temporary field office on the project site, or adjacent thereto, for the use of his representatives and personnel. He shall not be required to provide space for the ENGINEER or the OWNER'S on-site representative.

The CONTRACTOR shall maintain in this office a complete set of construction drawings, shop drawings, equipment details, specifications, working record drawings, change orders, etc.

Upon completion of the improvements, or as directed by the ENGINEER, the CONTRACTOR shall remove all such temporary structures and facilities from the site, they shall become the CONTRACTOR'S property, and he shall leave the site of the work in the condition required by the contract.

# WATER PIPING

#### Reinforcement of Adjacent Existing Utilities

Existing cast iron, or pressurized PVC utilities approximately at right angles to this CONTRACTOR'S trench, will not require additional support or protection, other than the exercise of care in placing new facilities adjacent to same, and in placing backfill, except when the unsupported span is over six feet (6'), in which case the existing pipe shall be supported by concrete piers at no added cost to the OWNER.

Vitrified clay, concrete pipe, and/or gravity PVC utilities, approximately at right angles to the ditch, will be removed to firm undisturbed ground on each side of the trench and the removed section replaced with epoxy-lined ductile iron pipe or pressure-type PVC pipe without additional compensation. Note that for sewer lines crossing potable water mains, more restrictive requirements are stated elsewhere herein.

Concrete, PVC, ductile iron, cast iron, steel, asbestos cement, and clay utilities parallel to and in the edge of the trench, shall be adequately supported without additional compensation.

# Trench Excavation for Sanitary Sewer and Potable Water Mains

The CONTRACTOR shall perform all necessary trench excavation. Unless otherwise specified herein, trench excavation will comply with the Trench Excavation section of ASTM D-2321. There shall be no separate classification of excavation.

In general, all pipeline excavation shall be made by open cut from the surface of the ground and shall be no greater in width or depth than is necessary to permit the proper construction of the work. The sides of the trench shall be cut and maintained as nearly vertical as is feasible to a point at least 12 inches above the top of the pipeline. If the trench walls are to be sloped for trench safety purposes (where other forms of trench safety are not required), then sloping may begin at this point.

The entire foundation area in the bottom of the excavation shall be firm, stable and of uniform density. A soil is stable if it provides dependable support for the pipe and undergoes only slight volumetric change with variation in its moisture content.

Unstable soil conditions in trench bottoms shall be stabilized before laying the pipe. As a minimum, this is accomplished by over-excavating the trench bottom to remove the unstable material and bringing it back to grade with appropriate bedding material. Additional stabilization may be required, including French Drains, well points, concrete backfill, or other to be agreed upon by the ENGINEER. No additional compensation is allowed for stabilization.

Materials shall not be disturbed below grade, except soft, wet, disintegrated, or other unsuitable materials which shall be removed to a depth below grade as is directed by the ENGINEER. Any rock or other extremely hard materials under the foundation area shall be removed to a depth not less than 6" below grade.

Excavate trenches to ensure that sides will be stable under all working conditions. Slope trench walls or provide supports in conformance with all state and national standards for safety. Open only as much trench as can be safely maintained by available equipment. Backfill all trenches as soon as practicable, but not later than the end of each working day.

Do not lay or embed pipe in standing or running water. At all times prevent flows from sewers, storm drains, runoff, and surface water from entering the trench. The providing of a proper foundation will be at the CONTRACTOR'S expense. Embedment for pipe, structure foundations, or the pipeline itself shall <u>not</u> be laid or poured in standing or running water, or on unstable foundations.

When standing or running water from any source is present in the work areas, dewater to maintain stability of in-situ and imported materials. Maintain water level below pipe bedding and foundation to provide a stable trench bottom. Use, as appropriate, sump pumps, well points, deep wells, geofabrics, perforated underdrains, or stone blankets of enough thickness to remove and control water in the trench. Dewatering shall be considered as incidental work and will not be paid for as separate items. When excavating while depressing groundwater, ensure the groundwater is always below the bottom of cut to prevent washout from behind sheeting or sloughing of exposed trench walls. Maintain control of water in the trench before, during, and after pipe installation, and until embedment is installed and enough backfill has been placed to prevent floatation of the pipe. To preclude loss of soil support, employ dewatering methods that minimize removal of fines and the creation of voids in in-situ materials.

Control running water emanating from drainage of surface or groundwater to preclude undermining of the trench bottom or walls, the foundation, or other zones of embedment. Provide dams, cutoffs, or other barriers periodically along the installation to preclude transport of water along the trench bottom. Backfill all trenches after the pipe is installed to prevent disturbance of pipe and embedment.

Use suitably graded materials in foundation or bedding layers or as drainage blankets for transport of running water to sump pits or other drains. Use well graded materials, along with perforated underdrains, to enhance transport of running water, as required. Select the gradation of the drainage materials to minimize migration of fines from surrounding materials.

Where trench walls are stable or supported, provide a width enough, but no greater than necessary, to ensure working room to properly and safely place and compact hauching and other embedment materials. The space between the pipe and trench wall must be wider than the compaction equipment used in the pipe zone. Maximum trench widths shall be as shown on the drawings. In addition to safety considerations, trench width in unsupported, unstable soils will depend on the size and stiffness of the pipe, stiffness of the embedment and in-situ soil, and depth of cover. Specially designed equipment may enable the satisfactory installation and embedment of pipe in trenches narrower than specified above. If it is determined that the use of such equipment provides an installation consistent with the requirements of this standard, minimum trench widths may be reduced, as approved by the ENGINEER.

When supports such as trench sheeting, trench jacks, trench shields or boxes are used, ensure that support of the pipe and its embedment is maintained throughout installation. Ensure that wall sheeting is sufficiently tight to prevent washing out of the trench wall from behind the sheeting. Provide tight support of trench walls below viaducts, existing utilities, or other obstruction that restrict driving of sheeting.

Unless otherwise directed by the ENGINEER, sheeting shall not be placed in or below the pipe zone to preclude loss of support of foundation and embedment materials. When specifically allowed by the ENGINEER, sheeting may be placed in or below the pipe zone, with the top of sheeting to be cut off 1.5 feet or more above the crown of the pipe. Leave rangers, whalers, and braces in place as required to support cutoff sheeting and the trench wall in the vicinity of the pipe zone. Timber sheeting to be left in place is considered a permanent structural member and should be treated against biological degradation (for example, attack by insects or other biological forms) as necessary, and against decay if above groundwater.

Do not disturb the installed pipe and its embedment when using movable trench boxes and shields. Movable supports should not be used below the top of the pipe zone unless approved methods are used for maintaining the integrity of embedment material. Before moving supports, place, and compact embedment to enough depths to ensure protection of the pipe. As supports are moved, finish placing and compacting embedment.

If the ENGINEER permits the use of sheeting or other trench wall supports below the pipe zone, ensure that pipe and foundation and embedment materials are not disturbed by support removal. Fill voids left on removal of supports and compact all materials to required densities.

If ledge rock, hard pan, shale, or other unyielding material, cobbles, rubble or debris, boulders, or stones larger than 1.5 in. (40 mm) are encountered in the trench bottom, excavate a minimum depth of 6 in. (150 mm) below the pipe bottom and replace with proper embedment material.

The CONTRACTOR shall be responsible for the satisfactory disposal of excess and unsuitable materials of any sort, and shall be responsible for backfilling, tamping, compacting, and refilling after settlement, of all excavated areas and other land, private and public, damaged or occupied by the CONTRACTOR in the performance of the contract, to as good condition as they were prior to the beginning of the work. It shall be his further responsibility to remove all surface obstruction to his work on easements or sites. He shall protect all pipes, conduits, signs, utility poles, wire, fences, building, and other public or private property improvements adjacent to or in the line of the work.

Trench backfilling as it relates to pavement repair subbase:

- Due to the varying depths of trenching and the varying types of native soils, the backfill under trenched areas that will receive pavement repair will conform to the requirements shown in the detail drawings.
- All backfill beneath trenched areas that are to receive pavement repair will be uniformly mechanically compacted in maximum 8" loose lifts to a minimum of 95% standard Proctor density at -1 to +3% of optimum moisture. The backfill material will be as shown in the

detail drawings. Hand-operated pneumatic or gasoline-powered equipment will be required for adequate compaction around valve stacks, manholes, cleanouts, etc.

allowed so that pavement repair at the top of the trench is minimized.

#### Installation and Backfill of Buried Pipe

CONTRACTOR shall comply with the "Materials", "Installation", and "Inspection, Handling and Storage" sections of ASTM D-2321, except as modified herein.

Excavation shall be as described elsewhere in these specifications and as shown on the drawings.

Install foundation and bedding as shown in the drawings and as specified herein, according to conditions in the trench bottom. Provide a firm, stable, and uniform bedding for the pipe barrel and any protruding features of its joint. Provide a minimum 4-inch thickness of bedding unless a greater thickness is specified.

When rock or unyielding material is present in the trench bottom, increase the thickness of bedding material to 6 inches minimum.

Where the trench bottom is unstable or shows a "quick" tendency, excavate all unstable material as a minimum, and replace with a foundation of material per Table I. Place and compact this foundation material in accordance with the drawings. Control of quick and unstable trench bottom conditions may also be accomplished with the use of appropriate geofabrics applied to the satisfaction of the ENGINEER and in strict accordance with the manufacturer's recommendations. For severe conditions, the ENGINEER may require a special foundation such as piles, sheeting, or concrete mats. Correction of unstable trench bottom shall not result in increased cost to OWNER.

Minimize localized loadings and differential settlement wherever the pipe crosses other utilities or subsurface structures. Provide a cushion of bedding material between the pipe and any such point of localized loading.

If the trench bottom is excavated below intended grade, fill the over-excavation with compatible foundation or bedding material and compact to a density not less than the minimum densities given in the drawings.

If trench sidewalls slough off during any part of excavating or installing the pipe, remove all sloughed and loose material from the trench.

All pipeline excavation made in one day shall have the pipe installed and backfilled the same day. Excavation of trench days ahead of pipe laying will not be permitted.

Excavations for pipeline appurtenances shall be backfilled as soon as is feasible.

• In all trenched areas that are to receive pavement repair, the pipe will be installed in trenches with near-vertical walls employing OSHA-approved trench-safety methods (hydraulic wall jacks, trench boxes, etc.). Sloping the sides of the trench will not be Place pipe and fittings in the trench with the invert conforming to the required elevations, slopes, and alignment. Ensure uniform pipe support. In special cases where the pipe is to be installed to a curved alignment, maintain angular joint deflection or pipe bending radius, or both, as indicated in the drawings.

Adjustment in grade shall be made by scraping away or filling in along the full length of the pipe with approved bedding material, properly compacted, and not by wedging, blocking up the conduit, or supporting the conduit on mounds of earth.

The width of trench excavation is set forth in the drawings and shall <u>not</u> be exceeded in the area from the bottom of the trench to a point 12" above the top of the pipe.

Pipe shall be laid with the bell pointing away from the last joint. Pipe sockets and barrels shall always be clean and free from dirt.

Comply with manufacturer's recommendations for assembly of joint components, lubrication, and making of joints. When pipe laying is interrupted, secure piping against movement and seal open ends to prevent the entrance of water, mud, or foreign material.

Mark, or verify that pipe ends are marked, to indicate insertion stop position, and ensure that pipe is inserted into pipe or fitting bells to this mark. Push spigot into bell using methods recommended by the manufacturer, keeping pipe true to line and grade. Protect the end of the pipe during homing and do not use excessive force that may result in over-assembled joints or dislodged gaskets. If full entry is not achieved, disassemble, and clean the joint and reassemble. Use only lubricant supplied or recommended for use by the pipe manufacturer.

When making solvent cement joints, follow recommendations of both the pipe and solvent cement manufacturer. If full entry is not achieved, disassemble, or remove and replace the joint. Allow freshly made joints to set for the recommended time before moving, burying, or otherwise disturbing the pipe.

When the pipe joint has been checked for line and grade, the bedding material shall be applied to the springline of the pipe or as set forth in the drawings, and compacted by hand tamps or handheld mechanical tamps, taking care not to damage or displace the pipe.

On pressure lines, all gasket joint or mechanical joint dead ends, fittings, flush valves, fire hydrants and offsets shall have Class D concrete thrust reaction blocking placed to provide for pressure reaction, as set forth in the drawings.

Where specifically called for on the drawings, additional joint restraints shall be provided.

All pipe shall then be further embedded and backfilled as set forth in the drawings and/or as described herein.

On highway right-of-way, backfill methods shall conform to the OWNER'S permits or as set forth herein and in the drawings, whichever is more restrictive.

All embedment material shall be per Table I. It shall not be the "grassed-off" overburden stripped from the site, but may be trench "tailings", provided these meet the requirements of Table I.

Where suitable embedment material, and/or suitable material for backfilling does not exist on the job site, the CONTRACTOR shall furnish such material from off the job site at his expense, and also dispose of the unsuitable material removed from the trench off of the job site, at his expense.

Place embedment materials by methods that will not disturb or damage the pipe. Work in and tamp the haunching material in the area between the bedding and the underside of the pipe before placing and compacting the remainder of the embedment in the pipe zone. Follow compaction requirements in the drawings for density requirements. Do not permit compaction equipment to contact and damage the pipe. Use compaction equipment and techniques that are compatible with materials used and location in the trench. Before using heavy compaction or construction equipment directly over the pipe, place enough backfill to prevent damage, excessive deflections, or other disturbance of the pipe. See minimum cover requirements elsewhere herein. Pipe embedment shall be compacted in maximum 8" loose lifts.

All excavations shall be backfilled to finished grades, shapes, and configurations as shown on the drawings. Finish backfill, after consolidation, shall not have a variance of over one inch in ten feet when measured with a straight edge perpendicular to the slope. Any subsequent settlement of finished areas shall be brought back to the final grade and configuration with additional material as required, so that upon the completion of the construction of the project, and for the twelve-month maintenance period, all areas shall have their final grade and shape. The CONTRACTOR, during the life of this contract, shall be responsible for so maintaining the trench and trench backfill as to permit safe passage for vehicles and pedestrians over the same. During inclement weather, he shall be responsible for removing any vehicle or livestock which may become stuck, stalled, or stranded in a trench, or in trench backfill, and shall utilize every available means to keep trenches and trench backfill across public thoroughfares and driveways safe and passable at all times.

Initial backfill, i.e., that backfill material used to a point at least 6 inches above the top of the pipe, shall be the same material as the embedment material. Unless otherwise noted on the drawings, the final backfill may be the native material removed from the trench, except that rock removed from the trench must be reduced to a size no larger than 3" maximum dimension before reuse. Broken concrete larger than 3" in maximum dimension, trees, or other lumpy materials, or materials frozen, or of a perishable, spongy, or otherwise improper nature shall never be used in the backfill.

