

STATE OF TEXAS
DEPARTMENT OF TRANSPORTATION

PLANS OF PROPOSED
STATE HIGHWAY IMPROVEMENT

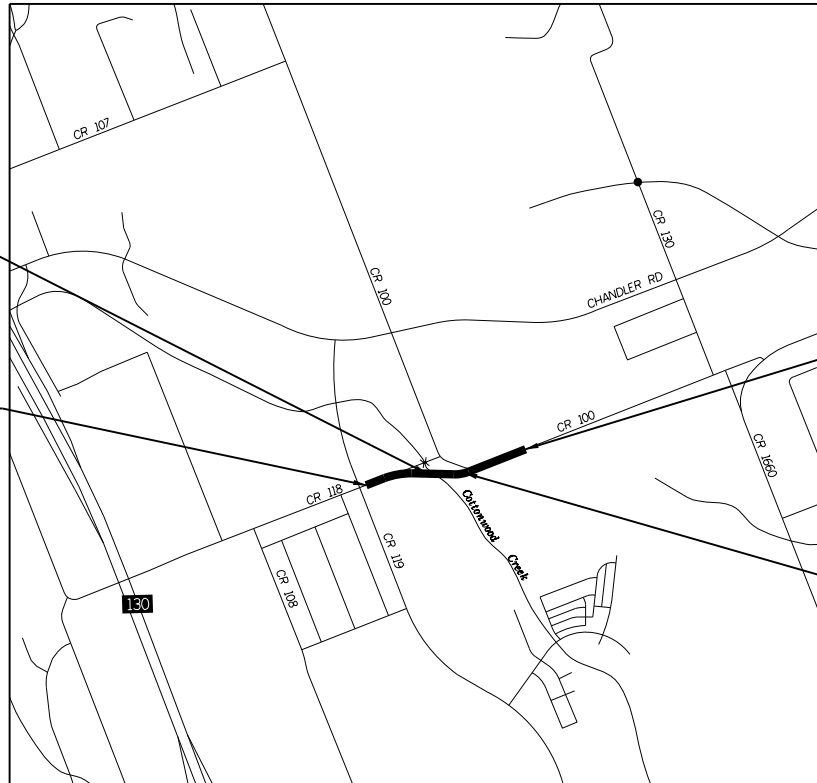
FEDERAL AID PROJECT NUMBER
BR 2020(733), ETC.
CSJ: 0914-05-204, ETC.

| CSJ | ROADWAY LENGTH | | BRIDGE LENGTH | | TOTAL LENGTH | |
|-------------|----------------|-------|---------------|-------|--------------|-------|
| | (FT) | (MI) | (FT) | (MI) | (FT) | (MI) |
| 0914-05-220 | 2470.00 | 0.468 | 0.00 | 0.000 | 2470.00 | 0.468 |
| 0914-05-204 | 0.00 | 0.000 | 250.00 | 0.047 | 250.00 | 0.047 |
| TOTAL | 2470.00 | 0.468 | 250.00 | 0.047 | 2720.00 | 0.515 |

WILLIAMSON COUNTY
CR 118

FROM: CR 118 AT COTTONWOOD CREEK
TO: ., ETC

FOR THE CONSTRUCTION OF BRIDGE REPLACEMENT AND NEW ROAD
CONSISTING OF REPLACING BRIDGE AND APPROACHES,
REALIGNMENT OF CR 118 ON BOTH SIDES OF NEW BRIDGE



BEGIN CSJ 0914-05-204
@ CR 118
STA 115+23.00

BEGIN PROJECT
BEGIN CSJ 0914-05-220
@ CR 118
STA: 101+19.00

END PROJECT
END CSJ 0914-05-220
@ CR 118
STA: 128+39.00

END CSJ 0914-05-204
@ CR 118
STA 117+73.00

LOCATION MAP NOT TO SCALE

EXCEPTIONS: NONE
EQUATIONS: NONE
RAILROAD CROSSINGS: NONE

| CONT | SECT | JOB | HIGHWAY |
|------|------------|-----------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | | SHEET NO. |
| AUS | WILLIAMSON | | 1 |

DESIGN SPEED

60 MPH

A. D. T.

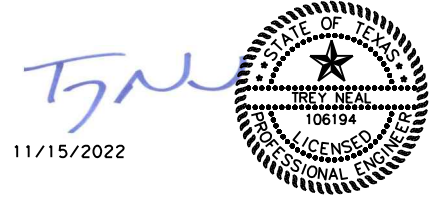
2023: 600 VPD
2043: 800 VPD

FINAL PLANS

NAME OF CONTRACTOR: _____
DATE OF LETTING: _____
DATE WORK BEGAN: _____
DATE WORK COMPLETED: _____
DATE WORK ACCEPTED: _____
FINAL CONTRACT COST: _____
LIST OF APPROVED CHANGE ORDERS:

I CERTIFY THAT THIS PROJECT WAS CONSTRUCTED IN
SUBSTANTIAL COMPLIANCE WITH THE FINAL AS-BUILT
PLANS AND SPECIFICATIONS.

AREA ENGINEER _____ P. E. _____ DATE _____



CORRECT: _____
CONSULTING ENG. (TBPE FIRM REG. F-928)

RECOMMENDED FOR LETTING: 1/5/2023
DocuSigned by: Suzanna Ceballos P.E.
E181616785C74 DISTRICT DESIGN ENGINEER

SUBMITTED FOR LETTING: 1/5/2023
DocuSigned by: [Signature]
089654558998492... AREA ENGINEER

APPROVED FOR LETTING: 1/5/2023
DocuSigned by: [Signature]
8912AF18D454418 DIRECTOR OF TRANSPORTATION, PLANNING & DEVELOPMENT



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TDLR INSPECTION NOT REQUIRED

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF
TRANSPORTATION ON NOVEMBER 1, 2014 AND SPECIFICATION ITEMS
LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT:
REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID
CONSTRUCTION CONTRACTS (FORM FHWA 1273, JULY 2022)

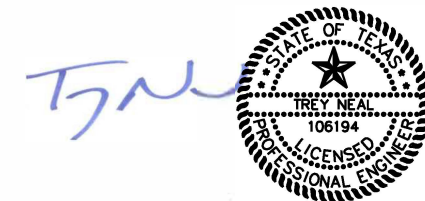
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| SHEET NO. | DESCRIPTION |
|---------------------------------------|--|
| GENERAL | |
| 1 | TITLE SHEET |
| 2 | INDEX OF SHEETS |
| 3 | PROJECT LAYOUT |
| 4 | EXISTING TYPICAL SECTIONS |
| 5 | PROPOSED TYPICAL SECTIONS |
| 6, 6A-6I | GENERAL NOTES |
| 7, 7A | ESTIMATE & QUANTITY |
| 8-9 | QUANTITY SUMMARY |
| TRAFFIC CONTROL PLAN | |
| 10 | TRAFFIC CONTROL PLAN DETOUR LAYOUT |
| 11 | TRAFFIC CONTROL PLAN PHASE 1 |
| 12 | TRAFFIC CONTROL PLAN PHASE 2 |
| 13 | TRAFFIC CONTROL PLAN PHASE 3 |
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| 26 | *WZ(RCD)-13 |
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| 28 | SURVEY CONTROL INDEX SHEET |
| 29 | HORIZONTAL ALIGNMENT DATA |
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| 36-38 | DRIVEWAY PLAN AND PROFILE |
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| 40-41 | *GF(31)TR TL3-20 |
| 42 | *GF(31)MS-19 |
| 43 | *BED-14 |
| 44 | *SGT(10S)31-16 |
| 45 | *SGT(11S)31-18 |
| 46 | *SGT(12S)31-18 |
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* THE STANDARD SHEETS SPECIFICALLY IDENTIFIED ABOVE HAVE BEEN SELECTED BY ME OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.



DESIGN ENGINEER

1/27/2023
DATE

Kimley»Horn

F-928

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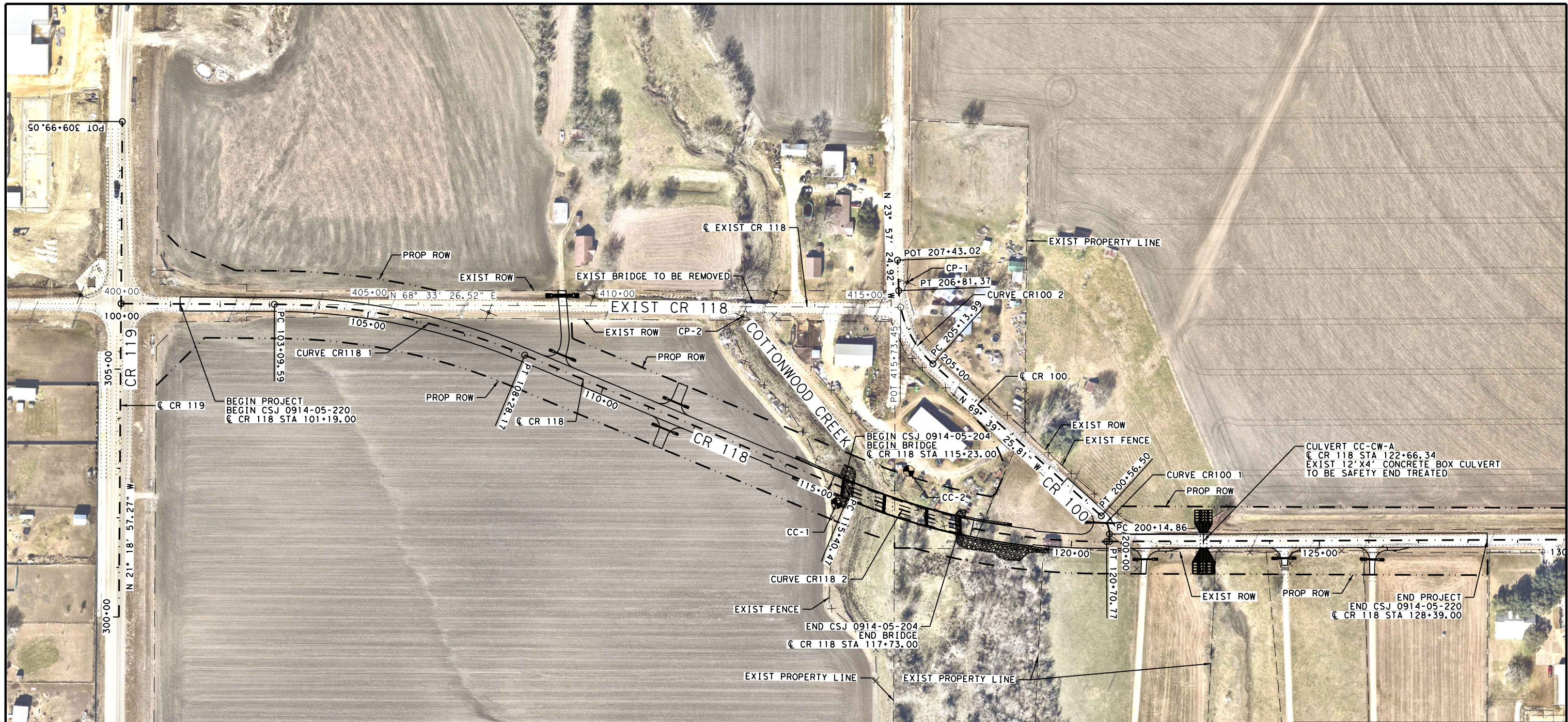
CR 118 AT COTTONWOOD CREEK

INDEX OF SHEETS

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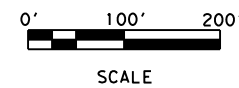
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| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | | COUNTY | SHEET NO. |
| | AUS | | WILLIAMSON | 2 |

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LEGEND

- SURVEY CONTROL POINT
- BORING LOCATION



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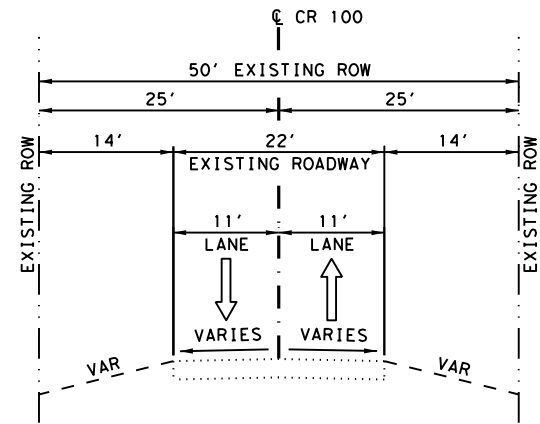
CR 118 AT COTTONWOOD CREEK

PROJECT LAYOUT

SHEET 1 OF 1

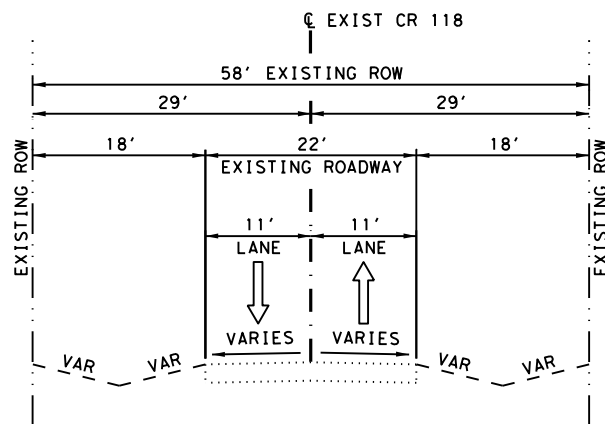
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| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 3 |

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EXISTING CR 100 TYPICAL SECTION

CR 100 STA 200+00.00 TO STA 201+58.00




EXISTING CR 118 TYPICAL SECTION

EXIST CR 118 STA 400+00.00 TO STA 417+73.45

NOTES:

- 1. EXISTING ROADWAYS TO BE OBLITERATED AND REALIGNED.

TJN
 11/3/2022


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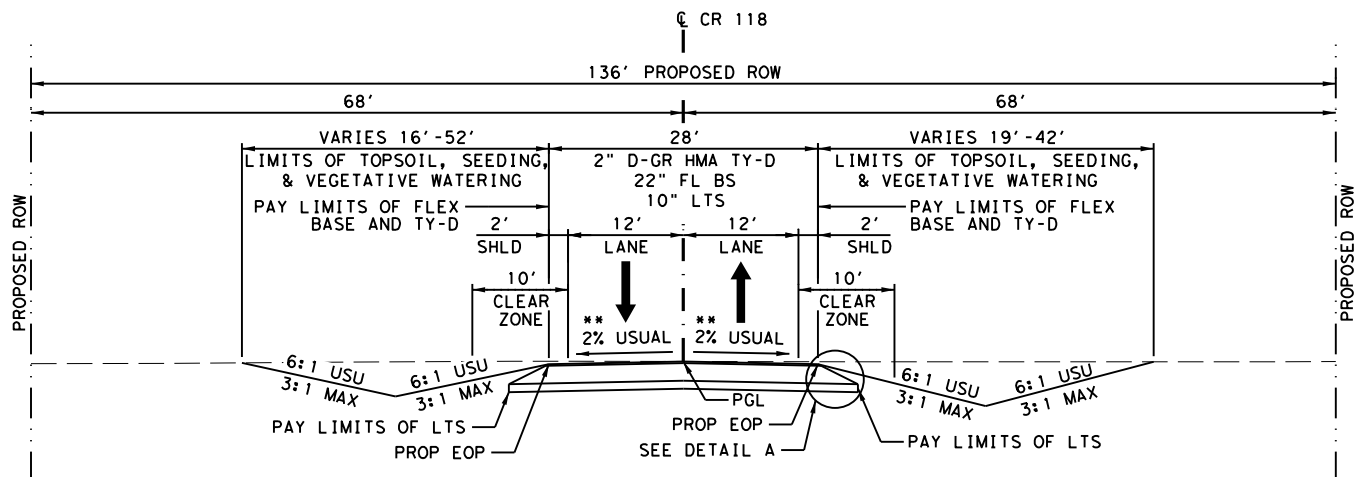
CR 118 AT COTTONWOOD CREEK

EXISTING TYPICAL SECTIONS

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
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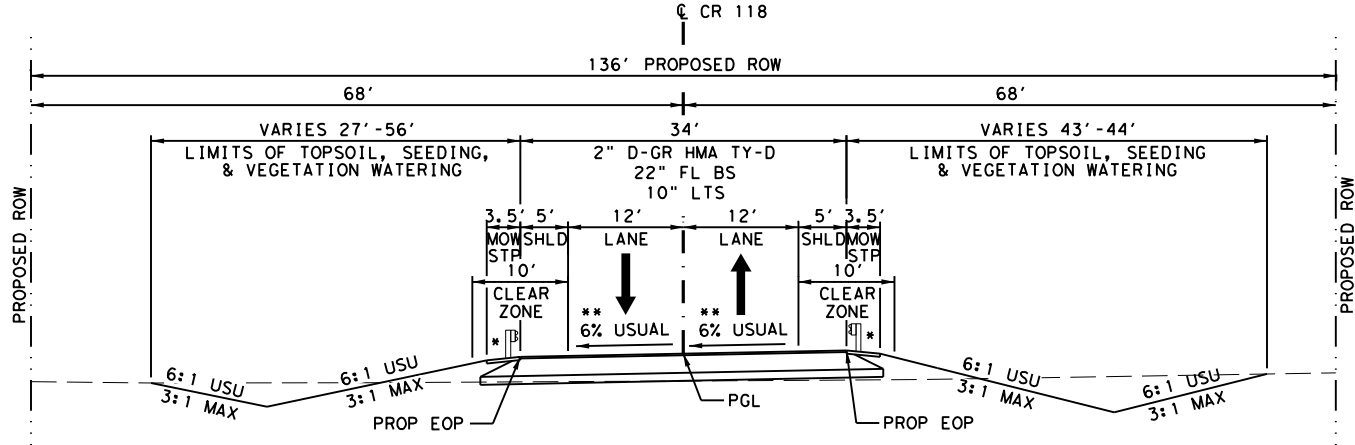
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PROPOSED CR 118 TYPICAL SECTION

CR 118 STA 101+19.00 TO STA 113+73.00
 CR 118 STA 119+56.00 TO STA 128+39.00

**SEE SUPERELEVATION TABLE THIS SHEET AND TRANSITION DIAGRAMS ON ROADWAY PLAN AND PROFILE SHEETS FOR MORE INFORMATION



PROPOSED CR 118 TYPICAL SECTION

CR 118 STA 113+73.00 TO STA 115+23.00
 CR 118 STA 117+73.00 TO STA 119+56.00

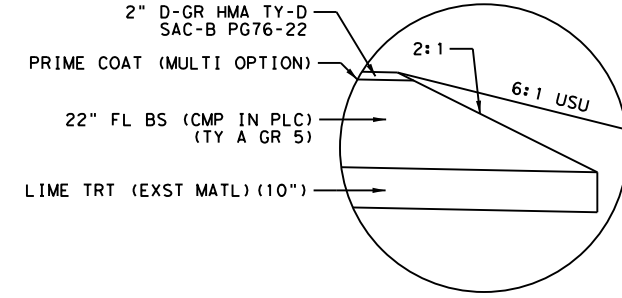
*SEE PLAN AND PROFILE FOR LIMITS OF MBGF

**SEE SUPERELEVATION TABLE THIS SHEET AND TRANSITION DIAGRAMS ON ROADWAY PLAN AND PROFILE SHEETS FOR MORE INFORMATION

| CR 118 SUPERELEVATION TABLE | | | |
|-----------------------------|---------|---------|--|
| STATION | WB LANE | EB LANE | COMMENTS |
| 101+19.00 | -1.93% | -3.45% | MATCH EXIST / BEGIN CROSS SLOPE TRANSITION |
| 104+37.00 | 6.00% | -6.00% | END CROSS SLOPE TRANSITION |
| FULL SUPERELEVATION | | | |
| 107+65.00 | 6.00% | -6.00% | BEGIN SUPERELEVATION TRANSITION |
| 110+85.00 | -2.00% | -2.00% | END SUPERELEVATION TRANSITION |
| NORMAL CROWN | | | |
| 112+48.00 | -2.00% | -2.00% | BEGIN CROSS SLOPE TRANSITION |
| 115+68.00 | -6.00% | 6.00% | END CROSS SLOPE TRANSITION |
| FULL SUPERELEVATION | | | |
| 120+03.00 | -6.00% | 6.00% | BEGIN SUPERELEVATION TRANSITION |
| 123+23.00 | -2.00% | -2.00% | END SUPERELEVATION TRANSITION |
| NORMAL CROWN | | | |
| 127+76.00 | -2.00% | -2.00% | BEGIN CROSS SLOPE TRANSITION |
| 128+39.00 | -0.44% | -2.86% | END CROSS SLOPE TRANSITION/MATCH EXIST |

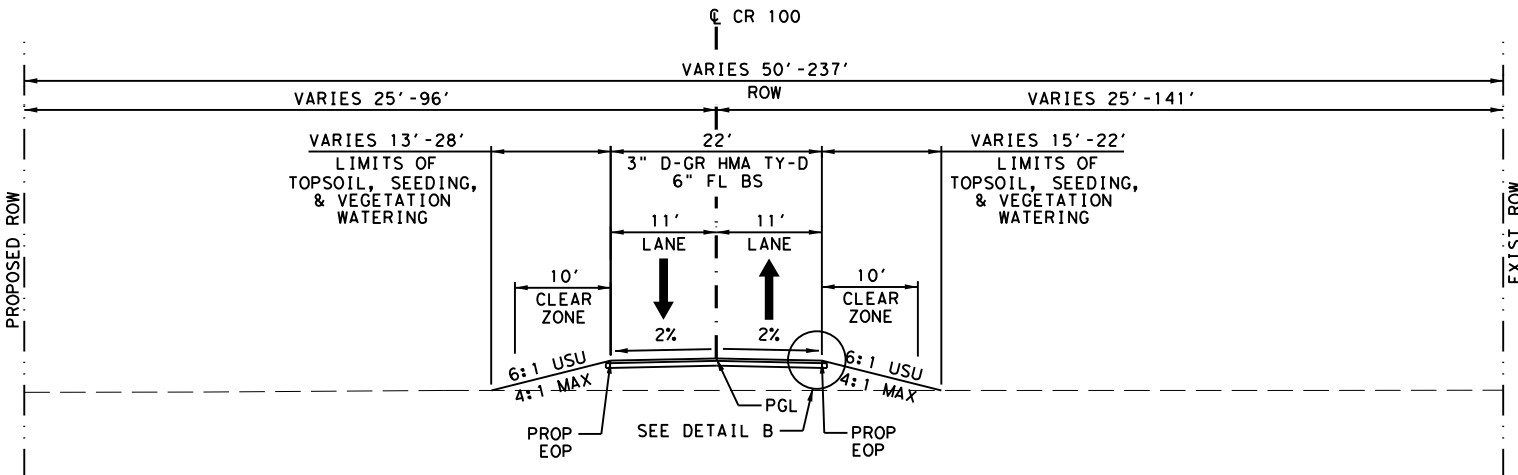
NOTES:

- PROPOSED BRIDGE FROM CR 118 STATION 115+23.00 TO STATION 117+73.00. SEE BRIDGE TYPICAL SECTION FOR MORE INFORMATION.



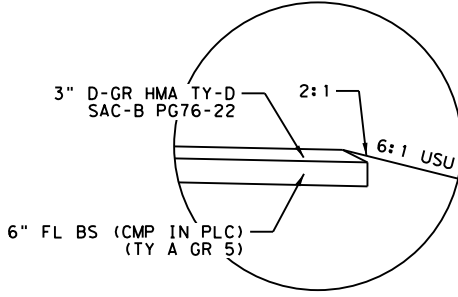
DETAIL A

TJN
 11/28/2022



PROPOSED CR 100 TYPICAL SECTION

CR 100 STA 200+00.00 TO STA 201+58.00



DETAIL B

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 CR 118 AT COTTONWOOD CREEK
 PROPOSED TYPICAL SECTIONS
 SHEET 1 OF 1

| | | | |
|------|------|------------|-----------|
| CONT | SECT | JOB | HIGHWAY |
| 0914 | 05 | 204, ETC. | CR 118 |
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GENERAL NOTES: Version: November 4, 2022

| Item | Description | **Rate |
|--------------------|---|---|
| **204 | Sprinkling (Dust) (Item 132) (Item 247) | 30 GAL/CY 30 GAL/CY 30 GAL/CY |
| **210 | Rolling (Flat Wheel) (Item 247) (Item 316) | 1 HR/200 TON 1 HR/6000 SY |
| **210 | Rolling (Tamping and Heavy Tamping) | 1 HR/200 CY |
| **210 | Rolling (Lt Pneumatic Tire) (Item 132) (Item 247) (Item 316 - Seal Coat) (Item 316 - Two Course) | 1 HR/500 CY 1 HR/200 TON 1 HR/6000 SY 1 HR/3000 SY |
| 247 | Flexible Base (CMP IN PLC) | 132 LB/CF |
| 310 | Prime Coat | 0.20 GAL/SY |
| 314 | Emulsified Asphalt Treatment (SS-1 or MS-2) | 0.30 GAL/SY |
| 316 | Underseals Asphalts (Multi Option) | 0.20 GAL/SY |
| | Surface Treatments | |
| | Seal Coat | |
| | Grade 4 | |
| | Asphalt | 0.38 GAL/SY |
| | Aggregate | 1 CY/120 SY |
| | Grade 5 | |
| | Asphalt | 0.32 GAL/SY |
| | Aggregate | 1 CY/150 SY |
| | Two Course Surface Treatment | |
| | Asphalt 1st Application | 0.28 GAL/SY |
| | Asphalt 2nd Application | 0.24 GAL/SY |
| | Aggregate 1st Application Grade 4 | 1 CY/110 SY |
| | Aggregate 2nd Application Grade 4 | 1 CY/130 SY |
| 341/3076, 344/3077 | Dense-Graded Hot-Mix Asphalt and Superpave | 110 LB/SY/IN |
| 342/3079 | Permeable Friction Course (PFC) | 90.0 LB/SY/IN |
| 346/3080 | Stone-Matrix Asphalt | 113 LB/SY/IN |
| 347/3081 | Thin Overlay Mixtures (TOM) SAC B SAC A | 113.0 LB/SY/IN 116.0LB/SY/IN |
| 350 | Microsurfacing | 25 LB/SY |
| 3084 | Bonding Course | 0.09 GAL/SY |
| 3085 | UnderSeal Course | 0.20 GAL/SY |
| | Tack Coat | 0.08 GAL/SY |

** For Informational Purposes Only

GENERAL

Contractor questions on this project are to be addressed to the following individual(s):

Georgetown Jason.Hudson@txdot.gov
Georgetown John.Peters@txdot.gov

Questions and requests for documents will be accepted via the Letting Pre-Bid Q&A web page. All questions and any corresponding responses that are generated will be posted through the same Letting Pre-Bid Q&A web page. This webpage can be accessed from the Notice to Contractors dashboard located at the following Address:
<https://tableau.txdot.gov/views/ProjectInformationDashboard/NoticetoContractors>

The Letting Pre-Bid Q&A web page for each project can be accessed by using the dashboard to navigate to the project you are interested in by scrolling or filtering the dashboard using the controls on the left. Hover over the blue hyperlink for the project you want to view the Q&A for and click on the link in the window that pops up.

References to manufacturer's trade name or catalog numbers are for the purpose of identification only. Similar materials from other manufacturers are permitted if they are of equal quality, comply with the specifications for this project, and are approved.

If work is performed at Contractor's option, when inclement weather is impending, and the work is damaged by subsequent precipitation, the Contractor is responsible for all costs associated with replacing the work, if required.

The roadbed will be free of organic material prior to placing any section of the pavement structure.

Equip all construction equipment used in roadway work with highly visible omnidirectional flashing warning lights.

Provide a smooth, clean sawcut along the existing asphalt or concrete pavement structure, as directed. Consider subsidiary to the pertinent Items.

Use a self-contained vacuum broom to sweep the roadway and keep it free of sediment as directed. The contractor will be responsible for any sweeping above and beyond the normal maintenance required to keep fugitive sediment off the roadway as directed by the Engineer.

Damage to existing pipes and SET's due to Contractor operations will be repaired at Contractor's expense.

All locations used for storing construction equipment, materials, and stockpiles of any type, within the right of way, will be as directed. Use of right of way for these purposes will be restricted to those locations where driver sight distance to businesses and side street intersections is not obstructed and at other locations where an unsightly appearance will not exist. The Contractor will not have exclusive use of right of way but will cooperate in the use of the right of way with the city/county and various public utility companies as required.

Coordinate and obtain approval for all bridgework over existing roadways.

Bridge Vertical Clearance and Traffic Handling.

Notify TxDOT project staff and the local bridge engineer 10 business days prior to the following: change in vertical clearance, placing beams/girders over traffic, opening or removing traffic from a bridge or portion of a bridge, and completion of bridge work. This requirement includes bridge class culverts. Provide vertical clearance for all structures (including signal mast arms, span wires, and overhead sign bridge structures) within the project limit. Submit information and notices to local bridge engineer at AUS_BRG_Notify@txdot.gov.

During evacuation periods for Hurricane events the Contractor will cooperate with Department for the restricting of Lane Closures and arranging for Traffic Control to facilitate Coastal Evacuation Efforts.

ITEM 5 – CONTROL OF THE WORK

Place construction stakes at intervals of no more than 100 ft. This work is subsidiary.

Electronic Shop Drawing Submittals.

Submit electronic shop drawing submittals according to the current [Guide to Electronic Shop Drawing Submittal](https://www.txdot.gov/business/resources/specifications/shop-drawings.html) (<https://www.txdot.gov/business/resources/specifications/shop-drawings.html> (TxDOT.gov Business > Resources - General > Shop Drawings). Pre-approved producers can be found online at TxDOT.gov > Business > Resources - Material Producer List. Use the following contact list for all submittals that are not required to be sent to Bridge Division and to copy the Engineer for all submittals to the Bridge Division.

Submittal Contact List

Georgetown Jason.Hudson@txdot.gov AUS_GE-ShopReview@txdot.gov

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with “Standard Operating Procedure for Alternate Precast Proposal Submission” found online at <https://www.txdot.gov/inside-txdot/forms-publications/consultants-contractors/publications/bridge.html#design>. Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

ITEM 6 - CONTROL OF MATERIALS

Give a minimum of 1 business day notice for materials, which require inspection at the Plant.

For structures with paint containing hazardous materials, provide locations of material removal 60 days prior to begin removal. For metal elements to be removed, mechanical shear or unbolting for removal and disposal does not require paint abatement but requires 60 day advance notice.

For Federally Funded Contracts, comply with the latest provisions of Build America, Buy America Act (BABA Act) of the Bipartisan Infrastructure Law, by submitting a notarized original of the TxDOT Construction Material Buy America Certification Form for all items classified as construction materials. This form is not required for materials classified as a manufactured product. Refer to the Buy America Material Classification Sheet, located at the following link, for clarification on material categorization. <https://www.txdot.gov/business/resources/materials/buy-america-material-classification-sheet.html>

ITEM 7 – LEGAL RELATIONS AND RESPONSIBILITIES

TxDOT will coordinate with TDLR regarding pedestrian elements and sidewalks. The contractor will procure and provide all permits, licenses, and inspections; pay all charges, fees, and taxes regarding TDLR rules governing industrialized housing and buildings.

Roadway closures during key dates and/or special events are prohibited. See notes for Item 502 for the key dates and/or special events.

Refer to the Environmental Permits, Issues and Commitments (EPIC) plan sheets for additional requirements and permits.

When any abandoned well is encountered, cease construction operations in this area and notify the Engineer who will coordinate the proper plugging procedures. A water well driller licensed in the State of Texas must be used to plug a well.

Perform maintenance of vehicles or equipment at designated maintenance sites. Keep a spill kit on-site during fueling and maintenance. This work is subsidiary.

Maintain positive drainage for permanent and temporary work for the duration of the project. Be responsible for any items associated with the temporary or interim drainage and all related maintenance. This work is subsidiary.

Suspend all activities near any significant recharge features, such as sinkholes, caves, or any other subterranean openings that are discovered during construction or core sampling. Do not proceed until the designated Geologist or TCEQ representative is present to evaluate and approve remedial action.

Locate aboveground storage tanks kept on-site for construction purposes in a contained area as to not allow any exposure to soils. The containment will be sized to capture 150% of the total capacity of the storage tanks.

Work over or near Bodies of Water (lakes, rivers, ponds, creeks, dry waterways, etc.).

Keep on site a universal spill kit adequate for the body of water and the work being performed. Debris is not allowed to fall into the ordinary high-water level (OHWL). Debris that falls into the OHWL must be removed at the end of each work day. Debris that falls into the floodway must be removed at the end of each work week or prior to a rain event. Install and maintain traffic

control devices to maintain a navigable corridor for water traffic, except during bridge demo and beam placement. This work is subsidiary.

Obtain written approval from the Engineer for temporary fill or crossings not specifically addressed in the plans. Provide a signed sketch of the location 60 business days prior to begin work at the location. Complete and return any forms provided by TxDOT. Approval of the work is not guaranteed. Unapproved work is not a compensable impact.

DSHS Asbestos and Demolition Notification.

Complete and provide the Texas Department of State Health Services (DSHS) notification form to the Engineer and email to AUS_BRG_Notify@txdot.gov at least 30 calendar days prior to bridge removal or renovation for each phase or step of work. Notify the Engineer via email of any changes to the work start and end dates.

Migratory Birds and Bats.

Migratory birds and bats may be nesting within the project limits and concentrated on roadway structures such as bridges and culverts. Remove all old and unoccupied migratory bird nests from any structures, trees, etc. between September 16 and February 28. Prevent migratory birds from re-nesting between March 1 and September 15. Prevention shall include all areas within 25 ft. of proposed work. All methods used for the removal of old nesting areas and the prevention of re-nesting must be submitted to TxDOT 30 business days prior to begin work. This work is subsidiary.

If active nests are encountered on-site during construction, all construction activity within 25 ft. of the nest must stop. Contact the Engineer to determine how to proceed.

Tree and Brush Trimming and Removal.

Work will be conducted September 16 thru February 28. Work conducted outside this timeframe will require a bird survey. Submit a survey request to TxDOT 30 business days prior to begin work.

No extension of time or compensation will be granted for a delay or suspension due to the above bird, bat, and tree/brush requirements.

Back Up Alarm.

For hours 9 P to 5 A, utilize a non-intrusive, self-adjusting noise level reverse signal alarm. This is not applicable to hotmix or seal coat operations. This is subsidiary.

Vegetation BMP

- Minimize the amount of vegetation cleared. Removal of native vegetation, particularly mature native trees and shrubs should be avoided.
- The use of any non-native vegetation in landscaping and revegetation is discouraged. Locally adapted native species should be used.
- The use of seed mix that contains seeds from only regional ecotype native species is recommended.

Water Quality BMP

In addition to BMP required for a TCEQ Storm Water Pollution Prevention Plan and/or 401 Water Quality Certification:

- Minimize the use of equipment in streams and riparian areas during construction. When possible, equipment access should be from banks, bridge decks, or barges.
- When temporary stream crossings are unavoidable, remove stream crossings once they are no longer needed and stabilize banks and soils around the crossing.

Aquatic Amphibian and Reptile BMP

- Minimize impacts to wetlands, temporary and permanent open water features, including depressions, and riverine habitats.
- Maintain the existing hydrologic regime and any connections between wetlands and other aquatic features.
- Use barrier fencing to direct animal movements away from construction activities and areas of potential wildlife-vehicle collisions in construction areas directly adjacent, or that may directly impact, potential habitat for the target species.
- Apply hydro mulching and/or hydroseeding in areas for soil stabilization and/or revegetation of disturbed areas around wetlands and in riparian areas. If erosion control blankets or mats will be used, the product should not contain netting, but should only contain loosely woven natural fiber netting in which the mesh design allows the threads to move, therefore allowing expansion of the mesh openings. Plastic netting should be avoided.
- Project specific locations (PSLs) proposed within state-owned ROW should be located in uplands away from aquatic features.

Terrestrial Amphibian and Reptile BMP

- For open trenches and excavated pits, install escape ramps at an angle of less than 45 degrees (1:1) in areas left uncovered. Visually inspect excavation areas for trapped wildlife prior to backfilling
- Avoid or minimize disturbing or removing cover objects, such as downed trees, rotting stumps, brush piles, and leaf litter. If avoidance or minimization is not practicable, consider removing cover objects prior to the start of the project and replace them at project completion.
- Examine heavy equipment stored on site before use, particularly after rain events when reptile and amphibian movements occur more often, to ensure use will not harm individuals that might be seeking temporary refuge.
- Due to increased activity (mating) of reptiles and amphibian during the spring, construction activities like clearing or grading should attempt to be scheduled outside of the spring (March-May) season. Also, timing ground disturbing activities before October when reptiles and amphibians become less active and may be using burrows in the project area is also encouraged.

ITEM 8 – PROSECUTION AND PROGRESS

The sequence of work shown on the plans demonstrates a volume of work available in each phase of construction that will ensure the Contractor is not impacted by the unclear ROW, railroad, and utilities. A deviation from the sequence of work shown on the plans must be approved by the Engineer.

Special Provision 008-003 has been included to amend Standard Article 8.1 to extend the begin work date due to Materials manufacturing.

A CPM schedule in Primavera format and a PSSR is required. Use software fully compatible with Primavera P6.

ITEM 100 - PREPARING RIGHT OF WAY

Prep ROW must not begin until accessible trees designated for preservation have been protected, items listed in the EPIC have been addressed, and SW3P controls installed in accessible areas.

Backfill material will be Type B Embankment using ordinary compaction.

Follow Item 752.4 Work Methods and Item 752 general notes when removing or working on or near trees and brush.

Unless shown otherwise in the plans or a designated non-mow area, perform trimming or removal for areas within 30 ft. of edge of pavement under construction. Trim or remove to provide minimum of 5 ft. of horizontal clearance and 7 ft. of vertical clearance for the following: sidewalks, paths, guard fence, rails, signs, object markers, and structures. Trim to provide a minimum of 14 ft. vertical clearance under all trees. This work is subsidiary.

ITEM 105 – REMOVING TREATED AND UNTREATED BASE AND ASPHALT PAVEMENT

Existing typical is based on information available. This typical may not account for all maintenance work such as overlays or pavement repairs. A change in material type or thickness does not warrant additional payment. Payment is full compensation for removing all material to the depth specified.

ITEM 110 – EXCAVATION

The Engineer will define unsuitable material.

ITEM 132 – ALL EMBANKMENT

At no time will the retaining wall backfill material exceed the adjacent embankment operation by more than one lift. At no time will the embankment adjacent to the retaining wall backfill exceed the wall backfill by any elevation. Embankment placed over the area of MSE backfill must meet the same backfill requirements for the type specified under Item 423.

The Engineer will define unsuitable material. Material which the Contractor might deem to be unsuitable due to moisture content will not be considered unsuitable material.

Prior to begin embankment of existing area, correct or replace unstable material to a depth of 6 in. below existing grade. Embankment areas will be inspected prior to beginning work.

Rock or broken concrete produced by the project is allowed in earth embankments. The size of the rock or broken concrete will not exceed the layer thickness requirements in Section 132.3.4., “Compaction Methods.” The material will not be placed vertically within 5 ft. of the finished subgrade elevation.

Embankment placed vertically within 5 ft. of the finished subgrade elevation or within the edges of the subgrade and treated with lime, cement, or other calcium-based additives must have a sulfate content less than 3000 ppm. Allow 5 business days for testing. Treatment of sulfate material 3000 ppm to 7000 ppm requires 7 days of mellowing and continuous water curing, in accordance TxDOT guidelines for Treatment of Sulfate-Rich Soils and Bases in Pavement Structures (9/2005). Material over 7000 ppm is not allowed.

ITEM 160 - TOPSOIL

Off-site topsoil will have a minimum PI of 25.

No Sandy Loam allowed.

Obtain approval of the actual depth of the topsoil sources for both on-site and off-site sources.

Construct topsoil stockpiles of no more than five (5) feet in height.

It is permissible to use topsoil dikes for erosion control berms within the right of way, as directed.

Seed or track slopes within 14 days of placement.

Salvage topsoil from sites of excavation and embankment. Maximum salvage depth is 6 inches.

Windrowing of topsoil obtained from the Right of Way (ROW) is not allowed.

ITEM 168 – VEGETATIVE WATERING

Water all areas of project to be seeded or sodded.

Maintain the seedbed in a condition favorable for the growth of grass. Watering can be postponed immediately after a rainfall on the site of ½ inch or greater but will be resumed before the soil dries out. Continue watering until final acceptance.

Vegetative watering rates and quantities are based on ¼ inch of watering per week over a 3-month watering cycle. The actual rates used and paid for will be as directed and will be based on prevailing weather conditions to maintain the seedbed.

Obtain water at a source that is metered (furnish a current certification of the meter being used) or furnish the manufacturer’s specifications showing the tank capacity for each truck used. Notify the Engineer, each day that watering takes place, before watering, so that meter readings or truck counts can be verified.

ITEM 169 – SOIL RETENTION BLANKETS

Type A blankets containing straw fibers are not allowed. Type B and D blankets shall be a spray type blanket.

ITEM 247 - FLEXIBLE BASE

The layer thickness will be 4 in. to 6 in. unless shown on the plans. Placing in a single layer is allowed when total thickness of base is 8 in. or less. When placed in multiple layers, compact the bottom and middle layers to at least 95% and 98% of the maximum dry density, respectively. When placed in a single layer or the final layer, compact to at least 100%.

Correction of subgrade soft spots is subsidiary.

Complete per plans the subgrade, ditches, slopes, and drainage structures prior to the placement of base.

Do not use a vibratory roller to compact base placed directly on top of a drainage structure.

Grade 4 will have the same material requirements as Grade 5 except minimum compressive strength at lateral pressure 3 psi will be 70 psi and at lateral pressure 15 psi will be 150 psi. Grade 4 does not have a minimum compressive strength at lateral pressure 0 psi.

Flex base may use ordinary compaction. Proof rolling of the base is required and subsidiary.

ITEMS 260 THRU 276 – SUBGRADE TREATMENTS AND BASE

Use ordinary compaction for subgrade treatment.

Three weeks prior to treatment, provide a sample of soil or flexible base to be treated.

ITEM 260 - LIME TREATMENT (ROAD-MIXED)

Apply 45 pounds per square yard.

For sulfate content greater than 3000 ppm, mix in an additional 4.0% points above optimum moisture after initial mixing and prior to mellow.

If the sulfate content is greater than 7000 ppm, do not treat. Undercut the unsuitable material to the depth per bid item for lime treatment and replace unsuitable material in accordance with Item 110. Payment will be made in accordance with Item 110.

ITEMS 341, 344, & 3076 THRU 348/3082 - HOT-MIX ASPHALT PAVEMENT

Core holes may be filled with an Asphaltic patching material meeting the requirements of DMS-9203 or with SCM meeting requirements of DMS-9202.

Install transverse butt joints with 50 ft. H: 1 in. V transition from the new ACP to the existing surface. Install a butt joint with 24 in. H: 1 in. V transition from the new ACP to a driveway, pullout or intersection. Saw cut the existing pavement at the butt joints. This work is subsidiary.

Use a device to create a maximum 3H:1V notched wedge joint on all longitudinal joints of 2 in. or greater. This work is subsidiary.

Prior to milling, core the existing pavement to verify thickness. This work is subsidiary.

Ensure placement sequence to avoid excess distance of longitudinal joint lap back not to exceed one day's production rates.

Submit any proposed adjustments or changes to a JMF before production of the new JMF.

Tack every layer. Do not dilute tack coat. Apply it evenly through a distributor spray bar.

Provide a minimum transition of 10' for intersections, 10' for commercial driveways, and 6' for residential driveways unless otherwise shown on the plans.

Irregularities will require the replacement of a full lane width using an asphalt paver. Replace the entire subplot if the irregularities are greater than 40% of the subplot area.

Lime or an approved anti-stripping agent must be used when crushed gravel is utilized to meet a SAC "A" requirement.

When using RAP or RAS, include the management methods of processing, stockpiling, and testing the material in the QCP submitted for the project. If RAP and RAS are used in the same mix, the QCP must document that both of these materials have dedicated feeder bins for each recycled material. Blending of RAP and RAS in one feeder bin or in a stockpile is not permitted.

Asphalt content and binder properties of RAP and RAS stockpiles must be documented when recycled asphalt content greater than 20% is utilized.

No RAS is allowed in surface courses.

Department approved warm-mix additives is required for all surface mix application when RAP is used. Dosage rates will be approved during JMF approval.

The Hamburg Wheel Test will have a minimum rut depth of 3mm except for SMA with HPG or PG 76.

ITEMS 341/3076 - DENSE-GRADED HOT-MIX ASPHALT

Use the SGC for design and production testing of all mixtures. Design all Type D mixtures as a surface mix, maximum 15% RAP and no RAS. Contractor may not use a substitute PG binder for 76-22. When using substitute binders, mold specimens for mix design and production at the temperature required for the substitute binder used to produce the HMA.

The Hamburg Wheel minimum number of passes for PG 64 or lower is reduced to 7,000. The Engineer may accept Hamburg Wheel test results for production and placement if no more than 1 of the 5 most recent tests is below the specified number of passes and the failing test is no more than 2,000 passes below the specified number of passes.

ITEM 400 - EXCAVATION AND BACKFILL FOR STRUCTURES

Unless shown on the plans, the following backfill will apply to cutting and restoring flexible pavement. Backfill with cement-stabilized backfill. The cement-stabilized backfill is subsidiary.

Cap the backfill with Type B hot-mix to a depth equal to the adjacent hot-mix. At locations where the backfill surface is final, place 1-1/2 in. Type D for the surface. The minimum hot-mix depth will be 4 in.

Unless shown on the plans, flowable fill option 1 item will be used for pavement widening.

Saw-cut the pavement at the edge of the excavation. This work is subsidiary.

Backfill the bridge ends in accordance with the limits shown on TxDOT "CSAB" Standard. Use material in accordance with "CSAB" or Item 423, Type BS. The "CSAB" optional bond breaker materials are allowed. This work is subsidiary.

ITEM 416 - DRILLED SHAFT FOUNDATIONS

Stake all Foundations, for approval, before beginning drilling operations.

Calculate the vertical signal head clearance before placing any signal pole foundation.

For mast-arm signal and strain pole anchor bolts, set two in tension and two in compression.

Obtain approval of placement prior to placing concrete.

Remove spoils from a flood plain at the end of each workday.

ITEMS 420, 425, 441, & 462 - STRUCTURES

Bridge Vertical Clearance and Traffic Handling.

Notify TxDOT project staff and the local bridge engineer 10 business days prior to the following: change in vertical clearance, placing beams/girders over traffic, opening or removing traffic from a bridge or portion of a bridge, and completion of bridge work. This requirement includes bridge class culverts. Provide vertical clearance for all structures (including signal mast arms, span wires, and overhead sign bridge structures) within the project limit. Submit information and notices to local bridge engineer at AUS_BRG_Notify@txdot.gov.

ITEM 420 – CONCRETE SUBSTRUCTURES

Do not use PMDF in areas where a "Free Joint" is indicated in the plans.

Check the sign plans for locations of clearance signs and brackets on structures, which will require inserts in the pre-stressed beams.

Where Retaining Walls are integral parts of the abutment header, do not place the abutment cap prior to backfilling the wall and the abutment area up to the elevation of the bottom of the abutment cap.

Mass placements are defined as placements with a least dimension greater than or equal to 5 ft., or designated elsewhere on the plans.

The "H" values shown on Bridge Layouts are estimated column heights. Calculate the actual column heights based on field conditions.

Perform work during good weather unless otherwise directed. If work is performed at Contractor's option, when inclement weather is impending, and the work is damaged by the weather, the Contractor is responsible for all costs associated with repairs/replacement.

Upon completion of the structure, stencil the National Bridge Inventory (NBI) number (structure number) using black paint and 4 in. tall numbers at 4 locations designated by TxDOT. This work is subsidiary.

Bonding agents are required at construction joints. Do not use membrane curing for structural concrete as defined in Item 421, Table 8.

Remove all loose Formwork and other Materials from the floodplain or drainage areas daily.

ITEM 425 - PRECAST PRESTRESSED CONCRETE STRUCTURAL MEMBERS

Conduct a pre-placement meeting for the erection of structural members.

ITEM 432 - RIPRAP

Mow strip riprap will be 4 in. and all other riprap will be 5 in. unless otherwise shown on the plans. Mow strip for cable barrier may be placed monolithically with the barrier foundations if using concrete in accordance with Item 543. Fiber reinforcement is not allowed except in mow strip for cable barrier if foundation and mow strip are placed monolithically. GFRP is allowed reinforcement for all applications.

Saw-cut existing riprap then epoxy 12 in. long No. 3 or No. 4 bars 6 in. deep at a maximum spacing of 18 in. in each direction to tie new riprap to existing riprap. This work is subsidiary.

Provide Type A Grade 3 or 5 flexible base for cement stabilized riprap. Compressive strengths for flexible base are waived.

SGT approach taper, paid for using mow strip item, will be installed using concrete, flexible base coated with SS-1 at a rate of 0.12 GAL/SY, or HMA Type B/C/D. Placement will be ordinary compaction and does not require placement using an asphalt paver.

ITEM 450 - RAILING

Use the elliptical tube option for rails T401, T402, and C402.

ITEM 454 - BRIDGE EXPANSION JOINTS

Apply protection System II in accordance with Item 446 to armor joint.

ITEM 496 - REMOVING STRUCTURES

Submit a demolition plan to the Engineer. Have the plan signed and sealed by a licensed professional engineer when the structure will continue to accommodate traffic after removal has begun and the removal impacts any part of the structure below the deck or riding surface. If applicable, the plan must detail requirements for meeting the U.S. Army Corps of Engineers'

Section 404 Permit. The demolition plan must detail handling of roadway and waterway traffic. Waterway traffic must be maintained at all times unless a closure is approved by the Engineer.

No debris is allowed to fall into a body of water. Debris that falls into the water must be removed at the end of each work day. Debris that falls into the floodway must be removed at the end of each work week or prior to a rain event.

ITEM 502 - BARRICADES, SIGNS, AND TRAFFIC HANDLING

Table 1

| Roadway | Limits | Allowable Closure Time |
|----------|--|------------------------|
| IH 35 | All (1 lane closed) | 9 P to 5 A |
| IH 35 | All (2 lanes closed, see allowable work below) | 9 P to 5 A |
| IH 35 | All (2 lanes closed, all work) | 11 P to 5 A |
| SH 45 | US 183 to SH130 | 8 P to 5 A |
| LP 1 | William Cannon to Parmer Lane | 8 P to 5 A |
| US 183 | SH 29 to FM 1327 | 8 P to 5 A |
| SH 71 | SH 130 to IH 35 | 8 P to 5 A |
| SH 71 | SH 304 to Tahitian Drive | 8 P to 5 A |
| SH 71 | US 290 W to RM 3238 | 8 P to 5 A |
| US 290 W | IH 35 to Nutty Brown Rd | 8 P to 5 A |
| US 290 E | IH 35 to SH 95 | 8 P to 5 A |
| FM 734 | FM 1431 to US 290 E | 8 P to 5 A |
| US 79 | IH 35 to Bus 79 in Taylor | 8 P to 5 A |
| RM 1431 | Lohmans Ford Rd to IH 35 | 8 P to 5 A |
| SH 29 | LP 332 western terminus to SH 130 | 8 P to 5 A |
| SH 80 | Charles Austin to River Road | 8 P to 5 A |
| RM 2222 | All | 8 P to 5 A |
| RM 620 | All | 8 P to 5 A |
| RM 2244 | All | 8 P to 5 A |
| SPUR 69 | All | 8 P to 5 A |
| LP 360 | All | 8 P to 5 A |
| LP 343 | All | 8 P to 5 A |
| LP 275 | All | 8 P to 5 A |
| FM 1325 | All | 8 P to 5 A |
| All | Within 200' of a signalized intersection | 9 P to 5 A |
| All | All (Full Closure, see allowable work below) | 11 P to 4 A |

Table 2

| Roadway | Limits | Allowable Closure Time |
|---------|--|------------------------|
| CR 118 | CR 118 at Cottonwood Creek to ., ETC?? | 8 P to 6 A |

Table 3 (Mobile Operations)

| Roadway | Allowable Sun Night thru Fri Noon | Allowable Sat thru Sun Morn |
|----------------------------|-----------------------------------|-----------------------------|
| Within Austin City Limits | 10 A to 2 P and 7 P to 6 A | 7 P to 10 A |
| Outside Austin City Limits | 9 A to 3 P and 7 P to 7 A | 6 P to 11 A |

| | | |
|------------------|-------------|-------------|
| IH 35 main lanes | 10 P to 5 A | 9 P to 9 A |
| AADT over 50,000 | 8 P to 6 A | 8 P to 10 A |

For roadways without defined allowable closure times, nighttime lane closures will be allowed from 8 P to 6 A. Unless stated, daytime or Friday night lane closures will not be allowed and one lane in each direction will remain open at all times for all roadways.

Full closures only allowed Friday night thru Monday morning for bridge beam installation, bridge demolition, or OSB truss removal/installation. Full closures only allowed for roadways with frontage roads or if a designated detour route is provided in the plans.

No closures will be allowed on the weekends, working day prior, and working day after the National Holidays defined in the Standard Specifications, Good Friday, and Easter weekend. Closures the Sunday of the Super Bowl will not be allowed from 1 P to 11 P. No closures will be allowed on Friday and the weekends for projects within 20 miles of Formula 1 at COTA, ACL Fest, SXSW, ROT Rally, UT home football games (includes games not on a Friday or weekend), sales tax holiday, Dell Match Play (includes Thursday), Rodeo Austin, or other special events that could be impacted by the construction. All lanes will be open by noon of the day before these special events.

To account for directional traffic volumes, begin and end times of closures may be shifted equally by the Engineer. The closure duration will remain. Added compensation is not allowed.

Submit an emailed request for a lane closure (LCN) to TxDOT. The email will be submitted in the format provided. Receive concurrence prior to implementation. Submit a cancellation of lane closures a minimum of 18 hours prior to implementation. Blanket requests for extended periods are not allowed. Max duration of a request is 2 weeks prior to requiring resubmittal.

Provide 2 hour notice prior to implementation and immediately upon removal of the closure.

For roadways listed in Table 1: Submit the request 96 hours prior to implementation.

For roadways not listed in Table 1: Submit the request a minimum of 48 hours prior to the closure and by the following deadline immediately prior to the closure: 11A on Tuesday or 11A on Friday. For all roadways: Submit request for traffic detours and full roadway closures 168 hours prior to implementation. Submit request for nighttime work 96 hours to implementation date.

Cancellations of accepted closures (not applicable to full closures or detours) due to weather will not require resubmission in accordance with the above restrictions if the work is completed during the next allowable closure time.

Closures that conflict with adjacent contractor will be prioritized according to critical path work per latest schedule. Conflicting critical path or non-critical work will be approved for first LCN submitted. Denial of a closure due to prioritization or other reasons will not be reason for time suspension, delay, overhead, etc.

Meet with the Engineer prior to lane closures to ensure that sufficient equipment, materials, devices, and workers will be used. Take immediate action to modify current and future traffic control, if at any time the queue becomes greater than 20 minutes.

Consider inclement weather prior to implementing the lane closures. Do not set up traffic control when the pavement is wet.

Cover, relocate, or remove existing small, large, and overhead signs that conflict with traffic control. Cover large and overhead signs to remain using latest standard TS-CD. This work is subsidiary.

Install all permanent signs, delineation, and object markers required for the operation of the roadway before opening to traffic. Use of temporary mounts is allowed or may be required until the permanent mounts are installed or not impacted by construction. Maintain the temporary mounts. This work is subsidiary.

Place a 28-inch cone, meeting requirements of BC (10), on top of foundations that have protruding studs. This work is subsidiary.

Edge condition treatment types must be in accordance with the TxDOT standard. Installation and removal of a safety slope is subsidiary.

To determine a speed limit or an advisory speed limit, submit a request to TxDOT 60 business days prior to manufacture of the sign.

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

ITEM 504 - FIELD OFFICE AND LABORATORY

Projects with HMAC, furnish a Type D structure for the Engineer's exclusive use. The structure will include high speed internet service with WIFI signal, one desk, two chairs, and one file cabinet. Provide a minimum of three 120-volt circuits with 20-amp breakers and at most two grounded convenience outlets per circuit.

ITEM 506 - TEMPORARY EROSION, SEDIMENTATION, AND ENV CONTROLS

If SW3P plan sheets are not provided, place the control measures as directed.

Install, maintain, remove control measures in areas of the right of way utilized by the Contractor that are outside the limits of disturbance required for construction. Permanently stabilize the area. This work is subsidiary.

Erosion control measures must be initiated immediately in areas where construction activities have ceased and will not resume for a period exceeding 14 calendar days. Vertical track all exposed soil, stockpiles, and slopes. Re-track after each rain event or every 14 days, whichever occurs first. Sheep foot roller is allowed for vertical tracking. This work is subsidiary.

Unless a specific pay item is provided in the plans, the installation of the 6:1 or flatter for RFD side slopes in the safety zone will be subsidiary to pertinent bid items.

ITEMS 528, 529, 530, 531, & 536 – MISCELLANEOUS CONSTRUCTION

Reinforcement will be in accordance with Section 432.3.1 unless shown on the plans. Fiber reinforcement is not allowed. GFRP is allowed reinforcement for all applications. Class A and B Concrete are allowed to use Coarse Aggregate Grades 1-8.

Unless shown on the plans, all concrete will be 5 in. thick and have 2 in. sand, base, or RAP bedding. Furnish base meeting the requirement for any type or grade in accordance with Item 247. Compressive strengths for flexible base are waived. RAP must be 100% passing a 1 in. sieve. Bedding and flexible base must be placed using ordinary compaction.

Expansion joints will be placed every 40 ft. Expansion joints must be 1 in. wide asphalt board and flush with the surface. The bottom of the asphalt board will be at half the depth of the concrete. The reinforcement will be continuous thru the expansion joint.

If roots are encountered verify with the Engineer before accommodating or removing 2 in. diameter or larger roots. Root removal must be in accordance with Section 752.4.2. Roots may remain in the bedding or base. For improvements within 6 in. of a root, the concrete thickness may be reduced by 1 in. and the bedding increased by 1 in. to minimize impacts to the roots. Adjust bedding and surface profile to provide a 1 in. bedding cushion around the roots. The surface profile may be adjusted to the extent allowed by ADA. This work is subsidiary.

ITEM 530 – INTERSECTIONS, DRIVEWAYS, AND TURNOUTS

Notify property owners at least 48 hr. before beginning work on their driveway. Provide a list of each notification and contact before each closure. Only close driveways for reconstruction if duration and alternate access are approved. Install and maintain material across a work zone as temporary access. This work is subsidiary.

For ACP or SURF TREAT, the pavement structure will match the adjacent roadway unless detailed on the plans. HMA, including surface, may use a maximum allowable quantity of 40%

RAP and 5% RAS for private driveways, public driveways for 2-lane roadways or smaller, and turnouts. Blending of 2 or more sources is allowed.

For CONC, the pavement structure will be 6 in. thick and have 3 in. flexible base bedding unless detailed on the plans.

ITEMS 540, 542, & 544 - METAL BEAM GUARD FENCE AND GUARDRAIL END TREATMENTS

Furnish round timber posts for guard fence. Steel posts for low fill culvert applications is subsidiary including use of low fill culvert application due to other concrete structures such as inlets. Long span application at inlets may be used as an alternate to low fill culvert. Unless otherwise specified on the plans, use of low fill culvert or long span at inlets will be subsidiary to pertinent items. Stake the locations for approval before installation. Adjust the limits of the fence to meet field conditions. Install delineators before opening the road to traffic.

Retain all materials. Existing materials that are structurally sound and dent free may be reused. All reused material will be from this project and in compliance with current standards. Structurally sound rust spots with the largest dimension of 4 in. may be cleaned and repaired in accordance with Section 540.3.5. Punch or field drill holes in the metal rail element to accommodate post spacing. Additional holes for splice or connections are not allowed. Space the field holes in accordance with the latest standard but no closer than the minimum spacing shown on the current standard.

Remove, replace, and install mow strip block out material. Construct new block outs and backfill unused block outs with class B concrete. This work is subsidiary.

Repair of mow strip damage, not caused by contractor negligence, and installation of new mow strip will be paid with appropriate bid items. Backfill and shoulder up of area around fence and mow strip will be paid using embankment item.

ITEM 644 – SMALL ROADSIDE SIGN ASSEMBLIES

Triangular slip base that use set screws to secure the post will require 1 of the set screws to penetrate the post by drilling a hole in the post at the location of the screw. All set screws shall be treated with anti-seize compound.

ITEM 658 – DELINEATOR AND OBJECT MARKER ASSEMBLIES

Installation and maintenance of portable CTB reflectors will be subsidiary to the barrier.

Flexible posts YFLX and WFLX must be tubular in shape. The “flat” flexible posts are not allowed.

ITEM 666 - RETROREFLECTORIZED PAVEMENT MARKINGS

Notify the Engineer at least 24 hr. before beginning work.

Place longitudinal markings nightly for IH 35 main lanes or roadways with AADT greater than 100,000. Use of temporary flexible reflective roadway marker tabs is subsidiary and at the Contractor's option. Replace missing or damaged tabs nightly. If using tabs, place longitudinal markings weekly by 5 AM Friday for all weekday work and by 5 AM Monday for all weekend work. Failure to maintain tabs or place longitudinal markings by deadline will require nightly placement of longitudinal markings.

When the raised portion of a profile marking is placed as a separate operation from the pavement marking, the raised portion must be placed first then covered with TY I.

When using black shadow to cover existing stripe apply a non-retroreflective angular abrasive bead drop. The marking color shall be adjusted to resemble the pavement color. If Item 677 is not used prior to placement of black shadow, scrape the top of the marking with a blade or large piece of equipment unless surface is a seal coat. The scraping of the marking is subsidiary.

ITEM 677 - ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS

Dispose of removed materials and debris at locations off the right of way.

Elimination using a pavement marking will not be allowed in lieu of methods listed in specification.

Remove pavement markings on concrete surfaces by a blasting method. Flail milling will be allowed when total quantity of removal on concrete surfaces is less than 1000 ft.

Strip seal is only method allowed on seal coat surface unless project includes placement of a new surface. If total quantity of removal on a seal coat surface is less than 2000 ft., elimination using a pavement marking is allowed if a test section is approved by the Engineer. Test section shall demonstrate the thermo marking color matches the existing pavement color.

Remove pavement markings outside the limits of the new surface by a blasting method.

Use a TRAIL or a non-retroreflective paint to cover stripe remnants that remain after elimination. The test requirements for these materials are waived. The paint color shall be adjusted to resemble the existing pavement color. Installation and maintenance is subsidiary.

ITEM 752 – TREE AND BRUSH REMOVAL

Follow Item 752.4 Work Methods and Item 752 general notes when removing or working on or near trees and brush even if Item 752 is not included as a pay item.

Flailing equipment is not allowed. Burning brush is not allowed in urban areas or on ROW. Use hand methods or other means of removal if doing work by mechanical methods is impractical.

Prior to begin tree pruning, send email confirmation to the Engineer that training and demonstration of work methods has been provided to the employees. This work is subsidiary.

Shredded vegetation may be blended, at a rate not to exceed 15 percent by volume, with Item 160 if the maximum dimension is not greater than 2 in.

ITEM 3085 – UNDERSEAL COURSE

No emulsified asphalt material allowed under PFC or SMA, except for use with Item 316, on roadways with ADT greater than 100,000.

The minimum application rates are listed in Table UC. The target shear bond strengths are listed in Table UCS. The informational test cores shall be taken once a shift for first 5 lots of placement or a change to placement method of bonding course, bonding material, or hot mix material. The remaining informational test cores shall be taken once every 3 lots for surface mix. Informational tests are not required for non-surface mix beyond the first 5 lots unless there is a change to placement method of bonding course, bonding material, or hot mix material. Results from these informational tests will not be used for specification compliance.

Table UC

| Material | Minimum Application Rate (mat >1" gal. per square yard) | Minimum Application Rate (mat <= 1" gal. per square yard) |
|----------------------------------|--|--|
| TRAIL – Hot Asphalt | 0.15 | 0.10 |
| Spray Applied Underseal Membrane | 0.15 | 0.15 |
| Seal Coat – Tier II emulsion | 0.25 | 0.25 |
| Seal Coat – Tier II asphalt | 0.23 | 0.23 |

Table UCS

| Material | Minimum Shear Strength (psi) |
|---------------------------------|---------------------------------|
| SMA – Stone-Matrix Asphalt | 60.0 |
| PFC – Permeable Friction Course | 40.0 |
| All Other Materials | 40.0 |

ITEM 6001 – PORTABLE CHANGEABLE MESSAGE SIGN

Provide 3 PCMS. Provide a replacement within 12 hours. PCMS will be available for traffic control, event notices, roadway conditions, service announcements, etc.

Place PCMS 10 calendar days prior to begin work stating “Road Work Begin Soon, Contact 832-7000 For Info”.

Place PCMS at time of LCN request. Place the PCMS at the expected end of queue caused by the closure. When the closure is active, revise the message to reflect the actual condition during the closure, such as “RIGHT LN CLOSED XXX FT”.



Estimate & Quantity Sheet

CONTROLLING PROJECT ID 0914-05-204

DISTRICT Austin
HIGHWAY CR 118

COUNTY Williamson

| CONTROL SECTION JOB | | | | 0914-05-204 | | 0914-05-220 | | TOTAL EST. | TOTAL FINAL |
|---------------------|----------|---|------|-------------|-------|-------------|-------|------------|-------------|
| PROJECT ID | | | | A00129347 | | A00188371 | | | |
| COUNTY | | | | Williamson | | Williamson | | | |
| HIGHWAY | | | | CR 118 | | CR 118 | | | |
| ALT | BID CODE | DESCRIPTION | UNIT | EST. | FINAL | EST. | FINAL | | |
| | 100-6002 | PREPARING ROW | STA | | | 28.700 | | 28.700 | |
| | 104-6010 | REMOVING CONC (RIPRAP) | CY | | | 2.000 | | 2.000 | |
| | 105-6037 | REMOVING STAB BASE AND ASPH PAV(0"-16") | SY | | | 4,935.000 | | 4,935.000 | |
| | 106-6002 | OBLITERATING ABANDONED ROAD | SY | | | 1,989.000 | | 1,989.000 | |
| | 110-6001 | EXCAVATION (ROADWAY) | CY | | | 3,900.000 | | 3,900.000 | |
| | 132-6003 | EMBANKMENT (FINAL)(ORD COMP)(TY B) | CY | | | 4,602.000 | | 4,602.000 | |
| | 160-6003 | FURNISHING AND PLACING TOPSOIL (4") | SY | | | 14,713.000 | | 14,713.000 | |
| | 164-6035 | DRILL SEEDING (PERM) (RURAL) (CLAY) | SY | | | 14,713.000 | | 14,713.000 | |
| | 164-6071 | BROADCAST SEED (TEMP)(WARM OR COOL) | SY | | | 14,713.000 | | 14,713.000 | |
| | 168-6001 | VEGETATIVE WATERING | MG | | | 24.800 | | 24.800 | |
| | 169-6002 | SOIL RETENTION BLANKETS (CL 1) (TY B) | SY | | | 14,713.000 | | 14,713.000 | |
| | 247-6366 | FL BS (CMP IN PLC)(TY A GR 5)(FNAL POS) | CY | | | 4,785.000 | | 4,785.000 | |
| | 260-6009 | LIME TRT (EXST MATL)(10") | SY | | | 9,951.000 | | 9,951.000 | |
| | 260-6043 | LIME (HYD, COM OR QK)(SLURRY) | TON | | | 224.000 | | 224.000 | |
| | 310-6001 | PRIME COAT (MULTI OPTION) | GAL | | | 1,544.000 | | 1,544.000 | |
| | 400-6005 | CEM STABIL BKFL | CY | 141.000 | | | | 141.000 | |
| | 402-6001 | TRENCH EXCAVATION PROTECTION | LF | | | 32.000 | | 32.000 | |
| | 416-6004 | DRILL SHAFT (36 IN) | LF | 261.000 | | | | 261.000 | |
| | 420-6013 | CL C CONC (ABUT) | CY | 47.400 | | | | 47.400 | |
| | 420-6029 | CL C CONC (CAP) | CY | 32.000 | | | | 32.000 | |
| | 420-6037 | CL C CONC (COLUMN) | CY | 5.600 | | | | 5.600 | |
| | 422-6001 | REINF CONC SLAB | SF | 9,000.000 | | | | 9,000.000 | |
| | 422-6015 | APPROACH SLAB | CY | 67.000 | | | | 67.000 | |
| | 425-6037 | PRESTR CONC GIRDER (TX40) | LF | 1,236.700 | | | | 1,236.700 | |
| | 432-6031 | RIPRAP (STONE PROTECTION)(12 IN) | CY | 459.000 | | 339.000 | | 798.000 | |
| | 432-6045 | RIPRAP (MOW STRIP)(4 IN) | CY | | | 23.300 | | 23.300 | |
| | 450-6006 | RAIL (TY T223) | LF | 540.000 | | | | 540.000 | |
| | 454-6018 | SEALED EXPANSION JOINT (4 IN) (SEJ - M) | LF | 73.000 | | | | 73.000 | |
| | 464-6003 | RC PIPE (CL III)(18 IN) | LF | | | 192.000 | | 192.000 | |
| | 464-6005 | RC PIPE (CL III)(24 IN) | LF | | | 125.000 | | 125.000 | |
| | 467-6084 | SET (TY I)(S=12 FT)(HW=6FT)(4:1)(C) | EA | | | 1.000 | | 1.000 | |
| | 467-6086 | SET (TY I)(S=12 FT)(HW=7FT)(4:1)(C) | EA | | | 1.000 | | 1.000 | |
| | 467-6363 | SET (TY II) (18 IN) (RCP) (6: 1) (P) | EA | | | 10.000 | | 10.000 | |
| | 467-6395 | SET (TY II) (24 IN) (RCP) (6: 1) (P) | EA | | | 8.000 | | 8.000 | |
| | 496-6005 | REMOV STR (WINGWALL) | EA | | | 2.000 | | 2.000 | |
| | 496-6007 | REMOV STR (PIPE) | LF | | | 89.000 | | 89.000 | |
| | 496-6009 | REMOV STR (BRIDGE 0 - 99 FT LENGTH) | EA | 1.000 | | | | 1.000 | |



Estimate & Quantity Sheet

CONTROLLING PROJECT ID 0914-05-204

DISTRICT Austin
HIGHWAY CR 118

COUNTY Williamson

| CONTROL SECTION JOB | | | | 0914-05-204 | | 0914-05-220 | | TOTAL EST. | TOTAL FINAL |
|---------------------|-----------|--|------|-------------|-------|-------------|-------|------------|-------------|
| PROJECT ID | | | | A00129347 | | A00188371 | | | |
| COUNTY | | | | Williamson | | Williamson | | | |
| HIGHWAY | | | | CR 118 | | CR 118 | | | |
| ALT | BID CODE | DESCRIPTION | UNIT | EST. | FINAL | EST. | FINAL | | |
| | 500-6001 | MOBILIZATION | LS | 1.000 | | | | 1.000 | |
| | 502-6001 | BARRICADES, SIGNS AND TRAFFIC HANDLING | MO | | | 13.000 | | 13.000 | |
| | 506-6002 | ROCK FILTER DAMS (INSTALL) (TY 2) | LF | | | 226.000 | | 226.000 | |
| | 506-6003 | ROCK FILTER DAMS (INSTALL) (TY 3) | LF | | | 348.000 | | 348.000 | |
| | 506-6004 | ROCK FILTER DAMS (INSTALL) (TY 4) | LF | | | 320.000 | | 320.000 | |
| | 506-6011 | ROCK FILTER DAMS (REMOVE) | LF | | | 1,120.000 | | 1,120.000 | |
| | 506-6038 | TEMP SEDMT CONT FENCE (INSTALL) | LF | | | 445.000 | | 445.000 | |
| | 506-6039 | TEMP SEDMT CONT FENCE (REMOVE) | LF | | | 445.000 | | 445.000 | |
| | 506-6041 | BIODEG EROSN CONT LOGS (INSTL) (12") | LF | | | 100.000 | | 100.000 | |
| | 506-6043 | BIODEG EROSN CONT LOGS (REMOVE) | LF | | | 100.000 | | 100.000 | |
| | 506-6053 | ROCK FILTER DAMS (INSTALL) (TY 2) (6:1) | LF | | | 226.000 | | 226.000 | |
| | 530-6005 | DRIVEWAYS (ACP) | SY | | | 903.000 | | 903.000 | |
| | 530-6008 | TURNOUTS (ACP) | SY | | | 66.000 | | 66.000 | |
| | 540-6001 | MTL W-BEAM GD FEN (TIM POST) | LF | | | 175.000 | | 175.000 | |
| | 540-6006 | MTL BEAM GD FEN TRANS (THRIE-BEAM) | EA | | | 4.000 | | 4.000 | |
| | 544-6001 | GUARDRAIL END TREATMENT (INSTALL) | EA | | | 4.000 | | 4.000 | |
| | 560-6011 | MAILBOX INSTALL-S (TWW-POST) TY 4 | EA | | | 4.000 | | 4.000 | |
| | 644-6001 | IN SM RD SN SUP&AM TY10BWG(1)SA(P) | EA | | | 5.000 | | 5.000 | |
| | 644-6004 | IN SM RD SN SUP&AM TY10BWG(1)SA(T) | EA | | | 1.000 | | 1.000 | |
| | 644-6076 | REMOVE SM RD SN SUP&AM | EA | | | 14.000 | | 14.000 | |
| | 658-6014 | INSTL DEL ASSM (D-SW)SZ (BRF)CTB (BI) | EA | | | 8.000 | | 8.000 | |
| | 658-6047 | INSTL OM ASSM (OM-2Y)(WC)GND | EA | | | 4.000 | | 4.000 | |
| | 658-6060 | REMOVE DELIN & OBJECT MARKER ASSMS | EA | | | 6.000 | | 6.000 | |
| | 658-6062 | INSTL DEL ASSM (D-SW)SZ 1(BRF)GF2(BI) | EA | | | 13.000 | | 13.000 | |
| | 666-6048 | REFL PAV MRK TY I (W)24"(SLD)(100MIL) | LF | | | 26.000 | | 26.000 | |
| | 666-6342 | REF PROF PAV MRK TY I(W)4"(SLD)(100MIL) | LF | | | 5,660.000 | | 5,660.000 | |
| | 666-6345 | REF PROF PAV MRK TY I(Y)4"(SLD)(100MIL) | LF | | | 5,478.000 | | 5,478.000 | |
| | 672-6009 | REFL PAV MRKR TY II-A-A | EA | | | 67.000 | | 67.000 | |
| | 3076-6070 | D-GR HMA TY-D PG 76-22 SAC-B (EXEMPT) | TON | | | 925.000 | | 925.000 | |
| | 6001-6002 | PORTABLE CHANGEABLE MESSAGE SIGN | EA | | | 3.000 | | 3.000 | |
| | 6120-6001 | DEAD END ROADWAY BARRICADE | LF | | | 20.000 | | 20.000 | |
| 08 | | CONTRACTOR FORCE ACCOUNT SAFETY CONTINGENCY (NON-PARTICIPATING) | LS | | | 1.000 | | 1.000 | |
| | | CONTRACTOR FORCE ACCOUNT LAW ENFORCEMENT (NON-PARTICIPATING) | LS | | | 1.000 | | 1.000 | |
| | | CONTRACTOR FORCE ACCOUNT EROSION CONTROL MAINTENANCE (NON-PARTICIPATING) | LS | | | 1.000 | | 1.000 | |

FILENAME: pw:\kh-pw-bentley.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\1. General\CR118_GEN_SUM.dgn
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| SUMMARY OF EROSION CONTROL ITEMS | | | | | | | | | | | | | | | | | |
|----------------------------------|------------------|--|---|--|------------------------|---|--|--|--|--|---------------------------------|---|--|---|--|---|----------------------------------|
| SHEET NO. | STATION | 0160 6003 | 0164 6035 | 0164 6071 | 0168 6001 | 0169 6002 | 0432 6031 | 0506 6002 | 0506 6003 | 0506 6004 | 0506 6011 | 0506 6038 | 0506 6039 | 0506 6041 | 0506 6043 | 0506 6053 | 6120 6001 |
| | | FURNISHING AND PLACING TOPSOIL (4") | DRILL SEEDING (PERM) (RURAL) (CLAY) | BROADCAST SEED (TEMP) (WARM OR COOL) | VEGETATIVE WATERING | SOIL RETENTION BLANKETS (CL 1) (TY B) | RIPRAP (STONE PROTECTION) (12 IN) | ROCK FILTER DAMS (INSTALL) (TY 2) | ROCK FILTER DAMS (INSTALL) (TY 3) | ROCK FILTER DAMS (INSTALL) (TY 4) | ROCK FILTER DAMS (REMOVE) | TEMP SDMT CONT FENCE (INSTALL) | TEMP SDMT CONT FENCE (REMOVE) | BIODEG EROSN CONT LOGS (INSTL) (12") | BIODEG EROSN CONT LOGS (REMOVE) | ROCK FILTER DAMS (INSTL) (TY 2) (6:1) | DEAD END ROADWAY BARRICADE |
| CR 118 | | SY | SY | SY | MG | SY | CY | LF | LF | LF | LF | LF | LF | LF | LF | LF | LF |
| 1 OF 3 | BEGIN TO 112+00 | 6421 | 6421 | 6421 | 10.8 | 6421 | | 30 | | 160 | 220 | 361 | 361 | | | 30 | |
| 2 OF 3 | 112+00 TO 124+00 | 5714 | 5714 | 5714 | 9.6 | 5714 | | 127 | 250 | 80 | 584 | 84 | 84 | | | 127 | |
| 3 OF 3 | 124+00 TO END | 2578 | 2578 | 2578 | 4.4 | 2578 | 294 | 69 | 98 | 80 | 316 | | | | | 69 | 20 |
| TOTAL | | 14713 | 14713 | 14713 | 24.8 | 14713 | 294 | 226 | 348 | 320 | 1120 | 445 | 445 | 100 | 100 | 226 | 20 |

| SUMMARY OF ROADWAY ITEMS | | | | | | | | | | | | | | | |
|--------------------------|------------------|------------------|--|----------------------------------|---|---------------------------------|---------------------------------|--------------------|-------------------|---------------------------------------|--|--|--|--|--|
| SHEET NO. | STATION | 0100 6002 | 0247 6366 | 0260 6009 | 0260 6043 | 0310 6001 | 0432 6045 | 0530 6005 | 0530 6008 | 0540 6001 | 0540 6006 | 0544 6001 | 0560 6011 | 3076 6070 | |
| | | PREPARING ROW | FL BS (CMP IN PLC) (TY A GR 5) (FNAL POS) | LIME TRT (EXST MATL) (10") | LIME (HYD, COM OR QK) (SLURRY) | PRIME COAT (MULTI OPTION) | RIPRAP (MOW STRIP) (4 IN) | DRIVEWAYS (ACP) | TURNOUTS (ACP) | MTL W-BEAM GD FEN (TIM POST) | MTL BEAM GD FEN TRANS (THRIE-BEAM) | GUARDRAIL END TREATMENT (INSTALL) | MAILBOX INSTALL-S (TWW-POST) TY 4 | D-GR HMA TY-D SAC-B PG76-22 (EXEMPT) | |
| CR 118 | | STA | CY | SY | TON | GAL | CY | SY | SY | LF | EA | EA | EA | TON | |
| 1 OF 5 | BEGIN TO 106+00 | 4.81 | 917 | 1945 | 43.8 | 300 | | | | | | | | 165 | |
| 2 OF 5 | 106+00 TO 112+00 | 6.00 | 1141 | 2423 | 54.5 | 374 | | 590 | | | | | | 206 | |
| 3 OF 5 | 112+00 TO 118+00 | 6.00 | 624 | 1300 | 29.3 | 205 | 11.6 | | | 50 | 4 | 2 | | 113 | |
| 4 OF 5 | 118+00 TO 124+00 | 7.44 | 1268 | 2510 | 56.5 | 391 | 11.7 | 106 | 22 | 125 | | 2 | 1 | 290 | |
| 5 OF 5 | 124+00 TO END | 4.39 | 835 | 1773 | 39.9 | 274 | | 207 | 44 | | | | 3 | 151 | |
| TOTAL | | 28.7 | 4785 | 9951 | 224 | 1544 | 23.3 | 903 | 66 | 175 | 4 | 4 | 4 | 925 | |


NOTES:
 1. ALL QUANTITIES ON THIS SHEET PERTAIN TO CSJ 0914-05-220.

| SUMMARY OF PARALLEL DRAINAGE QUANTITIES | | | | | | | | | |
|---|------------------|------------------------------------|--|--------------------------------|--------------------------------|---|---|--|--|
| SHEET NO. | STATION | 0402 6001 | 0432 6031 | 0464 6003 | 0464 6005 | 0467 6084 | 0467 6086 | 0467 6363 | 0467 6395 |
| | | TRENCH EXCAVATION PROTECTION | RIPRAP (STONE PROTECTION) (12 IN) | RC PIPE (CL III) (18 IN) | RC PIPE (CL III) (24 IN) | SET (TY I) (S=12 FT) (HW=6 FT) (4:1) (C) | SET (TY I) (S=12 FT) (HW=7 FT) (4:1) (C) | SET (TY II) (18 IN) (RCP) (6:1) (P) | SET (TY II) (24 IN) (RCP) (6:1) (P) |
| CR 118 | | LF | CY | LF | LF | EA | EA | EA | EA |
| 1 OF 5 | BEGIN TO 106+00 | | | | | | | | |
| 2 OF 5 | 106+00 TO 112+00 | | | 108 | 80 | | | 6 | 4 |
| 3 OF 5 | 112+00 TO 118+00 | | | | | | | | |
| 4 OF 5 | 118+00 TO 124+00 | 32 | 45 | 84 | | 1 | 1 | 4 | |
| 5 OF 5 | 124+00 TO END | | | | 45 | | | | 4 |
| TOTAL | | 32 | 45 | 192 | 125 | 1 | 1 | 10 | 8 |

| SUMMARY OF SIGNING AND PAVEMENT MARKING ITEMS | | | | | | | | | | |
|---|------------------|--|--|---|--|--|---|--|--|-------------------------------|
| SHEET NO. | STATION | 0644 6001 | 0644 6004 | 0658 6014 | 0658 6047 | 0658 6062 | 0666 6048 | 0666 6342 | 0666 6345 | 0672 6009 |
| | | IN SM RD SN SUP&AM TY10BWG (1) SA (P) | IN SM RD SN SUP&AM TY10BWG (1) SA (T) | IN STL DEL ASSM (D-SW) SZ (BRF) CTB (BI) | IN STL OM ASSM (OM-2Y) (WC) GND | IN STL DEL ASSM (D-SW) SZ1 (BRF) GF2 (BI) | REFL PAV MRK TY I (W) 24" (SLD) (100MIL) | REFL PROF PAV MRK TY I (W) 4" (SLD) (100 MIL) | REFL PROF PAV MRK TY I (Y) 4" (SLD) (100 MIL) | REFL PAV MRKR TY II-A-A |
| CR 118 | | EA | EA | EA | EA | EA | LF | LF | LF | EA |
| 1 OF 3 | BEGIN TO 112+00 | 1 | | | | | | 2162 | 2162 | 27 |
| 2 OF 3 | 112+00 TO 124+00 | 4 | 1 | 8 | 4 | 13 | 26 | 2620 | 2438 | 30 |
| 3 OF 3 | 124+00 TO END | | | | | | | 878 | 878 | 10 |
| TOTAL | | 5 | 1 | 8 | 4 | 13 | 26 | 5660 | 5478 | 67 |



F-928



CR 118 AT COTTONWOOD CREEK

QUANTITY SUMMARY

SHEET 1 OF 2

| | | | |
|------|------|------------|-----------|
| CONT | SECT | JOB | HIGHWAY |
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 8 |

FILENAME: pw:\kh-pw-bentley.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\1. General\CR118_GEN_SUM02.dgn
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| SUMMARY OF REMOVAL ITEMS | | | | | | | | | |
|--------------------------|-----------------------|------------------------|--|-----------------------------|----------------------|------------------|-----------------------------------|------------------------|------------------------------------|
| SHEET NO. | STATION | 0104 | 0105 | 0106 | 0496 | 0496 | 0496 | 0644 | 0658 |
| | | 6010 | 6037 | 6002 | 6005 | 6007 | 6009 | 6076 | 6060 |
| | | REMOVING CONC (RIPRAP) | REMOVING STAB BASE AND ASPH PAV (0"-16") | OBLITERATING ABANDONED ROAD | REMOV STR (WINGWALL) | REMOV STR (PIPE) | REMOV STR (BRIDGE 0-99 FT LENGTH) | REMOVE SM RD SN SUP&AM | REMOVE DELIN & OBJECT MARKER ASSMS |
| CR 118 | | CY | SY | SY | EA | LF | EA | EA | EA |
| 1 | OF 3 BEGIN TO 411+00 | | 2316 | 1384 | | 35 | | 1 | |
| 2 | OF 3 411+00 TO 122+00 | 2 | 1057 | 605 | | | 1 | 12 | 4 |
| 3 | OF 3 122+00 TO END | | 1562 | | 2 | 54 | | 1 | 2 |
| TOTAL | | 2 | 4935 | 1989 | 2 | 89 | 1 | 14 | 6 |



| SUMMARY OF EARTHWORK ITEMS | | |
|---------------------------------|----------------------|--------------------------------------|
| STATION TO STATION CL CR 118 | 0110 6001 | 0132 6003 |
| | EXCAVATION (ROADWAY) | EMBANKMENT (FINAL) (ORD COMP) (TY B) |
| | CY | CY |
| 101+19.00 TO 102+00.00 | 0.0 | 12.1 |
| 102+00.00 TO 103+00.00 | 29.5 | 48.8 |
| 103+00.00 TO 104+00.00 | 51.7 | 34.6 |
| 104+00.00 TO 105+00.00 | 79.3 | 26.3 |
| 105+00.00 TO 106+00.00 | 208.8 | 27.0 |
| 106+00.00 TO 107+00.00 | 290.3 | 3.2 |
| 107+00.00 TO 108+00.00 | 338.0 | 1.9 |
| 108+00.00 TO 109+00.00 | 329.6 | 22.2 |
| 109+00.00 TO 110+00.00 | 346.1 | 22.2 |
| 110+00.00 TO 111+00.00 | 369.0 | 0.0 |
| 111+00.00 TO 112+00.00 | 196.9 | 7.6 |
| 112+00.00 TO 113+00.00 | 68.0 | 95.7 |
| 113+00.00 TO 114+00.00 | 39.5 | 440.5 |
| 114+00.00 TO 115+00.00 | 40.8 | 934.9 |
| 115+00.00 TO 116+00.00 | 25.5 | 582.6 |
| 116+00.00 TO 117+00.00 | BRIDGE | |
| 117+00.00 TO 118+00.00 | 107.8 | 720.8 |
| 118+00.00 TO 118+25.00 | 58.4 | 310.5 |
| 118+25.00 TO 118+50.00 | 75.8 | 223.1 |
| 118+50.00 TO 118+75.00 | 86.0 | 177.6 |
| 118+75.00 TO 119+00.00 | 72.7 | 136.7 |
| 119+00.00 TO 119+25.00 | 56.7 | 73.8 |
| 119+25.00 TO 119+50.00 | 49.0 | 38.3 |
| 119+50.00 TO 120+00.00 | 84.2 | 73.0 |
| 120+00.00 TO 121+00.00 | 197.7 | 112.1 |
| 121+00.00 TO 122+00.00 | 153.5 | 79.4 |
| 122+00.00 TO 123+00.00 | 57.1 | 95.5 |
| 123+00.00 TO 124+00.00 | 31.8 | 65.7 |
| 124+00.00 TO 125+00.00 | 24.5 | 41.7 |
| 125+00.00 TO 126+00.00 | 18.8 | 26.9 |
| 126+00.00 TO 127+00.00 | 60.9 | 42.6 |
| 127+00.00 TO 128+00.00 | 92.7 | 77.8 |
| 128+00.00 TO 128+39.00 | 28.1 | 26.5 |
| TOTAL | 3669 | 4582 |

| SUMMARY OF EARTHWORK ITEMS | | |
|---------------------------------|----------------------|--------------------------------------|
| STATION TO STATION CL CR 100 | 0110 6001 | 0132 6003 |
| | EXCAVATION (ROADWAY) | EMBANKMENT (FINAL) (ORD COMP) (TY B) |
| | CY | CY |
| 200+14.00 TO 200+50.00 | 87.2 | 14.1 |
| 200+50.00 TO 201+00.00 | 100.3 | 0.0 |
| 201+00.00 TO 201+50.00 | 41.6 | 0.5 |
| 201+50.00 TO 201+58.00 | 0.0 | 3.7 |
| TOTAL | 231 | 20 |

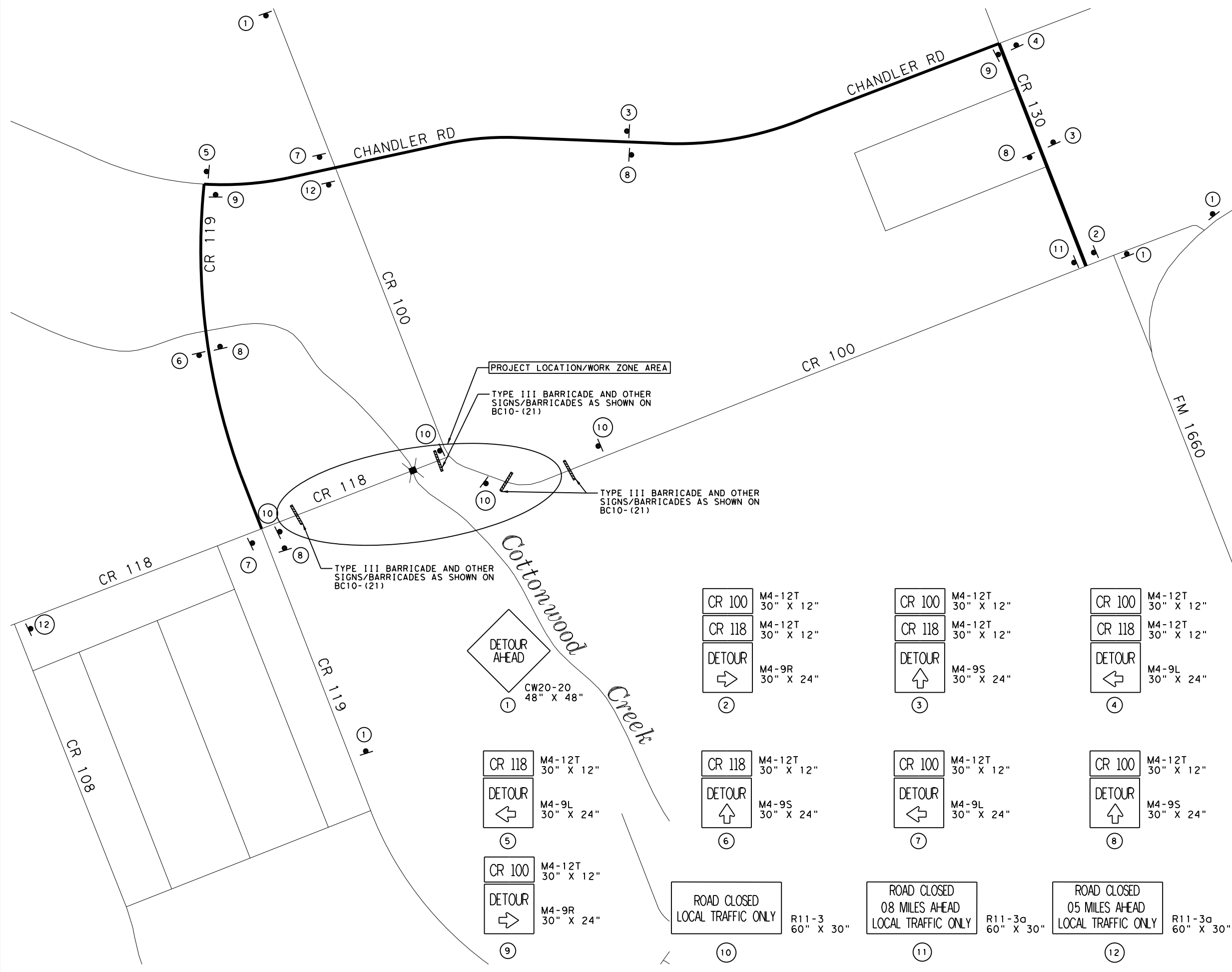
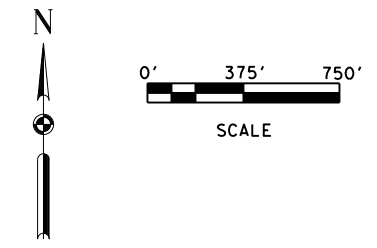
| SUMMARY OF WORKZONE TRAFFIC CONTROL ITEMS | | |
|---|--|----------------------------------|
| DESCRIPTION | 0502 | 6001 |
| | 6001 | 6002 |
| | BARRICADES, SIGNS AND TRAFFIC HANDLING | PORTABLE CHANGEABLE MESSAGE SIGN |
| | MO | EA |
| CR 118 | 13 | 3 |
| TOTAL | 13 | 3 |

NOTES:

1. ALL QUANTITIES ON THIS SHEET PERTAIN TO CSJ 0914-05-220.

| | | | | |
|---|------|------|------------|-----------|
|  | | | | |
|  | | | | |
| CR 118 AT COTTONWOOD CREEK QUANTITY SUMMARY | | | | |
| SHEET 2 OF 2 | | | | |
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | | COUNTY | SHEET NO. |
| | AUS | | WILLIAMSON | 9 |

FILENAME: \\kn-pw-bentley.com\kn-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\2. Traffic Control Plan\CR118_TCP_DET.dgn
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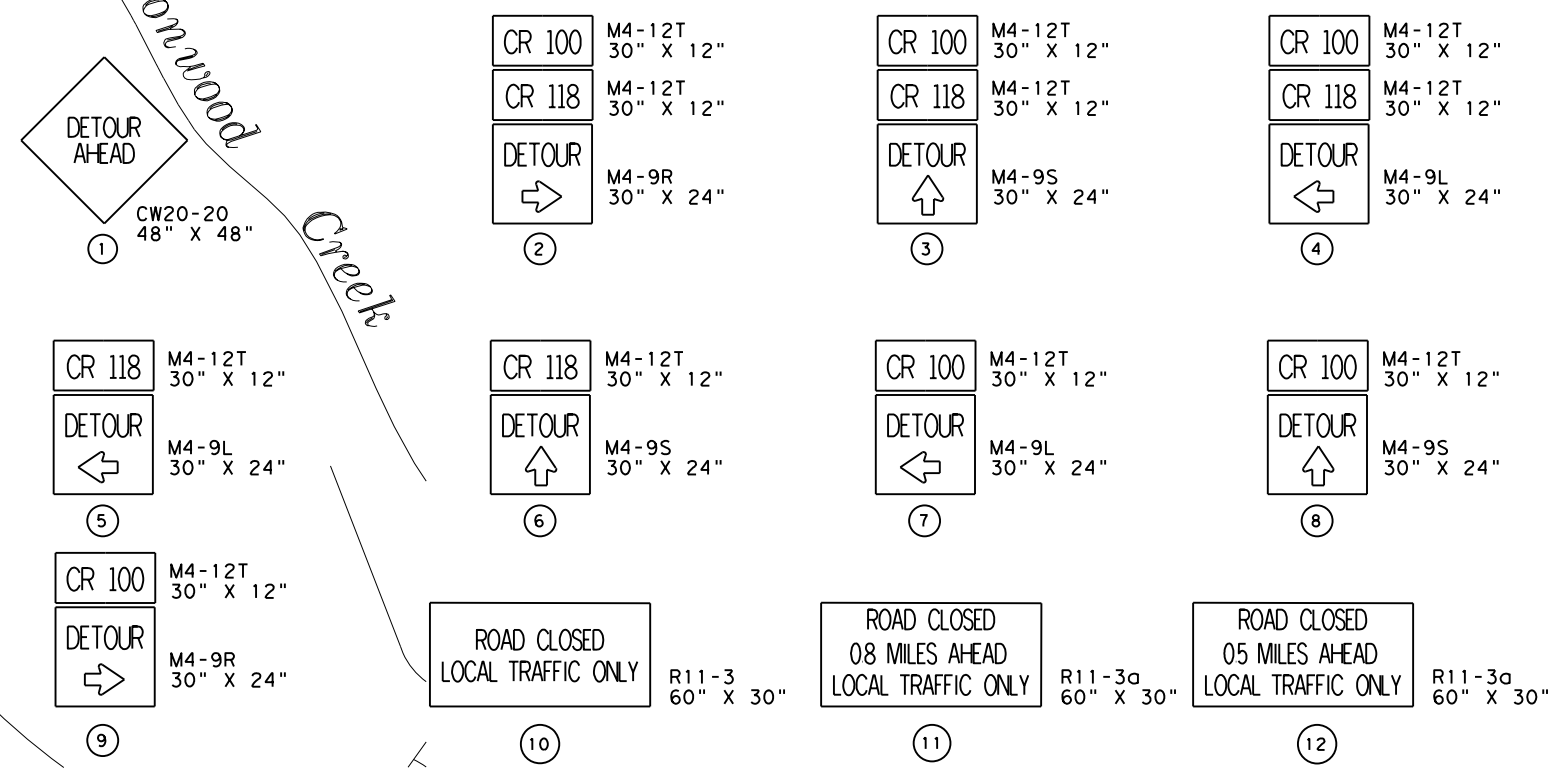
TCP NARRATIVE

- PHASE 1**
1. PLACE WORK ZONE APPROACH SIGNAGE IN ACCORDANCE WITH BC STANDARD SHEETS.
 2. PLACE EROSION CONTROL MEASURES AS NOTED ON SW3P SHEETS.
 3. CONSTRUCT PROPOSED COUNTY ROAD 118 BRIDGE OVER COTTONWOOD CREEK AS SHOWN IN PLANS.
 4. CONSTRUCT PERMANENT PAVEMENT FROM @ CR 118 STA 107+50 TO @ CR 118 STA 120+00.
- PHASE 2**
1. PLACE DETOUR SIGNAGE AND BARRICADES AS NOTED ON DETOUR LAYOUT.
 2. PLACE EROSION CONTROL MEASURES AS NOTED ON SW3P SHEETS.
 3. CONSTRUCT PROPOSED COUNTY ROAD 118 CONNECTION TO EXISTING COUNTY ROAD 118 AS SHOWN IN THE PLANS.
- PHASE 3**
1. SAFETY END TREAT EXISTING CULVERT AT @ CR 118 STA 122+66 AS SHOWN ON CULVERT LAYOUT.
 2. CONSTRUCT PROPOSED COUNTY ROAD 118 CONNECTION TO COUNTY ROAD 100 AS SHOWN IN THE PLANS.
 3. REMOVE EXISTING COUNTY ROAD 118 BRIDGE.
 4. OBLITERATE EXISTING ROAD 118 AND COUNTY ROAD 100 AS SHOWN IN THE PLANS.
 5. REOPEN ROAD UPON COMPLETION OF CONSTRUCTION.
- SIGNS MAY BE ADJUSTED TO FIT EXISTING DRIVEWAYS WITH PERMISSION OF ENGINEER.
- ACCESS TO ADJOINING DRIVEWAYS MUST BE MAINTAINED AT ALL TIMES.

GENERAL NOTES

THE CONTRACTOR SHALL NOTIFY THE PROPER CITY, COUNTY, EMERGENCY MEDICAL SERVICES, FIRE DEPARTMENT, POLICE DEPARTMENT, TEXAS DEPARTMENT OF PUBLIC SAFETY AND THE ENGINEER WHEN MAJOR TRAFFIC CHANGES ARE TO BE PERFORMED. THE NOTIFICATION MUST BE PROVIDED AT LEAST FOURTEEN (14) DAYS PRIOR TO THE CHANGE.

TJN
 11/3/2022



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CR 118 AT COTTONWOOD CREEK
TRAFFIC CONTROL PLAN
DETOUR LAYOUT

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
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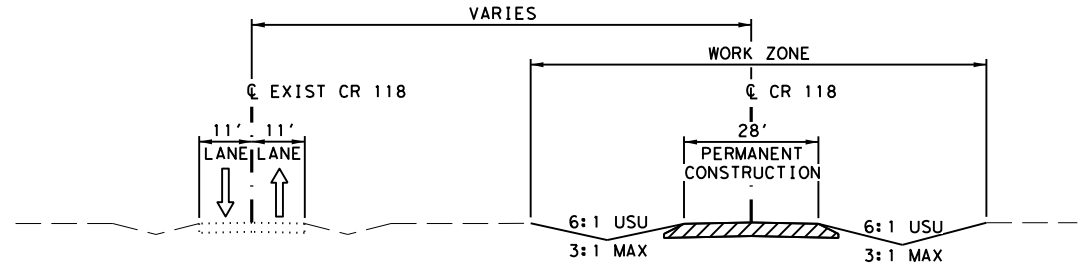
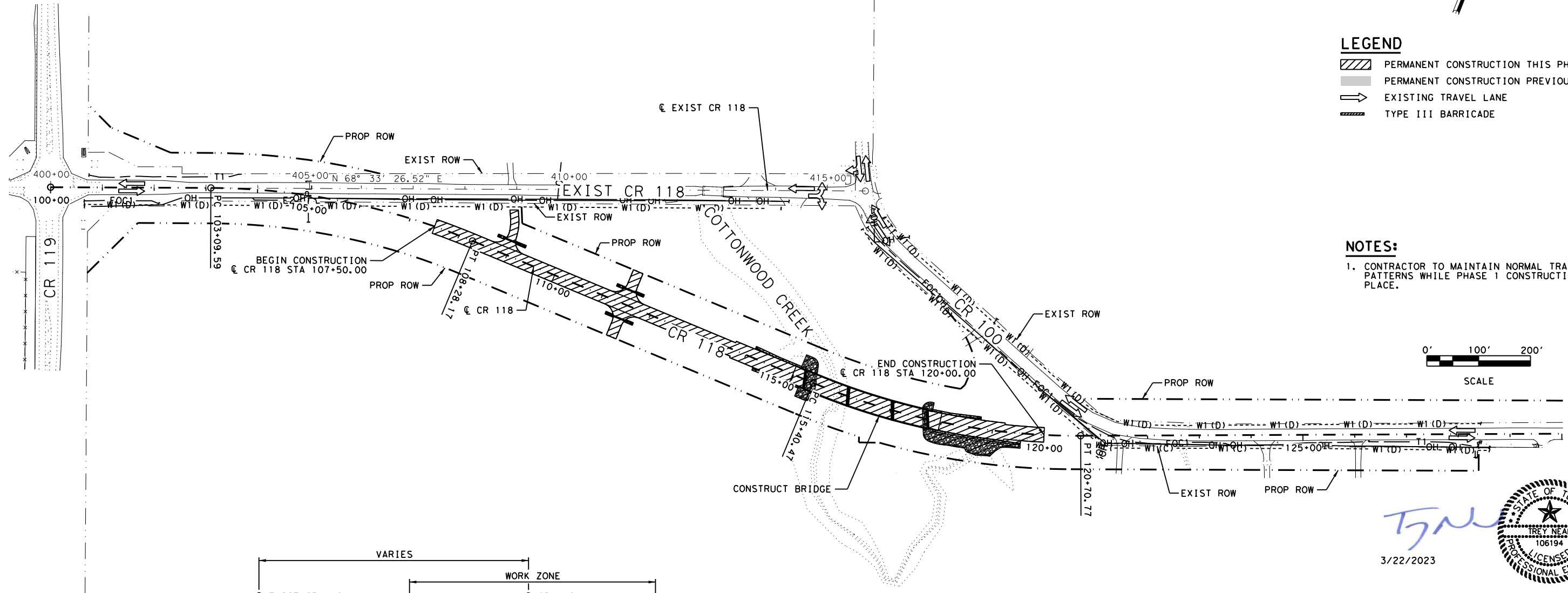
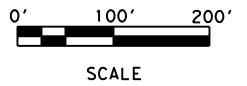


LEGEND

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- PERMANENT CONSTRUCTION PREVIOUS PHASE
- EXISTING TRAVEL LANE
- TYPE III BARRICADE

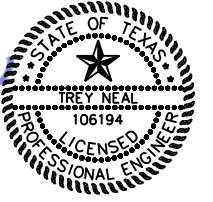
NOTES:

1. CONTRACTOR TO MAINTAIN NORMAL TRAFFIC PATTERNS WHILE PHASE 1 CONSTRUCTION TAKES PLACE.



PHASE 1 TYPICAL SECTION

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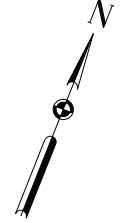
 Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
TRAFFIC CONTROL PLAN
PHASE 1





SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
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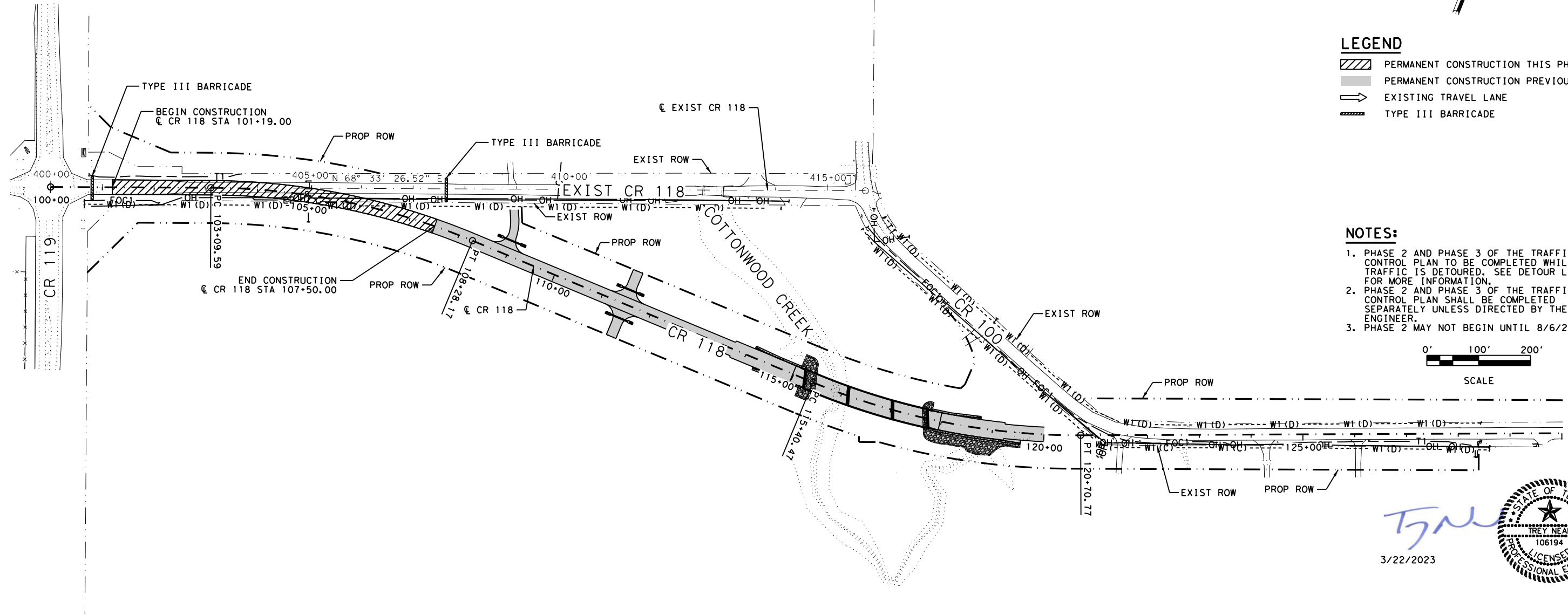
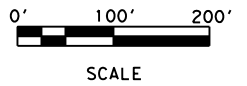


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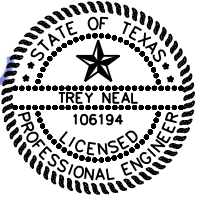
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-  PERMANENT CONSTRUCTION PREVIOUS PHASE
-  EXISTING TRAVEL LANE
-  TYPE III BARRICADE

NOTES:

1. PHASE 2 AND PHASE 3 OF THE TRAFFIC CONTROL PLAN TO BE COMPLETED WHILE TRAFFIC IS DETOURED. SEE DETOUR LAYOUT FOR MORE INFORMATION.
2. PHASE 2 AND PHASE 3 OF THE TRAFFIC CONTROL PLAN SHALL BE COMPLETED SEPARATELY UNLESS DIRECTED BY THE ENGINEER.
3. PHASE 2 MAY NOT BEGIN UNTIL 8/6/2024.



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

CR 118 AT COTTONWOOD CREEK
TRAFFIC CONTROL PLAN
PHASE 2

SHEET 1 OF 1

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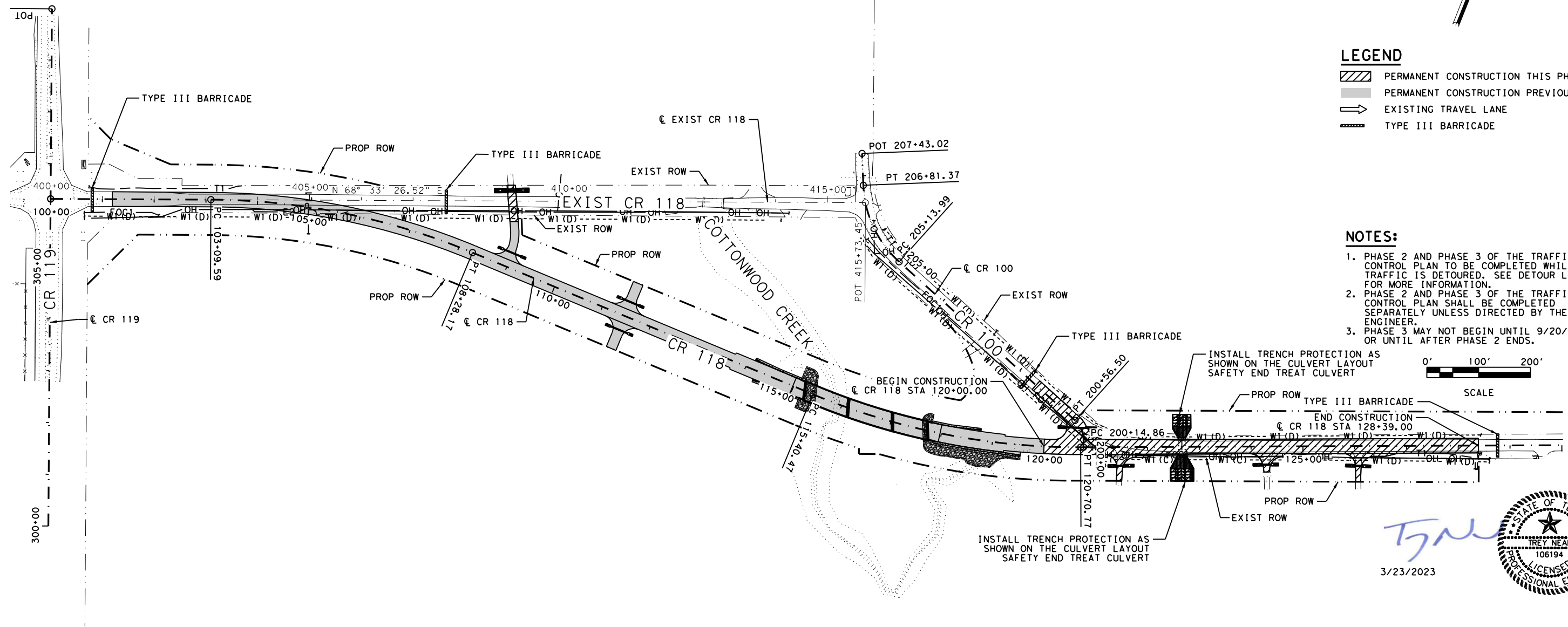
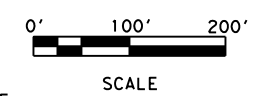


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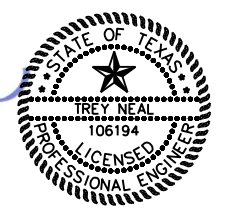
-  PERMANENT CONSTRUCTION THIS PHASE
-  PERMANENT CONSTRUCTION PREVIOUS PHASE
-  EXISTING TRAVEL LANE
-  TYPE III BARRICADE

NOTES:

1. PHASE 2 AND PHASE 3 OF THE TRAFFIC CONTROL PLAN TO BE COMPLETED WHILE TRAFFIC IS DETOURED. SEE DETOUR LAYOUT FOR MORE INFORMATION.
2. PHASE 2 AND PHASE 3 OF THE TRAFFIC CONTROL PLAN SHALL BE COMPLETED SEPARATELY UNLESS DIRECTED BY THE ENGINEER.
3. PHASE 3 MAY NOT BEGIN UNTIL 9/20/2024 OR UNTIL AFTER PHASE 2 ENDS.



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3/23/2023



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CR 118 AT COTTONWOOD CREEK
TRAFFIC CONTROL PLAN
PHASE 3

SHEET 1 OF 1

| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
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BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
2. The development and design of the Traffic Control Plan (TCP) is the responsibility of the Engineer.
3. The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop, sign and seal Contractor proposed changes.
4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
6. When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
7. The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
9. The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES, CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
12. The Engineer has the final decision on the location of all traffic control devices.
13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:



1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel," or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

1. Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources.
2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

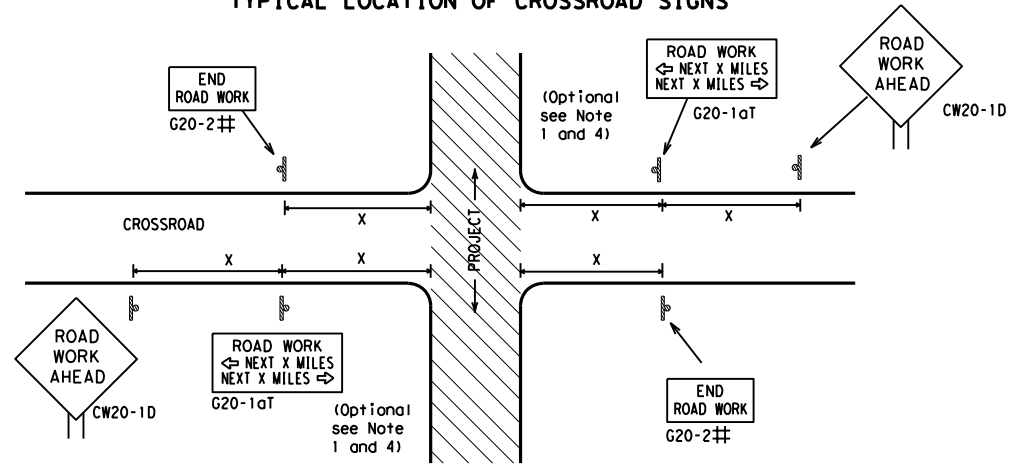
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| <p>THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov</p> |
| COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD) |
| DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS) |
| MATERIAL PRODUCER LIST (MPL) |
| ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)" |
| STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD) |
| TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD) |
| TRAFFIC ENGINEERING STANDARD SHEETS |

SHEET 1 OF 12

| | | | |
|--|---------------|--|------------|
|  Texas Department of Transportation | |  Traffic Safety Division Standard | |
| <p>BARRICADE AND CONSTRUCTION GENERAL NOTES AND REQUIREMENTS</p> <p>BC (1) -21</p> | | | |
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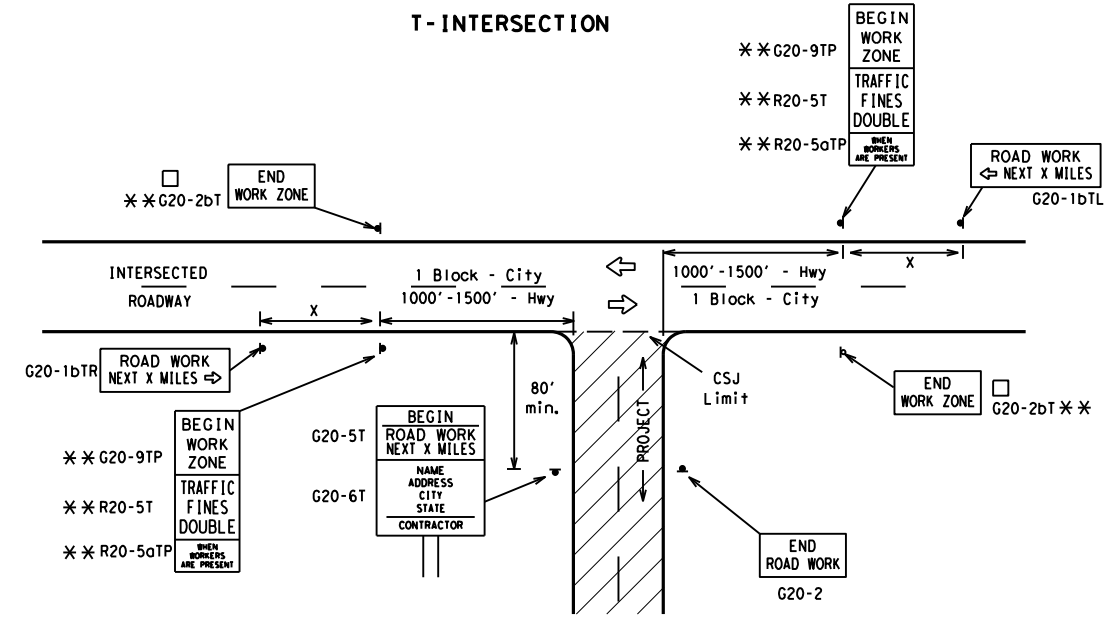
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TYPICAL LOCATION OF CROSSROAD SIGNS



- ## May be mounted on back of "ROAD WORK AHEAD" (CW20-1D) sign with approval of Engineer. (See note 2 below)
- The typical minimum signing on a crossroad approach should be a "ROAD WORK AHEAD" (CW20-1D) sign and a (G20-2) "END ROAD WORK" sign, unless noted otherwise in plans.
 - The Engineer may use the reduced size 36" x 36" ROAD WORK AHEAD (CW20-1D) sign mounted back to back with the reduced size 36" x 18" "END ROAD WORK" (G20-2) sign on low volume crossroads (see Note 4 under "Typical Construction Warning Sign Size and Spacing"). See the "Standard Highway Sign Designs for Texas" manual for sign details. The Engineer may omit the advance warning signs on low volume crossroads. The Engineer will determine whether a road is low volume as per TMUTCD Part 5. This information shall be shown in the plans.
 - Based on existing field conditions, the Engineer/Inspector may require additional signs such as FLAGGER AHEAD, LOOSE GRAVEL, or other appropriate signs. When additional signs are required, these signs will be considered part of the minimum requirements. The Engineer/Inspector will determine the proper location and spacing of any sign not shown on the BC sheets, Traffic Control Plan sheets or the Work Zone Standard Sheets.
 - The "ROAD WORK NEXT X MILES" (G20-1aT) sign shall be required at high volume crossroads to advise motorists of the length of construction in either direction from the intersection. The Engineer will determine whether a roadway is considered high volume.
 - Additional traffic control devices may be shown elsewhere in the plans for higher volume crossroads.
 - When work occurs in the intersection area, appropriate traffic control devices, as shown elsewhere in the plans or as determined by the Engineer/Inspector, shall be in place.

T-INTERSECTION



CSJ LIMITS AT T-INTERSECTION

- The Engineer will determine the types and location of any additional traffic control devices, such as a flagger and accompanying signs, or other signs, that should be used when work is being performed at or near an intersection.
- If construction closes the road at a T-intersection, the Contractor shall place the "CONTRACTOR NAME" (G20-6T) sign behind the Type 3 Barricades for the road closure (see BC(10) also). The "ROAD WORK NEXT X MILES" left arrow (G20-1bTL) and "ROAD WORK NEXT X MILES" right arrow (G20-1bTR) signs shall be replaced by the detour signing called for in the plans.

TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING^{1,5,6}

| Sign Number or Series | SIZE | | SPACING | |
|---------------------------------------|-------------------|--------------------|------------------|----------------------------------|
| | Conventional Road | Expressway/Freeway | Posted Speed MPH | Sign Δ Spacing "x" Feet (Apprx.) |
| CW20 ⁴ | 48" x 48" | 48" x 48" | 30 | 120 |
| CW21 | | | 35 | 160 |
| CW22 | | | 40 | 240 |
| CW23 | | | 45 | 320 |
| CW25 | | | 50 | 400 |
| CW1, CW2, CW7, CW8, CW9, CW11, CW14 | 36" x 36" | 48" x 48" | 55 | 500 ² |
| CW3, CW4, CW5, CW6, CW8-3, CW10, CW12 | 48" x 48" | 48" x 48" | 60 | 600 ² |
| | | | 65 | 700 ² |
| | | | 70 | 800 ² |
| | | | 75 | 900 ² |
| | | | 80 | 1000 ² |
| | | | * | * ³ |

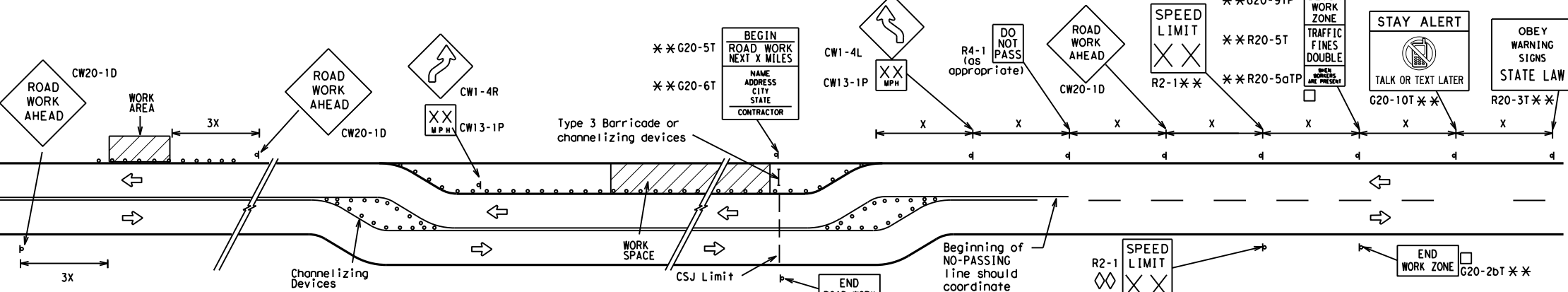
* For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.

Δ Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

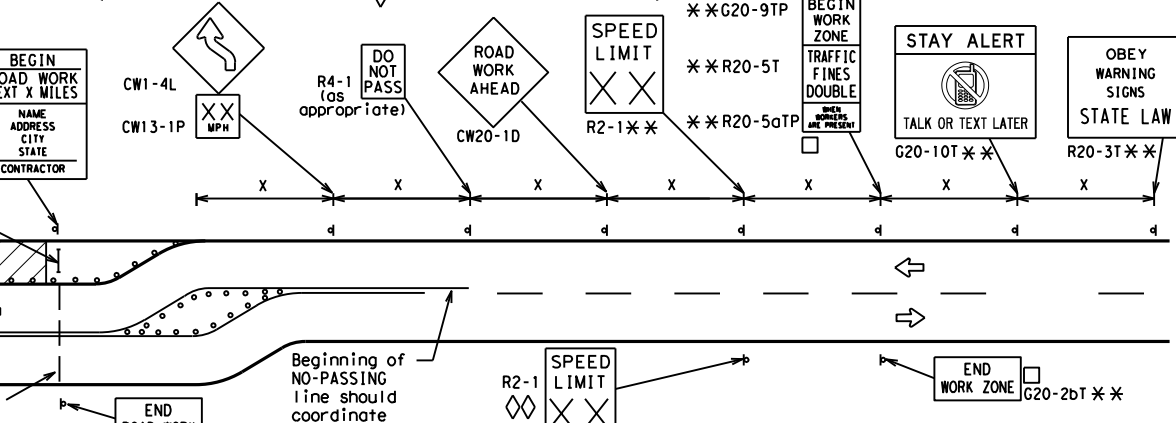
- Special or larger size signs may be used as necessary.
- Distance between signs should be increased as required to have 1500 feet advance warning.
- Distance between signs should be increased as required to have 1/2 mile or more advance warning.
- 36" x 36" "ROAD WORK AHEAD" (CW20-1D) signs may be used on low volume crossroads at the discretion of the Engineer as per TMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- Only diamond shaped warning sign sizes are indicated.
- See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design sizes.

WORK AREAS IN MULTIPLE LOCATIONS WITHIN CSJ LIMITS

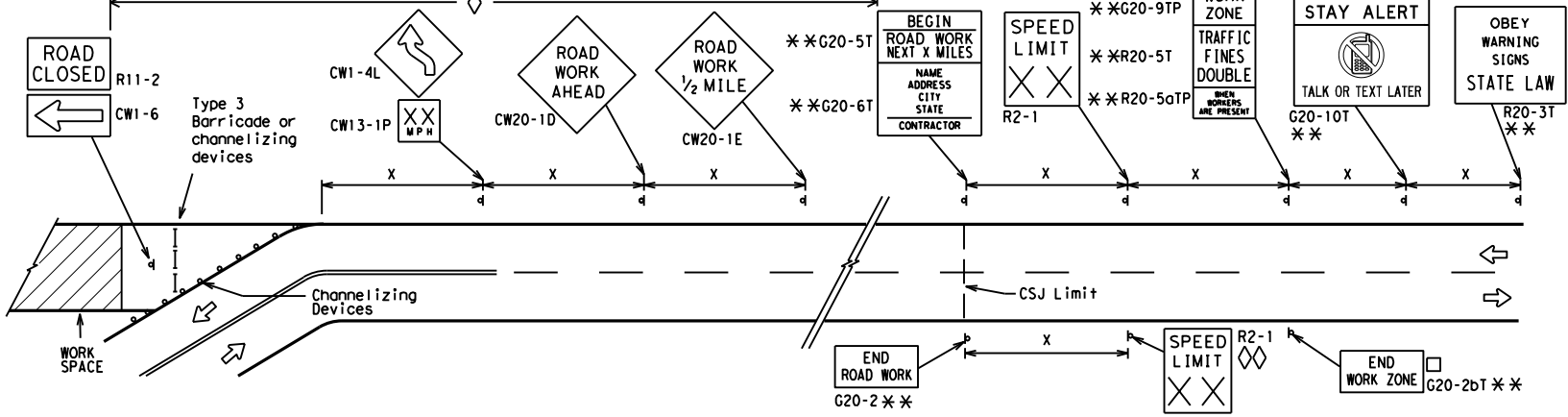


When extended distances occur between minimal work spaces, the Engineer/Inspector should ensure additional "ROAD WORK AHEAD" (CW20-1D) signs are placed in advance of these work areas to remind drivers they are still within the project limits. See the applicable TCP sheets for exact location and spacing of signs and channelizing devices.

SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING AT THE CSJ LIMITS



SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING DOWNSTREAM OF THE CSJ LIMITS



NOTES

- The Contractor shall determine the appropriate distance to be placed on the G20-1 series signs and "BEGIN ROAD WORK NEXT X MILES" (G20-5T) sign for each specific project. This distance shall replace the "x" and shall be rounded to the nearest whole mile with the approval of the Engineer. No decimals shall be used.
- The "BEGIN WORK ZONE" (G20-9TP) and "END WORK ZONE" (G20-2bT) shall be used as shown on the sample layout when advance signs are required outside the CSJ Limits. They inform the motorist of entering or leaving a part of the work zone lying outside the CSJ Limits where traffic fines may double if workers are present.
 - CSJ limit signing is required for highway construction and maintenance work, with the exception of mobile operations.
 - Area for placement of "ROAD WORK AHEAD" (CW20-1D) sign and other signs or devices as called for on the Traffic Control Plan.
 - Contractor will install a regulatory speed limit sign at the end of the work zone.

LEGEND

| | |
|-------|---|
| — | Type 3 Barricade |
| ○ ○ ○ | Channelizing Devices |
| ■ | Sign |
| X | See Typical Construction Warning Sign Size and Spacing chart or the TMUTCD for sign spacing requirements. |

SHEET 2 OF 12



BARRICADE AND CONSTRUCTION PROJECT LIMIT

BC(2)-21

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TYPICAL APPLICATION OF WORK ZONE SPEED LIMIT SIGNS

Work zone speed limits shall be regulatory, established in accordance with the "Procedures for Establishing Speed Zones," and approved by the Texas Transportation Commission, or by City Ordinance when within Incorporated City Limits.

Reduced speeds should only be posted in the vicinity of work activity and not throughout the entire project. Regulatory work zone speed signs (R2-1) shall be removed or covered during periods when they are not needed.



GUIDANCE FOR USE:

LONG/INTERMEDIATE TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit should be included on the design of the traffic control plans when restricted geometrics with a lower design speed are present in the work zone and modification of the geometrics to a higher design speed is not feasible.

Long/Intermediate Term Work Zone Speed Limit signs, when approved as described above, should be posted and visible to the motorist when work activity is present. Work activity may also be defined as a change in the roadway that requires a reduced speed for motorists to safely negotiate the work area, including:

- rough road or damaged pavement surface
- substantial alteration of roadway geometrics (diversions)
- construction detours
- grade
- width
- other conditions readily apparent to the driver

As long as any of these conditions exist, the work zone speed limit signs should remain in place.

SHORT TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit may be included on the design of the traffic control plans when workers or equipment are not behind concrete barrier, when work activity is within 10 feet of the traveled way or actually in the traveled way.

Short Term Work Zone Speed Limit signs should be posted and visible to the motorists only when work activity is present. When work activity is not present, signs shall be removed or covered. (See Removing or Covering on BC(4)).

GENERAL NOTES

- Regulatory work zone speed limits should be used only for sections of construction projects where speed control is of major importance.
- Regulatory work zone speed limit signs shall be placed on supports at a 7 foot minimum mounting height.
- Speed zone signs are illustrated for one direction of travel and are normally posted for each direction of travel.
- Frequency of work zone speed limit signs should be:

| | |
|--------------------|----------------|
| 40 mph and greater | 0.2 to 2 miles |
| 35 mph and less | 0.2 to 1 mile |
- Regulatory speed limit signs shall have black legend and border on a white reflective background (See "Reflective Sheeting" on BC(4)).
- Fabrication, erection and maintenance of the "ADVANCE SPEED LIMIT" (CW3-5) sign, "WORK ZONE" (G20-5aP) plaque and the "SPEED LIMIT" (R2-1) signs shall not be paid for directly, but shall be considered subsidiary to Item 502.
- Turning signs from view, laying signs over or down will not be allowed, unless as otherwise noted under "REMOVING OR COVERING" on BC(4).
- Techniques that may help reduce traffic speeds include but are not limited to:
 - Law enforcement.
 - Flagger stationed next to sign.
 - Portable changeable message sign (PCMS).
 - Low-power (drone) radar transmitter.
 - Speed monitor trailers or signs.
- Speeds shown on details above are for illustration only. Work Zone Speed Limits should only be posted as approved for each project.
- For more specific guidance concerning the type of work, work zone conditions and factors impacting allowable regulatory construction speed zone reduction see TxDOT form #1204 in the TxDOT e-form system.

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SHEET 3 OF 12



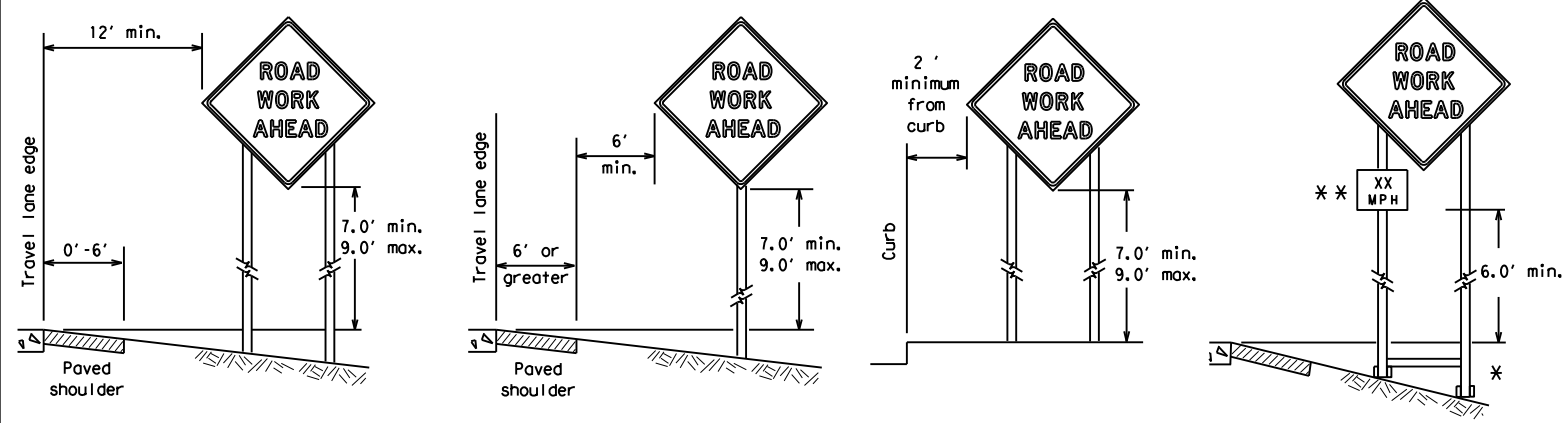
BARRICADE AND CONSTRUCTION WORK ZONE SPEED LIMIT

BC (3) - 21

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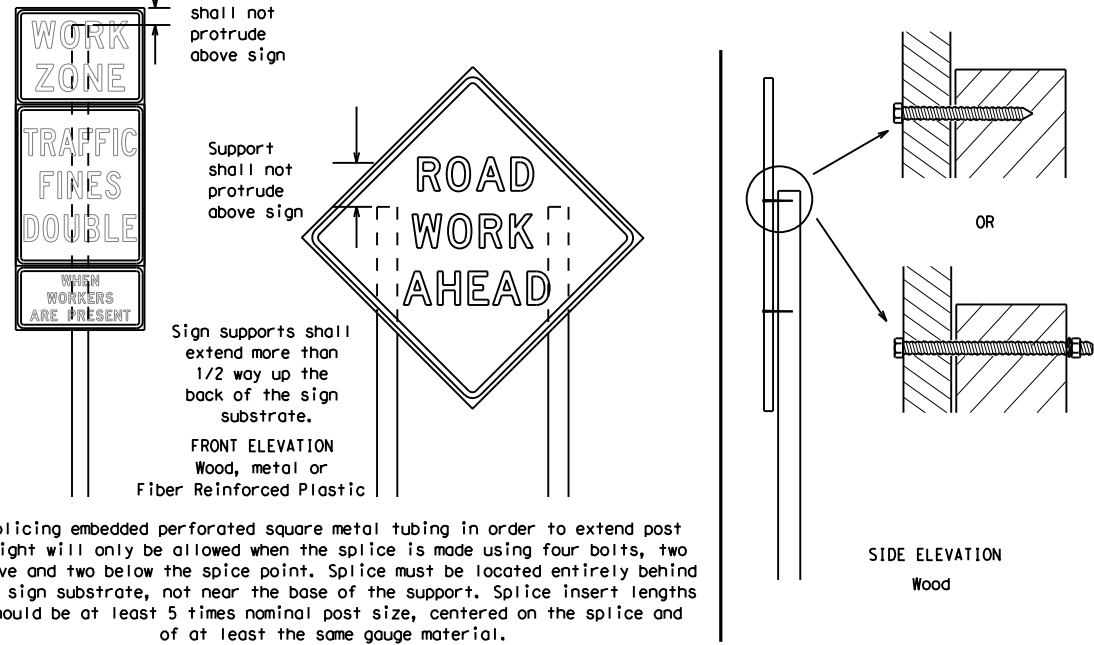
TYPICAL MINIMUM CLEARANCES FOR LONG TERM AND INTERMEDIATE TERM SIGNS



* When placing skid supports on unlevel ground, the leg post lengths must be adjusted so the sign appears straight and plumb. Objects shall NOT be placed under skids as a means of leveling.

** When plaques are placed on dual-leg supports, they should be attached to the upright nearest the travel lane. Supplemental plaques (advisory or distance) should not cover the surface of the parent sign.

ATTACHMENT FOR SIGN SUPPORTS



Splicing embedded perforated square metal tubing in order to extend post height will only be allowed when the splice is made using four bolts, two above and two below the splice point. Splice must be located entirely behind the sign substrate, not near the base of the support. Splice insert lengths should be at least 5 times nominal post size, centered on the splice and of at least the same gauge material.

GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports.
- All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and guide the traveling public safely through the work zone.
- The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so the Engineer can verify the correct procedures are being followed.
- The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used for identification shall be 1 inch.
- The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)

- The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in regard to crashworthiness and duration of work requirements.
 - Long-term stationary - work that occupies a location more than 3 days.
 - Intermediate-term stationary - work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting more than one hour.
 - Short-term stationary - daytime work that occupies a location for more than 1 hour in a single daylight period.
 - Short, duration - work that occupies a location up to 1 hour.
 - Mobile - work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except as shown for supplemental plaques mounted below other signs.
- The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above the ground.
- Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to appropriate Long-term/Intermediate sign height.
- Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

SIZE OF SIGNS

- The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

SIGN SUBSTRATES

- The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports.
- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave.
- All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6" centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300 for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).
- White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background.
- Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL}, shall be used for rigid signs with orange backgrounds.

SIGN LETTERS

- All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any intersections where the sign may be seen from approaching traffic.
- Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely covered when not required.
- When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.
- Burlap shall NOT be used to cover signs.
- Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

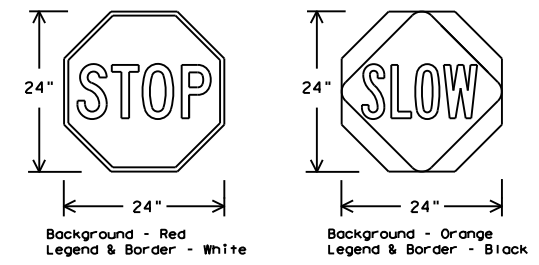
- Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used.
- The sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted for use as sign support weights.
- Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used.
- Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list.
- Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

- Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

STOP/SLOW PADDLES

- STOP/SLOW paddles are the primary method to control traffic by flaggers. The STOP/SLOW paddle size should be 24" x 24".
- STOP/SLOW paddles shall be retroreflective when used at night.
- STOP/SLOW paddles may be attached to a staff with a minimum length of 6' to the bottom of the sign.
- Any lights incorporated into the STOP or SLOW paddle faces shall only be as specifically described in Section 6E.03 Hand Signaling Devices in the TMUTCD.



| SHEETING REQUIREMENTS (WHEN USED AT NIGHT) | | |
|--|--------|--|
| USAGE | COLOR | SIGN FACE MATERIAL |
| BACKGROUND | RED | TYPE B OR C SHEETING |
| BACKGROUND | ORANGE | TYPE B _{FL} OR C _{FL} SHEETING |
| LEGEND & BORDER | WHITE | TYPE B OR C SHEETING |
| LEGEND & BORDER | BLACK | ACRYLIC NON-REFLECTIVE FILM |

CONTRACTOR REQUIREMENTS FOR MAINTAINING PERMANENT SIGNS WITHIN THE PROJECT LIMITS

- Permanent signs are used to give notice of traffic laws or regulations, call attention to conditions that are potentially hazardous to traffic operations, show route designations, destinations, directions, distances, services, points of interest, and other geographical, recreational, specific service (LOGO), or cultural information. Drivers proceeding through a work zone need the same, if not better route guidance as normally installed on a roadway without construction.
- When permanent regulatory or warning signs conflict with work zone conditions, remove or cover the permanent signs until the permanent sign message matches the roadway condition. For details for covering large guide signs see the TS-CD standard.
- When existing permanent signs are moved and relocated due to construction purposes, they shall be visible to motorists at all times.
- If existing signs are to be relocated on their original supports, they shall be installed on crashworthy bases as shown on the SMD Standard sheets. The signs shall meet the required mounting heights shown on the BC Sheets or the SMD Standards. This work should be paid for under the appropriate pay item for relocating existing signs.
- If permanent signs are to be removed and relocated using temporary supports, the Contractor shall use crashworthy supports as shown on the BC standard sheets, TLRS standard sheets or the CWZTCD list. The signs shall meet the required mounting heights shown on the BC, or the SMD standard sheets during construction. This work should be paid for under the appropriate pay item for relocating existing signs.
- Any sign or traffic control device that is struck or damaged by the Contractor or his/her construction equipment shall be replaced as soon as possible by the Contractor to ensure proper guidance for the motorists. This will be subsidiary to Item 502.



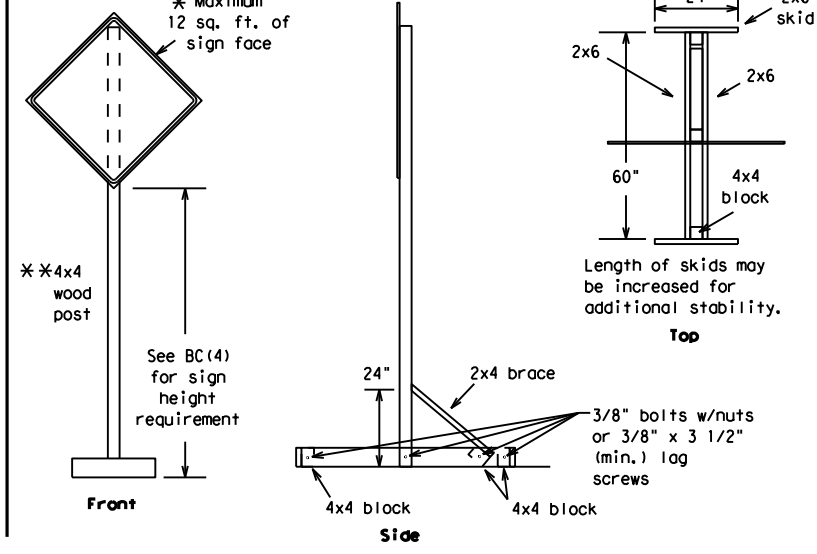
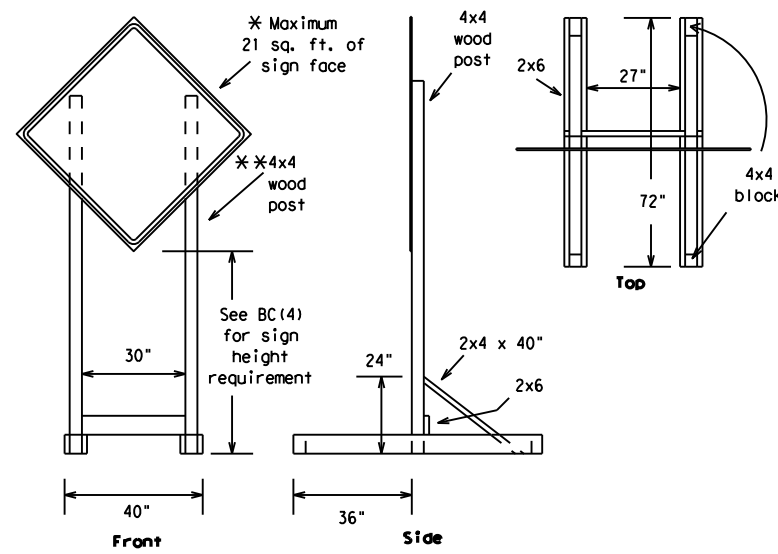
BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

BC (4) - 21

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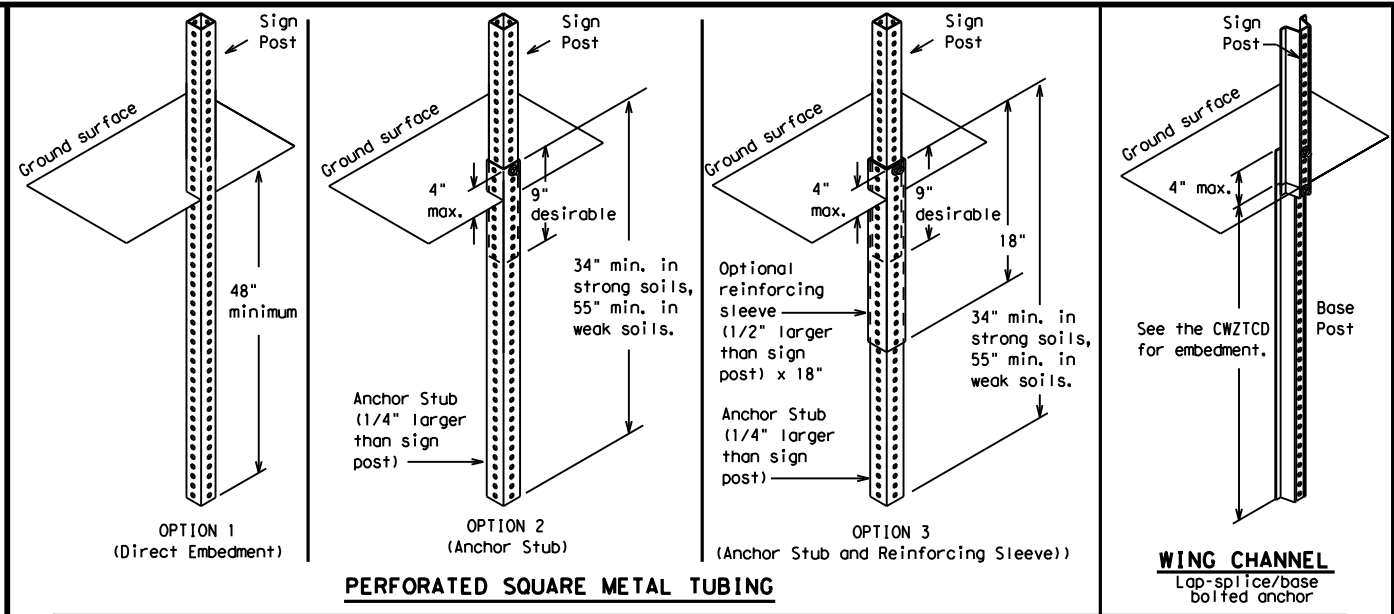
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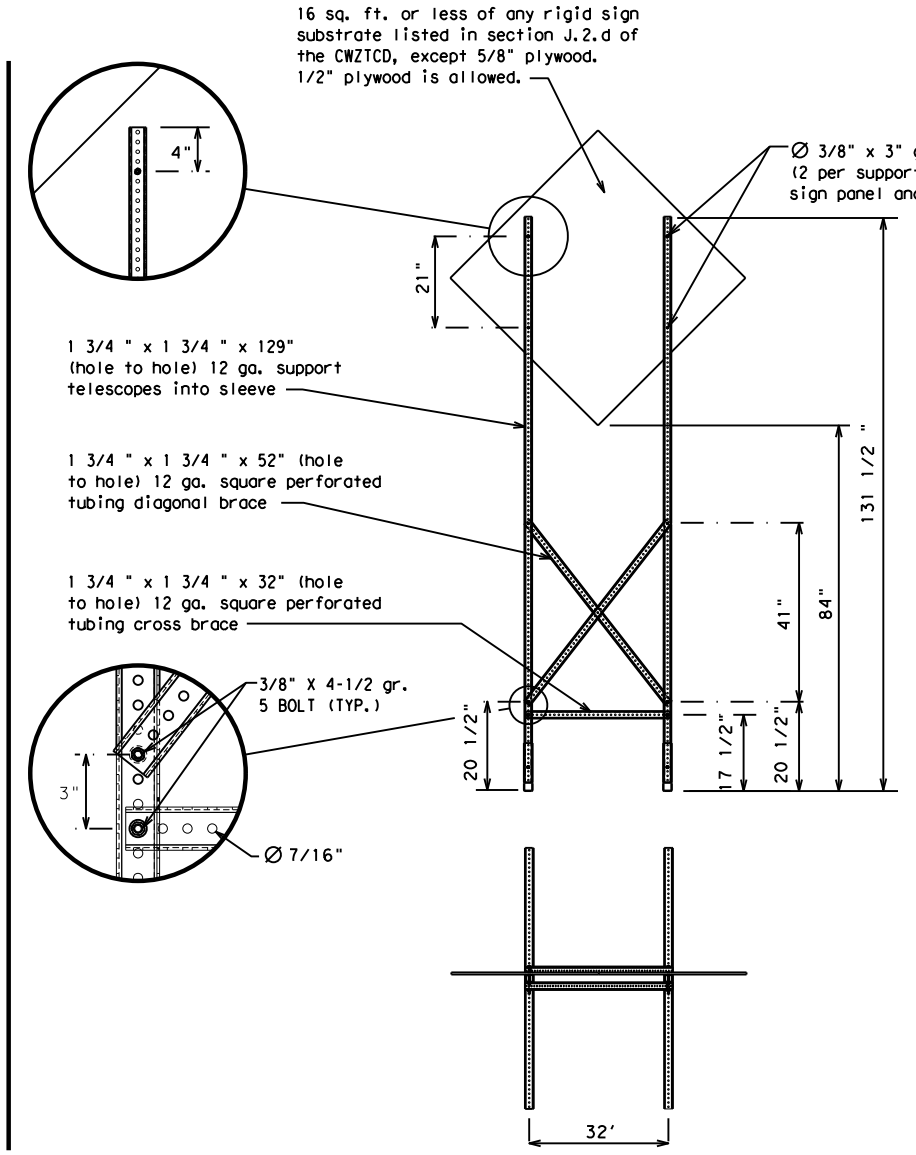
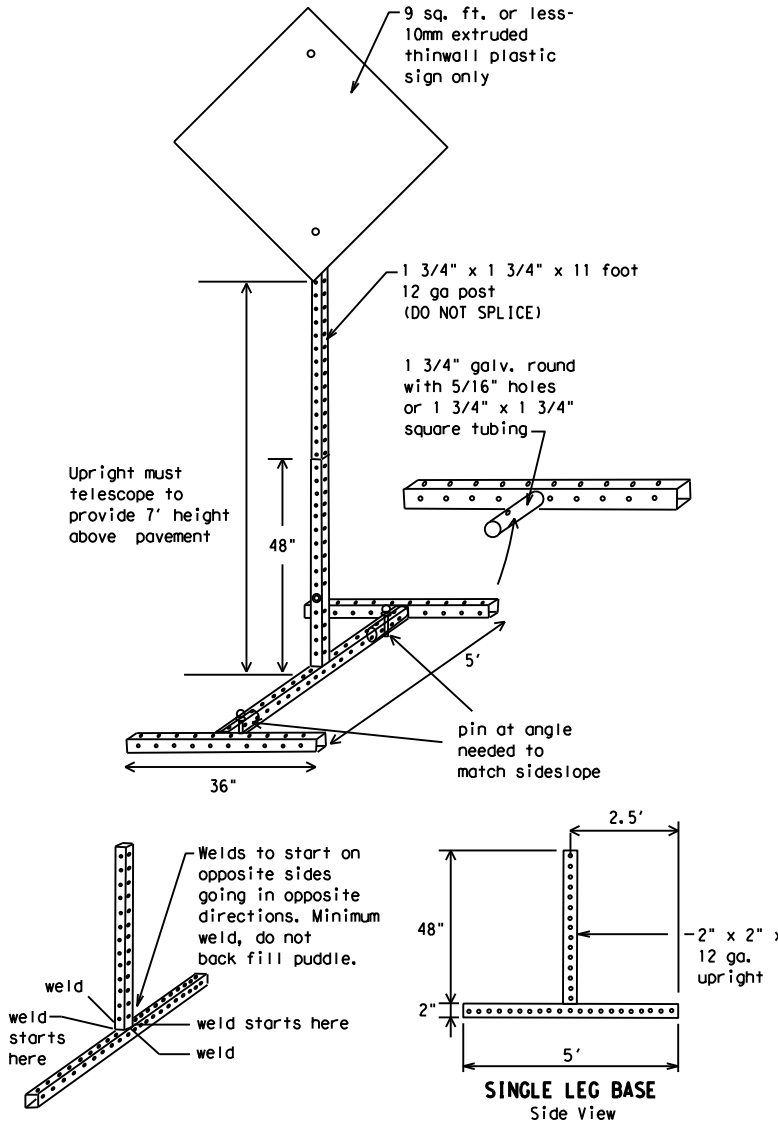
SKID MOUNTED WOOD SIGN SUPPORTS

* LONG/INTERMEDIATE TERM STATIONARY - PORTABLE SKID MOUNTED SIGN SUPPORTS



GROUND MOUNTED SIGN SUPPORTS

Refer to the CWZTCD and the manufacturer's installation procedure for each type sign support. The maximum sign square footage shall adhere to the manufacturer's recommendation. Two post installations can be used for larger signs.



SKID MOUNTED PERFORATED SQUARE STEEL TUBING SIGN SUPPORTS

* LONG/INTERMEDIATE TERM STATIONARY - PORTABLE SKID MOUNTED SIGN SUPPORTS

WEDGE ANCHORS
Both steel and plastic Wedge Anchor Systems as shown on the SMD Standard Sheets may be used as temporary sign supports for signs up to 10 square feet of sign face. They may be set in concrete or in sturdy soils if approved by the Engineer. (See web address for "Traffic Engineering Standard Sheets" on BC(1)).

OTHER DESIGNS
MORE DETAILS OF APPROVED LONG/INTERMEDIATE AND SHORT TERM SUPPORTS CAN BE FOUND ON THE CWZTCD LIST. SEE BC(1) FOR WEBSITE LOCATION.

- GENERAL NOTES**
- Nails may be used in the assembly of wooden sign supports, but 3/8" bolts with nuts or 3/8" x 3 1/2" lag screws must be used on every joint for final connection.
 - No more than 2 sign posts shall be placed within a 7 ft. circle, except for specific materials noted on the CWZTCD List.
 - When project is completed, all sign supports and foundations shall be removed from the project site. This will be considered subsidiary to Item 502.
- * See BC(4) for definition of "Work Duration."
 - ** Wood sign posts MUST be one piece. Splicing will NOT be allowed. Posts shall be painted white.
 - See the CWZTCD for the type of sign substrate that can be used for each approved sign support.

SHEET 5 OF 12



BARRICADE AND CONSTRUCTION TYPICAL SIGN SUPPORT

BC(5) - 21

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WHEN NOT IN USE, REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES

(The Engineer may approve other messages not specifically covered here.)

PORTABLE CHANGEABLE MESSAGE SIGNS

- The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO," "FOR," "AT," etc.
- Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- Do not use the word "Danger" in message.
- Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- Do not display messages that scroll horizontally or vertically across the face of the sign.
- The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- Each line of text should be centered on the message board rather than left or right justified.
- If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

Phase 1: Condition Lists

Road/Lane/Ramp Closure List

| | |
|-----------------------|--------------------------|
| FREEWAY CLOSED X MILE | FRONTAGE ROAD CLOSED |
| ROAD CLOSED AT SH XXX | SHOULDER CLOSED XXX FT |
| ROAD CLSD AT FM XXXX | RIGHT LN CLOSED XXX FT |
| RIGHT X LANES CLOSED | RIGHT X LANES OPEN |
| CENTER LANE CLOSED | DAYTIME LANE CLOSURES |
| NIGHT LANE CLOSURES | I-XX SOUTH EXIT CLOSED |
| VARIOUS LANES CLOSED | EXIT XXX CLOSED X MILE |
| EXIT CLOSED | RIGHT LN TO BE CLOSED |
| MALL DRIVEWAY CLOSED | X LANES CLOSED TUE - FRI |
| XXXXXXXX BLVD CLOSED | |

Other Condition List

| | |
|--------------------------|-------------------------|
| ROADWORK XXX FT | ROAD REPAIRS XXXX FT |
| FLAGGER XXXX FT | LANE NARROWS XXXX FT |
| RIGHT LN NARROWS XXXX FT | TWO-WAY TRAFFIC XX MILE |
| MERGING TRAFFIC XXXX FT | CONST TRAFFIC XXX FT |
| LOOSE GRAVEL XXXX FT | UNEVEN LANES XXXX FT |
| DETOUR X MILE | ROUGH ROAD XXXX FT |
| ROADWORK PAST SH XXXX | ROADWORK NEXT FRI-SUN |
| BUMP XXXX FT | US XXX EXIT X MILES |
| TRAFFIC SIGNAL XXXX FT | LANES SHIFT * |

* LANES SHIFT in Phase 1 must be used with STAY IN LANE in Phase 2.

Phase 2: Possible Component Lists

Action to Take/Effect on Travel List

| | |
|----------------------|----------------------|
| MERGE RIGHT | FORM X LINES RIGHT |
| DETOUR NEXT X EXITS | USE XXXXX RD EXIT |
| USE EXIT XXX | USE EXIT I-XX NORTH |
| STAY ON US XXX SOUTH | USE I-XX E TO I-XX N |
| TRUCKS USE US XXX N | WATCH FOR TRUCKS |
| WATCH FOR TRUCKS | EXPECT DELAYS |
| EXPECT DELAYS | PREPARE TO STOP |
| REDUCE SPEED XXX FT | END SHOULDER USE |
| USE OTHER ROUTES | WATCH FOR WORKERS |
| STAY IN LANE * | |

Location List

| |
|--------------------------|
| AT FM XXXX |
| BEFORE RAILROAD CROSSING |
| NEXT X MILES |
| PAST US XXX EXIT |
| XXXXXXXX TO XXXXXX |
| US XXX TO FM XXXX |

Warning List

| |
|-----------------------|
| SPEED LIMIT XX MPH |
| MAXIMUM SPEED XX MPH |
| MINIMUM SPEED XX MPH |
| ADVISORY SPEED XX MPH |
| RIGHT LANE EXIT |
| USE CAUTION |
| DRIVE SAFELY |
| DRIVE WITH CARE |

** Advance Notice List

| |
|-----------------------|
| TUE-FRI XX AM-X PM |
| APR XX-XX X PM-X AM |
| BEGINS MONDAY |
| BEGINS MAY XX |
| MAY X-X XX PM - XX AM |
| NEXT FRI-SUN |
| XX AM TO XX PM |
| NEXT TUE AUG XX |
| TONIGHT XX PM-XX AM |

** See Application Guidelines Note 6.

APPLICATION GUIDELINES

- Only 1 or 2 phases are to be used on a PCMS.
- The 1st phase (or both) should be selected from the "Road/Lane/Ramp Closure List" and the "Other Condition List".
- A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

- The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- Roadway designations IH, US, SH, FM and LP can be interchanged as appropriate.
- EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can be interchanged as appropriate.
- Highway names and numbers replaced as appropriate.
- ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- AHEAD may be used instead of distances if necessary.
- FT and MI, MILE and MILES interchanged as appropriate.
- AT, BEFORE and PAST interchanged as needed.
- Distances or AHEAD can be eliminated from the message if a location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC, THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

FULL MATRIX PCMS SIGNS

- When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under "PORTABLE CHANGEABLE MESSAGE SIGNS" above.
- When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the Engineer, it shall maintain the legibility/visibility requirement listed above.
- When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and shall not substitute for, or replace that sign.
- A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(7), for the same size arrow.

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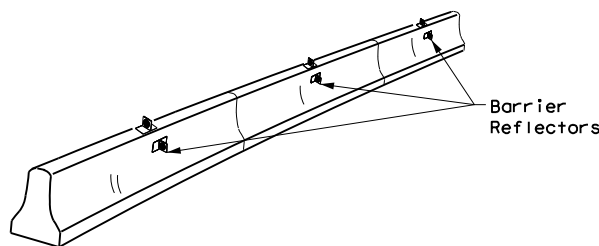
| WORD OR PHRASE | ABBREVIATION | WORD OR PHRASE | ABBREVIATION |
|------------------------|--------------|----------------|--------------|
| Access Road | ACCS RD | Major | MAJ |
| Alternate | ALT | Miles | MI |
| Avenue | AVE | Miles Per Hour | MPH |
| Best Route | BEST RTE | Minor | MNR |
| Boulevard | BLVD | Monday | MON |
| Bridge | BRDG | Normal | NORM |
| Cannot | CANT | North | N |
| Center | CTR | Northbound | (route) N |
| Construction Ahead | CONST AHD | Parking | PKING |
| CROSSING | XING | Road | RD |
| Detour Route | DETOUR RTE | Right Lane | RT LN |
| Do Not | DONT | Saturday | SAT |
| East | E | Service Road | SERV RD |
| Eastbound | (route) E | Shoulder | SHLDR |
| Emergency | EMER | Slippery | SLIP |
| Emergency Vehicle | EMER VEH | South | S |
| Entrance, Enter | ENT | Southbound | (route) S |
| Express Lane | EXP LN | Speed | SPD |
| Expressway | EXPWY | Street | ST |
| XXXX Feet | XXXX FT | Sunday | SUN |
| Fog Ahead | FOG AHD | Telephone | PHONE |
| Freeway | FRWY, FWY | Temporary | TEMP |
| Freeway Blocked | FWY BLKD | Thursday | THURS |
| Friday | FRI | To Downtown | TO DWNTN |
| Hazardous Driving | HAZ DRIVING | Traffic | TRAF |
| Hazardous Material | HAZMAT | Travelers | TRVLR |
| High-Occupancy Vehicle | HOV | Tuesday | TUES |
| Highway | HWY | Time Minutes | TIME MIN |
| Hour(s) | HR, HRS | Upper Level | UPR LEVEL |
| Information | INFO | Vehicles (s) | VEH, VEHS |
| It Is | ITS | Warning | WARN |
| Junction | JCT | Wednesday | WED |
| Left | LFT | Weight Limit | WT LIMIT |
| Left Lane | LFT LN | West | W |
| Lane Closed | LN CLOSED | Westbound | (route) W |
| Lower Level | LWR LEVEL | Wet Pavement | WET PVMT |
| Maintenance | MAINT | Will Not | WONT |

Roadway designation # IH-number, US-number, SH-number, FM-number

| | | | |
|---|-----------|-------|--------------|
| | | | |
| <h3>BARRICADE AND CONSTRUCTION PORTABLE CHANGEABLE MESSAGE SIGN (PCMS)</h3> | | | |
| <h2>BC (6) - 21</h2> | | | |
| FILE: | bc-21.dgn | DWG: | TxDOT |
| REVISED: | 0914 05 | DATE: | 204, ETC. |
| DATE: | 9-07 8-14 | DIST: | COUNTY |
| DATE: | 7-13 5-21 | AUS: | WILLIAMSON |
| | | | SHEET NO. 19 |

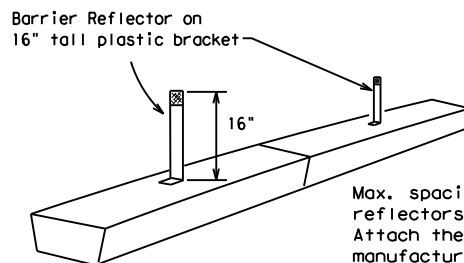
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- Barrier Reflectors shall be pre-qualified, and conform to the color and reflectivity requirements of DMS-8600. A list of prequalified Barrier Reflectors can be found at the Material Producer List web address shown on BC(1).
- Color of Barrier Reflectors shall be as specified in the TMUTCD. The cost of the reflectors shall be considered subsidiary to Item 512.



CONCRETE TRAFFIC BARRIER (CTB)

- Where traffic is on one side of the CTB, two (2) Barrier Reflectors shall be mounted in approximately the midsection of each section of CTB. An alternate mounting location is uniformly spaced at one end of each CTB. This will allow for attachment of a barrier grapple without damaging the reflector. The Barrier Reflector mounted on the side of the CTB shall be located directly below the reflector mounted on top of the barrier, as shown in the detail above.
- Where CTB separates two-way traffic, three barrier reflectors shall be mounted on each section of CTB. The reflector unit on top shall have two yellow reflective faces (Bi-Directional) while the reflectors on each side of the barrier shall have one yellow reflective face, as shown in the detail above.
- When CTB separates traffic traveling in the same direction, no barrier reflectors will be required on top of the CTB.
- Barrier Reflector units shall be yellow or white in color to match the edgeline being supplemented.
- Maximum spacing of Barrier Reflectors is forty (40) feet.
- Pavement markers or temporary flexible-reflective roadway marker tabs shall NOT be used as CTB delineation.
- Attachment of Barrier Reflectors to CTB shall be per manufacturer's recommendations.
- Missing or damaged Barrier Reflectors shall be replaced as directed by the Engineer.
- Single slope barriers shall be delineated as shown on the above detail.

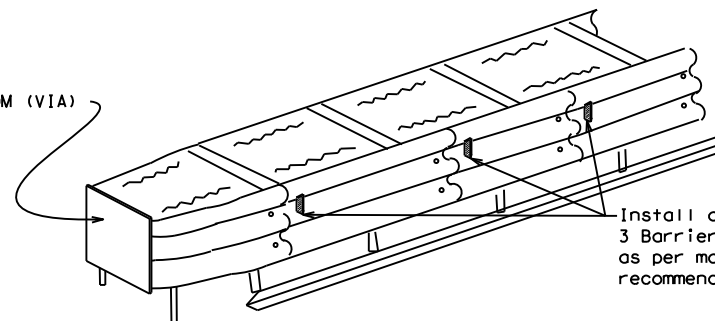


LOW PROFILE CONCRETE BARRIER (LPCB) USED IN WORK ZONES

LPCB is approved for use in work zone locations, where the posted speed is 45mph, or less. See Roadway Standard Sheet LPCB.

Max. spacing of barrier reflectors is 20 feet. Attach the delineators as per manufacturer's recommendations.

LOW PROFILE CONCRETE BARRIER (LPCB)



Install a minimum of 3 Barrier Reflectors as per manufacturer's recommendations.

DELINEATION OF END TREATMENTS

END TREATMENTS FOR CTB'S USED IN WORK ZONES

End treatments used on CTB's in work zones shall meet the appropriate crashworthy standards as defined in the Manual for Assessing Safety Hardware (MASH). Refer to the CWZTCD List for approved end treatments and manufacturers.

BARRIER REFLECTORS FOR CONCRETE TRAFFIC BARRIER AND ATTENUATORS

WARNING LIGHTS

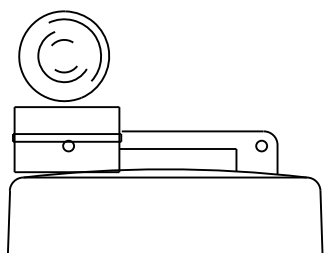
- Warning lights shall meet the requirements of the TMUTCD.
- Warning lights shall NOT be installed on barricades.
- Type A-Low Intensity Flashing Warning Lights are commonly used with drums. They are intended to warn of or mark a potentially hazardous area. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "FL". The Type A Warning Lights shall not be used with signs manufactured with Type B_{FL} or C_{FL} Sheeting meeting the requirements of Departmental Material Specification DMS-8300.
- Type-C and Type D 360 degree Steady Burn Lights are intended to be used in a series for delineation to supplement other traffic control devices. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "SB".
- The Engineer/Inspector or the plans shall specify the location and type of warning lights to be installed on the traffic control devices.
- When required by the Engineer, the Contractor shall furnish a copy of the warning lights certification. The warning light manufacturer will certify the warning lights meet the requirements of the latest ITE Purchase Specifications for Flashing and Steady-Burn Warning Lights.
- When used to delineate curves, Type-C and Type D Steady Burn Lights should only be placed on the outside of the curve, not the inside.
- The location of warning lights and warning reflectors on drums shall be as shown elsewhere in the plans.

WARNING LIGHTS MOUNTED ON PLASTIC DRUMS

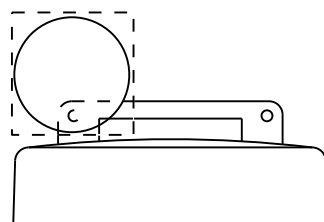
- Type A flashing warning lights are intended to warn drivers that they are approaching or are in a potentially hazardous area.
- Type A random flashing warning lights are not intended for delineation and shall not be used in a series.
- A series of sequential flashing warning lights placed on channelizing devices to form a merging taper may be used for delineation. If used, the successive flashing of the sequential warning lights should occur from the beginning of the taper to the end of the merging taper in order to identify the desired vehicle path. The rate of flashing for each light shall be 65 flashes per minute, plus or minus 10 flashes.
- Type C and D steady-burn warning lights are intended to be used in a series to delineate the edge of the travel lane on detours, on lane changes, on lane closures, and on other similar conditions.
- Type A, Type C and Type D warning lights shall be installed at locations as detailed on other sheets in the plans.
- Warning lights shall not be installed on a drum that has a sign, chevron or vertical panel.
- The maximum spacing for warning lights on drums should be identical to the channelizing device spacing.

WARNING REFLECTORS MOUNTED ON PLASTIC DRUMS AS A SUBSTITUTE FOR TYPE C (STEADY BURN) WARNING LIGHTS

- A warning reflector or approved substitute may be mounted on a plastic drum as a substitute for a Type C, steady burn warning light at the discretion of the Contractor unless otherwise noted in the plans.
- The warning reflector shall be yellow in color and shall be manufactured using a sign substrate approved for use with plastic drums listed on the CWZTCD.
- The warning reflector shall have a minimum retroreflective surface area (one-side) of 30 square inches.
- Round reflectors shall be fully reflectorized, including the area where attached to the drum.
- Square substrates must have a minimum of 30 square inches of reflectorized sheeting. They do not have to be reflectorized where it attaches to the drum.
- The side of the warning reflector facing approaching traffic shall have sheeting meeting the color and retroreflectivity requirements for DMS 8300-Type B or Type C.
- When used near two-way traffic, both sides of the warning reflector shall be reflectorized.
- The warning reflector should be mounted on the side of the handle nearest approaching traffic.
- The maximum spacing for warning reflectors should be identical to the channelizing device spacing requirements.



Type C Warning Light or approved substitute mounted on a drum adjacent to the travel way.