Consolidation of cohesionless material by watering (jetting or puddling) shall only be used under controlled conditions when approved by the ENGINEER. At all times, conform to the lift thicknesses and minimum density requirements in the drawings.

To preclude damage to the pipe and disturbance to pipe embedment, a minimum depth of backfill above the pipe should be maintained before allowing vehicles or heavy construction equipment to traverse the pipe trench. For embedment materials installed to the minimum densities required in the drawings, provide cover (that is, depth of backfill above top of pipe) of at least 30 in. (0.8 m)

or one pipe diameter (whichever is larger) before allowing vehicles or construction equipment to traverse the trench surface.

All pressure conduits shall have a minimum cover as follows: 3" diameter and smaller - 30"; 4" thru 8" - 36"; and 10" and larger - 42", as measured from the natural ground surface to the top of the pipe. Pressure pipe buried in unstratified rock may be raised 6" upon authorization from the ENGINEER. Notwithstanding the above, all pipe must be buried a minimum of 36" below all highway and railroad borrow ditches.

Changes in depth of cut for pressure lines, necessitated by conflicting existing underground utilities will not be a basis for additional compensation, but will be considered as incidental work. Such changes shall not reduce the specified pipe cover.

All pressure conduit crossing creeks shall be installed by directional drilling. The minimum cover shall be 48" and the maximum shall be 60".

All gravity pipelines shall have the depth of bury specified in the detail drawings.

Provide support for vertical risers as commonly found at sewer service connections, cleanouts, and drop manholes to preclude vertical or lateral movement. Prevent the direct transfer of thrust due to surface loads and settlement and ensure adequate support at points of connection to main lines.

When excavating for a service line connection, excavate material from above the top of the existing pipe before removing material from the sides of the pipe. Materials and density of service line embedment should conform to the specifications for the existing line, or with this specification, whichever is more stringent.

Secure caps and plugs to the pipe to prevent movement and resulting leakage under test and service pressures.

Use flexible water stops, resilient connectors, or other flexible systems approved by the ENGINEER to make watertight connections to manholes and other structures.

Pipe shall be protected during handling against impact shock and free fall. No cracked or damaged pipe or joint shall be installed in the line. Handle and store pipe and fittings in accordance with recommendations of the manufacturer.

If faults, caverns, or subsidence are discovered during construction, all work shall halt, and the ENGINEER will be contacted to inspect.

EMBEDMENT N
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	BLE I					
	RIAL REQUIREMENTS					
Gravity Sewer Mains						
Particle Size Limitations           Sieve Size         % Passing						
<u>1 1/2"</u>	<u> </u>					
#4 90-100						
#200	0-10					
	(P.I.) less than 12					
	s Smaller Than 6"					
	ze Limitations					
Sieve Size						
1 1/2"	100					
#4	50-100					
#200	5-50					
Plasticity Index (P.I.) less than 25						
Pressure Mair	Pressure Mains 6" and Larger					
Particle Siz	Particle Size Limitations					
Sieve Size	Sieve Size % Passing					
1 1/2"	100					
#4	10-100					
#200	0-10					
	(P.I.) less than 12					
	ial (When Required)					
Particle Siz	ze Limitations					
Sieve Size	% Passing					
1 1/2"	100					
#4	10-50					
#200	0-5					
Plasticity Index	x (P.I.) less than 9					

#### Mechanically Tamped Backfill

Where specifically called for in these specifications or on the drawings, the material used in the backfill process shall be uniformly, mechanically tamped, with hand-operated pneumatic or gasoline tools in 6" to 8" loose lifts, to 95% of standard Proctor density (ASTM 698) at slightly above optimum moisture. Mechanical compaction shall extend to the finished ground surface.

# Polyvinyl Chloride (PVC) Pipe for Potable Water Mains and Wastewater Force Mains

Plastic pipe shall be manufactured from material meeting the requirements of ASTM D-1784, 12454B compounds. The finished pipe shall conform to ASTM D-2241, the gasket joint to ASTM D-3139 and the gaskets to ASTM F-477. Each joint shall bear the seal of approval of NSF, indicating that the pipe is suitable for conveying potable water. The pipe shall bear markings on each joint showing the pressure rating, type, grade and manufacturer's run or lot. Manufacturer's certificate of test and compliance shall be furnished on all PVC pipe.

Any plastic pipe field inspected and found not to conform as to wall thickness or diameter shall be rejected, and all pipe of the same run or lot number shall be rejected and replaced with acceptable pipe at the CONTRACTOR'S expense. Field tests will be made on random samples of pipe.

14" through 36" PVC shall conform to Unibell PVC Pipe Association specification Uni-B-11 and shall have the pressure rating and DR specified on the drawings and/or bid schedule. Pipe shall have cast iron O.D. and rubber gasket joints.

6" through 12" PVC shall be either Class 160 SDR 26 gasket-joint, Class 200 SDR 21 gasket joint, or AWWA C-900 DR 18 or DR 25, whichever is specified in the drawings and/or the Bid Schedule. Where AWWA C-900 pipe is specified, it shall have cast iron equivalent O.D., and be listed by Underwriters Laboratories as well as NSF. It shall be furnished in 20' lengths, with rubber gasket joints.

3" and 4" PVC shall be Class 200 SDR 21, as shown on the drawings and/or bid schedule and shall be gasket joint. 2-1/2" and smaller PVC shall be Class 200, rubber-ring gasket joint, SDR 21, with integral bell.

All physical and chemical tests shall be made at 73 degrees Fahrenheit. These tests shall include, but are not limited to, quick burst test, sustained pressure test, acetone immersion test, vice test, and drop impact test as set forth in applicable ASTM and NSF Standards.

Plastic pipe delivered to the project well in advance of its installation shall be protected from excessive warpage, discoloration, and heat deformation. Pipe shall not be strung for laying more than thirty days in advance of the laying time. Ends of plastic pipe shall be protected so that same shall not be scratched or abraded prior to installation. Any damaged ends shall be removed prior to installation.

Pipe may be cut by using a medium tooth saw and miter box. Fine emery cloth or sandpaper shall be used to remove burrs.

No male threaded PVC pipe will be allowed. Solvent-weld pipe shall be adequately snaked to allow for expansion or contraction.

# High Density Polyethylene Pipe

High density polyethylene (HDPE) pipe and fittings shall be a PE 3408 high density, extra molecular weight polyethylene pipe, specified by ASTM D3350 as having a cell classification of PE 345434C. Dimensions and workmanship are specified by ASTM F714. The finished pipe shall conform to AWWA C-906.

HDPE pipe shall have all joints welded by mans of butt-fusion providing a continuous nongasketed pipeline, capable of hydrostatic pressure testing to 12 times its designed pressure rating.

Diameter and Standard Dimension Ratio (SDR) shall be as shown on the drawings.

Anchoring system shall be as shown on the drawings.

HDPE pipe shall be Driscopipe 1000 as manufactured by Phillips Driscopipe, Inc. or approved equal.

# **Fittings and Blocking**

All 6" and larger cast iron, ductile iron, asbestos cement pressure and gravity pipe, and PVC pressure pipe, shall have ductile iron (not gray iron) fittings. These fittings shall conform to the American National Standard for Ductile-Iron and Gray-Iron Fittings, 3" through 48", for Water and Other Liquids, AWWA Standard C110, or the American National Standard for Ductile Iron Compact Fittings 3" through 24" for water service, AWWA C153. Ductile iron fittings shall be "Tyler", "U.S. Pipe", "American Pipe", "Sigma" or approved equal, and shall be manufactured in the USA. Each fitting shall have cast on its exterior the appropriate AWWA designation.

MJ fittings are required and shall be rated for 350 psi working pressure. Flange fittings will be rated for 250 psi minimum working pressure, and will have ANSI Class 125 flanges, unless otherwise specified.

The ductile iron fittings shall have a 1/16 inch cement-lining and asphaltic seal coat all meeting AWWA C-104.

For gravity service, the ductile fittings shall be rated for a minimum of 125 psi working pressure.

Bolts and nuts for mechanical joints or flanged ends shall be of high-strength, zinc-plated carbon steel, and shall conform to ASTM Designation A 307 (Grade B).

All buried ductile iron fittings shall be mechanical joint. All pressure fittings above ground, inside of structures, or on disturbed foundations shall be flanged.

All mechanical joint fittings shall be blocked with concrete (as shown in the detail drawings), then wrapped with polyethylene. Concrete blocking shall be placed at the time the fitting is installed.

SDR 35 PVC gravity sewer pipe shall have SDR 35 PVC gasketed push-on fittings. PVC fittings shall conform to ASTM resin specification D-1784, the dimensions shall conform to ASTM D-3034, the gasket shall conform to ASTM D-3212 and the joint material shall meet the requirements of ASTM F-477.

SDR 26 PVC pressure rated pipe (ASTM D-2241) used for gravity service shall use SDR 26 PVC gasketed fittings.

Where fittings are specified in the bid schedule to be paid by weight, payment will be based on the weight of the fitting only, exclusive of gaskets, bolts, nuts, glands, blocking and so forth. Where fittings are not specifically itemized on the bid schedule, they are considered incidental to the other items of construction shown.

The weight of fittings listed in the bid schedule is based on "compact" fittings unless "full body" fittings are specified.

#### Hydrostatic Testing - Water and Wastewater Pressure Mains

After backfilling mains and before pavement repairs, each section of pressure line constructed shall be tested with a hydraulic test pressure of not less than 150 psi over a continuous period of not less than four hours. The CONTRACTOR shall furnish adequate equipment to make these tests. The test pressure shall not be allowed to fall below 140 psi, at which pressure the pump will be started and the line loss measured directly by tank measurement or read off a totalizing meter. Re-pressurizing shall be done each hour, or sooner, as may be required to maintain the test pressure within the prescribed limits. The final re-pressurizing will be made at the end of the final hour of the test. Total water used will be the sum of the quantities required to re-pressurize the line to the original test pressure. 100% of the pressure mains laid will be tested. New meter service lines will be tested. Where existing service lines are reused and transferred to the new main, the connection of the corporation stop into the new main will be done before the pressure test, but the existing service line shall not be tested.

During the filling of the mains, and before applying the test pressure, all air shall be expelled from the mains.

If the test indicates a leakage more than a rate equal to 10 gallons per inch of diameter of the pipe per mile over a 24-hour period, then the CONTRACTOR shall be required to find the leaks and eliminate same. All known leaks shall be stopped, regardless of allowable leakage.

Tests may be combined with sterilization and shall be observed by representatives of the interested parties.

# Sterilization of Potable Water Mains

All newly constructed or existing repaired water lines and appurtenances shall be sterilized for 24 hours at 50 ppm available chlorine as per AWWA C-651-99, by this CONTRACTOR before the lines are placed into service. After completion of the pipelines, or any valved section thereof, the main shall be flushed and then chlorinated. Any of the following methods of procedure (arranged in the order of preference) shall be followed, subject to the approval of the ENGINEER:

Chlorine Gas-Water Mixture Sodium Hypochlorite and Water Mixture Calcium Hypochlorite and Water Mixture

The use of dry calcium hypochlorite in the pipe is not allowed. Upon completion of sterilization, all lines shall again be flushed, until the chlorine residual at the sterilized section terminus is less than 2 ppm. One bacteriological sample per 1000 feet of new pipe shall be collected by the CONTRACTOR who shall arrange and pay for their testing at an acceptable laboratory. Written test results shall be supplied to the ENGINEER.

The following table provides an estimate of chlorine required to produce a 50 ppm concentration, per 100 feet of pipe. It is provided for field reference only, and adherence thereto does not relieve the CONTRACTOR of full responsibility for a successful test:

Pipe Size (Inches)	100% Chlorine (Pounds)	1% Chlorine Solution (Gallons)
4	0.027	0.33
6	0.061	0.73
8	0.108	1.30
10	0.170	2.04
12	0.240	2.88

#### Mechanical Joint Thrust Restraint

Thrust restraint mechanical joint follower glands shall be used to restrain plain end pipe in all ductile iron mechanical joint fittings. These glands shall meet the following requirements:

The mechanical joint restraint shall be designed to fit standard mechanical joint bells with standard T-head bolts conforming to ANSI/AWWA C-111/A21.11 and ANSI/AWWA C21.53 of latest revision.

Glands shall be manufactured of ductile iron conforming to ASTM A536 grade 65-45-12. Setscrews shall be of hardened ductile iron and require the same torque in all sizes. Steel setscrews are not permitted.

Glands shall be listed with Underwriters Laboratories and/or approved by Factory Mutual.

Mechanical joint restraint shall be incorporated in the design of the follower gland.  $3^{\circ} - 5^{\circ}$  deflection is required for 6" through 12" MJ joints. The restraint mechanism shall consist of a plurality of individually activated gripping surfaces to maximize restraint capability. Glands shall be manufactured of ductile-iron conforming to ASTM A536-80. The gland shall be such that it can replace the standardized mechanical joint gland and can be used with the standardized mechanical joint bell conforming to ANSI/AWWA A21.11/C111 and ANSI/AWWA A21.53/C153 of latest revision. Twist-off nuts, sized same as tee-head bolts, shall be used to insure proper actuating of restraining devices. The restraining glands shall have a pressure rating equal to that of the pipe on which it is used.

Only uncut solid pipe shall be used between fittings if the distance is less than 18 feet laying length.

Mechanical joint thrust restraint glands are required on all mechanical joint connections in addition to concrete thrust blocking described elsewhere herein.

These devices shall have the stated pressure rating with a minimum safety factor of 2:1 and shall be EBAA IRON, Inc. Megalug Series 1100 for ductile iron pipe or Series 2000 PV for PVC pipe or approved equal.

# Polyethylene Encasement of Ductile-Iron Piping and Appurtenances

The CONTRACTOR shall install polyethylene encasement around underground installations of ductile-iron pipe, ductile iron fittings, valves, and other ductile iron appurtenances.

Polyethylene film shall be manufactured of virgin polyethylene material conforming to ASTM Standard Specification D-1248-78.

Polyethylene encasement shall prevent contact between the pipe and the surrounding backfill and bedding material but is not intended to be a completely airtight and watertight enclosure. Overlaps shall be secured by using adhesive tape, plastic string, or any other material capable of holding the polyethylene encasement in place until backfilling operations are completed.

Use the same backfill material as that specified for pipe without polyethylene wrapping, exercising care to prevent damage to the polyethylene wrapping when placing backfill. Backfill material shall be free from cinders, refuse, boulders, rocks, stones, or other material that could damage polyethylene.

#### Corrosion Protection of Buried Threaded Components

After buried fittings have been installed, but prior to backfilling, the CONTRACTOR shall thoroughly coat nuts and bolts of all components with Rubber-In-A-Can.

Rubber-In-A-Can shall be manufactured by Share Corporation, P.O. Box 245013, Milwaukee, WI 53224, 800-776-7192, <u>www.sharecorpo.com</u>.