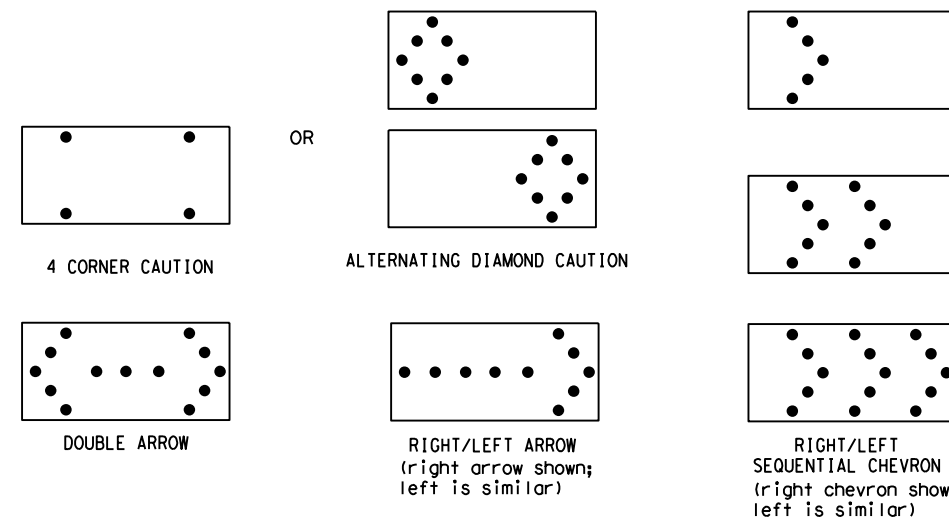


Warning reflector may be round or square. Must have a yellow reflective surface area of at least 30 square inches

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Arrow Boards may be located behind channelizing devices in place for a shoulder taper or merging taper, otherwise they shall be delineated with four (4) channelizing devices placed perpendicular to traffic on the upstream side of traffic.

- The Flashing Arrow Board should be used for all lane closures on multi-lane roadways, or slow moving maintenance or construction activities on the travel lanes.
- Flashing Arrow Boards should not be used on two-lane, two-way roadways, detours, diversions or work on shoulders unless the "CAUTION" display (see detail below) is used.
- The Engineer/Inspector shall choose all appropriate signs, barricades and/or other traffic control devices that should be used in conjunction with the Flashing Arrow Board.
- The Flashing Arrow Board should be able to display the following symbols:



- The "CAUTION" display consists of four corner lamps flashing simultaneously, or the Alternating Diamond Caution mode as shown.
- The straight line caution display is NOT ALLOWED.
- The Flashing Arrow Board shall be capable of minimum 50 percent dimming from rated lamp voltage. The flashing rate of the lamps shall not be less than 25 nor more than 40 flashes per minute.
- Minimum lamp "on time" shall be approximately 50 percent for the flashing arrow and equal intervals of 25 percent for each sequential phase of the flashing chevron.
- The sequential arrow display is NOT ALLOWED.
- The flashing arrow display is the TxDOT standard; however, the sequential chevron display may be used during daylight operations.
- The Flashing Arrow Board shall be mounted on a vehicle, trailer or other suitable support.
- A Flashing Arrow Board SHALL NOT BE USED to laterally shift traffic.
- A full matrix PCMS may be used to simulate a Flashing Arrow Board provided it meets visibility, flash rate and dimming requirements on this sheet for the same size arrow.
- Minimum mounting height of trailer mounted Arrow Boards should be 7 feet from roadway to bottom of panel.

| REQUIREMENTS | | | |
|--------------|--------------|-------------------------------|-----------------------------|
| TYPE | MINIMUM SIZE | MINIMUM NUMBER OF PANEL LAMPS | MINIMUM VISIBILITY DISTANCE |
| B | 30 x 60 | 13 | 3/4 mile |
| C | 48 x 96 | 15 | 1 mile |

ATTENTION
 Flashing Arrow Boards shall be equipped with automatic dimming devices.

WHEN NOT IN USE, REMOVE THE ARROW BOARD FROM THE RIGHT-OF-WAY OR PLACE THE ARROW BOARD BEHIND CONCRETE TRAFFIC BARRIER OR GUARDRAIL.

FLASHING ARROW BOARDS

SHEET 7 OF 12

TRUCK-MOUNTED ATTENUATORS

- Truck-mounted attenuators (TMA) used on TxDOT facilities must meet the requirements outlined in the Manual for Assessing Safety Hardware (MASH).
- Refer to the CWZTCD for the requirements of Level 2 or Level 3 TMAs.
- Refer to the CWZTCD for a list of approved TMAs.
- TMAs are required on freeways unless otherwise noted in the plans.
- A TMA should be used anytime that it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the work performance.
- The only reason a TMA should not be required is when a work area is spread down the roadway and the work crew is an extended distance from the TMA.



BARRICADE AND CONSTRUCTION ARROW PANEL, REFLECTORS, WARNING LIGHTS & ATTENUATOR

BC (7) - 21

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GENERAL NOTES

- For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

Pre-qualified plastic drums shall meet the following requirements:

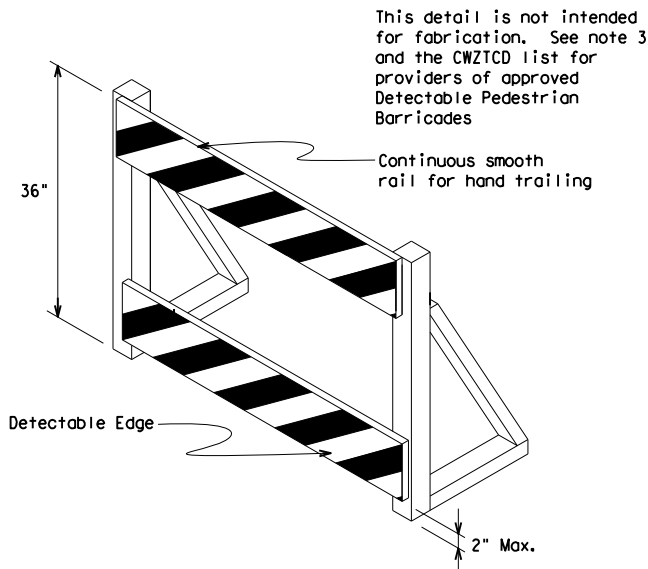
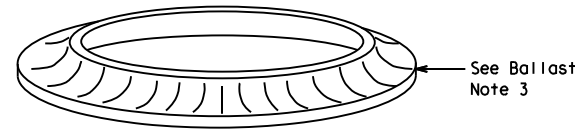
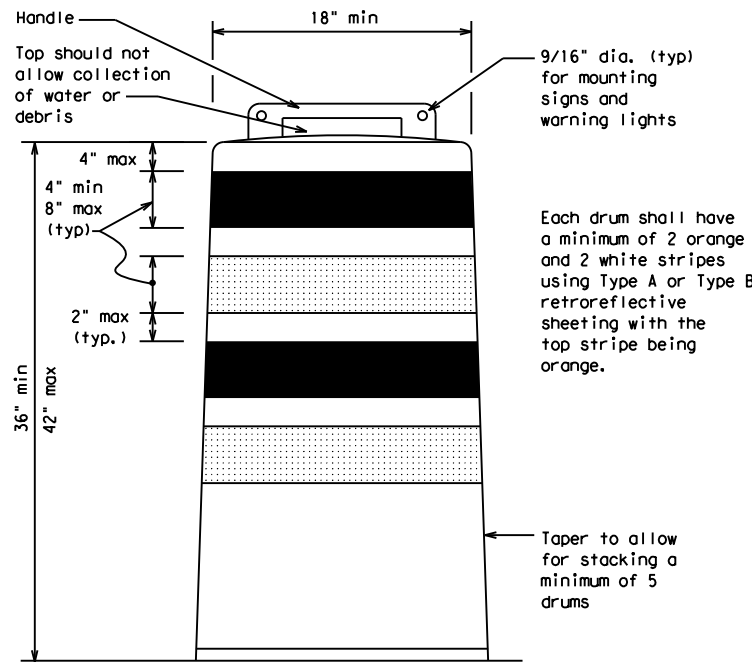
- Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- Drum body shall have a maximum unballasted weight of 11 lbs.
- Drum and base shall be marked with manufacturer's name and model number.

RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

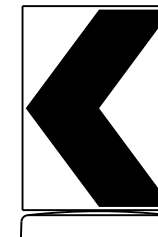
BALLAST

- Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- Ballast shall not be placed on top of drums.
- Adhesives may be used to secure base of drums to pavement.

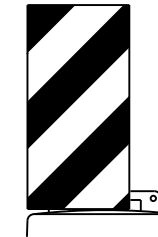


DETECTABLE PEDESTRIAN BARRICADES

- When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
- Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian path.
- Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian movements.
- Warning lights shall not be attached to detectable pedestrian barricades.
- Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.



18" x 24" Sign
(Maximum Sign Dimension)
Chevron CW1-8, Opposing Traffic Lane
Divider, Driveway sign D70a, Keep Right
R4 series or other signs as approved
by Engineer



12" x 24"
Vertical Panel
mount with diagonals
sloping down towards
travel way

Plywood, Aluminum or Metal sign
substrates shall NOT be used on
plastic drums

SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS

- Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD.
- Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL} Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
- Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
- Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
- Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection.
- Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts.
- Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans.
- R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.

SHEET 8 OF 12

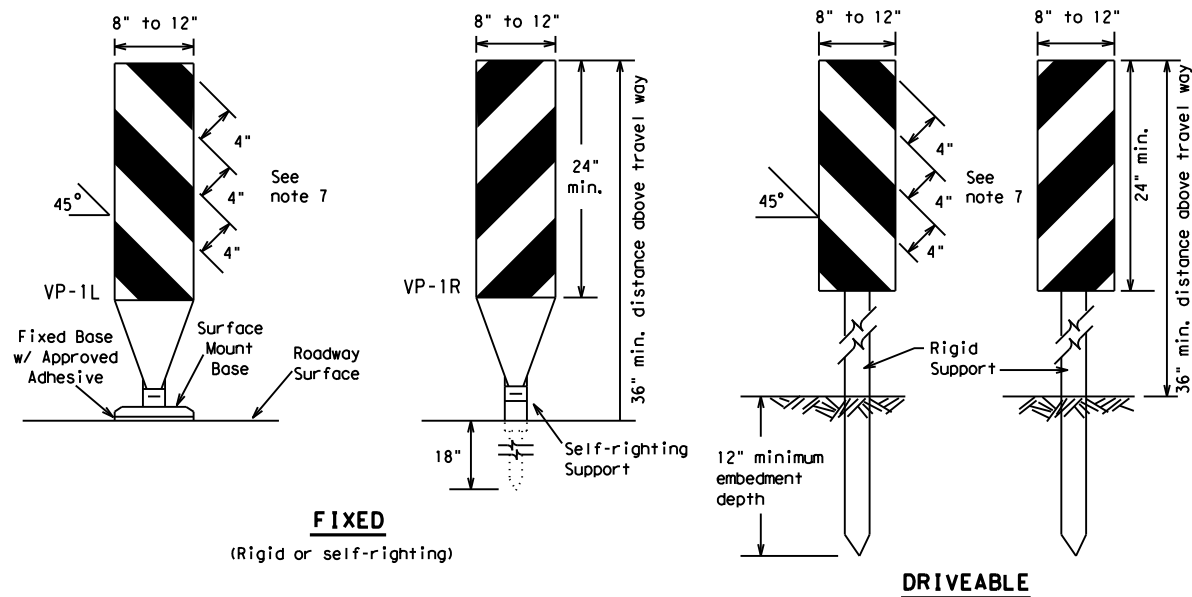


BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC (8) - 21

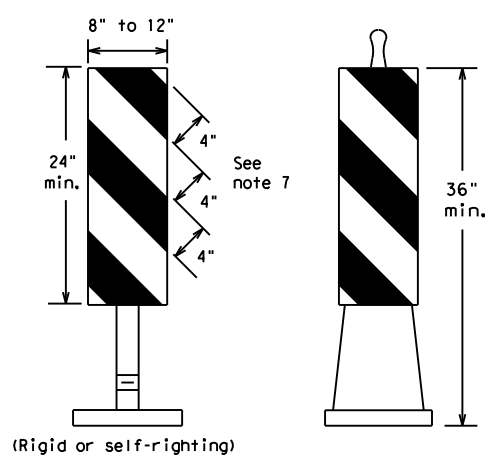
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FIXED
(Rigid or self-righting)

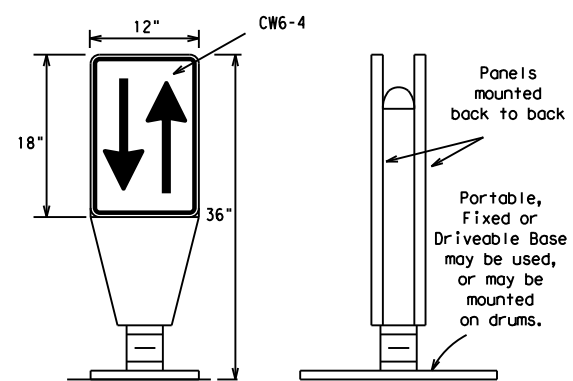
DRIVEABLE



PORTABLE

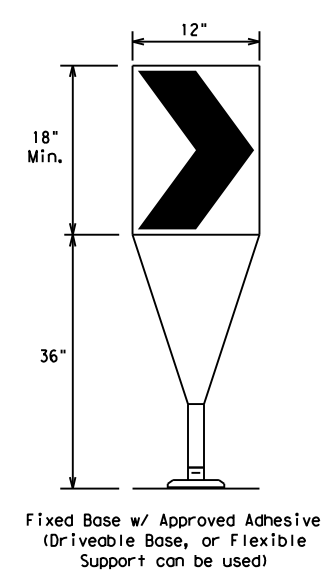
VERTICAL PANELS (VPs)

- Vertical Panels (VP's) are normally used to channelize traffic or divide opposing lanes of traffic.
- VP's may be used in daytime or nighttime situations. They may be used at the edge of shoulder drop-offs and other areas such as lane transitions where positive daytime and nighttime delineation is required. The Engineer/Inspector shall refer to the Roadway Design Manual for additional requirements on the use VP's for drop-offs.
- VP's should be mounted back to back if used at the edge of cuts adjacent to two-way two lane roadways. Stripes are to be reflective orange and reflective white and should always slope downward toward the travel lane.
- VP's used on expressways and freeways or other high speed roadways, may have more than 270 square inches of retroreflective area facing traffic.
- Self-righting supports are available with portable base. See "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- Sheeting for the VP's shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300, unless noted otherwise.
- Where the height of reflective material on the vertical panel is 36 inches or greater, a panel stripe of 6 inches shall be used.



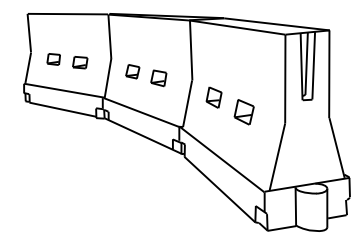
OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

- Opposing Traffic Lane Dividers (OTLD) are delineation devices designed to convert a normal one-way roadway section to two-way operation. OTLD's are used on temporary centerlines. The upward and downward arrows on the sign's face indicate the direction of traffic on either side of the divider. The base is secured to the pavement with an adhesive or rubber weight to minimize movement caused by a vehicle impact or wind gust.
- The OTLD may be used in combination with 42" cones or VPs.
- Spacing between the OTLD shall not exceed 500 feet. 42" cones or VPs placed between the OTLD's should not exceed 100 foot spacing.
- The OTLD shall be orange with a black non-reflective legend. Sheeting for the OTLD shall be retroreflective Type B_{FL} or Type C_{FL} conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.



- The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- Chevrons, when used, shall be erected on the outside of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- To be effective, the chevron should be visible for at least 500 feet.
- Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type B_{FL} or Type C_{FL} conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS



LONGITUDINAL CHANNELIZING DEVICES (LCD)

- LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- LCDs may be used instead of a line of cones or drums.
- LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings.
- Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH) urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

GENERAL NOTES

- Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.

| Posted Speed | Formula | Minimum Desirable Taper Lengths * * | | | Suggested Maximum Spacing of Channelizing Devices | |
|--------------|--------------------------|-------------------------------------|------------|------------|---|--------------|
| | | 10' Offset | 11' Offset | 12' Offset | On a Taper | On a Tangent |
| 30 | L = WS ² / 60 | 150' | 165' | 180' | 30' | 60' |
| 35 | | 205' | 225' | 245' | 35' | 70' |
| 40 | | 265' | 295' | 320' | 40' | 80' |
| 45 | L = WS | 450' | 495' | 540' | 45' | 90' |
| 50 | | 500' | 550' | 600' | 50' | 100' |
| 55 | | 550' | 605' | 660' | 55' | 110' |
| 60 | | 600' | 660' | 720' | 60' | 120' |
| 65 | | 650' | 715' | 780' | 65' | 130' |
| 70 | | 700' | 770' | 840' | 70' | 140' |
| 75 | | 750' | 825' | 900' | 75' | 150' |
| 80 | | 800' | 880' | 960' | 80' | 160' |

* * * Taper lengths have been rounded off.
L=Length of Taper (FT.) W=Width of Offset (FT.)
S=Posted Speed (MPH)

SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12



BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC (9) - 21

| | | | | |
|-----------------------|-----------|------------|-----------|-----------|
| FILE: bc-21.dgn | DN: TxDOT | CK: TxDOT | DW: TxDOT | CR: TxDOT |
| © TxDOT November 2002 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 9-07 8-14 | DIST | COUNTY | SHEET NO. | |
| 7-13 5-21 | AUS | WILLIAMSON | 22 | |

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TYPE 3 BARRICADES

1. Refer to the Compliant Work Zone Traffic Control Devices List (CWZTCD) for details of the Type 3 Barricades and a list of all materials used in the construction of Type 3 Barricades.
2. Type 3 Barricades shall be used at each end of construction projects closed to all traffic.
3. Barricades extending across a roadway should have stripes that slope downward in the direction toward which traffic must turn in detouring. When both right and left turns are provided, the chevron striping may slope downward in both directions from the center of the barricade. Where no turns are provided at a closed road, striping should slope downward in both directions toward the center of roadway.
4. Striping of rails, for the right side of the roadway, should slope downward to the left. For the left side of the roadway, striping should slope downward to the right.
5. Identification markings may be shown only on the back of the barricade rails. The maximum height of letters and/or company logos used for identification shall be 1".
6. Barricades shall not be placed parallel to traffic unless an adequate clear zone is provided.
7. Warning lights shall NOT be installed on barricades.
8. Where barricades require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand is recommended. The sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight. Sand bags shall not be stacked in a manner that covers any portion of a barricade rails reflective sheeting. Rock, concrete, iron, steel or other solid objects will NOT be permitted. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs. Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall not be used for sandbags. Sandbags shall only be placed along or upon the base supports of the device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners.
9. Sheeting for barricades shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300 unless otherwise noted.

Barricades shall NOT be used as a sign support.



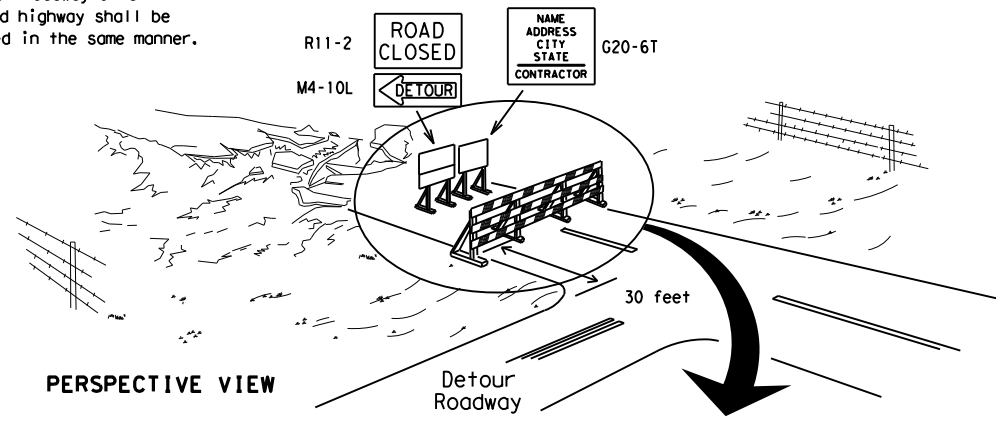
TYPICAL STRIPING DETAIL FOR BARRICADE RAIL



Stiffener may be inside or outside of support, but no more than 2 stiffeners shall be allowed on one barricade.

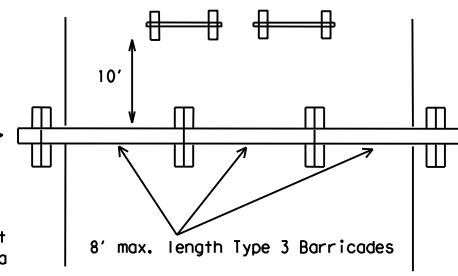
TYPICAL PANEL DETAIL FOR SKID OR POST TYPE BARRICADES

Each roadway of a divided highway shall be barricaded in the same manner.



PERSPECTIVE VIEW

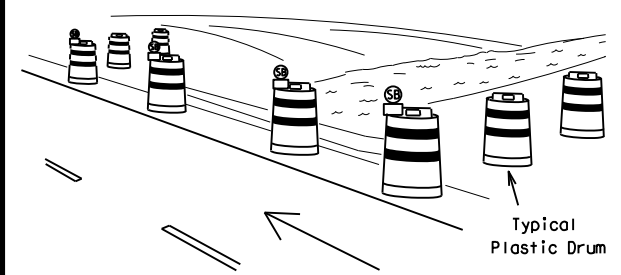
The three rails on Type 3 barricades shall be reflectorized orange and reflective white stripes on one side facing one-way traffic and both sides for two-way traffic. Barricade striping should slant downward in the direction of detour.



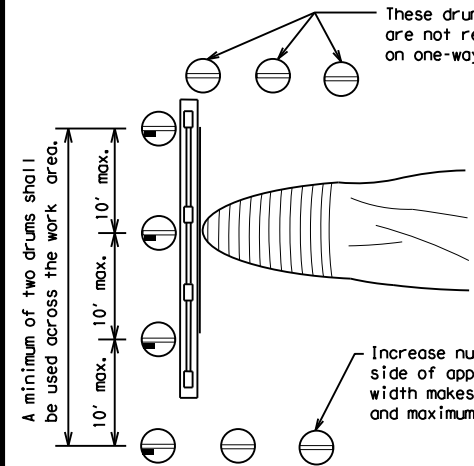
PLAN VIEW

1. Signs should be mounted on independent supports at a 7 foot mounting height in center of roadway. The signs should be a minimum of 10 feet behind Type 3 Barricades.
2. Advance signing shall be as specified elsewhere in the plans.

TYPE 3 BARRICADE (POST AND SKID) TYPICAL APPLICATION



PERSPECTIVE VIEW

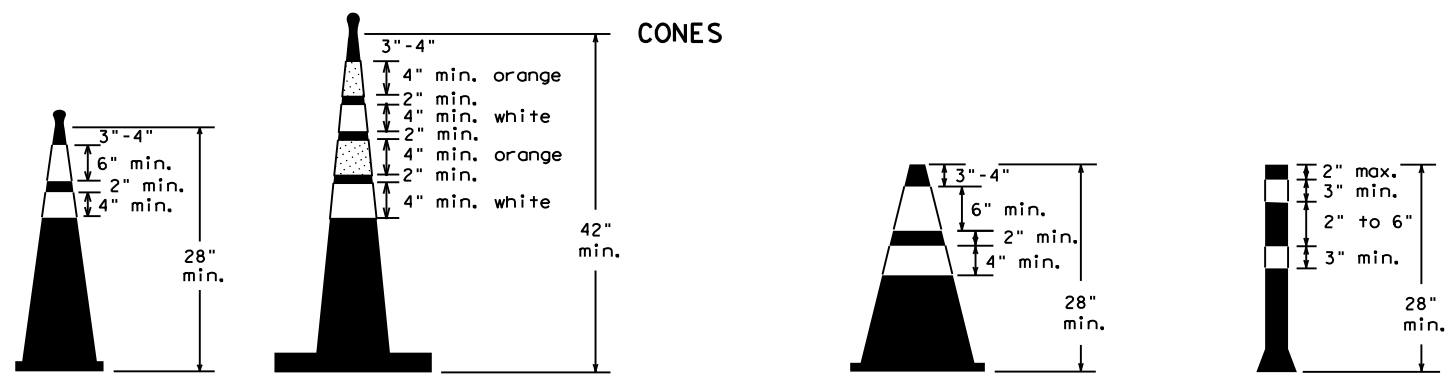


PLAN VIEW

1. Where positive redirection capability is provided, drums may be omitted.
2. Plastic construction fencing may be used with drums for safety as required in the plans.
3. Vertical Panels on flexible support may be substituted for drums when the shoulder width is less than 4 feet.
4. When the shoulder width is greater than 12 feet, steady-burn lights may be omitted if drums are used.
5. Drums must extend the length of the culvert widening.

| LEGEND | |
|--------|---|
| | Plastic drum |
| | Plastic drum with steady burn light or yellow warning reflector |
| | Steady burn warning light or yellow warning reflector |

CULVERT WIDENING OR OTHER ISOLATED WORK WITHIN THE PROJECT LIMITS



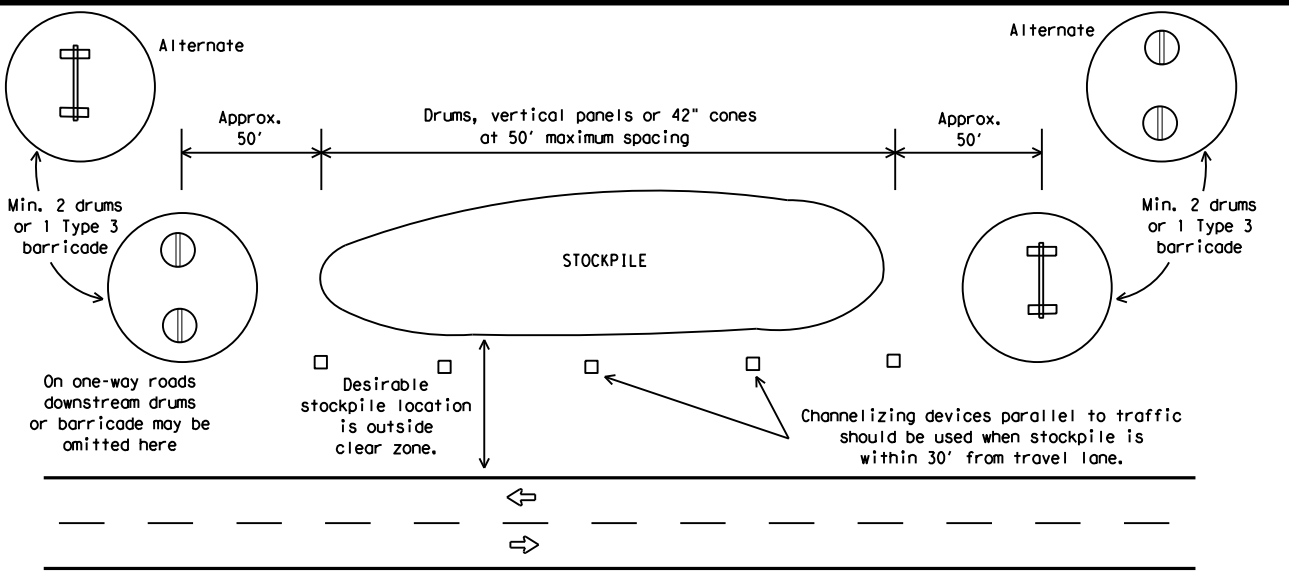
Two-Piece cones

One-Piece cones

Tubular Marker

28" Cones shall have a minimum weight of 9 1/2 lbs.
 42" 2-piece cones shall have a minimum weight of 30 lbs. including base.

1. Traffic cones and tubular markers shall be predominantly orange, and meet the height and weight requirements shown above.
2. One-piece cones have the body and base of the cone molded in one consolidated unit. Two-piece cones have a cone shaped body and a separate rubber base, or ballast, that is added to keep the device upright and in place.
3. Two-piece cones may have a handle or loop extending up to 8" above the minimum height shown, in order to aid in retrieving the device.
4. Cones or tubular markers shall have white or white and orange reflective bands as shown above. The reflective bands shall have a smooth, sealed outer surface and meet the requirements of Departmental Material Specification DMS-8300 Type A or Type B.
5. 28" cones and tubular markers are generally suitable for short duration and short-term stationary work as defined on BC(4). These should not be used for intermediate-term or long-term stationary work unless personnel is on-site to maintain them in their proper upright position.
6. 42" two-piece cones, vertical panels or drums are suitable for all work zone durations.
7. Cones or tubular markers used on each project should be of the same size and shape.



TRAFFIC CONTROL FOR MATERIAL STOCKPILES

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC (10) - 21

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| 7-13 5-21 | AUS | WILLIAMSON | 23 | |

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WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Additional supplemental pavement marking details may be found in the plans or specifications.
- Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

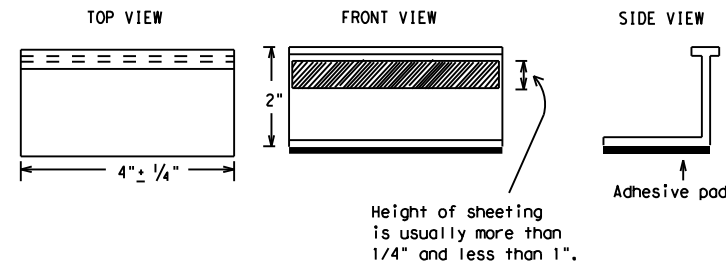
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- Blast cleaning may be used but will not be required unless specifically shown in the plans.
- Over-painting of the markings SHALL NOT BE permitted.
- Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



**STAPLES OR NAILS SHALL NOT BE USED TO SECURE
TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARKER
TABS TO THE PAVEMENT SURFACE**

- Temporary flexible-reflective roadway marker tabs used as guidemarks shall meet the requirements of DMS-8242.
- Tabs detailed on this sheet are to be inspected and accepted by the Engineer or designated representative. Sampling and testing is not normally required, however at the option of the Engineer, either "A" or "B" below may be imposed to assure quality before placement on the roadway.
 - Select five (5) or more tabs at random from each lot or shipment and submit to the Construction Division, Materials and Pavement Section to determine specification compliance.
 - Select five (5) tabs and perform the following test. Affix five (5) tabs at 24 inch intervals on an asphaltic pavement in a straight line. Using a medium size passenger vehicle or pickup, run over the markers with the front and rear tires at a speed of 35 to 40 miles per hour, four (4) times in each direction. No more than one (1) out of the five (5) reflective surfaces shall be lost or displaced as a result of this test.
- Small design variances may be noted between tab manufacturers.
- See Standard Sheet WZ(STPM) for tab placement on new pavements. See Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARKS

- Raised pavement markers used as guidemarks shall be from the approved product list, and meet the requirements of DMS-4200.
- All temporary construction raised pavement markers provided on a project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applied or butyl rubber pad for all surfaces, or thermoplastic for concrete surfaces.

Guidemarks shall be designated as:
 YELLOW - (two amber reflective surfaces with yellow body).
 WHITE - (one silver reflective surface with white body).

| DEPARTMENTAL MATERIAL SPECIFICATIONS | |
|--|----------|
| PAVEMENT MARKERS (REFLECTORIZED) | DMS-4200 |
| TRAFFIC BUTTONS | DMS-4300 |
| EPOXY AND ADHESIVES | DMS-6100 |
| BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS | DMS-6130 |
| PERMANENT PREFABRICATED PAVEMENT MARKINGS | DMS-8240 |
| TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS | DMS-8241 |
| TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS | DMS-8242 |

A list of prequalified reflective raised pavement markers, non-reflective traffic buttons, roadway marker tabs and other pavement markings can be found at the Material Producer List web address shown on BC(1).

SHEET 11 OF 12



BARRICADE AND CONSTRUCTION PAVEMENT MARKINGS

BC(11)-21

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|----------------------|-----------|------------|-----------|-----------|
| FILE: bc-21.dgn | DN: TxDOT | CR: TxDOT | DW: TxDOT | CK: TxDOT |
| ©TxDOT February 1998 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 2-98 9-07 5-21 | DIST | COUNTY | SHEET NO. | |
| 1-02 7-13 | AUS | WILLIAMSON | 24 | |
| 11-02 8-14 | | | | |

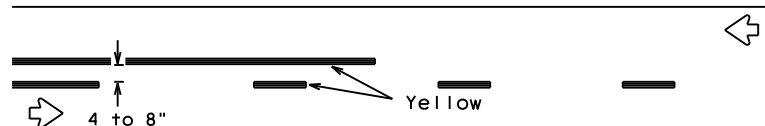
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PAVEMENT MARKING PATTERNS

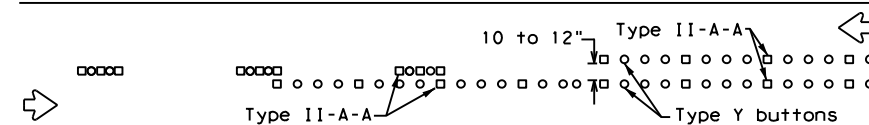


REFLECTORIZED PAVEMENT MARKINGS - PATTERN A

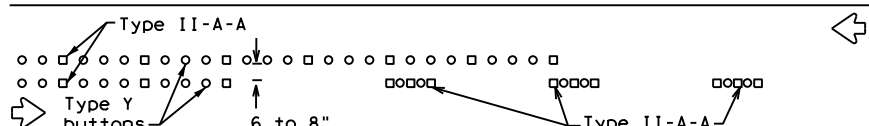


REFLECTORIZED PAVEMENT MARKINGS - PATTERN B

Pattern A is the TxDOT Standard, however Pattern B may be used if approved by the Engineer. Prefabricated markings may be substituted for reflectORIZED pavement markings.



RAISED PAVEMENT MARKERS - PATTERN A



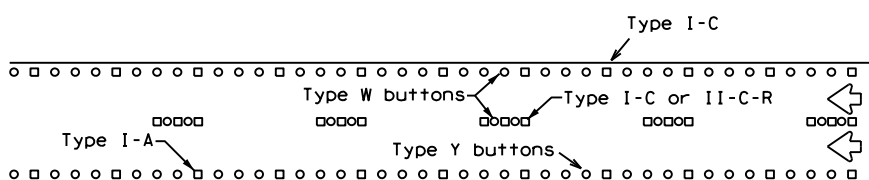
RAISED PAVEMENT MARKERS - PATTERN B

CENTER LINE & NO-PASSING ZONE BARRIER LINES FOR TWO-LANE, TWO-WAY HIGHWAYS



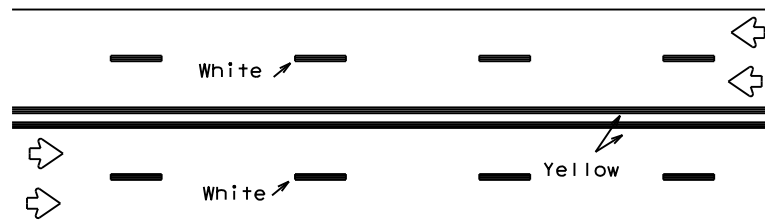
REFLECTORIZED PAVEMENT MARKINGS

Prefabricated markings may be substituted for reflectORIZED pavement markings.



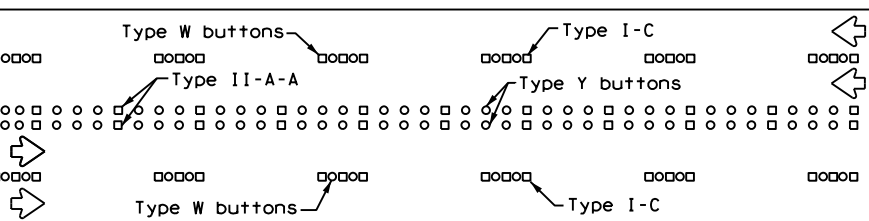
RAISED PAVEMENT MARKERS

EDGE & LANE LINES FOR DIVIDED HIGHWAY



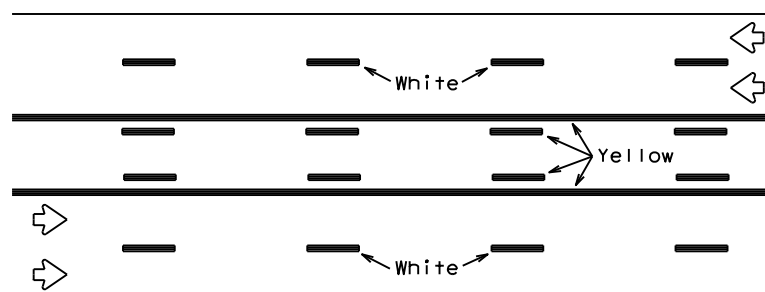
REFLECTORIZED PAVEMENT MARKINGS

Prefabricated markings may be substituted for reflectORIZED pavement markings.



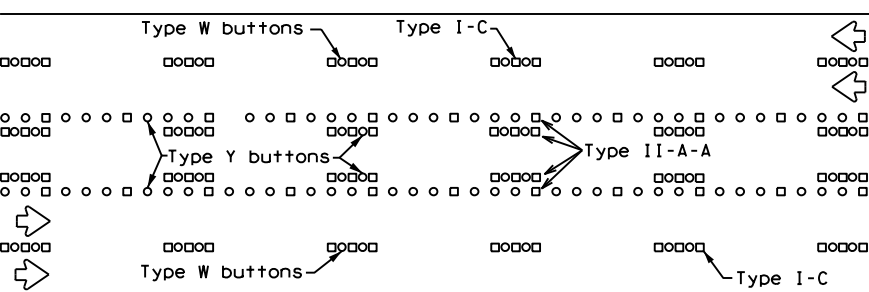
RAISED PAVEMENT MARKERS

LANE & CENTER LINES FOR MULTILANE UNDIVIDED HIGHWAYS



REFLECTORIZED PAVEMENT MARKINGS

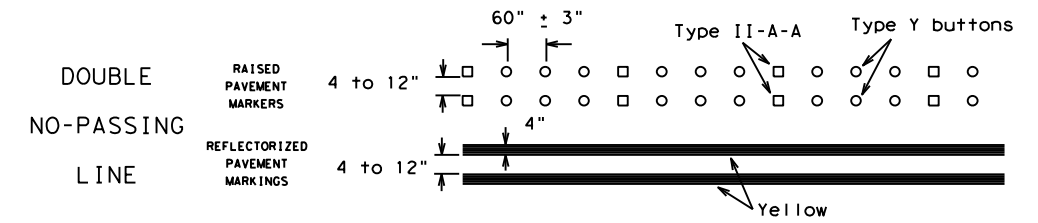
Prefabricated markings may be substituted for reflectORIZED pavement markings.



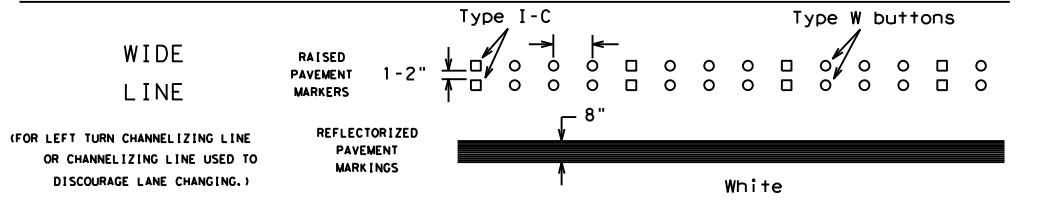
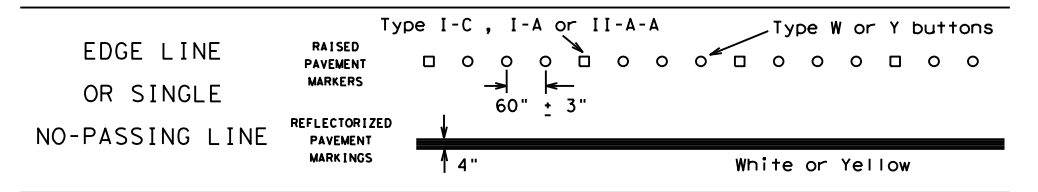
RAISED PAVEMENT MARKERS

TWO-WAY LEFT TURN LANE

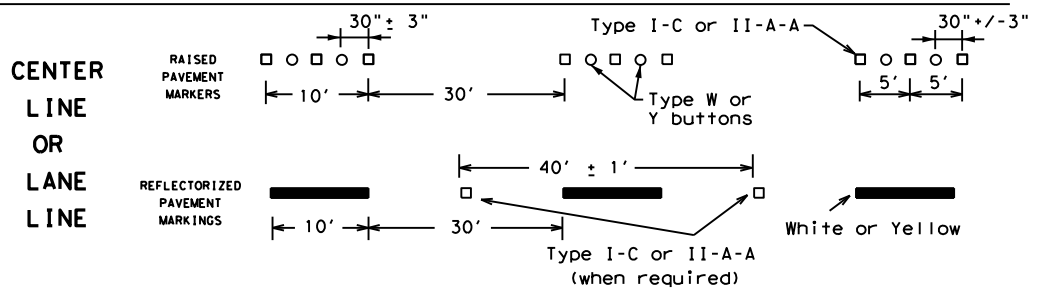
STANDARD WORK ZONE PAVEMENT MARKINGS DETAILS



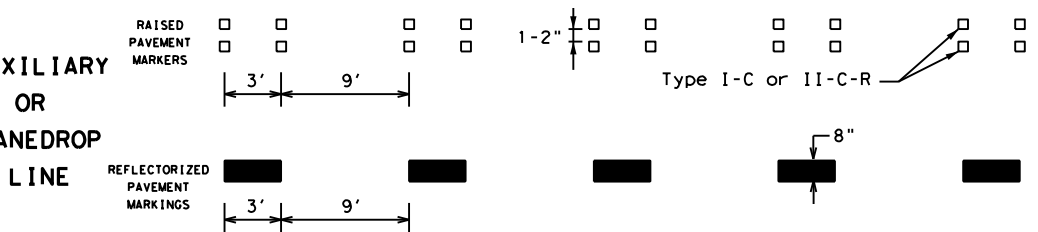
SOLID LINES



BROKEN LINES

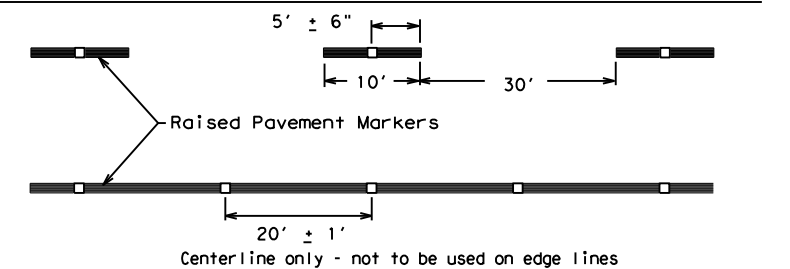


AUXILIARY OR LANEDROP LINE



REMOVABLE MARKINGS WITH RAISED PAVEMENT MARKERS

If raised pavement markers are used to supplement REMOVABLE markings, the markers shall be applied to the top of the tape at the approximate mid length of tape used for broken lines or at 20 foot spacing for solid lines. This allows an easier removal of raised pavement markers and tape.



SHEET 12 OF 12



BARRICADE AND CONSTRUCTION PAVEMENT MARKING PATTERNS

BC(12)-21

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| FILE: bc-21.dgn | DN: TxDOT | CK: TxDOT | OW: TxDOT | CR: TxDOT |
| ©TxDOT February 1998 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 1-97 9-07 5-21 | DIST | COUNTY | SHEET NO. | |
| 2-98 7-13 | AUS | WILLIAMSON | 25 | |
| 11-02 8-14 | | | | |

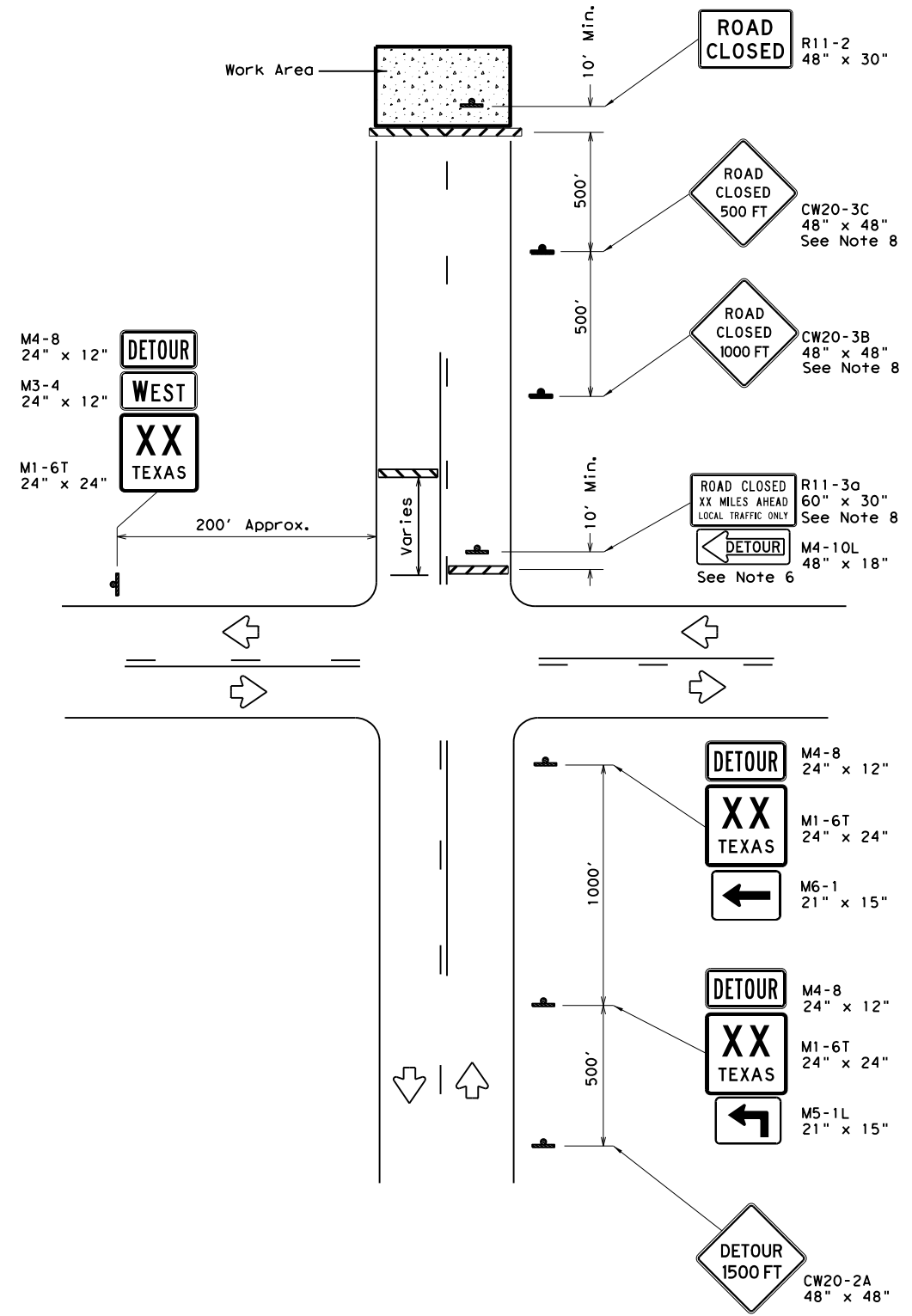
Raised pavement markers used as standard pavement markings shall be from the approved products list and meet the requirements of Item 672 "RAISED PAVEMENT MARKERS."

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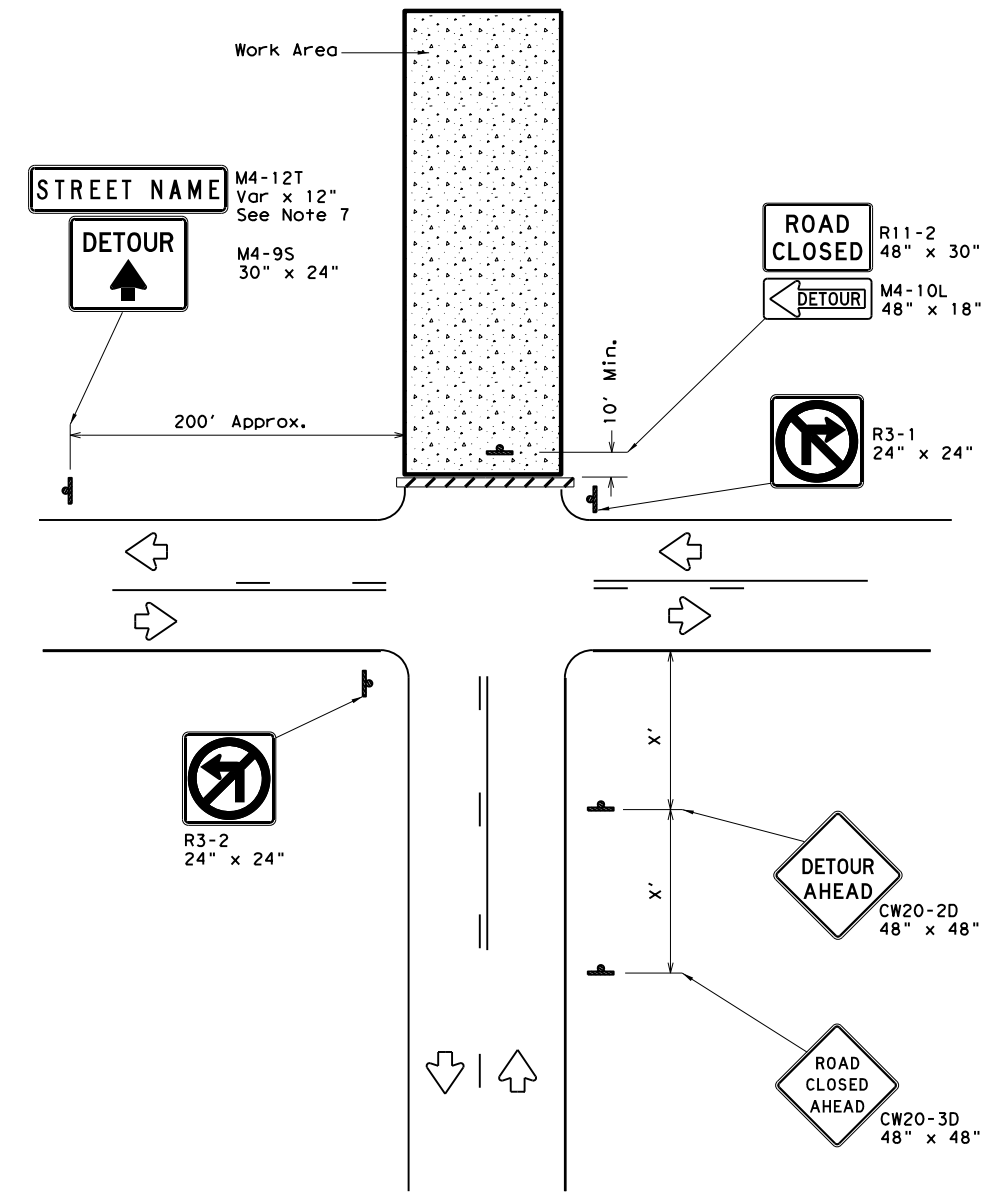
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ROAD CLOSURE BEYOND THE INTERSECTION
 Signing for a Numbered Route with an Off-Site Detour



ROAD CLOSURE AT THE INTERSECTION
 Signing for an Un-numbered Route with an Off-Site Detour

| LEGEND | |
|--------|------------------|
| | Type 3 Barricade |
| | Sign |

| Posted Speed * | Minimum Sign Spacing "x" Distance |
|----------------|-----------------------------------|
| 30 | 120' |
| 35 | 160' |
| 40 | 240' |
| 45 | 320' |
| 50 | 400' |
| 55 | 500' |
| 60 | 600' |
| 65 | 700' |
| 70 | 800' |
| 75 | 900' |

* Conventional Roads Only

GENERAL NOTES

1. This sheet is intended to provide details for temporary work zone road closures. For permanent road closure details see the D&OM standards.
2. Barricades used shall meet the requirements shown on Barricade and Construction Standard BC(10) and listed on the Compliant Work Zone Traffic Control Devices list (CWZTCD).
3. Stockpiled materials shall not be placed on the traffic side of barricades.
4. Barricades at the road closure should extend from pavement edge to pavement edge.
5. Detour signing shown is intended to illustrate the type of signing that is appropriate for numbered routes or un-numbered routes as labeled. It does not indicate the full extent of detour signing required. Detour routes should be signed as shown elsewhere in the plans.
6. If the road is open for a significant distance beyond the intersection or there are significant origin/destination points beyond the intersection, the signs and barricades at this location should be located at the edge of the traveled way.
7. The Street Name (M4-12T) sign is to be placed above the DETOUR (M4-9S) sign.
8. For urban areas where there is a shorter distance between the intersection and the actual closure location, the ROAD CLOSED XX MILES AHEAD (R11-3a) sign may be replaced with a ROAD CLOSED TO THRU TRAFFIC (R11-4) sign. If adequate space does not exist between the intersection and the closure a single ROAD CLOSED AHEAD (CW20-3D) sign spaced as per the table above may replace the ROAD CLOSED 1000 FT (CW20-3B) and ROAD CLOSED 500 FT (CW20-3C) signs.
9. Signs and barricades shown shall be subsidiary to Item 502. Locations where these details will be required shall be as shown elsewhere in the plans.

Texas Department of Transportation
 Traffic Operations Division Standard

WORK ZONE ROAD CLOSURE DETAILS

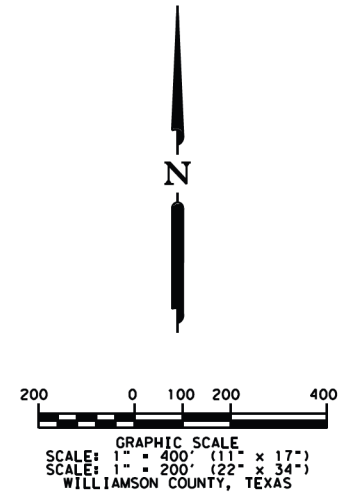
WZ (RCD) - 13

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| 1-97 4-98 7-13 | DIST | COUNTY | SHEET NO. | |
| 2-98 3-03 | AUS | WILLIAMSON | 26 | |

SURVEY CONTROL LAYOUT SHEET



VICINITY MAP
NOT TO SCALE

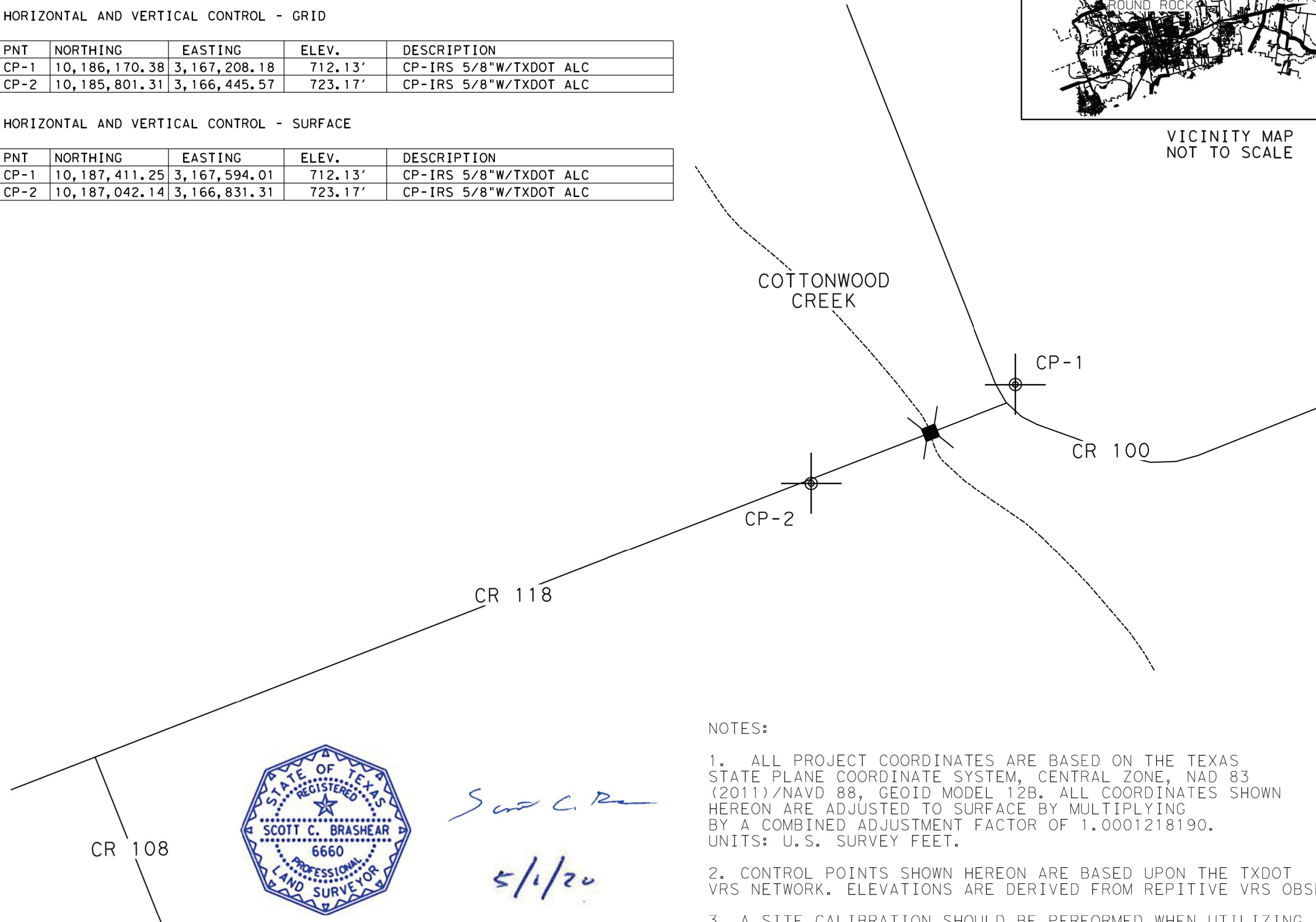


HORIZONTAL AND VERTICAL CONTROL - GRID

| PNT | NORTHING | EASTING | ELEV. | DESCRIPTION |
|------|---------------|--------------|---------|------------------------|
| CP-1 | 10,186,170.38 | 3,167,208.18 | 712.13' | CP-IRS 5/8"W/TXDOT ALC |
| CP-2 | 10,185,801.31 | 3,166,445.57 | 723.17' | CP-IRS 5/8"W/TXDOT ALC |

HORIZONTAL AND VERTICAL CONTROL - SURFACE

| PNT | NORTHING | EASTING | ELEV. | DESCRIPTION |
|------|---------------|--------------|---------|------------------------|
| CP-1 | 10,187,411.25 | 3,167,594.01 | 712.13' | CP-IRS 5/8"W/TXDOT ALC |
| CP-2 | 10,187,042.14 | 3,166,831.31 | 723.17' | CP-IRS 5/8"W/TXDOT ALC |



NOTES:

1. ALL PROJECT COORDINATES ARE BASED ON THE TEXAS STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NAD 83 (2011)/NAVD 88, GEOID MODEL 12B. ALL COORDINATES SHOWN HEREON ARE ADJUSTED TO SURFACE BY MULTIPLYING BY A COMBINED ADJUSTMENT FACTOR OF 1.0001218190. UNITS: U. S. SURVEY FEET.
2. CONTROL POINTS SHOWN HEREON ARE BASED UPON THE TXDOT VRS NETWORK. ELEVATIONS ARE DERIVED FROM REPITIVE VRS OBSERVATIONS.
3. A SITE CALIBRATION SHOULD BE PERFORMED WHEN UTILIZING THE CONTROL SHOWN HEREIN.

Scott C. Brashear
5/1/20



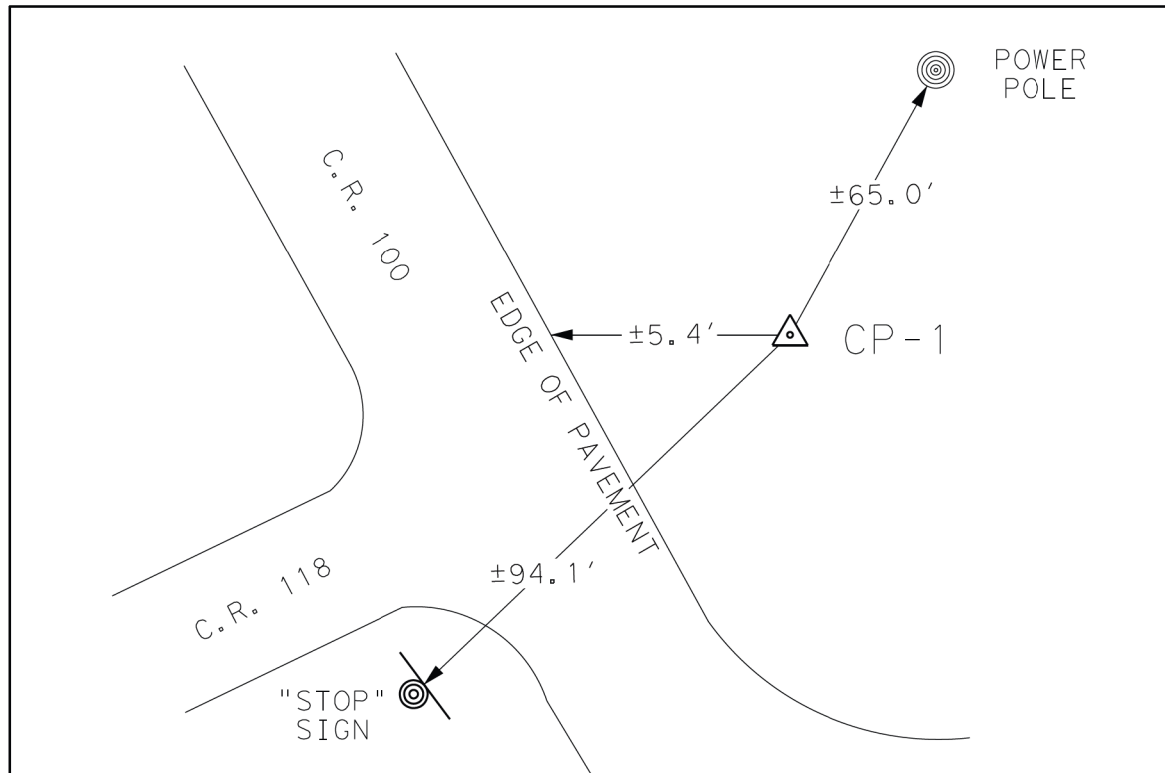
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Survey Date: APRIL, 2020



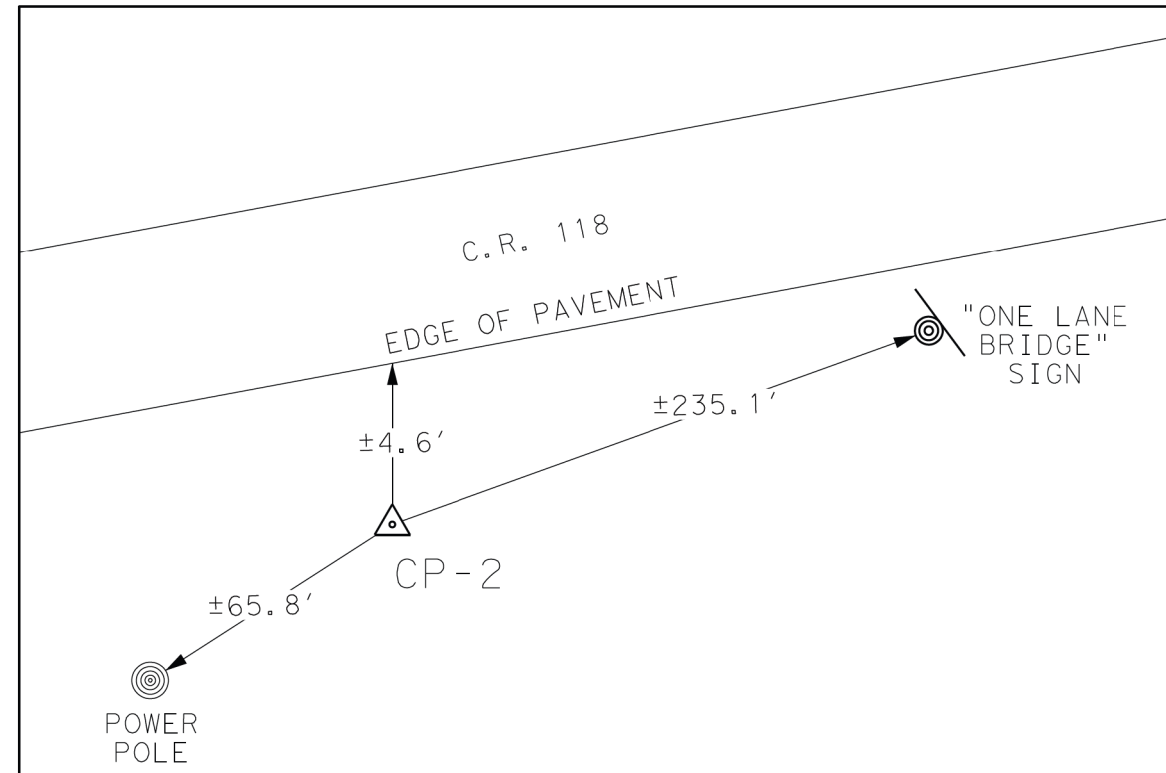
**SURVEY CONTROL
LAYOUT SHEET**

| | | | | |
|---------------------|----------|--|---------------------|--|
| FHWA TEXAS DIVISION | | FEDERAL AID PROJECT NO. BR2020(733), ETC. | SHEET NO. 1 OF 2 | |
| STATE | DISTRICT | COUNTY | | |
| TEXAS | AUS | WILLIAMSON | | |
| CONTROL | SECTION | JOB | HIGHWAY NO. | |
| 0914 | 05 | 204, ETC. | CR 118 | |



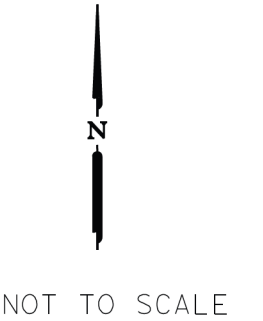
CP-1 IS A 5/8" IRON ROD WITH A TXDOT ALUMINUM CAP SET IN CONCRETE LOCATED IN THE EAST RIGHT-OF-WAY OF C.R. 100 ± 65.0 FEET SOUTHWEST OF A POWER POLE, ± 5.4 FEET EAST OF THE EDGE OF PAVEMENT OF C.R. 100 AND ± 94.1 FEET NORTHEAST OF A "STOP" SIGN.

| SURFACE COORDINATES | | GRID COORDINATES | |
|---------------------|-----------------|------------------|-----------------|
| N | = 10,187,411.25 | N | = 10,186,170.38 |
| E | = 3,167,594.01 | E | = 3,167,208.18 |
| ELEV | = 712.13' | ELEV | = 712.13' |



CP-2 IS A 5/8" IRON ROD WITH A TXDOT ALUMINUM CAP SET IN CONCRETE LOCATED IN THE SOUTH RIGHT-OF-WAY OF C.R. 118 ± 65.8 FEET NORTHEAST OF A POWER POLE, ± 4.6 FEET SOUTH OF THE EDGE OF PAVEMENT OF C.R. 118 AND ± 235.1 FEET SOUTHWEST OF A "ONE LANE BRIDGE" SIGN.

| SURFACE COORDINATES | | GRID COORDINATES | |
|---------------------|-----------------|------------------|-----------------|
| N | = 10,187,042.14 | N | = 10,185,801.31 |
| E | = 3,166,831.31 | E | = 3,166,445.57 |
| ELEV | = 723.17' | ELEV | = 723.17' |



NOTES:

- ALL PROJECT COORDINATES ARE BASED ON THE TEXAS STATE PLANE COORDINATE SYSTEM, CENTRAL ZONE, NAD 83 (2011)/NAVD 88, GEOID MODEL 12B. ALL COORDINATES SHOWN HEREON ARE ADJUSTED TO SURFACE BY MULTIPLYING BY A COMBINED ADJUSTMENT FACTOR OF 1.0001218190. UNITS: U.S. SURVEY FEET.
- CONTROL POINTS SHOWN HEREON ARE BASED UPON THE TXDOT VRS NETWORK. ELEVATIONS ARE DERIVED FROM REPITIVE VRS OBSERVATIONS.
- A SITE CALIBRATION SHOULD BE PERFORMED WHEN UTILIZING THE CONTROL SHOWN HEREIN.



Scott C. Brashear
5/1/20

Survey Date: APRIL, 2020

SAM 4801 Southwest Parkway
Building Two, Suite 100
Austin, Texas 78735
(512) 447-0575
Fax: (512) 326-3029
Texas Firm Registration No. 10064300

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SURVEY CONTROL INDEX SHEET

| | | | |
|---------------------|-----------------|--|-----------------------|
| FHWA TEXAS DIVISION | | FEDERAL AID PROJECT NO. BR2020(733), ETC. | SHEET NO. 2 OF 2 |
| STATE TEXAS | DISTRICT AUS | COUNTY WILLIAMSON | |
| CONTROL 0914 | SECTION 05 | JOB 204, ETC. | HIGHWAY NO. CR 118 |

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 PLOTTED: 11/3/2022 12:48:10 PM

CR 118

Beginning chain CL CR 118 description
 Feature: Road_Centerline

Point 23 N 10,186,784.4058 E 3,166,135.6762 Sta 100+00.00
 Course from 23 to PC CR118 1 N 68° 33' 26.52" E Dist 309.5913

Curve Data

Curve CR118 1
 P.I. Station 105+72.22 N 10,186,993.5904 E 3,166,668.2855
 Delta = 22° 20' 24.32" (RT)
 Degree = 4° 18' 28.63"
 Tangent = 262.6245
 Length = 518.5778
 Radius = 1,330.0000
 External = 25.6812
 Long Chord = 515.2991
 Mid. Ord. = 25.1948
 P.C. Station 103+09.59 N 10,186,897.5829 E 3,166,423.8388
 P.T. Station 108+28.17 N 10,186,989.4769 E 3,166,930.8778
 C.C. = N 10,185,659.6401 E 3,166,910.0462
 Back = N 68° 33' 26.52" E
 Ahead = S 89° 06' 09.16" E
 Chord Bear = N 79° 43' 38.68" E

Course from PT CR118 1 to PC CR118 2 S 89° 06' 09.16" E Dist 712.3016

Curve Data

Curve CR118 2
 P.I. Station 118+09.19 N 10,186,974.1113 E 3,167,911.7773
 Delta = 22° 50' 41.88" (LT)
 Degree = 4° 18' 28.63"
 Tangent = 268.7182
 Length = 530.2974
 Radius = 1,330.0000
 External = 26.8749
 Long Chord = 526.7917
 Mid. Ord. = 26.3426
 P.C. Station 115+40.47 N 10,186,978.3202 E 3,167,643.0920
 P.T. Station 120+70.77 N 10,187,074.5466 E 3,168,161.0206
 C.C. = N 10,188,308.1570 E 3,167,663.9237
 Back = S 89° 06' 09.16" E
 Ahead = N 68° 03' 08.96" E
 Chord Bear = N 79° 28' 29.90" E

Course from PT CR118 2 to 24 N 68° 03' 08.96" E Dist 1,038.7625
 Point 24 N 10,187,462.7914 E 3,169,124.5005 Sta 131+09.53

Ending chain CL CR 118 description

EXIST CR 118

Beginning chain CL EXIST CR118 description

Point 81 N 10,186,784.4058 E 3,166,135.6762 Sta 400+00.00
 Course from 81 to 82 N 68° 33' 26.52" E Dist 1,573.4452
 Point 82 N 10,187,359.6093 E 3,167,600.2139 Sta 415+73.45

Ending chain CL EXIST CR118 description

CR 100

Beginning chain CL CR 100 description
 Feature: Road_Centerline

Point 67 N 10,187,076.6916 E 3,168,166.3438 Sta 200+00.00
 Course from 67 to PC CL CR100 1 N 21° 56' 51.04" W Dist 14.8614

Curve Data

Curve CR 100 1
 P.I. Station 200+36.97 N 10,187,110.9834 E 3,168,152.5255
 Delta = 47° 42' 34.78" (LT)
 Degree = 114° 35' 29.61"
 Tangent = 22.1098
 Length = 41.6345
 Radius = 50.0000
 External = 4.6703
 Long Chord = 40.4421
 Mid. Ord. = 4.2713
 P.C. Station 200+14.86 N 10,187,090.4760 E 3,168,160.7892
 P.T. Station 200+56.50 N 10,187,118.6696 E 3,168,131.7947
 C.C. = N 10,187,071.7881 E 3,168,114.4129
 Back = N 21° 56' 51.04" W
 Ahead = N 69° 39' 25.81" W
 Chord Bear = N 45° 48' 08.43" W

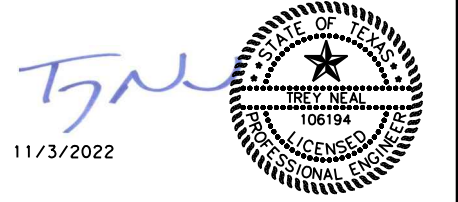
Course from PT CL CR100 1 to PC CL CR100 2 N 69° 39' 25.81" W Dist 457.4966

Curve Data

Curve CR 100 2
 P.I. Station 206+02.41 N 10,187,308.4987 E 3,167,619.9455
 Delta = 45° 40' 00.17" (RT)
 Degree = 27° 17' 01.34"
 Tangent = 88.4198
 Length = 167.3772
 Radius = 210.0000
 External = 17.8553
 Long Chord = 162.9819
 Mid. Ord. = 16.4561
 P.C. Station 205+13.99 N 10,187,277.7122 E 3,167,702.8325
 P.T. Station 206+81.37 N 10,187,389.3011 E 3,167,584.0427
 C.C. = N 10,187,474.5716 E 3,167,775.9514
 Back = N 69° 39' 25.81" W
 Ahead = N 23° 57' 24.92" W
 Chord Bear = N 46° 47' 25.00" W

Course from PT CL CR100 2 to 68 N 23° 57' 24.92" W Dist 61.6543
 Point 68 N 10,187,445.6440 E 3,167,559.0080 Sta 207+43.02

Ending chain CL CR 100 description

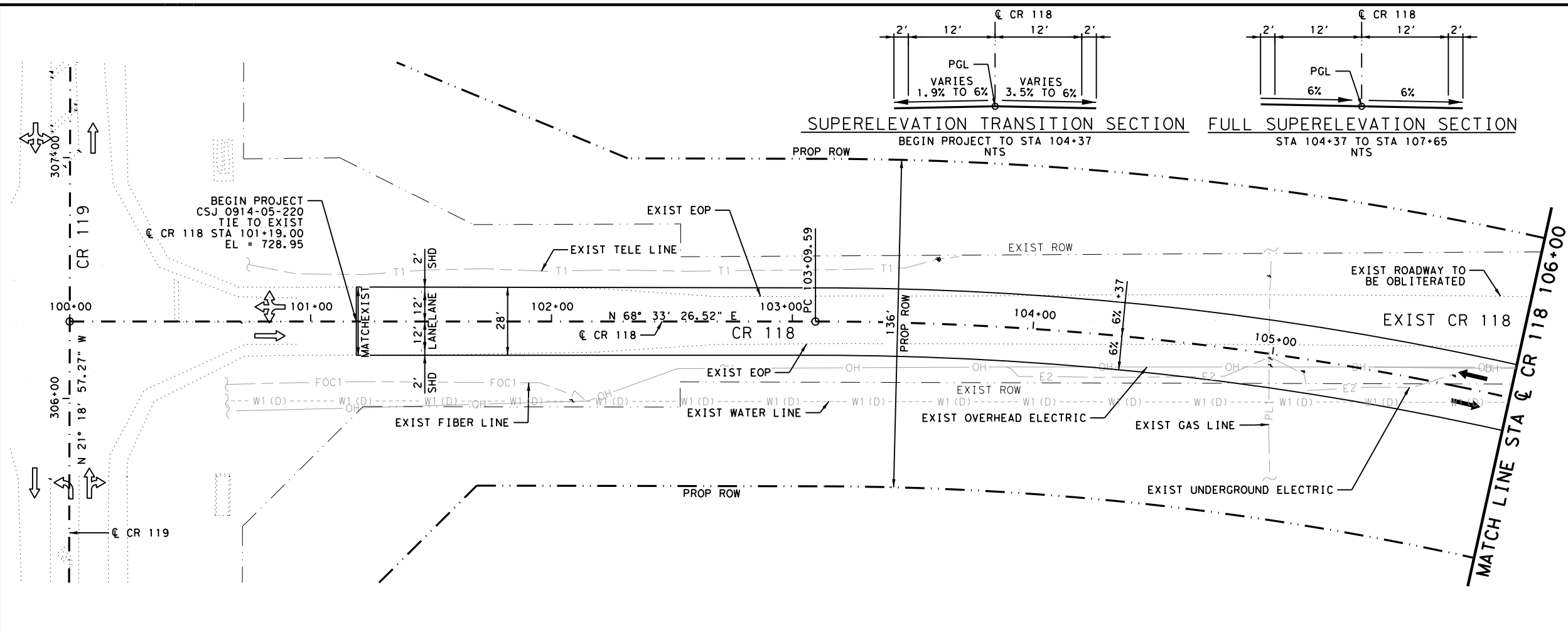


CR 118 AT COTTONWOOD CREEK
 HORIZONTAL ALIGNMENT
 DATA

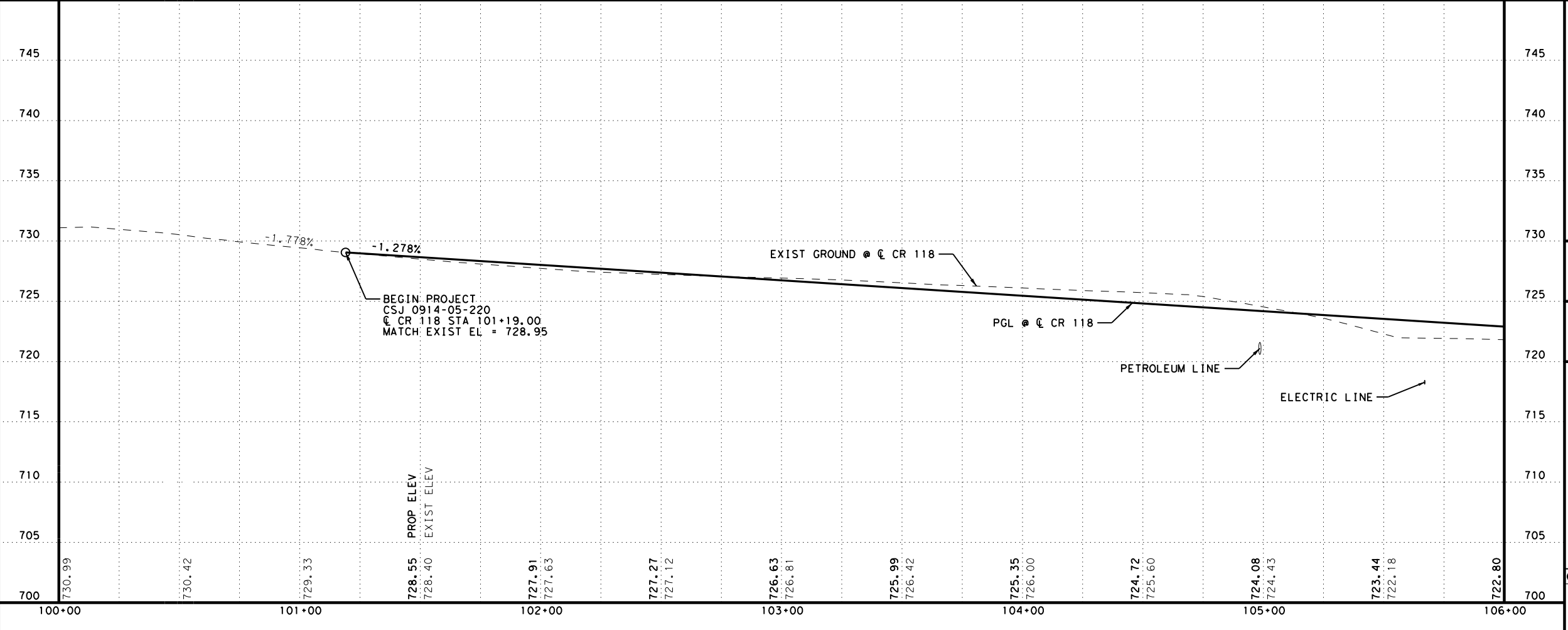
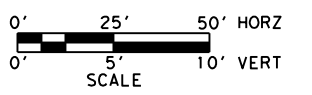
SHEET 1 OF 1

| | | | | |
|-------------|------|------|------------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | | COUNTY | SHEET NO. |
| | AUS | | WILLIAMSON | 29 |

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NOTES:
 1. REFER TO EROSION CONTROL LAYOUT AND REMOVAL LAYOUT FOR MORE INFORMATION ON GRADING OBLITERATED ROAD.



TJN
 11/3/2022
 STATE OF TEXAS
 TROY NEAL
 106194
 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

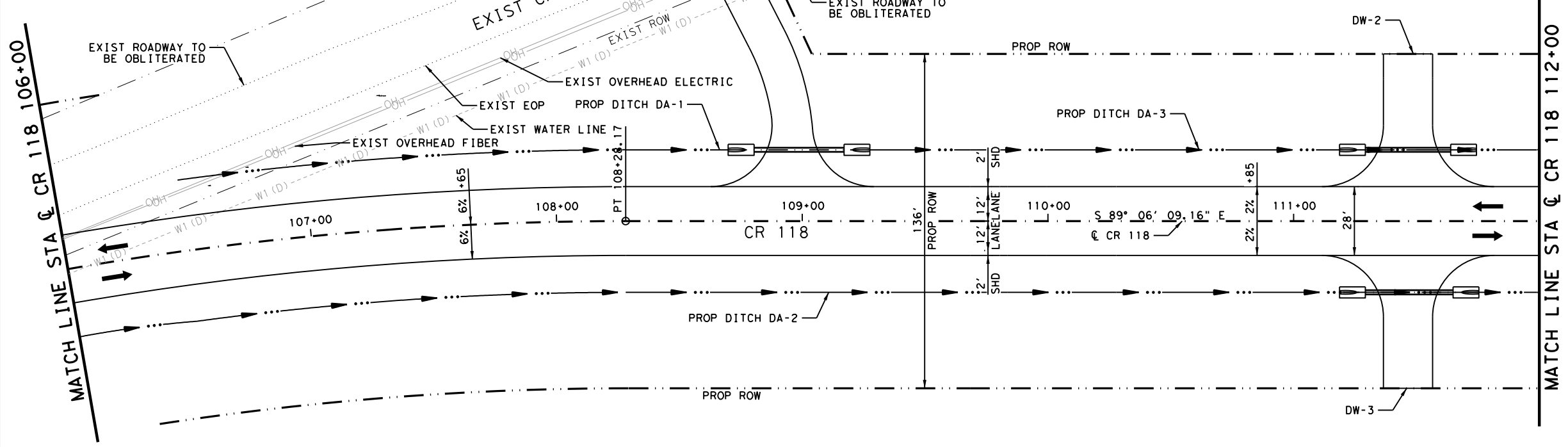
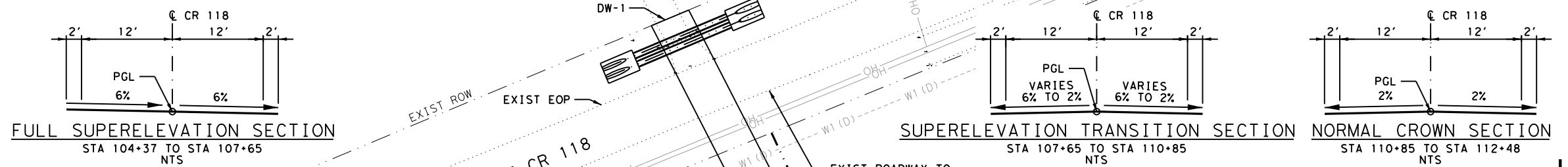
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 Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
 PLAN AND PROFILE

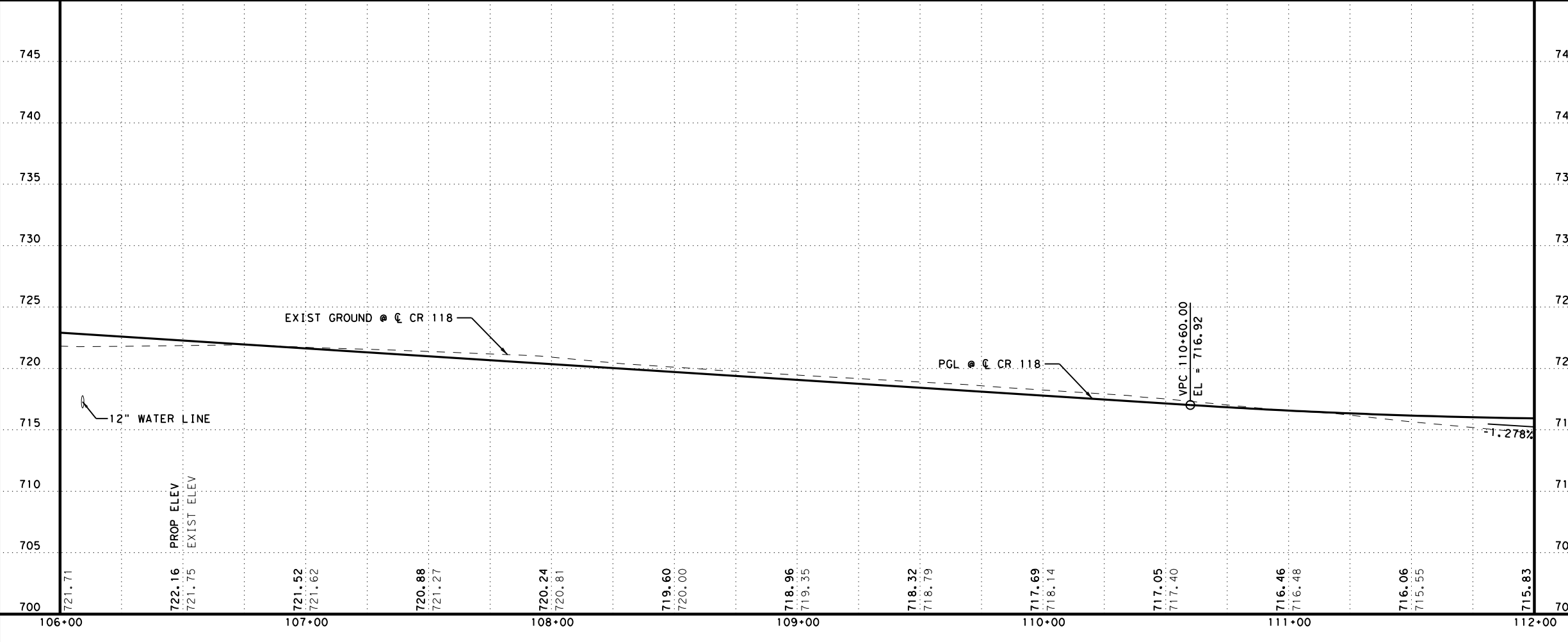
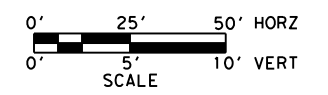
SHEET 1 OF 5


| | | | |
|------|------------|-----------|-----------|
| CONT | SECT | JOB | HIGHWAY |
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | | SHEET NO. |
| AUS | WILLIAMSON | | 30 |

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NOTES:
 1. REFER TO EROSION CONTROL LAYOUT AND REMOVAL LAYOUT FOR MORE INFORMATION ON GRADING OBLITERATED ROAD.





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 11/3/2022



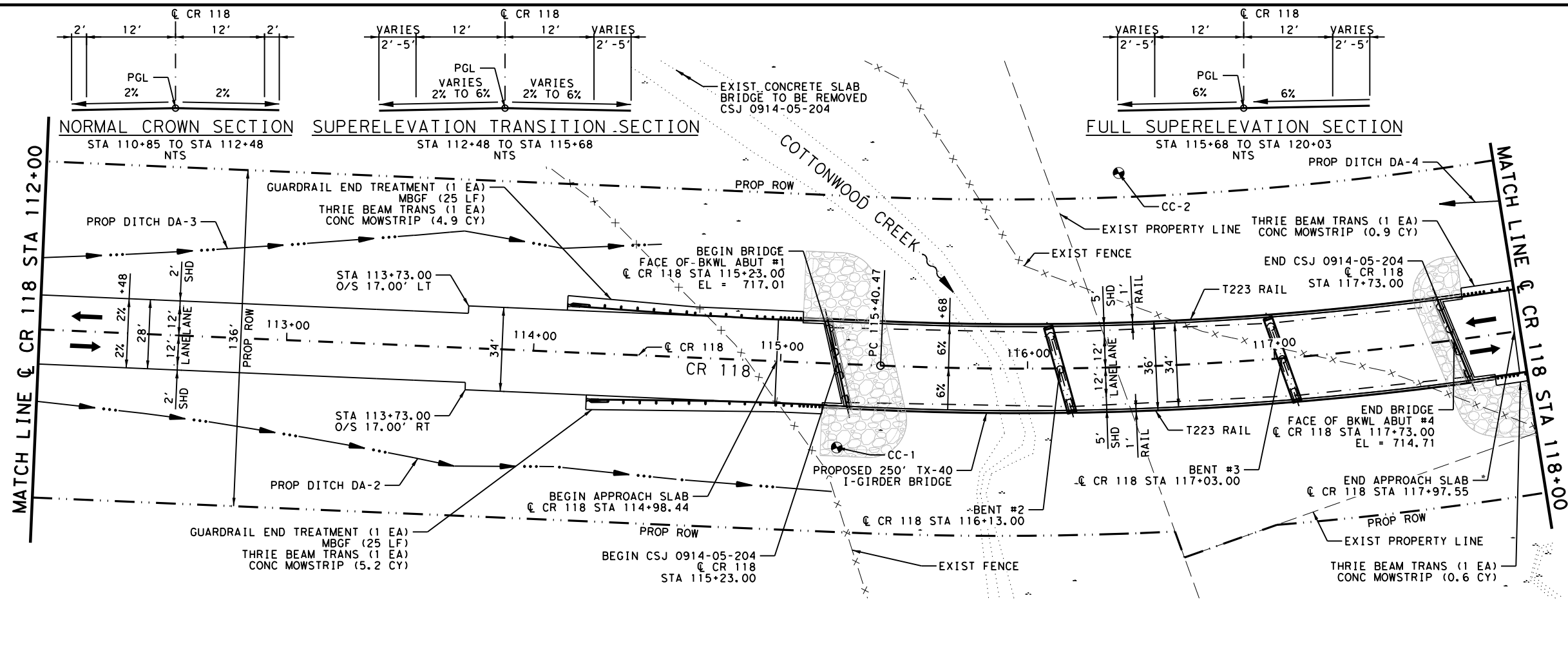
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 Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK

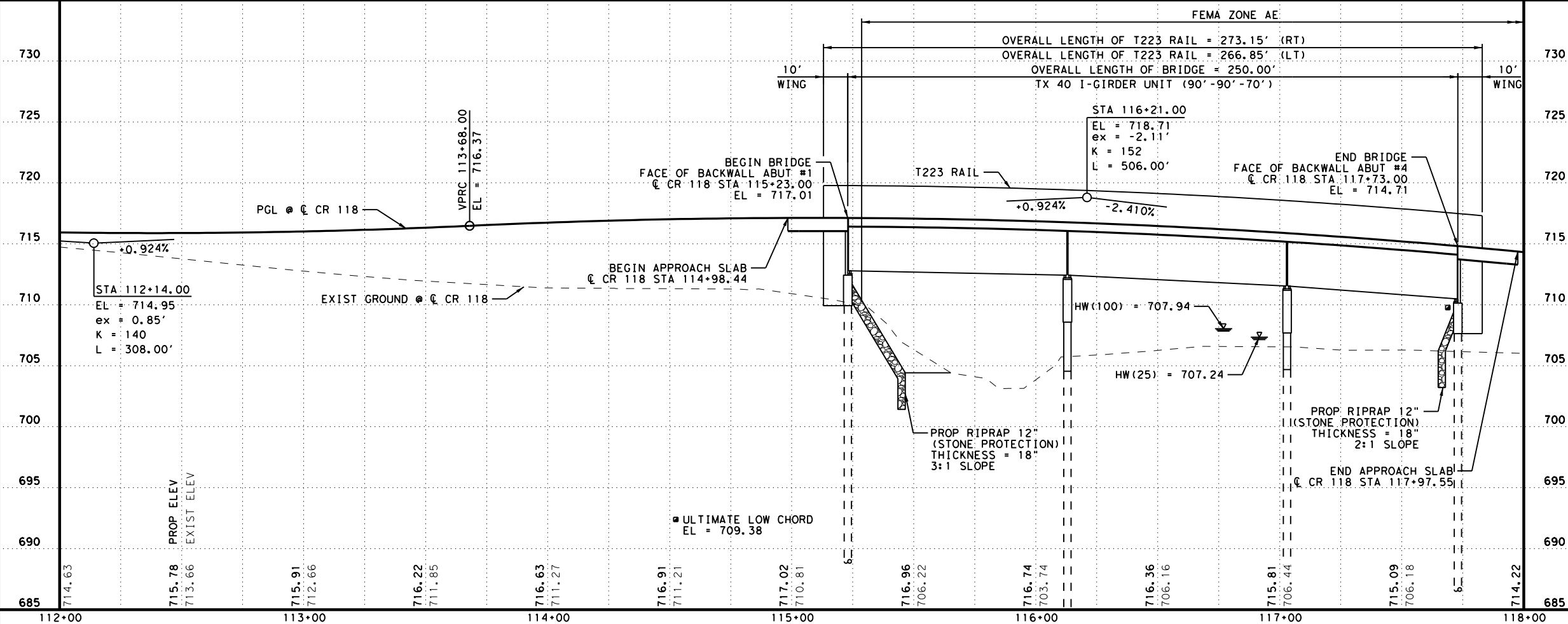
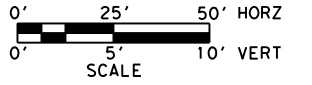
PLAN AND PROFILE

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|--------------|----------------|------|-----------|-----------|
| SHEET 2 OF 5 | | | | |
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST COUNTY | | | SHEET NO. |
| | AUS WILLIAMSON | | | 31 |

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- NOTES:**
- 1. ALL EXISTING FENCE WITHIN THE 136' WIDE PROPOSED RIGHT OF WAY CORRIDOR TO BE REMOVED, SUBSIDIARY TO ITEM 100 PREPARING ROW.
 - 2. REFER TO EROSION CONTROL LAYOUT AND REMOVAL LAYOUT FOR MORE INFORMATION ON REMOVING THE EXISTING BRIDGE STRUCTURE.



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TREY NEAL
106194
LICENSED PROFESSIONAL ENGINEER

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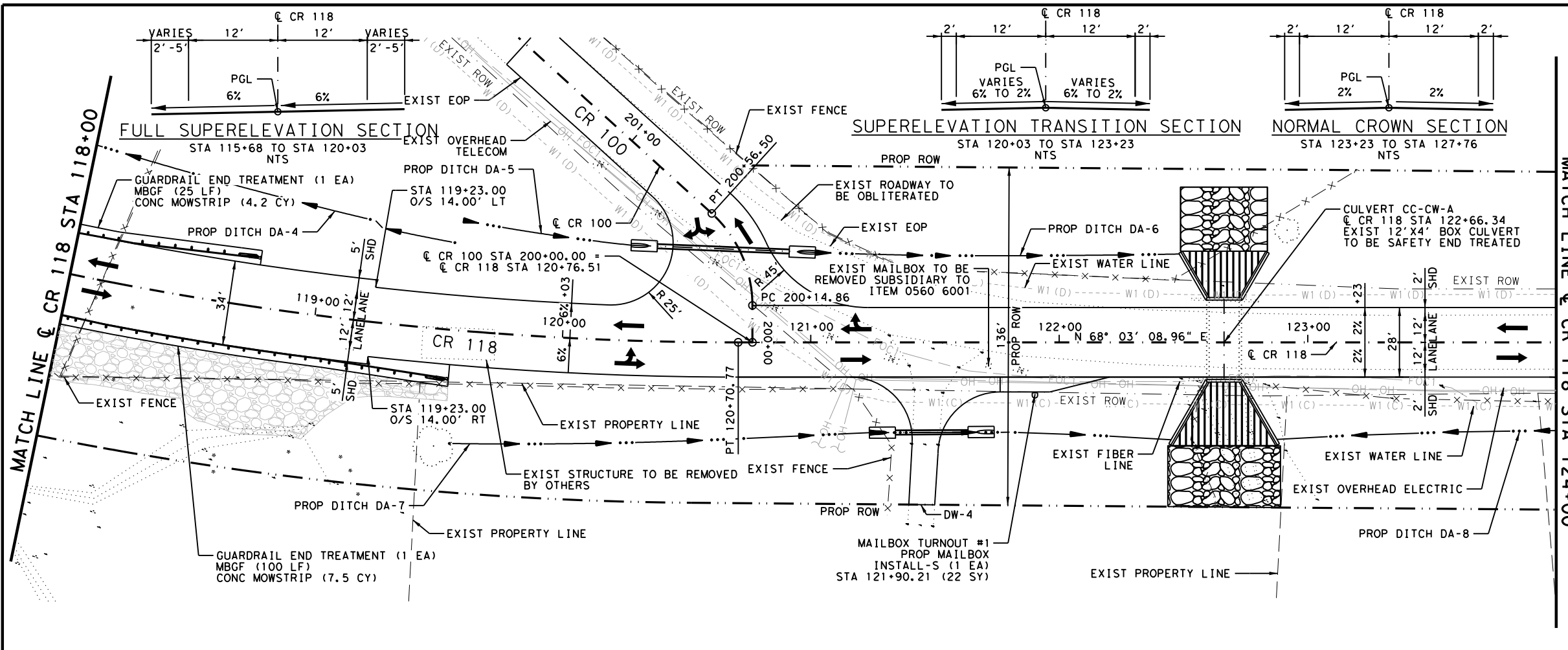
CR 118 AT COTTONWOOD CREEK

PLAN AND PROFILE

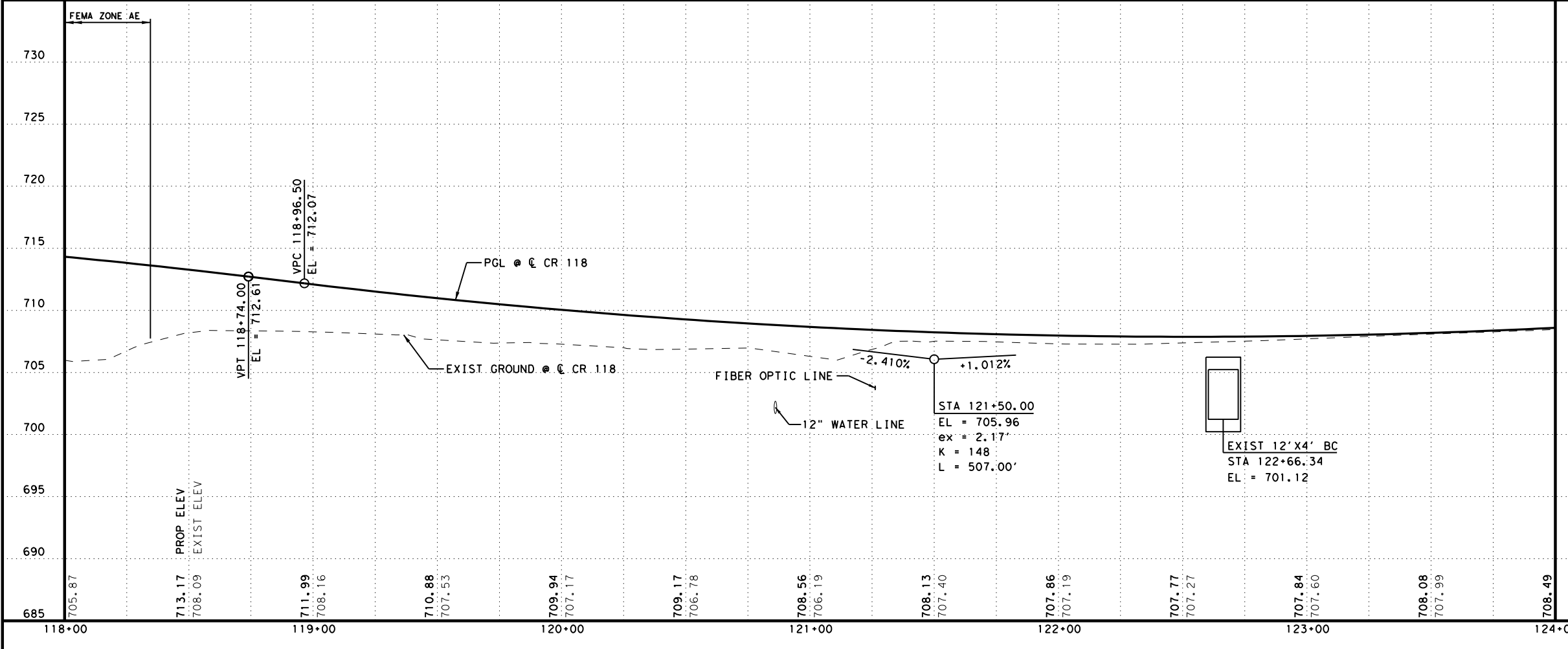
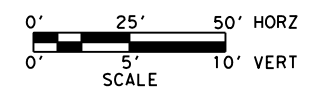
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| CONT | SECT | JOB | HIGHWAY |
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| 0914 | 05 | 204, ETC. | CR 118 |
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- NOTES:**
1. SEE INTERSECTION PLAN AND PROFILE SHEET FOR MORE INFORMATION.
 2. ALL EXISTING FENCE LINES WITHIN THE 136' WIDE PROPOSED RIGHT OF WAY CORRIDOR TO BE REMOVED, SUBSIDIARY TO ITEM 100 PREPARING ROW.



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 11/3/2022

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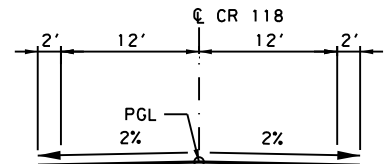
Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
 PLAN AND PROFILE

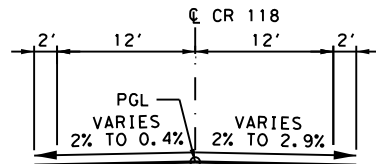
SHEET 4 OF 5

| | | | | |
|-------------|------|------------|-----------|-----------|
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| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 33 |

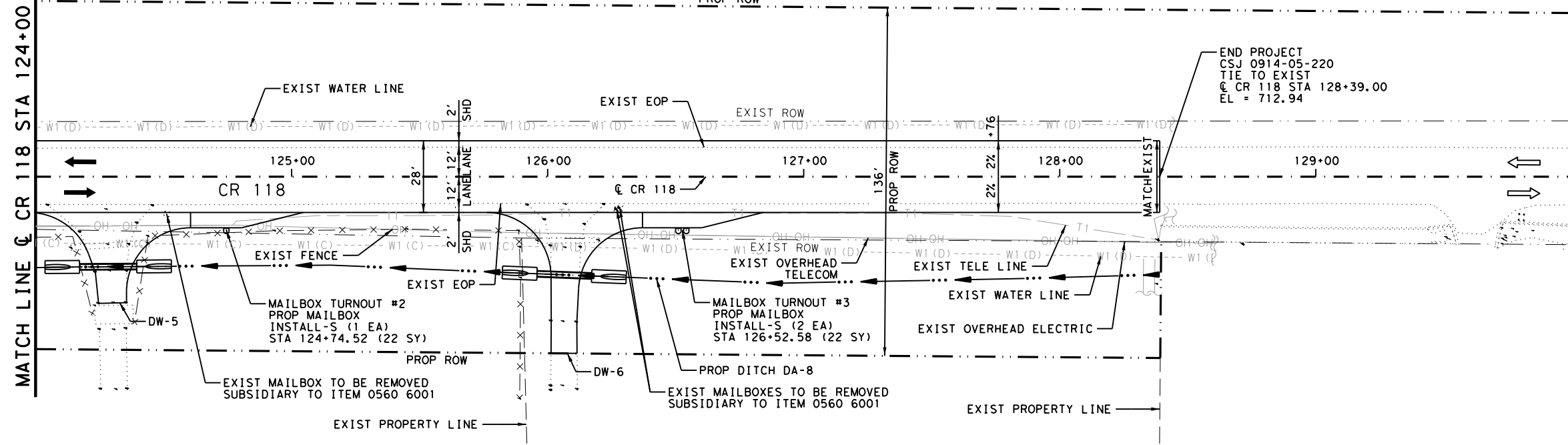
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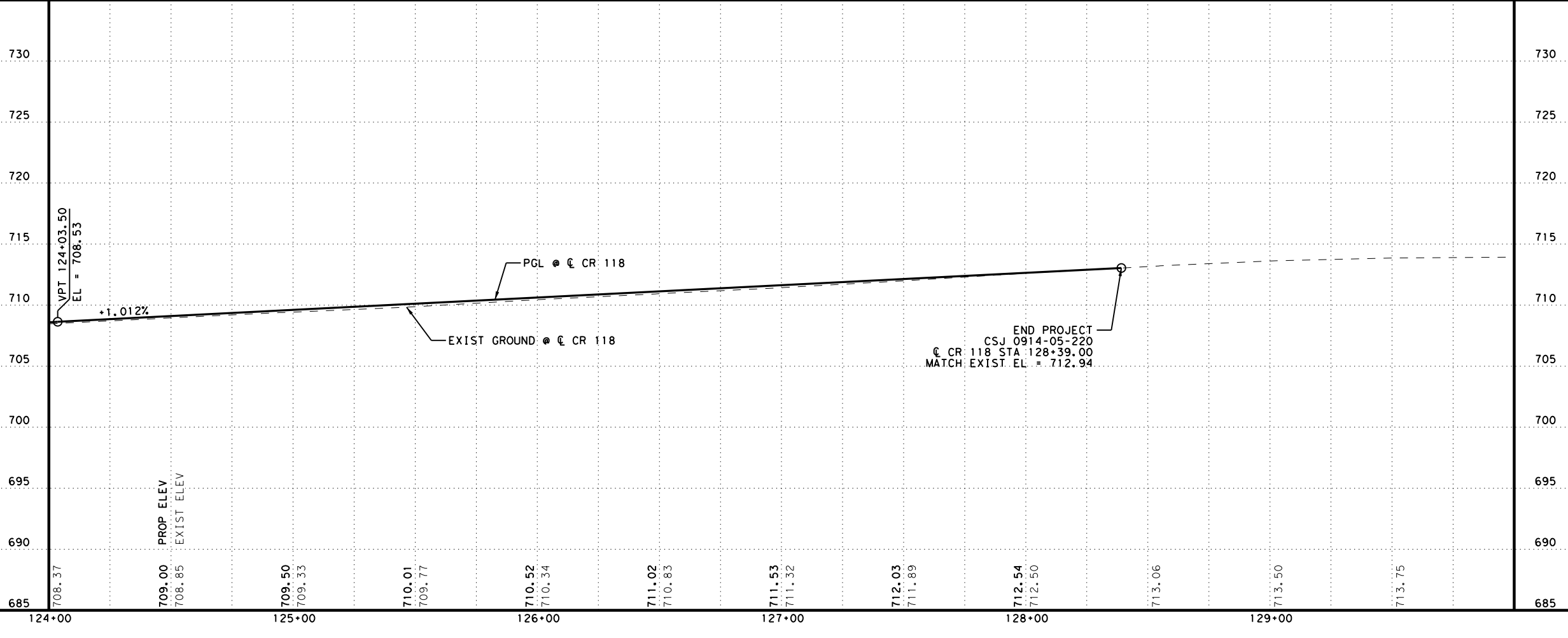
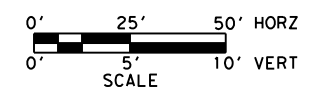
NORMAL CROWN SECTION
 STA 120+03 TO STA 127+76
 NTS



CROSS SLOPE TRANSITION SECTION
 STA 127+76 TO END PROJECT
 NTS



- NOTES:**
- ALL EXISTING FENCE LINES WITHIN THE 136' WIDE PROPOSED RIGHT OF WAY CORRIDOR TO BE REMOVED, SUBSIDIARY TO ITEM 100 PREPARING ROW.



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 11/3/2022



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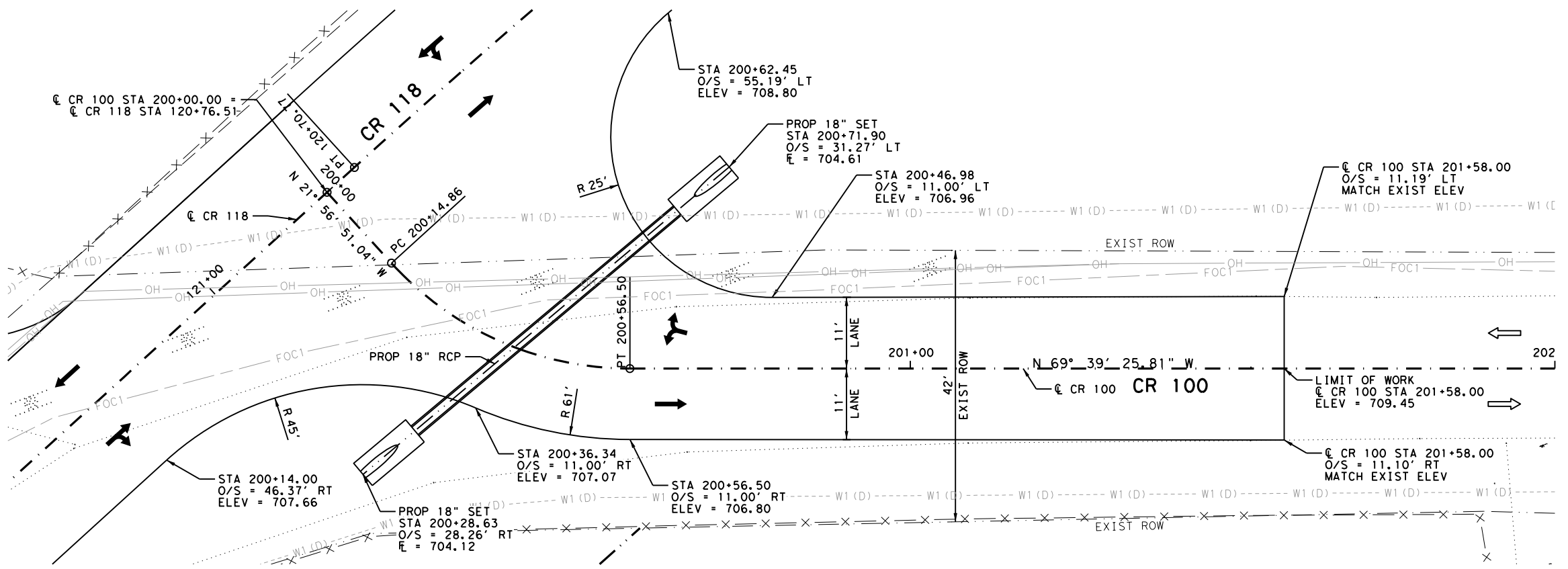
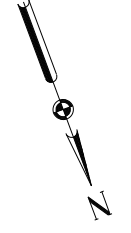
CR 118 AT COTTONWOOD CREEK

PLAN AND PROFILE

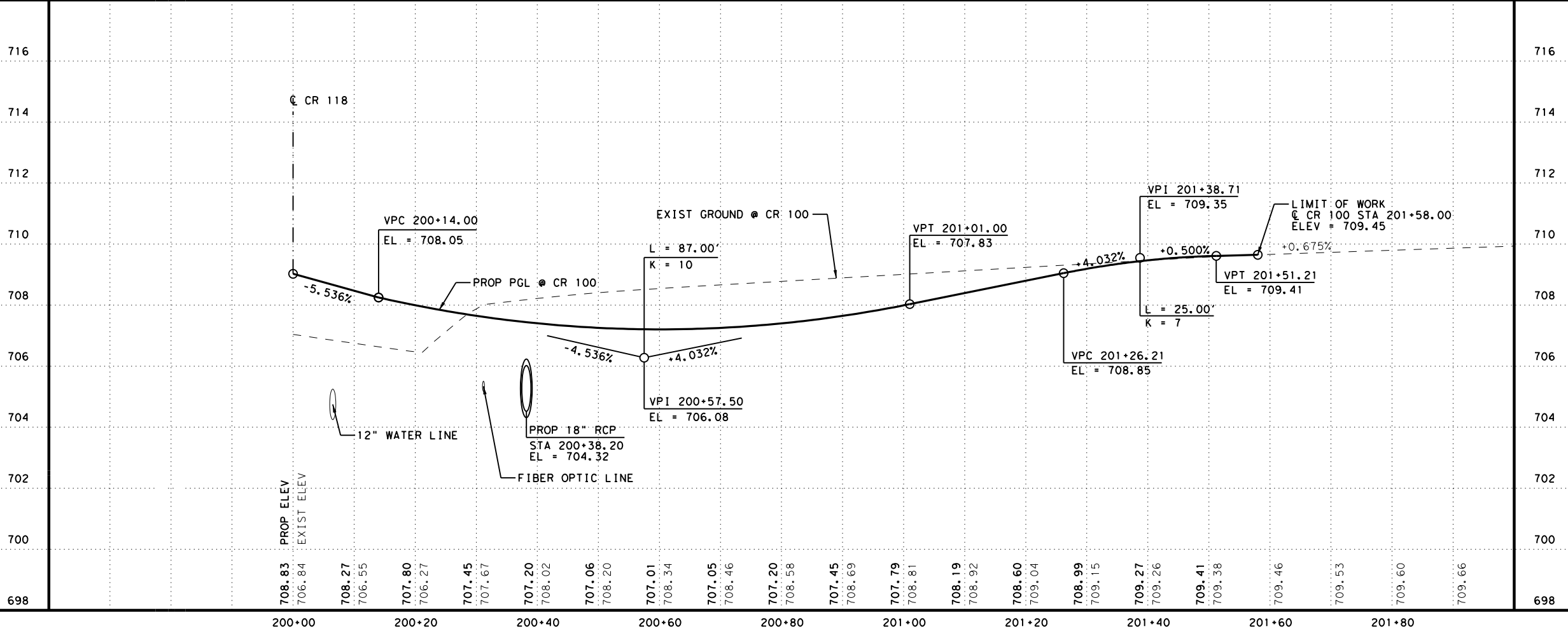
SHEET 5 OF 5

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 34 |

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INSTALL:
 RC PIPE (CL III) (18") (54 LF)
 SET (TY II) (6:1) (18") (2 EA)



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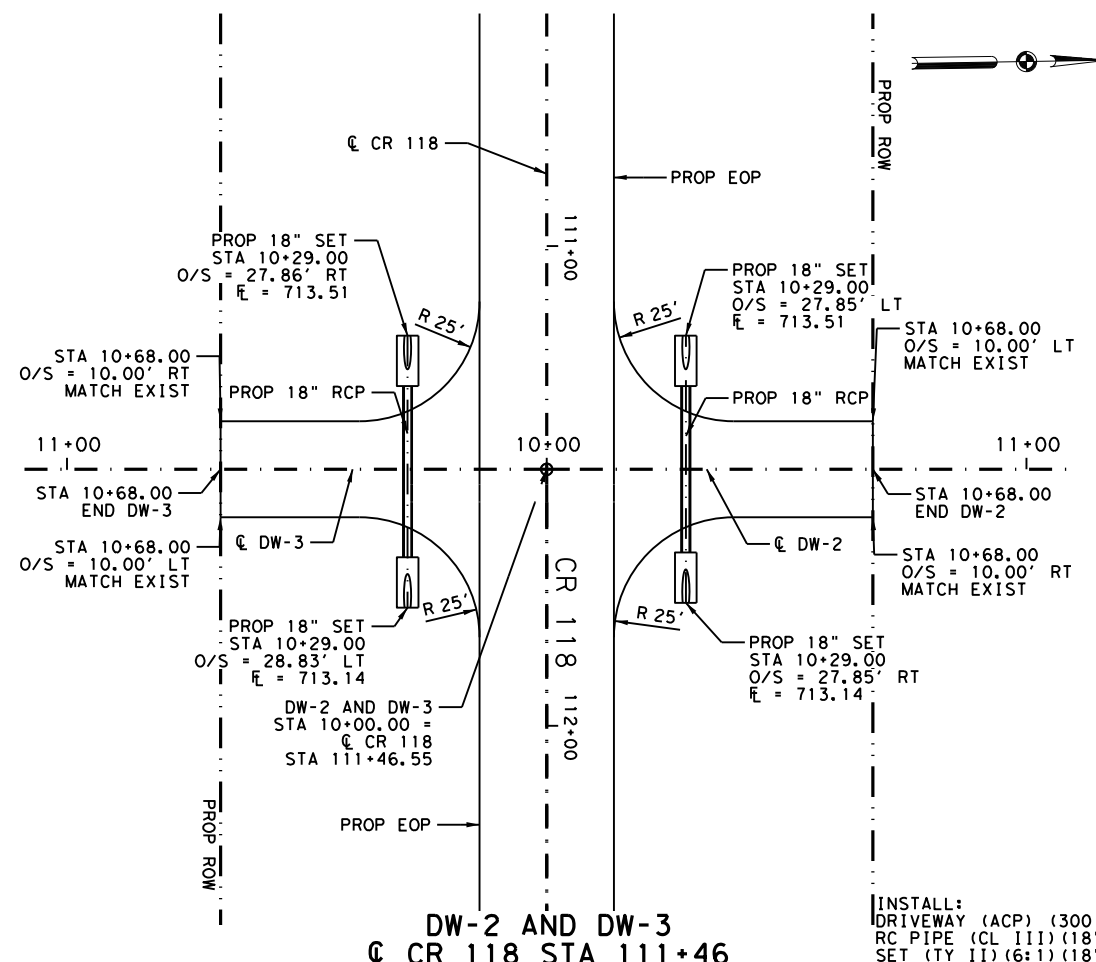
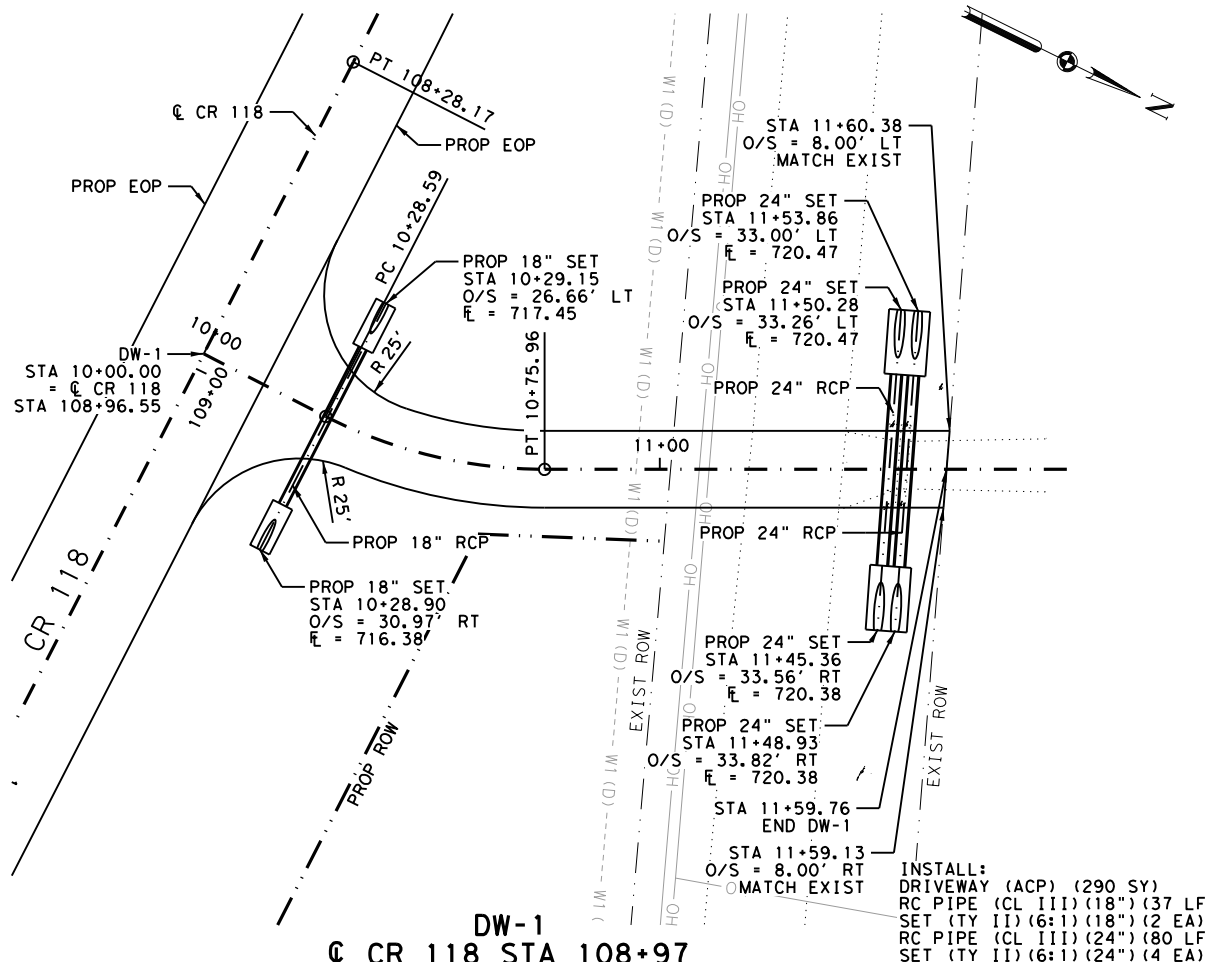
CR 118 AT COTTONWOOD CREEK

INTERSECTION
 PLAN AND PROFILE

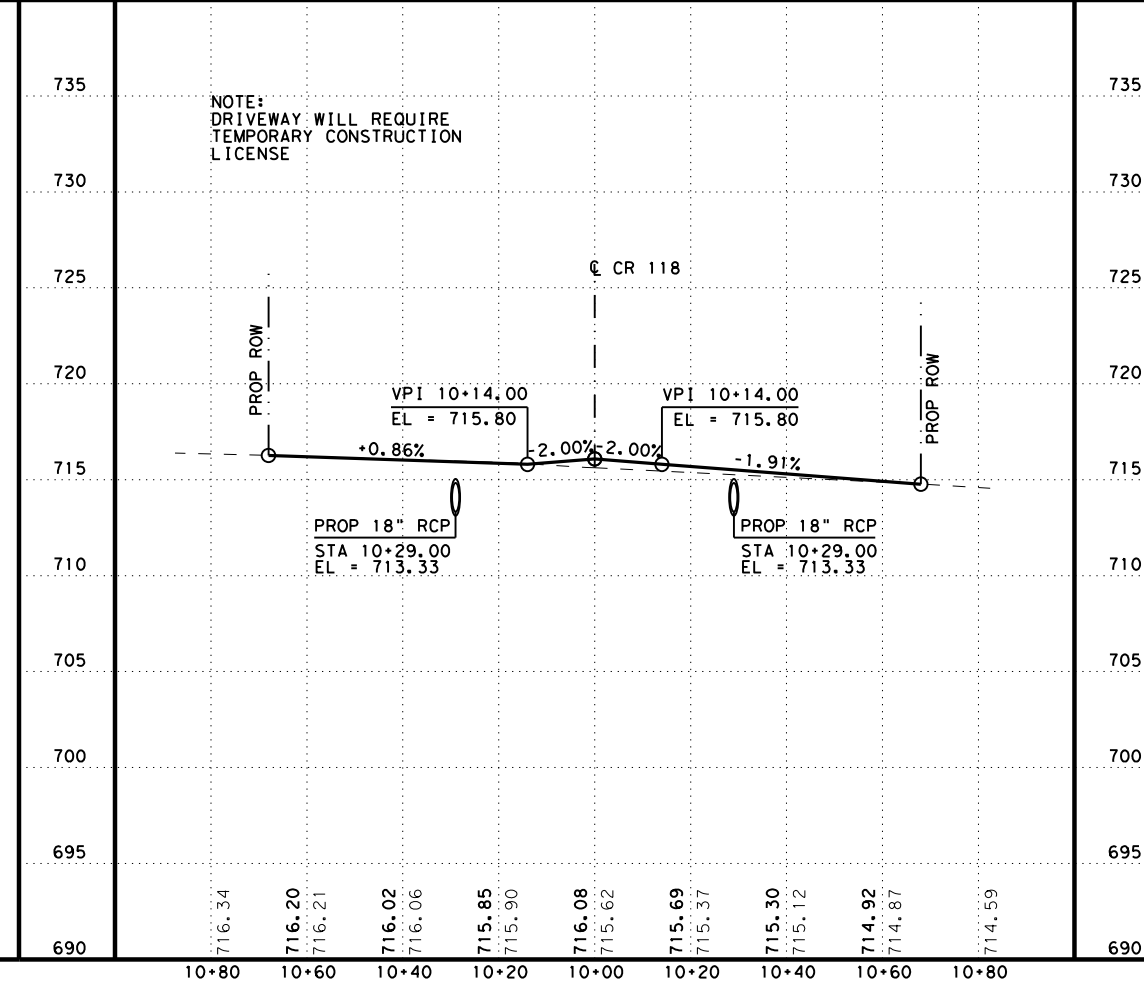
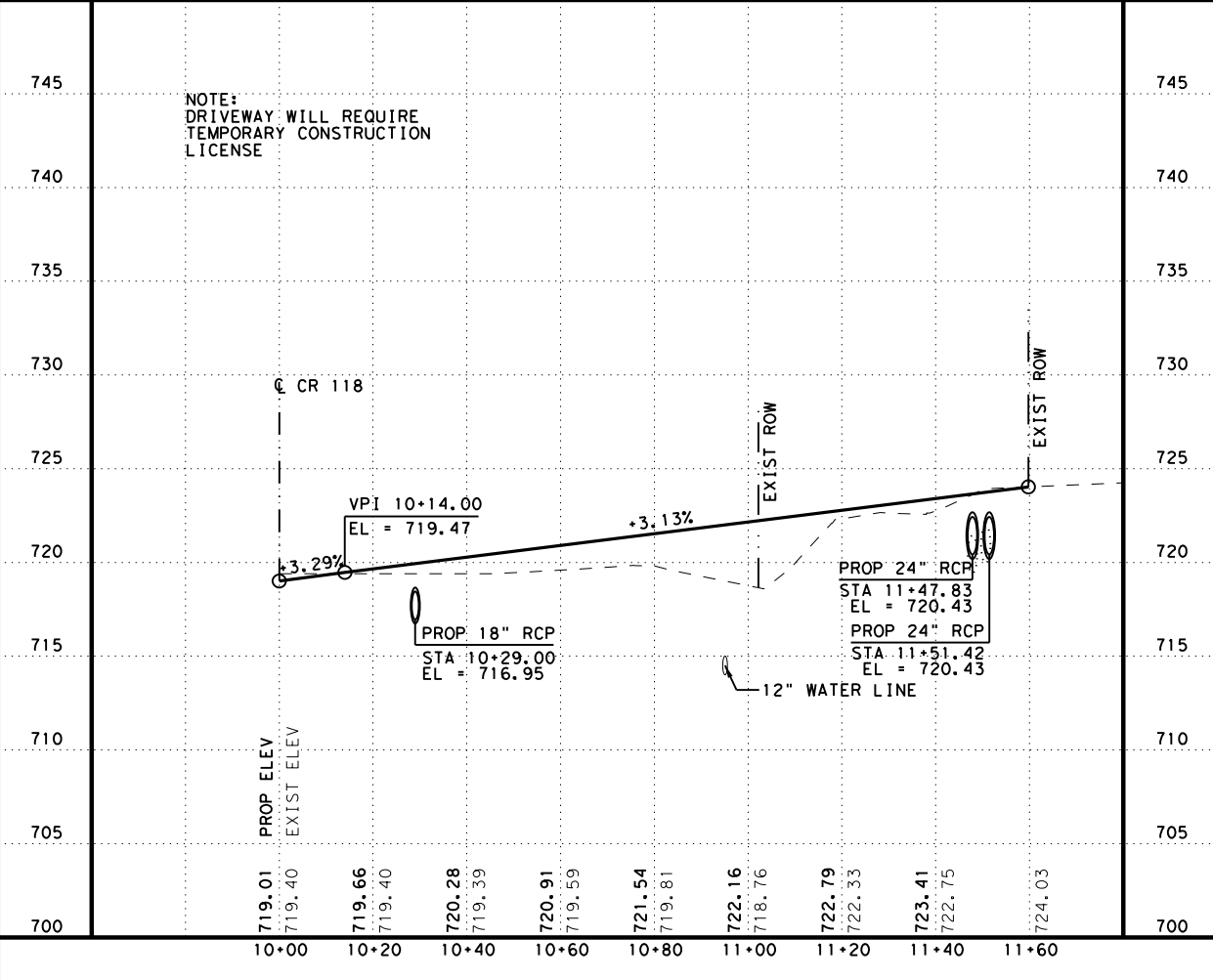
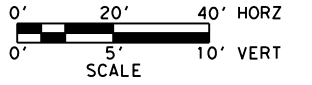
SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 35 |

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NOTES:
 1. REFER TO REMOVAL SHEETS FOR PIPE REMOVAL INFORMATION.



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 11/4/2022

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Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK

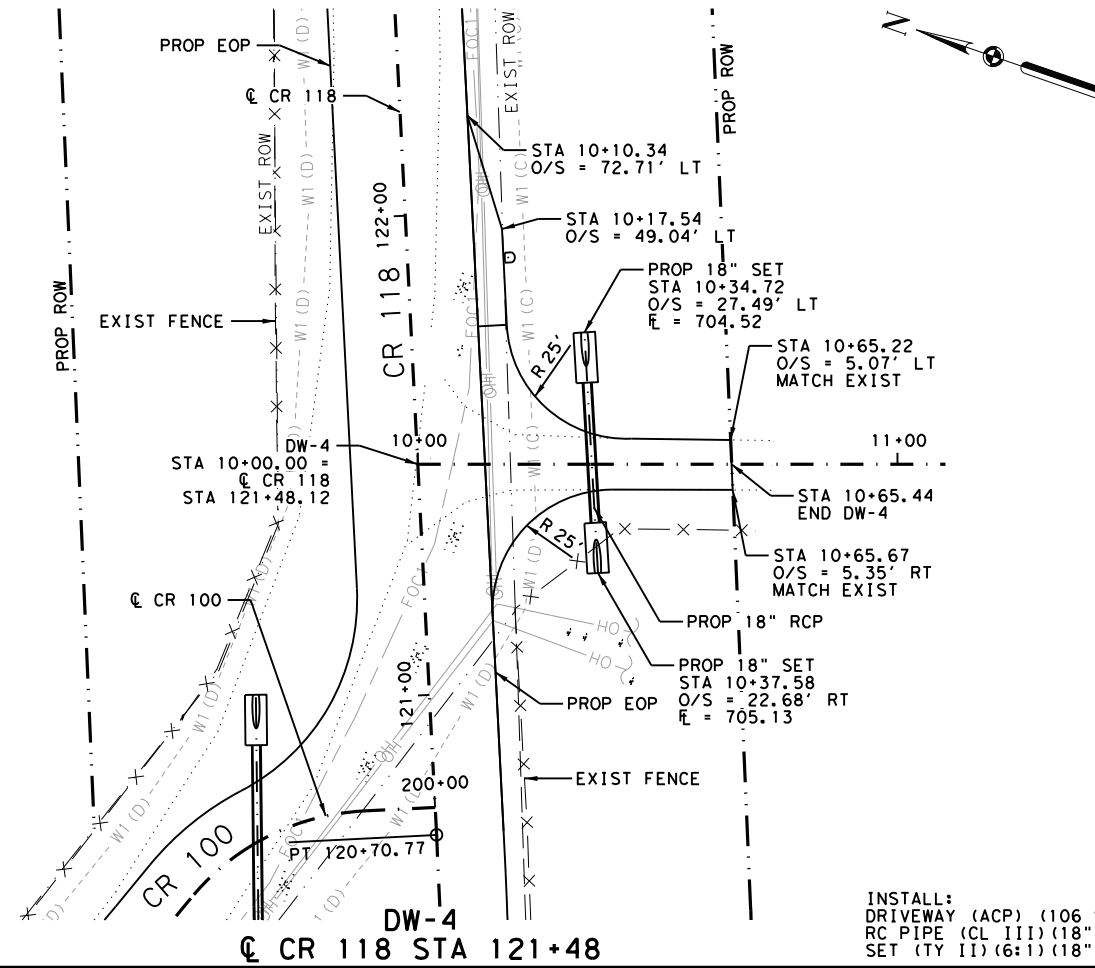
DRIVEWAY
 PLAN AND PROFILE

SHEET 1 OF 3

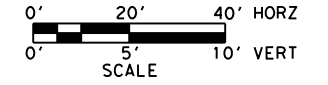
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| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
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| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 36 |

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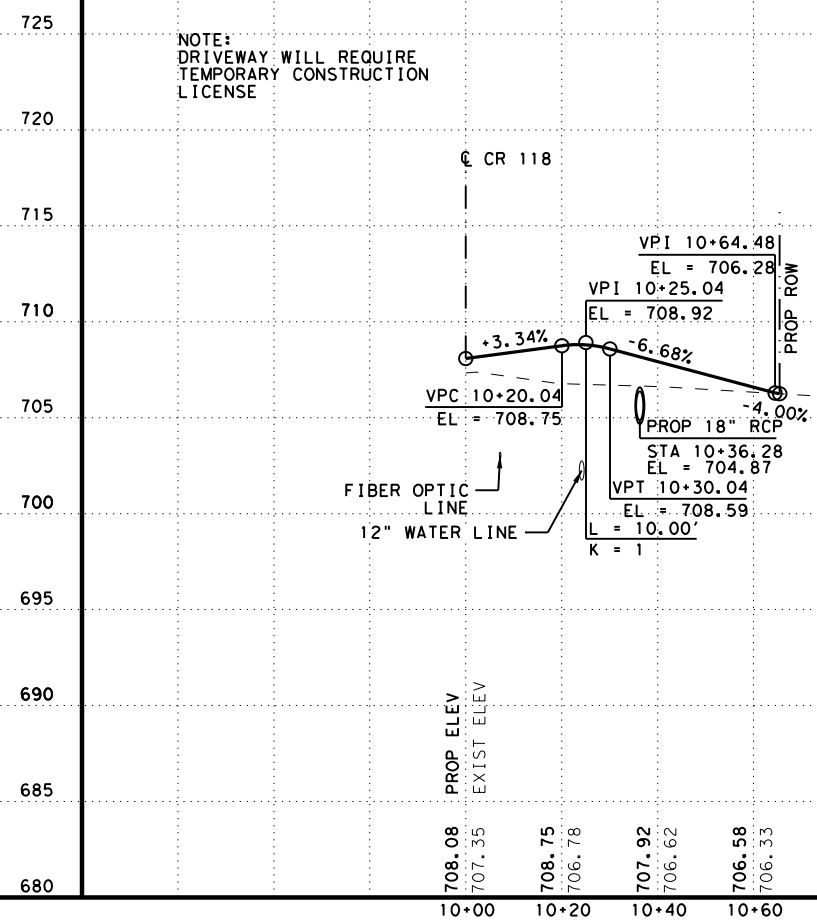
SSEE DRIVEWAY PLAN AND PROFILE SHEET 1 OF 3
 FOR MORE INFORMATION ON DRIVEWAY 3



INSTALL:
 DRIVEWAY (ACP) (106 SY)
 RC PIPE (CL III) (18") (30 LF)
 SET (TY II) (6:1) (18") (2 EA)



SSEE DRIVEWAY PLAN AND PROFILE SHEET 1 OF 3
 FOR MORE INFORMATION ON DRIVEWAY 3



NOTE:
 DRIVEWAY WILL REQUIRE
 TEMPORARY CONSTRUCTION
 LICENSE

TJN
 11/3/2022

 TREY NEAL
 106194
 LICENSED PROFESSIONAL ENGINEER

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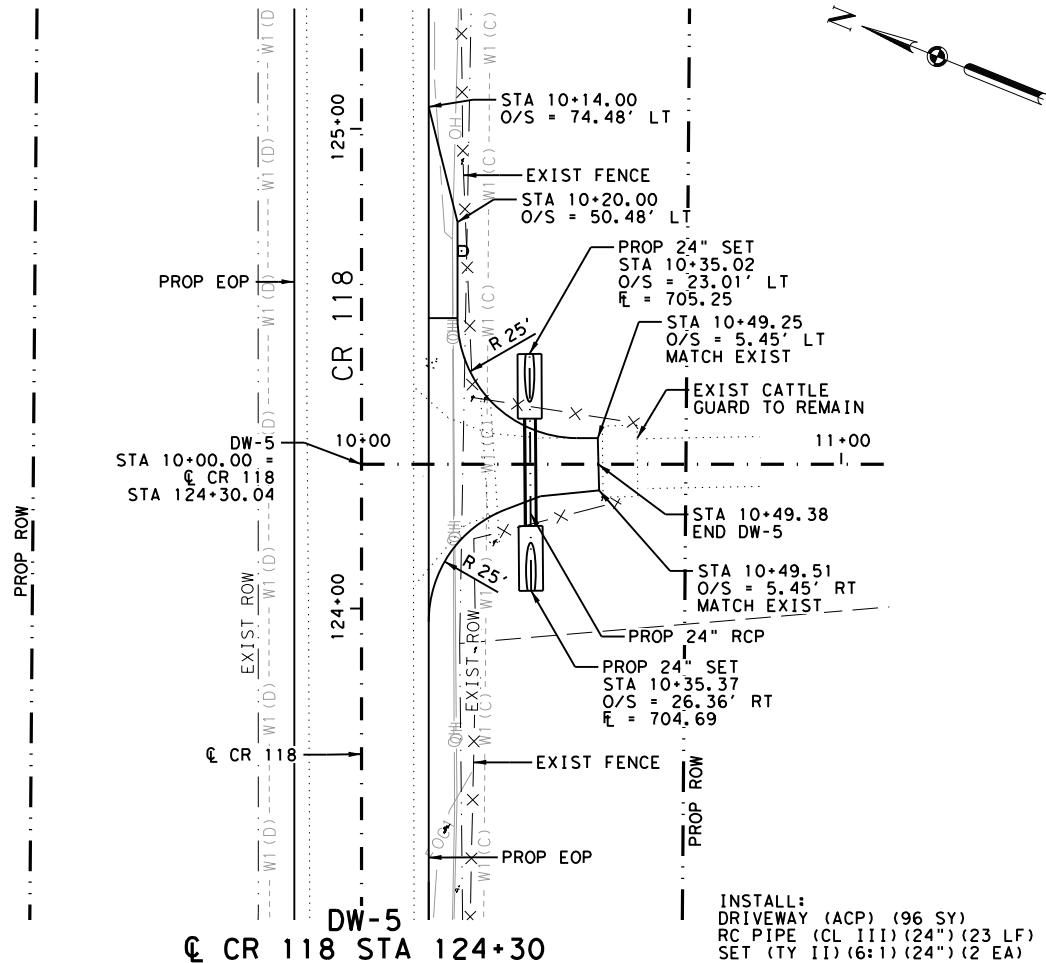
CR 118 AT COTTONWOOD CREEK

DRIVEWAY
 PLAN AND PROFILE

SHEET 2 OF 3

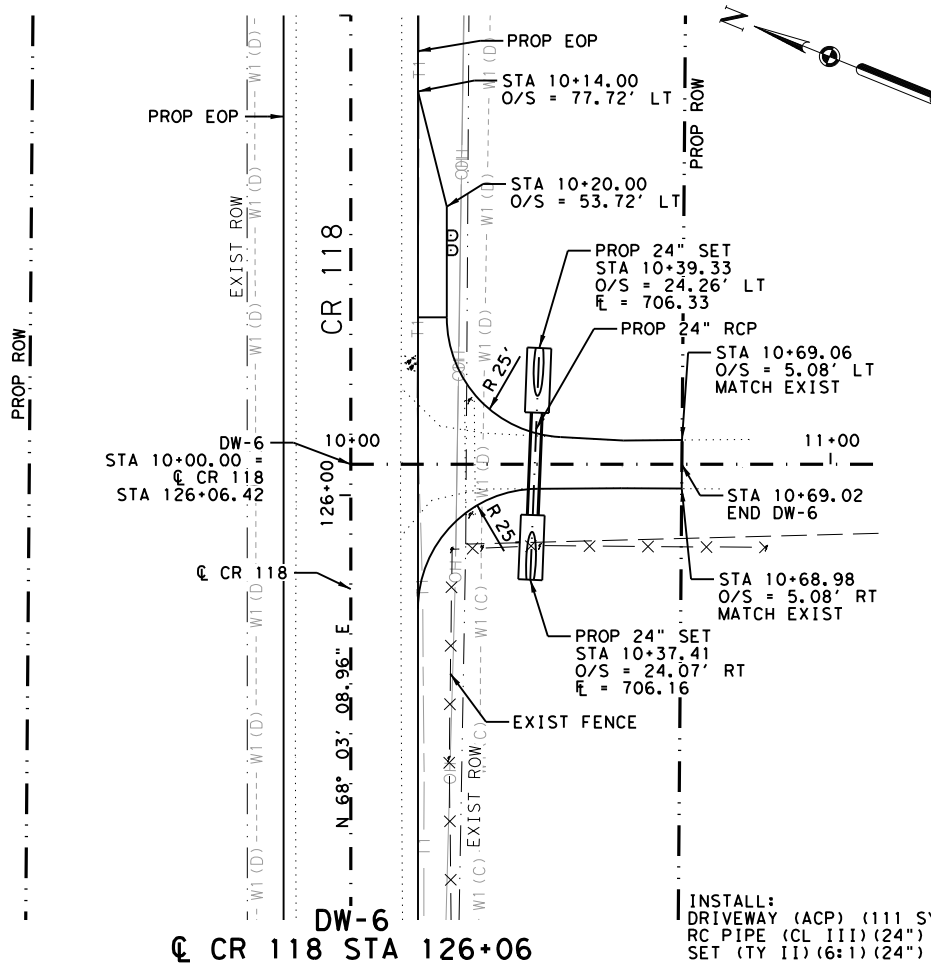
| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 37 |

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CR 118 STA 124+30

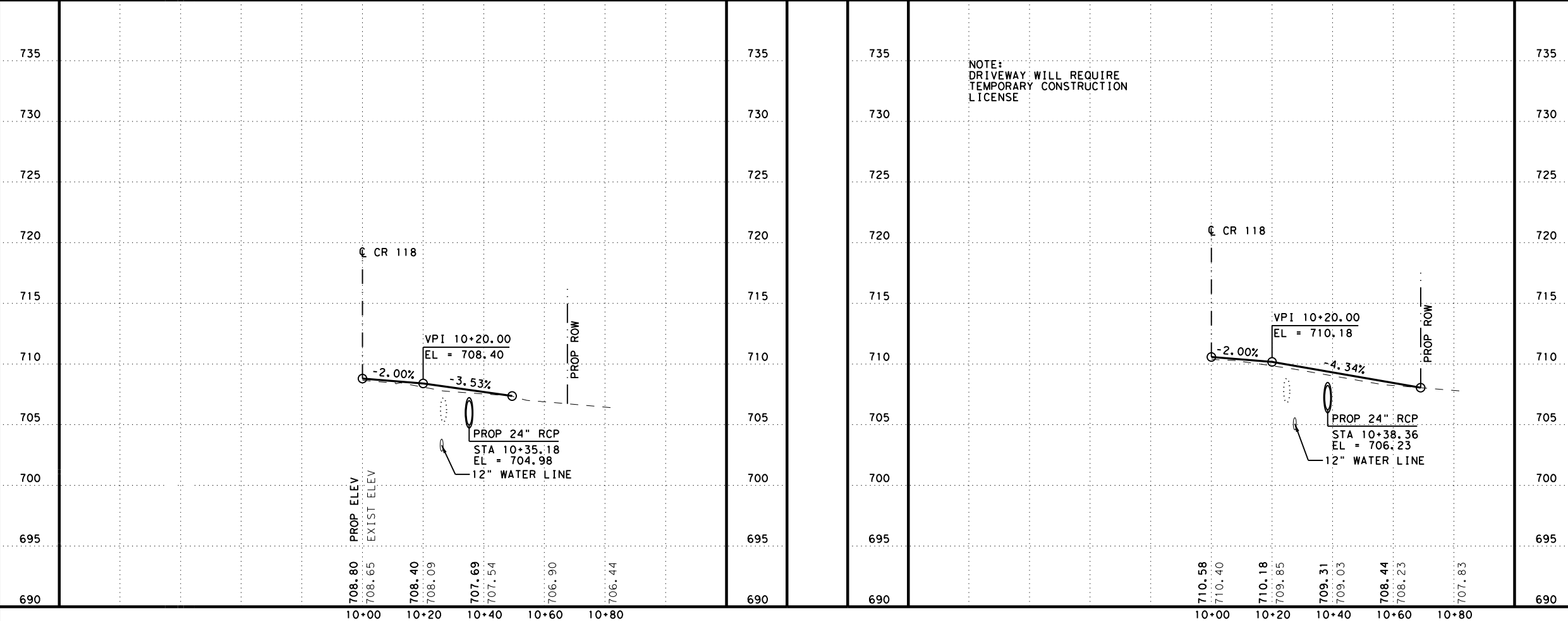
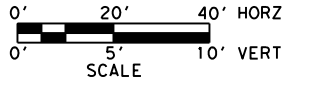
INSTALL:
 DRIVEWAY (ACP) (96 SY)
 RC PIPE (CL III) (24") (23 LF)
 SET (TY II) (6:1) (24") (2 EA)



CR 118 STA 126+06

INSTALL:
 DRIVEWAY (ACP) (111 SY)
 RC PIPE (CL III) (24") (22 LF)
 SET (TY II) (6:1) (24") (2 EA)

NOTES:
 1. SEE REMOVAL SHEETS FOR PIPE REMOVAL INFORMATION.



NOTE:
 DRIVEWAY WILL REQUIRE
 TEMPORARY CONSTRUCTION
 LICENSE

TJN
 11/3/2022

 TREY NEAL
 106194
 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

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 Texas Department of Transportation

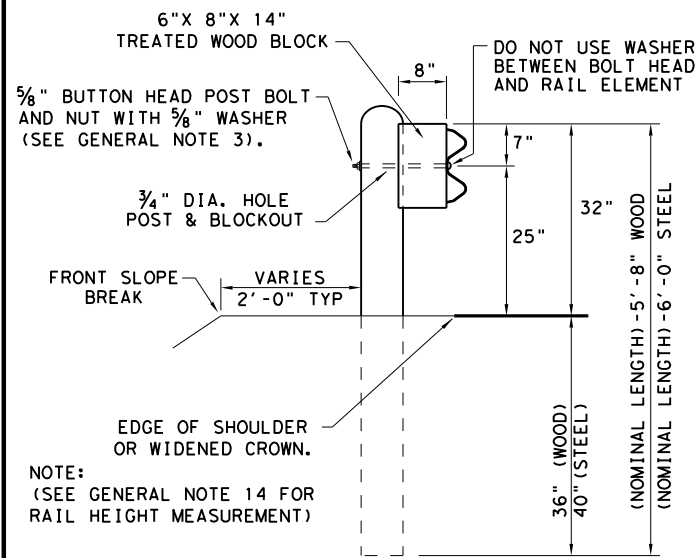
CR 118 AT COTTONWOOD CREEK
 DRIVEWAY
 PLAN AND PROFILE

SHEET 3 OF 3

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 38 |

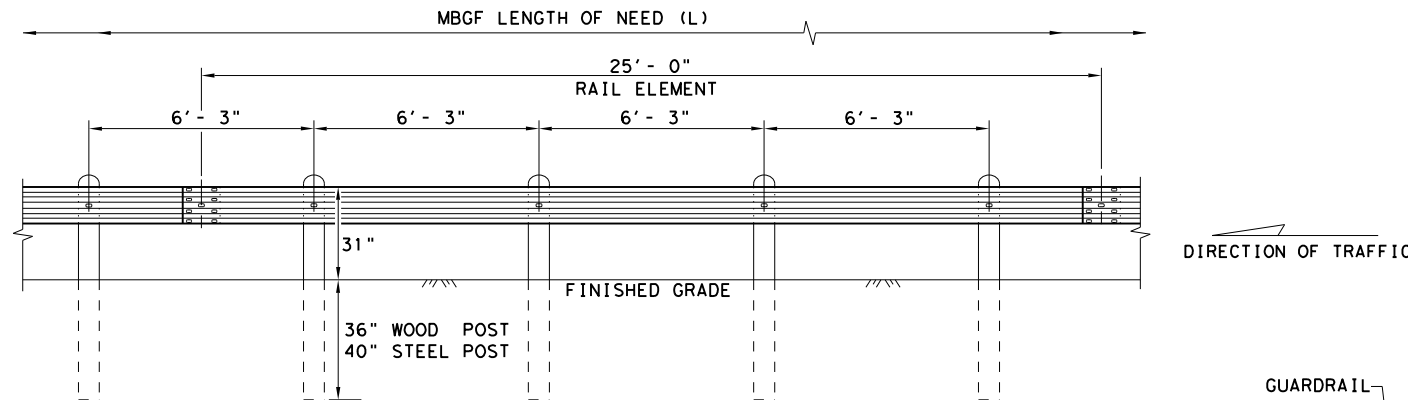
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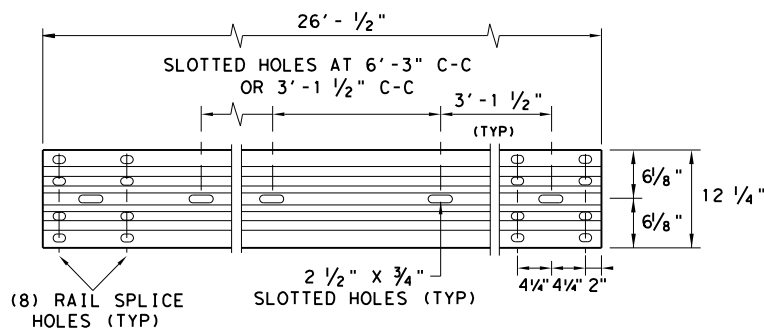
TYPICAL POST PLACEMENT

NOTE: ** "WOOD" INDICATES DIMENSIONS FOR BOTH ROUND AND RECTANGULAR WOOD POST SYSTEMS.



ELEVATION MID-SPAN RAIL SPLICE

SHOWING A 25' - 0" SECTION OF W-BEAM RAIL. (SEE GENERAL NOTE 2)



ELEVATION 25' - 0" (NOM.) W-BEAM SECTION

NOTES: SEE GENERAL NOTE 2 FOR ALLOWABLE RAIL TYPES. SEE RAIL SPLICE DETAIL FOR REQUIRED HARDWARE.

NOTE: FOUR TYPES OF BUTTON-HEAD GUARD RAIL BOLTS COME WITH A RECESSED NUT.

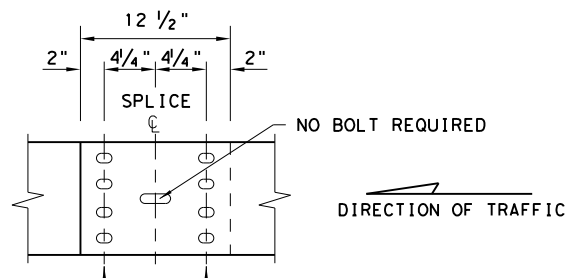
SPLICE BOLT LENGTH VARIES

FBB01 = 1 1/4"
 FBB02 = 2"

POST & BLOCK LENGTH
 FBB03 = 10"
 FBB04 = 18"

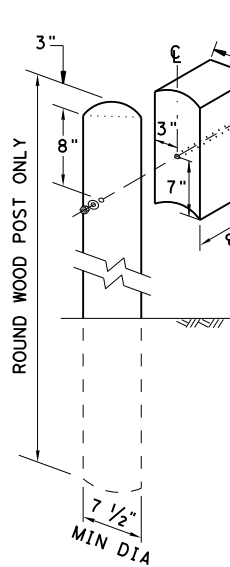
BUTTON HEAD BOLT

NOTE: SEE GENERAL NOTE 3 FOR SPLICE & POST BOLT DETAILS.



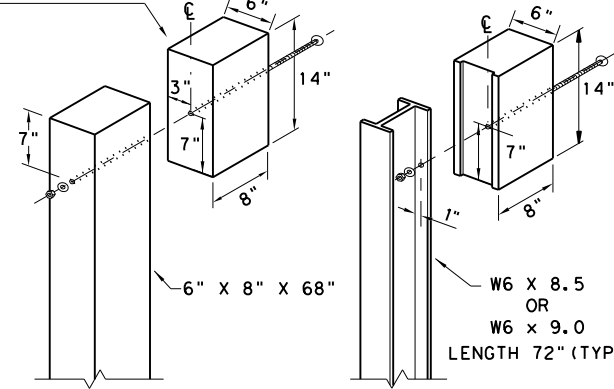
MID-SPAN RAIL SPLICE DETAIL

NOTE: GF(31), MID-SPAN RAIL SPLICES ARE REQUIRED WITH 6'-3" POST SPACINGS.



WOOD BLOCK TO ROUND WOOD POST

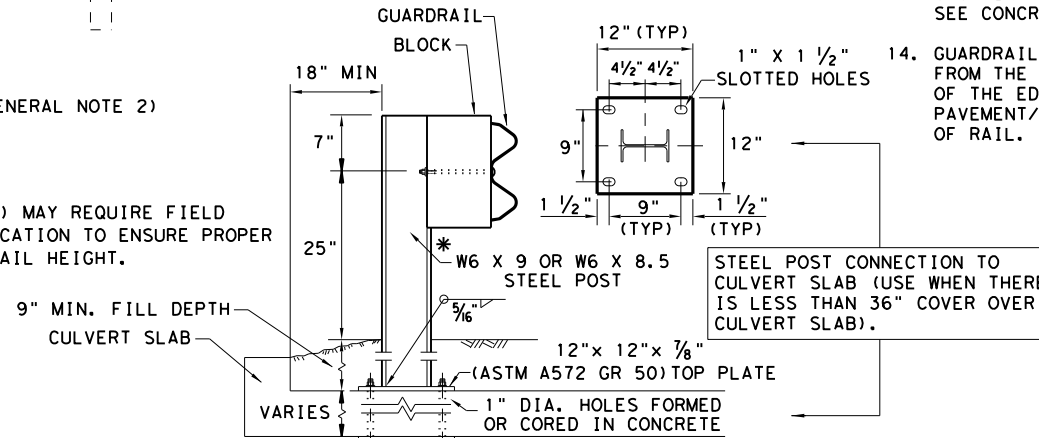
NOTE: TOENAIL WITH ONE 16D GALV. NAIL TO PREVENT BLOCK ROTATION.



WOOD BLOCK TO RECTANGULAR WOOD POST

ROUTED WOOD BLOCK TO I-BEAM STEEL POST

* POST(S) MAY REQUIRE FIELD MODIFICATION TO ENSURE PROPER GUARDRAIL HEIGHT.



LOW FILL CULVERT POST

12" x 12" x 1/4" (ASTM A36) STEEL BOTTOM PLATE WITH 1" DIA. HOLES REQUIRED WITH BOLT-THROUGH INSTALLATION.

NOTE: TWO INSTALLATION OPTIONS.

1. **BOLT-THROUGH OPTION:** REQUIRES A 6" MIN. SLAB THICKNESS. 7/8" DIA (ASTM A449) HEAVY HEX BOLTS WITH TWO HARDENED WASHER EACH AND HEAVY HEX NUTS. NOTE: BOLT LENGTH = SLAB PLUS 2 1/4" MIN.

2. **EPOXY ANCHOR OPTION:** THIS OPTION MAY ONLY BE USED IF THE CULVERT SLAB IS 9" MIN. THICK. THREADED ANCHOR RODS MUST BE 7/8" DIA. ASTM A449 OR A193 GRADE B7 WITH HEAVY HEX NUT, AND ONE HARDENED WASHER EACH. EMBED ANCHOR RODS 6" WITH HILTI HIT RE 500 EPOXY ADHESIVE. OTHER TYPE III CLASS C EPOXY ADHESIVES MEETING THE REQUIREMENTS OF DMS-6100, "EPOXIES AND ADHESIVES", MAY BE USED IF IT CAN BE DEMONSTRATED THAT THEY MEET OR EXCEED THE STRENGTH OF HILTI HIT RE 500 WITH THE SAME EMBEDMENT DEPTH AND THREADED ROD DIA. FOLLOW THE MANUFACTURER'S REQUIREMENTS FOR INSTALLING EPOXIED THREADED RODS. EXTEND RODS 1/4" MIN. BEYOND NUT.

NOTE: CULVERTS OF 25 FT. OR LESS, SEE GF(31)LS STANDARD FOR "LONG SPAN" OPTION.

GENERAL NOTES

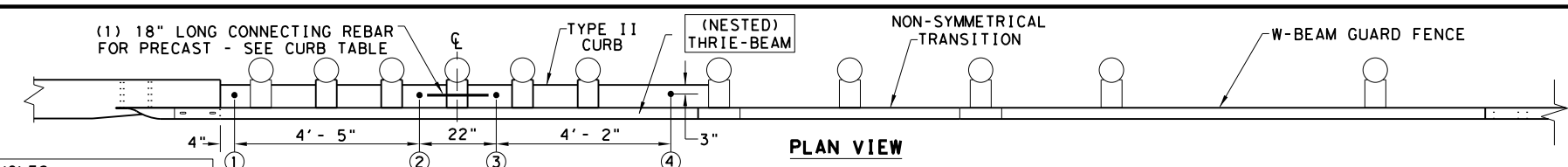
1. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST, OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. THE EXACT POSITION OF MBGF SHALL BE SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER. STEEL POSTS TO BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING."
2. RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED IN THE PLANS. THE CONTRACTOR MAY FURNISH RAIL ELEMENTS OF 25'-0", OR 12'-6" (NOM.) LENGTHS. RAIL ELEMENTS MAY HAVE SLOTTED HOLES AT 3'-1 1/2" C-C OR 6'-3" C-C. A SPECIAL LENGTH OF RAIL MAY BE MANUFACTURED TO ACCOMMODATE THE DOWNSTREAM ANCHOR TERMINAL (DAT) AND THE TRANSITION SECTIONS OF GUARDRAIL.
3. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 3/8" WASHER (FWC16G) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING BOLT LENGTH TO MEET REQUIRED LENGTH.
4. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING." FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
5. CROWN SHALL BE WIDENED TO ACCOMMODATE THE METAL BEAM GUARD FENCE.
6. THE LATERAL APPROACH TO THE GUARD FENCE, SHALL HAVE A MAXIMUM SLOPE OF 1V:10H.
7. IF SHOWN ELSEWHERE IN THE PLANS OR AS DIRECTED BY THE ENGINEER, THE GUARD FENCE MAY BE FLARED AT A RATE OF 25:1 OR FLATTER.
8. UNLESS OTHERWISE SHOWN IN THE PLANS, GUARD FENCE PLACED IN THE VICINITY OF CURBS SHALL BE POSITIONED SO THAT THE FACE OF CURB IS LOCATED DIRECTLY BELOW OR BEHIND THE FACE OF THE RAIL. RAIL PLACED OVER CURBS SHALL BE INSTALLED SO THAT THE POST BOLT IS LOCATED APPROXIMATELY 25 INCHES ABOVE THE GUTTER PAN OR EDGE OF SHOULDER.
9. APPLICATIONS IN SOLID ROCK ARE ONLY ALLOWED WITH STEEL POSTS. IF SOLID ROCK IS ENCOUNTERED WITHIN 0 TO 18" OF THE FINISHED GRADE, DRILL A 24" DIA. HOLE, 24" INTO THE ROCK. IF SOLID ROCK IS ENCOUNTERED BELOW 18", DRILL A 12" DIA. HOLE, 12" INTO THE ROCK OR TO THE STANDARD EMBEDMENT DEPTH, WHICHEVER MAYBE LESS. ANY EXCESS POST LENGTH, AFTER MEETING THESE DEPTHS, MAY BE FIELD CUT TO ENSURE PROPER GUARDRAIL MOUNTING HEIGHT. BACKFILL WITH COARSE AGGREGATE MATERIAL.
10. POSTS SHALL NOT BE SET IN CONCRETE, OF ANY DEPTH.
11. SPECIAL FABRICATION WILL BE REQUIRED AT INSTALLATION LOCATIONS HAVING A CURVATURE OF LESS THAN 150 FT. RADIUS.
12. UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. THE CONSTRUCTION DIVISION, TXDOT MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210 ONLY PRODUCERS ON THE MPL MAY FURNISH COMPOSITE MATERIAL BLOCKS.
13. FOR THE LOW FILL CULVERT OPTION, POSTS LOCATED PARTIALLY OR WHOLLY BETWEEN PRECAST BOX CULVERT UNITS, THE USE OF A CAST-IN-PLACE CONCRETE CLOSURE BETWEEN BOXES IS REQUIRED. THE LENGTH OF THE CAST-IN-PLACE CONCRETE CLOSURE SHALL ACCOMMODATE THE PLACEMENT OF THE LOW FILL CULVERT OPTION. SEE CONCRETE CLOSURE DETAILS ON BRIDGE STANDARD SCP-MD.
14. GUARDRAIL HEIGHT MEASUREMENT: WHEN THE GUARDRAIL IS LOCATED ABOVE PAVEMENT, MEASURE THE HEIGHT FROM THE PAVEMENT TO THE TOP OF THE W-BEAM RAIL. WHEN THE GUARDRAIL IS LOCATED UP TO 2 FT. OFF OF THE EDGE OF PAVEMENT OR FOR A PAVEMENT OVERLAY, USE A 10-FOOT STRAIGHTEDGE TO EXTEND THE PAVEMENT/SHOULDER SLOPE TO THE BACK OF RAIL, MEASURE FROM THE BOTTOM OF STRAIGHTEDGE TO THE TOP OF RAIL. FOR GUARDRAIL LOCATED DOWN A 10:1 SLOPE, MEASURE FROM THE NOMINAL TERRAIN.

NOTE: TRANSITIONS TO BRIDGE RAILS OR TRAFFIC BARRIERS. SEE GF(31)TL3 TR STANDARD FOR HIGH-SPEED TL-3 TRANSITIONS. SEE GF(31)TL2 TR STANDARD FOR LOW-SPEED TL-2 TRANSITIONS.

| | | | | |
|---|-----------|------------|-----------|--------------------------------|
| | | | | Design Division Standard |
| METAL BEAM GUARD FENCE TL-3 MASH COMPLIANT GF(31)-19 | | | | |
| FILE: gf3119.dgn | DN: TxDOT | CK: KM | DW: VP | CK: CGL/AG |
| © TXDOT: NOVEMBER 2019 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | SHEET NO. | |
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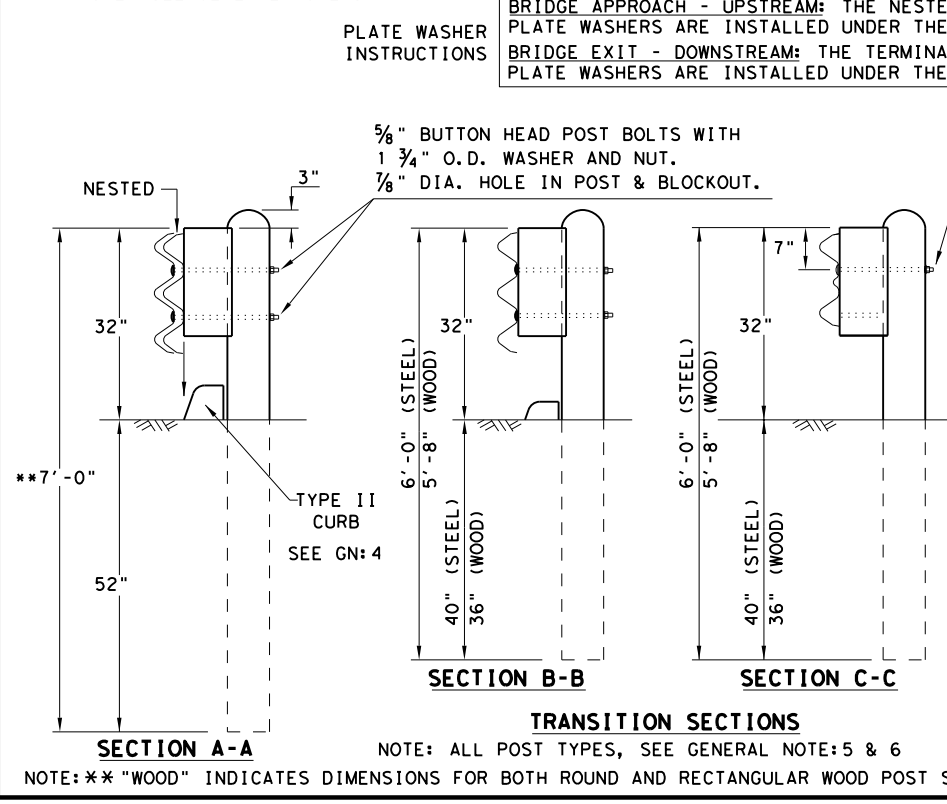
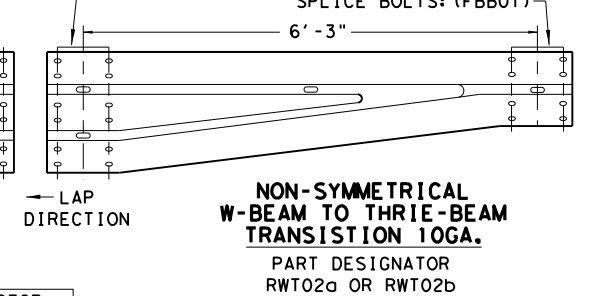
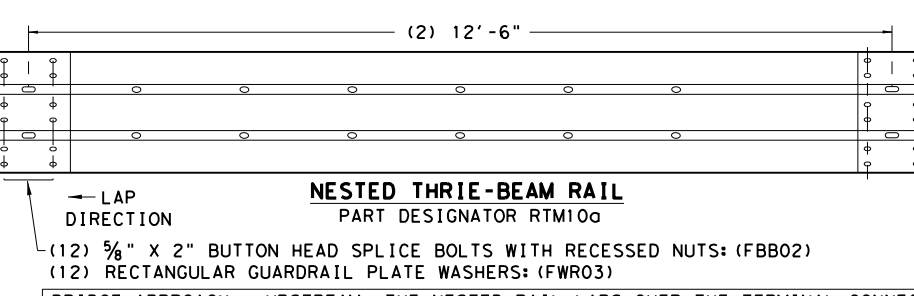
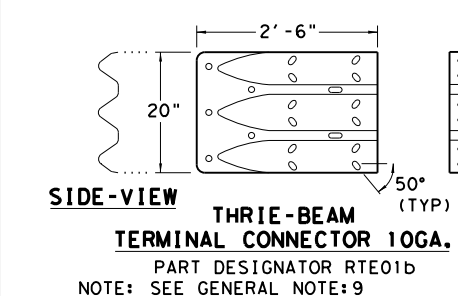
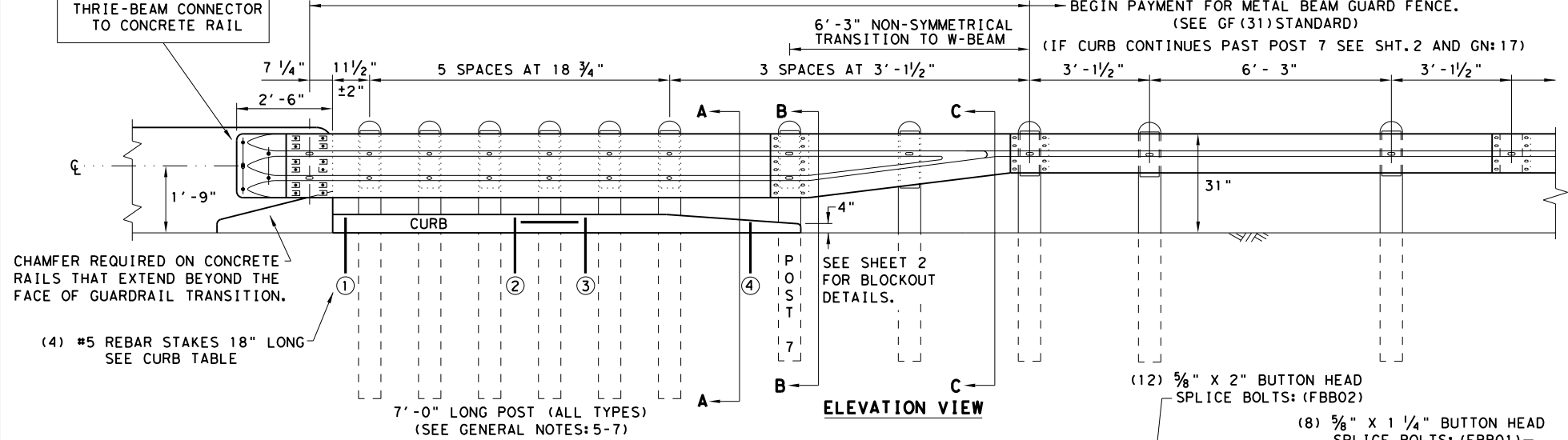
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- (5) 1" DIA. HOLES.
- (5) 3/8" DIA. HEAVY HEX HEAD BOLTS (FACING TRAFFIC SIDE) (ASTM F3125 GR A325 OR A449).
- (10) 1 3/4" O.D. WASHER UNDER EACH HEX BOLT HEAD AND NUT.
- (5) 3/8" DIA. HEAVY HEX NUTS (ASTM A194 OR A563).

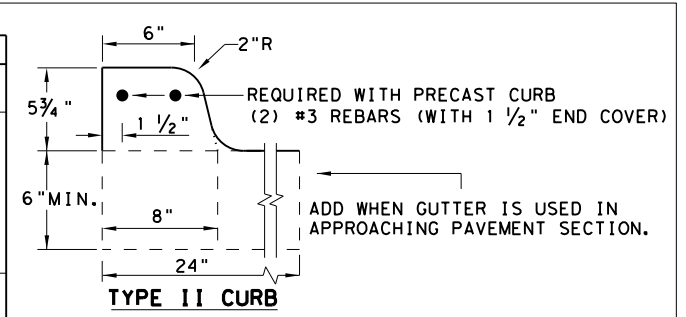
NOTE: HEAVY HEX BOLT LENGTH WILL VARY DEPENDING ON WIDTH CONCRETE RAIL, LEAVE 1" OF BOLT LENGTH PAST THE 3/8" HEX NUT. TRIM AS REQUIRED.