A thorough coating of Rubber-In-A-Can shall prevent contact between the nuts and bolt and the surrounding backfill and bedding material.

It is not intended to be cover the fitting, valve, or appurtenance body—the polyethylene wrap specified elsewhere herein is required for that purpose. The poly wrap is to protect the body of the component from corrosion, while the Rubber-In-A-Can is required for protection of the threaded components.

Use the same backfill material as that specified for pipe without this coating, exercising care to prevent damage to the Rubber-In-A-Can coating material when placing backfill. Backfill material shall be free from cinders, refuse, boulders, rocks, stones, or other material that could damage the coating.

#### Existing Water Service Transfers (New Main to Meter)

After the new mains covered by this contract have been installed, (including new corporation stops, service saddles, one-piece service lines, and locking curb stops for existing services), tested, sterilized and placed into service, this CONTRACTOR shall transfer existing water services where shown on the drawings to be transferred, from the new main to the existing meter.

The CONTRACTOR shall arrange the transfers at the customer's convenience. He shall uncover the existing service at the meter box, turn the existing meter stop <u>off</u>, and disconnect the existing service line from the meter. The existing meter and meter box will be reused unless otherwise shown on the drawings. All ends of existing service lines with curb stops at the meters shall be removed from the meter box, and buried outside meter box.

Meter and box shall be reused, but shall be relocated at a spot designated by the owner. All work on private property shall be conducted or supervised by a licensed plumber.

The bid item for this work includes the corporation stop, service saddle, curb stop, and service line from the new main to the existing meter, and any pavement repair required.

#### Floatation of Structures or Pipelines

This CONTRACTOR shall exercise the greatest of care to prevent the displacement of any structure or pipeline by external water pressure. All such displacement shall be corrected by this CONTRACTOR at his expense, and, if necessary, in the ENGINEER'S opinion, the displaced structure or pipe shall be removed and replaced at the CONTRACTOR'S expense.

Every concrete structure shall be provided with an adequate opening (properly water-stopped) to permit entrance of groundwater during construction, or an approved relief valve.

Upon completion of the structure's backfill, and before placing the unit in service, these antifloatation openings shall be properly plugged, if directed by the ENGINEER.

All pipelines shall be immediately "blinded-in", and at least partially backfilled, to prevent floatation.

# Existing Line Abandonment

After the new lines covered by this contract have been installed, tested, sterilized and placed into service, and required customer services have been transferred from existing parallel lines to the new mains, certain existing lines, where shown on the drawings, will be cut, capped, and taken out of service. Lines 2" and smaller shall be capped using PVC solvent welded caps and lines larger than 2" shall use mechanical joint caps. The abandonment of these lines is outlined on the construction drawings insofar as the piping arrangement of the existing system is known, but the existing piping arrangement is not guaranteed.

Where noted on drawings, pipe shall be abandoned by filling with flowable fill.

# Brass Goods

All brass goods shall meet AWWA current requirements and shall be that material commonly referred to as waterworks brass. They shall be screwed or flared connections and shall meet AWWA Standard C-800.

Furthermore, all brass goods shall comply with NSF-61, as well as NSF/ANSI 372 ("lead free" requirement of the Reduction of Lead in Drinking Water Act).

#### Galvanized Iron Pipe

Galvanized steel pipe shall be Schedule 40 with screwed connections. All fittings shall be malleable iron, hot-dipped-galvanized after fabrication. Threaded joints shall be doped.

# Solid Sleeves

Solid sleeves shall be used to join two plain-end pieces of pipe. The sleeves shall be ductile-iron pipe with mechanical joints connections on each end. Solids sleeves shall allow the pipe to be inserted beyond the depth of the socket that contains the gasket.

Solid sleeves shall be installed with restrained joint retainer glands, as defined elsewhere herein.

Solid sleeves shall be connected to single pieces of pipe between mechanical joint fittings to prevent separation of the pipe-to-pipe connection – unless that connection is also restrained.

Solid sleeves shall be protected from corrosion by a double wrap of polyethylene, as described elsewhere herein.

# BORES

#### Encasement Pipe

Where lines are shown on the drawings to be installed in bores and encased, the drawings set forth a bore and encasement pipe size for each carrier pipe.

Encasement pipe shall be steel, of the configuration and wall thickness shown on the drawings, or in the OWNER'S permits. Joints shall be securely welded, and the encasement shall be installed to the straightest alignment feasible.

The exterior of the pipe shall be protected from corrosion with a bituminous coating.

End seals constructed of minimum ¹/₄" Neoprene sheets and stainless steel circle clamps (applied to connect the Neoprene sheets to the pipe and to the casing) shall be used to prevent soil from washing into the annual space between the pipe and the casing. Install on both ends of casing. End seals shall not be installed if any water is in the casing.

# Bores

No State or Federal highway, or railroad, shall be open cut. All crossings of these facilities, and any others so shown on the drawings, shall only be by methods approved by TxDOT, without damage to them, or disturbing their use. Such methods shall be done in strict compliance with the terms and conditions of Highway and/or Railroad Permits issued to the OWNER.

The OWNER shall furnish copies of the permits to the CONTRACTOR and it shall be this CONTRACTOR'S responsibility to place each bore at the location specified by the OWNER'S permit. Any permit revisions caused by bore mis-location will be at the CONTRACTOR'S expense.

Bores shall be for the length of the crossings as shown by the OWNER'S permits.

Bores shall not be made without adequate advance notification (minimum 48 hours) to the respective permitted facility owner's representative.

Bore diameters shall not be more than 3" greater than the encasement pipe O.D. or, 3" greater than the carrier pipe joint O.D. where the permit does not require encasement.

All bores under state or federal roadways shall be "dry" bores with encasement as shown in the drawings. Driveway bores within state right-of-way may be "wet" bores without encasement.

The Texas Department of Transportation and/or respective railroad may impose additional requirements, which will be outlined in the OWNER'S permits and are considered a portion of this contract.

#### Carrier Pipe in Bores

The carrier pipe installed inside the encasement pipe shall be of the same material as the main line, unless noted otherwise on the drawings.

On bell-and-spigot carrier pipes, the carrier pipe bells shall not rest on the encasement pipe flow line. Casing spacers, as per the detail drawings, shall be provided to properly position the pipe in the casings, protect the bells, and prevent carrier pipe floatation. The lengths of main line pipe joints may be adjusted to cause the minimum number of carrier pipe joints to occur inside the encasement pipe.

#### Casing Spacers

Casing spacers shall be used to install the carrier pipe inside the encasement pipe. Casing spacers shall fasten tightly onto the carrier pipe so that when the carrier pipe is being installed the spacers will not move along the pipeline. Casing spacers shall be doubled on each end of the encasement.

Each casing spacer shall be capable of providing support for the carrier pipe in service at a maximum spacing of 10'. Calculations shall be provided to the ENGINEER by the casing spacer manufacturer showing that the casing spacer will support the service load at the recommended spacing, including a factor of safety of two (2). Casing spacers used under this specification shall meet or exceed the specifications described herein as projection-type casing spacers.

Projection-type casing spacers shall be constructed of preformed sections of high density polyethylene. The flexible sections shall be joined around the pipe to provide a minimum of 16

plastic projections per spacer section. Projection-type casing spacers shall be "RACI" Type F/G spacers, manufactured by Recon Pipe Corporation of Vernon, British Columbia, telephone (604) 545-2227, or equivalent.

# VALVES

# Gate Valves

Valve size and location shall be as shown on the drawings. Valves inside of structures or above grade shall have hand wheels. Buried valves shall be equipped with valve boxes as described elsewhere herein. Valves inside of structures or above grade shall be flanged; buried valves 2" and larger shall be mechanical joint. Buried valves smaller than 2" shall have threaded ends with brass pipe nipples in each end. All valves furnished under these specifications shall comply in all respects to AWWA specifications.

All buried valves shall be anchored in Class B concrete as per the detail drawings, so that no stress or strain, because of operating the valve, will be transferred to the pipe.

This CONTRACTOR shall furnish to the OWNER a valve operating wrench for each type of valve.

a. <u>2" and Larger</u>

2" and larger valves furnished under this specification shall comply in all respects to AWWA C-509 or C-515 for resilient seated gate valves. The valves shall be non-rising stem with 2" nut operator (unless handwheel is shown - handwheels to be ductile iron, if specified) and mechanical joint connections. Valve body, bonnet, stuffing box and operating nut shall be ductile iron. Wedge shall be constructed of ductile iron, fully encapsulated in synthetic rubber, except for guide and wedge nut areas. The valve shall be designed for a minimum 200 psi working pressure, 400 psi test pressure. The design shall allow 0-ring seals to be changed while the valve is in service. Valve body and bonnet shall be covered inside and out (ferrous parts only) with asphalt varnish or a fusion-bonded epoxy acceptable for use in potable water. Valve shall open to the left (counterclockwise).

#### b. 1-1/2" and Smaller

1-1/2" and smaller valves shall be ASTM B-62 brass, of the plug type, and manufactured from materials, which meet the various ASTM specifications applying to materials for use in waterworks service. The bronze body and bronze plug shall comply strictly with ASTM B-62 material specifications, commonly referred to as waterworks brass. The valves furnished under these specifications shall be manufactured and tested to AWWA C-800 standards. 1-1/2" and smaller valves shall be fastened to mains with all brass service saddles and brass threaded nipples, as per the detail drawings. Valve shall open to the left (counterclockwise). The handwheel shall be epoxy-coated ductile iron to prevent corrosion.

#### Valve Boxes

All buried valves shall have screw-type, three-piece, 5" minimum diameter, C.I. valve boxes. The three pieces shall include an upper female-threaded section with drop-in top; a male-threaded barrel bottom section; and a flared base section with minimum inside dimension at the base of 14 inches. Tyler 6860 Series with #6 base or approved equal. The CONTRACTOR shall determine the depth of bury of the water main and the assembled box extension required. Valve boxes shall be set to not transfer surface impact loads to the pipe or the valve. Boxes shall be plumb, with the operating nut set in the center of the box. Each top shall be set 1" above finished grade when the surface is earthen, or flush with the surface when in pavement.

Only in the case of buried butterfly valves with a buried gearbox will the 14" base not be required. Provide a standard 8" bottom non-slotted valve box base in this situation. The weight of the box shall be transferred to compacted backfill around the valve instead of onto the valve itself.

# Gray Iron Castings

Gray (cast or ductile) iron castings shall conform to ASTM A-48, and shall be free of blowholes, cracks, warping, and burnt-on sand, and shall be reasonably smooth. Covers shall fit properly into frames and shall seat uniformly. Angles shall be filleted, and risers shall be sharp and true.

Additional specific requirements for gray iron valve boxes, fittings, valves, manhole and cleanout covers, and similar appurtenances, may also be included under other sections herein.

# Tapping Sleeves and Valves

Tapping sleeves and valves shall be used where shown on the drawings, and the sleeve and valve are considered one unit for payment purposes.

Tapping valves shall meet AWWA specifications and requirements outlined for gate valves elsewhere herein, except that they shall be especially designed for use with tapping sleeve. They shall be complete with valve box and marker, and concrete blocking. Valves shall be flange by MJ, unless otherwise noted.

The tapping sleeve shall be of cast iron or ductile iron body, with corrosion-resistant steel alloy bolts. Ends shall be mechanical joint. The outlet shall be flanged, for connection to the valve. Small and large O.D. range gaskets shall be provided. The sleeve shall be designed for a minimum of 150 psi operating pressure and 250 psi test pressure. It shall be provided in two sections, which bolt together to surround the pipe circumference, with the outlet cast as an integral portion of the sleeve.

Tapping sleeves which consist of two half-circle fabricated sections of steel, gasket lined and longitudinally crimped to allow deformation by tightening the bolts and thus provide built-in range over a variety of pipe O.D.'s may also be used, providing that the main body, neck, and bolts are constructed of Type 304 <u>stainless</u> steel, and designed to withstand a minimum of 150 psi working pressure and 250 psi test pressure.

The tapped plug removed from the line shall be delivered to the ENGINEER.

All metallic components shall be poly-wrapped to reduce corrosion.

# **FLOW METERS**

# Water Meter Boxes

Existing meter boxes shall be reused unless otherwise shown on the drawings.

The meter box shall be set on firm ground so that the top of the meter box is 1/2" above the ground surface, and the box is level and plumb. The ground shall be carefully backfilled and tamped around the meter box so as to leave a neat appearance.

# **CONCRETE**

# Concrete

This specification includes structural concrete, for walls, floors, roof, beams, footings and all other portions of buildings, tanks and structures, and for pavements and any other flatwork. Where not specifically indicated herein, the latest versions of the requirements of ACI 301 – "Specifications for Structural Concrete for Buildings," ACI 302 - "Guide for Concrete Floor and Slab Construction," ACI 316R - "Recommendations for Construction of Concrete Pavements and Concrete Bases," or ACI 350R - "Environmental Engineering Concrete Structures," as appropriate, shall govern.

- Cement Cement for concrete shall be Type 1 Portland cement, which shall conform to (a) the requirements of the current specifications for Portland cement, ASTM designation C150. The CONTRACTOR shall furnish, in advance of construction, a certified analysis and a statement from the manufacturer that it meets the C150 specification.
- Water Water for use in mixing and curing concrete shall be reasonably clean and shall (b) be free from injurious amounts of oil, acid, alkali, salts, organic or vegetable matter, or other deleterious substances.

When comparative tests are made with distilled water, any indication of unsoundness, marked change in time of setting, or a reduction of more than 10 percent mortar strength shall be sufficient cause for rejection of the water under test. If the CONTRACTOR desires to use any water source for concrete mixing other than the normal, local, potable water supply, it shall be his sole responsibility to have all tests performed and to bear the cost of the testing. The testing laboratory and tests shall be approved by the ENGINEER.

Concrete Admixtures – All Class A (structural) concrete may contain a chloride-free liquid (c)admixture for reducing water requirements and retarding the set of the concrete. The admixture shall meet ASTM C-494. An admixture to entrain 4% to 6% air meeting ASTM

C-260, shall be required in exposed concrete or concrete for tank walls and floors. It shall be added in strict accordance with the manufacturer's recommendation. Other admixtures must be submitted to the ENGINEER in advance, for approval.

(d) a manner as to prevent intrusion of foreign materials and segregation of sizes.

If stored on the ground, the sites of the stockpiles shall be grubbed, cleaned of all vegetation and leveled off. The bottom six inch layer of aggregate stock pile shall not be disturbed and shall not be used in the work.

At the time of use, the aggregates shall be free from frozen material and all foreign material such as grass, wood, sticks, burlap, paper, or dirt, which may have become mixed with the aggregates in stockpiles or in handling.

Aggregate scales shall be tested and certified correct at least annually, and at such other times as the ENGINEER may direct.