NOTE: CURB IS A REQUIRED COMPONENT FOR THE TRANSITION TO FUNCTION PROPERLY. SEE GENERAL NOTES: 2-4 AND 16-17.



| THRIE-BEAM TERMINAL - CURB TABLE | |
|---|--------------|
| PRECAST CURB FULL LENGTH EQUALS 12'-2" | |
| THE PRECAST CURB MAY BE FORMED INTO TWO SECTIONS. | |
| CURB (1) | LENGTH 5'-8" |
| CURB (2) | LENGTH 6'-6" |
| TAPER CURB (2) TO A HEIGHT OF 4" AT POST 7 | |
| CONNECTING PRECAST CURB SECTIONS (1) & (2): | |
| FORM OR CORE 1" DIA. HOLE 9" LONG INTO EACH CURB END. | |
| USE (1) #5 GR.60 REBAR 18" LONG TO CONNECT BOTH CURBS. | |
| SECURING PRECAST OR CAST-IN-PLACE TO FINISHED GRADE *: | |
| FORM OR CORE (4) 1" DIA. HOLES, SEE PLAN AND ELEVATION VIEWS FOR HOLE LOCATIONS. DRIVE (4) #5 GR.60 REBAR STAKES 18" LONG INTO THE GROUND AND 1/2" BELOW TOP OF CURB. | |
| FILL HOLES WITH APPROVED GROUT MIXTURE. | |

* NOTES: NOT NEEDED FOR CAST-IN-PLACE. SEE TYPE II CURB DETAIL FOR REBAR AND COVER REQUIREMENTS. PERCUSSION DRILLING IS NOT PERMITTED WITH: TYPE II CURB, BRIDGE RAIL OR CONCRETE TRAFFIC RAIL.



NOTE: OPTIONS FOR TYPE II CURB:
 1. PRECAST
 2. CAST-IN-PLACE

GENERAL NOTES

1. CONTACT THE DESIGN DIVISION FOR DRAINAGE CUT OUT OPTIONS NEEDED WITHIN THE CURB SECTION OF THE THRIE-BEAM TRANSITION. (512) 416-2678
2. CONCRETE CURB MAY BE CAST-IN-PLACE OR PRECAST AS SHOWN ON THIS SHEET. WHEN USED IN CONJUNCTION WITH THE THRIE-BEAM TRANSITIONS, CURB SHALL BE TYPE II (5-3/4" HEIGHT); SEE CURRENT CCGG STANDARD SHEET FOR FURTHER DETAILS. IF OTHER CURB HEIGHTS ARE SHOWN IN THE PLANS IN CONJUNCTION WITH THE TRANSITION, THE CURB HEIGHT MAY BE FROM 4" TO 8" WITH A RELATIVELY VERTICAL FACE. CONCRETE CURB SHALL BE CONTINUOUS TO THE SEVENTH POST UNLESS OTHERWISE SHOWN IN THE PLANS. SEE GENERAL NOTE:17 FOR CIRCUMSTANCES WHERE CURB CONTINUES PAST POST 7.
3. CONCRETE CURB TYPE II SUBSIDIARY TO "METAL BEAM GUARD FENCE TRANSITION". IF NO ADDITIONAL CURB IS INDICATED BEYOND THE TRANSITION, THEN ANY CURB HEIGHT GREATER THAN 4" WILL BE TAPERED DOWN BEGINNING AT THE LAST 7 FT. POST TO A MAXIMUM HEIGHT OF 4" AT POST 7. IF SHOWN ELSEWHERE IN THE PLANS, ADDITIONAL CURB UNDERNEATH GUARDRAIL WILL BE PAID FOR BY THE LINEAR FOOT.
4. UNLESS OTHERWISE SHOWN IN THE PLANS, TRANSITIONS SHALL BE PLACED WITH THE BLOCKOUT FACE IN FRONT OF OR DIRECTLY ABOVE THE CURB FACE. SEE SECTION A-A.
5. FOR ROUND WOOD POST SYSTEMS, ALL ROUND WOOD POSTS SHALL BE 7 1/2" DIA. MINIMUM THROUGHOUT THE THRIE-BEAM TRANSITION.
6. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. REFER TO GF (31) STANDARD SHEET.
7. THE POST LENGTH SHALL BE MARKED ON ALL 7'-0" LONG POSTS BY THE MANUFACTURER. THE MARK SHALL BE LOCATED WITHIN THE TOP 1 FT. REGION OF THE POST, AT LEAST 3/8" IN HEIGHT, AND VISIBLE AFTER INSTALLATION. WOODEN POSTS SHALL BE MARKED WITH A BRAND, AND STEEL POSTS WITH A STENCIL BEFORE GALVANIZING.
8. POSTS SHALL NOT BE SET IN CONCRETE, OF ANY DEPTH.
9. RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED ON THE PLANS. THE THRIE-BEAM TERMINAL CONNECTOR AND THE THRIE-BEAM TRANSITION TO W-BEAM SHALL BE OF THE SAME MATERIAL, BUT SHALL NOT BE LESS THAN 10 GAUGE. CONTRACTOR SHALL VERIFY THAT THE LOCATIONS OF BOLT HOLES MATCH THOSE IN THE THRIE-BEAM TERMINAL CONNECTOR PRIOR TO ORDERING MATERIALS.
10. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 3/8" WASHER (FWC16G) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING BOLT LENGTH TO MEET REQUIRED LENGTH.
11. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
12. CROWN SHALL BE WIDENED TO ACCOMMODATE TRANSITIONS.
13. WHERE SOLID ROCK IS ENCOUNTERED, CONTACT THE DESIGN DIVISION FOR ADDITIONAL GUIDANCE. (512) 416-2678
14. UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. TxDOT'S MATERIALS AND TESTS DIVISION MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210. ONLY PRODUCERS ON THE MPL CAN FURNISH COMPOSITE MATERIAL BLOCKS.
15. REFER TO GF (31) STANDARD SHEET & BRIDGE RAILING DETAILS FOR ADDITIONAL DETAILS.
16. THE INSTALLATION OF THE TYPE II CURB IS CRITICAL FOR THE PERFORMANCE OF THE THRIE-BEAM TRANSITION SYSTEM. THE CURB PREVENTS (VEHICLE WHEEL SNAGGING) AT THE CONCRETE RAIL AND IS REQUIRED TO MEET MASH CRASH TEST CRITERIA.
17. IF CURB EXTENDS BEYOND POST 7, 25' OF NESTED W-BEAM GUARDRAIL SHALL BE INSTALLED BEYOND THE PAY LIMITS OF THRIE-BEAM TRANSITION SECTION, (SEE SHT.2). PAYMENT FOR THIS 25' SECTION WILL BE BY LINEAR FOOT, PAY ITEM "0540 6XXX MTL W-BEAM GD FEN (NESTED) (TIM POST)" OR "540 6XXX MTL W-BEAM GD FEN (NESTED) (STEEL POST)" AS APPLICABLE FOR POST TYPE. SEE SHT.2 FOR ADDITIONAL INFORMATION.

HIGH-SPEED TRANSITION
SHEET 1 OF 2

METAL BEAM GUARD FENCE
THRIE-BEAM TRANSITION
TL-3 MASH COMPLIANT
GF (31) TR TL3-20

| | | | | |
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| ©TxDOT: NOVEMBER 2020 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 40 |

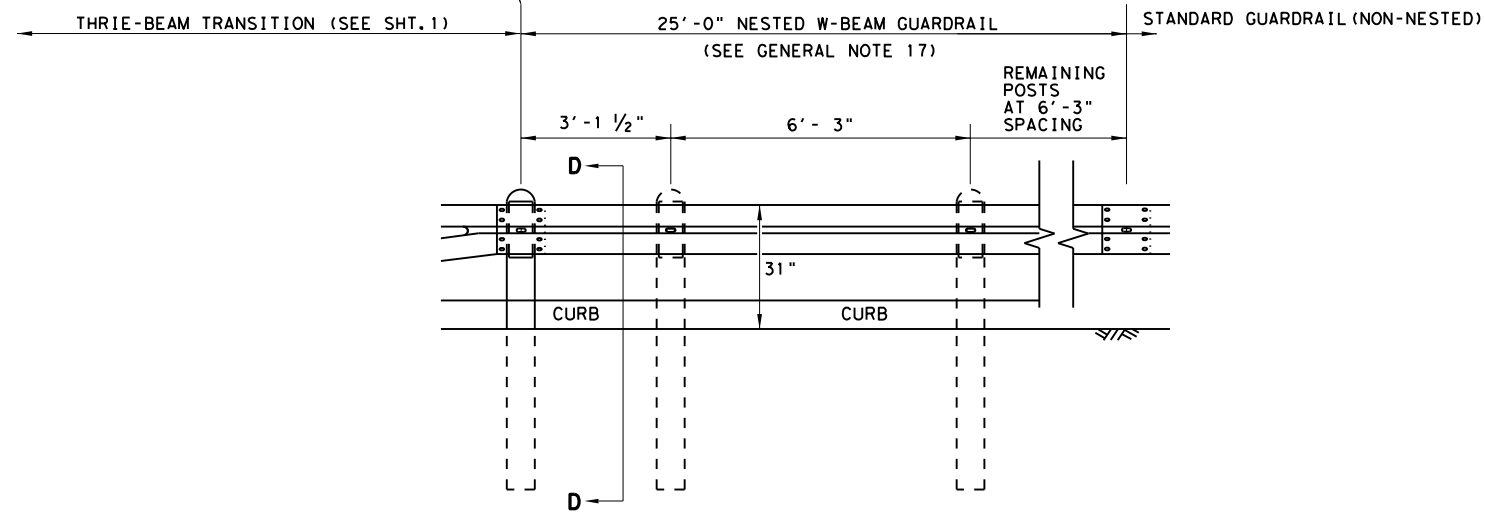
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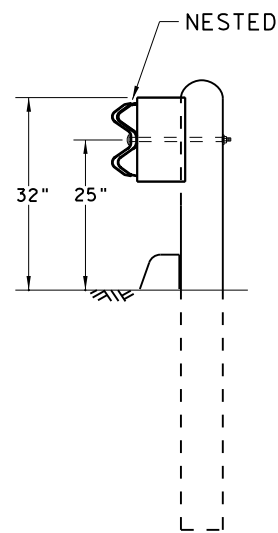
REQUIRED ALTERNATIVE FOR CONTINUOUS CURB EXTENDING PAST POST 7 (SEE SHT. 1 GENERAL NOTE 17)

END PAYMENT FOR METAL BEAM GUARD FENCE TRANSITION.
 BEGIN PAYMENT FOR METAL BEAM GUARD FENCE.

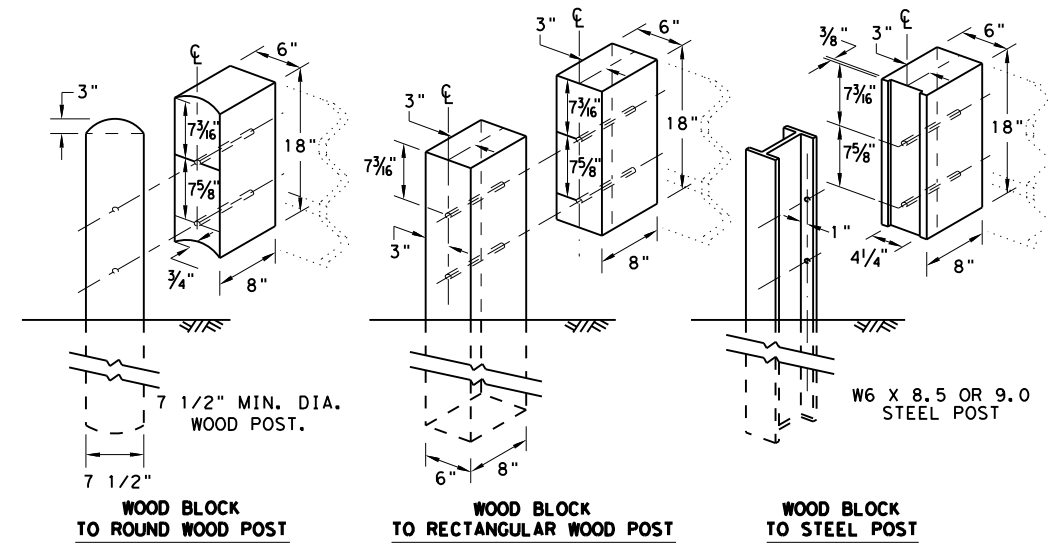
(SEE GF (31) STANDARD SHEET)



ELEVATION VIEW



SECTION D-D



THREE BEAM TRANSITION BLOCKOUT DETAILS

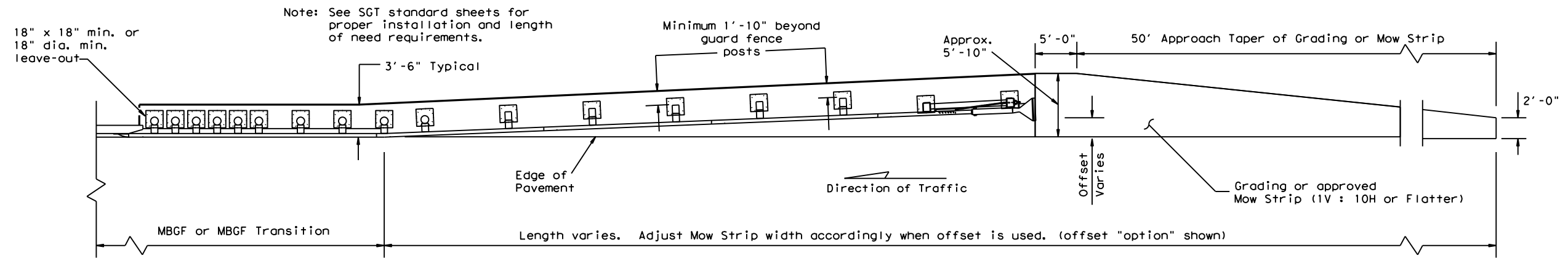
HIGH-SPEED TRANSITION

SHEET 2 OF 2

| | | | | | |
|--|-----------|------------|--------|--------------------------|--------|
| | | | | Design Division Standard | |
| METAL BEAM GUARD FENCE THREE-BEAM TRANSITION TL-3 MASH COMPLIANT GF (31) TR TL3-20 | | | | | |
| FILE: gf31tr+1320.dgn | DN: TXDOT | CK: KM | DW: KM | CK: CGL/AG | |
| ©TXDOT: NOVEMBER 2020 | CONT | SECT | JOB | HIGHWAY | |
| REVISIONS | | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. | |
| | AUS | WILLIAMSON | | 41 | |

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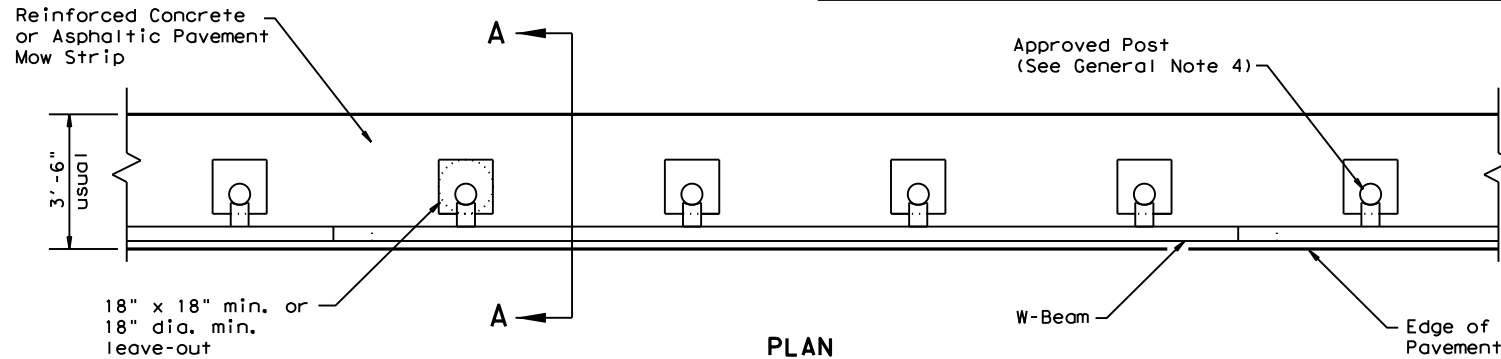
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Note: See SGT standard sheets for proper installation and length of need requirements.

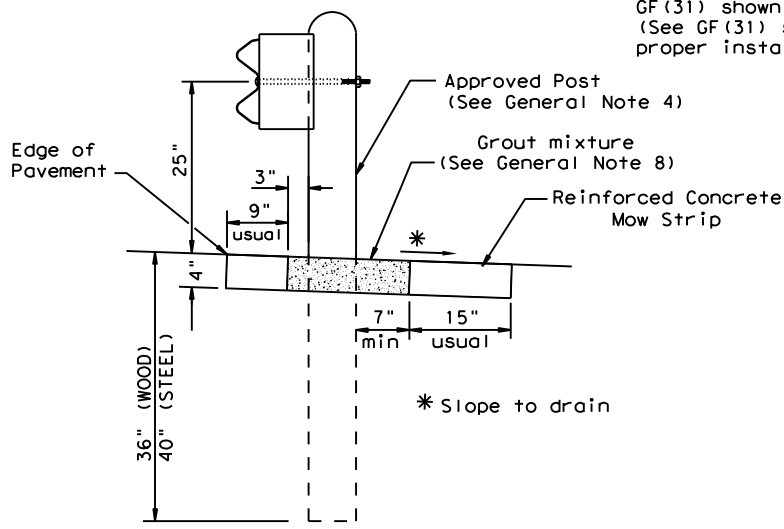
GRADING AND MOW STRIP AT GUARDRAIL END TREATMENTS

Note: Site Condition(s)
 Site conditions may exist where grading is required for the proper installation of metal guard fence and end treatments.
 Approach grading or mow strip may be decreased or eliminated, as directed by the Engineer.



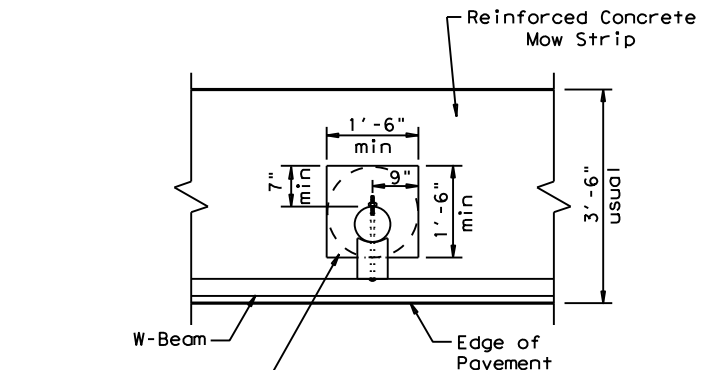
PLAN

GF(31) shown with Mow Strip
 (See GF(31) standard sheet for proper installation)



SECTION A-A

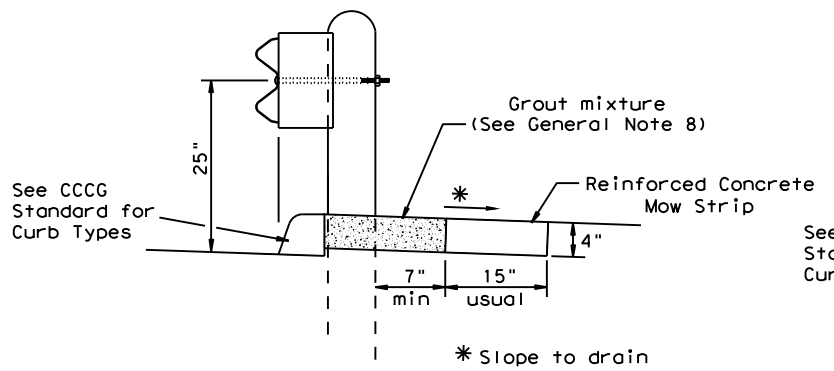
Typical



MOW STRIP DETAIL

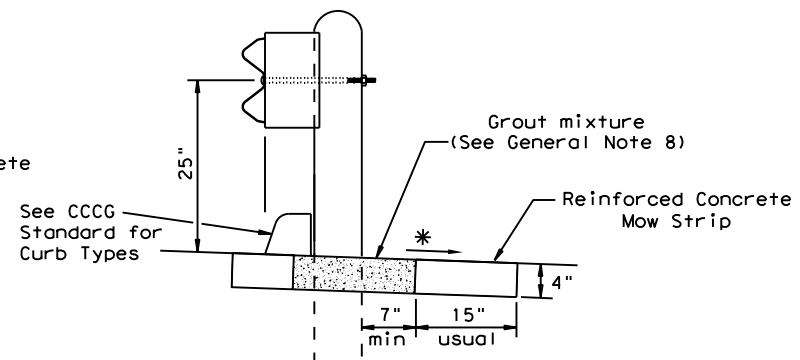
Reinforced Concrete Mow Strip with 18\"/>

- GENERAL NOTES**
1. This mow strip design is for use with metal beam guard fence, guard fence transitions, and guard fence end treatments. See applicable GF(31) MBGF or GF(31) Transition Standard sheet for additional information.
 2. Mow strips shall be reinforced concrete with (wire mesh or synthetic fiber), as shown on the plans and will be paid for under the pertinent bid item. Reinforced concrete shall be placed in accordance with Item 432, "Riprap." The use of the synthetic fiber in lieu of steel reinforcing is acceptable, provided the fiber producer is on the Department Material Producer List (MPL), maintained by TxDOT, Construction Division.
 3. The leave-out behind the post shall be a minimum of 7".
 4. Only steel (W6 x 8.5 or W6 x 9.0), or 7 1/2" Dia. round wood posts are acceptable for use in the mow strip. See GF(31) Standard for additional details.
 5. Other curb placement options may be used. Curbs are not considered part of the mow strip and will be paid for under other pertinent bid item.
 6. Thickness of the mow strip will be 4".
 7. The limits of payment for reinforced concrete will include leave-outs for the posts.
 8. The leave-outs shall be filled with a Grout mixture consisting of: 2719 pounds sand, 188 pounds Type I or II cement, and 550 pounds of water per cubic yard, with a 28-day compressive strength of approximately 230 psi or less. Provide grout with a consistency that will flow into and completely fill all voids. Due to auger size, larger leave-out dimensions are acceptable from both an impact performance and maintenance repair standpoint (Suggested Maximum leave-out of 20"). Payment for furnishing and placing the grout mixture will be subsidiary to the pay item of riprap mow strip.



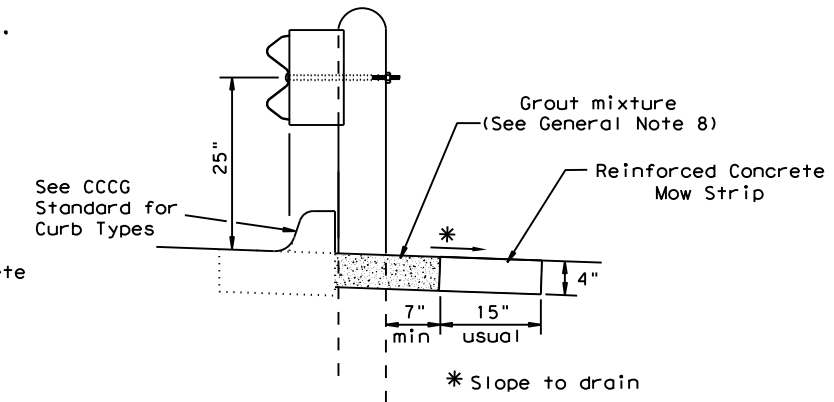
CURB OPTION (1)

This option will increase the post embedment throughout the system.



CURB OPTION (2)

Curb shown on top of mow strip

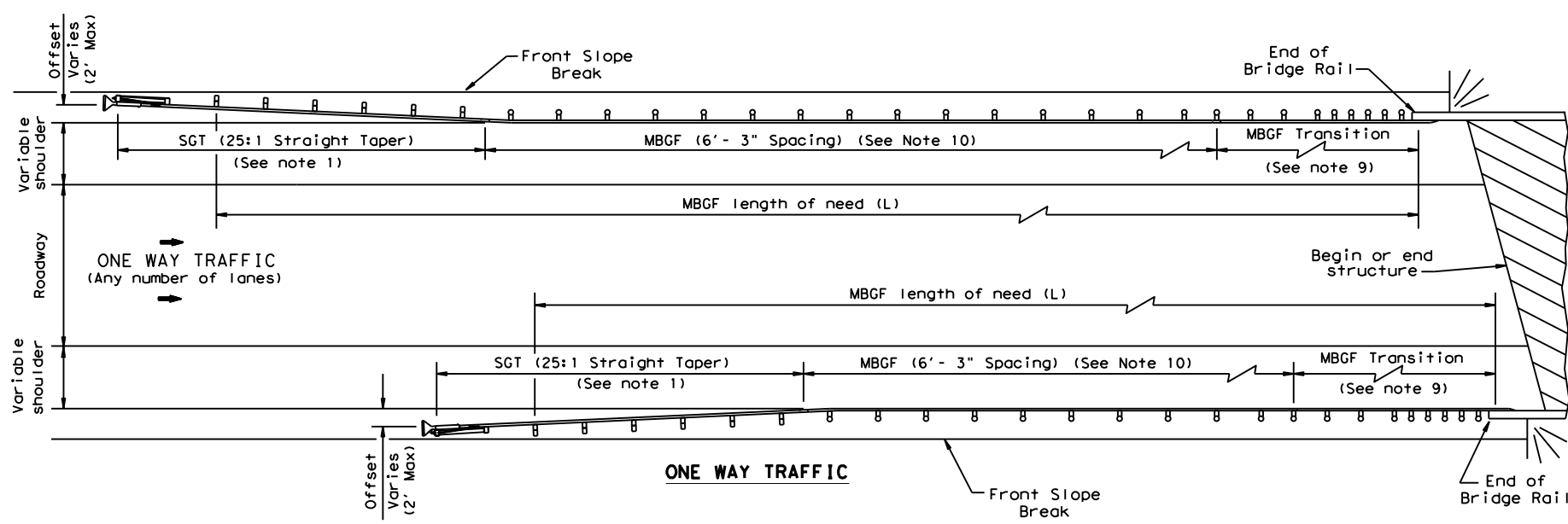
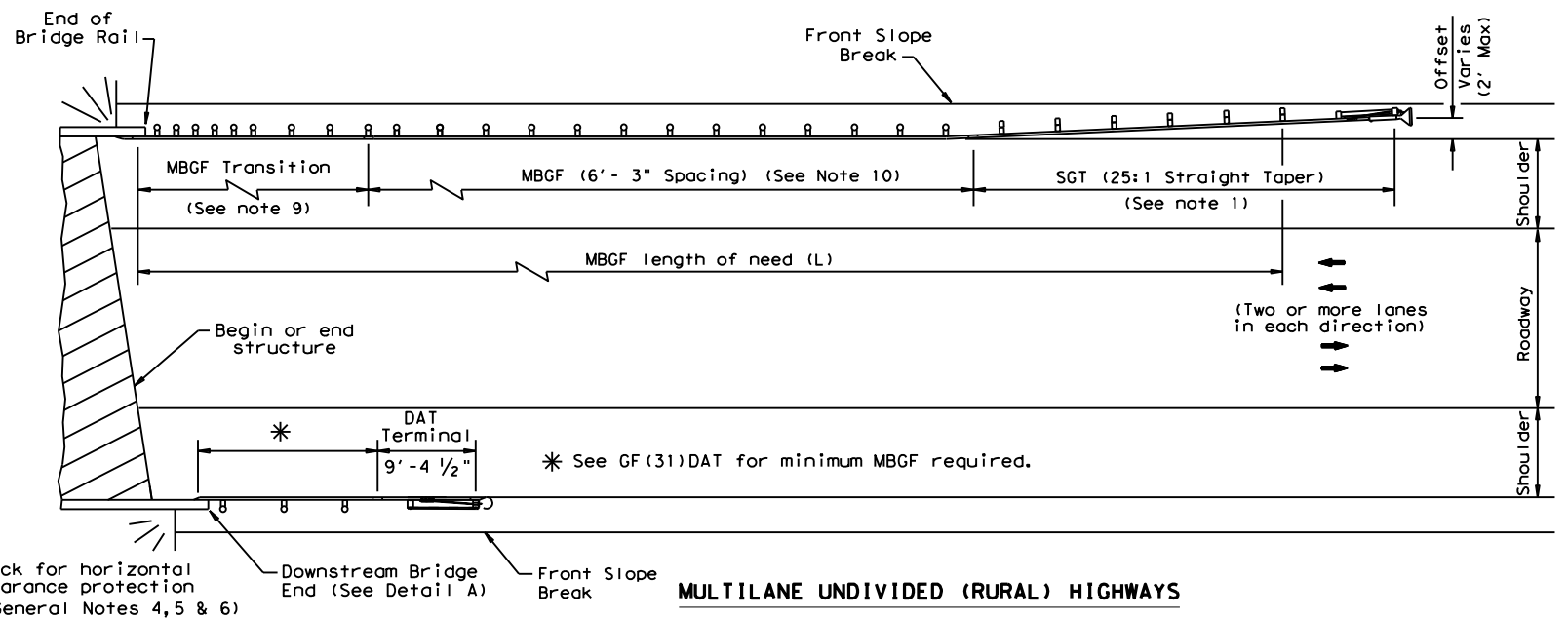
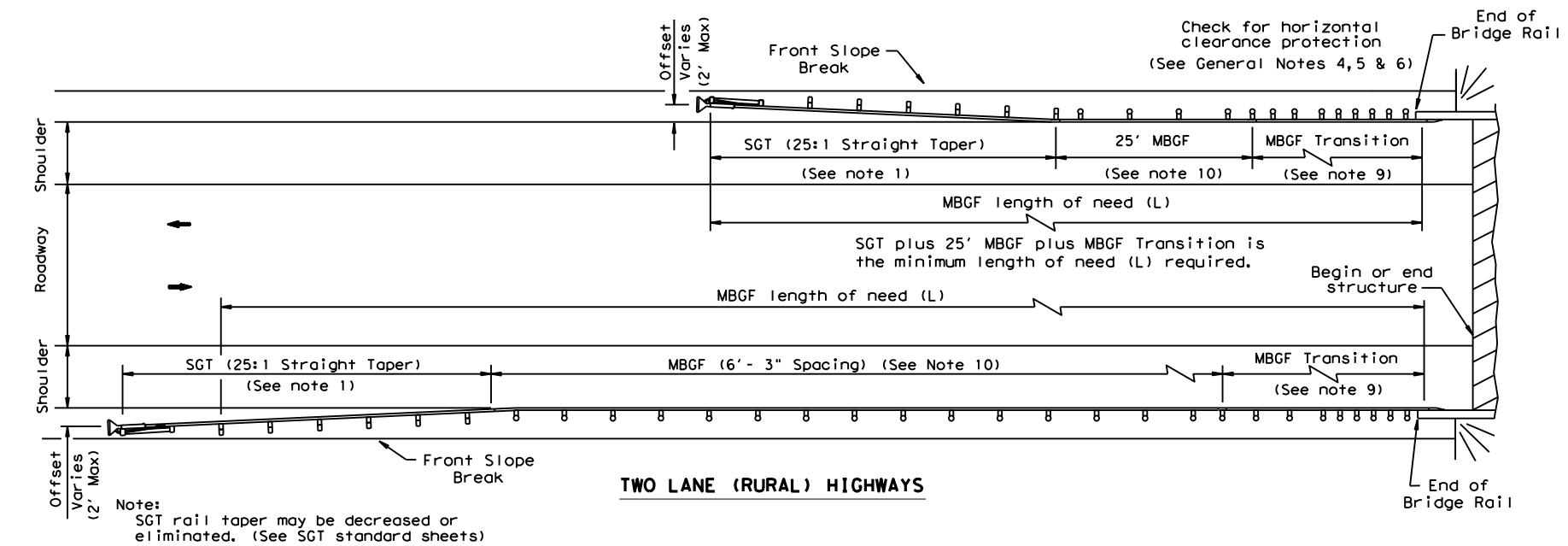


CURB OPTION (3)

| | | | |
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| | | Design Division Standard | |
| METAL BEAM GUARD FENCE (MOW STRIP) TL-3 MASH COMPLIANT GF(31)MS-19 | | | |
| FILE: gf31ms19.dgn | DN: TxDOT | CK: KM | DW: VP |
| © TXDOT: NOVEMBER 2019 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| | DIST | COUNTY | SHEET NO. |
| | AUS | WILLIAMSON | 42 |

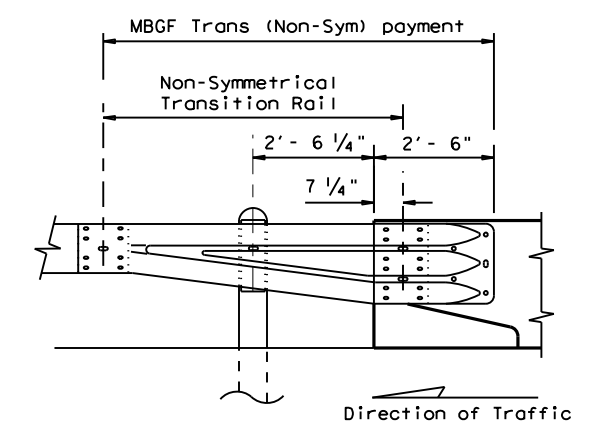
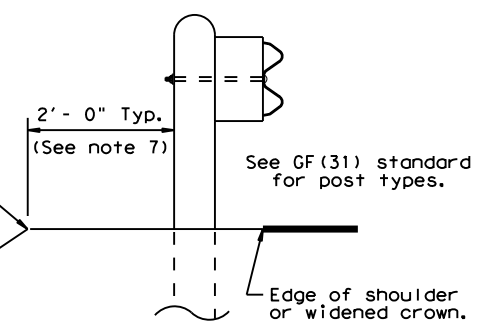
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GENERAL NOTES

1. For more detail: See GF(31), SGT()31, GF(31)TR, and GF(31)TL2 standard sheets.
2. Quantities of metal beam guard fence (MBGF) at individual bridge ends are as shown in the plans.
3. Use average daily traffic (ADT) for the current year to determine MBGF length of need in accordance with the Roadway Design Manual unless otherwise specified. Where significant traffic volume growth is anticipated on low volume (0-750 ADT) highways, use length determinations for the higher volume category.
4. MBGF may not be required to shield departure end of bridge unless other obstacles within the horizontal clearance limits or opposing traffic indicate a MBGF consideration.
5. Downstream anchor terminals (DAT) are only for downstream end anchorage use, outside the horizontal clearance area of opposing traffic.
6. Direct connection of MBGF to concrete rails are only for downstream rail connections outside the horizontal clearance area of opposing traffic. (This requires a minimum of three standard line posts plus the DAT terminal, See Detail A)
7. The crown shall be widened to accommodate MBGF. Typically the "front slope" break should be 2'-0" from the back of the MBGF post. This applies to new construction on new alignment or where existing roadway cross section is to be widened to increase roadway width. This does not apply to rehabilitation work where existing roadway crown width is to be retained (See Typical Cross Section at MBGF).
8. For restrictive bridge widths: The MBGF should be properly transitioned from the existing bridge rail to the adjoining MBGF (See MBGF Transition Standards). Metal beam guard fence at these bridge locations shall be flared at the rate of 25:1 or flatter, and be of the length necessary to locate the terminal end at the 2 ft. "maximum" offset from the shoulder edge in the approach direction.
9. Transition length and post spacing will vary depending on the transition type. Transition type will be shown elsewhere in the plans.
10. A minimum 25' length of MBGF will be required.

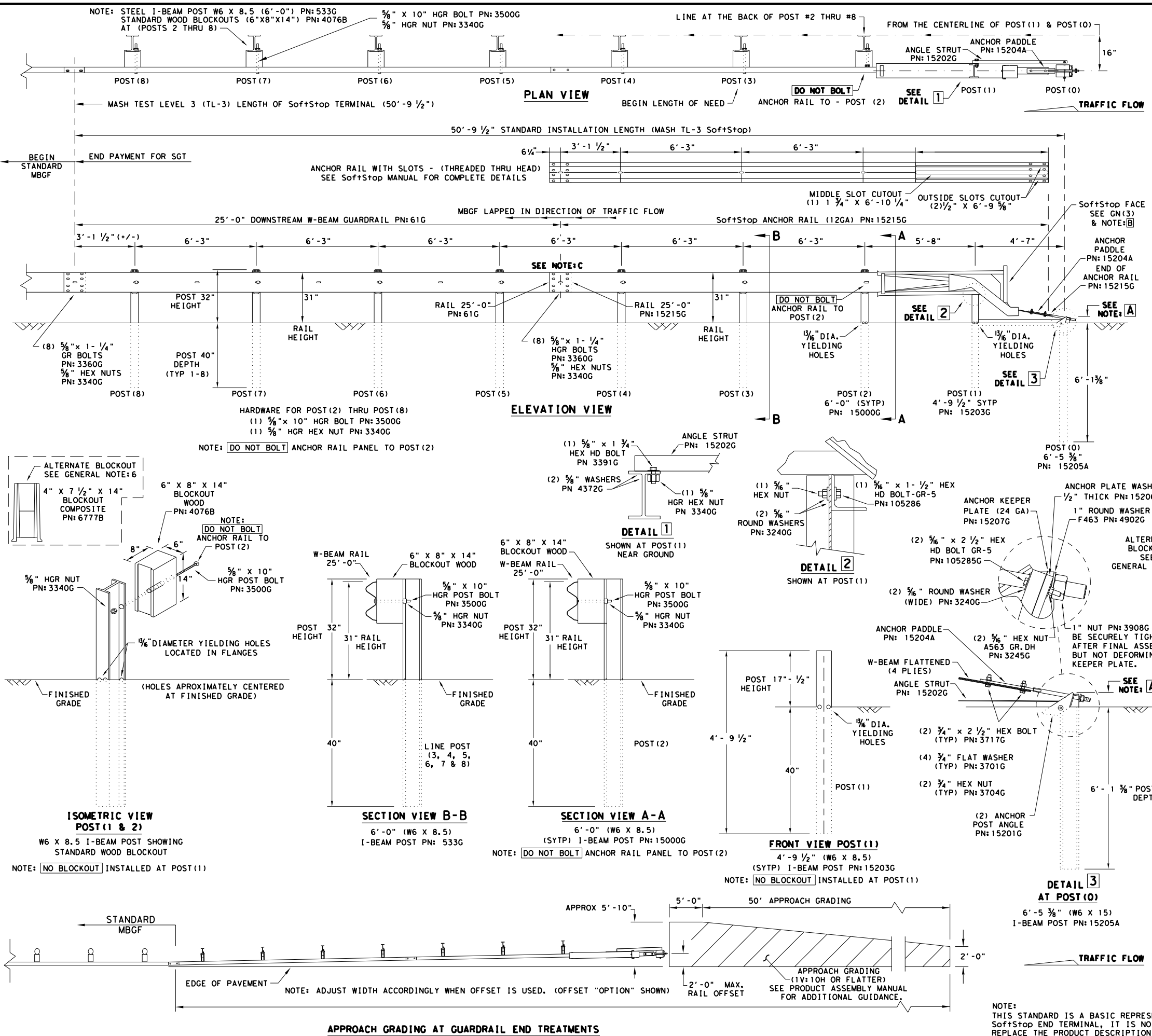


Note: All rail elements shall be lapped in the direction of adjacent traffic.

| | | | |
|--|-----------|---------------------------------|-----------|
| | | Design Division Standard | |
| BRIDGE END DETAILS (METAL BEAM GUARD FENCE APPLICATIONS TO RIGID RAILS) | | | |
| BED-14 | | | |
| FILE: bed14.dgn | DN: TxDOT | CK: AM | DW: BD/VP |
| © TxDOT: December 2011 | CONT | SECT | JOB |
| REVISED APRIL 2014 | 0914 | 05 | 204, ETC. |
| SEE (MEMO 0414) | DIST | COUNTY | SHEET NO. |
| | AUS | WILLIAMSON | 43 |

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- GENERAL NOTES**
- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: TRINITY HIGHWAY AT 1(888)323-6374, 2525 N. STEMMONS FREEWAY, DALLAS, TX 75207
 - FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE SoftStop END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL. PN: 620237B
 - APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
 - FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TxDOT'S LATEST ROADWAY MOW STRIP STANDARD.
 - HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
 - A COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
 - IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL AND REFER TO THE LATEST ROADWAY MBBG STANDARD FOR INSTALLATION GUIDANCE.
 - POSTS SHALL NOT BE SET IN CONCRETE.
 - IT IS ACCEPTABLE TO INSTALL THE SoftStop IMPACT HEAD PARALLEL TO THE GRADE LINE OR WITH AN UPWARD TILT.
 - DO NOT ATTACH THE SoftStop SYSTEM DIRECTLY TO A RIGID BARRIER.
 - UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE SoftStop SYSTEM BE CURVED.
 - A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCRoaching ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.

NOTE: A THE INSTALLATION HEIGHT OF FULLY ASSEMBLED ANCHOR POST WILL VARY FROM 3-3/4" MIN. TO 4" MAX. ABOVE FINISHED GRADE.

NOTE: B PART PN: 5852B RIGHT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING) PART PN: 5851B LEFT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING)

NOTE: C W-BEAM SPLICE LOCATED BETWEEN LINE POST (4) AND LINE POST (5) GUARDRAIL PANEL 25'-0" PN: 61G ANCHOR RAIL 25'-0" PN: 15215G LAP GUARDRAIL IN DIRECTION OF TRAFFIC FLOW.

| PART | QTY | MAIN SYSTEM COMPONENTS |
|----------|-----|--|
| 620237B | 1 | PRODUCT DESCRIPTION ASSEMBLY MANUAL (LATEST REV.) |
| 15208A | 1 | SoftStop HEAD (SEE MANUAL FOR RIGHT-LEFT APPROACH) |
| 15215G | 1 | SoftStop ANCHOR RAIL (12GA) WITH CUTOUT SLOTS |
| 61G | 1 | SoftStop DOWNSTREAM W-BEAM RAIL (12GA) (25'-0") |
| 15205A | 1 | POST #0 - ANCHOR POST (6'-5 3/8") |
| 15203G | 1 | POST #1 - (SYTP) (4'-9 1/2") |
| 15000G | 1 | POST #2 - (SYTP) (6'-0") |
| 533G | 6 | POST #3 THRU #8 - I-BEAM (W6 X 8.5) (6'-0") |
| 4076B | 7 | BLOCKOUT - WOOD (ROUTED) (6" x 8" x 14") |
| 6777B | 7 | BLOCKOUT - COMPOSITE (4" x 7 1/2" x 14") |
| 15204A | 1 | ANCHOR PADDLE |
| 15207G | 1 | ANCHOR KEEPER PLATE (24 GA) |
| 15206G | 1 | ANCHOR PLATE WASHER (1/2" THICK) |
| 15201G | 2 | ANCHOR POST ANGLE (10" LONG) |
| 15202G | 1 | ANGLE STRUT |
| HARDWARE | | |
| 4902G | 1 | 1" ROUND WASHER F436 |
| 3908G | 1 | 1" HEAVY HEX NUT A563 GR.DH |
| 3717G | 2 | 3/4" x 2 1/2" HEX BOLT A325 |
| 3701G | 4 | 3/4" ROUND WASHER F436 |
| 3704G | 2 | 3/4" HEAVY HEX NUT A563 GR.DH |
| 3360G | 16 | 5/8" x 1 1/4" W-BEAM RAIL SPLICE BOLTS HGR |
| 3340G | 25 | 5/8" W-BEAM RAIL SPLICE NUTS HGR |
| 3500G | 7 | 5/8" x 10" HGR POST BOLT A307 |
| 3391G | 1 | 5/8" x 1 3/4" HEX HD BOLT A325 |
| 4489G | 1 | 5/8" x 9" HEX HD BOLT A325 |
| 4372G | 4 | 5/8" WASHER F436 |
| 105285G | 2 | 5/8" x 2 1/2" HEX HD BOLT GR-5 |
| 105286G | 1 | 5/8" x 1 1/2" HEX HD BOLT GR-5 |
| 3240G | 6 | 5/8" ROUND WASHER (WIDE) |
| 3245G | 3 | 5/8" HEX NUT A563 GR.DH |
| 5852B | 1 | HIGH INTENSITY REFLECTIVE SHEETING - SEE NOTE: B |

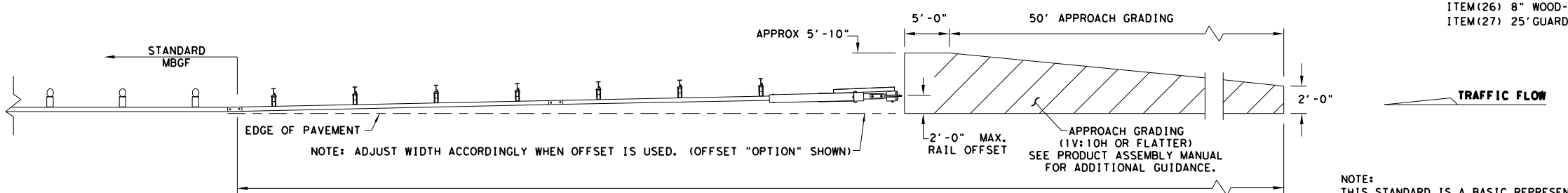
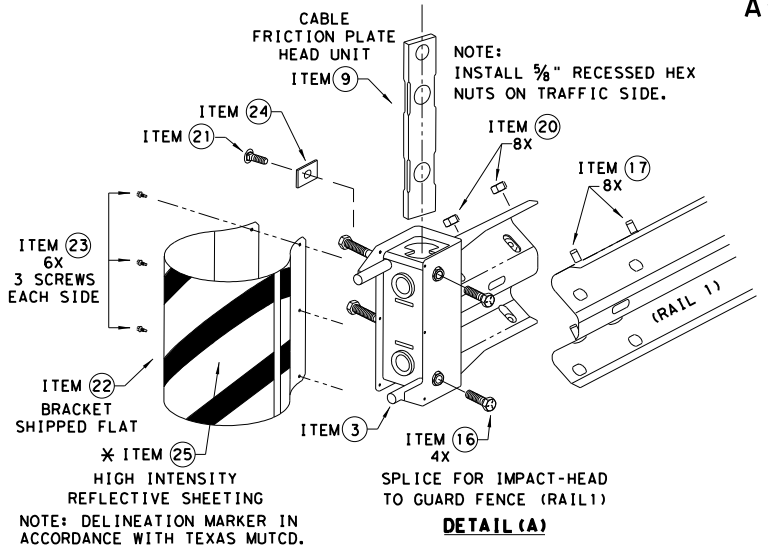
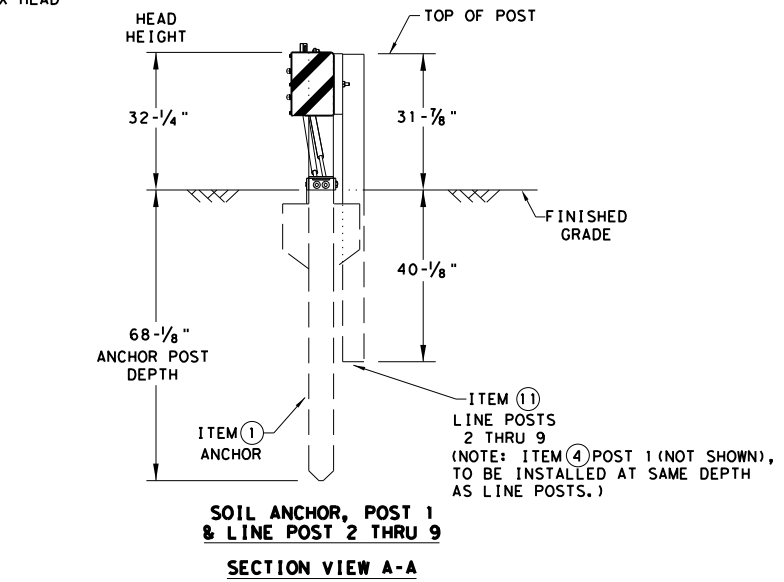
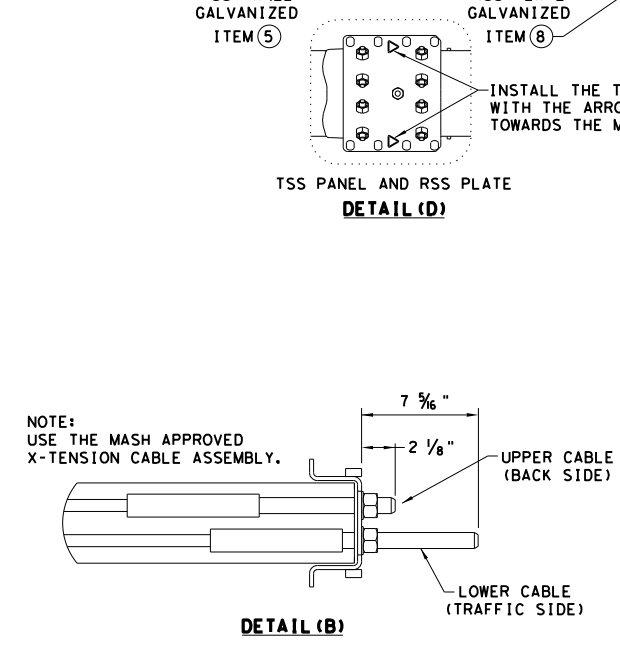
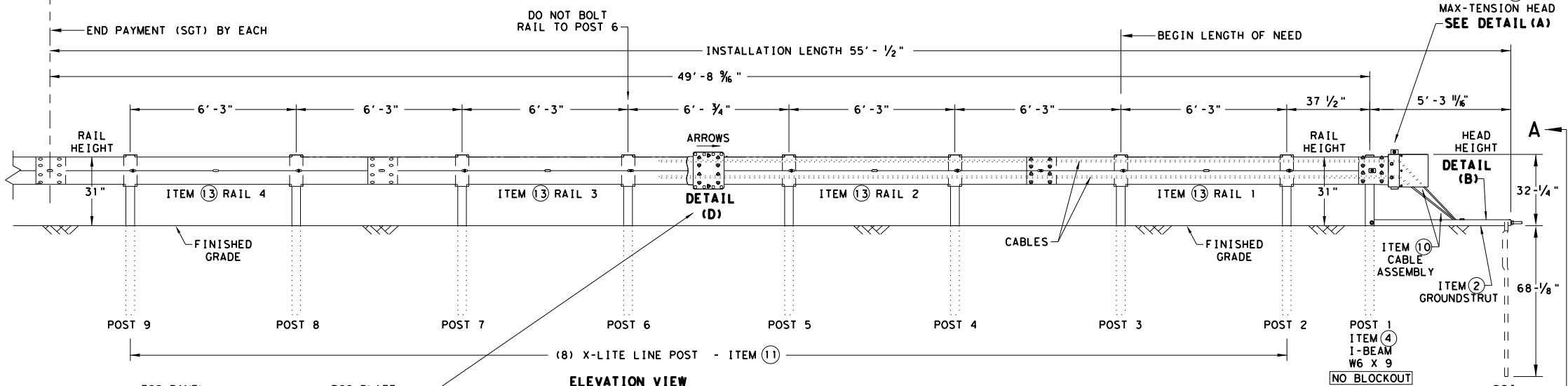
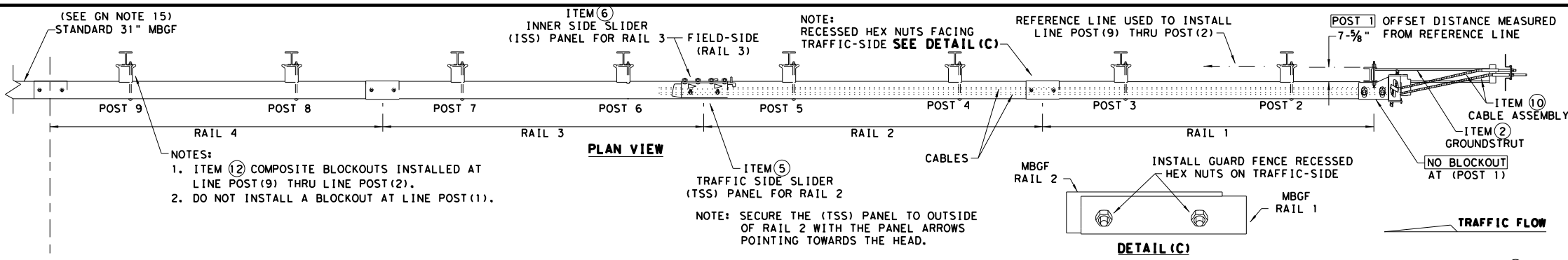
Texas Department of Transportation
 Design Division Standard

**TRINITY HIGHWAY
 SOFTSTOP END TERMINAL
 MASH - TL-3
 SGT (10S) 31-16**

| | | | | |
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GENERAL NOTES

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: LINDSAY TRANSPORTATION SOLUTIONS (LTS) - BARRIER SYSTEMS, INC. AT (707) 374-6800
- FOR INSTALLATION, REPAIR, & MAINTENANCE REFER TO THE MAX-TENSION INSTALLATION INSTRUCTION MANUAL, P/N MANMAX REV D (ECN 3516).
- APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TxDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- ALL STEEL COMPONENTS ARE GALVANIZED PER ASTM A123 OR EQUIVALENT UNLESS OTHERWISE STATED.
- SYSTEM SHOWN USING STEEL WIDE FLANGE POST WITH COMPOSITE BLOCKOUTS.
- COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- REFER TO INSTALLATION MANUAL FOR SPECIFIC PANEL LAPPING GUIDANCE.
- IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL FOR INSTALLATION GUIDANCE.
- POSTS SHALL NOT BE SET IN CONCRETE.
- A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POST TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST.
- MAX-TENSION SYSTEM SHALL NEVER BE INSTALLED WITHIN A CURVED SECTION OF GUARDRAIL.
- IF A DELINEATION MARKER IS REQUIRED, MARKER SHALL BE IN ACCORDANCE WITH TEXAS MUTCD.
- THE SYSTEM IS SHOWN WITH 12'-6" MBGF PANELS, 25'-0" MBGF PANELS ARE ALSO ALLOWED.
- A MINIMUM OF 12'-6" OF 12GA. MBGF IS REQUIRED IMMEDIATELY DOWNSTREAM OF THE MAX-TENSION SYSTEM.

| ITEM # | PART NUMBER | DESCRIPTION | QTY |
|--------|-----------------|--|-----|
| 1 | BSI-1610060-00 | SOIL ANCHOR - GALVANIZED | 1 |
| 2 | BSI-1610061-00 | GROUND STRUT - GALVANIZED | 1 |
| 3 | BSI-1610062-00 | MAX-TENSION IMPACT HEAD | 1 |
| 4 | BSI-1610063-00 | W6x9 I-BEAM POST 6FT. -GALVANIZED | 1 |
| 5 | BSI-1610064-00 | TSS PANEL - TRAFFIC SIDE SLIDER | 1 |
| 6 | BSI-1610065-00 | ISS PANEL - INNER SIDE SLIDER | 1 |
| 7 | BSI-1610066-00 | TOOTH - GEOMET | 1 |
| 8 | BSI-1610067-00 | RSS PLATE - REAR SIDE SLIDER | 1 |
| 9 | B061058 | CABLE FRICTION PLATE - HEAD UNIT | 1 |
| 10 | BSI-1610069-00 | CABLE ASSEMBLY - MASH X-TENSION | 2 |
| 11 | BSI-1012078-00 | X-LITE LINE POST - GALVANIZED | 8 |
| 12 | B090534 | 8" W-BEAM COMPOSITE-BLOCKOUT XT110 | 8 |
| 13 | BSI-4004386 | 12'-6" W-BEAM GUARD FENCE PANELS 12GA. | 4 |
| 14 | BSI-1102027-00 | X-LITE SQUARE WASHER | 1 |
| 15 | BSI-2001886 | 3/8" X 7" THREAD BOLT HH (GR.5)GEOMET | 1 |
| 16 | BSI-2001885 | 3/4" X 3" ALL-THREAD BOLT HH (GR.5)GEOMET | 4 |
| 17 | 4001115 | 5/8" X 1 1/4" GUARD FENCE BOLTS (GR.2)MGAL | 48 |
| 18 | 2001840 | 5/8" X 10" GUARD FENCE BOLTS MGAL | 8 |
| 19 | 2001636 | 5/8" WASHER F436 STRUCTURAL MGAL | 2 |
| 20 | 4001116 | 5/8" RECESSED GUARD FENCE NUT (GR.2)MGAL | 59 |
| 21 | BSI-2001888 | 3/8" X 2" ALL THREAD BOLT (GR.5)GEOMET | 1 |
| 22 | BSI-1701063-00 | DELINEATION MOUNTING (BRACKET) | 1 |
| 23 | BSI-2001887 | 1/4" X 3/4" SCREW SD HH 410SS | 7 |
| 24 | 4002051 | GUARDRAIL WASHER RECT AASHTO FWRO3 | 1 |
| 25 | SEE NOTE BELOW | HIGH INTENSITY REFLECTIVE SHEETING | 1 |
| 26 | 4002337 | 8" W-BEAM TIMBER-BLOCKOUT, PDB01B | 8 |
| 27 | BSI-4004431 | 25' W-BEAM GUARDRAIL PANEL, 8-SPACE, 12GA. | 2 |
| 28 | MANMAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTIONS | 1 |

* TO BE PROVIDED BY DISTRIBUTOR OR CONTRACTOR.
 ** ALTERNATIVE ITEMS NOT SHOWN.
 ITEM (26) 8" WOOD-BLOCKOUTS
 ITEM (27) 25' GUARD FENCE PANELS

Texas Department of Transportation
 Design Division Standard

**MAX-TENSION END TERMINAL
 MASH - TL-3**

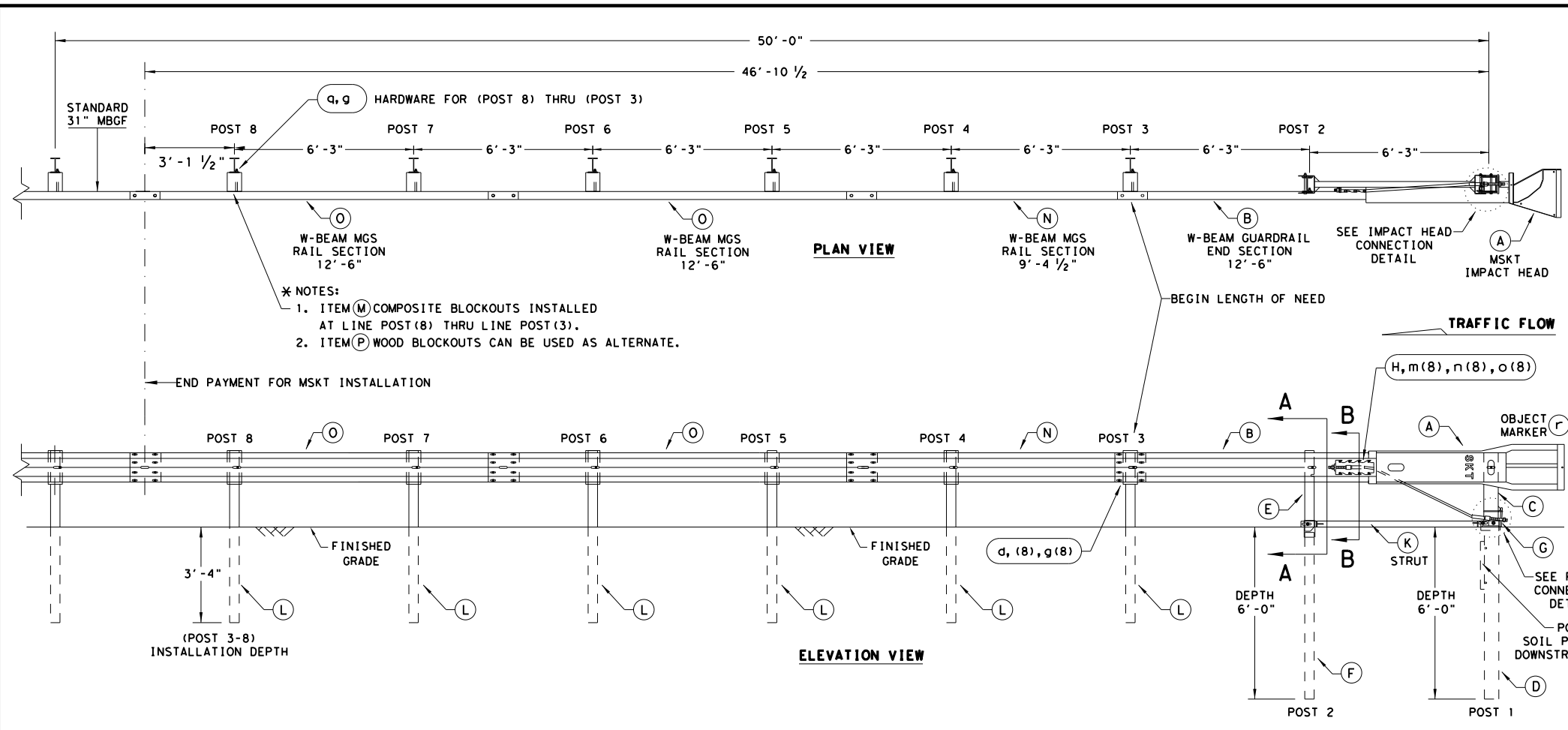
SGT (11S) 31-18

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| © TxDOT: FEBRUARY 2018 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| AUS | WILLIAMSON | | | 45 |

NOTE: THIS STANDARD IS A BASIC REPRESENTATION OF THE MAX-TENSION END TERMINAL, IT IS NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL.

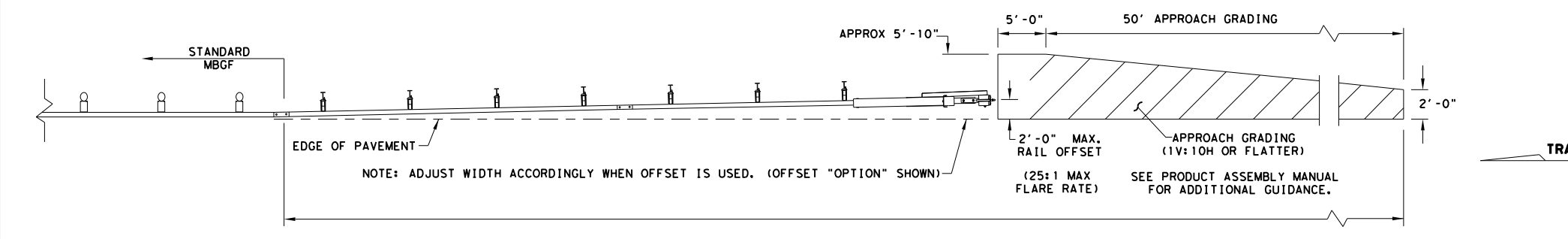
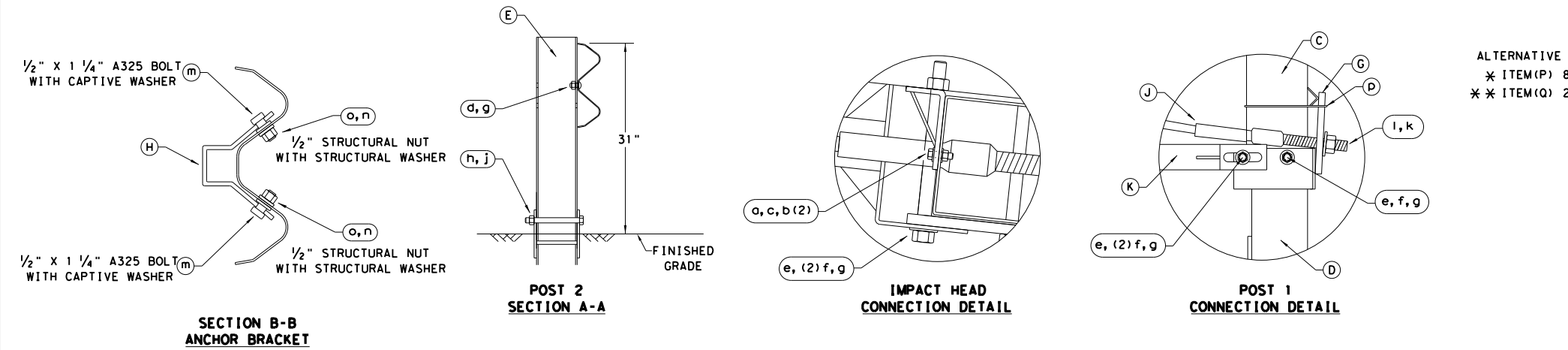
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- GENERAL NOTES**
- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: ROAD SYSTEMS, INC. (432)263-2435. 3616 OLD HOWARD COUNTY AIRPORT, BIG SPRING, TX 79720
 - FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE: MSKT END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL (PUBLICATION-062717).
 - APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
 - FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
 - HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
 - SYSTEM SHOWN USING STEEL WIDE FLANGE POSTS WITH COMPOSITE BLOCKOUTS.
 - A COMPOSITE MATERIAL BLOCKOUTS THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
 - IF SOLID ROCK IS ENCOUNTERED IN THE AREA OF (POST 1) AND / OR (POST 2) CONTACT THE MANUFACTURER, & REFER TO THE LATEST ROADWAY MOW STRIP STANDARD FOR INSTALLATION GUIDANCE.
 - POSTS SHALL NOT BE SET IN CONCRETE.
 - SYSTEM MUST BE ATTACHED TO STANDARD 31" MBGF.
 - UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE MSKT SYSTEM BE CURVED.
 - A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCRANCHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.
 - THE SYSTEM IS SHOWN WITH TWO 12'-6" MBGF PANELS, ONE 25'-0" MBGF PANEL IS ALSO ALLOWED IN ITS PLACE.
 - A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POSTS 3-8 TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST. SPECIAL DRIVING CAP TO BE USED ON LOWER POSTS 1 & 2 TO PREVENT DAMAGE TO THE WELDED PLATES.

| ITEM | QTY | MAIN SYSTEM COMPONENTS | ITEM NUMBERS |
|----------------|-----|---|--------------|
| A | 1 | MSKT IMPACT HEAD | MS3000 |
| B | 1 | W-BEAM GUARDRAIL END SECTION, 12 Go. | SF1303 |
| C | 1 | POST 1 - TOP (6" X 6" X 1/8" TUBE) | MTPHP1A |
| D | 1 | POST 1 - BOTTOM (6' W6X15) | MTPHP1B |
| E | 1 | POST 2 - ASSEMBLY TOP | UHP2A |
| F | 1 | POST 2 - ASSEMBLY BOTTOM (6' W6X9) | HP2B |
| G | 1 | BEARING PLATE | E750 |
| H | 1 | CABLE ANCHOR BOX | S760 |
| J | 1 | BCT CABLE ANCHOR ASSEMBLY | E770 |
| K | 1 | GROUND STRUT | MS785 |
| L | 6 | W6X9 OR W6X8.5 STEEL POST | P621 |
| M | 6 | COMPOSITE BLOCKOUTS | CBSP-14 |
| N | 1 | W-BEAM MGS RAIL SECTION (9'-4 1/2") | G12025 |
| O | 2 | W-BEAM MGS RAIL SECTION (12'-6") | G1203A |
| P | 6 | WOOD BLOCKOUT 6" X 8" X 14" | P675 |
| Q | 1 | W-BEAM MGS RAIL SECTION (25'-0") | G1209 |
| SMALL HARDWARE | | | |
| o | 2 | 5/8" x 1" HEX BOLT (GRD 5) | B5160104A |
| b | 4 | 5/8" WASHER | W0516 |
| c | 2 | 5/8" HEX NUT | N0516 |
| d | 25 | 5/8" Dia. x 1 1/4" SPLICE BOLT (POST 2) | B580122 |
| e | 2 | 5/8" Dia. x 9" HEX BOLT (GRD A449) | B580904A |
| f | 3 | 5/8" WASHER | W050 |
| g | 33 | 5/8" Dia. H.G.R NUT | N050 |
| h | 1 | 3/4" Dia. x 8 1/2" HEX BOLT (GRD A449) | B340854A |
| j | 1 | 3/4" Dia. HEX NUT | N030 |
| k | 2 | 1 ANCHOR CABLE HEX NUT | N100 |
| l | 2 | 1 ANCHOR CABLE WASHER | W100 |
| m | 8 | 1/2" x 1 1/4" A325 BOLT WITH CAPTIVE WASHER | SB12A |
| n | 8 | 1/2" STRUCTURAL NUTS | N012A |
| o | 8 | 1 1/8" O.D. x 3/8" I.D. STRUCTURAL WASHERS | W012A |
| p | 1 | BEARING PLATE RETAINER TIE | CT-100ST |
| q | 6 | 5/8" x 10" H.G.R. BOLT | B581002 |
| r | 1 | OBJECT MARKER 18" X 18" | E3151 |



NOTE: TXDOT GENERIC APPROACH GRADING LAYOUT USED FOR ALL TANGENT TYPE END TREATMENTS.

NOTE: THIS STANDARD IS A BASIC REPRESENTATION OF THE MSKT END TERMINAL, IT IS NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL.

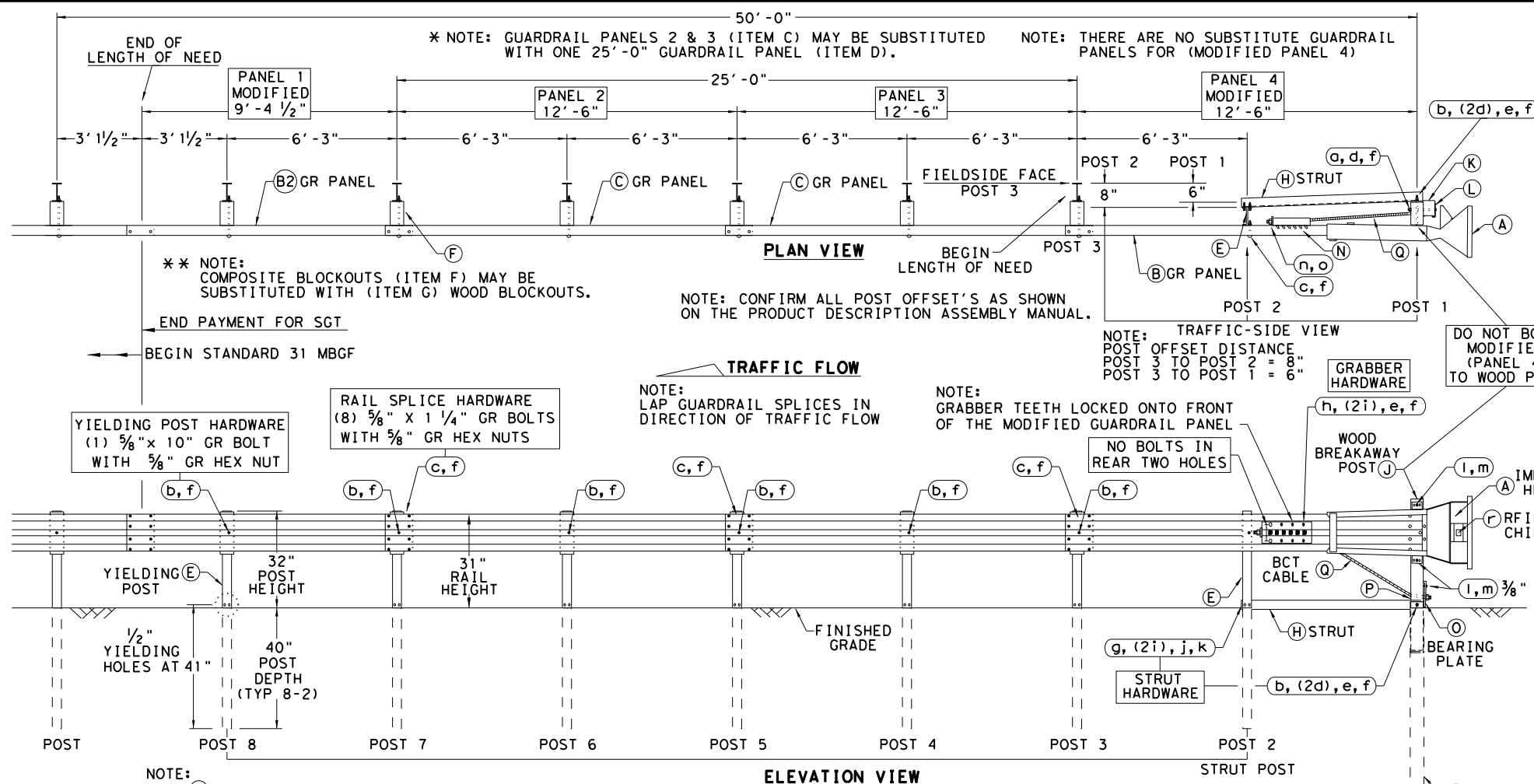
Design Division Standard

SINGLE GUARDRAIL TERMINAL
 MSKT-MASH-TL-3
 SGT (12S) 31-18

| | | | | |
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| © TXDOT: APRIL 2018 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | | 0914 | 05 204, ETC. | CR 118 |
| DIST | COUNTY | | | SHEET NO. |
| AUS | WILLIAMSON | | | 46 |

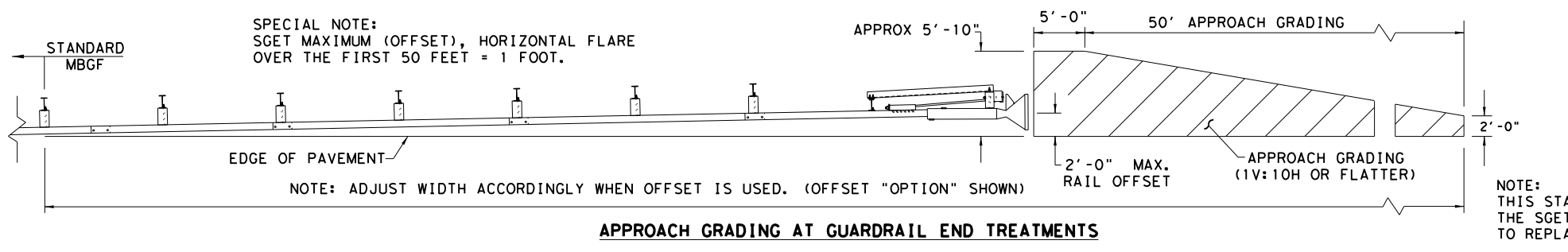
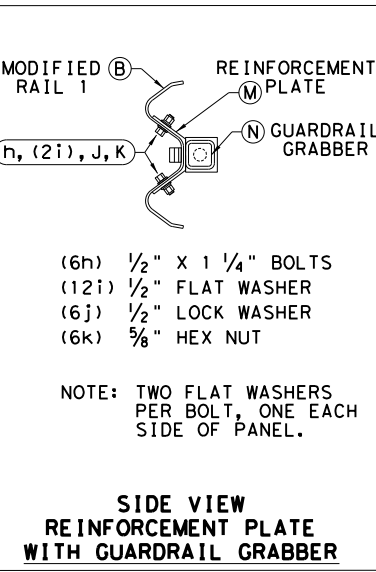
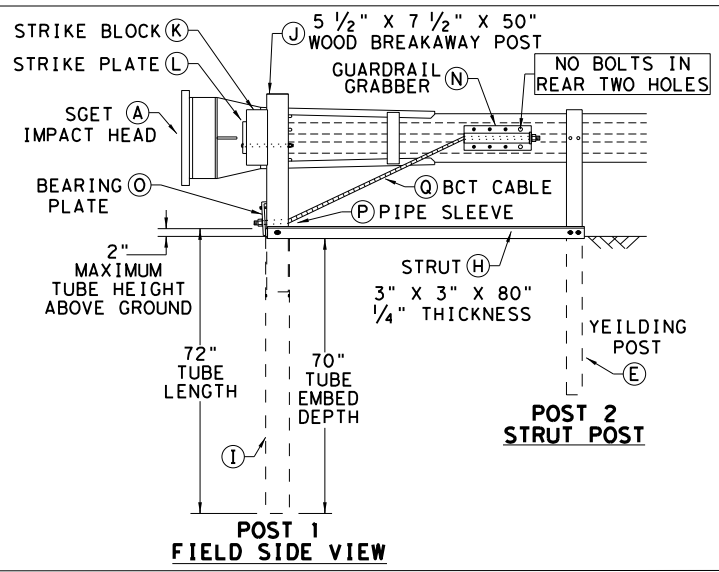
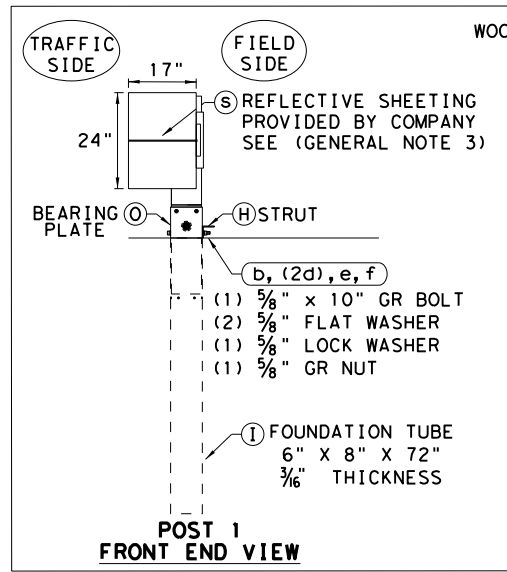
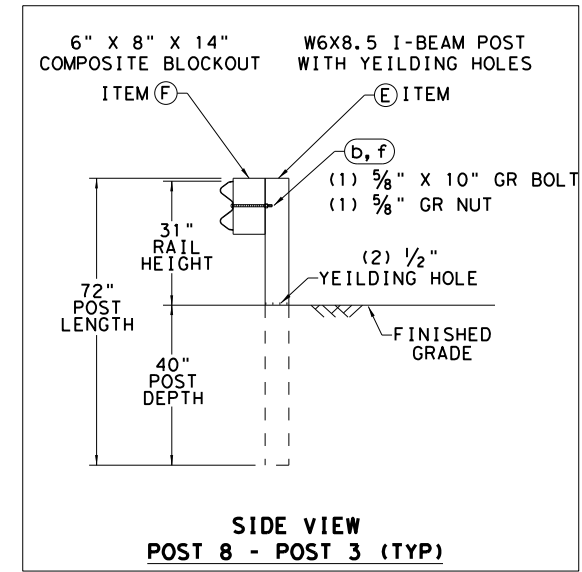
DISCLAIMER: THE USE OF THIS STANDARD IS GOVERNED BY THE "TEXAS ENGINEERING PRACTICE ACT". NO WARRANTY OF ANY KIND IS MADE BY TXDOT FOR ANY PURPOSE WHATSOEVER. TXDOT ASSUMES NO RESPONSIBILITY FOR THE CONVERSION OF THIS STANDARD TO OTHER FORMATS OR FOR INCORRECT RESULTS OR DAMAGES RESULTING FROM ITS USE.

DATE: 11/23/2022
 FILE: c:\pwworking\10221691\sgt153120.dgn



- GENERAL NOTES**
- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202
 - FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL.
 - MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
 - THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.
 - FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
 - (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS.
 - POSTS SHALL NOT BE SET IN CONCRETE.
 - IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.
 - HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
 - A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
 - THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.

| ITEM | QTY | MAIN SYSTEM COMPONENTS | ITEM # |
|----------------|-----|--|----------|
| A | 1 | SGET IMPACT HEAD | SIH1A |
| B | 1 | MODIFIED GUARDRAIL PANEL 12'-6" 12GA | 126SPZGP |
| B2 | 1 | MODIFIED GUARDRAIL PANEL 9'-4 1/2" 12GA | GP94 |
| C | 2 | STANDARD GUARDRAIL PANEL 12'-6" 12GA | GP126 |
| D | 1 | STANDARD GUARDRAIL PANEL 25'-0" 12GA | GP25 |
| E | 7 | MODIFIED YIELDING I-BEAM POST W6x8.5 | YP6MOD |
| F | 6 | COMPOSITE BLOCKOUT 6" X 8" X 14" | CBO8 |
| G | 6 | WOOD BLOCKOUT 6" X 8" X 14" | WBO8 |
| H | 1 | STRUT 3" X 3" X 80" X 1/4" A36 ANGLE | STR80 |
| I | 1 | FOUNDATION TUBE 6" X 8" X 72" X 3/8" | FNDT6 |
| J | 1 | WOOD BREAKAWAY POST 5 1/2" X 7 1/2" X 50" | WBRK50 |
| K | 1 | WOOD STRIKE BLOCK | WSBK14 |
| L | 1 | STRIKE PLATE 1/4" A36 BENT PLATE | SPLT8 |
| M | 1 | REINFORCEMENT PLATE 12 GA. GR55 | REPLT17 |
| N | 1 | GUARDRAIL GRABBER 2 1/2" X 2 1/2" X 16 1/2" | GGR17 |
| O | 1 | BEARING PLATE 8" X 8 5/8" X 5/8" A36 | BPLT8 |
| P | 1 | PIPE SLEEVE 4 1/4" X 2 3/8" O.D. (2 1/8" I.D.) | PSLV4 |
| Q | 1 | BCT CABLE 3/4" X 81" LENGTH | CBL81 |
| SMALL HARDWARE | | | |
| q | 1 | 5/8" X 12" GUARDRAIL BOLT 307A HDG | 12GRBLT |
| b | 7 | 5/8" X 10" GUARDRAIL BOLT 307A HDG | 10GRBLT |
| c | 33 | 5/8" X 1 1/4" GR SPlice BOLTS 307A HDG | 1GRBLT |
| d | 3 | 5/8" FLAT WASHER F436 A325 HDG | 58FW436 |
| e | 1 | 5/8" LOCK WASHER HDG | 58LW |
| f | 39 | 5/8" GUARDRAIL HEX NUT HDG | 58HN563 |
| g | 2 | 1/2" X 2" STRUT BOLT A325 HDG | 2BLT |
| h | 6 | 1/2" X 1 1/4" PLATE BOLT A325 HDG | 125BLT |
| i | 16 | 1/2" FLAT WASHER F436 A325 HDG | 12FWF436 |
| j | 8 | 1/2" LOCK WASHER HDG | 12LW |
| k | 8 | 1/2" HEX NUT A563 HDG | 12HN563 |
| l | 4 | 3/8" X 3" HEX LAG SCREW GR5 HDG | 38LS |
| m | 4 | 3/8" FLAT WASHER F436 A325 HDG | 38FW844 |
| n | 2 | 1" FLAT WASHER F436 A325 HDG | 1FWF436 |
| o | 2 | 1" HEX NUT A563HD HDG | 1HN563 |
| p | 1 | 18" TO 24" LONG ZIP TIE RATED 175-200LB | ZPT18 |
| q | 1 | 1 1/2" X 4" SCH-40 PVC PIPE | PSPCR4 |
| r | 1 | RFID CHIP RATED MIL-STD-810F | RFID810F |
| s | 1 | IMPACT HEAD REFLECTIVE SHEETING | RS30M |



NOTE: THIS STANDARD IS A BASIC REPRESENTATION OF THE SGET TERMINAL SYSTEM AND IS NOT INTENDED TO REPLACE THE MANUFACTURER'S ASSEMBLY MANUAL.

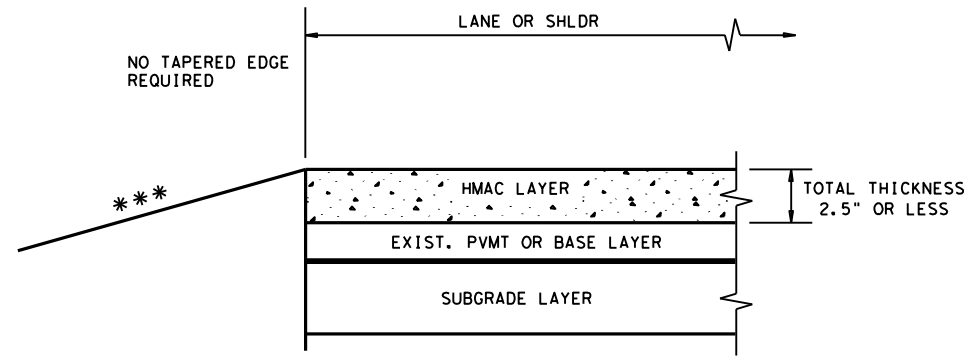
Texas Department of Transportation
 Design Division Standard

SPIG INDUSTRY, LLC
SINGLE GUARDRAIL TERMINAL
SGET - TL-3 - MASH
SGT (15) 31-20

| | | | | |
|---------------------|-----------|------------|-----------|---------|
| FILE: sg153120.dgn | DN: TXDOT | CK: KM | DW: VP | CK: VP |
| © TXDOT: APRIL 2020 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 47 | |

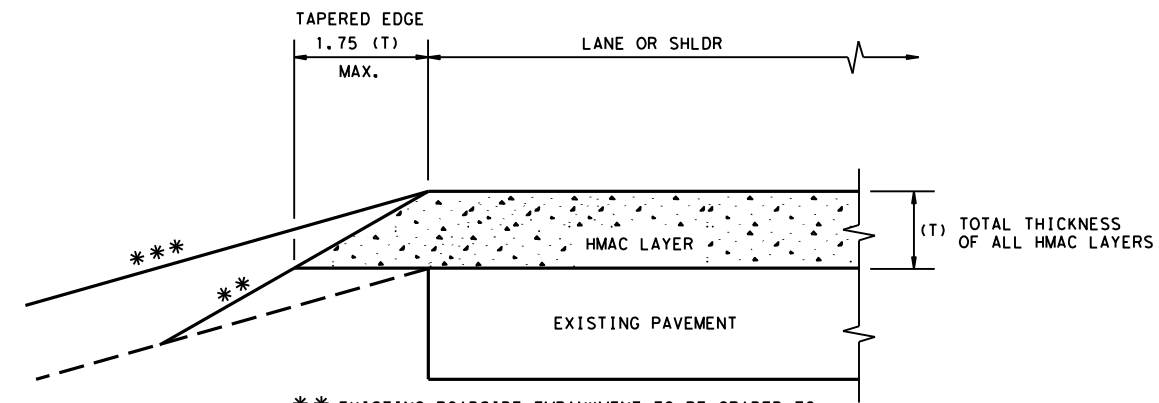
DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TxDOT for any purpose whatsoever. TxDOT assumes no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use.

DATE: 11/3/2022
 FILE: c:\pwworkh\1\0221691\tehmoc11 (3).dgn



*** SEE TYPICAL SECTION FOR ROADSIDE DETAILS

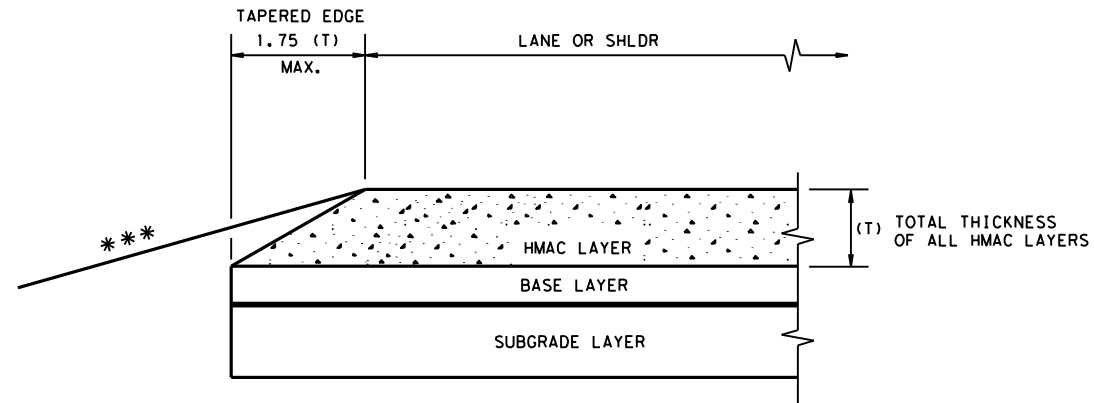
CONDITION - 1
 THIN HMAC SURFACES OR HMAC OVERLAY
 WITH THICKNESS OF 2.5" OR LESS



** EXISTING ROADSIDE EMBANKMENT TO BE GRADED TO PRODUCE A SMOOTH LEVEL SURFACE FOR PLACEMENT OF TAPERED EDGE. THIS WORK IS SUBSIDIARY TO THE VARIOUS BID ITEMS.

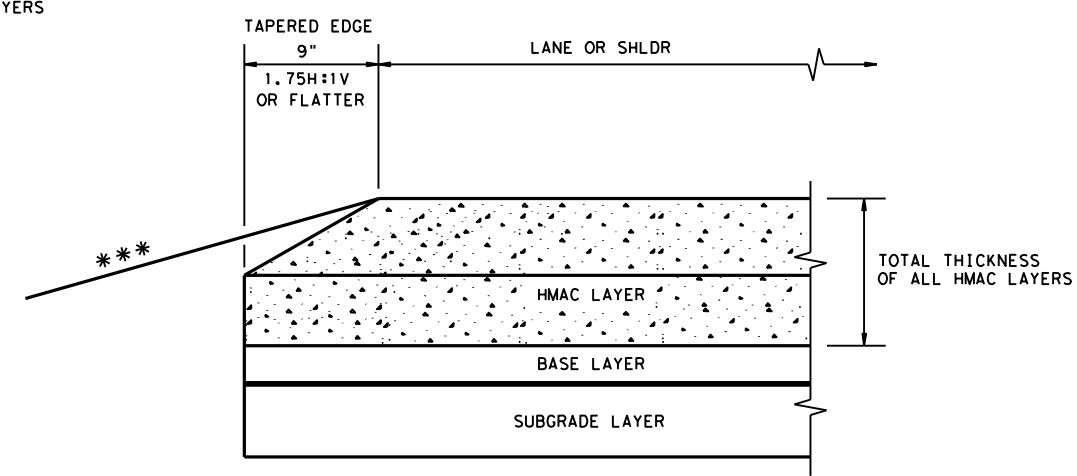
*** SEE TYPICAL SECTION FOR ROADSIDE DETAILS

CONDITION - 2
 OVERLAY OF EXISTING PAVEMENT
 HMAC THICKNESS 2.5" TO 5"



*** SEE TYPICAL SECTION FOR ROADSIDE DETAILS

CONDITION - 3
 NEW OR RECONSTRUCTED PAVEMENT
 HMAC THICKNESS 2.5" TO 5"



*** SEE TYPICAL SECTION FOR ROADSIDE DETAILS

CONDITION - 4
 NEW OR RECONSTRUCTED PAVEMENT
 HMAC THICKNESS 5" OR GREATER

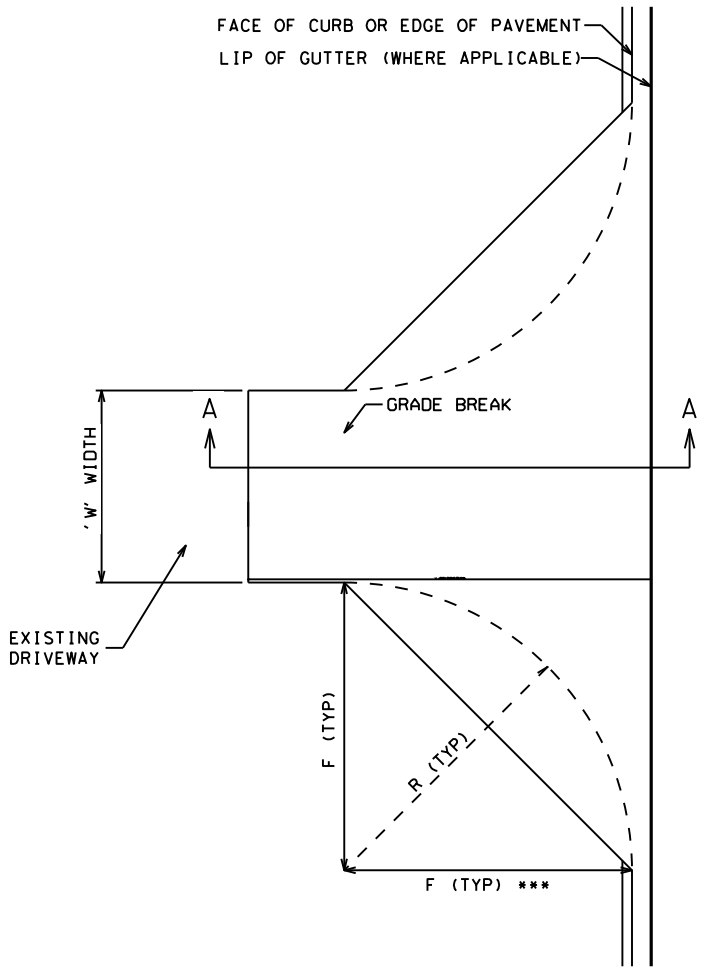
GENERAL NOTES

- UNLESS OTHERWISE SHOWN IN THE PLANS, A VERTICAL EDGE IS PERMISSIBLE FOR HMAC PLACED GREATER THAN 5" BELOW THE EDGE OF PAVEMENT AND FOR THICKNESS OF HMAC LESS THAN 2.5".
- FOR FURTHER INFORMATION REGARDING THE ROADSIDE AND PAVEMENT DETAILS, SEE TYPICAL SECTIONS.
- PAYMENT FOR TAPERED EDGE WILL BE IN ACCORDANCE WITH APPLICABLE ITEMS IN THE CONTRACT.
- THE SLOPE OF THE TAPERED EDGE SHALL BE 1.75H:1V OR FLATTER.
- THE TAPERED EDGE SHALL BE PRODUCED BY USE OF A SCREED ATTACHMENT CAPABLE OF PRODUCING A SMOOTH COMPACTED SURFACE. ADDITIONAL COMPACTING EFFORT BEHIND THE SCREED IS NOT REQUIRED.

(NOT TO SCALE)

| | | | | | | |
|--|------------|--------|-----------|-----------|--------------------------|--|
| | | | | | Design Division Standard | |
| TAPERED EDGE DETAILS HMAC PAVEMENT | | | | | | |
| TE (HMAC) - 11 | | | | | | |
| FILE: tehmoc11.dgn | DN: TxDOT | CK: RL | DW: KB | CK: | | |
| © TxDOT January 2011 | CONT | SECT | JOB | HIGHWAY | | |
| REVISIONS | | | 0914 05 | 204, ETC. | CR 118 | |
| DIST | COUNTY | | SHEET NO. | | | |
| AUS | WILLIAMSON | | 48 | | | |

FACE OF CURB OR EDGE OF PAVEMENT
LIP OF GUTTER (WHERE APPLICABLE)



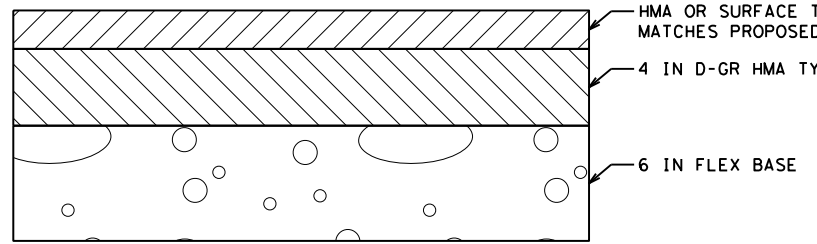
DRIVEWAY PLAN

| FLARE OR RADIUS | FARM/RANCH | RESIDENTIAL | COMMERCIAL |
|-----------------|------------|-------------|------------|
| "F" OR "R" (FT) | 25 | 25 | 25 |

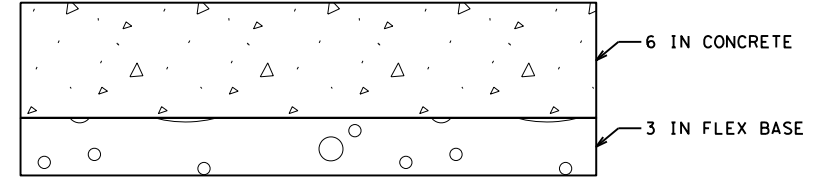
THESE ARE STANDARD DIMENSIONS UNLESS OTHERWISE SHOWN ELSEWHERE ON THE PLANS.

FLARES ARE TYPICALLY USED FOR SUBURBAN/URBAN (CURBED) ROADWAYS. RADII ARE TYPICALLY USED FOR RURAL OR UNCURBED ROADWAYS.

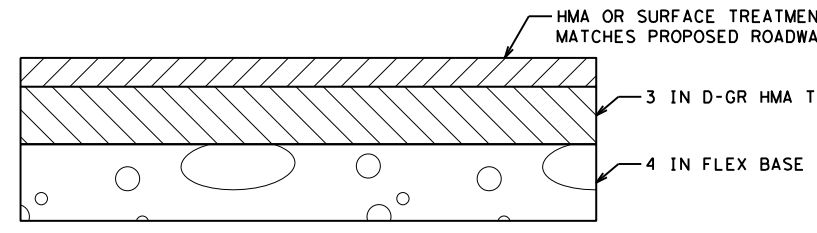
*** THIS 'F' DIMENSION MAY BE REDUCED TO KEEP WORK WITHIN THE ROW.



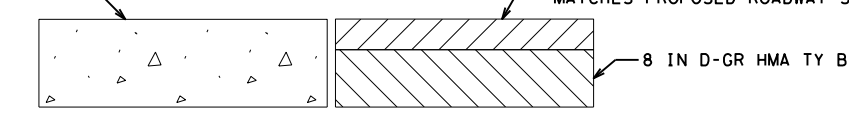
HMA OR SURFACE TREATMENT - COMMERCIAL



CONCRETE - ALL DRIVEWAY TYPES

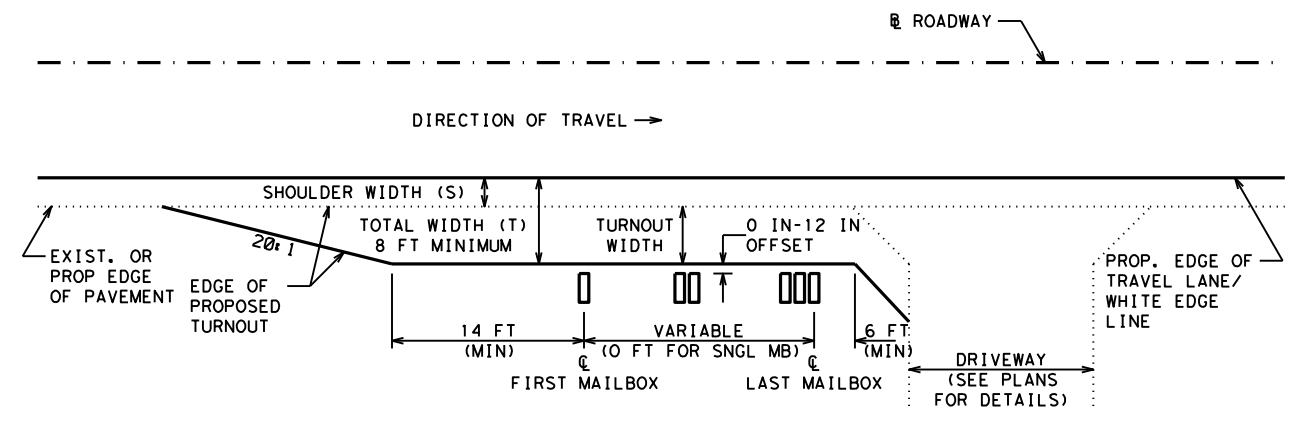


HMA OR SURFACE TREATMENT - FARM/RANCH/RESIDENTIAL

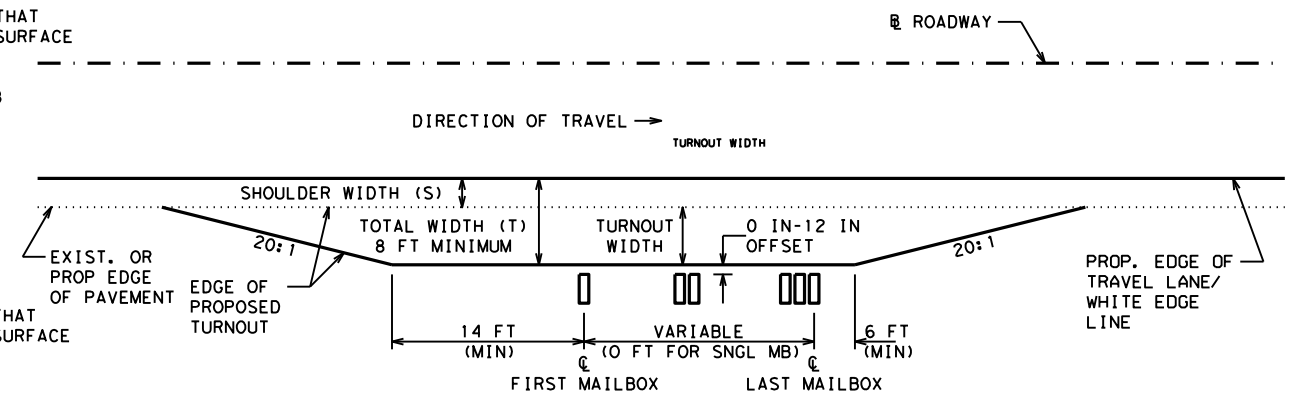


FAST TRACK ACP (TYPE 3) OR CONCRETE

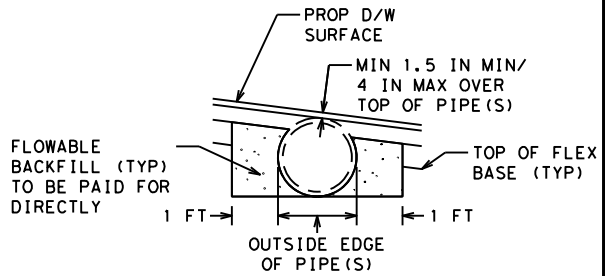
DRIVEWAY AND TURNOUT TYPICAL SECTIONS



MAILBOX TURNOUT PLAN WITH DRIVEWAY

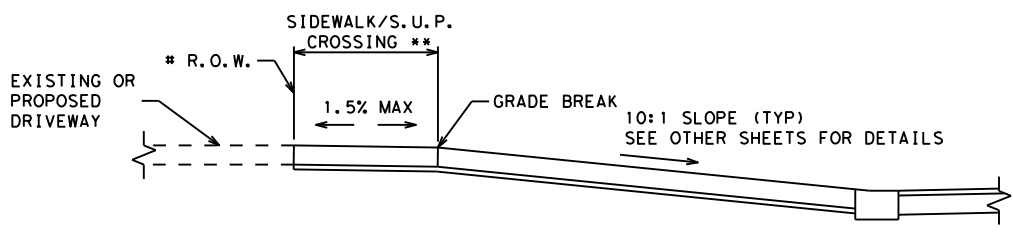


MAILBOX TURNOUT PLAN WITHOUT DRIVEWAY



LOW FILL DRIVEWAY

ONLY ONE PIPE SHOWN SEE ELSEWHERE ON THE PLANS FOR SPECIFIC DRIVEWAY DETAILS



DRIVEWAY WITH GUTTER SECTION A-A

ENSURE GRADE BREAK DOES NOT EXCEED 8% UNLESS OTHERWISE DIRECTED. PROVIDE ABSOLUTE MINIMUM SIDEWALK CROSSING WIDTH OF 4' FOR DRIVEWAYS WIDTH OF 20' OR LESS

** LOCATE SIDEWALK CROSSING TO ALIGN WITH ADJACENT SIDEWALK; SIDEWALK/S.U.P. WIDTH AND LOCATION SHOWN ELSEWHERE ON THE PLANS.

GENERAL NOTES

PROVIDE EXPANSION 20 FT C-C FOR WIDTH OR LENGTH OVER 25 FT. EXPANSION JOINT PER AUS STANDARD FOR SIDEWALK (MCPSWMD).

REINFORCEMENT WILL BE IN ACCORDANCE WITH ITEM 432.3.1 USING NO. 3 OR NO. 4 BARS.

FIBER REINFORCEMENT IS NOT ALLOWED. CLASS A CONCRETE IS ALLOWED TO USE COARSE AGGREGATE GRADES 1-8.

IN LIEU OF PFC OR TOM, SURFACE MUST BE 1.5" D-GR HMA TY D. IF SURFACE IS A MULTIPLE COURSE SURFACE TREATMENT, ALL COURSES MUST BE PLACED ON DRIVEWAY. SURFACE HMA IS PG 76-22. NON SURFACE HMA IS PG 64-22 AND MAY BE BLADE LAID.

FURNISH BASE MEETING THE REQUIREMENTS FOR ANY TYPE OR GRADE IN ACCORDANCE WITH ITEM 247. BASE COMPRESSIVE STRENGTHS ARE WAIVED.

THE BASE UNDER THE CONCRETE MAY BE REPLACED WITH CONCRETE AT A RATIO OF 3 INCHES OF BASE EQUALS 2 INCHES OF CONCRETE.

FAST TRACK DRIVEWAYS MUST BE CLOSED, CONSTRUCTED, AND REOPENED WITHIN 24 HOURS.

IF ROOTS ARE ENCOUNTERED VERIFY WITH THE ENGINEER PRIOR TO ACCOMMODATING OR REMOVING 2 IN. DIAMETER OR LARGER ROOTS. ROOT REMOVAL MUST BE IN ACCORDANCE WITH ITEM 752.4.2. ROOTS MAY REMAIN IN THE BASE. FOR IMPROVEMENTS WITHIN 6 IN. OF A ROOT, THE CONCRETE THICKNESS MAY BE REDUCED BY 1 IN. AND THE BASE INCREASED BY 1 IN. TO MINIMIZE IMPACTS TO THE ROOTS. ADJUST BASE AND SURFACE PROFILE TO PROVIDE A 1 IN. BASE CUSHION AROUND THE ROOTS. THE SURFACE PROFILE MAY BE ADJUSTED TO THE EXTENT ALLOWED BY ADA. THIS WORK IS SUBSIDIARY.

Texas Department of Transportation Austin District Standard

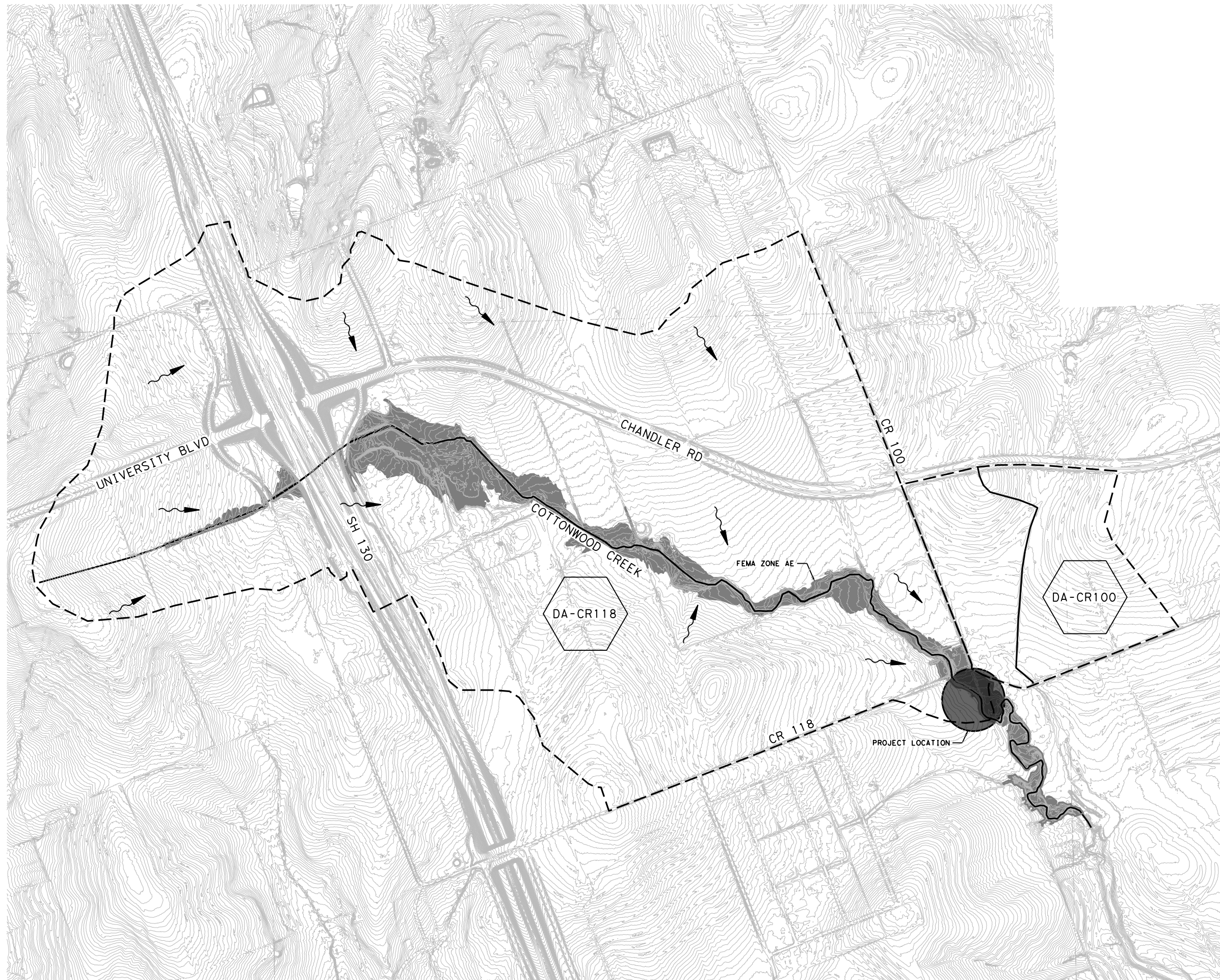
DRIVEWAYS AND MAILBOX TURNOUTS

DWMB-22 (AUS)

| | | | | |
|---|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| 01/16/16 SHEET CREATED | 0914 | 05 | 204, ETC. | CR 118 |
| 04/19/19 APPROVED | DIST | COUNTY | | SHEET NO. |
| 11/20/21 TABLE REVISED, GN ADDED, PLAN & PROFILE MODIFIED | AUS | WILLIAMSON | | 49 |
| 01/22/22 ADDED TURNOUT INFO | | | | |

DATE: 11/3/2022 12:51:27 PM
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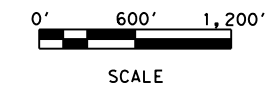


LEGEND

- DRAINAGE AREA BOUNDARY
- LONGEST FLOW PATH
- EXISTING CONTOURS
- FEMA ZONE AE
- FLOW ARROWS

NOTES:

1. REFER TO THE HYDROLOGIC DATA SHEET FOR DETAILED CALCULATIONS.
2. COTTONWOOD CREEK IS LOCATED IN A FEMA ZONE AE FLOODPLAIN PER FIRM PANEL 48491C0505F DATED DECEMBER 20, 2019.
3. DRAINAGE AREA DELINEATED BASED ON TNRIS 2016 LIDAR 1 FT CONTOURS.



ASJ
 11/3/2022

 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

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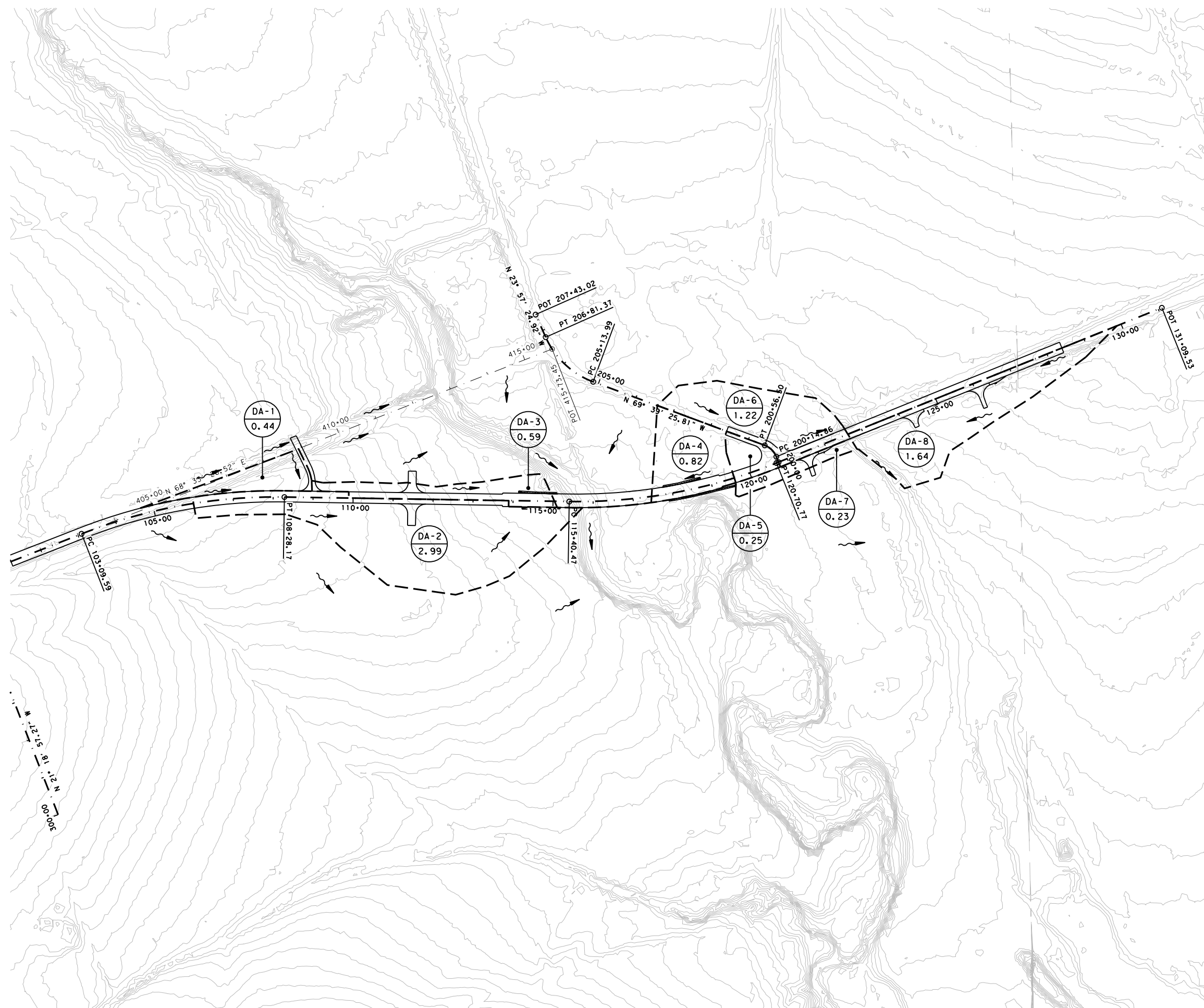
CR 118 AT COTTONWOOD CREEK

EXTERNAL DRAINAGE AREA MAP

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 50 |

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LEGEND

- DA-X
X.XX AREA ID
AREA (ACRES)
- DRAINAGE AREA
- ~> DIRECTION OF FLOW

AJH
 11/3/2022
 STATE OF TEXAS
 AUSTIN S. HELTON
 124922
 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

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CR 118 AT COTTONWOOD CREEK

INTERNAL DRAINAGE AREA MAP

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
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| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 51 |

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| PROPOSED DITCH ANALYSIS - DITCH DA-1 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 106+50 | 721.72 | | | | | | | | | | | |
| 107+00 | 719.86 | 3.72% | CLASS C | 0 | 1.28 | 0.23 | 6 | 4 | 0.03 | 2.20 | 0.56 | 0.52 |
| 107+50 | 719.22 | 1.28% | CLASS C | 0 | 1.71 | 0.28 | 6 | 4 | 0.03 | 1.48 | 0.56 | 0.22 |
| 108+00 | 718.55 | 1.34% | CLASS C | 0 | 2.50 | 0.27 | 6 | 4 | 0.03 | 1.50 | 0.56 | 0.23 |
| 108+50 | 717.78 | 1.54% | CLASS C | 0 | 2.44 | 0.27 | 6 | 4 | 0.03 | 1.58 | 0.56 | 0.26 |

| PROPOSED DITCH ANALYSIS - DITCH DA-2 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 106+00 | 719.62 | | | | | | | | | | | |
| 106+50 | 718.82 | 1.60% | CLASS C | 0 | 2.34 | 0.54 | 6 | 4 | 0.03 | 2.58 | 3.78 | 0.54 |
| 107+00 | 718.18 | 1.28% | CLASS C | 0 | 2.49 | 0.56 | 6 | 4 | 0.03 | 2.38 | 3.78 | 0.45 |
| 107+50 | 717.54 | 1.28% | CLASS C | 0 | 2.50 | 0.56 | 6 | 4 | 0.03 | 2.38 | 3.78 | 0.45 |
| 108+00 | 716.92 | 1.24% | CLASS C | 0 | 2.50 | 0.57 | 6 | 4 | 0.03 | 2.35 | 3.78 | 0.44 |
| 108+50 | 716.34 | 1.16% | CLASS C | 0 | 2.50 | 0.57 | 6 | 4 | 0.03 | 2.29 | 3.78 | 0.42 |
| 109+00 | 715.82 | 1.04% | CLASS C | 0 | 2.50 | 0.59 | 6 | 4 | 0.03 | 2.20 | 3.78 | 0.38 |
| 109+50 | 715.34 | 0.96% | CLASS C | 0 | 2.50 | 0.60 | 6 | 4 | 0.03 | 2.13 | 3.78 | 0.36 |
| 110+00 | 714.83 | 1.02% | CLASS C | 0 | 2.50 | 0.59 | 6 | 4 | 0.03 | 2.18 | 3.78 | 0.37 |
| 110+50 | 714.25 | 1.16% | CLASS C | 0 | 2.50 | 0.57 | 6 | 4 | 0.03 | 2.29 | 3.78 | 0.42 |
| 111+00 | 713.68 | 1.14% | CLASS C | 0 | 2.50 | 0.58 | 6 | 4 | 0.03 | 2.28 | 3.78 | 0.41 |
| 112+00 | 713.05 | 0.63% | CLASS C | 0 | 1.90 | 0.64 | 6 | 4 | 0.03 | 1.82 | 3.78 | 0.25 |
| 112+50 | 712.38 | 1.34% | CLASS C | 0 | 1.58 | 0.56 | 6 | 4 | 0.03 | 2.42 | 3.78 | 0.47 |
| 113+00 | 711.71 | 1.34% | CLASS C | 0 | 1.41 | 0.56 | 6 | 4 | 0.03 | 2.42 | 3.78 | 0.47 |
| 113+50 | 711.04 | 1.34% | CLASS C | 0 | 1.35 | 0.56 | 6 | 4 | 0.03 | 2.42 | 3.78 | 0.47 |
| 114+00 | 710.69 | 0.70% | CLASS C | 0 | 1.27 | 0.66 | 4.8 | 4 | 0.03 | 1.95 | 3.78 | 0.29 |
| 114+50 | 709.56 | 2.26% | CLASS C | 0 | 1.97 | 0.55 | 4 | 4 | 0.03 | 3.10 | 3.78 | 0.78 |
| 115+00 | 709.30 | 0.52% | CLASS C | 0 | 1.85 | 0.73 | 4 | 4 | 0.03 | 1.78 | 3.78 | 0.24 |

| PROPOSED DITCH ANALYSIS - DITCH DA-3 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 109+50 | 715.94 | | | | | | | | | | | |
| 110+00 | 715.07 | 1.74% | CLASS C | 0 | 2.50 | 0.36 | 6 | 4 | 0.03 | 2.04 | 1.30 | 0.39 |
| 110+50 | 714.29 | 1.56% | CLASS C | 0 | 2.50 | 0.36 | 6 | 4 | 0.03 | 1.96 | 1.30 | 0.35 |
| 111+00 | 713.68 | 1.22% | CLASS C | 0 | 2.41 | 0.38 | 6 | 4 | 0.03 | 1.79 | 1.30 | 0.29 |
| 112+00 | 713.04 | 0.64% | CLASS C | 0 | 1.08 | 0.43 | 6 | 4 | 0.03 | 1.40 | 1.30 | 0.17 |
| 112+50 | 712.27 | 1.54% | CLASS C | 0 | 0.92 | 0.37 | 6 | 4 | 0.03 | 1.95 | 1.30 | 0.35 |
| 113+00 | 711.50 | 1.54% | CLASS C | 0 | 0.91 | 0.37 | 6 | 4 | 0.03 | 1.95 | 1.30 | 0.35 |
| 113+50 | 710.73 | 1.54% | CLASS C | 0 | 0.91 | 0.37 | 6 | 4 | 0.03 | 1.95 | 1.30 | 0.35 |
| 114+00 | 710.53 | 0.40% | CLASS C | 0 | 0.66 | 0.49 | 5 | 4 | 0.03 | 1.21 | 1.30 | 0.12 |
| 114+50 | 709.00 | 3.06% | CLASS C | 0 | 1.94 | 0.35 | 4 | 4 | 0.03 | 2.66 | 1.30 | 0.67 |

| PROPOSED DITCH ANALYSIS - DITCH DA-4 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 118+00 | 704.15 | | | | | | | | | | | |
| 118+50 | 704.39 | 0.48% | CLASS C | 0 | 4.13 | 0.42 | 6 | 4 | 0.03 | 1.19 | 1.04 | 0.13 |
| 119+00 | 704.63 | 0.48% | CLASS C | 0 | 4.30 | 0.42 | 6 | 4 | 0.03 | 1.19 | 1.04 | 0.13 |
| 119+50 | 704.80 | 0.34% | CLASS C | 0 | 2.82 | 0.45 | 6 | 4 | 0.03 | 1.05 | 1.04 | 0.09 |

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CR 118 AT COTTONWOOD CREEK

DITCH TABLES

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| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
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| | DIST | | COUNTY | SHEET NO. |
| | AUS | | WILLIAMSON | 52 |

FILENAME: \\kn-pw-bentley.com\kn-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\5. Drainage\CR118_DRC-DITCH02.dgn
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| PROPOSED DITCH ANALYSIS - DITCH DA-5 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 120+00 | 704.70 | | | | | | | | | | | |
| 120+50 | 704.53 | 0.34% | CLASS C | 0 | 2.55 | 0.29 | 6 | 4 | 0.03 | 0.78 | 0.32 | 0.06 |


| PROPOSED DITCH ANALYSIS - DITCH DA-6 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 121+00 | 704.13 | | | | | | | | | | | |
| 121+50 | 704.03 | 0.20% | CLASS C | 0 | 2.22 | 0.61 | 6 | 4 | 0.03 | 0.99 | 1.86 | 0.08 |
| 122+00 | 703.93 | 0.20% | CLASS C | 0 | 1.60 | 0.61 | 6 | 4 | 0.03 | 0.99 | 1.86 | 0.08 |

| PROPOSED DITCH ANALYSIS - DITCH DA-7 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 120+00 | 705.98 | | | | | | | | | | | |
| 120+50 | 705.63 | 0.70% | CLASS C | 0 | 1.30 | 0.24 | 6 | 4 | 0.03 | 1.00 | 0.29 | 0.11 |
| 121+00 | 705.29 | 0.68% | CLASS C | 0 | 1.26 | 0.24 | 6 | 4 | 0.03 | 0.99 | 0.29 | 0.10 |
| 122+00 | 704.11 | 1.18% | CLASS C | 0 | 0.96 | 0.22 | 6 | 4 | 0.03 | 1.21 | 0.29 | 0.16 |
| 122+50 | 703.34 | 1.54% | CLASS C | 0 | 0.87 | 0.21 | 6 | 4 | 0.03 | 1.34 | 0.29 | 0.20 |

| PROPOSED DITCH ANALYSIS - DITCH DA-8 | | | | | | | | | | | | |
|--------------------------------------|---------------------|-------------|---------|-------------------|------------------|-------------------|-------------------|------------------|-----------|-----------------------|--------------------|---------------------|
| STATION | FLOW LINE ELEVATION | DITCH GRADE | LINING | FLAT BOTTOM WIDTH | DITCH DEPTH (FT) | NORMAL DEPTH (FT) | FRONT SLOPE (X:1) | BACK SLOPE (X:1) | CHANNEL n | DESIGN VELOCITY (FPS) | DESIGN Q (2) (CFS) | DESIGN SHEAR STRESS |
| 123+00 | 703.51 | | | | | | | | | | | |
| 123+50 | 704.08 | 1.14% | CLASS C | 0 | 0.82 | 0.46 | 6 | 4 | 0.03 | 1.96 | 2.07 | 0.33 |
| 124+00 | 704.64 | 1.12% | CLASS C | 0 | 0.97 | 0.46 | 6 | 4 | 0.03 | 1.94 | 2.07 | 0.32 |
| 124+50 | 705.21 | 1.14% | CLASS C | 0 | 1.91 | 0.46 | 6 | 4 | 0.03 | 1.96 | 2.07 | 0.33 |
| 125+00 | 705.78 | 1.14% | CLASS C | 0 | 1.61 | 0.46 | 6 | 4 | 0.03 | 1.96 | 2.07 | 0.33 |
| 125+50 | 706.04 | 0.52% | CLASS C | 0 | 1.78 | 0.53 | 6 | 4 | 0.03 | 1.46 | 2.07 | 0.17 |
| 126+00 | 706.22 | 0.36% | CLASS C | 0 | 2.22 | 0.57 | 6 | 4 | 0.03 | 1.27 | 2.07 | 0.13 |
| 126+50 | 706.39 | 0.34% | CLASS C | 0 | 2.12 | 0.58 | 6 | 4 | 0.03 | 1.24 | 2.07 | 0.12 |
| 127+00 | 706.67 | 0.56% | CLASS C | 0 | 2.53 | 0.53 | 6 | 4 | 0.03 | 1.50 | 2.07 | 0.18 |
| 127+50 | 707.33 | 1.32% | CLASS C | 0 | 2.49 | 0.45 | 6 | 4 | 0.03 | 2.07 | 2.07 | 0.37 |
| 128+00 | 707.98 | 1.30% | CLASS C | 0 | 1.84 | 0.45 | 6 | 4 | 0.03 | 2.06 | 2.07 | 0.36 |


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CR 118 AT COTTONWOOD CREEK

DITCH TABLES

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| | AUS | | WILLIAMSON | 53 |

FILENAME: \\kh-pw-bentley.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\5. Drainage\CR118_DRC_HYGCALC.dgn
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| HEC-HMS Input Parameters | | | | | | | | | | | | | |
|--------------------------|--------------|-----------|------------------|-------------------------|----------------------|----------------------|-----------|-----------|------------|------------|------------|-------------|-------------|
| Name | Area (SQ MI) | Area (AC) | Weighted Curve # | Designated Design Storm | Precipitation Depth | | | | | | | | |
| | | | | | T ₁ (min) | T ₂ (min) | 2-yr (in) | 5-yr (in) | 10-yr (in) | 25-yr (in) | 50-yr (in) | 100-yr (in) | 500-yr (in) |
| DA-CR 118 | 1.79 | 1145.00 | 81 | 25 yr | 103 | 62 | 3.97 | 5.20 | 6.37 | 8.15 | 9.68 | 11.40 | 16.50 |

| FLOW COMPUTATION (Q) | | | | | | |
|----------------------|-------------|-------------|-------------|--------------|--------------|--|
| 2 yr (cfs) | 10 yr (cfs) | 25 yr (cfs) | 50 yr (cfs) | 100 yr (cfs) | 500 yr (cfs) | |
| 567 | 1448 | 2168 | 2815 | 3524 | 5762 | |

| | |
|-----------------|-----------------------------|
| Loss Method: | SCS Curve Number |
| Surface Method: | SCS Unit Hydrograph |
| Precipitation: | SCS Storm - Atlas 14 Depths |

| CR 118 Time of Concentration | |
|------------------------------|----------------------|
| Calculation Method | Kerby Kirpich Method |
| Kerby Variables | |
| Overland Flow Roughness | 0.4 |
| Slope (ft/ft) | 0.028 |
| Length (ft) | 1200 |
| Kerpich Flow Variables | |
| Slope (ft/ft) | 0.009 |
| Length (ft) | 12518 |
| Time of Concentration (min) | 103 |

| Rational Input Parameters | | | | | | | | | | |
|---------------------------|-----------|------|----------|----------------|----------------|-----------------|-----------------|-----------------|------------------|------------------|
| Name | AREA (AC) | C | TC (MIN) | I 2-YR (IN/HR) | I 5-YR (IN/HR) | I 10-YR (IN/HR) | I 25-YR (IN/HR) | I 50-YR (IN/HR) | I 100-YR (IN/HR) | I 500-YR (IN/HR) |
| DA-CR 100 Culvert | 142.00 | 0.33 | 34 | 2.51 | 3.37 | 4.04 | 4.98 | 5.74 | 6.55 | 8.63 |

| Rational Flows, Q (cfs) | | | | | | | |
|-------------------------|------|------|-------|-------|-------|--------|--------|
| Name | 2-YR | 5-YR | 10-YR | 25-YR | 50-YR | 100-YR | 500-YR |
| DA-CR 100 Culvert | 118 | 158 | 189 | 233 | 269 | 307 | 404 |

NOTES:

1. THE HYDROLOGY DATA WAS OBTAINED FROM THE COTTONWOOD CREEK FEMA EXISTING CONDITIONS EFFECTIVE MODEL, DATED MAY 2019 AND PROVIDED BY TXDOT.
2. RAINFALL DATA OBTAINED FROM NOAA ATLAS 14.
3. RUNOFF MODELED IN HECHMS V 3.5 USING SCS LOSS AND TRANSFORM METHODS, AND SCS STORM WITH RAINFALL DEPTHS FROM NOAA ATLAS 14.

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CR 118 AT COTTONWOOD CREEK
 HYDROLOGIC DATA SHEET

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| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
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| | AUS | WILLIAMSON | | 54 |

FILENAME: pw:\k\k-pw-bentley.com\kn-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\5. Drainage\CR118_DRC-HYD.CALC.dgn
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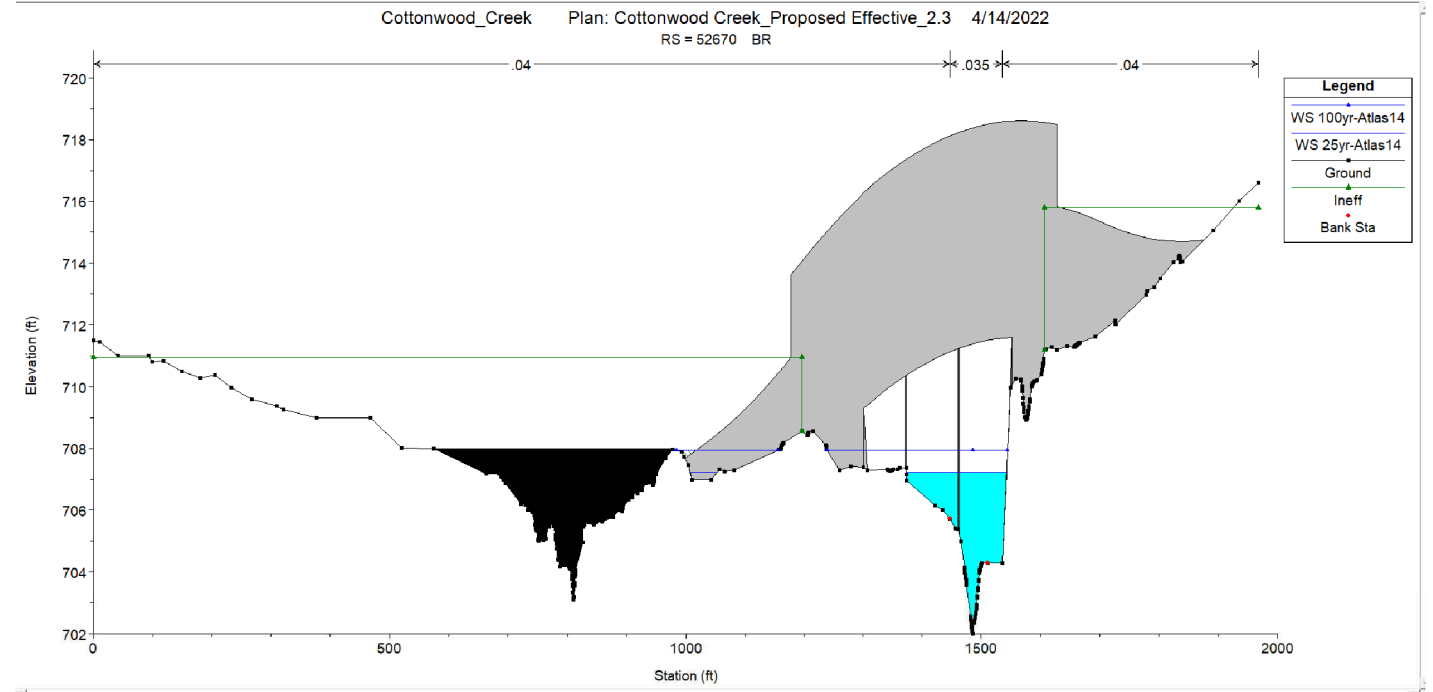
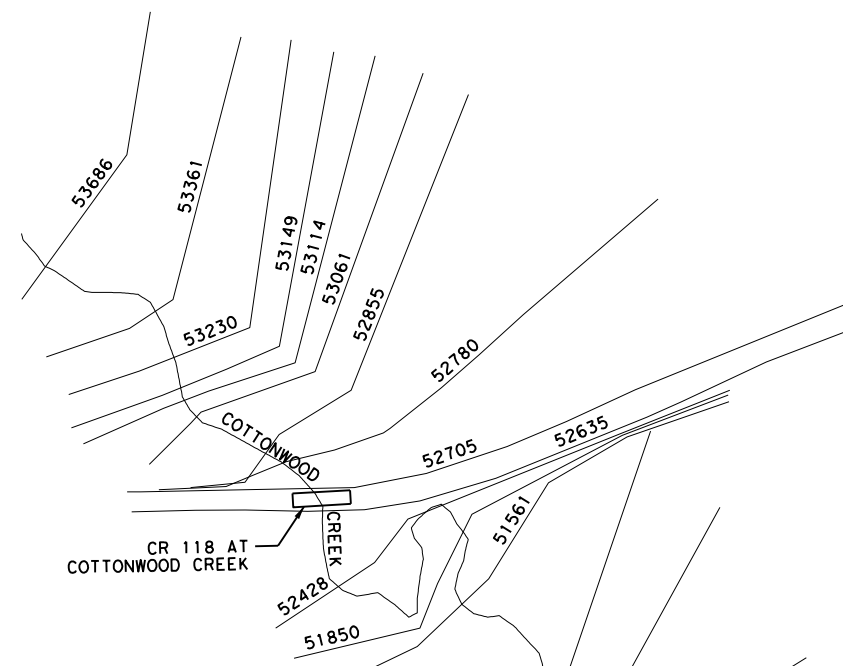
HEC-RAS HYDRAULIC CALCULATIONS

| River Sta | Profile | Plan | Q Total (cfs) | Min Ch El (ft) | W.S. Elev (ft) | Crit W.S. (ft) | E.G. Elev (ft) | E.G. Slope (ft/ft) | Vel Chn (ft/s) | Flow Area (sq ft) | Top Width (ft) | Froude # Ch1 |
|-----------|---------------|-------------------|---------------|----------------|----------------|----------------|----------------|--------------------|----------------|-------------------|----------------|--------------|
| 53686 | 25yr-Atlas14 | Revised Effective | 2140 | 706.36 | 713.84 | 713.84 | 714.86 | 0.005244 | 9.6 | 330.54 | 148.86 | 0.7 |
| 53686 | 25yr-Atlas14 | Proposed | 2140 | 706.36 | 713.84 | 713.84 | 714.86 | 0.005244 | 9.6 | 330.54 | 148.86 | 0.7 |
| 53686 | 100yr-Atlas14 | Revised Effective | 3480 | 706.36 | 714.89 | 714.89 | 715.93 | 0.004894 | 10.37 | 524.03 | 257.85 | 0.7 |
| 53686 | 100yr-Atlas14 | Proposed | 3480 | 706.36 | 714.89 | 714.89 | 715.93 | 0.004894 | 10.37 | 524.03 | 257.85 | 0.7 |
| 53361 | 25yr-Atlas14 | Revised Effective | 2160 | 704.87 | 712.67 | 709.74 | 713.09 | 0.001322 | 5.46 | 465.34 | 149.25 | 0.37 |
| 53361 | 25yr-Atlas14 | Proposed | 2160 | 704.87 | 711.49 | 709.74 | 712.23 | 0.002845 | 7.05 | 330.03 | 81.76 | 0.53 |
| 53361 | 100yr-Atlas14 | Revised Effective | 3520 | 704.87 | 713.7 | 711.11 | 714.35 | 0.00183 | 7.06 | 637.28 | 312.37 | 0.45 |
| 53361 | 100yr-Atlas14 | Proposed | 3520 | 704.87 | 712.77 | 711.11 | 713.83 | 0.003294 | 8.71 | 479.98 | 151.88 | 0.59 |
| 53230 | 25yr-Atlas14 | Revised Effective | 2160 | 703.95 | 712.56 | 709.14 | 712.91 | 0.001055 | 4.94 | 508.52 | 481.99 | 0.33 |
| 53230 | 25yr-Atlas14 | Proposed | 2160 | 703.95 | 711.21 | 709.14 | 711.84 | 0.002402 | 6.46 | 356.89 | 108.65 | 0.48 |
| 53230 | 100yr-Atlas14 | Revised Effective | 3520 | 703.95 | 713.52 | 710.69 | 714.09 | 0.001551 | 6.53 | 686.47 | 827.26 | 0.41 |
| 53230 | 100yr-Atlas14 | Proposed | 3520 | 703.95 | 712.34 | 710.69 | 713.38 | 0.003209 | 8.44 | 476.65 | 430.87 | 0.57 |
| 53149 | 25yr-Atlas14 | Revised Effective | 2180 | 703.38 | 712.65 | 707.82 | 712.73 | 0.00028 | 2.81 | 1169.55 | 417.64 | 0.17 |
| 53149 | 25yr-Atlas14 | Proposed | 2180 | 703.38 | 711.41 | 707.81 | 711.62 | 0.000707 | 3.99 | 684.97 | 269.86 | 0.27 |
| 53149 | 100yr-Atlas14 | Revised Effective | 3550 | 703.38 | 713.7 | 709.02 | 713.81 | 0.000342 | 3.37 | 2013.43 | 1073.94 | 0.2 |
| 53149 | 100yr-Atlas14 | Proposed | 3550 | 703.38 | 712.81 | 709.14 | 713.03 | 0.000658 | 4.36 | 1239.66 | 431.35 | 0.27 |
| 53129 | | | | | | | | | | | | |
| | | | Mult Open | | | | | | | | | |
| 53114 | 25yr-Atlas14 | Revised Effective | 2180 | 703.15 | 710.9 | 708.42 | 711.62 | 0.002536 | 6.92 | 340.95 | 96.33 | 0.49 |
| 53114 | 25yr-Atlas14 | Proposed | 2180 | 703.15 | 710.75 | 708.42 | 711.52 | 0.002769 | 7.12 | 327.43 | 90.94 | 0.5 |
| 53114 | 100yr-Atlas14 | Revised Effective | 3550 | 703.15 | 711.06 | 710.26 | 712.86 | 0.0061 | 10.91 | 358.08 | 112.91 | 0.76 |
| 53114 | 100yr-Atlas14 | Proposed | 3550 | 703.15 | 710.71 | 710.27 | 712.79 | 0.007511 | 11.67 | 324.09 | 90.43 | 0.83 |
| 53061 | 25yr-Atlas14 | Revised Effective | 2200 | 702.94 | 710.86 | 709.63 | 711.38 | 0.002701 | 6.39 | 503.13 | 307.82 | 0.5 |
| 53061 | 25yr-Atlas14 | Proposed | 2200 | 702.94 | 710.66 | 709.63 | 711.33 | 0.003463 | 7.05 | 442.23 | 304.53 | 0.56 |
| 53061 | 100yr-Atlas14 | Revised Effective | 3580 | 702.94 | 711.24 | 711.24 | 712.13 | 0.004549 | 8.7 | 622.5 | 319.27 | 0.66 |
| 53061 | 100yr-Atlas14 | Proposed | 3580 | 702.94 | 711.24 | 711.24 | 712.13 | 0.004549 | 8.7 | 622.5 | 319.27 | 0.66 |
| 52855 | 25yr-Atlas14 | Revised Effective | 2220 | 702.11 | 708.62 | 708.58 | 710.14 | 0.010287 | 10.46 | 255.35 | 118.18 | 0.93 |
| 52855 | 25yr-Atlas14 | Proposed | 2220 | 702.11 | 708.63 | 708.58 | 710.14 | 0.010162 | 10.42 | 256.94 | 119 | 0.92 |
| 52855 | 100yr-Atlas14 | Revised Effective | 3610 | 702.11 | 710.05 | 710.05 | 710.98 | 0.005613 | 9.19 | 592.96 | 272.72 | 0.71 |
| 52855 | 100yr-Atlas14 | Proposed | 3610 | 702.11 | 710.05 | 710.05 | 710.98 | 0.005613 | 9.19 | 592.96 | 272.72 | 0.71 |
| 52780.* | 25yr-Atlas14 | Revised Effective | 2220 | 702.06 | 708.38 | 708.38 | 709.19 | 0.006918 | 7.81 | 371.94 | 242.97 | 0.71 |
| 52780.* | 25yr-Atlas14 | Proposed | 2220 | 702.06 | 708.38 | 708.38 | 709.19 | 0.006918 | 7.81 | 371.94 | 242.97 | 0.71 |
| 52780.* | 100yr-Atlas14 | Revised Effective | 3610 | 702.06 | 709.15 | 709.15 | 709.94 | 0.006264 | 8.41 | 643.59 | 395.84 | 0.69 |
| 52780.* | 100yr-Atlas14 | Proposed | 3610 | 702.06 | 709.15 | 709.15 | 709.94 | 0.006262 | 8.41 | 643.67 | 395.84 | 0.69 |
| 52705 | 25yr-Atlas14 | Revised Effective | 2220 | 702.01 | 707.3 | 707.3 | 708.38 | 0.010334 | 8.79 | 297.09 | 238.14 | 0.91 |
| 52705 | 25yr-Atlas14 | Proposed | 2220 | 702.01 | 707.68 | 707.68 | 708.43 | 0.006334 | 7.47 | 398.34 | 413.67 | 0.72 |
| 52705 | 100yr-Atlas14 | Revised Effective | 3610 | 702.01 | 708.3 | 708.3 | 709.21 | 0.006902 | 8.74 | 583.07 | 979.28 | 0.78 |
| 52705 | 100yr-Atlas14 | Proposed | 3610 | 702.01 | 708.29 | 708.29 | 709.21 | 0.007041 | 8.81 | 578.31 | 976.38 | 0.78 |
| 52670 | | | Bridge | | | | | | | | | |
| 52635 | 25yr-Atlas14 | Revised Effective | 2220 | 702.83 | 707.24 | | 707.38 | 0.001636 | 4.14 | 877.07 | 537.54 | 0.38 |
| 52635 | 25yr-Atlas14 | Proposed | 2220 | 702.83 | 707.25 | | 707.51 | 0.002616 | 5.24 | 627.5 | 539.03 | 0.47 |
| 52635 | 100yr-Atlas14 | Revised Effective | 3610 | 702.83 | 707.9 | | 708.08 | 0.001878 | 4.93 | 1257.59 | 604.23 | 0.41 |
| 52635 | 100yr-Atlas14 | Proposed | 3610 | 702.83 | 708 | | 707.08 | 0.002747 | 6.06 | 847.69 | 604.72 | 0.5 |
| 52428 | 25yr-Atlas14 | Revised Effective | 2250 | 702.83 | 706.86 | | 707.11 | 0.004391 | 5.2 | 684.87 | 467.13 | 0.58 |
| 52428 | 25yr-Atlas14 | Proposed | 2250 | 702.83 | 706.89 | | 706.34 | 0.004336 | 5.2 | 660.62 | 471.18 | 0.57 |
| 52428 | 100yr-Atlas14 | Revised Effective | 3660 | 702.83 | 707.74 | | 707.9 | 0.002345 | 4.63 | 1450.92 | 1039.64 | 0.44 |
| 52428 | 100yr-Atlas14 | Proposed | 3660 | 702.83 | 707.75 | | 706.81 | 0.003186 | 5.41 | 1068.81 | 1040.76 | 0.52 |
| 51850 | 25yr-Atlas14 | Revised Effective | 2310 | 698.22 | 705.44 | | 704.44 | 0.002544 | 6.79 | 500.64 | 172.95 | 0.49 |
| 51850 | 25yr-Atlas14 | Proposed | 2310 | 698.22 | 705.44 | | 704.44 | 0.002544 | 6.79 | 500.64 | 172.95 | 0.49 |
| 51850 | 100yr-Atlas14 | Revised Effective | 3760 | 698.22 | 707.02 | | 705.26 | 0.001591 | 6.29 | 1207.88 | 841.85 | 0.41 |
| 51850 | 100yr-Atlas14 | Proposed | 3760 | 698.22 | 707.02 | | 705.26 | 0.001591 | 6.29 | 1207.88 | 841.85 | 0.41 |

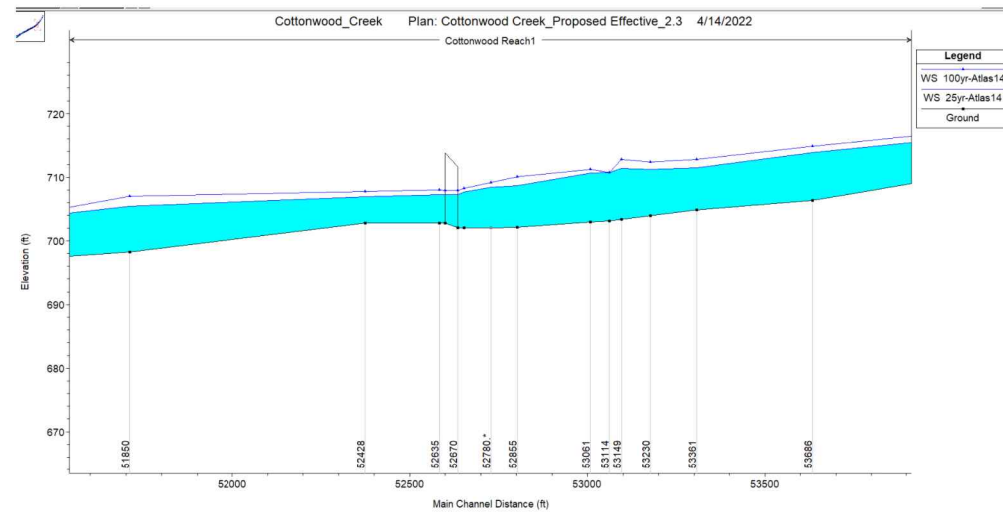
NOTES:

- HEC-RAS VERSION 4.1.0 WAS USED FOR HYDRAULIC ANALYSIS AND DESIGN.
- THE TAILWATER BOUNDARY CONDITION USED WAS NORMAL DEPTH WITH A SLOPE OF 0.00318 FT/FT.
- DESIGN FLOW BASED ON 25-YR FREQUENCY.
- THERE ARE NO INSURABLE STRUCTURES THAT WOULD BE ADVERSLY AFFECTED FOR THE 25 YR AND 100 YR DESIGN FREQUENCY.
- COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS COMPLETED 9/22/2022.
- RIVER STATION 53129 WAS MODELED AS A MULTIPLE OPENING CROSSING IN THE EFFECTIVE MODEL.
- 52780* IS A HEC-RAS INTERPOLATED CROSS SECTION.

HEC-RAS CROSS SECTION LOCATIONS



PROPOSED CR 118 US BRIDGE FACE



WATER SURFACE PROFILE

Signature: *AH*
 11/3/2022
 Austin S. Helton
 State of Texas
 124922
 Licensed Professional Engineer



CR 118 AT COTTONWOOD CREEK

HYDRAULIC DATA SHEET

SHEET 1 OF 2

| | | | |
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| CONT | SECT | JOB | HIGHWAY |
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| DIST | | | SHEET NO. |
| AUS | | | 55 |

HEC-RAS HYDRAULIC CALCULATION BRIDGE OUTPUT

| Plan: Revised Eff Cottonwood Reach1 RS: 53129 Open#2: Bridge Profile: 25yr-Atlas14 | | | | |
|--|------------|------------------------|--------------|--------------|
| E.G. US. (ft) | 712.74 | Element | Inside BR US | Inside BR DS |
| W.S. US. (ft) | 712.55 | E.G. Elev (ft) | 712.74 | 712.61 |
| Q Total (cfs) | 2043.04 | W.S. Elev (ft) | 712.55 | 712.31 |
| Q Bridge (cfs) | 1870.98 | Crit W.S. (ft) | 708.59 | 708.39 |
| Q Weir (cfs) | 172.06 | Max Chl Dpth (ft) | 9.17 | 9.16 |
| Weir Sta Lft (ft) | 1032.71 | Vel Total (ft/s) | 8.5 | 8.25 |
| Weir Sta Rgt (ft) | 1110.96 | Flow Area (sq ft) | 220.06 | 226.75 |
| Weir Submerg | 0 | Froude # Chl | 0.49 | 0.48 |
| Weir Max Depth (ft) | 1.17 | Specif Force (cu ft) | 1716.24 | 1706.55 |
| Min El Weir Flow (ft) | 711.58 | Hydr Depth (ft) | | |
| Min El Prs (ft) | 710.34 | W.P. Total (ft) | 92.89 | 92.98 |
| Delta EG (ft) | 1.12 | Conv. Total (cfs) | 16602.5 | 17441.5 |
| Delta WS (ft) | 1.57 | Top Width (ft) | | |
| BR Open Area (sq ft) | 220.06 | Frctn Loss (ft) | | |
| BR Open Vel (ft/s) | 8.5 | C & E Loss (ft) | | |
| Coef of Q | | Shear Total (lb/sq ft) | 2.24 | 2.09 |
| Br Sel Method | Press/Weir | Power Total (lb/ft s) | 1032.71 | 1098.51 |

| Plan: Proposed Cottonwood Reach1 RS: 52670 Profile: 25yr-Atlas14 | | | | |
|--|-------------|------------------------|--------------|--------------|
| E.G. US. (ft) | 708.43 | Element | Inside BR US | Inside BR DS |
| W.S. US. (ft) | 707.68 | E.G. Elev (ft) | 708.02 | 707.65 |
| Q Total (cfs) | 2220 | W.S. Elev (ft) | 707.24 | 707.22 |
| Q Bridge (cfs) | 2220 | Crit W.S. (ft) | 707.02 | 706.75 |
| Q Weir (cfs) | | Max Chl Dpth (ft) | 5.23 | 4.39 |
| Weir Sta Lft (ft) | | Vel Total (ft/s) | 6.49 | 4.56 |
| Weir Sta Rgt (ft) | | Flow Area (sq ft) | 341.95 | 486.8 |
| Weir Submerg | | Froude # Chl | 0.74 | 0.44 |
| Weir Max Depth (ft) | | Specif Force (cu ft) | 976.69 | 994.9 |
| Min El Weir Flow (ft) | 714.3 | Hydr Depth (ft) | 2.06 | 2.17 |
| Min El Prs (ft) | 711.6 | W.P. Total (ft) | 171.51 | 238.04 |
| Delta EG (ft) | 0.92 | Conv. Total (cfs) | 26087.2 | 34636.6 |
| Delta WS (ft) | 0.43 | Top Width (ft) | 166.31 | 224.44 |
| BR Open Area (sq ft) | 1178.1 | Frctn Loss (ft) | 0.19 | 0.05 |
| BR Open Vel (ft/s) | 6.49 | C & E Loss (ft) | 0.17 | 0.09 |
| Coef of Q | | Shear Total (lb/sq ft) | 0.9 | 0.52 |
| Br Sel Method | Energy only | Power Total (lb/ft s) | 0 | 0 |


| Plan: Revised Eff Cottonwood Reach1 RS: 53129 Open#2: Bridge Profile: 100yr-Atlas14 | | | | |
|---|------------|------------------------|--------------|--------------|
| E.G. US. (ft) | 713.82 | Element | Inside BR US | Inside BR DS |
| W.S. US. (ft) | 713.64 | E.G. Elev (ft) | 713.82 | 713.68 |
| Q Total (cfs) | 2254.53 | W.S. Elev (ft) | 713.64 | 713.02 |
| Q Bridge (cfs) | 1691.39 | Crit W.S. (ft) | 708.92 | 708.73 |
| Q Weir (cfs) | 563.13 | Max Chl Dpth (ft) | 10.26 | 9.87 |
| Weir Sta Lft (ft) | 1032.71 | Vel Total (ft/s) | 7.69 | 7.46 |
| Weir Sta Rgt (ft) | 1110.96 | Flow Area (sq ft) | 220.06 | 226.75 |
| Weir Submerg | 0.27 | Froude # Chl | 0.42 | 0.42 |
| Weir Max Depth (ft) | 2.25 | Specif Force (cu ft) | 1865.52 | 1780.23 |
| Min El Weir Flow (ft) | 711.58 | Hydr Depth (ft) | | |
| Min El Prs (ft) | 710.34 | W.P. Total (ft) | 92.89 | 92.98 |
| Delta EG (ft) | 0.97 | Conv. Total (cfs) | 16602.5 | 17441.6 |
| Delta WS (ft) | 1.26 | Top Width (ft) | | |
| BR Open Area (sq ft) | 220.06 | Frctn Loss (ft) | | |
| BR Open Vel (ft/s) | 7.69 | C & E Loss (ft) | | |
| Coef of Q | | Shear Total (lb/sq ft) | 2.73 | 2.54 |
| Br Sel Method | Press/Weir | Power Total (lb/ft s) | 1032.71 | 1098.51 |

| Plan: Proposed Cottonwood Reach1 RS: 52670 Profile: 100yr-Atlas14 | | | | |
|---|-------------|------------------------|--------------|--------------|
| E.G. US. (ft) | 709.21 | Element | Inside BR US | Inside BR DS |
| W.S. US. (ft) | 708.29 | E.G. Elev (ft) | 708.96 | 708.53 |
| Q Total (cfs) | 3610 | W.S. Elev (ft) | 707.94 | 707.93 |
| Q Bridge (cfs) | 3610 | Crit W.S. (ft) | 707.91 | 707.36 |
| Q Weir (cfs) | | Max Chl Dpth (ft) | 5.93 | 5.1 |
| Weir Sta Lft (ft) | | Vel Total (ft/s) | 7.25 | 5.58 |
| Weir Sta Rgt (ft) | | Flow Area (sq ft) | 497.95 | 647.1 |
| Weir Submerg | | Froude # Chl | 0.59 | 0.48 |
| Weir Max Depth (ft) | | Specif Force (cu ft) | 1677.42 | 1721.24 |
| Min El Weir Flow (ft) | 714.3 | Hydr Depth (ft) | 2.14 | 2.85 |
| Min El Prs (ft) | 711.6 | W.P. Total (ft) | 242.17 | 245.3 |
| Delta EG (ft) | 0.87 | Conv. Total (cfs) | 40104.4 | 52157.3 |
| Delta WS (ft) | 0.29 | Top Width (ft) | 232.95 | 227.06 |
| BR Open Area (sq ft) | 1178.1 | Frctn Loss (ft) | 0.22 | 0.06 |
| BR Open Vel (ft/s) | 7.25 | C & E Loss (ft) | 0.22 | 0.13 |
| Coef of Q | | Shear Total (lb/sq ft) | 1.04 | 0.79 |
| Br Sel Method | Energy only | Power Total (lb/ft s) | 0 | 0 |

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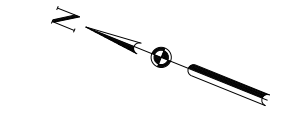
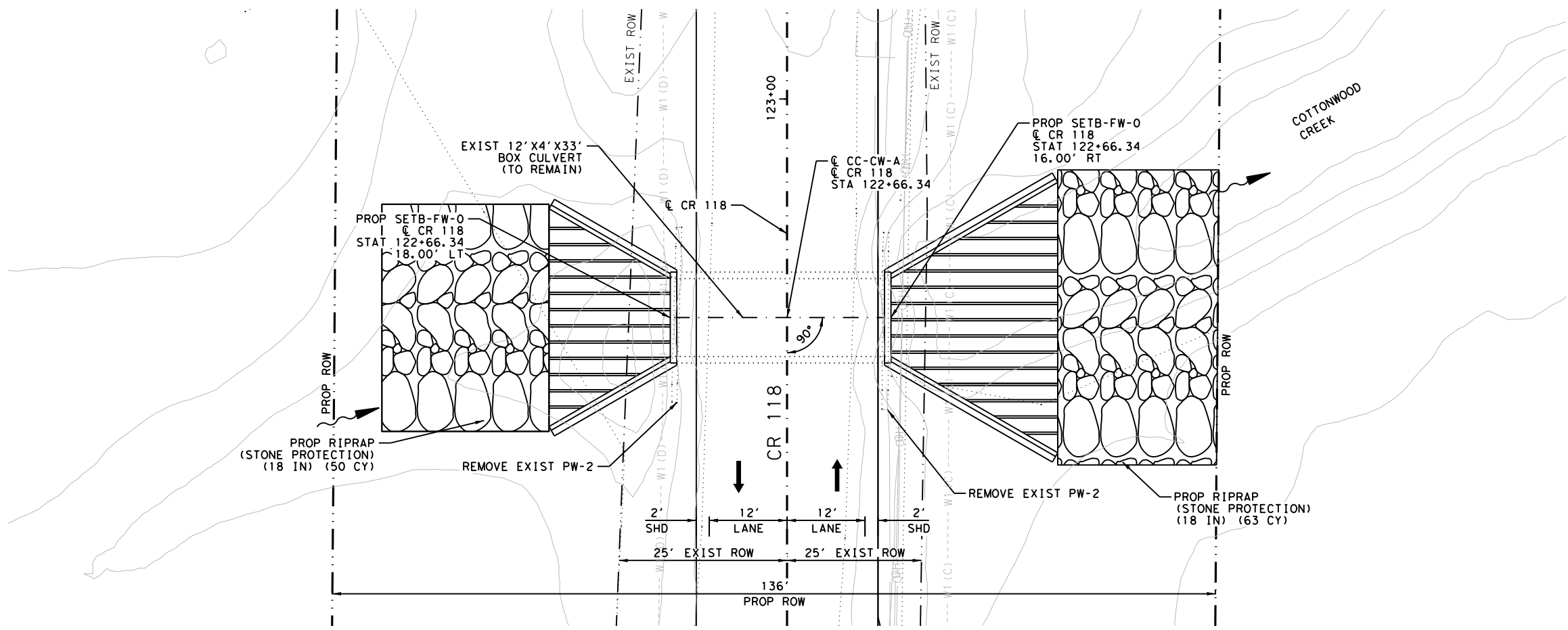
CR 118 AT COTTONWOOD CREEK

HYDRAULIC DATA SHEET

SHEET 2 OF 2

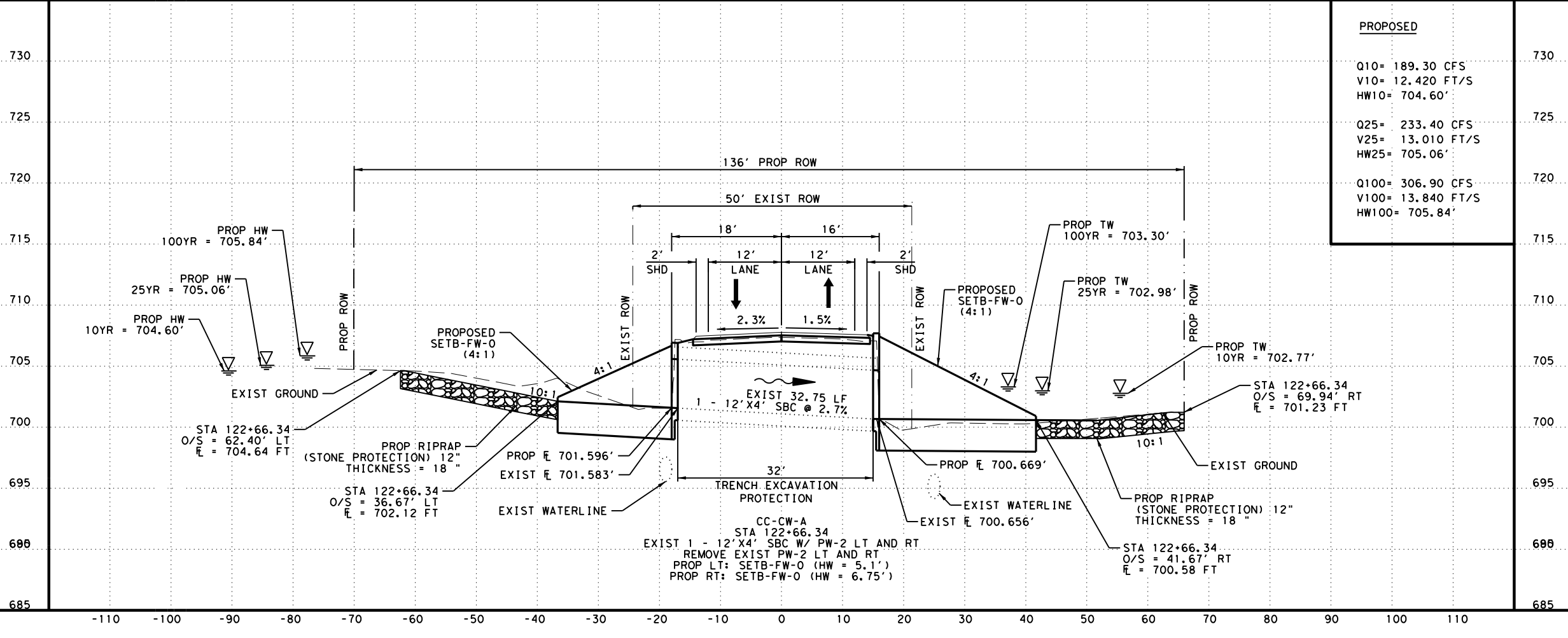
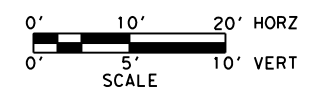
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| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | | COUNTY | SHEET NO. |
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LEGEND
 FLOW ARROW
 TRAFFIC DIRECTION

NOTES:
 1. LOCATION OF EXISTING UTILITES ARE APPROXIMATE. CONTRACTOR TO VERIFY PRIOR TO CONSTRUCTION.
 2. SEE HYDRAULIC CALCULATIONS FOR DETAILED HYDRAULIC ANALYSIS.



| PROPOSED | |
|----------|-------------|
| Q10= | 189.30 CFS |
| V10= | 12.420 FT/S |
| HW10= | 704.60' |
| Q25= | 233.40 CFS |
| V25= | 13.010 FT/S |
| HW25= | 705.06' |
| Q100= | 306.90 CFS |
| V100= | 13.840 FT/S |
| HW100= | 705.84' |

11/3/2022

 AUSTIN S. HELTON
 124922
 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

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CR 118 AT COTTONWOOD CREEK

CULVERT LAYOUT
 CC-CW-A

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
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CULVERT CC-CW-A - EXISTING

| SITE DATA | |
|--------------------|---------------------|
| Site Data Option: | Culvert Invert Data |
| Inlet Station: | 82.55 ft |
| Inlet Elevation: | 701.58 ft |
| Outlet Station: | 115.59 ft |
| Outlet Elevation: | 700.66 ft |
| Number of Barrels: | 1 |

| ROADWAY DATA | |
|------------------------|-----------|
| Roadway Profile Shape: | Irregular |
| Roadway Surface: | Paved |
| Roadway Top Width: | 22.00 ft |

| TAILWATER DATA | |
|---------------------------|---------------------|
| Tailwater Channel Option: | Trapezoidal Channel |
| Channel Slope: | 0.0100 |
| Channel Manning's n: | 0.0450 |
| Channel Invert Elevation: | 700.66 ft |

| CULVERT DATA | |
|----------------------|----------------------------|
| Barrel Shape: | Concrete Box |
| Barrel Span: | 12.00 ft |
| Barrel Rise: | 4.00 ft |
| Barrel Material: | Concrete |
| Embedment: | 0.00 in |
| Barrel Manning's n: | 0.0120 |
| Culvert Type: | Straight |
| Inlet Configuration: | Square Edge (90°) Headwall |

| CULVERT SUMMARY TABLE: EXISTING | | | | | | | | | | | | |
|---------------------------------|-----------------------|-------------------------|--------------------------|--------------------------|---------------------------|-----------|-------------------|---------------------|-------------------|----------------------|------------------------|---------------------------|
| Discharge Names | Total Discharge (cfs) | Culvert Discharge (cfs) | Headwater Elevation (ft) | Inlet Control Depth (ft) | Outlet Control Depth (ft) | Flow Type | Normal Depth (ft) | Critical Depth (ft) | Outlet Depth (ft) | Tailwater Depth (ft) | Outlet Velocity (ft/s) | Tailwater Velocity (ft/s) |
| 2 Year | 117.60 | 117.60 | 703.98 | 2.398 | 0.889 | 1-S2n | 0.663 | 1.439 | 0.876 | 1.668 | 11.182 | 3.523 |
| 5 Year | 157.90 | 157.90 | 704.50 | 2.914 | 1.262 | 1-S2n | 0.789 | 1.752 | 1.101 | 1.923 | 11.956 | 3.812 |
| 10 Year | 189.30 | 189.30 | 704.87 | 3.289 | 1.551 | 1-S2n | 0.895 | 1.977 | 1.268 | 2.096 | 12.439 | 4.000 |
| 25 Year | 233.40 | 233.40 | 705.38 | 3.796 | 1.966 | 1-S2n | 1.023 | 2.273 | 1.493 | 2.313 | 13.029 | 4.227 |
| 50 Year | 269.00 | 269.00 | 705.79 | 4.203 | 2.343 | 5-S2n | 1.120 | 2.499 | 1.666 | 2.470 | 13.451 | 4.387 |
| 100 Year | 306.90 | 306.90 | 706.23 | 4.648 | 2.805 | 5-S2n | 1.219 | 2.279 | 1.846 | 2.625 | 13.858 | 4.540 |

| SUMMARY OF FLOWS AT CROSSING: EXISTING | | | | | |
|--|-----------------|-----------------------|-------------------------|-------------------------|-------------|
| Headwater Elevation (ft) | Discharge Names | Total Discharge (cfs) | CC-CW-A Discharge (cfs) | Roadway Discharge (cfs) | Iterations |
| 703.98 | 2 Year | 117.60 | 117.60 | 0.00 | 1 |
| 704.50 | 5 Year | 157.90 | 157.90 | 0.00 | 1 |
| 704.87 | 10 Year | 189.30 | 189.30 | 0.00 | 1 |
| 705.38 | 25 Year | 233.40 | 233.40 | 0.00 | 1 |
| 705.79 | 50 Year | 269.00 | 269.00 | 0.00 | 1 |
| 706.23 | 100 Year | 306.90 | 306.90 | 0.00 | 1 |
| 707.20 | Overtopping | 382.82 | 382.82 | 0.00 | Overtopping |

- NOTES:**
- HY-8 VERSION 7.6 USED FOR CULVERT HYDRAULIC CALCULATIONS.
 - DESIGNATED STORM EVENT IS 10 YEAR.