(e) percentage by weight of ASTM C33, #57, as follows:

Nominal Size:	1 Inch	
Retained on	$1\frac{1}{2}$ inch sieve	0%
Retained on	1 inch sieve	0 to 5%
Retained on	¹ / ₂ inch sieve	40 to 75%
Retained on	#4 inch sieve	90 to 100%
Retained on	#8 sieve	95 to 100%

Where coarse aggregate is delivered to the job in two or more sizes or types, each size or type shall be separately and accurately measured by volume or weighed separately in such proportions as the ENGINEER may direct and shall not be mixed prior to batching.

The maximum size of aggregate shall be reduced in size to meet the following conditions:

- 1. the concrete is being placed, and
- 2. One-third of the thickness of slabs, and
- 3. Three-fourths of the clear space between reinforcement, and

Concrete Aggregates – General – The handling and storage of aggregates shall be in such

Coarse Aggregate for Concrete – The coarse aggregate shall be well graded in size from coarse to fine and when tested by laboratory methods shall meet the requirements for

• ASTM C33 gradation #57 is also equivalent to TxDOT Item 421 aggregate grade #4.

One-fifth of the least dimension between forms of that part of the structure in which

4. The maximum size aggregate is defined as the clear space between the sides (not diagonal) of the smallest square openings through which 95% of the weight of the aggregate can be passed.

Coarse aggregate shall consist of gravel, crushed gravel, crushed stone, or a combination of these.

Gravel, crushed gravel, and crushed stone shall consist of clean, hard durable particles, free from adherent coatings, thin, or elongated pieces, soft or disintegrated particles, dirt, organic or injurious matter.

When subjected to the sodium sulfate or magnesium sulfate soundness test (5 cycle), in accordance with TxDOT method 411-A, the loss will not exceed 12% (sodium) and 18% (magnesium sulfate).

Deleterious substances shall not exceed the following percentages by weight:

Material removed by decantation	1.0%
Shale, slate or other similar materials	1.0%
Clay lumps	0.25%
Soft Fragments	3.0%
Other deleterious substances, including	
friable, thin, elongated or laminated pieces	3.0%
The sum of all deleterious substances, exclusive	
of material removed by decantation, shall not	
exceed by weight	5.0%

The aggregate shall be free from a harmful excess of salt, alkali, vegetable matter, or other objectionable matter occurring either free or as adherent coatings.

(f) Fine Aggregate for Concrete – Fine aggregate shall consist of a sand or mixture of sands with or without a mineral filler. The sand or mixture of sands in fine aggregate shall consist of clean, hard, durable, uncoated grains, free from lumps.

When tested according to TxDOT test method TEX-612-J, the acid insoluble residue of the fine aggregate shall be not less than 28% by weight. When the fine aggregate is tested by method TEX-317-D, it shall have a mortar tensile strength equal to or greater than the Ottawa sand mortar.

The fine aggregate shall be free from a harmful excess of salt or alkali. It shall not contain more than 0.5% by weight of clay lumps.

When subjected to the color test for organic impurities (ASTM C-40) the sand or mixture of sand shall not show a color darker than the standard color.

The sand or mixture of sands shall be well-graded from coarse to fine, and when tested by laboratory methods (TEX-401-A), shall meet the following requirements for percentage by weight:

Retained on 3/8 inch sieve Retained on No. 4 sieve Retained on No. 8 sieve Retained on No. 16 sieve Retained on No. 30 sieve Retained on No. 50 sieve Retained on No. 100 sieve Retained on No. 200 sieve

If it is economical to use two sands of different gradations which when combined will meet these specifications, each sand shall be separately and accurately measured in such proportions as the ENGINEER may direct and shall not be mixed prior to batching.

The mineral filler, if required, shall be stone dust or crushed sand with a minimum of 95% passing the #30 sieve. It shall not exceed 5% by weight of the total quantity of fine aggregates.

- (g) Mixing and Placing Concrete The laboratory tests as follows:
  - 1) Coarse Aggregates
  - 2) Fine Aggregate
  - 3) Cement
  - 4) Concrete design mix

All equipment, tools, and machinery used for hauling, storing, and mixing and placing concrete shall be approved by the ENGINEER and shall be maintained in a condition to insure the completion of the work without excessive delays for repairs and replacements. All equipment necessary for the construction of the work shall be on the project and shall be acceptable to the ENGINEER as to condition before the CONTRACTOR will be permitted to begin construction.

All materials shall be separately and accurately measured by weight and volume.

Trucks shall be acceptable as to type and condition. They shall be provided with a closed watertight drum suitably mounted and shall be fitted with adequate blades capable of combining aggregates, cement and water into a thoroughly mixed and uniform mass of concrete, and to discharging the concrete without segregation.

Concrete shall be composed of cement, admixture, fine aggregate, mineral filler (if required), coarse aggregate and water, mixed in the proportions designated by the testing laboratory and in conformity with these specifications. The CONTRACTOR shall pay for

		0%
0%	to	5%
0%	to	20%
15%	to	50%
35%	to	75%
65%	to	90%
90%	to	100%
97%	to	100%

Mixing and Placing Concrete - The CONTRACTOR shall furnish the ENGINEER

all laboratory investigations and for the design and testing of the mix and aggregate materials.

The concrete mix shall be designed, and acceptance or rejection of the completed structure will be based upon the specified compressive strength in pounds per square inch at 28 days. Control will be accomplished by meeting a compressive strength in pounds per square inch at the age of seven days, equal to or greater than the following:

	Compression Testing: ASTM C-31						
	Compressive Lbs Per Squa 7 Days		Concrete Class	Minimum Sacks Cement	Maximum Water/Cement Ratio (Gal/Sk)	Cement	Air Entrained (if required)
	1300	2000	D	4	8	Portland Type 1	NA
Γ	2000	3000	В	5	6.5	Portland Type 1	4 - 6%
	2500	3700	А	6	6.0	Portland Type 1	4 - 6%

**Concrete Applications:** 

- All structural work (foundation, piers, beams, floors, roof slabs, walls, Type Abottoms, suspended walkways, manholes, boxes, inlets, water-holding floors, etc.): street pavement, curbs, and driveways.
- Type B-All sidewalks, ramps and steps.
- Type D-All pipe thrust-blocking, fence, bumper blocks, fills on structural concrete subgrade pipe encasement, etc.

The CONTRACTOR may substitute a stronger type for a weaker type. He shall not substitute in reverse order.

The concrete shall be uniform and workable. The quantity of water will be determined by the laboratory and will be such as to give the mixture the minimum amount of water consistent with the required workability. In general, the consistency of the concrete mixture shall be such that:

- The mortar clings to the aggregates. 1.
- 2. The concrete is not sufficiently fluid to segregate when transported to place of deposit.
- 3. The concrete, when dropped directly from the discharge chute of the mixer, will flatten out at the center of the pile, but the edges of the pile will stand up and not flow.
- 4. The mortar will show no free water when removed from the mixer.
- 5. The concrete will settle into place when deposited in the form. When transported in metal chutes at an angle of 30 degrees with the horizontal, it will slide and not flow into place.
- 6. The surface of the finished concrete will be free from "laitance" or a surface film

of water.

Any concrete failing to meet the above consistency requirements, although meeting the following slump requirements, will be unsatisfactory. The CONTRACTOR will change the mix to correct the unsatisfactory conditions.

Concrete shall contain no more water, within the slump limits specific for the various classes of the work, than is necessary to produce a concrete, which is workable and sufficiently plastic for the class of work intended. In general, the maximum slump in inches will be as follows:

#### Class of Work

Foundations and massive sections Floors and unformed walls Driveway, sidewalk, and pavements Formed heavy walls, slabs, beams & co Formed thin walls However, admixtures will be considered that allow the slump to increase - as long as the W/C ratio is not increased, or the strength is not decreased. All other requirements will still apply.

Tests for consistency of concrete shall conform to the current method of Slump Test for Consistency of Cement Concrete, ASTM Designation C-142.

During the progress of the work, the ENGINEER will cast test cylinders of the number and type he may desire for testing and maintaining a check on the compressive and strength of the concrete that is actually being placed.

The CONTRACTOR shall provide and maintain on site at his cost, facilities for the purpose of making and curing concrete test specimens. The cost of all materials used in test specimens shall be included in the contract unit price bid for the various items of the work and will not be paid for as a separate contract pay item. The cost of testing specimens shall be borne by the OWNER.

Concrete may be mixed in a central mixing plant or in a truck mixer, or it may be partially mixed at a central plant and the mixing completed in the truck mixer as herein set forth. Concrete so mixed shall be proportioned and batched as herein specified and shall meet all requirements specified for quality and strength.

The CONTRACTOR shall be responsible for producing a concrete that will have the required consistency and slump when delivered to the job. If the concrete as delivered to the job site is not of the required consistency or workability, the use of additional cement may be required. Concrete that is unsuitable, as delivered for placement, will be rejected.

Hand mixing of concrete will not be permitted except in case of emergency. Re-tempering

# Slump in Inches

	4
	4
	5
olumns	4
	5

or remixing of concrete will not be permitted.

All concrete not placed as herein specified within 60 minutes after adding water, will be rejected.

Except upon specific written authorization by the ENGINEER concrete shall not be placed when the overnight temperature is below 35 degrees F., or at any time when the temperature is less than 40 degrees F. and falling. Upon request of the CONTRACTOR and when such written permission is granted by the ENGINEER the CONTRACTOR shall furnish an approved enclosure, such as canvas covered framework, to enclose and protect all concrete so placed, and shall maintain the air temperature surrounding the concrete at not less than 50 degrees F. for not less than five days.

During hot summer weather, the temperature of the concrete mix at time of placement shall not exceed 90 degrees F.

Neither salt nor chemical admixtures shall be added to the concrete to prevent freezing. The CONTRACTOR shall be responsible for the quality and strength of concrete placed, and all concrete damaged in any way, including spalling and surface damage, by freezing or rain, shall be removed and replaced by the CONTRACTOR at his own cost and expense.

Concrete shall not be placed before the time of sunrise nor be placed later than will permit all finishing operations during sufficient natural light, unless directed by the ENGINEER.

Ready-mixed concrete shall conform to the current specifications for Ready-Mixed Concrete, ASTM Designation C-94.

The CONTRACTOR shall furnish all machinery, equipment, and devices of an approved type or design necessary for proper placing of the concrete, using methods approved by the ENGINEER, and conforming to the requirements of these specifications. All equipment shall be mortar-tight, and shall be kept clean, free from hardened concrete, and in proper working order at all times.

Concrete shall not be placed unless an on-site representative is present. The forms, reinforcement, etc., shall be reviewed and approved before ordering delivery of the concrete (allow at least one day advance notice for review).

Concrete shall be mixed, transported, and placed in conformity with these specifications. Concrete shall be handled to the place of final deposit in such a manner to prevent segregation, separation, loss of the ingredients, or the displacement of the reinforcement. The arrangement and use of chutes, when necessary, shall extend as near as possible to the final place of deposit of the concrete. Depositing a large quantity of concrete at one point and flowing or working it along the forms will not be permitted. A "tremie" or "elephant trunk" shall be utilized when dropping the concrete a distance in excess of five feet.

Placing concrete shall be so regulated that undue pressure will not be exerted against the

forms. Care shall be taken to fill each part of the forms by depositing concrete directly as near its final location as practical, to work the coarse aggregates back from the surface, and to mechanically work the concrete under and around the reinforcement bars, joints, waterstops and embedded structural shapes and equipment without displacing them.

Joints, water-stops, dowels, inserts, etc., shall be placed at the locations shown on the drawings and concrete shall be poured in one continuous operation between or around such.

Where work is unavoidably suspended between regularly planned construction joint locations, a joint and/or water-stop located and formed in a manner satisfactory to the ENGINEER shall be placed. Keys of the depth and width shown on the drawings shall be formed in the soft concrete and removing them after the concrete has hardened. Construction joints, water-stops, inserts, dowels, and reinforcing steel shall be free of dirt, scum, laitance or other inert materials before pouring the next section. Joints shall be wetted immediately prior to placing fresh concrete.

Concrete shall not be placed in or under water. Forms and foundations must be free of standing water, mud, and debris.

All construction joints located beneath finish grade or water level shall be waterproofed with PVC water-stops.

All concrete shall be protected against loss of moisture for a period of not less than 120 hours, by either forms, curing compound, wet mats, wet straw/sand, water immersion, or spray.

All horizontal slabs and each horizontal layer deposited in formed walls, beams, columns, and similar structures, shall be mechanically vibrated. The vibrator should be extended slightly into the underlying layer to bond the two layers together. The vibrator shall not be used to move the concrete horizontally. Vibration will not be required for curbs or sloped concrete surfaces unless specifically called for on the drawings.

All snap ties shall be broken off to the haunch embedded in the concrete and the resulting void shall be caulked with concrete grout and rubbed to achieve an appearance acceptable to the ENGINEER.

- (h) material, and structure or use where placed. Test samples will be so noted.
- (i)

Placement Record – The project on-site representative shall be furnished one ticketed copy of each batch of concrete delivered to the project. This ticket shall show the volume, type

Concrete Test Samples – The ENGINEER shall select at random batches of concrete to be tested and shall make test cylinders from any batch so selected. The CONTRACTOR shall furnish the test cylinders free of charge. One cylinder shall be broken at the end of seven days. If it is acceptable, a second cylinder shall be broken at the end of 28 days. If it is acceptable, the third cylinder shall be discarded. Should either sample not show acceptable strength, the third sample shall be broken at that time. If this sample shows acceptable strength, the unacceptable sample shall be deemed to have been in error. Any batch of concrete so sampled whose test fails to equal 90% of the specified strength, shall be cored on the site and tested at the CONTRACTOR'S expense.

If this coring bears out the test samples, then that portion of the structure shall be deemed to be in non-compliance with the intent of the contract documents and the CONTRACTOR, without any recourse to the OWNER or the ENGINEER, shall immediately remove and replace the concrete at his expense.

Should the concrete test results be less than specified, the ENGINEER, at his discretion, may request, and the CONTRACTOR shall provide, a revised mix design. No additional pours will be made until the revised design is submitted.

Forms – All straight channels, the inside of all concrete structures, and the exposed surface (i) plus twelve inches of the outside of the structures shall be formed of smooth new plywood or steel. All other portions of structures may be formed of used plywood or dressed lumber.

Form removal shall be carried out in such a manner to insure the complete safety of the structure. As a minimum, except for slabs on grade, forms shall remain in place for the various classes of work until the concrete has reached the minimum 7-day compressive strength specified herein for the class of concrete involved. The forms can be removed in 3 days if the CONTRACTOR can prove that the 7-day strength has already been achieved. (This testing will be paid for by the CONTRACTOR).

Forms for slabs on grade may be removed after 24 hours.

All exposed edges of concrete structures shall be chamfered  $\frac{3}{4}$ ".

All forms shall be adequately and rigidly braced to prevent misalignment. Any form failure resulting in misalignment of that structural section or "bulging" of its wall shall cause the section to be rejected and such section shall be removed and replaced if, in the judgment of the ENGINEER, such misalignment or bulging would affect the structure's use, function or appearance.

Forms shall be set such that the lines and surfaces of the columns, piers and walls, as well as slabs, ceilings, lintels, sills, parapets and other conspicuous lines shall not vary from plumb, level or the grade specified by more than 1/4" in any 20' of length. Sleeves, floor opening, wall openings, shall be located within  $+ \frac{1}{2}$ " of the locations shown on the drawings and cross-sectional dimensions of columns and beams and the thickness dimensions of slabs and walls shall be to the dimensions shown on the drawings within a tolerance of  $-\frac{1}{4}$ " to  $+\frac{1}{2}$ ".

All forming shall be carried out in such a manner as to insure the safety of the work during form construction and placement of the concrete.

Concrete Finishes – All concrete structures shall have the finish provided by the forms, (k)

except:

1.

- 2. equipment.
- 3. shall be wood float finish.
- 4.
- 5. above, shall be broom finished.

Where a cement base aggregate type coating is used as referred to above, it shall be heavy duty, designed for immersion service, and shall be "Thoro Systems" or equal, applied in strict accordance with the manufacturer's instructions. Manufacturer's literature shall be supplied the ENGINEER for product approval and the ENGINEER shall be the judge of acceptability of the finished appearance. Color shall be selected by the ENGINEER.

(1)include development tie bars.

> Steel reinforcement shall be deformed bars of open-hearth, new-billet-steel, yield strength 60,000 psi, conforming to the current specifications for Billet-Steel Bars for Concrete Reinforcement, ASTM Designation A-615. All reinforcing steel shall have been tested inside of the U.S.A. for compliance with these specifications.

> Wire for reinforcement shall be cold drawn from rods hot-rolled from open hearth billets, conforming to the current specifications for Cold Drawn Steel Wire for Concrete Reinforcement, ASTM Designation A-82. The welded wire fabric shall comply with ASTM A-185, Welded Steel Wire Fabric for Concrete Reinforcement. Fabric must be furnished flat-rolled fabric is not acceptable.

> Reinforcement bars shall be bent cold to the shapes indicated on the drawings. All bending shall be done in the shop. Bends shall be true to the shapes indicated.

Interior finished floor slabs of buildings and the interior walls of troughs carrying liquids shall be hard steel trowelled and sealed as described elsewhere herein.

Interior floor slabs of all boxes, manholes, and structures receiving concrete fill or equipment shall be wood float finished or as directed by the manufacturer of the

Structural base slabs, beams, and concrete fills below grade that are not exposed

Walls exposed above grade, of tanks, boxes, flumes, inlet/outlet structures and similar structures (not including sewer manholes) shall be hand-rubbed, or coated with a cement-base aggregate type, heavy duty coating, especially designed for said purpose, to product a smooth, pleasing surface which is consistent in color and texture and free of imperfections left by the forms or by honeycombs or similar voids. ENGINEER shall be the judge of acceptability of the finished appearance.

Sidewalks, pavements, and steps, and any other items exposed but not mentioned

Steel Reinforcement – Reinforcement is the metal embedded in concrete in such a manner that the reinforcement and concrete act together in resisting applied forces. This shall also

Reinforcement shall be stored above the ground surface upon skids, platforms or other supports, and shall be protected from mechanical injury and from surface deterioration by exposure to the weather. When placed in the work, the reinforcement shall be free from dirt, loose rust, scale, paint, oil or other foreign material.

Reinforcement bar laps shall be made as shown on the drawings and where not shown, they shall equal 40 times the bar diameter or the current CRSI standard, whichever is greater. Hooks and bends shall be made as shown on the drawings and where the dimension is not shown, the diameter of the hooks or bend shall equal eight times the bar diameter, with a straight length of bar at the end of hooks equal to four times the bar diameter, or the current CRSI standard, whichever is greater.

The reinforcement shall be supported on proper chairs and tied in place using approved methods so that it will not become displaced during the placing of concrete. The spacing of parallel bars shall not be less than three bar diameters center to center, with a minimum clearance between bars of 2". The distance from the center of bars to surface of concrete shall not be less than two bar diameters, with a minimum clear covering of three quarters inch, unless otherwise shown on the drawings. Bars in footings shall have a minimum clear cover above the soil of 3" unless otherwise shown on the drawings. The placing and securing of reinforcement in any section of the work shall be acceptable to the ENGINEER before any concrete is deposited in the section. In the event of any displacement of reinforcement from its proper setting, it shall be immediately replaced and secured to its original position.

Reinforcing steel shall be placed to the tolerances shown in the following table. Reinforcing steel shall not be heated for bending purposes.

Clear distance to formed surfaces $\pm \frac{1}{4}$ Minimum spacing between bars
Clear distance from unformed surface to top reinforcement:
1
Members 8" deep or less
Members more than 8" deep but less than 24" deep $1/4$ , $+ 1/2$
Members 24" deep or greater $\frac{1}{4}$ , + 1
Uniform spacing of bars, but the required number of
bars shall not be reduced <u>+</u> 2
Uniform spacing of stirrups and ties, but the required
Number of stirrups and ties shall not be reduced <u>+</u> 1
Longitudinal locations of bends and ends of reinforcement:
General <u>+</u> 2
Discontinuous ends of members $\pm \frac{1}{2}$
Length of bar laps $\pm 1 - \frac{1}{2}$
Embedded length:
For bar sizes No. 3 through 111
For bar sizes No. 14 and 182

Substitutions of sections, or modifications of details, or both, shall be made only when

approved by the ENGINEER.

Reinforcement may be rejected for failure to meet any of the requirements of the specifications, and specifically for the following:

- Reinforcement exceeding the allowable variations. 1.
- 2. Reinforcement not bent in accordance with the details.
- 3.

ASTM A-615 materials.

Water-Stops, Joint Fillers, Sealant (m)

> Waterstops shall be designed to stop water infiltration through cast-in-place concrete construction joints. It shall expand upon contact with water to form a positive seal against concrete. It shall consist of sodium bentonite to seal and fill voids and cracks in the concrete. The product shall be supplied in lightweight flexible coils and attached to existing concrete surfaces with an adhesive provided by the manufacturer, prior to the introduction of additional concrete. The product shall meet NSF-61 requirements for contact with potable water. Provide Volclay RX and Volclay WB-Adhesive.

> Joint filler (expansion joint material) shall be a premolded, semi-rigid grade asphalt board (ASTM D-1751 or AASHTO M-213). It shall be 1" thick. The filler shall be held down ³/₄" from the surface of the concrete and extend full depth of the joint. It shall be held firmly in place while placing concrete. Any protruding material shall be neatly trimmed.

> Sealants for pavement sidewalks, driveways, building floors, and similar will be a non-sag, self-leveling, or gun-grade synthetic joint sealant compound specifically recommended for adhesion and flexibility. It shall be capable of 100% elongation and installed in strict accordance with the manufacturer's directions. Use TREMCO Vulkam 45SSL, or equivalent.

material or painted.

For immersion service, concrete joint sealer shall be a two-component gun-grade synthetic rubber sealing compound, specifically recommended for continuous immersion services. It shall be Sonneborn "Sonolastic Two-Part," or equal, applied in strict accordance with the manufacturer's directions.

- (n) requirements.
  - 1.

Reinforcement with bends and kinks not shown on the details.

Dowels and tie bars shall be either straight or bent, smooth or deformed, as required, of

Above finished floor for non-immersed locations, and all caulk and sealant shall be clear silicone compound or shall be butyl rubber compound in a color to match the surrounding

Curing Materials – Materials for the curing of concrete shall conform to the following

Membrane-Forming Compounds – The membrane curing compound shall conform

to the requirements of Liquid Membrane-Forming Compounds for Curing Concrete, ASTM Designation C-309 for Type 2, white pigmented compound, unless otherwise specified or indicated. It shall be of such nature that it shall not produce permanent discoloration of concrete surfaces nor react deleteriously with the concrete.

The compound shall produce a firm, continuous uniform moisture-impermeable film free from pinholes and shall adhere satisfactorily to the surfaces of damp concrete. It shall, when applied to the damp concrete surface at the specified rate of coverage, dry to touch in one hour and dry through in not more than four hours under normal conditions suitable for concrete operations. It shall adhere in a tenacious film without running off or appreciably sagging. It shall not disintegrate, check, peel or crack during the required curing period.

The compound shall not peel or pick up under traffic and shall disappear from the surface of the concrete by gradual disintegration.

The compound shall be delivered to the job site in the manufacturer's original containers only, which shall be clearly labeled with the manufacturer's name, the trade name of the material and a batch number or symbol with which test samples may be correlated.

When tested in accordance with Water Retention by Concrete Curing Materials, ASTM Designation C-156, the liquid membrane-forming compound shall restrict the loss of water present in the test specimen at the time of application of the curing compound to not more than 0.55 grams per square centimeter (0.01 oz per square inch) of surface.

- 2. Cotton Mats Cotton mats are permissible providing the surface is covered completely and the mats are kept continuously damp.
- 3. Waterproof Paper Waterproof paper is permissible consisting of two sheet of plain kraft paper cemented together with a bituminous material in which are imbedded cords or strands of fiber running both directions of the paper. The paper shall be light in color; shall be free of visible defects; and shall have a uniform appearance. It shall be sufficiently strong and tough to permit its use under the conditions existing on streets and structural work without tearing or otherwise becoming unfit for the use for which it is intended. The paper shall conform to specifications for Sheet Materials for Curing Concrete, ASTM Designation C-171.

#### **Expansion Joints**

Expansion joints shall be constructed at the locations shown on the drawings.

The expansion joint shall consist of smooth dowels, expansion material, support chairs, cap strip, and sealant.

Smooth dowels shall be grade 60, of the diameter and length shown on the drawings. Steel shall be free of rust and deleterious material – provide expansion caps on one end of each bar.

The expansion material shall be flexible foam sheet designed for horizontal expansion joint applications. It shall consist of isomeric polymer in a small, closed-cell structure which will compress easily and recover to at least 99%. The material shall be compatible with generally accepted cold-applied sealants. Thickness will be ½". The material may be provided pre-cut to a 4" width or furnished in sheets and field cut to a 4" width. Use W.R. Meadows "Ceramar" or equal (Lofland Company, 1-800-288-5250).

The expansion joint support basket shall be welded wire basket designed for  $\frac{1}{2}$ " expansion material.

The cap strip will be ½" deep removable plastic strip designed to fit over the expansion material during the concrete pour, and later removed to provide a clean, sharp void for sealing. (Lofland Company or equal).

The expansion joint sealant shall be a non-sag, self-leveling, or gun-grade synthetic joint sealant compound specifically recommended for adhesion and flexibility. It shall be capable of 100% elongation and installed in strict accordance with the manufacturer's directions. Use TREMCO Vulkam 45SSL, or equivalent.

# MISCELLANEOUS

#### Clearing and Grubbing

CONTRACTOR shall perform clearing and grubbing along rights-of-way and on project sites, as required to construct the project. In so doing, observe the following:

No trees over 6" in diameter shall be removed along rights-of-way or easements unless specifically designated for removal on the drawings, without prior approval of the OWNER. Pipeline alignment may, upon approval of the ENGINEER, vary within the easement from a straight line to avoid tree removal.

On sites, all trees under 6" in diameter, and all trees of any size within 15' of any proposed pipeline, structure, or cut/fill sections over 6 inches in depth, shall be removed, unless specifically noted otherwise on the drawings.

On sites, all tree and brush roots, stumps, and wood shall be removed to a point three feet below the natural ground, and any on-site material, reused as project fill, shall have all roots removed therefrom.

All cleared and grubbed material shall be disposed of off the sites, rights-of-way, and easements in accordance with the established guidelines of the Texas Commission on Environmental Quality, at a site to be secured by the CONTRACTOR, at no additional cost to the OWNER.

All trimmed tree limbs and branches 1" and larger in diameter shall be cut square and flush with tree trunk or limb.

# Tracer Wire

Tracer tape shall be 14-gauge, solid core, with 30 mil HDPE insulation. Wire shall be terminated using 1" pipe and cap.

Wire shall be used with all pipe with the wire ending in an accessible location.

# Pipeline, Valve, and Appurtenance Markers

All pipeline, valve and appurtenance markers shall conform to the detail drawings. They shall be on 16-gauge steel, fastened with two tamper-proof screws to a 1" x 1/8" galvanized angle. The marker coating shall be baked enamel, 4 mil dry film thickness, orange background with black markings. A valve marker shall be provided for each valve installed on this project.

A pipeline marker shall be installed on every fence where the pipeline crosses beneath same. The marker shall be 3" x 12" with a baked enamel finish on 16-gauge steel sheet, as per the drawings. The marker shall be fastened to the fence wire with galvanized wire, top and bottom, or nailed to the fence post.

An appurtenance marker shall be installed for each air release valve, flush valve, and control valve on this project.

In lieu of the above valve and/or pipeline markers, this CONTRACTOR may furnish and install flexible valve and pipeline marker posts, 2-1/2" x 60" long, with blue and white vinyl, factoryapplied decals, labeled "CAUTION - Water Valve" or "CAUTION - Water Pipeline."

Posts shall be fabricated from glass fiber, marble and thermo-setting polymers for flexibility and resistance to deterioration. Marker posts shall be equivalent to Model No. CUM-375, (with appropriate decal), as manufactured by Carsonite Composites, LLC (800) 648-7916.

Markers shall not be a separate pay item.

# **Fencing Repairs**

Existing fences intersected by pipelines constructed under this contract shall be crossed by digging under from each side, and not disturbing the fence, where possible. When this procedure is not feasible, and the CONTRACTOR deems it necessary to cut or remove a fence across or adjacent to the pipeline, he shall notify the fence owner, and shall be responsible for preventing any pets or livestock contained therein from escaping.

For barbed wire fencing the CONTRACTOR will remove the fence at a corner or brace post and shall protect the existing posts and wire. Any post or wire lost or damaged as a result of this project shall be replaced with similar material, new, and of an equal or superior quality, at the CONTRACTOR'S expense.

Upon replacing the fence, it shall be stretched tight and shall be true to line, with wires parallel or fabric smooth, and with all usual appurtenances included. Posts shall be plumb and firmly embedded.

For chain link fencing, repairs shall be per technical specifications and construction standard.

Removing and replacing the fence, and all associated materials and work, are the responsibility of this CONTRACTOR and shall not be separate pay item.

# **Pavement Repairs**

a. conditions.