CULVERT CC-CW-A - PROPOSED

| SITE DATA | |
|--------------------|---------------------|
| Site Data Option: | Culvert Invert Data |
| Inlet Station: | 82.00 ft |
| Inlet Elevation: | 701.60 ft |
| Outlet Station: | 116.00 ft |
| Outlet Elevation: | 700.67 ft |
| Number of Barrels: | 1 |

| ROADWAY DATA | |
|------------------------|-----------|
| Roadway Profile Shape: | Irregular |
| Roadway Surface: | Paved |
| Roadway Top Width: | 28.00 ft |

| TAILWATER DATA | |
|---------------------------|---------------------|
| Tailwater Channel Option: | Trapezoidal Channel |
| Channel Slope: | 0.0100 |
| Channel Manning's n: | 0.0450 |
| Channel Invert Elevation: | 700.66 ft |

| CULVERT DATA | |
|----------------------|------------------------------------|
| Barrel Shape: | Concrete Box |
| Barrel Span: | 12.00 ft |
| Barrel Rise: | 4.00 ft |
| Barrel Material: | Concrete |
| Embedment: | 0.00 in |
| Barrel Manning's n: | 0.0120 |
| Culvert Type: | Straight |
| Inlet Configuration: | Square Edge (30-75 flare) Wingwall |

| CULVERT SUMMARY TABLE: PROPOSED | | | | | | | | | | | | |
|---------------------------------|-----------------------|-------------------------|--------------------------|--------------------------|---------------------------|-----------|-------------------|---------------------|-------------------|----------------------|------------------------|---------------------------|
| Discharge Names | Total Discharge (cfs) | Culvert Discharge (cfs) | Headwater Elevation (ft) | Inlet Control Depth (ft) | Outlet Control Depth (ft) | Flow Type | Normal Depth (ft) | Critical Depth (ft) | Outlet Depth (ft) | Tailwater Depth (ft) | Outlet Velocity (ft/s) | Tailwater Velocity (ft/s) |
| 2 Year | 117.60 | 117.60 | 703.76 | 2.165 | 0.886 | 1-S2n | 0.669 | 1.439 | 0.878 | 1.668 | 11.160 | 3.523 |
| 5 Year | 157.90 | 157.90 | 704.25 | 2.653 | 1.232 | 1-S2n | 0.806 | 1.752 | 1.102 | 1.923 | 11.937 | 3.812 |
| 10 Year | 189.30 | 189.30 | 704.60 | 3.002 | 1.514 | 1-S2n | 0.904 | 1.977 | 1.270 | 2.096 | 12.420 | 4.000 |
| 25 Year | 233.40 | 233.40 | 705.06 | 3.468 | 1.917 | 1-S2n | 1.033 | 2.273 | 1.495 | 2.313 | 13.010 | 4.227 |
| 50 Year | 269.00 | 269.00 | 705.44 | 3.840 | 2.295 | 1-S2n | 1.131 | 2.499 | 1.669 | 2.470 | 13.433 | 4.387 |
| 100 Year | 306.90 | 306.90 | 705.84 | 4.243 | 2.743 | 5-S2n | 1.231 | 2.279 | 1.848 | 2.625 | 13.840 | 4.540 |

| SUMMARY OF FLOWS AT CROSSING: PROPOSED | | | | | |
|--|-----------------|-----------------------|-------------------------|-------------------------|-------------|
| Headwater Elevation (ft) | Discharge Names | Total Discharge (cfs) | CC-CW-A Discharge (cfs) | Roadway Discharge (cfs) | Iterations |
| 703.76 | 2 Year | 117.60 | 117.60 | 0.00 | 1 |
| 704.25 | 5 Year | 157.90 | 157.90 | 0.00 | 1 |
| 704.60 | 10 Year | 189.30 | 189.30 | 0.00 | 1 |
| 705.06 | 25 Year | 233.40 | 233.40 | 0.00 | 1 |
| 705.44 | 50 Year | 269.00 | 269.00 | 0.00 | 1 |
| 705.84 | 100 Year | 306.90 | 306.90 | 0.00 | 1 |
| 707.69 | Overtopping | 455.14 | 455.14 | 0.00 | Overtopping |


 11/3/2022




CR 118 AT COTTONWOOD CREEK
 HYDRAULIC CALCULATIONS
 CULVERT CC-CW-A

SHEET 1 OF 1

| | | | |
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| AUS | | WILLIAMSON | 58 |

**50 YEAR SCOUR CALCULATIONS
PIER SCOUR COMPUTATION RESULTS**

| FREQ (yrs) | K ₁ | K ₂ | K ₃ | Y ₁ | a | Fr ₁ | V | y _s |
|------------|----------------|----------------|----------------|----------------|------|-----------------|------|----------------|
| 50 | 1.0 | 1.0 | 1.1 | 3.64 | 3.00 | 0.75 | 8.09 | 6.23 |

*VELOCITY IS THE VELOCITY AT THE PIER FROM HEC-RAS FLOW DISTRIBUTION ASSUMING MAXIMUM FLOW VELOCITY IN SECTION

CONTRACTION SCOUR COMPUTATION RESULTS

| FREQ (yrs) | Q ₁ | Q ₂ | W ₁ | W ₂ | y ₁ | y ₂ | y ₀ | y _c |
|------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|
| 50 | 1747 | 2104 | 62.97 | 59.58 | 4.30 | 5.24 | 3.58 | 1.66 |

**100 YEAR SCOUR CALCULATIONS
PIER SCOUR COMPUTATION RESULTS**

| FREQ (yrs) | K ₁ | K ₂ | K ₃ | Y ₁ | a | Fr ₁ | V | y _s |
|------------|----------------|----------------|----------------|----------------|------|-----------------|------|----------------|
| 100 | 1.0 | 1.0 | 1.1 | 3.92 | 3.00 | 0.78 | 8.81 | 6.52 |

*VELOCITY IS THE VELOCITY AT THE PIER FROM HEC-RAS FLOW DISTRIBUTION ASSUMING MAXIMUM FLOW VELOCITY IN SECTION

CONTRACTION SCOUR COMPUTATION RESULTS

| FREQ (yrs) | Q ₁ | Q ₂ | W ₁ | W ₂ | y ₁ | y ₂ | y ₀ | y _c |
|------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|----------------|
| 100 | 2407 | 2070 | 62.97 | 59.58 | 4.55 | 4.15 | 3.90 | 0.25 |

PIER SCOUR ANALYSIS

USING HEC-18 EQ 7.1

$$y_s = 2.0 * K_1 * K_2 * K_3 * y_1 * (a / y_1)^{0.6} * Fr_{1.0}^{0.04}$$

WHERE:

y_s = PIER SCOUR DEPTH (FT)

K₁ = CORRECTION FACTOR FOR PIER NOSE SHAPE (FOR A GROUP OF CYLINDERS, K₁ = 1.0)

K₂ = CORRECTION FACTOR FOR ANGLE OF ATTACK (ANGLE OF ATTACK = 8°)

K₃ = CORRECTION FACTOR FOR BED CONDITION (DUNE HEIGHT < 10', K₃ = 1.1)

y₁ = FLOW DEPTH DIRECTLY UPSTREAM OF THE PIER (FT)

a = PIER WIDTH (FT)

Fr₁ = V / (g*y₁)^{0.5} = FROUDE NUMBER UPSTREAM OF PIER, WHERE V = VELOCITY AT PIER (FT / SEC),

AND g = 32.2 FT / SEC² (GRAVITATIONAL CONSTANT)

LIVE BED CONTRACTION SCOUR ANALYSIS

USING HEC-18 EQ 6.2

$$y_c / y_1 = (Q_2 / Q_1)^{0.4} * (W_1 / W_2)^{0.4} \text{ and } y_c = y_2 - y_1$$

WHERE:

y_c = AVERAGE CONTRACTION SCOUR DEPTH

y₁ = AVERAGE DEPTH IN MAIN CHANNEL UPSTREAM OF CONTRACTED SECTION

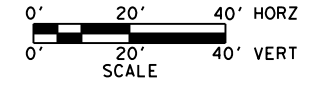
y₂ = AVERAGE DEPTH IN CONTRACTED SECTION

Q₁ = FLOW IN UPSTREAM CHANNEL TRANSPORTING SEDIMENT

Q₂ = FLOW IN CONCENTRATED CHANNEL

W₁ = BOTTOM WIDTH OF MAIN CHANNEL UPSTREAM OF CONTRACTED SECTION

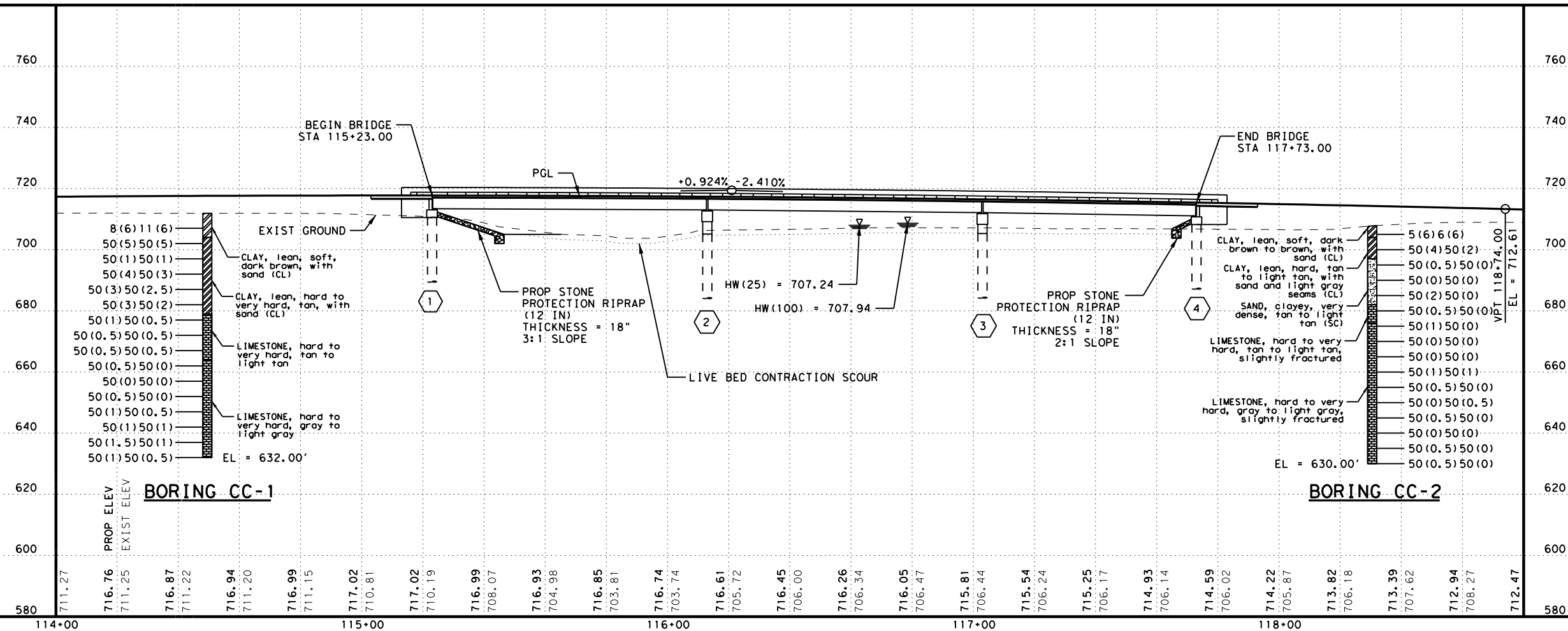
W₂ = BOTTOM WIDTH OF CONTRACTED SECTION LESS THE CUMULATIVE WIDTH OF PIERS



NOTES:

- SCOUR ANALYSIS BASED ON TXDOT GEOTECHNICAL MANUAL (GM) AND FHWA H.E.C.-18 "EVALUATING SCOUR AT BRIDGES".
- THE D50 SOIL PARTICLE SIZE FOR THIS PROJECT IS THE MINIMUM SIZE ALLOWED BY THE GM.
- THE MAXIMUM CALCULATED SCOUR WAS DURING THE 50 YEAR STORM AT A DISCHARGE OF 2104 CFS. THE BRIDGE WAS ANALYZED IN THE 50 AND 100 YR EVENTS.

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CR 118 AT COTTONWOOD CREEK

SCOUR ANALYSIS

SHEET 1 OF 1

| | | | |
|------|------------|-----------|-----------|
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| 0914 | 05 | 204, ETC. | CR 118 |
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| AUS | WILLIAMSON | | 59 |

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| Culvert Station and/or Creek Name followed by applicable end (Lt, Rt or Both) | Description of Box Culvert No. Spans ~ Span X Height | Max Fill Height (Ft) | Applicable Box Culvert Standard (4) | Applicable Wingwall or End Treatment Standard | Skew Angle (0°, 15°, 30° or 45°) | Side Slope or Channel Slope Ratio (SL:1) | T Culvert Top Slab Thickness (In) | U Culvert Wall Thickness (In) | C Estimated Curb Height (Ft) | Hw (1) Height of Wingwall (Ft) | A Curb to End of Wingwall (Ft) | B Offset of End of Wingwall (Ft) | Lw Length of Longest Wingwall (Ft) | Ltw Culvert Toewall Length (Ft) | Atw Anchor Toewall Length (Ft) | Riprap Apron (CY) | Class "C" Conc (Curb) (CY) (2) | Class "C" Conc (Wingwall) (CY) (3) | Total Wingwall Area (SF) |
|---|---|----------------------|-------------------------------------|---|----------------------------------|--|-----------------------------------|-------------------------------|------------------------------|--------------------------------|--------------------------------|----------------------------------|------------------------------------|---------------------------------|--------------------------------|-------------------|--------------------------------|------------------------------------|--------------------------|
| 122+66.34 (Lt) | 1 ~12' x 4' | 0.25' | SCP-12 | SETB-FW-0 | 0° | 4:1 | 12" | 12" | 0.250' | 5.000' | 18.667' | 10.777' | 21.554' | 14.000' | 33.554' | 0.0 | 0.1 | 10.2 | N/A |
| 122+66.34 (Rt) | 1 ~12' x 4' | 1.75' | SCP-12 | SETB-FW-0 | 0° | 4:1 | 12" | 12" | 0.250' | 5.000' | 18.667' | 10.777' | 21.554' | 14.000' | 33.554' | 0.0 | 0.1 | 10.2 | N/A |
| | | | | | | | | | | | | | | | | | | | |
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NOTES:
 Skew = 0° on SW-0, FW-0, SETB-CD, SETB-SW-0, and SETB-FW-0 standard sheets;
 30° maximum for safety end treatment

SL:1 = Horizontal : 1 Vertical

- Side slope at culvert for flared or straight wingwalls.
- Channel slope for parallel wingwalls.
- Slope must be 3:1 or flatter for safety end treatments.

T = Box culvert top slab thickness. Dimension can be found on the applicable box culvert standard sheet.

U = Box culvert wall thickness. Dimension can be found on the applicable box culvert standard sheet.

C = Curb height

See applicable wing or end treatment standard sheets for calculations of Hw, A, B, Lw, Ltw, Atw, and Total Wingwall Area.

Hw = Height of wingwall

A = Distance from face of curb to end of wingwall (not applicable to parallel or straight wingwalls)

B = Offset of end of wingwall (not applicable to parallel or straight wingwalls)

Lw = Length of longest wingwall.

Ltw = Length of culvert toewall (not applicable when using riprap apron)

Atw = Length of anchor toewall (applicable to safety end treatment only)

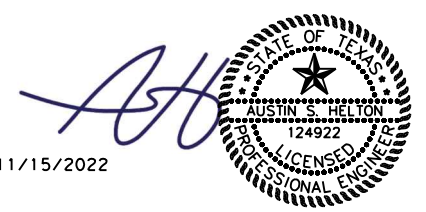
Total Wingwall Area = Wingwall area in sq. ft. for two wingwalls (one structure end) if Lt or Rt.
 Area for four wingwalls (two structure ends) if Both.

- ① Round the wall heights shown to the nearest foot for bidding purposes.
- ② Concrete volume shown is for box culvert curb only. For curbs using the Box Culvert Rail Mounting Details (RAC) standard sheet quantities shown must be increased by a factor of 2.25. If Class S concrete is required for the top slab of the culvert, also provide Class S concrete for the curb. Curb concrete is considered part of the Box Culvert for payment.
- ③ Concrete volume shown is total of wings, footings, culvert toewall (if any), anchor toewalls (if any) and wingwall toewalls. Riprap aprons, culverts, and curb quantities are not included.
- ④ Regardless of the type of culvert shown on this sheet, the Contractor has the option of furnishing cast-in-place or precast culverts unless otherwise shown elsewhere on the plans. If the Contractor elects to provide culverts of a different type than those shown on this sheet, it is the Contractor's responsibility to make the necessary adjustments to the dimensions and quantities shown.

SPECIAL NOTE:

This sheet is a supplement to the box culvert standards. It is to be filled out by the culvert specifier and provides dimensions for the construction of the box culvert wingwalls and safety end treatments.

An Excel 2010 spreadsheet to assist in completing this table can be downloaded from the Bridge Standards (English) web page on the TxDOT web site. The completed sheet must be signed, sealed, and dated by a licensed Professional Engineer.



Texas Department of Transportation
 Bridge Division Standard

BOX CULVERT SUPPLEMENT WINGS AND END TREATMENTS

BCS

| | | | | | |
|-----------|----------------|-----------|------------|-----------|-----------|
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| CONT | 0914 | SECT | 05 | JOB | 204, ETC. |
| REVISIONS | HIGHWAY | | CR 118 | | |
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**TABLE OF DIMENSIONS AND REINFORCING STEEL
(Wings for One Structure End)**

| Dimensions | | | | | Variable Reinforcing | | | | Estimated Quantities (3) | |
|--------------------------------|-------|-------|-------|----|----------------------|-------|---------|-------|--------------------------|--------------|
| Maximum Wingwall Height Hw (9) | W | X | Y | Z | Bars J1 | | Bars J2 | | Reinf (Lb/Ft) | Conc (CY/Ft) |
| | | | | | Size | Spa | Size | Spa | | |
| 2'-6" | 2'-5" | 1'-0" | 9" | 7" | #4 | 1'-0" | #4 | 1'-0" | 33.73 | 0.248 |
| 3'-0" | 2'-5" | 1'-0" | 9" | 7" | #4 | 1'-0" | #4 | 1'-0" | 37.07 | 0.261 |
| 3'-6" | 2'-5" | 1'-0" | 9" | 7" | #4 | 1'-0" | #4 | 1'-0" | 37.74 | 0.273 |
| 4'-0" | 2'-5" | 1'-0" | 9" | 7" | #4 | 1'-0" | #4 | 1'-0" | 38.41 | 0.285 |
| 4'-6" | 3'-2" | 1'-6" | 1'-0" | 7" | #4 | 1'-0" | #4 | 1'-0" | 41.75 | 0.330 |
| 5'-0" | 3'-2" | 1'-6" | 1'-0" | 7" | #4 | 1'-0" | #4 | 1'-0" | 45.09 | 0.343 |
| 5'-6" | 3'-2" | 1'-6" | 1'-0" | 7" | #4 | 1'-0" | #4 | 1'-0" | 45.75 | 0.355 |
| 6'-0" | 3'-2" | 1'-6" | 1'-0" | 7" | #4 | 1'-0" | #4 | 1'-0" | 46.42 | 0.367 |
| 7'-0" | 3'-8" | 1'-9" | 1'-3" | 7" | #4 | 1'-0" | #4 | 1'-0" | 52.77 | 0.414 |
| 8'-0" | 4'-2" | 2'-0" | 1'-6" | 8" | #5 | 1'-0" | #4 | 1'-0" | 60.19 | 0.486 |
| 9'-0" | 4'-8" | 2'-3" | 1'-9" | 8" | #4 | 6" | #4 | 6" | 81.49 | 0.535 |
| 10'-0" | 5'-2" | 2'-6" | 2'-0" | 8" | #5 | 6" | #4 | 6" | 97.25 | 0.584 |
| 11'-0" | 5'-8" | 2'-9" | 2'-3" | 8" | #6 | 6" | #5 | 6" | 133.65 | 0.634 |
| 12'-0" | 6'-2" | 3'-0" | 2'-6" | 9" | #7 | 6" | #5 | 6" | 162.29 | 0.721 |

TABLE OF WING WALL REINFORCING (Two-Wings)

| Bar | Size | No. | Spa |
|-----|------|-----|-------|
| D | #5 | ~ | 1'-0" |
| E | #4 | ~ | 1'-0" |
| F | #4 | ~ | 1'-0" |
| G | #6 | 4 | ~ |
| M | #4 | 4 | ~ |
| P | #4 | ~ | 1'-0" |
| R | #5 | 6 | ~ |
| V | #4 | ~ | 1'-0" |

TABLE OF ESTIMATED CULVERT TOEWALL QUANTITIES

| Bar | Size | No. | Spa |
|---------------|-------|-----|-------|
| L | #4 | ~ | 1'-6" |
| Q | #4 | 1 | ~ |
| Reinf (Lb/Ft) | 2.45 | | |
| Conc (CY/Ft) | 0.037 | | |

TABLE OF ESTIMATED ANCHOR TOEWALL QUANTITIES

| Bar | Size | No. | Spa |
|---------------|-------|-----|-------|
| K | #4 | ~ | 1'-0" |
| N | #5 | 6 | ~ |
| OL | #4 | 6 | ~ |
| Reinf (Lb/Ft) | 9.82 | | |
| Conc (CY/Ft) | 0.074 | | |

- Extend Bars P 3'-0" Min into bottom slab of box culvert.
- Adjust to fit as necessary to maintain 1#2" clear cover and 4" Min between bars.
- Quantities shown are based on an average wing height for two wings (one structure end). To determine total quantities for two wings multiply the tabulated values by Lw.
- Recommended values of slope are: 3:1, 4:1, and 6:1. Provide 3:1 or flatter slope.
- When shown elsewhere on the plans, construct 5" deep concrete riprap. Payment for riprap is as required by Item 432, "Riprap". Unless otherwise shown on the plans or directed by the Engineer, extend construction joints or grooved joints, oriented in the direction of flow, across the full distance of the riprap, at intervals of approximately 20'. When such riprap is provided, the culvert toewall shown in SECTION B-B is not required.
- At Contractor's option, end the culvert toewall flush with wingwall toewall. Adjust reinforcing as needed.
- 3" Min to 5'-0" Max. Estimated curb heights are shown elsewhere in the plans. For structures without railing and curbs taller than 1'-0", refer to the Extend Curb Details (ECD) standard sheet.
- For vehicle safety, reduce curb heights, if necessary, to provide a maximum 3" projection above finished grade. No changes will be made in quantities and no additional compensation will be allowed for this work.
- See Table of Maximum Wing Heights for various slopes. Height is limited based on a 33'-6" maximum safety pipe runner length.

TABLE OF MAXIMUM WING HEIGHTS (9)

| Side Slope | Hw Max |
|------------|--------|
| 3:1 | 11'-5" |
| 4:1 | 8'-10" |
| 6:1 | 6'-1" |

WING DIMENSION CALCULATIONS:

$$Hw = H + T + C - 0.250' \quad (9)$$

$$A = (Hw - 0.333') (SL)$$

$$B = (A) (\tan 30^\circ)$$

$$Lw = (A) + \cos 30^\circ$$

For cast-in-place culverts:

$$Ltw = (N) (S) + (N + 1) (U)$$

For precast culverts:

$$Ltw = (N) (2U + S) + (N - 1) (0.500')$$

$$Lc = (Ltw) - (2U)$$

$$Atw = (Lc) + (2B)$$

$$\text{Total Wingwall Area (two wings ~ SF)} = (Hw + 0.333') (Lw)$$

Hw = Height of wingwall (feet)
Atw = Anchor toewall length (feet)
Lw = Length of wingwall (feet)
N = Number of culvert barrels
SL:1 = Side slope ratio (horizontal : 1 vertical)
Ltw = Culvert toewall length (feet)
Lc = Culvert curb between wings (feet)

See applicable box culvert standard for H, S, T, and U values.
See Table of Maximum Wall Heights for limits on Hw.

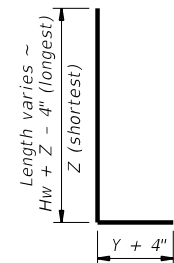
MATERIAL NOTES:

- Provide Grade 60 reinforcing steel.
- Provide galvanized reinforcing steel if required elsewhere in the plans.
- Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.
- Provide Class "C" concrete (f'c = 3,600 psi).
- Adjust reinforcing as necessary to provide a minimum clear cover of 1 1/2".
- Provide pipe runners and anchor pipes meeting the requirements of ASTM A53 (Type E or S, Gr B), ASTM A500 Gr B, or API 5LX52.
- Provide ASTM A307 bolts and nuts.
- Provide ASTM A36 steel plates.
- Galvanize all steel components, except reinforcing unless required elsewhere in the plans, after fabrication.
- Repair galvanizing damaged during transport or construction in accordance with the Item 445, "Galvanizing".
- For optional adhesive anchors, install adhesive anchorages in accordance with the manufacturer's instructions including hole size, drilling equipment and method, hole cleaning equipment and method, mixing and dispensing adhesive, and anchor insertion. Do not alter the manufacturer's mixing nozzle or dispenser. Provide anchorage rods that are clean and free of grease, oil, or any other foreign material. Demonstrate hole cleaning method to the Engineer for approval and continue the approved process for all anchorage locations. Test adhesive anchors in accordance with Item 450.3.3, "Tests." Test 3 anchors per 100 anchors installed.

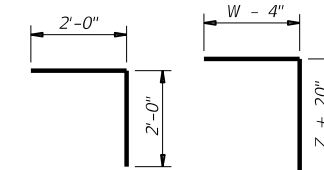
GENERAL NOTES:

- Designed according to AASHTO LRFD Bridge Design Specifications.
- The safety end treatments shown herein are intended for use in those installations where out of control vehicles are likely to traverse the openings approximately perpendicular to the pipe runners.
- Pipe runners are designed for a traversing load of 1,800 pounds at yield as recommended by Research Report 280-1, "Safety Treatment of Roadside Cross-Drainage Structures", Texas Transportation Institute, March 1981.
- When structure is founded on solid rock, depth of toewalls for culverts and wingwalls may be reduced or eliminated as directed by the Engineer.
- All bolts, nuts, washers, brackets, angles, and pipe runners are considered parts of the safety end treatment for payment.
- The quantities for pipe runners, reinforcing steel, and concrete, resulting from the formulas given herein are for Contractor's information only.
- See the Box Culvert Supplement (BCS) standard sheet for additional dimensions and information.

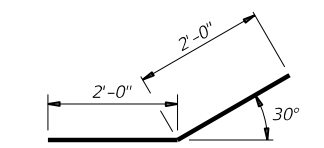
Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing dimensions are out-to-out of bars.



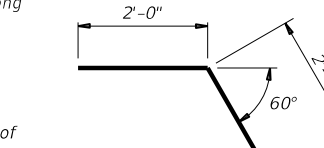
BARS J1 **BARS V**



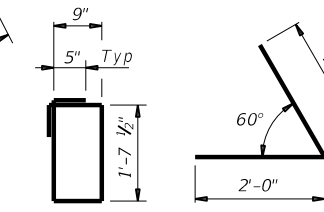
BARS L **BARS J2**



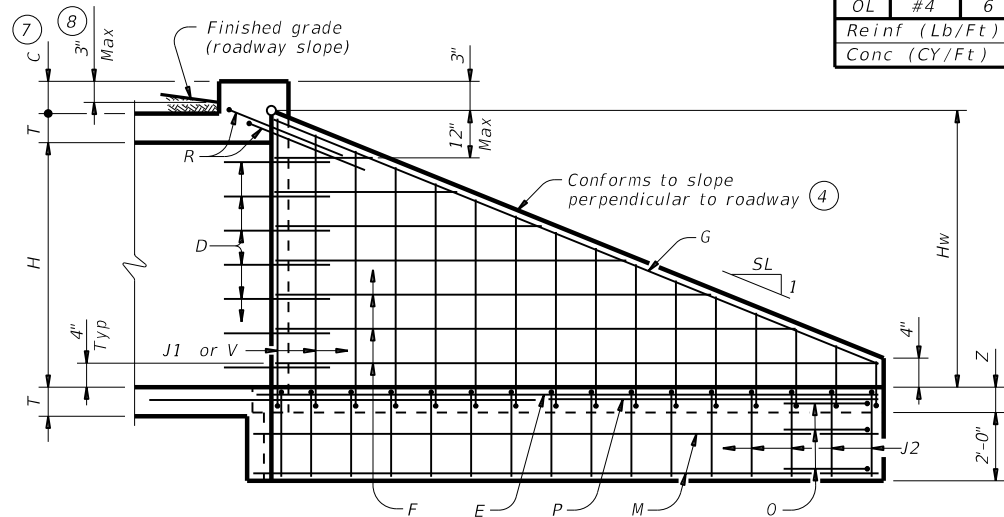
BARS D



BARS R

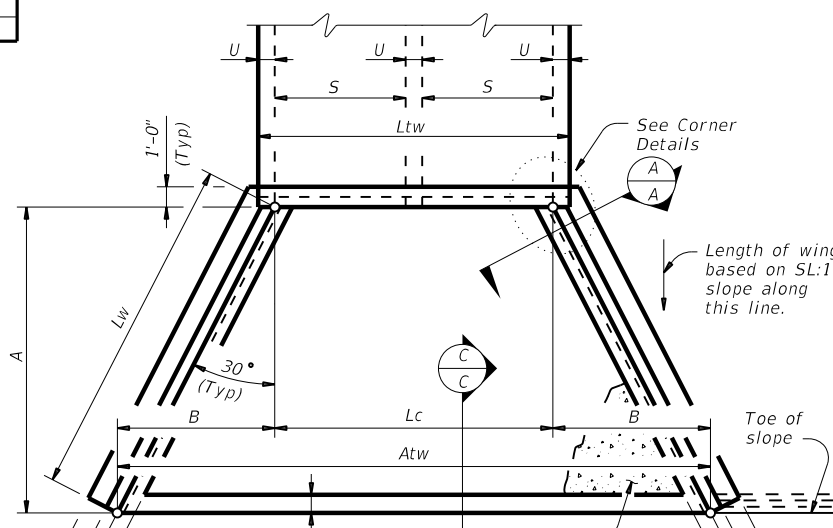


BARS K
(Length = 5'-5") **BARS OL**



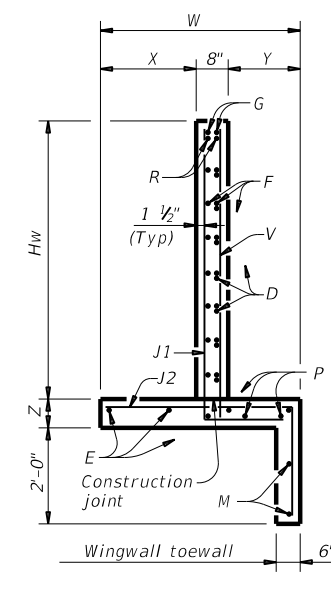
INSIDE ELEVATION OF WINGWALL

(Showing reinforcing. Culvert and culvert toewall reinforcing not shown for clarity.)

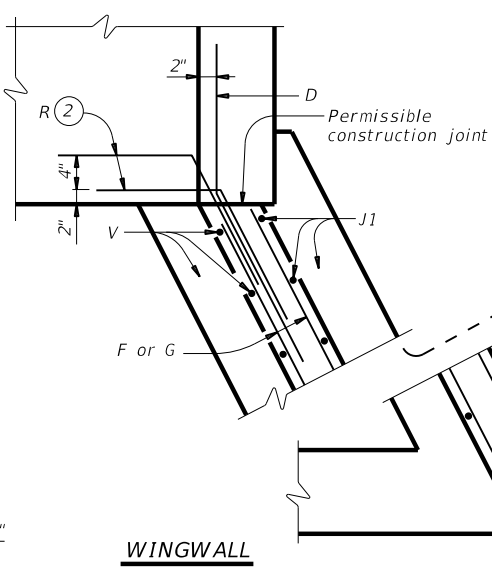


STRUCTURAL PLAN

(Showing dimensions.)

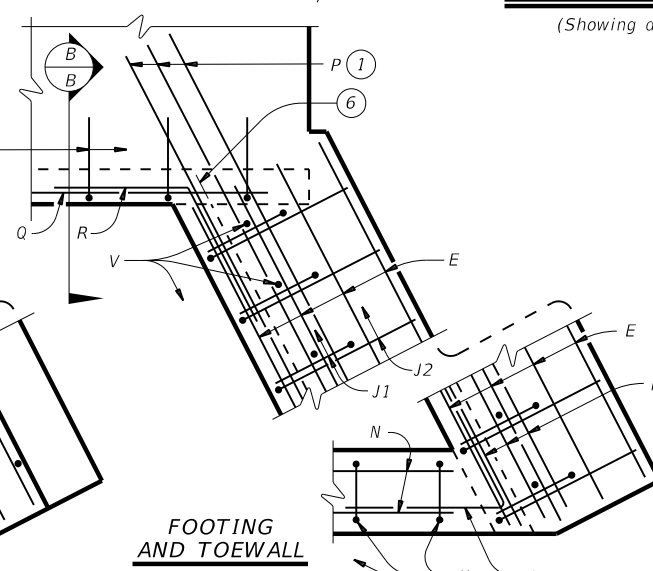


SECTION A-A

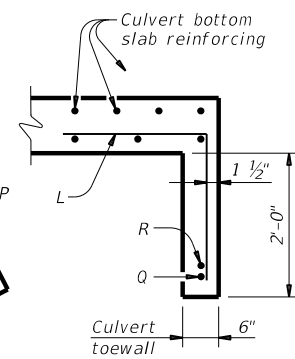


CORNER DETAILS

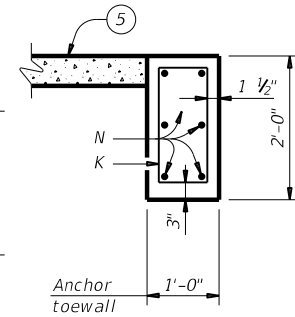
(Culvert and culvert toewall reinforcing not shown for clarity.)



FOOTING AND TOEWALL



SECTION B-B (5)



SECTION C-C

Texas Department of Transportation
Bridge Division Standard

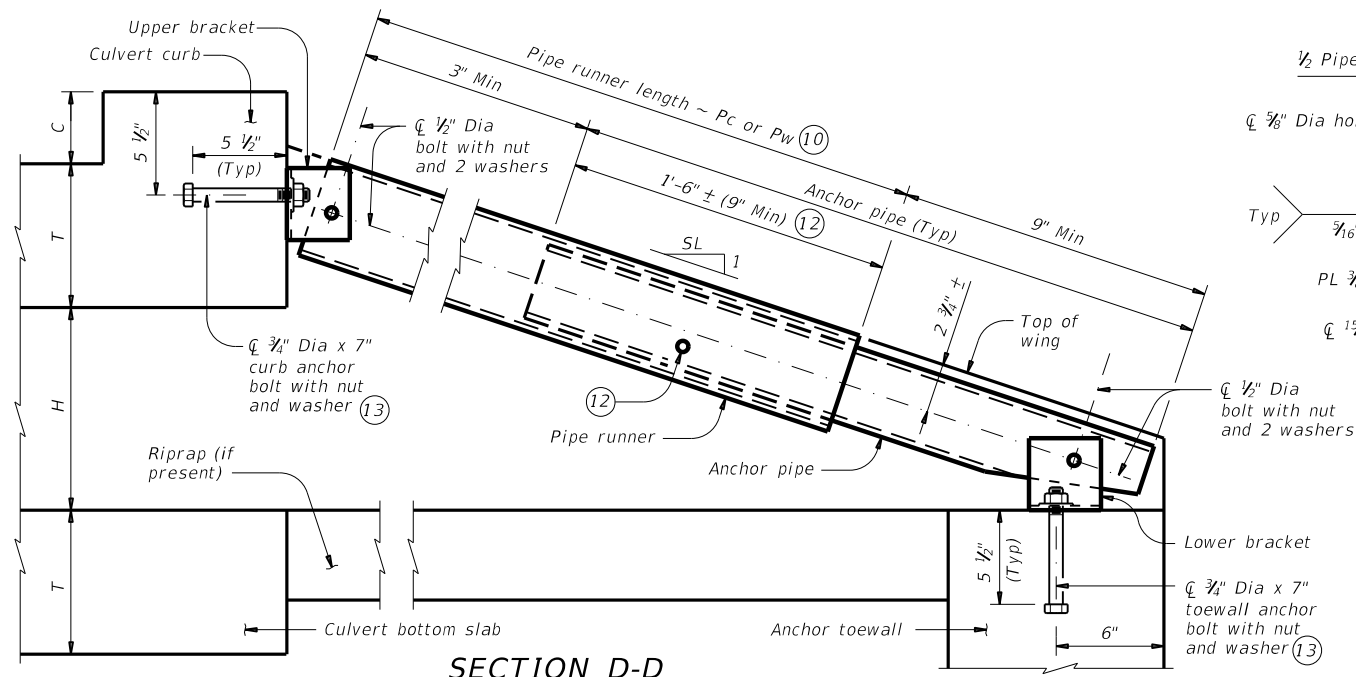
SAFETY END TREATMENT WITH FLARED WINGS
FOR 0° SKEW BOX CULVERTS
TYPE I ~ CROSS DRAINAGE

SETB-FW-0

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| DIST | COUNTY | | | SHEET NO. |
| AUS | WILLIAMSON | | | 61 |

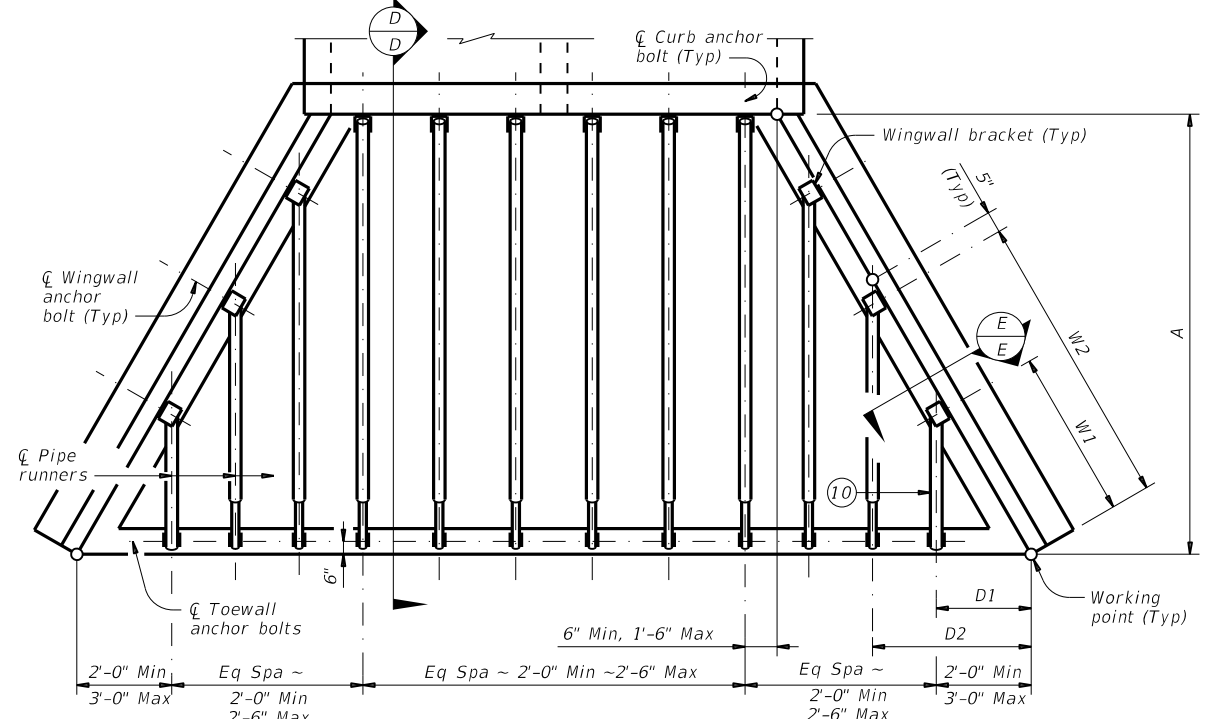
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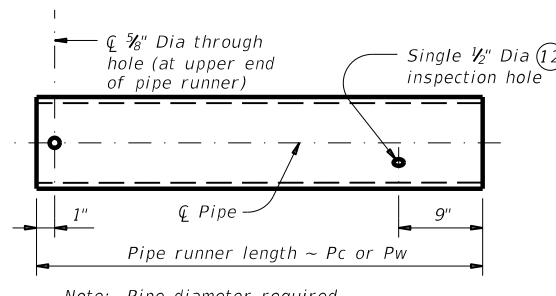


SECTION D-D

(Showing curb pipe runner. Except for upper bracket, wingwall pipe runners are similar.)

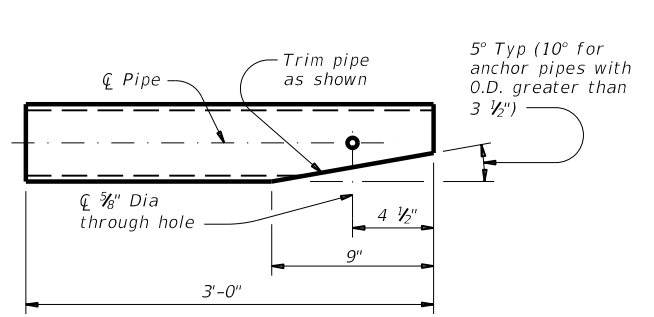


PIPE RUNNER PLAN

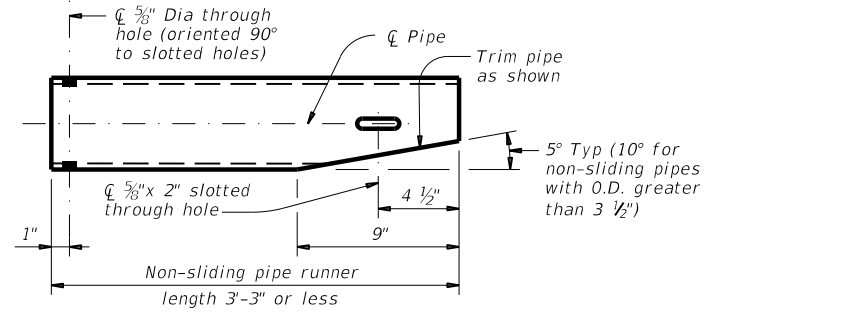


Note: Pipe diameter required for curb pipe runner is also used for wingwall pipe runner.

PIPE RUNNER DETAILS

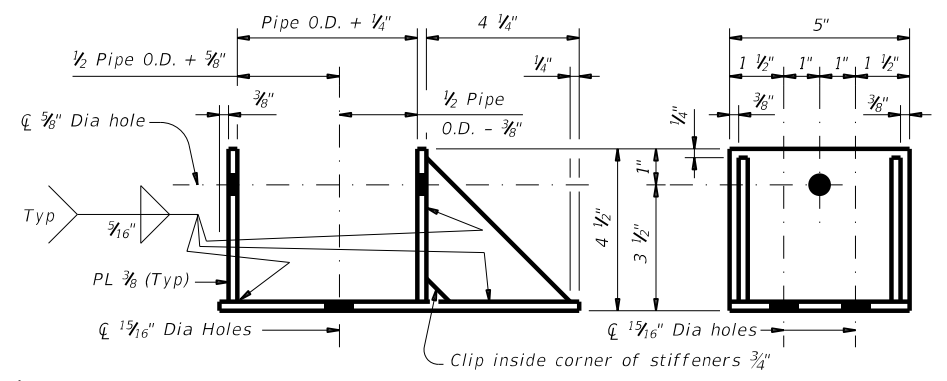


ANCHOR PIPE DETAILS



Note: Pipe size is the same as required for curb pipe runner. Adjust the corresponding lower bracket accordingly.

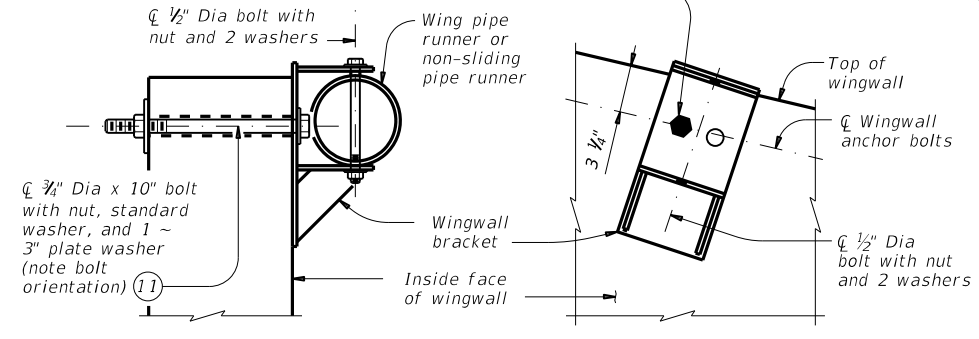
NON-SLIDING PIPE RUNNER DETAILS



ELEVATION

SIDE VIEW

Install 3/4 inch anchor bolt in hole nearest to the culvert curb. Other bolt hole is intended for use on the opposite hand wingwall.



SECTION E-E

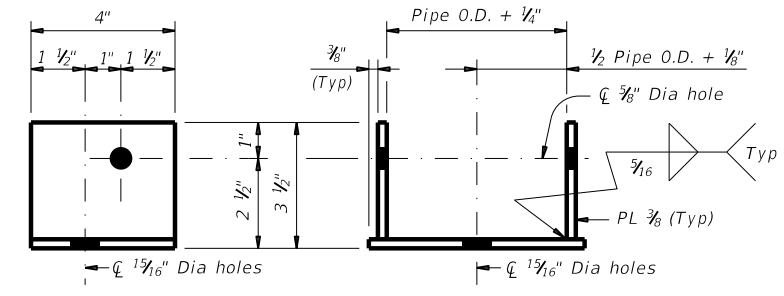
(Showing installed bracket.)

ELEVATION

(Showing installed bracket normal to wall. Pipe not shown for clarity.)

Note: Match wingwall bracket to the upper curb bracket size.

WINGWALL BRACKET DETAILS



SIDE VIEW

ELEVATION

Note: Match upper and lower brackets, except for the brackets used with non-sliding pipe runners, to the required pipe diameters as shown in the table.

UPPER AND LOWER BRACKET DETAILS

MAXIMUM PIPE RUNNER LENGTHS AND REQUIRED PIPE RUNNER SIZES

| Maximum Pipe Runner Length (Pc or Pw) | Required Pipe Runner Size | | | Required Anchor Pipe Size | | |
|---------------------------------------|---------------------------|-----------|-----------|---------------------------|-----------|-----------|
| | Pipe Size | Pipe O.D. | Pipe I.D. | Pipe Size | Pipe O.D. | Pipe I.D. |
| 9'-4" | 3" STD | 3.500" | 3.068" | 2" STD | 2.375" | 2.067" |
| 19'-0" | 4" STD | 4.500" | 4.026" | 3" STD | 3.500" | 3.068" |
| 33'-6" | 5" STD | 5.563" | 5.047" | 4" STD | 4.500" | 4.026" |

- 10 If pipe runner length (Pw) is 1'-9" or less replace the normal pipe runner and anchor pipe with a single non-sliding pipe runner. See Non-Sliding Pipe Runner Details for additional information.
- 11 At Contractor's option, 3/8" diameter hole may be formed or cored drilled. Percussion drilling is not permitted. Adjust placement of reinforcing steel as necessary to avoid bolt holes.
- 12 After installation of pipe runner, use the 1/2" inspection hole to ensure that the lap of the anchor pipe with the pipe runner is adequate.
- 13 At Contractor's option, an adhesive anchor may be used. Provide 3/4" Dia adhesive anchors that meet the requirements of ASTM A307 Gr A fully threaded rods. Embed threaded rods into curb, wingwalls, and toewall using a Type III, Class C, D, E, or F anchor adhesive. Minimum embedment depth is 5 1/2". Provide anchor adhesive able to achieve a basic bond strength in tension, Nba, of 20 kips. Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use.

PIPE RUNNER DIMENSION CALCULATIONS:

$$Wn = (2.000)(Dn) - (0.416')$$

$$Pwn = (Dn)(K2) - (2.063')$$

$$Pw1 \text{ Non-Sliding Pipe Runner (If required)} = (D1)(K2) - (0.563')$$

$$Pc = (A)(K1) - (1.688')$$

Wn = Distance from working point to centerline anchor bolt measured along bottom inside face of wing (feet)
 Dn = Distance from working point to centerline pipe runner measured along outside face of anchor toewall (feet)
 Pw = Wingwall pipe runner length (feet)
 Pc = Curb pipe runner length (feet)
 K = Constant values for use in formulas

| Slope SL:1 | K1 | K2 |
|------------|---------|---------|
| 3:1 | ~ 1.054 | ~ 1.826 |
| 4:1 | ~ 1.031 | ~ 1.785 |
| 6:1 | ~ 1.014 | ~ 1.756 |

n = Wing pipe runner number

Texas Department of Transportation
SAFETY END TREATMENT WITH FLARED WINGS
 FOR 0° SKEW BOX CULVERTS TYPE I ~ CROSS DRAINAGE

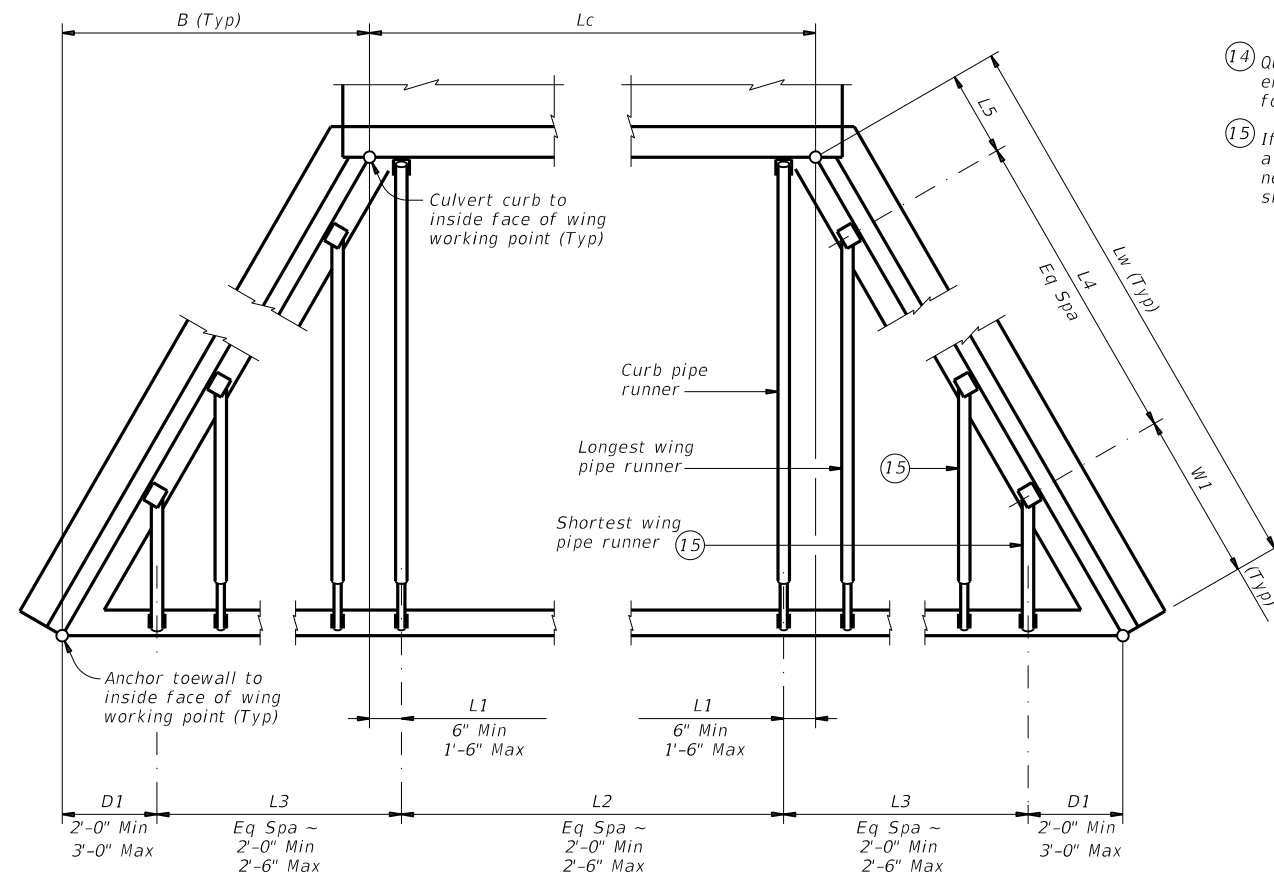
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| Culvert Station and/or Creek name followed by applicable end (Lt, Rt or Both) (14) | Lc (Ft) | L1 (Ft) | L2 | | | D1 (Ft) | L3 | | | W1 (Ft) | L4 | | | L5 (Ft) | Curb Pipe Runner (Pc) | | Longest Wing Pipe Runner (Pw) (Ft) | Shortest Wing Pipe Runner (Pw) (Ft) | Non-Sliding Wing Pipe Runner (if applicable) (Ft) | Curb, Wing, and/or Non-Sliding Pipe Runners | | 3'-0" Anchor Pipe | |
|--|---------|---------|---------|-------------|---------------------|---------|---------|-------------|---------------------|---------|---------|-------------|---------------------|---------|-----------------------|-------------|------------------------------------|-------------------------------------|---|---|------------------------|---------------------|------------------------|
| | | | No. Spa | Spa at (Ft) | Overall Length (Ft) | | No. Spa | Spa at (Ft) | Overall Length (Ft) | | No. Spa | Spa at (Ft) | Overall Length (Ft) | | No. | Length (Ft) | | | | Size (3", 4" or 5") | Total Length (Ft) (14) | Size (2", 3" or 4") | Total Length (Ft) (14) |
| | | | | | | | | | | | | | | | | | | | | | | | |
| 122+66.34 (Lt) | 12.000' | 1.000' | 4 | 2.500' | 10.000' | 3.000' | 4 | 2.194' | 8.777' | 5.583' | 3 | 4.389' | 13.166' | 2.805' | 5 | 17.563' | 15.042' | 3.292' | N/A | 4" | 161.146' | 3" | 39.000' |
| 122+66.34 (Rt) | 12.000' | 1.000' | 4 | 2.500' | 10.000' | 3.000' | 4 | 2.194' | 8.777' | 5.583' | 3 | 4.389' | 13.166' | 2.805' | 5 | 17.563' | 15.042' | 3.292' | N/A | 4" | 161.146' | 3" | 39.000' |



PIPE RUNNER LAYOUT

- (14) Quantities shown are for one structure end if Lt or Rt. Quantities shown are for two structure ends if Both.
- (15) If the outermost wing pipe runner is a non-sliding pipe runner, consider the next outermost wing pipe runner as the shortest.

SPECIAL NOTE:
 This tabular sheet is to be filled out by the culvert specifier and provides information for the construction details and quantities of pipe runners.
 An Excel 2010 spreadsheet to assist in completing this table can be downloaded from the Bridge Standards (English) web page on the TxDOT web site. The completed sheet must be signed, sealed, and dated by a licensed Professional Engineer.
 Note that the tabular quantities are given for estimating purposes only. It is likely that these quantities will change due to field conditions. Therefore, all dimensions must be verified by the Contractor in the field prior to fabrication of the safety end treatment components.

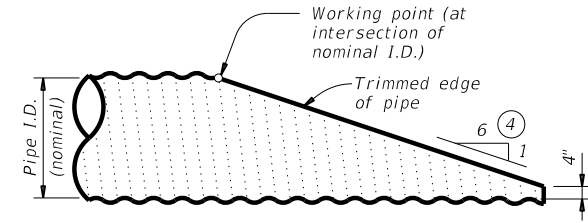


11/15/2022

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|--|-----------|------------|-----------|---------------------------------|--|
| | | | | Bridge Division Standard | |
| SAFETY END TREATMENT WITH FLARED WINGS FOR 0° SKEW BOX CULVERTS TYPE I ~ CROSS DRAINAGE | | | | | |
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| | AUS | WILLIAMSON | 63 | | |

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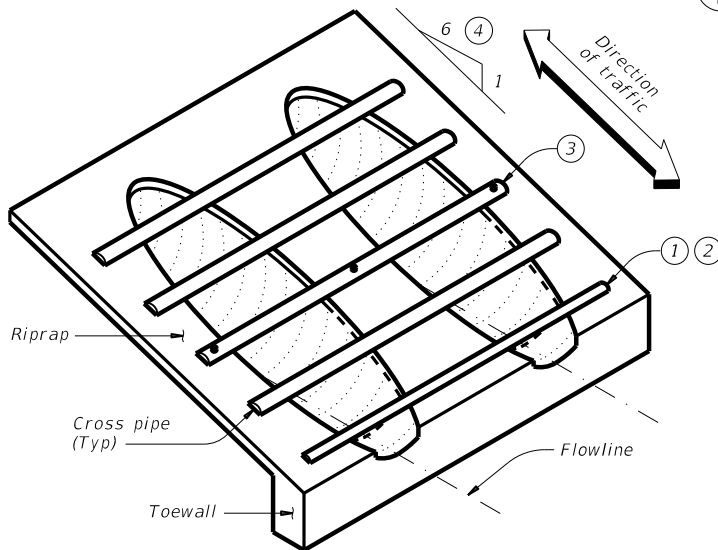
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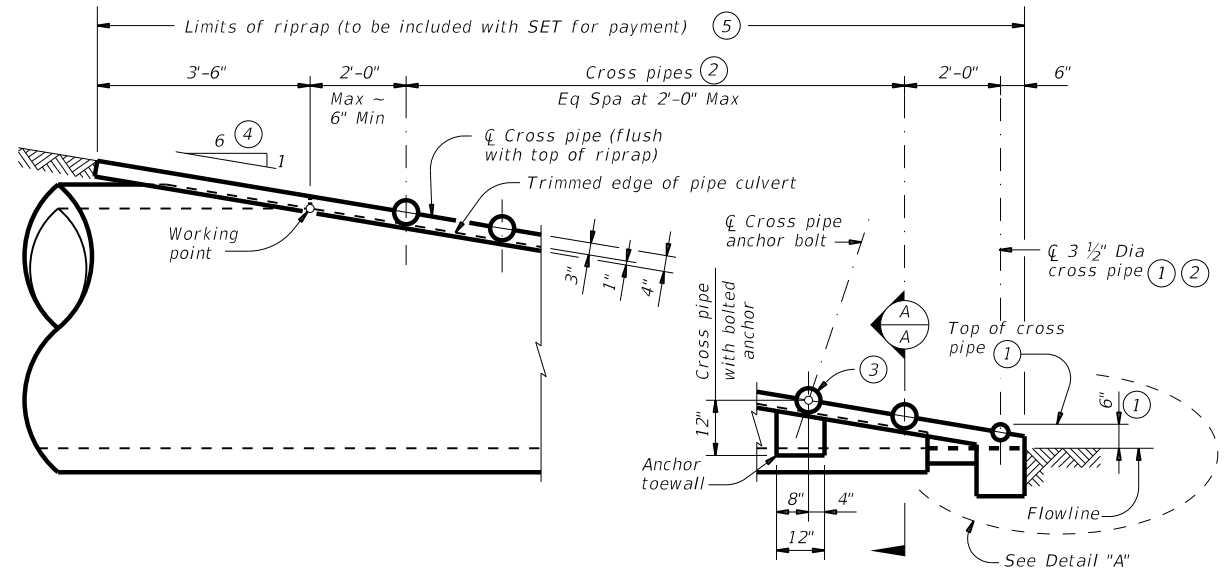
NOTE: All cross pipes, calculations, and dimensions are based on the pipe culverts mitered as shown in this detail. Alternate styles of mitered ends will require that appropriate adjustments be made to the values presented on this standard.

SIDE ELEVATION OF TYPICAL PIPE CULVERT MITER

(Showing corrugated metal pipe (CMP) culvert. Details at reinforced concrete pipe (RCP) culvert are similar.)

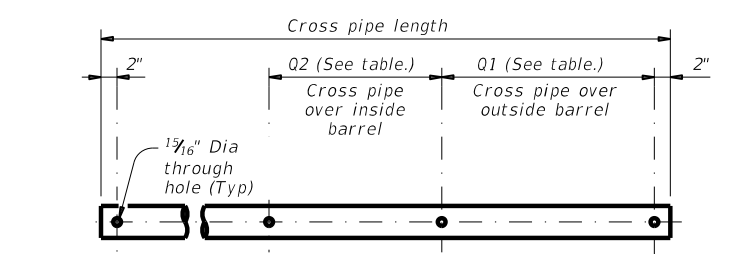


ISOMETRIC VIEW OF TYPICAL INSTALLATION

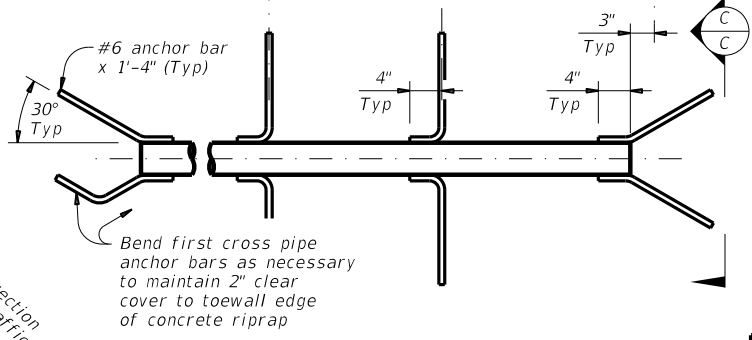


SIDE ELEVATION OF CAST-IN-PLACE CONCRETE

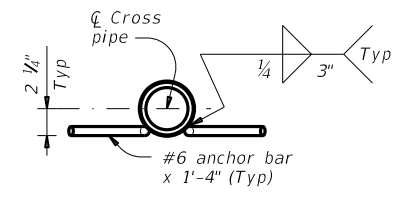
(Showing reinforced concrete pipe (RCP) culvert. Details at corrugated metal pipe (CMP) culvert are similar.)



PIPE WITH BOLTED ANCHOR

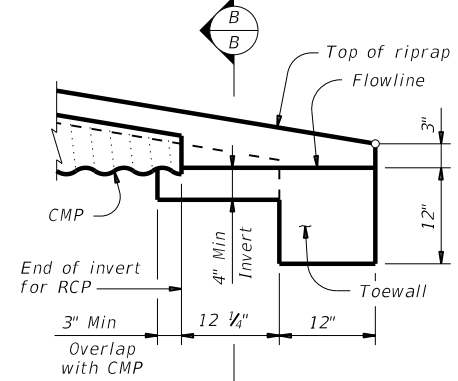


PIPE WITH ANCHOR BARS



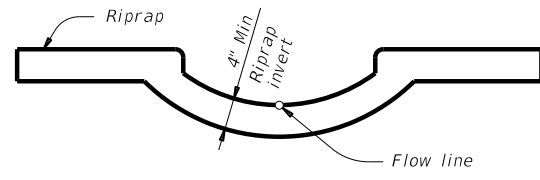
SECTION C-C

CROSS PIPE DETAILS



DETAIL "A"

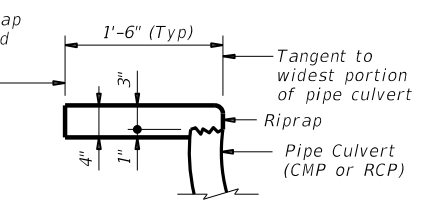
(Showing invert with corrugated metal pipe (CMP) culvert. Reinforced concrete pipe (RCP) culvert details are similar. Cross pipes not shown for clarity.)



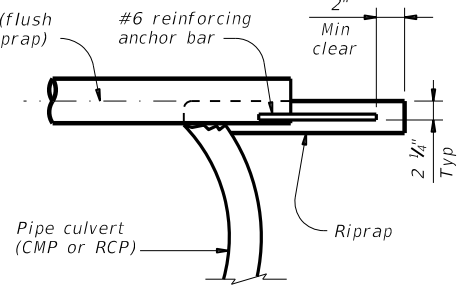
SECTION B-B

(Cross pipes not shown for clarity.)

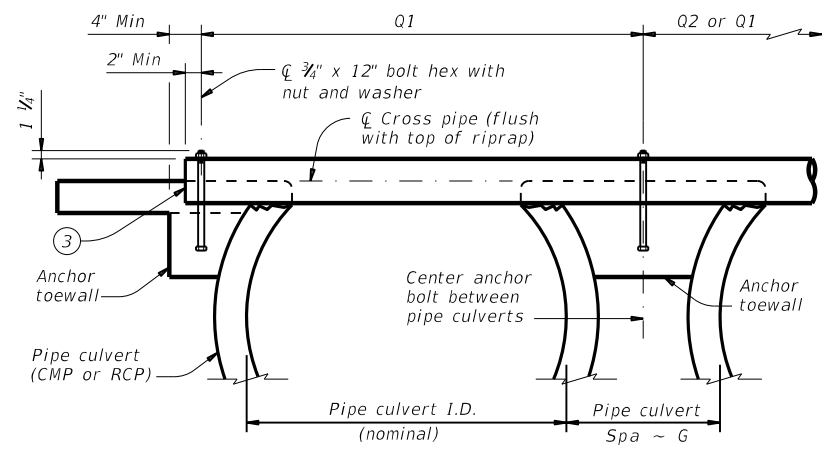
Limits of riprap (to be included with SET for payment) ⑤



SHOWING TYPICAL PIPE CULVERT AND RIPRAP



SHOWING CROSS PIPE WITH ANCHOR BAR



SECTION A-A

CROSS PIPE LENGTHS, REQUIRED PIPE SIZES, AND RIPRAP QUANTITIES

| Nominal Culvert I.D. | Conc Riprap (CY) ⑥ | Pipe Culvert Spa ~ G | Single Barrel ~ Q1 | Multi-Barrel ~ Q1 | Q2 | Conditions for Use of Cross Pipes | Cross Pipe Sizes |
|----------------------|--------------------|----------------------|--------------------|-------------------|----------|-----------------------------------|--------------------------|
| 12" | 0.6 | 0' - 9" | N/A | 2' - 1" | 1' - 9" | 3 or more pipe culverts | 3" Std (3.500" O.D.) |
| 15" | 0.7 | 0' - 11" | N/A | 2' - 5" | 2' - 2" | | |
| 18" | 0.8 | 1' - 2" | N/A | 2' - 10" | 2' - 8" | | |
| 21" | 0.9 | 1' - 4" | N/A | 3' - 2" | 3' - 1" | | |
| 24" | 0.9 | 1' - 7" | N/A | 3' - 6" | 3' - 7" | 3 or more pipe culverts | 3 1/2" Std (4.000" O.D.) |
| 27" | 1.0 | 1' - 8" | N/A | 3' - 10" | 3' - 11" | 2 or more pipe culverts | |
| 30" | 1.1 | 1' - 10" | N/A | 4' - 2" | 4' - 4" | All pipe culverts | |
| 33" | 1.2 | 1' - 11" | 4' - 2" | 4' - 5" | 4' - 8" | All pipe culverts | 4" Std (4.500" O.D.) |
| 36" | 1.3 | 2' - 1" | 4' - 5" | 4' - 9" | 5' - 1" | All pipe culverts | |
| 42" | 1.5 | 2' - 4" | 4' - 11" | 5' - 5" | 5' - 10" | All pipe culverts | 5" Std (5.563" O.D.) |
| 48" | 1.7 | 2' - 7" | 5' - 5" | 6' - 0" | 6' - 7" | All pipe culverts | |
| 54" | 2.0 | 3' - 0" | 5' - 11" | 6' - 9" | 7' - 6" | All pipe culverts | |
| 60" | 2.2 | 3' - 3" | 6' - 5" | 7' - 4" | 8' - 3" | All pipe culverts | |
| 66" | 2.4 | 3' - 3" | 6' - 11" | 7' - 10" | 8' - 9" | All pipe culverts | |
| 72" | 2.7 | 3' - 4" | 7' - 5" | 8' - 5" | 9' - 4" | All pipe culverts | |

- The proper installation of the first cross pipe is critical for vehicle safety. Place the top of the first cross pipe no more than 6" above the flow line.
- Provide cross pipes, except the first bottom pipe, of the size shown in the table. Provide a 3 1/2" standard pipe (4" O.D.) for the first bottom pipe.
- Install the third cross pipe from the bottom of the culvert using a bolted connection. Ensure that riprap concrete does not flow into the cross pipe so as to permit disassembly of the bolted connection to allow cleanout access. At the Contractor's option, install all other cross pipes using the bolted connection details.
- Match cross slope as shown elsewhere in the plans. Cross slope of 6:1 or flatter is required for vehicle safety.
- Riprap placed beyond the limits shown will be paid for as concrete riprap in accordance with Item 432, "Riprap".
- Quantities shown are for one end of one reinforced concrete pipe (RCP) culvert. For multiple pipe culverts or for corrugated metal pipe (CMP) culverts, quantities will need to be adjusted. Riprap quantities are for contractor's information only.

MATERIAL NOTES:

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise. Provide cross pipes that meet the requirements of ASTM A53 (Type E or S, Gr B), ASTM A500 (Gr B), or API 5LX52. Provide ASTM A307 bolts and nuts. Galvanize all steel components, except concrete reinforcing, after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

GENERAL NOTES:

Cross pipes are designed for a traversing load of 10,000 pounds at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981. Safety end treatments (SET) shown herein are intended for use in those installations where out of control vehicles are likely to traverse the openings approximately perpendicular to the cross pipes. Construct concrete riprap and all necessary inverts in accordance with the requirements of Item 432, "Riprap". Payment for riprap and toewall is included in the Price Bid for each Safety End Treatment.

Bridge Division Standard

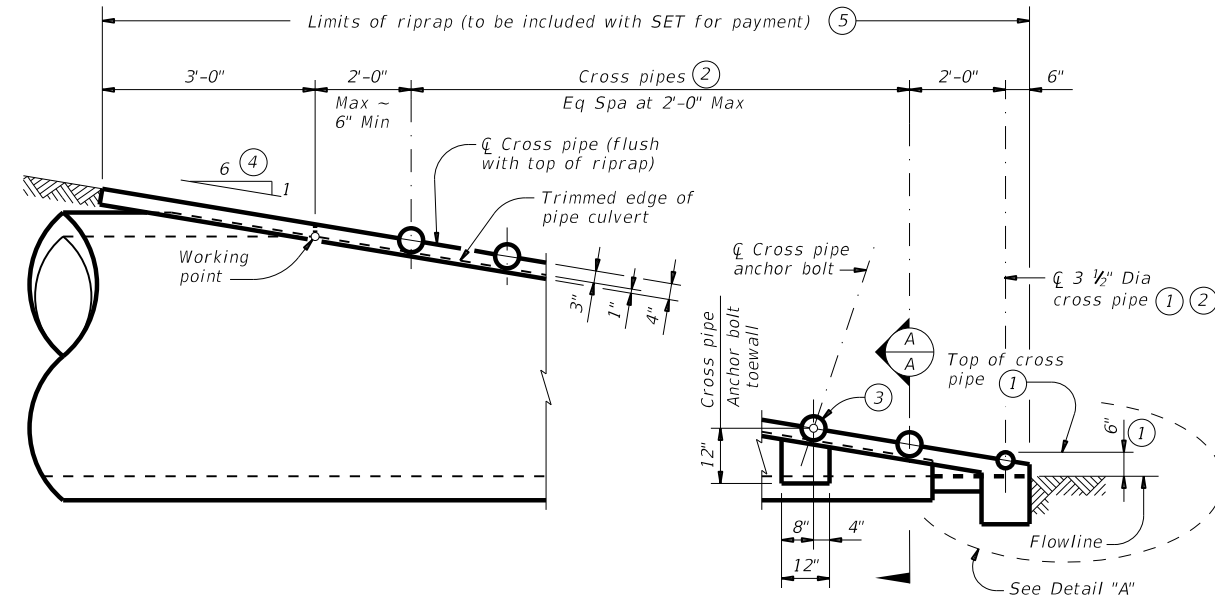
SAFETY END TREATMENT
 FOR 12" DIA TO 72" DIA
 PIPE CULVERTS
 TYPE II ~ PARALLEL DRAINAGE

SETP-PD

| | | | | |
|-----------------------|------------|-----------|-----------|---------|
| FILE: setppdse-20.dgn | DN: GAF | CK: CAT | DW: JRP | CK: GAF |
| ©TxDOT February 2020 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | SHEET NO. | | |
| AUS | WILLIAMSON | | | 64 |

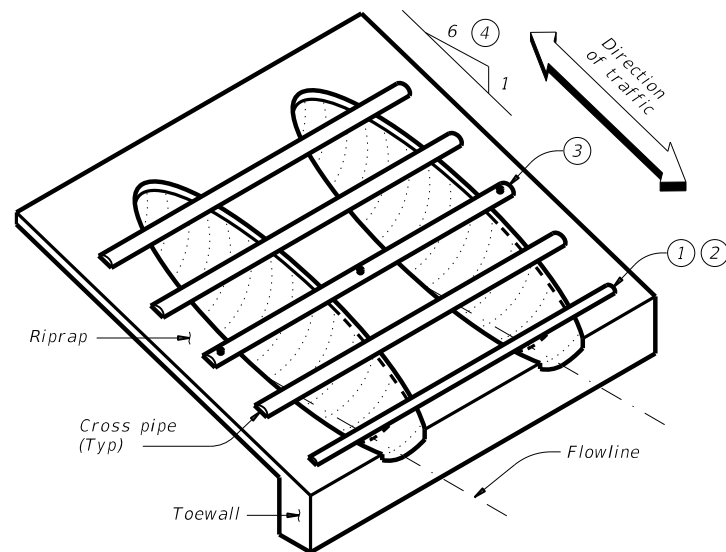
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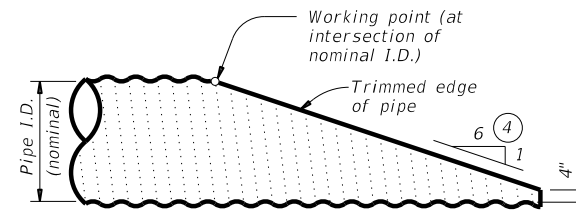


SIDE ELEVATION OF CAST-IN-PLACE CONCRETE

(Showing reinforced concrete pipe (RCP) culvert. Details of corrugated metal pipe (CMP) culvert are similar. pipe runners not shown for clarity.)



ISOMETRIC VIEW OF TYPICAL INSTALLATION



NOTE: All cross pipes, calculations, and dimensions are based on the pipe culverts mitered as shown in this detail. Alternate styles of mitered ends will require that appropriate adjustments be made to the values presented on this standard.

SIDE ELEVATION OF TYPICAL PIPE CULVERT MITER

(Showing corrugated metal pipe (CMP) culvert. Details of reinforced concrete pipe (RCP) culvert are similar.)

CROSS PIPE LENGTHS AND REQUIRED PIPE SIZES ②

| Corrugated Metal Pipe (CMP) Culverts | | | | | | | | | |
|--------------------------------------|--------------------|-------------------|-------------------|----------------------|--------------------|-------------------|----------|-----------------------------------|--------------------------|
| Design | Conc Riprap (CY) ⑥ | Pipe Culvert Span | Pipe Culvert Rise | Pipe Culvert Spa ~ G | Single Barrel ~ Q1 | Multi-Barrel ~ Q1 | Q2 | Conditions for Use of Cross Pipes | Cross Pipe Sizes |
| 1 | 0.6 | 17" | 13" | 1' - 0" | N/A | 2' - 8" | 2' - 5" | 3 or more pipe culverts | 3" Std (3.500" O.D.) |
| 2 | 0.7 | 21" | 15" | 1' - 2" | N/A | 3' - 1" | 2' - 11" | | 3 1/2" Std (4.000" O.D.) |
| 3 | 0.9 | 28" | 20" | 1' - 5" | N/A | 3' - 9" | 3' - 9" | | 4" Std (4.500" O.D.) |
| 4 | 1.0 | 35" | 24" | 1' - 8" | 4' - 4" | 4' - 6" | 4' - 7" | All pipe culverts | 4" Std (4.500" O.D.) |
| 5 | 1.2 | 42" | 29" | 1' - 11" | 4' - 11" | 5' - 2" | 5' - 5" | | |
| 6 | 1.4 | 49" | 33" | 2' - 2" | 5' - 6" | 5' - 11" | 6' - 3" | All pipe culverts | 5" Std (5.563" O.D.) |
| 7 | 1.6 | 57" | 38" | 2' - 5" | 6' - 2" | 6' - 8" | 7' - 2" | | |
| 8 | 1.8 | 64" | 43" | 2' - 10" | 6' - 9" | 7' - 6" | 8' - 2" | | |
| 9 | 1.9 | 71" | 47" | 3' - 2" | 7' - 4" | 8' - 3" | 9' - 1" | | |

| Reinforced Concrete Pipe (RCP) Culverts | | | | | | | | | |
|---|--------------------|-------------------|-------------------|----------------------|--------------------|-------------------|-------------|-----------------------------------|--------------------------|
| Design | Conc Riprap (CY) ⑥ | Pipe Culvert Span | Pipe Culvert Rise | Pipe Culvert Spa ~ G | Single Barrel ~ Q1 | Multi-Barrel ~ Q1 | Q2 | Conditions for Use of Cross Pipes | Cross Pipe Sizes |
| 1 | 0.6 | 22" | 13 1/2" | 1' - 0" | N/A | 3' - 1" | 2' - 10" | 3 or more pipe culverts | 3" Std (3.500" O.D.) |
| 2 | 0.7 | 26" | 15 1/2" | 1' - 2" | N/A | 3' - 6" | 3' - 4" | | 3 1/2" Std (4.000" O.D.) |
| 3 | 0.9 | 28 1/2" | 18" | 1' - 5" | N/A | 3' - 10" | 3' - 9 1/2" | | 4" Std (4.500" O.D.) |
| 4 | 1.0 | 36 1/4" | 22 1/2" | 1' - 8" | 4' - 5" | 4' - 7" | 4' - 8 1/4" | All pipe culverts | 4" Std (4.500" O.D.) |
| 5 | 1.2 | 43 3/4" | 26 3/8" | 1' - 11" | 5' - 1" | 5' - 4" | 5' - 6 3/4" | | |
| 6 | 1.4 | 51 1/8" | 31 5/16" | 2' - 2" | 5' - 8" | 6' - 1" | 6' - 5 1/4" | All pipe culverts | 5" Std (5.563" O.D.) |
| 7 | 1.6 | 58 1/2" | 36" | 2' - 5" | 6' - 4" | 6' - 10" | 7' - 3 1/2" | | |
| 8 | 1.8 | 65" | 40" | 2' - 10" | 6' - 10" | 7' - 7" | 8' - 3" | | |
| 9 | 1.9 | 73" | 45" | 3' - 2" | 7' - 6" | 8' - 5" | 9' - 3" | | |

- ① The proper installation of the first cross pipe is critical for vehicle safety. Place the top of the first cross pipe no more than 6" above the flow line.
- ② Provide cross pipes, except the first bottom pipe, of the size shown in the table. Provide a 3 #2" standard pipe (4" O.D.) for the first bottom pipe.
- ③ Install the third Cross Pipe from the bottom of the culvert using a bolted connection. Ensure that riprap concrete does not flow into the cross pipe so as to permit disassembly of the bolted connection to allow cleanout access. At the Contractor's option, install all other cross pipes using the bolted connection details.
- ④ Match cross slope as shown elsewhere in the plans. Cross slope of 6:1 or flatter is required for vehicle safety.
- ⑤ Riprap placed beyond the limits shown will be paid as concrete riprap in accordance with Item 432, "Riprap".
- ⑥ Quantities shown are for one end of one pipe culvert. For multiple Pipe Culverts, quantities will need to be adjusted. Riprap quantities are for Contractor's information only.

MATERIAL NOTES:

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.
 Provide cross pipes that meet the requirements of ASTM A53 (Type E or S, Gr B), ASTM A500 Gr B, or API 5LX52.
 Provide ASTM A307 bolts and nuts.
 Galvanize all steel components, except concrete reinforcing, after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

GENERAL NOTES:

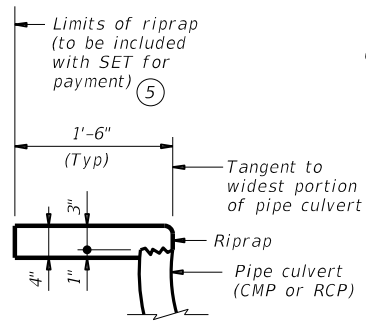
Pipe runners are designed for a traversing load of 10,000 pounds at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981.
 Safety end treatments (SET) shown herein are intended for use in those installations where out of control vehicles are likely to traverse the openings approximately perpendicular to the Pipe Runners.
 Construct concrete riprap and all necessary inverts in accordance with the requirements of Item 432, "Riprap".
 Payment for riprap and toewall is included in the price bid for each safety end treatment.

SHEET 1 OF 2

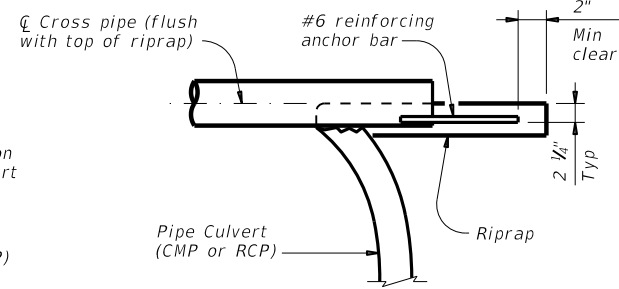
| | | | | | |
|--|------------|-----------|-----------|---------------------------------|--|
| | | | | Bridge Division Standard | |
| SAFETY END TREATMENT FOR DESIGN 1 TO 9 ARCH PIPE CULVERTS TYPE II ~ PARALLEL DRAINAGE | | | | | |
| SETP-PD-A | | | | | |
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| ©TxDOT February 2020 | CONT | SECT | JOB | HIGHWAY | |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | |
| DIST | COUNTY | | SHEET NO. | | |
| AUS | WILLIAMSON | | 65 | | |

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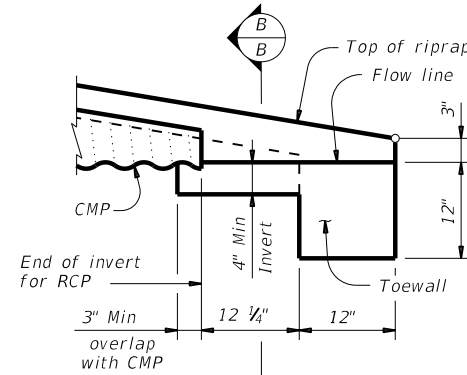
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SHOWING TYPICAL PIPE CULVERT AND RIPRAP

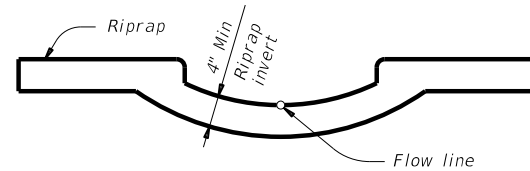


SHOWING CROSS PIPE WITH ANCHOR BAR



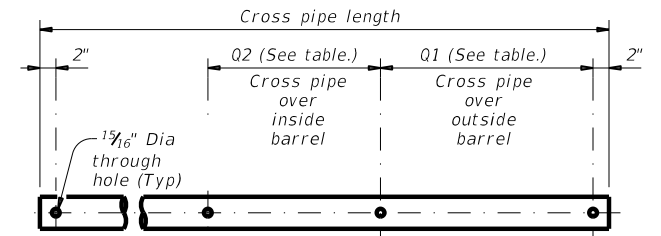
DETAIL "A"

(Showing invert with corrugated metal pipe (CMP) culvert. Reinforced concrete pipe (RCP) culvert details are similar. Cross pipes not shown for clarity.)

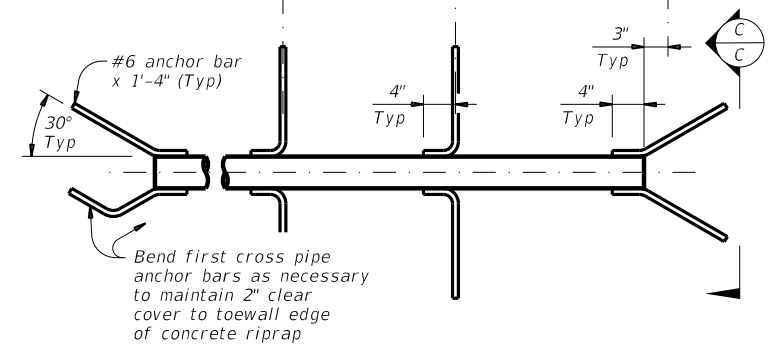


SECTION B-B

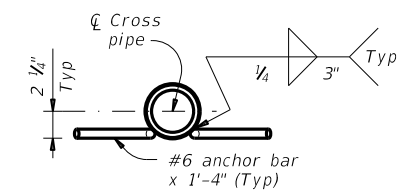
(Cross pipes not shown for clarity.)