> The trench shall be backfilled in accordance with Installation and Backfill of Buried Pipe, as described elsewhere herein.

> The finished pavement surface shall conform to the shape and sections shown on the drawings and to adjacent undisturbed surfaces.

> All workmanship and materials shall be in strict accordance with the latest version of the standard specifications for "Construction of Highways, Streets, and Bridges" as published by the Texas Department of Transportation.

> The pavement wearing surface shall not vary more than 1/4 inch when measured with a 10-foot straightedge laid on the surface in a direction parallel to the crown.

> All adjacent structures and painted surface shall be protected from asphalt spray and drift by covering with paper or in a manner acceptable to the ENGINEER. No liquid asphalt will be allowed to mar the appearance of finished concrete structures.

> This CONTRACTOR shall be responsible, during the contract maintenance period, to correct any holes, blemishes, depressions, and surface irregularities in the pavement surfaces caused by his faulty workmanship, materials, or heavy equipment, or earth settlement.

> Where pavement repairs are itemized in the bid schedule, the payment limits are determined by the dimensions shown in the detail drawings. Where pavement repairs are not itemized in the bid schedule, they are considered incidental to other work and are not a separate pay item.

All city streets and private driveways, open cut by this CONTRACTOR, shall be repaired by this CONTRACTOR. Prior to repair, this CONTRACTOR shall maintain the access over such open cuts in a passable condition to vehicular traffic, regardless of weather

- b. Gravel base, crushed aggregate base, or iron ore Gravel, crushed aggregate, or iron ore surfaces shall be repaired to the existing thickness, but not less than 6". They shall be compacted to minimum 95% standard Proctor ASTM D-698 at -1 to +3% optimum moisture. The CONTRACTOR shall match the existing material type. Crushed aggregate base shall be TxDOT Type "A", Grade 1, or Grade 2 crushed or broken aggregate.
- c. Asphalt Pavement Repairs Asphalt pavement shall be repaired with like base material of a thickness equal to that existing, but in no case less than 6" compacted thickness. The crushed rock base shall meet all applicable requirements of Texas Department Transportation, Item 247 Type A, Grade 1 or 2. The asphalt wearing surface shall be Item 340, Type D, hot-mixed, hot-laid and shall have a compacted minimum thickness of 2". The flexible base shall be tack coated prior to the asphalt overlay with 0.20 gallon per square yard of MC-30. The asphaltic wearing surface shall be rolled with a flat steel wheel roller.
- d. Concrete pavement and driveway repairs Concrete pavement walks, and drives shall be repaired immediately. The repair width shall be the width of the trench plus 12" on either side thereof. The line along which the pavement is removed shall be sawed a minimum of 1-1/2" deep before pavement is broken out. The thickness shall be not less than that existing, but in no case less than 5" for streets, 4" for driveway approaches and sidewalks. Any existing reinforcement steel shall be cut and bent back for concrete removal and then rebent and tied into place prior to concrete placement. All cut reinforcement shall be spliced with equal size bars of a length equal to 30 diameters, or wire mesh.

Existing concrete work that is non-reinforced shall be replaced as non-reinforced. Concrete for street and driveway pavement and curb and gutter shall be Class A. For sidewalks it shall be Class B.

e. Oil pavement repairs - Oil pavement shall have a repaired thickness equal to that existing, but in no case less than 3" compacted thickness.

Oil base pavement shall be composed of native sand and road oil. The sand and road oil will be mixed in proportion to one cubic yard (1 CY) of sand material per eleven (11) gallons of road oil. Materials shall be mixed by mechanical means until a homogenous mix is obtained. The installed mixture shall be thoroughly compacted.

f. Unimproved street surface repairs - Unimproved streets shall be repaired by standard backfill operations as specified herein, with mechanical compaction to minimum 95% standard proctor compaction at -1 to +3% of optimum moisture. However, care shall be taken to insure the finished backfill is of similar type material to that of the existing unimproved road, i.e. sandy topsoil should have a similar backfill surface and not clay, shale, etc.

#### Fine Grading

Fine grade all exposed earthen areas affected by construction, and elsewhere or as shown on the drawings. The finished surface shall be uniform and shall slope away from all structures. No slope shall be steeper than 4H:1V, unless specifically shown on the drawings.

Grade all sites, rights-of-way, and easements to prevent ponding of surface water, and to drain in a like direction as prior to construction, unless otherwise shown.

Areas to be seeded, sodded, or sprigged shall be fine graded and compacted, then disked a depth of two inches immediately prior to the placing of the grass. The top 2" of soil shall be available on-site material, unless specified otherwise.

Areas designated to re-grass themselves naturally shall be fine-graded, and disked.

Settlement or erosion, which affects the fine grading, shall be repaired as required until completion of the project and the warranty period.

#### Project Guarantee

Neither the final payment, nor any provision in the Contract, nor partial or entire use of the improvements embraced in this Contract by the OWNER or the public, shall constitute an acceptance of work not done in accordance with the Contract or relieve the CONTRACTOR of liability for faulty materials or workmanship.

This CONTRACTOR shall guarantee for a period of twelve calendar months from the date contained in the substantial completion certificate that all equipment and products are free from defects in their design, material, manufacture, and workmanship. Separate or individual warranties and guarantees by the suppliers of various equipment and products in lieu of the single source responsibility will not be acceptable. In the event any item is found defective during this twelve-month period, this CONTRACTOR shall provide all labor, replacement parts, supervision, and materials necessary to correct the defects at the project site without cost to the OWNER.

Any component so repaired shall be guaranteed by the CONTRACTOR to be defect-free for the ensuing twelve-month period, even if that period goes beyond the end of the original twelve-month period.

Manufacturer or supplier standard guarantees, which exceed the above twelve-month period shall be furnished to the OWNER; however, this in no way relieves this CONTRACTOR of his warranty period.

Each manufacturer and product supplier by furnishing his merchandise on this project, unconditionally warrants that, when said article or product is used as set forth herein, and operated and maintained within the said supplier's recommendations, the article will perform the services herein required of it.

The CONTRACTOR and his suppliers shall not be responsible for the project design concept.

# Silt Fence

The silt fence used in this project shall be 36" wide and shall be non-biodegradable and resistant so most soil chemicals, acids, and alkali with a pH range of 3 to 12. The silt fence shall adhere, at a minimum, to the values listed below in the table.

Property	Test Method	Minimum Avg. Roll Value
Grab Tensile	ASTM D4632	80 lb
Elongation	ASTM D4632	15%
Mullen Burst	<b>ASTM D3786</b>	180 psi
Puncture	ASTM D4833	35 lb
Trap Tear	ASTM D4533	35 lb
AOS	ASTM D4751	30 Sieve
Permittivity	ASTM D4491	<b>0.05</b> sec ⁻¹

The CONTRACTOR shall install, maintain, and remove the silt fence as part of this project.

# Re-grassing

CONTRACTOR shall furnish all labor, supervision, equipment, bedding materials, grass seed, sprigs, fertilizer and other items that may be necessary as follows:

a. <u>Bermuda Grass</u> - CONTRACTOR shall reseed all areas disturbed by his construction operations.

All seed must meet the requirements of the Texas Seed Law including the labeling requirements for showing pure live seed (PLS) name and type of seed. Seed furnished shall be of the previous season's crop and the date of analysis shown on each bag shall be within nine months of the time of delivery to the project. Each variety of seed shall be furnished and delivered in separate bags or containers. A sample of each variety of seed shall be furnished for analysis and testing when directed by the ENGINEER. The amount of seed planted per acre shall be of the type specified below.

Water shall be clean and free of industrial wastes and other substances harmful to the growth of grass or the area irrigated.

Topsoil shall be native soils salvaged from the top 6" of disturbed area and stockpiled.

Fertilizer shall have an analysis of 15-15-15, or as indicated on the plans.

Straw mulch shall be oat, wheat or rice-straw, Prairie Grass, Bermuda Grass, other straw or hay approved by the ENGINEER.

Cellulose fiber mulch shall be natural cellulose fiber mulch produced from grinding clean whole wood chips. The mulch shall be designed for use in conventional mechanical planting, hydraulic planting of seed or hydraulic mulching of grass seed, either alone or with fertilizers and other additives. The mulch shall be such, that when applied, the material shall form a strong, moisture-retaining mat without the need of an asphalt binder.

Wood Fiber mulch shall be produced from ground newsprint with a labeled ash content not to exceed 7 percent. The mulch shall be designed for use in conventional mechanical planting, hydraulic planting of seed or hydraulic mulching of grass seed, either alone or with fertilizers and other additives. The mulch shall be such, that when applied, the material shall form a strong, moisture-retaining mat without the need for an asphalt binder.

After the designated areas have been rough graded to the lines, grades and typical sections indicated or as provided for in other items of this contract and any other soil area disturbed by the construction, likewise prepared, suitable seed bed shall be prepared. The seed bed shall consist of either 4" of approved topsoil or 4" of approved salvaged topsoil cultivated and rolled sufficiently to a state of good tilth. The optimum depth for seeding shall be 1/4". Water shall be applied as required to prepare the seed bed. Seeding shall be performed in accordance with the requirements hereinafter described.

Broadcast and straw mulch seeded areas shall immediately be watered with a minimum of 5 gallons of water per square yard or as needed and in the manner and quantity as directed by the ENGINEER.

Hydraulic seeded areas shall be watered commencing after the tackifier has dried with a minimum of 5 gallons of water per square yard or as needed to keep the seed bed in a wet condition favorable for the growth of grass.

Watering applications shall be constantly maintained the seed bed in a wet condition favorable for the growth of grass. Watering can be postponed immediately after a rainfall of 1/2" or greater but shall be resumed before the soil dries out. The seed bed shall be prepared as specified above and hydraulic planting equipment, which can place all material in a single operation, shall be used.

From March 1, to September 15, the hydraulic planting mixture and minimum rate of application per 1000 square feet shall be:

Hulled Bermuda Seed (PLS-0.83) Water Soluble Fertilizer

Fiber Mulch Cellulose Wood 1 lb.

13 lbs.

45.9 lbs. 57.4 lbs.

1.4 lbs.
1.5 lbs.

From September 15 to November 1, add 3 pounds per 1000 square feet of winter rye with a PLS = 0.83 to above mixture, and 1/2 lb. unhulled Bermuda. Preferably the seeding should be accomplished in early spring - no seeding will be allowed from November 1, to February 28, without specific approval of the ENGINEER.

Straw mulch shall be spread uniformly over the area indicated or as designated by the ENGINEER at the rate of 2 to 2-1/2 tons of straw per acre. The actual rate of application will be designated by the ENGINEER. Straw may be hand or machine placed and shall be adequately secured.

Cellulose and wood fiber mulch shall be spread uniformly over the area indicated or as designated by the ENGINEER at the rate of 45 to 80 lbs. per 1000 square feet.

It shall be the CONTRACTOR's responsibility to maintain the initial shape and contour of the entire seeded/sprigged area, irrespective of the project's 12-month maintenance period, until such time as a 75% average grass coverage has been obtained.

# Cellular Grout

#### I – SCOPE OF WORK

This work consists of providing a non-pervious, Low Density Cellular Concrete (LDCC) to fill abandoned pipes or other structures at the location shown in the plans in accordance with the details in the plans and these specifications.

# II – MATERIALS

- A. Portland cement shall comply with ASTM C150 (Type I, II or III).
- B. Fly ash shall be Class C or Class F and compatible with foaming agent.
- C. Water shall be free from deleterious substances.
- D. Foaming agent shall conform to ASTM C796.

E. Admixtures for water reducing, retarding, accelerating, and other specific properties may be used when recommended by the manufacturer of the foaming agent.

F. LDCC shall have the following properties:

Range of Cast Density, PCF 24-30 Minimum Compressive Strength (28 Days), PSI 40 Flow Consistency per ASTM D6107 Greater than 7"

# III – SUBMITTALS

A. Mix design for LDCC, including materials to be used and their sources.

B. Resume of CONTRACTOR showing experience as specified below, including qualifications of CONTRACTOR's superintendent and/or foreman.

C. Description of equipment and placement methods to verify compliance with specifications.

#### **IV – PRODUCTION**

produce consistent foam with stable, uniform cellular structure. producing equipment, which is capable of meeting the specified properties. material shall be conveyed promptly in its final location. D. All equipment used must be approved by the foam manufacturer.

#### V-INSTALLATION

A. CONTRACTOR shall fill the existing abandonment as indicated in the plans or as directed by project engineer.

completely filled.

PSI or the allowable maximum pressure, whichever is greater and shall be water-tight. 3" male threaded NPT.

the next at pressures below maximum allowable pressure. procedure. If required, dewatering shall be continuous during installation. multiple lifts depending upon length and diameter of pipe being abandoned.

# VI – QUALITY CONTROL AND QUALITY ASSURANCE

that is satisfactory to the Engineer. Including the following:

LDCC.

abandoned pipes or other structures. of LDCC under similar conditions.

#### **B. TESTING**

1. Testing to be performed by the OWNER or approved agency.

- A. Foam generating equipment shall be used to produce a predetermined quantity of pre-formed foam which shall be mixed and blended with cementitious slurry. Equipment shall be calibrated to
- B. LDCC shall be produced utilizing specialized automated proportioning, mixing, and foam
- C. Avoid excessive handling of the material. After sufficient mixing of the foam with slurry, the
- B. LDCC shall be pumped through injection ports located at bulkhead to allow abandonment to be
- C. All bulkheads and injection ports installed shall be capable of withstanding a minimum of 30
- D. Injection and vent ports and pipes must be securely installed and be able to receive a minimum
- E. Space injection points at intervals that allow material to be forced from one injection point to
- F. All water and other residual materials must be removed from pipe prior to initiating filling
- G. LDCC shall be placed in a manner so as not to cause collapsing of material, which may require
- A. CONTRACTOR shall have a record of experience and quality of work placing foam concrete
  - 1. Shall be capable of developing a mix design, batching, mixing, handling, and placing
  - 2. Shall be regularly engaged in the production and pumping of LDCC for filling
  - 3. Workers included the CONTRACTOR's superintendent and /or foreman, shall be fully qualified to perform the work and have had previous experience in production and pumping
  - 2. A minimum of four (4) 3"x 6" cylinders shall be molded for each shift of operation.

3. LDCC to be tested at any age after three (3) days for compressive strength. At least two (2) specimens from each set should be tested at 28 days in accordance with ASTM C-495 unless otherwise approved by engineer.

4. CONTRACTOR shall record and measure wet cast densities at the point of placement regularly. Mix shall be adjusted as required to obtain the specified cast density at the point of placement.

5. CONTRACTOR shall record and measure flow consistency regularly in accordance with ASTM D6107.

#### Coordination of Testing and Sterilization with Installation

The practice of "installing only", and delaying testing, sterilization, operation, and appurtenances until after completion will <u>not</u> be permitted and will cause items which are installed only to be disallowed on monthly partial payment estimates.

#### Preconstruction Video Documentation

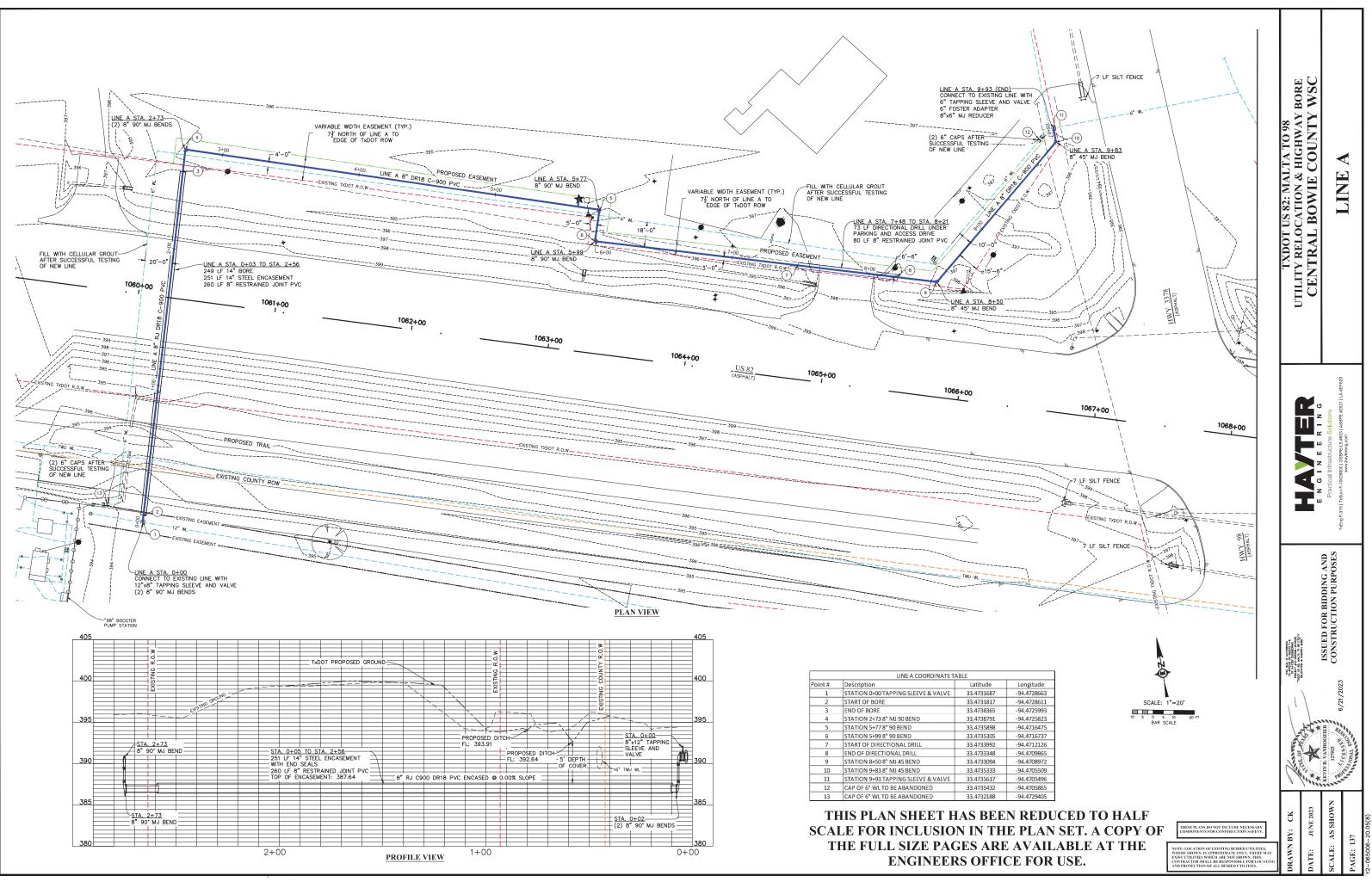
A video recording of the site shall be done by the CONTRACTOR <u>before</u> beginning any construction. This requirement will be done to document pre-existing conditions prior to construction. One "thumb-drive" copy of the recording will be given to the ENGINEER prior to the beginning of construction.

# **Buy American Accommodations**

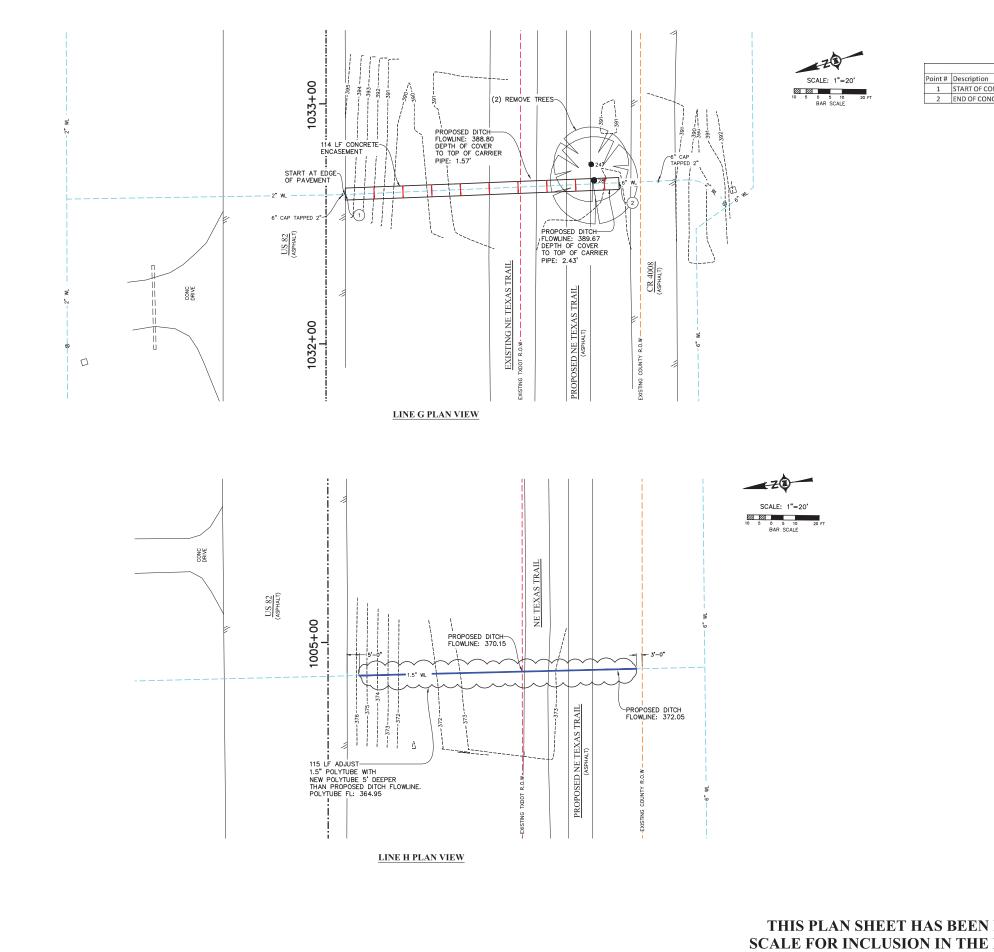
Iron and steel products used on this project shall comply with TxDOT's requirements for BUY AMERICA. Information is provided on the next page that outlines the BUY AMERICA requirements.

	SUMMARY OF UTILITY RELOCATION																				
		7332-6002	7332-6008	7332-6009	7332-6014	7332-6015	7332-6018	7332-6019	7332-6020	7332-6021	7332-6024	7332-6025	7332-6029	7332-6033	7332-6036	7332-6037	7332-6042	7332-6043	7332-6044	7332-6046	7332-6048
SHEET NO.	LINE	1.5" C901 DR 9 Polyethelene Tubing	8" C900 DR 18 PVC	8" RJ C900 DR 18 PVC	Concrete encasement of pipe	6" Tapping sleeve and valve	Testing/flushing location	Ductile iron fittings	Customer service transfer and relocation of meter	Connect to lines 3" and smaller	Testing/flushing location	Reseeding disturbed areas	3" RJ SDR 21 CL 200 PVC	8" Directional drill	6" Steel encasement	6" Dry Bore	14" Steel encasement	14" Dry Bore	Sand embedment	Abandon existing 6" waterline with cellular grout.	Abandon existing 2" waterline with cellular grout.
		LF	LF	LF	LF	EA	EA	TON	EA	EA	EA	LF	LF	LF	LF	LF	LF	LF	LF	LF	LF
ROW CSJ#0	919-19-094																				
137	LINE A	-	653	340	)-	1	-	0.47	1	-	1	-	-	73	-	-	251	249	-	975	-
138	LINE G	-	-	-	114		-	-	-	-	-	114	-	-	-	-	-	-	114	-	-
138	LINE H	115		-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
139	LINE I	-	-	-	-	- :	1	0.31	-	1	-	-	236	-	220	220	-	-	-	150	75
140	LINE L	-	-	-	120		-	-	-	-	-	120	-	-	-	-	-	-	120	) -	-
PROJECT	TOTALS	115	653	340	234	1	1	0.78	1	1	1	234	236	73	220	220	251	249	234	1125	75

6046 existing 6" he with grout. F 975 150 1125	7332-6048 Abandon existing 2" waterline with cellular grout LF - - - 75 - 75	TXDOT US 82: MALTA TO 98 ITELLITY DELOCATION & HICHWAY BODE	CENTRAL BOWIE COUNTY WSC		SUMMARY OF UTILITY RELUCATION	
					ו אבווק דיס וסן ואסטונידי הטעבטטטן לאסטר בובובא אינטטן איסטר ב וויבאב ון באיווברי עבא אינאי אוקאפופוקן גמווי	
			NULL OF THE SECOND	VIN R. VAN 1279	SSTONAL ENG	
	SEP PLAN DO NOT INCLEDE N MPONENTS FOR CONSTRUCTIO OCATION OF EXISTING BURBE SUMW, IS A REGUMATE DN UNTOR SHALL DE RESPONSIE DTECTION OF ALL BURED ET	DRAWN BY: CK	DATE: JUNE 2023	SCALE: AS SHOWN	PAGE: 136	Y2-065006-20.05(8)



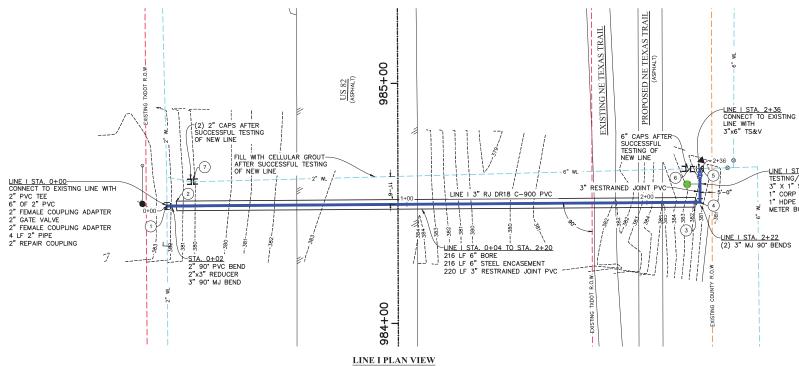
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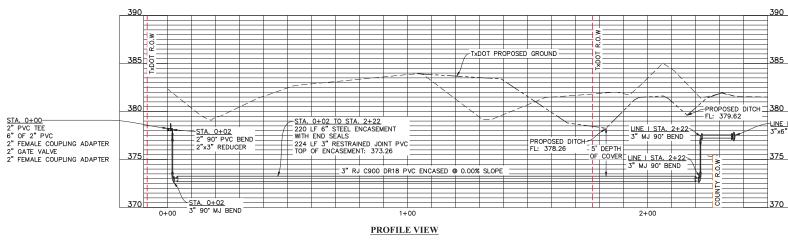
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	TXDOT US 82: MALTA TO 98 UTILITY RELOCATION & HIGHWAY BORE				
			REVIN R. VANHOOZIRR ISSUED FOR BIDDING AND CONSTRUCTION PURPOSES 6/21/2003	ACOMIL DES	
REDUCED TO HALF PLAN SET. A COPY OF AVAILABLE AT THE E FOR USE.	DRAWN BY: CK	DATE: JUNE 2023	SCALE: AS SHOWN	PAGE: 138	Y2-065006-20.05(8)

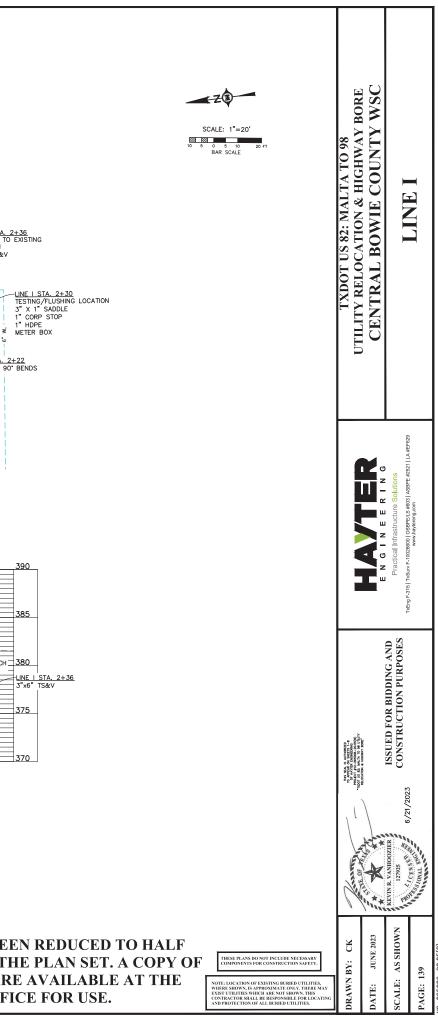


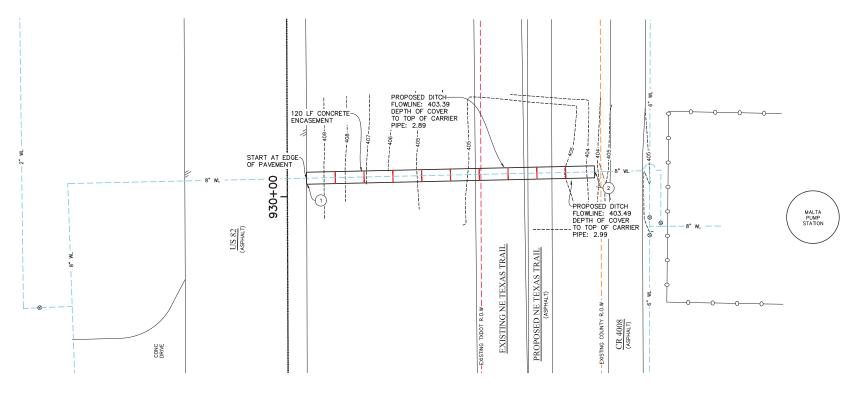
	LINE I COORDINATE TABLE							
Point #	Description	Latitude	Longitude					
1	STATION 0+00 2" TEE	33.4807270	-94.4960303					
2	START OF BORE	33.4807166	-94.4960345					
3	END OF BORE	33.4801551	-94.4962656					
4	STATION 2+22 3" 90 BEND	33.4801499	-94.4962678					
5	STATION 2+36 6" TS&V	33.4801371	-94.4962242					
6	CAP OF 6" WL TO BE ABANDONED	33.4801475	-94.4962205					
7	CAP OF 2" WL TO BE ABANDONED	33.4805792	-94.4960160					

NOTE: TO INSTALL STATION 0+00 TEE.
CLOSE EXISTING VALVE ON SOUTHERN
SIDE OF HIGHWAY, THIS DISRUPTION
OF SERVICE SHALL BE AS MINIMAL
AS POSSIBLE



THIS PLAN SHEET HAS BEEN REDUCED TO HALF SCALE FOR INCLUSION IN THE PLAN SET. A COPY OF THE FULL SIZE PAGES ARE AVAILABLE AT THE ENGINEERS OFFICE FOR USE.

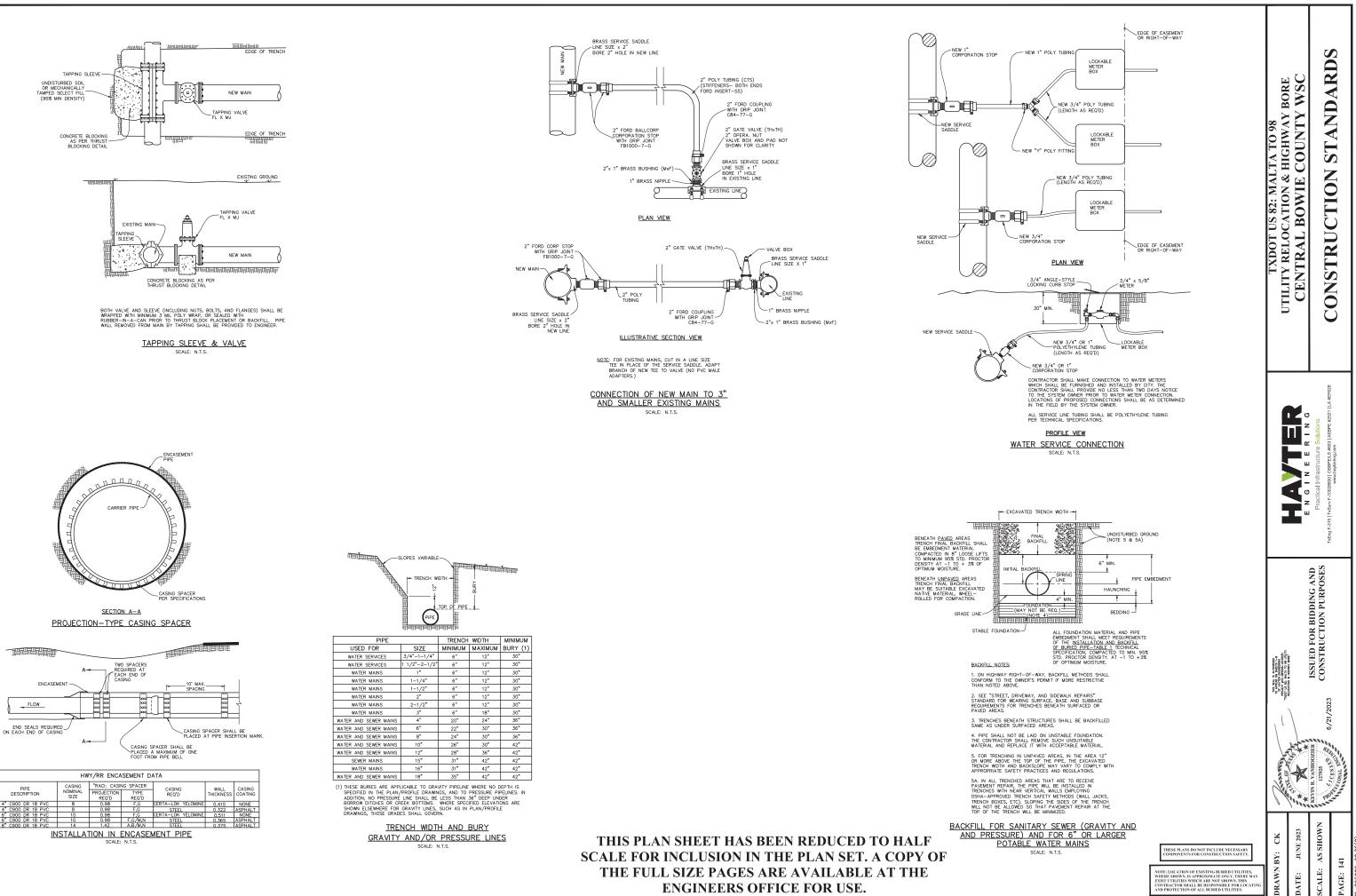


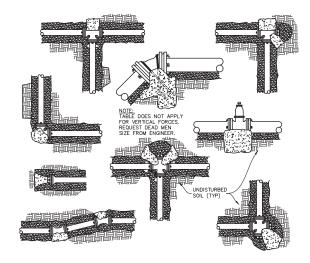


LINE L PLAN VIEW

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View       Kiewie         View       Kiewie         Miewie       Kiewie         Mie	TXDOT US 82: MALTA TO 98 UTILITY RELOCATION & HIGHWAY BORE CENTRAL BOWIE COUNTY WSC	A REFECT	
		Practical Infrastructure urv F-10028600   OSBPELS #6 www.haytereng.con	
	A constant of states a second of states and second of	NUR VANIOZIER IPAS (LENS) 6/21/2023 CONSTRUCTION PURPOSES 6/21/2023	
EDUCED TO HALF LAN SET. A COPY OF AILABLE AT THE FOR USE. VAILABLE AT THE FOR USE.	DRAWN BY: CK DATE: JUNE 2023	NW	Y2-065006-20.05(8)



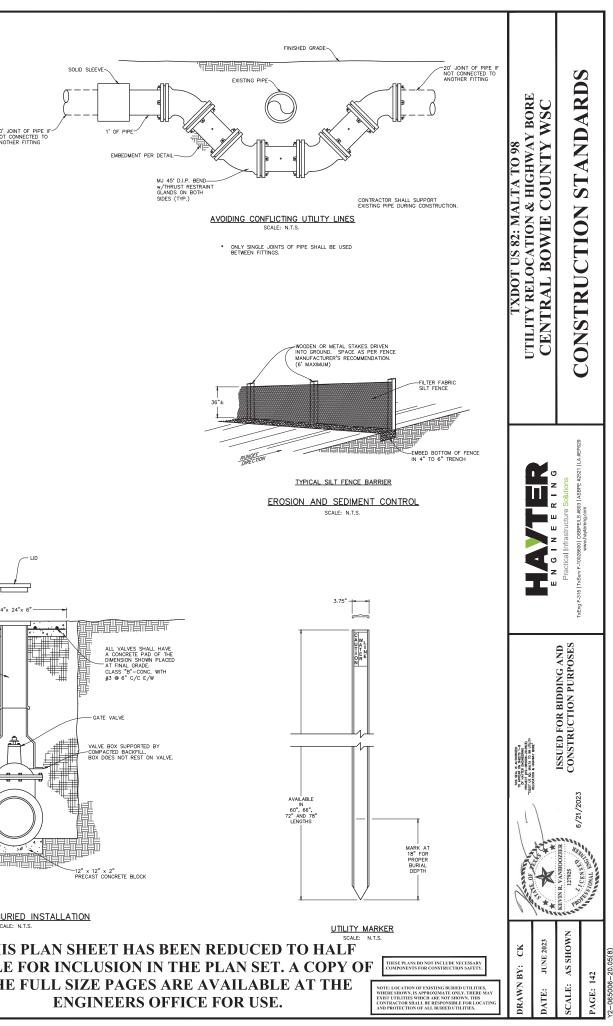


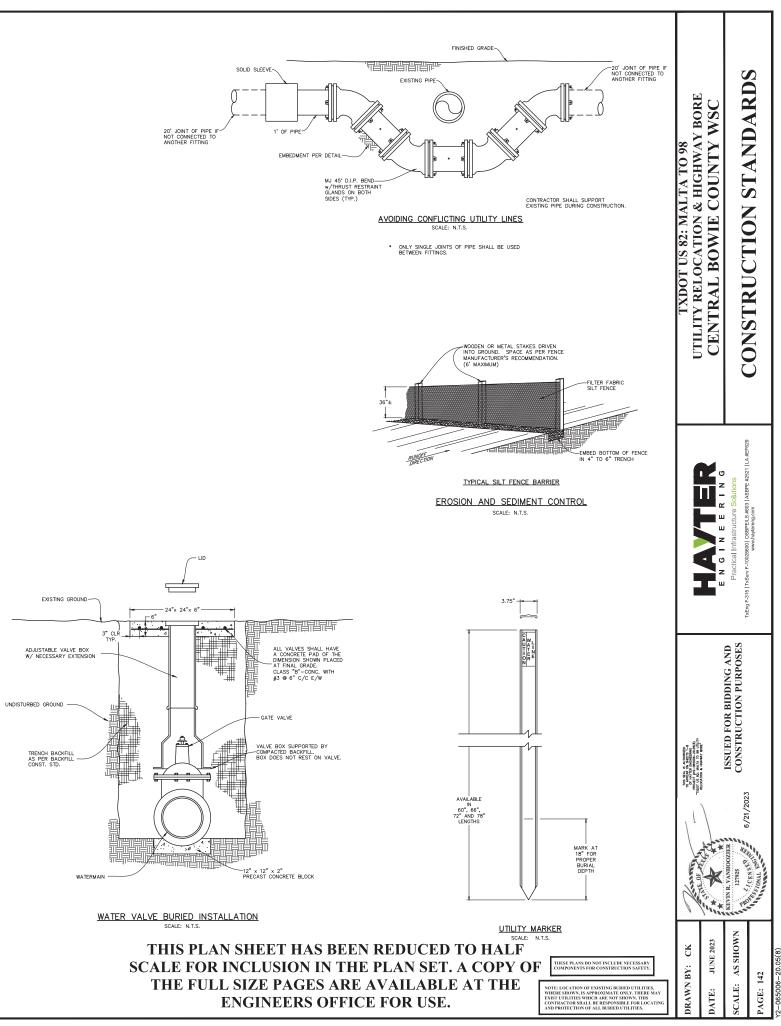
MINIMUM SQ.FT. BEARING AREA REQUIRED AT 150 PSI.

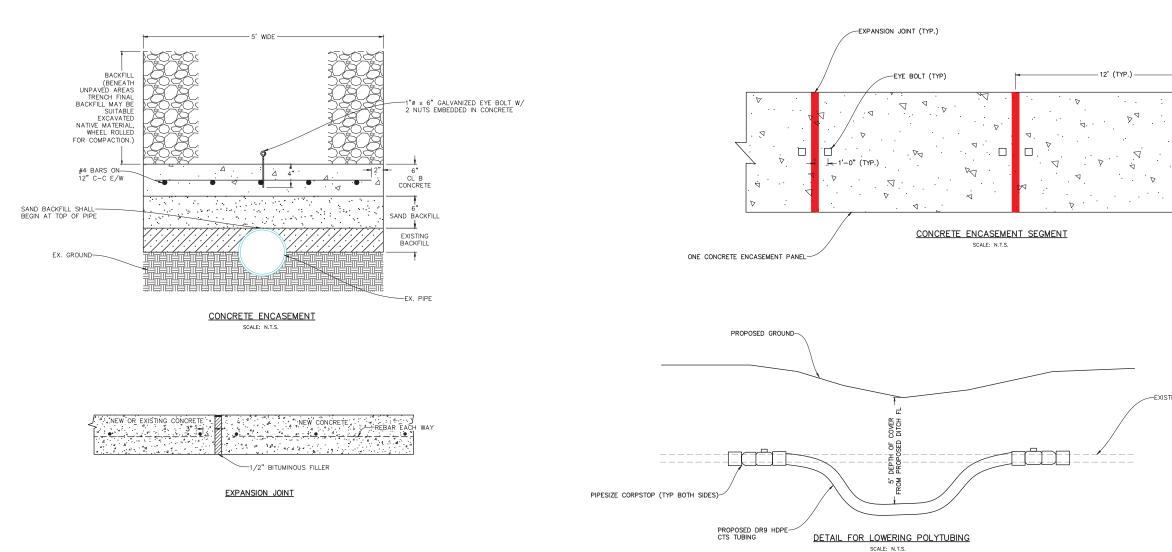
PIPE	11-1/4*	22-1/2	45°	90*	VALVES, TEES & DEAD ENDS	1. AREA IS BASED ON 1000 PSF ALLOWABLE
2"	0.2	0.2	0.5	0.8	0.6	BEARING PRESSURE.
4"	0.5	0.8	1.7	2.7	2.8	2. WHEN SOFT CLAY IS ENCOUNTERED THE
6"	0.9	1.5	3.0	6.0	4.0	BEARING AREAS SHALL BE DOUBLED. 3. USE CONCRETE FORMS TO KEEP CONCRETE
8"	1.4	2.9	6.0	10.0	8.5	CLEAR OF ALL BOLTS AND FLANGES.
10"	2.0	4.5	9.0	16.2	12.0	<ol> <li>TABLE IS BASED ON AWWA NO. 23, PVC PIPE DESIGN MANUAL.</li> </ol>
12"	3.0	6.8	12.0	21.0	16.0	5. ALL FITTINGS (INCLUDING NUTS AND BOLTS)
18"	6.2	14.0	23.0	50.0	30.0	TO BE DOUBLE WRAPPED WITH
20"	11	21	42.2	78	55	MINIMUM 3 MIL POLY WRAP.
						<ol> <li>6. THRUST RESTRAINT GLANDS ARE REQUIRED ON ALL ML CONNECTIONS IN ADDITION TO</li> </ol>

CONCRETE THRUST BLOCKING SCHEDULE

SCALE: N.T.S.







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	Autor a server a servera More constant a serv	A CONSTRUCTION PURPOSES 12795 12795 6/21/2023 6/21/2023 6/21/2023	
REDUCED TO HALF PLAN SET. A COPY OF VAILABLE AT THE FOR USE.	URIED UTILITIES, E ONLY, THERE MAY T SHOWN, THIS SIBLE FOR LOCCATING	06-140	