PIPE WITH BOLTED ANCHOR

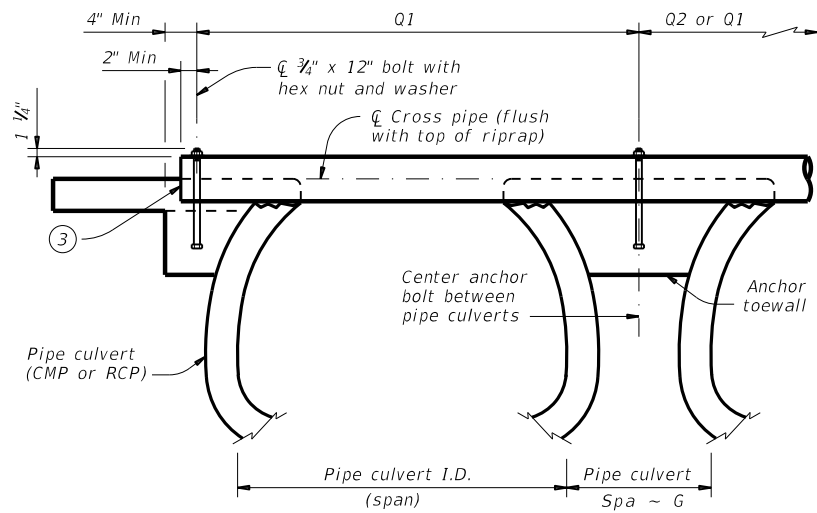


PIPE WITH ANCHOR BARS



SECTION C-C

CROSS PIPE DETAILS



SHOWING CROSS PIPE WITH BOLTED ANCHOR

SECTION A-A

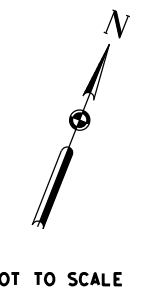
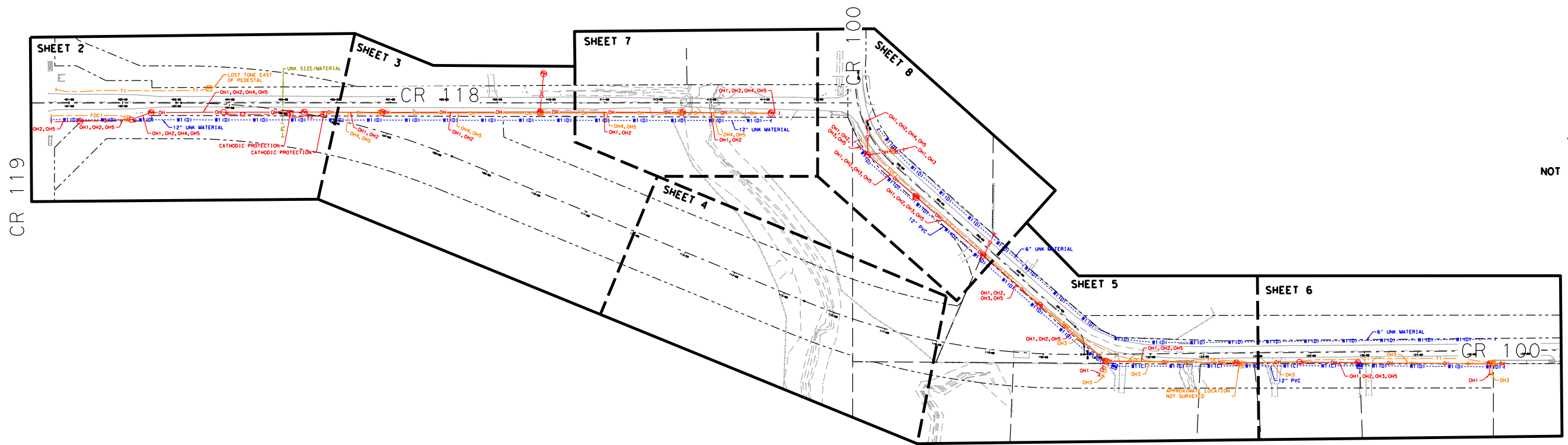
SHEET 2 OF 2



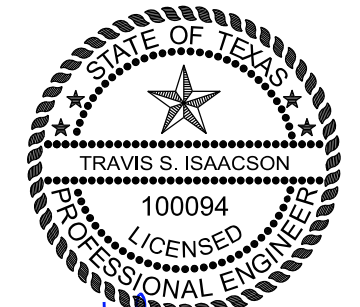
**SAFETY END TREATMENT
 FOR DESIGN 1 TO 9
 ARCH PIPE CULVERTS
 TYPE II ~ PARALLEL DRAINAGE**

SETP-PD-A

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| FILE: setppase-20.dgn | DN: GAF | CK: TxDOT | DW: JRP | CK: GAF |
| ©TxDOT February 2020 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | | SHEET NO. | |
| AUS | WILLIAMSON | | 66 | |



The Rios Group, Inc.
TBPE Firm # F-14595



Travis S. Isaacson
11-04-2022

SPECIAL NOTES

1. ALL PIPE SIZES WERE TAKEN FROM UTILITY RECORDS WHERE POSSIBLE. THE UTILITIES DEPICTED WERE INVESTIGATED BY THE RIOS GROUP, INC.. ALL OTHER PLAN INFORMATION, NOTABLY THE BACKGROUND INFORMATION, WAS PROVIDED BY OTHERS AND THE RIOS GROUP, INC. DISCLAIMS RESPONSIBILITY FOR ITS ACCURACY.
2. EXISTING SUBSURFACE UTILITY INVESTIGATIONS WERE COMPLETED ON 02/11/2022. THE RIOS GROUP, INC. EXPRESSLY DISCLAIMS ANY AND ALL RESPONSIBILITY FOR NEW UTILITY INSTALLATIONS, MODIFICATIONS, AND/OR ADJUSTMENTS TO EXISTING UTILITIES AFTER THE COMPLETION DATE.
3. UTILITY LOCATIONS ON THESE DRAWINGS ARE INTENDED FOR DESIGN PURPOSES AND NOT CONSTRUCTION. THEY REFLECT SUBSURFACE UTILITIES AT THE TIME OF FIELD INVESTIGATION. CALL TEXAS ONE CALL SYSTEM (800)245-4545 FOR UTILITY LOCATIONS 48 HOURS PRIOR TO ANY WORK.
4. WHERE POSSIBLE, WATER, GAS, AND COMMUNICATION SERVICE LINES WERE DESIGNATED. HOWEVER, SOME SERVICE LINES ARE CONSTRUCTED OF NON-CONDUCTIVE MATERIAL AND UTILITY COMPANY DRAWINGS MAY NOT SHOW SERVICE LINE LOCATIONS. THEREFORE ALL SERVICE LINES MAY NOT BE SHOWN.

QUALITY LEVELS

- Quality Level "D" - Information derived from existing records and/or oral collection.
- Quality Level "C" - Information obtained by surveying and plotting visible above ground utility features and by using professional judgment in correlating information to Quality Level "D" information.
- Quality Level "B" - Designate: Two-dimensional horizontal mapping. This information is obtained through the application and interpretation of appropriate non-destructive surface geophysical methods. Utility indications are referenced to established survey control. Incorporates Quality Levels "C" and "D" information to produce Quality Level "B" information.
- Quality Level "A" - Locate: Precise horizontal and vertical location of utilities obtained by the actual exposure and subsequent measurement of subsurface utilities at a specific point. Diameters shown are verified visually and may not be exact.

MATERIAL ABBREVIATIONS

- STL - STEEL
- PE - POLYETHYLENE
- AC - TRANSITE
- CI - CAST IRON
- DI - DUCTILE IRON
- PVC - POLYVINYL CHLORIDE
- DBC - DIRECT BURIED CABLE
- RCP - REINFORCED CONCRETE PIPE
- VC - VITRIFIED CLAY
- FG - FIBERGLASS
- CSC - CONCRETE/STEEL CYLINDER
- CMP - CORRUGATED METAL PIPE
- CONC - CONCRETE
- CLAY - CLAY
- UNK - UNKNOWN

90% PLANS



CR 118 AT COTTONWOOD CREEK

S.U.E. PLAN SHEET

INDEX LAYOUT

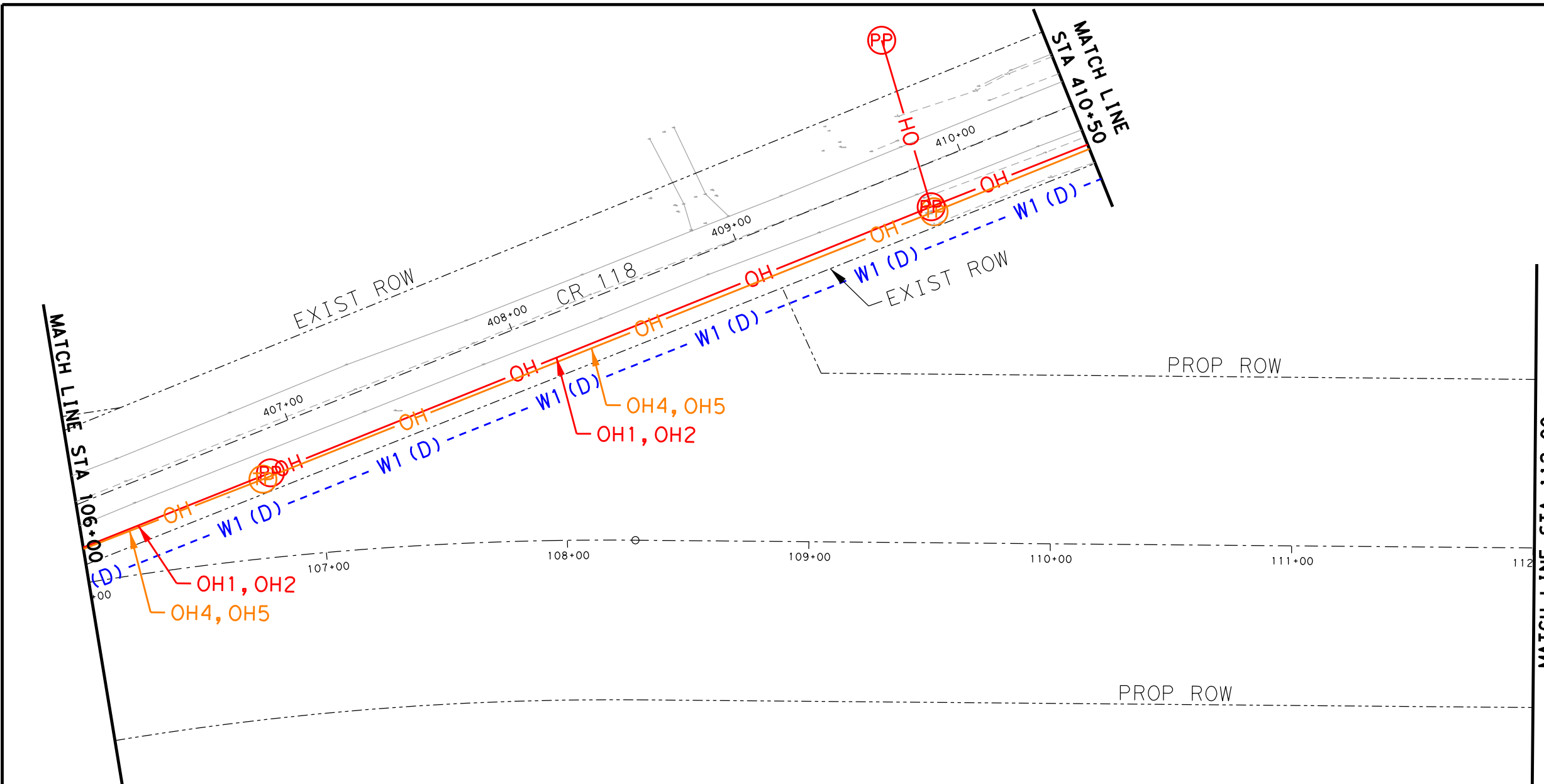
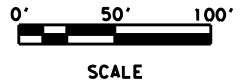
SHEET 01 OF 08

| FED. RD. DIV. NO. | FEDERAL AID PROJECT NO. | HIGHWAY NO. |
|-------------------|-------------------------|-------------|
| 6 | BR 2020 (733) | CR 118 |
| STATE | DIST. | COUNTY |
| TEXAS | AUS | WILLIAMSON |
| CONT. | SECT. | JOB |
| 0914 | 05 | 204, 220 |

Subsurface Utility Engineering (SUE) Certification

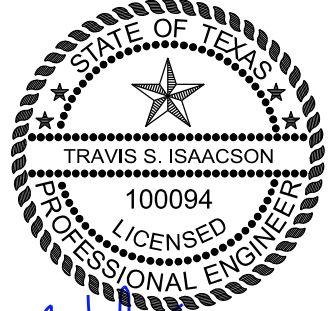
The engineer's seal hereon is to certify that the utilities shown have been investigated in accordance with standard SUE industry practices. Where indicated utility sizes and materials taken from best available records. All other information hereon has been provided by others and is not a part of this certification.

FILENAME: sssssssssssssssssssssssssssssssssssssss
 PLOTTED: sssDATESS sssSYTIMESS



MATCH LINE STA 112+00

The Rios Group, Inc.
TBPE Firm # F-14595



Travis S. Isaacson
11-04-2022

90% PLANS

LEGEND OF UTILITY TYPES

ABANDONED UTILITY —X—X—X—X—
 PROPOSED UTILITY ————
 UNKNOWN UTILITY - - - - -

COMMUNICATIONS QL "B"

LUMEN (TELE) ——— T1 ———
 UNITE PRIVATE NETWORK (FO/DUCT) ——— FOC1 ———

ELECTRIC / POWER QL "B"

ONCOR ——— E1 ———
 PRIVATE ——— E2 ———

GAS / PETROLEUM QL "B"

ATMOS ——— PL1 ———

POTABLE WATER QL "C"

JONAH WATER - - - - - W1 (C) - - - - -
 QL "D" JONAH WATER - - - - - W1 (D) - - - - -

OVERHEAD UTILITY QL "C" ——— OH ———

OVERHEAD UTILITY QL "C" ——— OH ———

OH1 - ONCOR
 OH2 - FIBERLIGHT
 OH3 - LUMEN (TELE)
 OH4 - UNITE PRIVATE NETWORK (FOC)
 OH5 - SPECTRUM (FOC)

LEGEND OF UTILITY SYMBOLS

END CAP []
 QUALITY LEVEL CHANGE []
 TEST HOLE []
 UTILITY CONTINUATION []

FIBER HANDHOLE []
 TELEPHONE MANHOLE []
 TELEPHONE PEDESTAL []
 TELEPHONE POLE []
 TELEPHONE POLE W/RISER []
 ELECTRIC HANDHOLE []
 ELECTRIC POLE (POWER) []
 ELECTRIC POLE W/RISER []
 WATER METER []

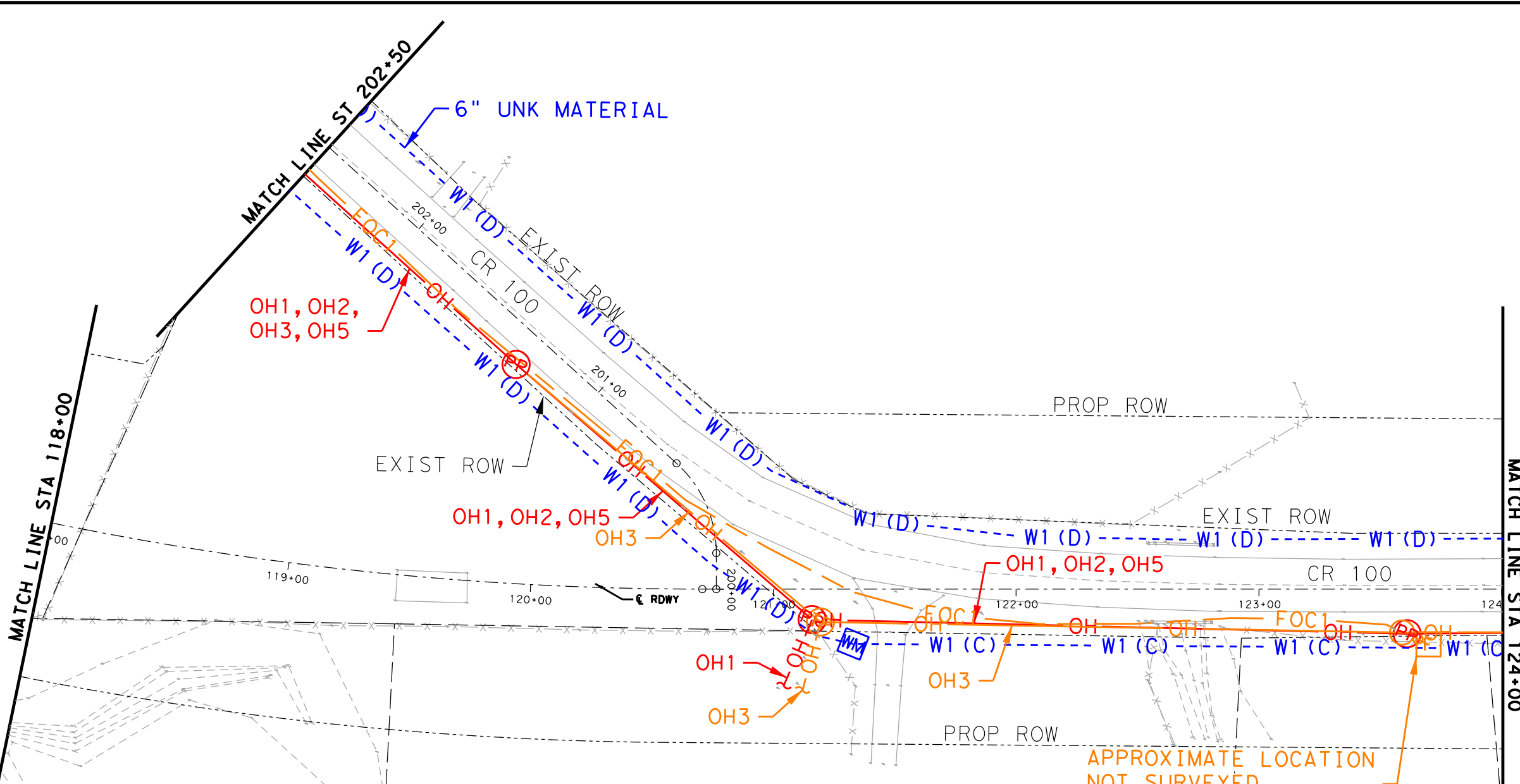
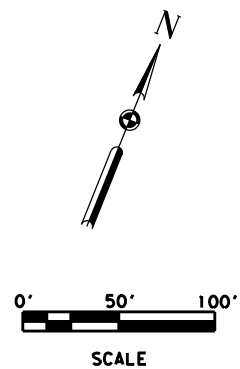
Subsurface Utility Engineering (SUE) Certification

The engineer's seal hereon is to certify that the utilities shown have been investigated in accordance with standard SUE industry practices. Where indicated utility sizes and materials taken from best available records. All other information hereon has been provided by others and is not a part of this certification.

| | | |
|-------------------|-------------------------|-------------|
| FED. RD. DIV. NO. | FEDERAL AID PROJECT NO. | HIGHWAY NO. |
| 6 | BR 2020(733) | CR 118 |
| STATE | DIST. | COUNTY |
| TEXAS | AUS | WILLIAMSON |
| CONT. | SECT. | JOB |
| 0914 | 05 | 204,220 |

SHEET NO. 69

FILENAME: sss
 PLOTTED: sssDATEss sssSYTIMEsss



The Rios Group, Inc.
TBPE Firm # F-14595



Travis S. Isaacson
11-04-2022

90% PLANS

LEGEND OF UTILITY TYPES

| | |
|---------------------------------|-------------------------------|
| ABANDONED UTILITY | — X — X — X — X — |
| PROPOSED UTILITY | ———— |
| UNKNOWN UTILITY | - - - - - |
| COMMUNICATIONS | |
| LUMEN (TELE) | QL "B" — — — — T1 — — — — |
| UNITE PRIVATE NETWORK (FO/DUCT) | — — — — FOC1 — — — — |
| ELECTRIC / POWER | |
| ONCOR PRIVATE | QL "B" — — — — E1 — — — — |
| | — — — — E2 — — — — |
| GAS / PETROLEUM | |
| ATMOS | QL "B" — — — — PL1 — — — — |
| POTABLE WATER | |
| JONAH WATER | QL "C" — — — — W1 (C) — — — — |
| JONAH WATER | QL "D" — — — — W1 (D) — — — — |
| OVERHEAD UTILITY | |

OVERHEAD UTILITY

| | |
|-----------------------------------|---------------------------|
| OH1 - ONCOR | QL "C" — — — — OH — — — — |
| OH2 - FIBERLIGHT | |
| OH3 - LUMEN (TELE) | |
| OH4 - UNITE PRIVATE NETWORK (FOC) | |
| OH5 - SPECTRUM (FOC) | |

LEGEND OF UTILITY SYMBOLS

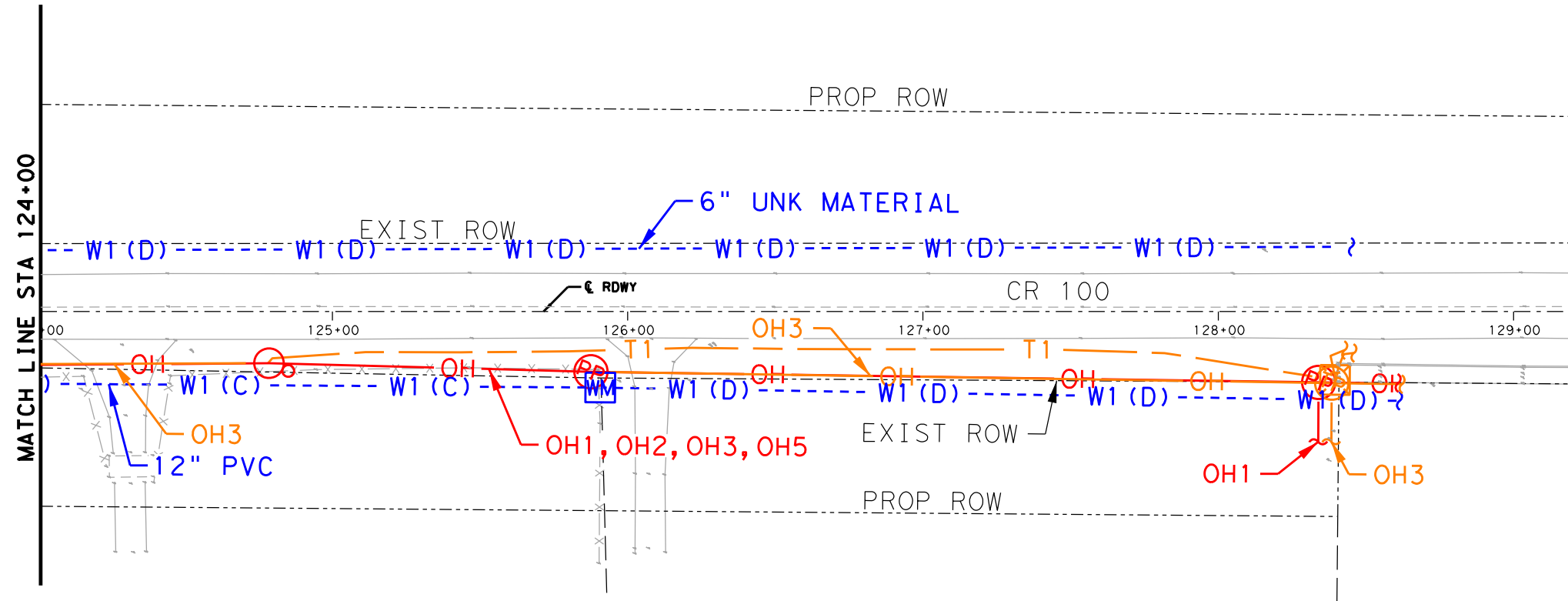
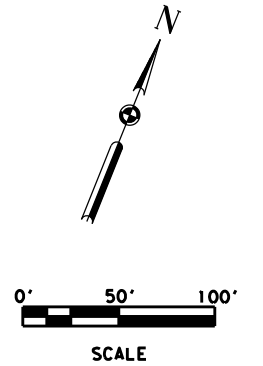
| | | |
|------------------------|----|----|
| END CAP | [| |
| QUALITY LEVEL CHANGE | ↓ | |
| TEST HOLE | ⊙ | |
| UTILITY CONTINUATION | ? | |
| FIBER HANDHOLE | ⊙ | F |
| TELEPHONE MANHOLE | ⊙ | T |
| TELEPHONE PEDESTAL | ⊙ | P |
| TELEPHONE POLE | ⊙ | ⊙ |
| TELEPHONE POLE W/RISER | ⊙ | ⊙ |
| ELECTRIC HANDHOLE | ⊙ | E |
| ELECTRIC POLE (POWER) | ⊙ | PP |
| ELECTRIC POLE W/RISER | ⊙ | PP |
| WATER METER | WM | |

Subsurface Utility Engineering (SUE) Certification

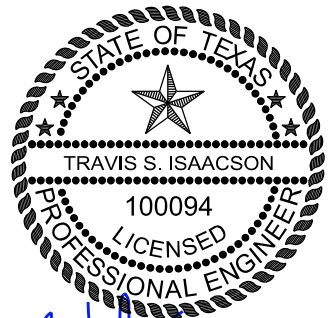
The engineer's seal hereon is to certify that the utilities shown have been investigated in accordance with standard SUE industry practices. Where indicated utility sizes and materials taken from best available records. All other information hereon has been provided by others and is not a part of this certification.

FILENAME: sssssssssssssssssssssssssssssssssssssss
PLOTTER: sssDATESSS sssSYTIMESSS

| | | |
|---|-------------------------|-----------------|
| | | |
| | | |
| CR 118 AT COTTONWOOD CREEK | | |
| S.U.E. PLAN SHEET | | |
| ALIGNMENT STA 118+00 TO ALIGNMENT STA 124+00 | | |
| SHEET 05 OF 08 | | |
| FED. RD. DIST. NO. | FEDERAL AID PROJECT NO. | HIGHWAY NO. |
| 6 | BR 2020(733) | CR 118 |
| STATE | DIST. | COUNTY |
| TEXAS | AUS | WILLIAMSON |
| CONT. | SECT. | JOB |
| 0914 | 05 | 204, 220 |
| | | SHEET NO. 71 |



The Rios Group, Inc.
 TBPE Firm # F-14595



Travis Isaacson
 11-04-2022

90% PLANS



CR 118 AT COTTONWOOD CREEK

S.U.E. PLAN SHEET

ALIGNMENT STA 124+00 TO
 END OF PROJECT

SHEET 06 OF 08

| FED. RD. DIV. NO. | FEDERAL AID PROJECT NO. | HIGHWAY NO. | |
|-------------------|-------------------------|-------------|--------------|
| 6 | BR 2020(733) | CR 118 | |
| STATE | DIST. | COUNTY | |
| TEXAS | AUS | WILLIAMSON | |
| CONT. | SECT. | JOB | |
| 0914 | 05 | 204, 220 | |
| | | | SHEET NO. 72 |

LEGEND OF UTILITY TYPES

| | |
|---------------------------------|-----------------------|
| ABANDONED UTILITY | ---X---X---X---X--- |
| PROPOSED UTILITY | ----- |
| UNKNOWN UTILITY | ----- |
| COMMUNICATIONS | |
| LUMEN (TELE) | QL "B" --- T1 --- |
| UNITE PRIVATE NETWORK (FO/DUCT) | --- FOC1 --- |
| ELECTRIC / POWER | |
| ONCOR | QL "B" --- E1 --- |
| PRIVATE | --- E2 --- |
| GAS / PETROLEUM | |
| ATMOS | QL "B" --- PL1 --- |
| POTABLE WATER | |
| JONAH WATER | QL "C" --- W1 (C) --- |
| JONAH WATER | QL "D" --- W1 (D) --- |
| OVERHEAD UTILITY | |

OVERHEAD UTILITY

| | |
|-----------------------------------|-------------------|
| OH1 - ONCOR | QL "C" --- OH --- |
| OH2 - FIBERLIGHT | |
| OH3 - LUMEN (TELE) | |
| OH4 - UNITE PRIVATE NETWORK (FOC) | |
| OH5 - SPECTRUM (FOC) | |

LEGEND OF UTILITY SYMBOLS

| | |
|------------------------|------|
| END CAP | [|
| QUALITY LEVEL CHANGE | ⌋ |
| TEST HOLE | ⊗ |
| UTILITY CONTINUATION | ? |
| FIBER HANDHOLE | (F) |
| TELEPHONE MANHOLE | (I) |
| TELEPHONE PEDESTAL | (TP) |
| TELEPHONE POLE | (E) |
| TELEPHONE POLE W/RISER | (PP) |
| ELECTRIC HANDHOLE | (WM) |
| ELECTRIC POLE (POWER) | |
| ELECTRIC POLE W/RISER | |
| WATER METER | |

Subsurface Utility Engineering (SUE) Certification

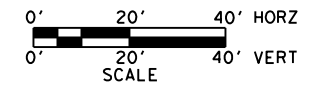
The engineer's seal hereon is to certify that the utilities shown have been investigated in accordance with standard SUE industry practices. Where indicated utility sizes and materials taken from best available records. All other information hereon has been provided by others and is not a part of this certification.

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 PLOTTED: sssDATEsss sssSYTIMEsss

FILENAME: pw:\kh-pw-bent\kim-horn\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\7. Bridge\CR118\CR118_BRC_LAYOUT
 PLOTTED: 11/22/2022 11:00:37 AM

HORIZONTAL CURVE DATA

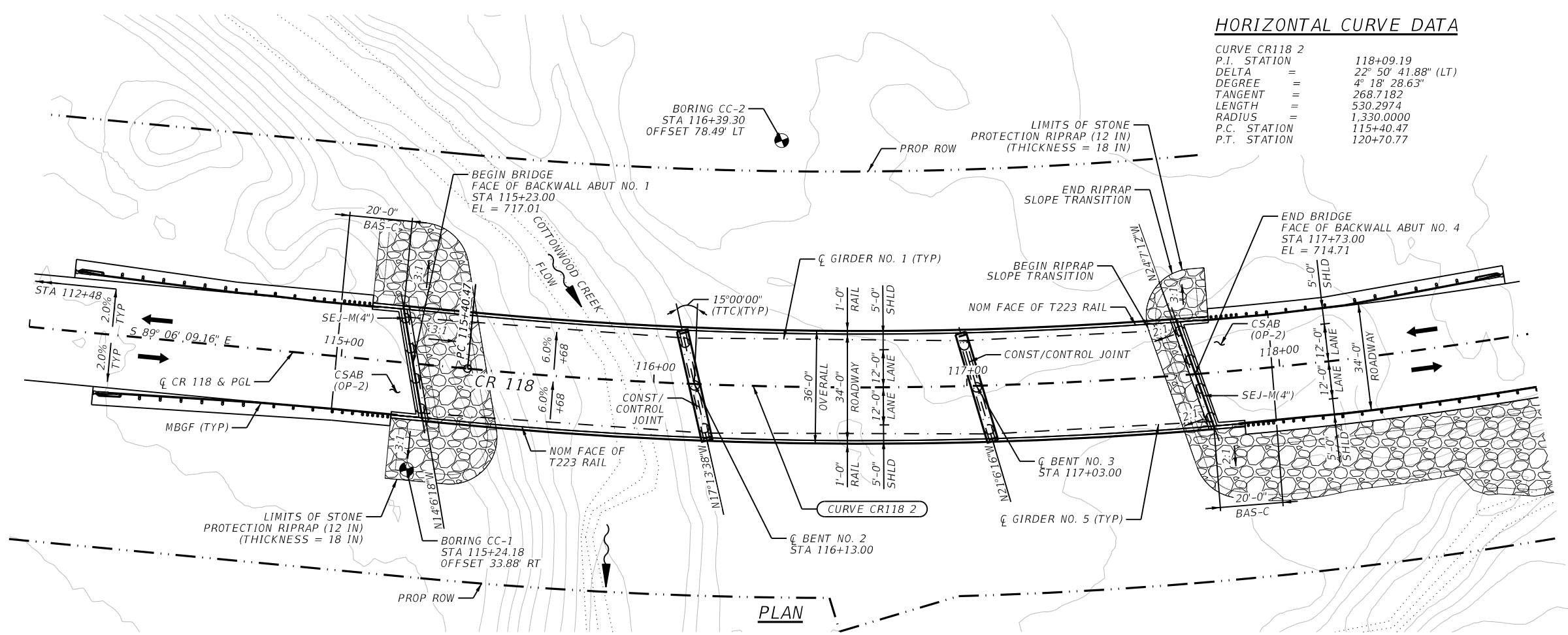
| | |
|---------------|---------------------|
| CURVE CR118 2 | |
| P.I. STATION | 118+09.19 |
| DELTA | 22° 50' 41.88" (LT) |
| DEGREE | 4° 18' 28.63" |
| TANGENT | 268.7182 |
| LENGTH | 530.2974 |
| RADIUS | 1,330.0000 |
| P.C. STATION | 115+40.47 |
| P.T. STATION | 120+70.77 |



NOTES:

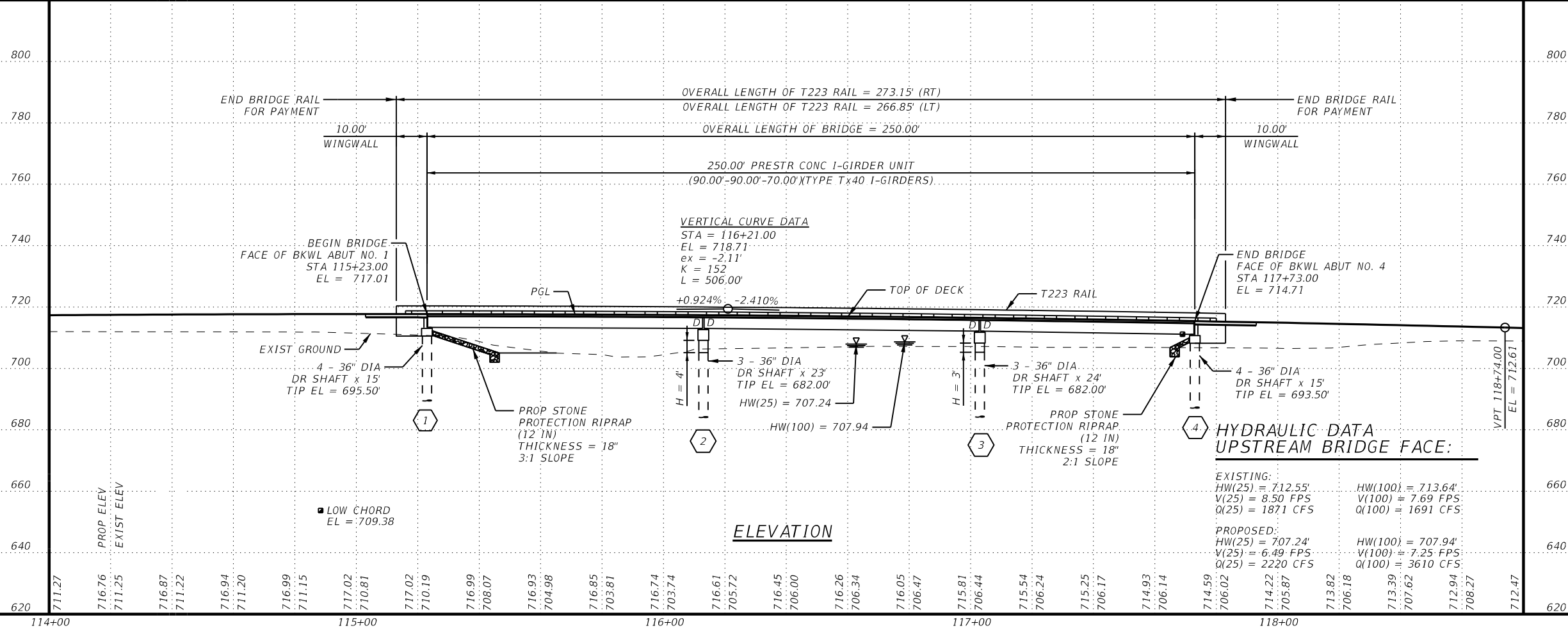
- DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION (2020) AND CURRENT INTERIMS.
- ALL DIMENSIONS ARE HORIZONTAL AND MUST BE CORRECTED FOR GRADE AND CROSS SLOPE.
- THE "H" VALUES SHOWN ARE ESTIMATED COLUMN HEIGHTS AND ARE FOR BIDDING PURPOSES ONLY. THE CONTRACTOR IS RESPONSIBLE FOR CALCULATING THE ACTUAL COLUMN HEIGHTS BASED ON FIELD CONDITIONS.
- PROPOSED CR 118 BRIDGE AT COTTONWOOD CREEK IS ALONG NEW ALIGNMENT. EXISTING BRIDGE IS NORTH OF PROPOSED BRIDGE.
- BEARING CONDITIONS:
D = DOWEL; BLANK = NO DOWEL
- SEE BRIDGE BORING LOGS SHEET FOR GEOTECHNICAL INFORMATION.
- ALL ABUTMENTS AND BENTS SHALL BE SKEWED 15°0'0" TANGENT TO CURVE (TTC).

| |
|------------------------------------|
| DESIGN SPEED = 60 MPH |
| ADT (2023) = 600 VPD |
| ADT (2043) = 800 VPD |
| FUNCT CLASS = RURAL MINOR ARTERIAL |
| EXIST NBI NO: 14-246-0-AA04-94-001 |
| PROP NBI NO: 14-246-0-AA04-94-501 |



PLAN

HL-93 LOADING



ELEVATION



Kimley»Horn F-928

Texas Department of Transportation

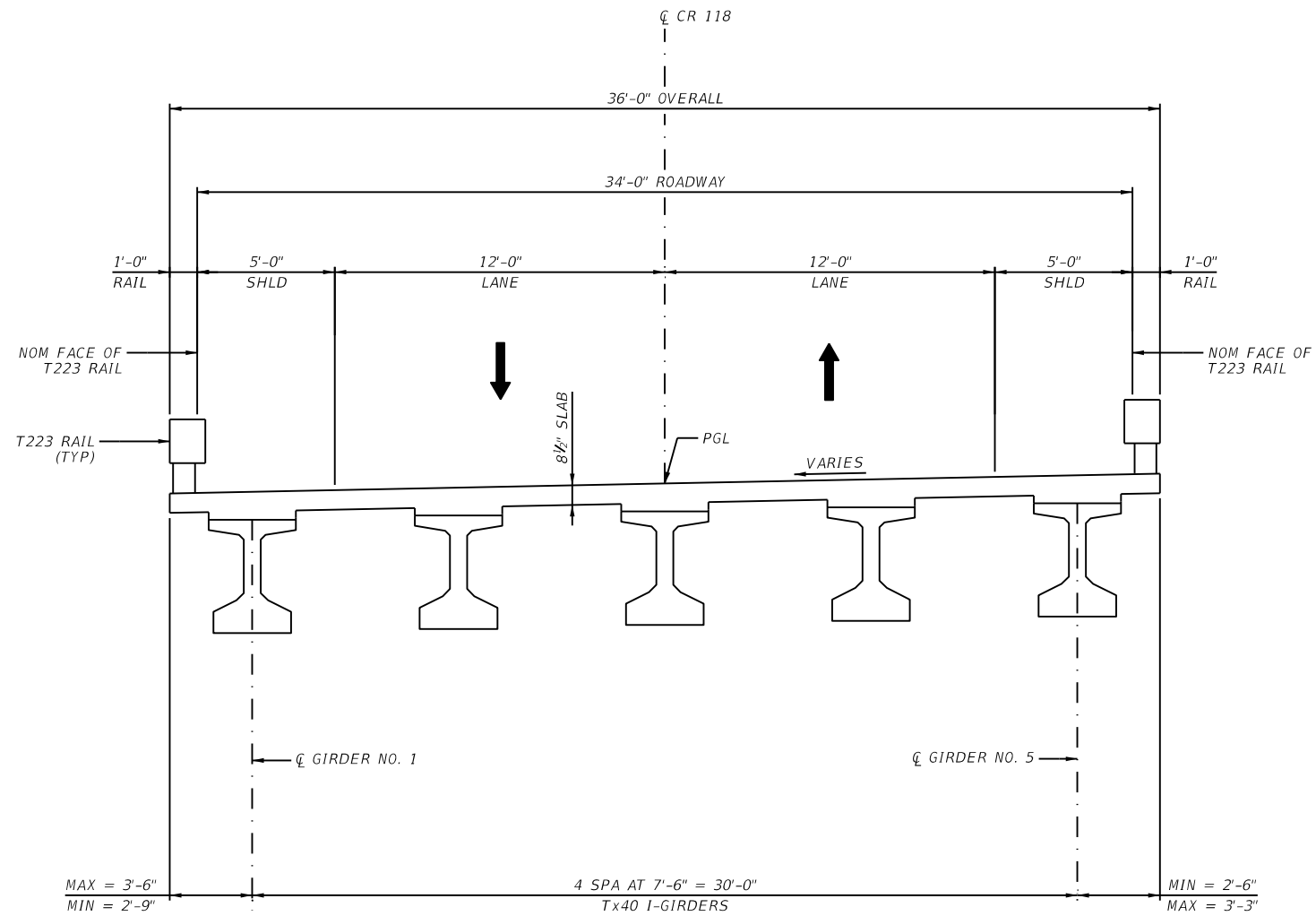
CR 118 AT COTTONWOOD CREEK

BRIDGE LAYOUT

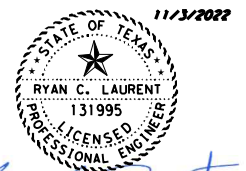
| | | | | | | | |
|------------------|--|-------------------|--|-------------------|--|-------------------|--|
| EXISTING: | | HW(25) = 712.55' | | HW(100) = 713.64' | | | |
| V(25) = 8.50 FPS | | V(100) = 7.69 FPS | | Q(25) = 1871 CFS | | Q(100) = 1691 CFS | |
| PROPOSED: | | HW(25) = 707.24' | | HW(100) = 707.94' | | | |
| V(25) = 6.49 FPS | | V(100) = 7.25 FPS | | Q(25) = 2220 CFS | | Q(100) = 3610 CFS | |

| | | | | |
|--------------|------|------------|-----------|--|
| SHEET 1 OF 1 | | | | |
| CONT | SECT | JOB | HIGHWAY | |
| 0914 | 05 | 204, ETC. | CR 118 | |
| DIST | | COUNTY | SHEET NO. | |
| AUS | | WILLIAMSON | 75 | |

FILENAME: p:\kh-pw\ben\kimley.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\7. Bridge\CR118\CR118_BRG_TYP.dgn
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PROPOSED BRIDGE TYPICAL SECTION



Ryan C. Laurent

Kimley»Horn F-928




CR 118 AT COTTONWOOD CREEK
 BRIDGE TYPICAL SECTION

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 76 |

FILENAME: pw:\kh-pw-bent\ley.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\7. Bridge\CR118\CR118_BRC.OTY
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| SUMMARY OF ESTIMATED QUANTITIES - CR 118 AT COTTONWOOD CREEK | | | | | | | | | | | | |
|--|----------------------|-----------------|---------------------|------------------|-----------------|--------------------|-----------------|---------------|---------------------------|-----------------------------------|----------------|---|
| CSJ: 0914-05-204 | ITEM NO. | 0400 6005 | 0416 6004 | 0420 6013 | 0420 6029 | 0420 6037 | 0422 6001 | 0422 6015 | 0425 6037 | 0432 6031 | 0450 6006 | 0454 6018 |
| BRIDGE ELEMENT | BID ITEM DESCRIPTION | CEM STABIL BKFL | DRILL SHAFT (36 IN) | CL C CONC (ABUT) | CL C CONC (CAP) | CL C CONC (COLUMN) | REINF CONC SLAB | APPROACH SLAB | PRESTR CONC GIRDER (TX40) | RIPRAP (STONE PROTECTION) (12 IN) | RAIL (TY T223) | SEALED EXPANSION JOINT (4 IN) (SEJ - M) |
| | | CY | LF | CY | CY | CY | SF | CY | LF | CY | LF | LF |
| NBI#: 14-246-0-AA04-94-501 | | 141 | 120 | 47.4 | | | | 67 | | 459 | 40.0 | 73 |
| 2 - ABUTMENTS | | | 141 | | 32.0 | 5.6 | | | | | | |
| 2 - BENTS | | | | | | | 9,000 | | 1,236.70 | | 500.0 | |
| 1 - 250.00' PRESTR CONC TX40 I-GIRDER UNIT | | | | | | | | | | | | |
| | TOTAL | 141 | 261 | 47.4 | 32.0 | 5.6 | 9,000 | 67 | 1,236.70 | 459 | 540.0 | 73 |

11/3/2022

 RYAN C. LAURENT
 131995
 LICENSED PROFESSIONAL ENGINEER
Ryan C. Laurent

Kimley»Horn F-928

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 Texas Department of Transportation

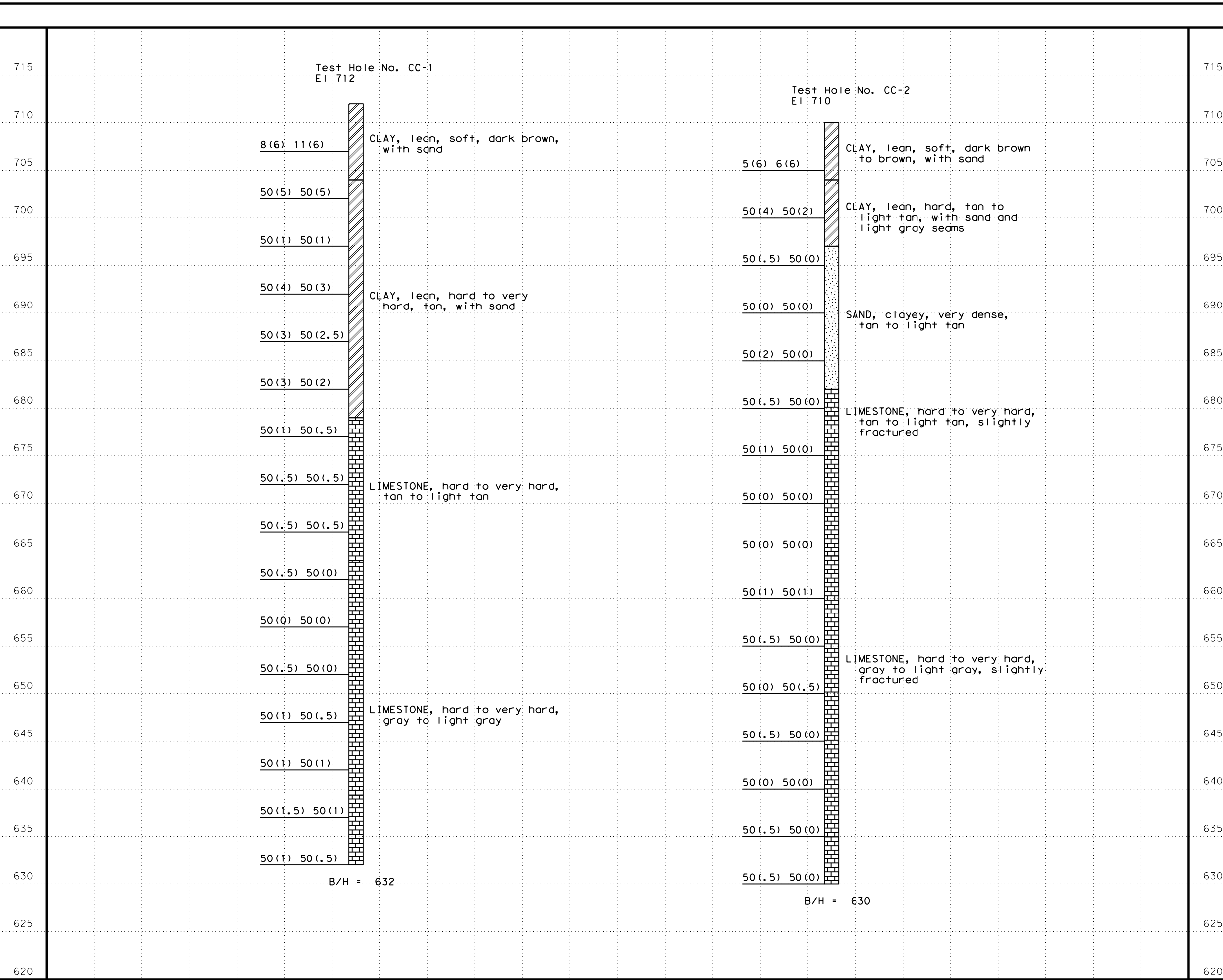
CR 118 AT COTTONWOOD CREEK

ESTIMATED QUANTITIES

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 77 |

DATE F:\118\118\2022\knh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData4 - DesignPlan Set7. Bridge\CR118\CR118_GEN_BOR01.dgn
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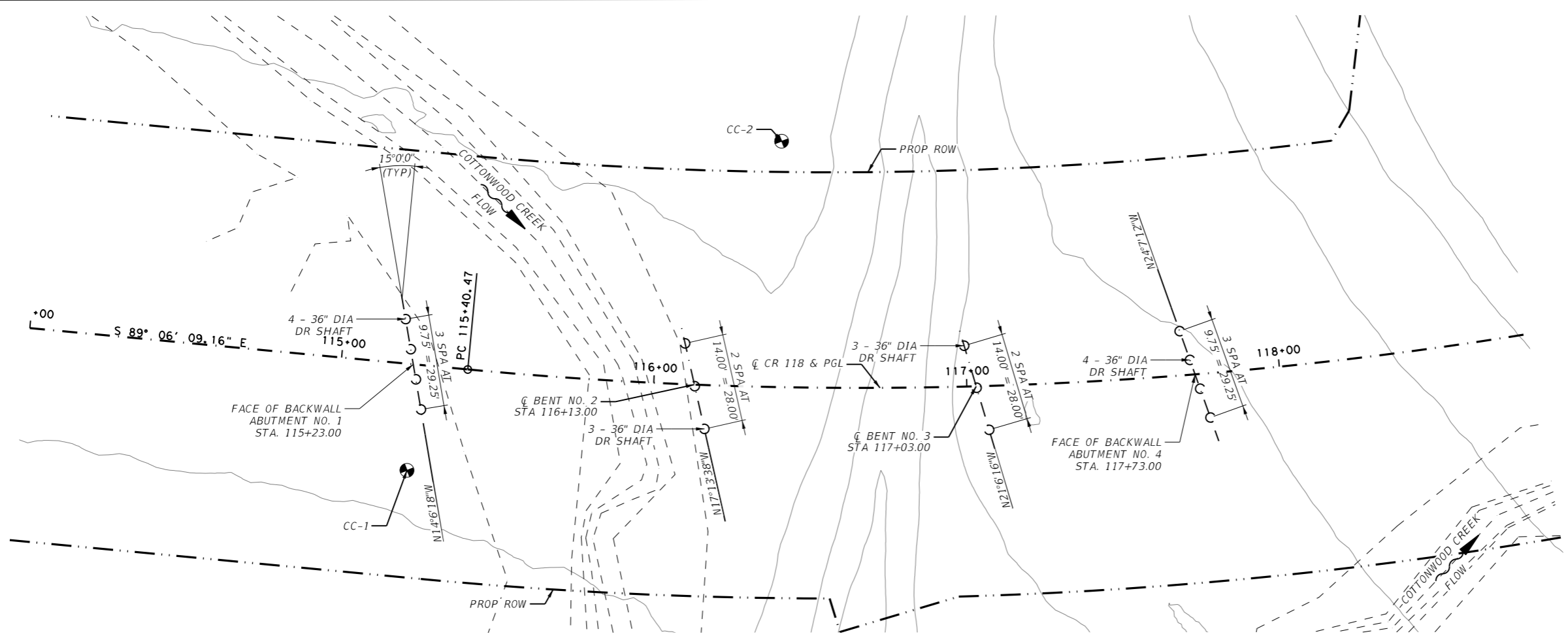
CR 118 AT COTTONWOOD CREEK

SOIL BORE LOG

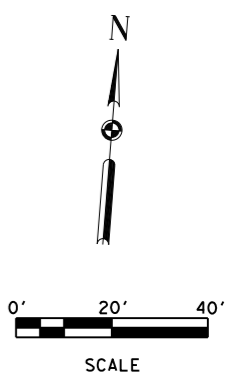
SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 78 |

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FOUNDATION LAYOUT



11/3/2022
 STATE OF TEXAS
 RYAN C. LAURENT
 131995
 PROFESSIONAL ENGINEER
Ryan C. Laurent

Kimley»Horn F-928

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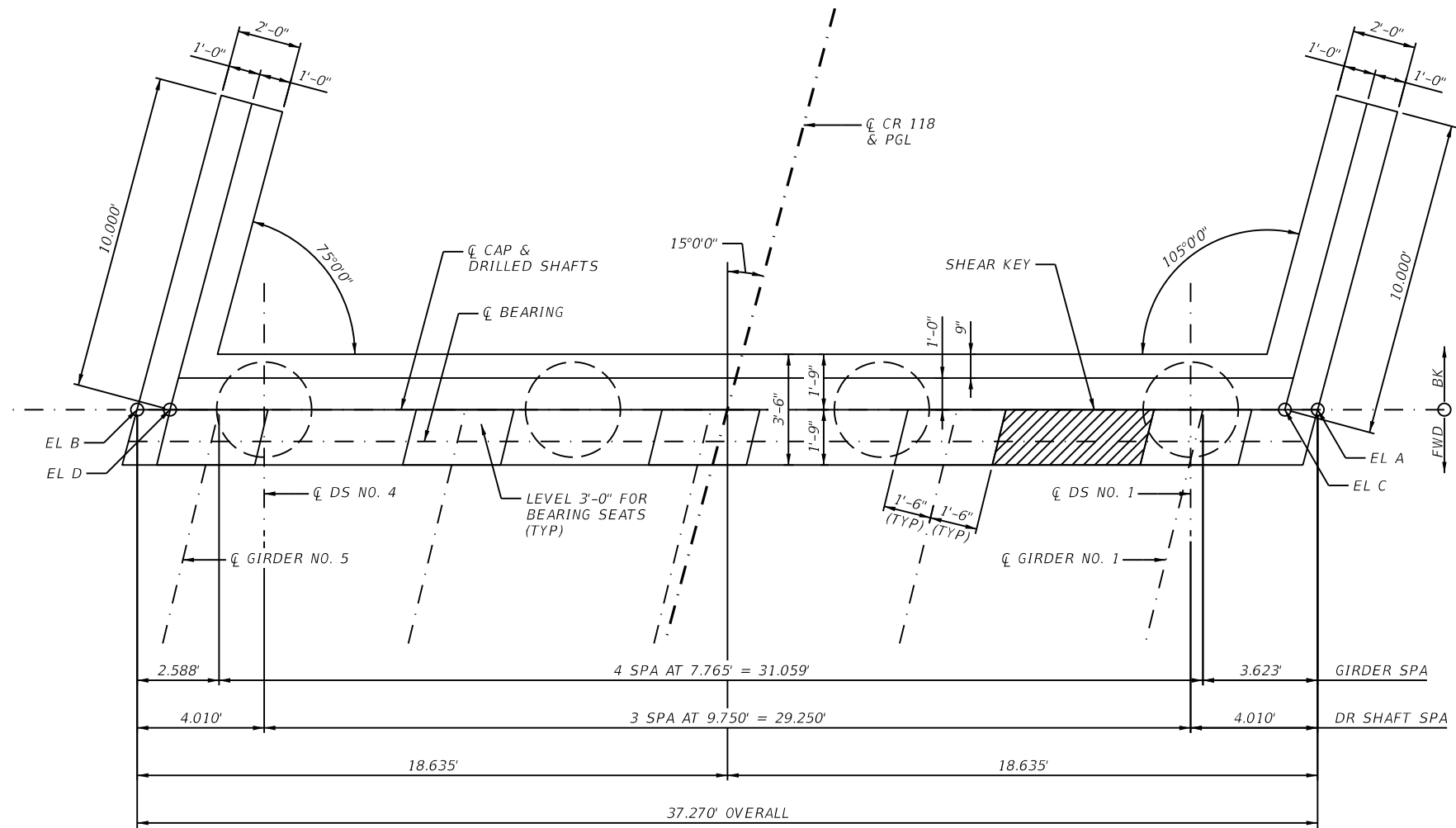
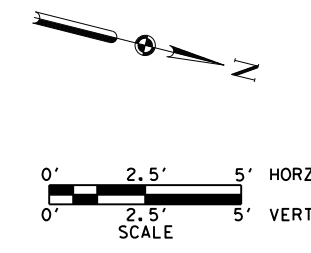
 Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
 FOUNDATION LAYOUT

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 79 |

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PLAN

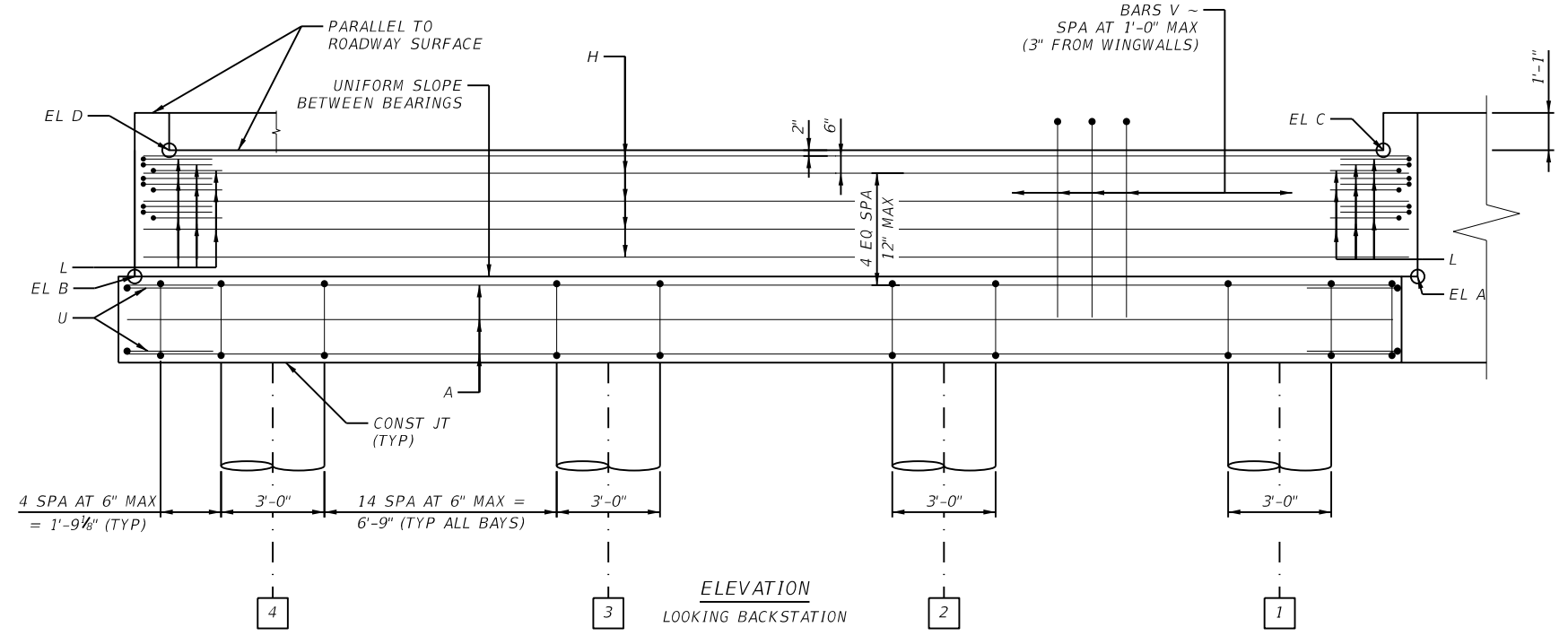
- NOTES:**
- DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION (2020) AND INTERIM REVISIONS THERETO.
 - SEE BRIDGE LAYOUT FOR HEADER SLOPE AND FOUNDATION TYPE, SIZE AND LENGTH.
 - SEE COMMON FOUNDATION DETAILS (FD) STANDARD FOR ALL FOUNDATION DETAILS AND NOTES.
 - SEE T223 RAIL DETAILS FOR RAIL ANCHORAGE IN WINGWALL.
 - SEE SHEAR KEY (IGSK) STANDARD FOR ALL SHEAR KEY DETAILS AND NOTES.
 - SEE ABUTMENT DETAILS SHEET FOR WINGWALL DETAILS, BEARING SEAT DETAILS, CONTROL ELEVATIONS, AND BAR DETAILS.

COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.
REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR.

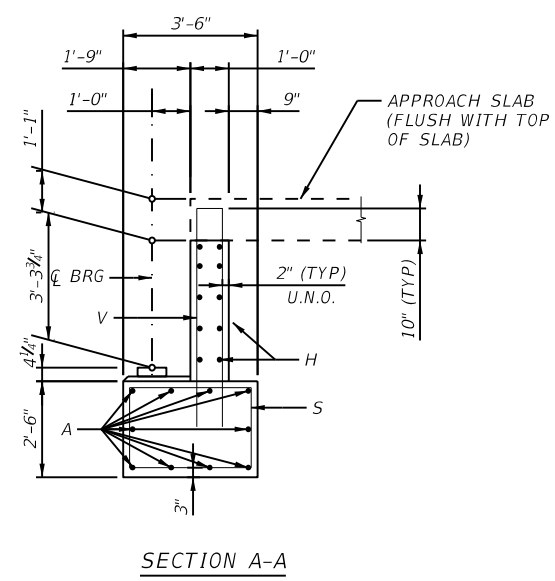
MATERIAL NOTES:
PROVIDE CLASS C CONCRETE, $f'_c = 3,600$ PSI
PROVIDE GRADE 60 REINFORCING STEEL.
CALCULATED FOUNDATION LOAD:
103 TONS/SHAFT



Ryan C. Laurent



ELEVATION
LOOKING BACKSTATION



SECTION A-A

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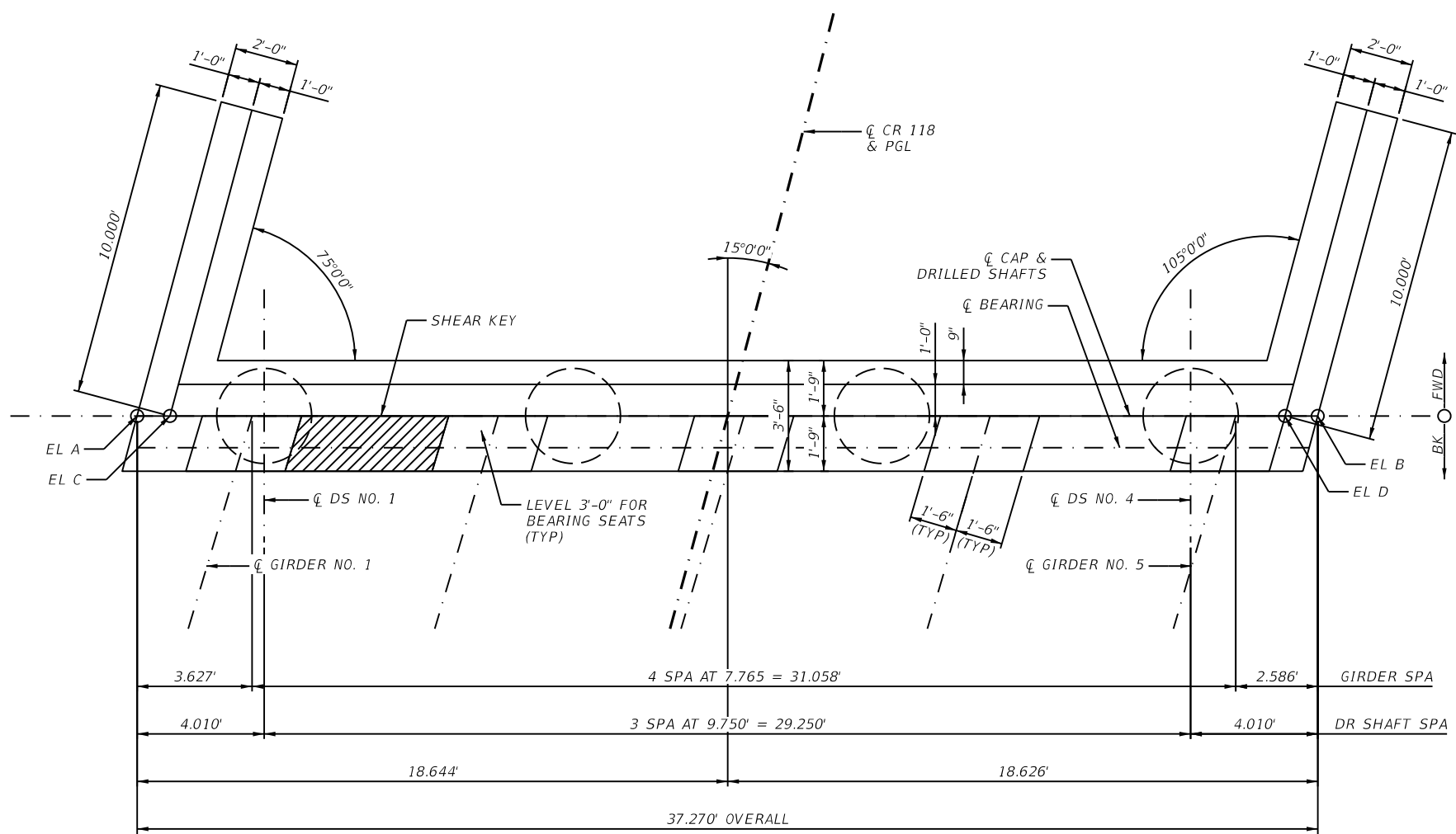
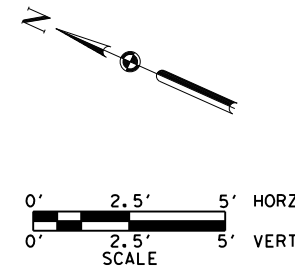
CR 118 AT COTTONWOOD CREEK

ABUTMENT NO. 1

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 80 |

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PLAN

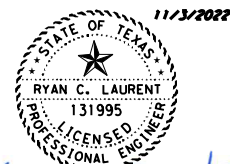
NOTES:

- DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION (2020) AND INTERIM REVISIONS THERETO.
- SEE BRIDGE LAYOUT FOR HEADER SLOPE AND FOUNDATION TYPE, SIZE AND LENGTH.
- SEE COMMON FOUNDATION DETAILS (FD) STANDARD FOR ALL FOUNDATION DETAILS AND NOTES.
- SEE T223 RAIL DETAILS FOR RAIL ANCHORAGE IN WINGWALL.
- SEE SHEAR KEY (IGSK) STANDARD FOR ALL SHEAR KEY DETAILS AND NOTES.
- SEE ABUTMENT DETAILS SHEET FOR WINGWALL DETAILS, BEARING SEAT DETAILS, CONTROL ELEVATIONS, AND BAR DETAILS.

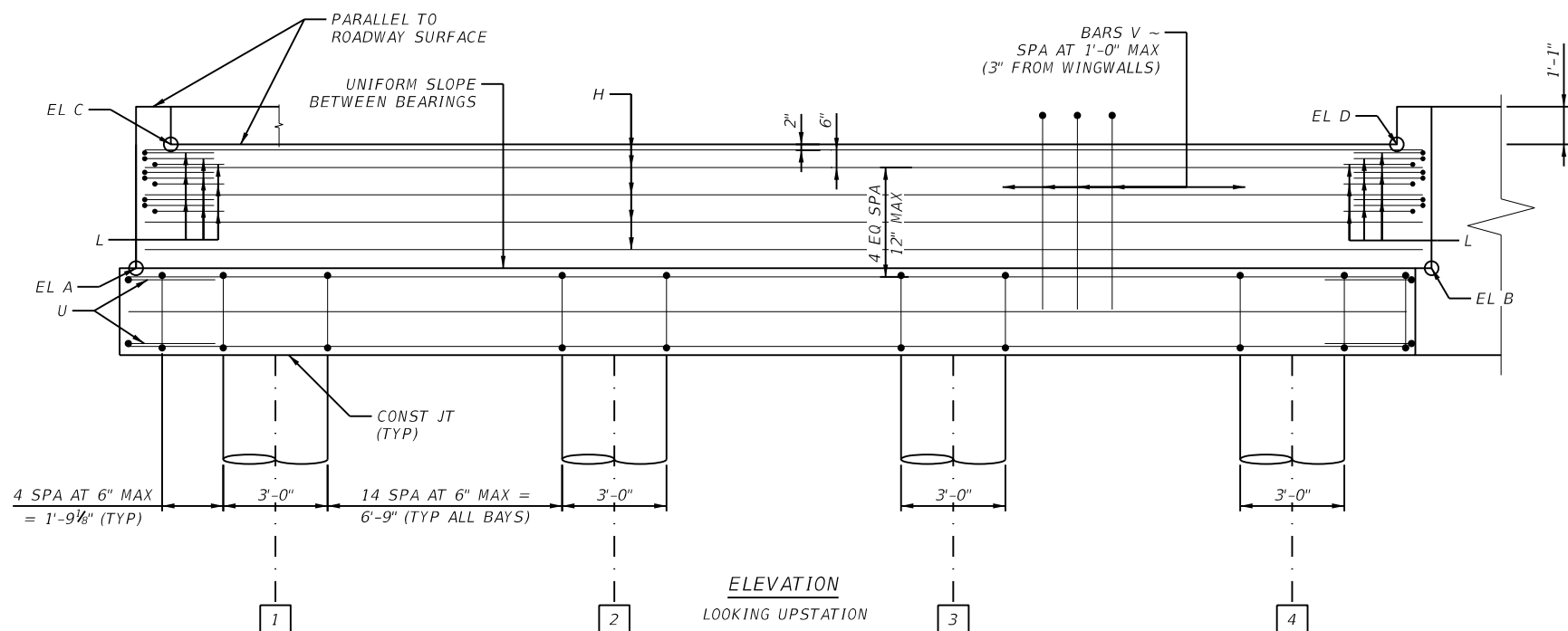
COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.
 REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR.

MATERIAL NOTES:

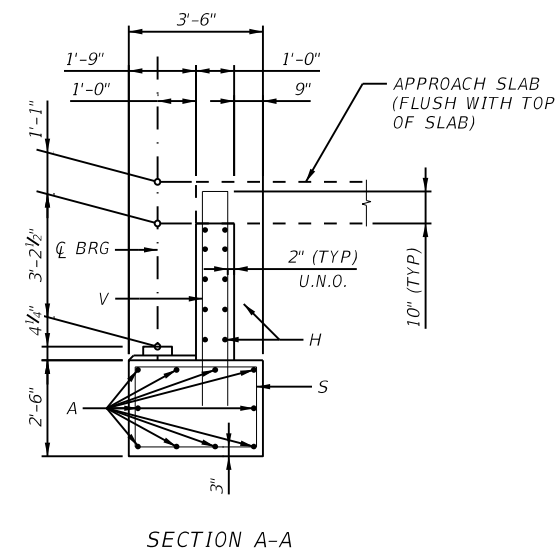
PROVIDE CLASS C CONCRETE, $f'_c = 3,600$ PSI
 PROVIDE GRADE 60 REINFORCING STEEL.
 CALCULATED FOUNDATION LOAD:
 103 TONS/SHAFT



Ryan C. Laurent



ELEVATION
 LOOKING UPSTATION



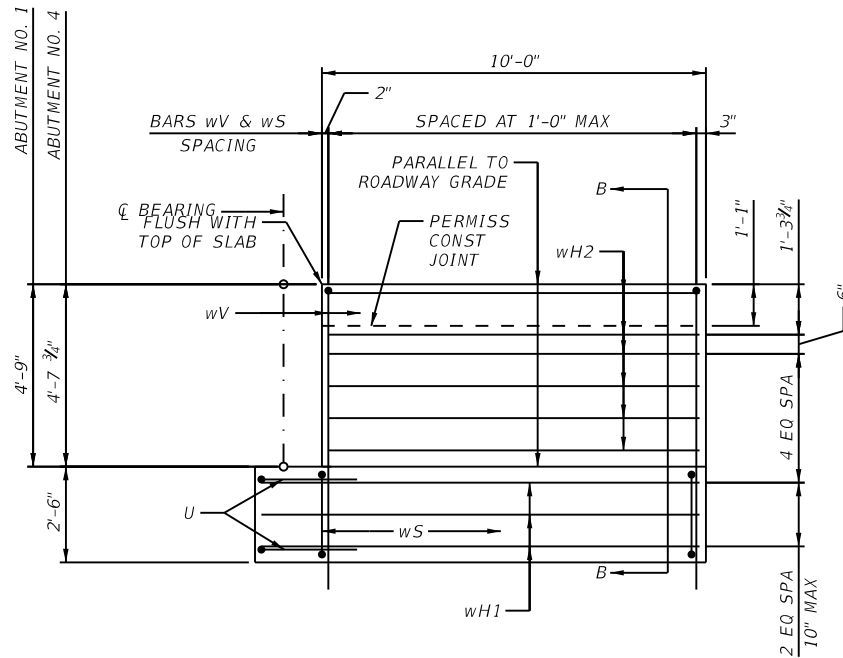
SECTION A-A

Kimley»Horn F-928
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 Texas Department of Transportation
 CR 118 AT COTTONWOOD CREEK
 ABUTMENT NO. 4

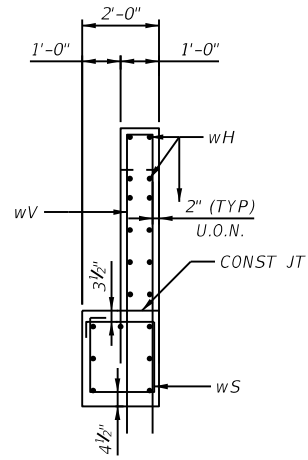
SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|---------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 81 | |

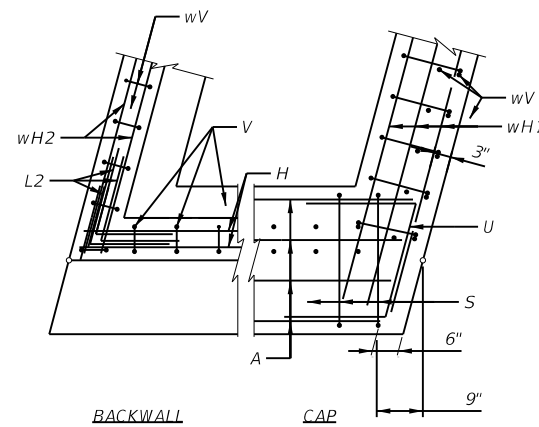
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WINGWALL ELEVATION



SECTION B-B



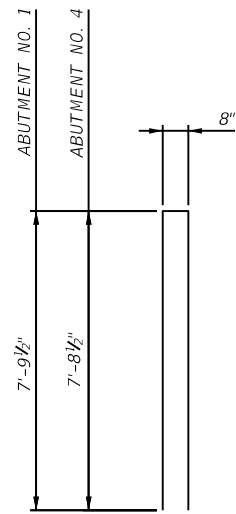
CORNER DETAILS

| TABLE OF ESTIMATED QUANTITIES | | | | |
|-------------------------------|-----|------|---------|--------|
| ABUTMENT 1 | | | | |
| BAR | NO. | SIZE | LENGTH | WEIGHT |
| A | 4 | #11 | 37'-4" | 794 |
| H | 10 | #6 | 36'-11" | 555 |
| L1 | 9 | #6 | 4'-0" | 54 |
| L2 | 9 | #6 | 4'-0" | 54 |
| S | 50 | #5 | 11'-6" | 648 |
| U | 4 | #6 | 8'-2" | 49 |
| V | 35 | #5 | 16'-3" | 593 |
| wH1 | 14 | #6 | 11'-5" | 240 |
| wH2 | 24 | #6 | 9'-8" | 349 |
| wS | 22 | #4 | 7'-10" | 115 |
| wV | 22 | #5 | 16'-3" | 373 |
| REINFORCING STEEL * | | | LB | 3,824 |
| CLASS "C" CONCRETE (ABUT) | | | CY | 23.8 |

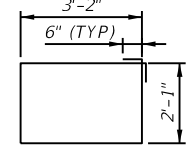
* FOR CONTRACTOR'S INFORMATION ONLY

| TABLE OF ESTIMATED QUANTITIES | | | | |
|-------------------------------|-----|------|---------|--------|
| ABUTMENT 4 | | | | |
| BAR | NO. | SIZE | LENGTH | WEIGHT |
| A | 4 | #11 | 37'-4" | 794 |
| H | 10 | #6 | 36'-11" | 555 |
| L1 | 9 | #6 | 4'-0" | 54 |
| L2 | 9 | #6 | 4'-0" | 54 |
| S | 50 | #5 | 11'-6" | 648 |
| U | 4 | #6 | 8'-2" | 49 |
| V | 35 | #5 | 16'-1" | 587 |
| wH1 | 14 | #6 | 11'-5" | 240 |
| wH2 | 24 | #6 | 9'-8" | 349 |
| wS | 22 | #4 | 7'-10" | 115 |
| wV | 22 | #5 | 16'-1" | 369 |
| REINFORCING STEEL * | | | LB | 3,814 |
| CLASS "C" CONCRETE (ABUT) | | | CY | 23.6 |

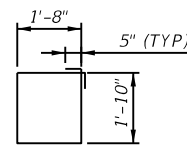
* FOR CONTRACTOR'S INFORMATION ONLY



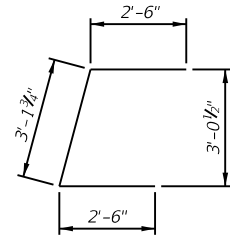
ABUTMENT NO. 1
 ABUTMENT NO. 4



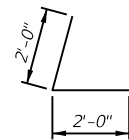
BARS S



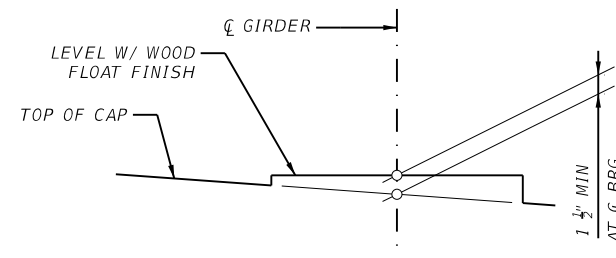
BARS wS



BARS U

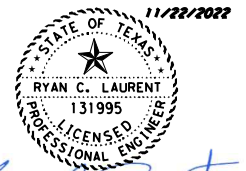


BARS L



BEARING SEAT DETAIL

(BEARING SURFACE MUST BE CLEAN AND FREE OF ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD.)



Ryan C. Laurent

| | CONTROL ELEVATIONS | | | |
|--------|--------------------|----------|-----------------|----------|
| | TOP OF CAP | | TOP OF BACKWALL | |
| | EL A | EL B | EL C | EL D |
| ABUT 1 | 711.274' | 713.135' | 715.933' | 715.925' |
| ABUT 4 | 709.057' | 711.049' | 713.707' | 713.548' |

| | CONTROL ELEVATIONS | | | |
|--------|-----------------------|----------|----------|----------|
| | TOP OF DRILLED SHAFT* | | | |
| | DS 1 | DS 2 | DS 3 | DS 4 |
| ABUT 1 | 708.974' | 709.461' | 709.948' | 710.435' |
| ABUT 4 | 706.771' | 707.292' | 707.813' | 708.335' |

* ELEVATIONS AT G OF DRILLED SHAFT

| | BEARING SEAT ELEVATIONS | | | | |
|--------------|-------------------------|----------|----------|----------|----------|
| | GRDR 1 | GRDR 2 | GRDR 3 | GRDR 4 | GRDR 5 |
| ABUT 1 (FWD) | 711.580' | 711.981' | 712.386' | 712.755' | 713.131' |
| ABUT 4 (BK) | 709.376' | 709.791' | 710.206' | 710.621' | 711.036' |

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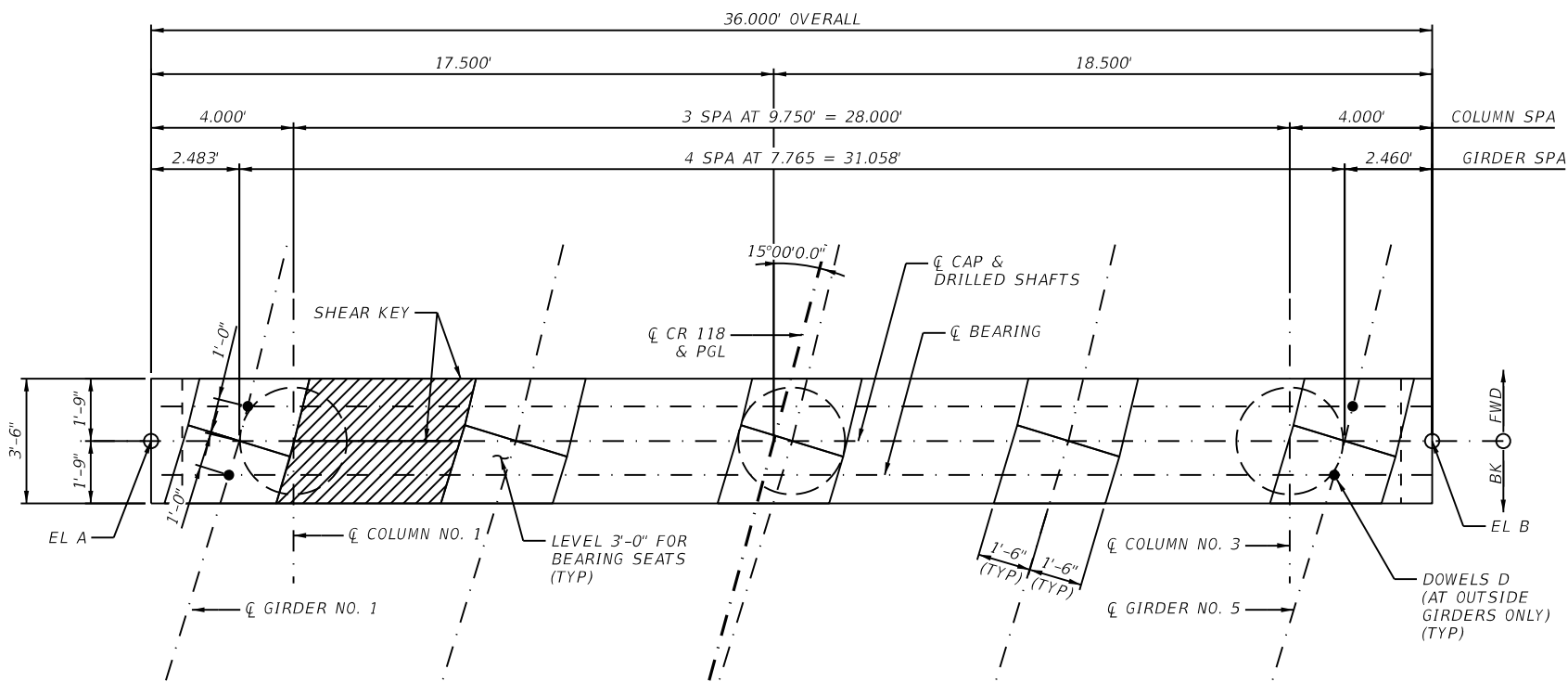
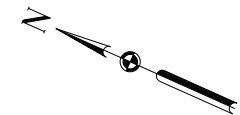
CR 118 AT COTTONWOOD CREEK

ABUTMENT DETAILS

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 82 |

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PLAN

NOTES:

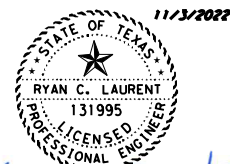
1. DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION (2020) AND INTERIM REVISIONS THERETO.
 2. SEE BRIDGE LAYOUT FOR HEADER SLOPE AND FOUNDATION TYPE, SIZE AND LENGTH.
 3. SEE COMMON FOUNDATION DETAILS (FD) STANDARD FOR ALL FOUNDATION DETAILS AND NOTES.
 4. SEE SHEAR KEY (IGSK) STANDARD FOR ALL SHEAR KEY DETAILS AND NOTES.
 5. SEE BENT DETAILS SHEET FOR BEARING SEAT DETAILS, CONTROL ELEVATIONS, AND BAR DETAILS.
- ① MEASURED PARALLEL TO THE TOP OF CAP-CROSS SLOPE.

COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.
 REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR.

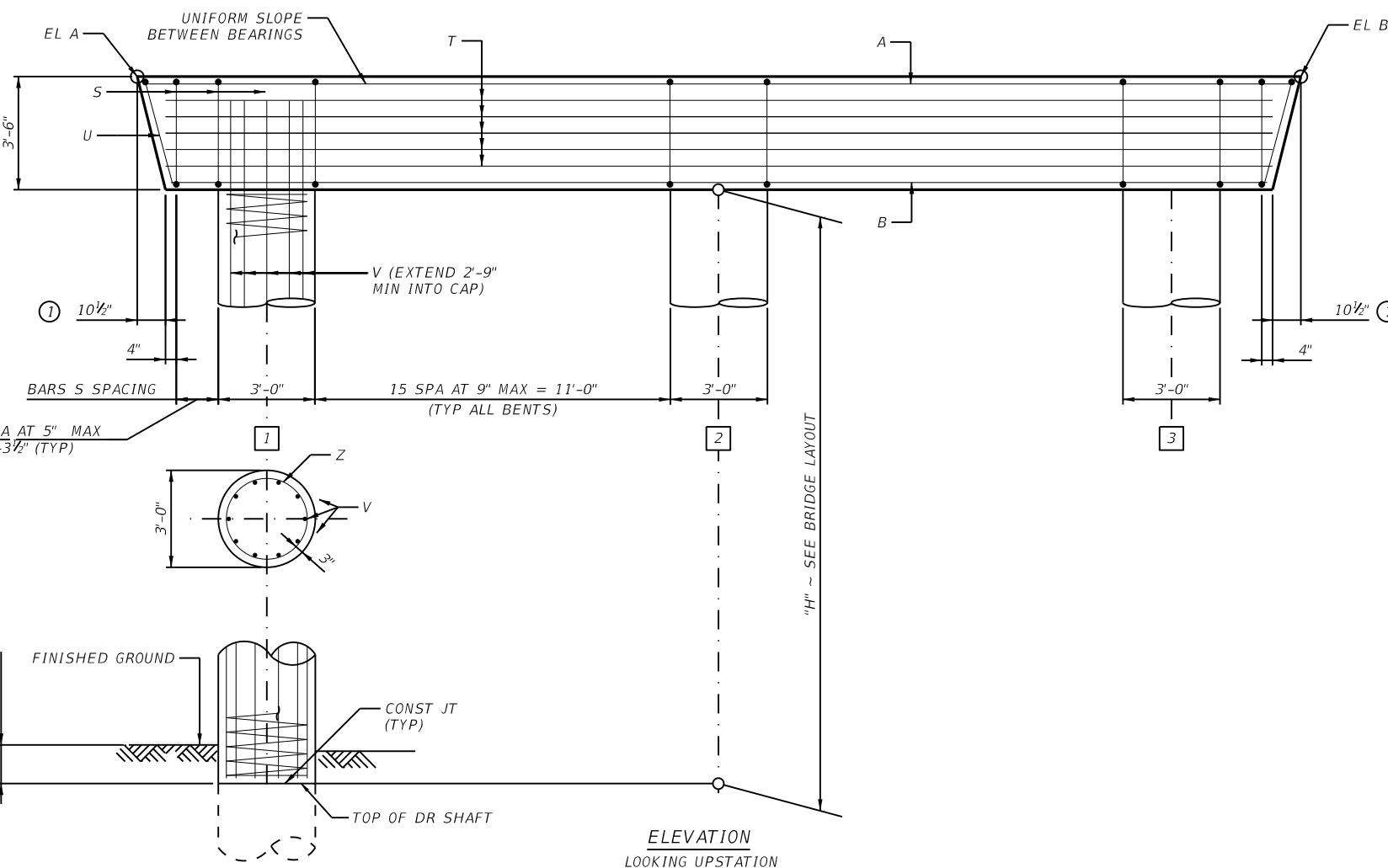
MATERIAL NOTES:

PROVIDE CLASS C CONCRETE, $f'_c = 3,600$ PSI
 PROVIDE GRADE 60 REINFORCING STEEL.

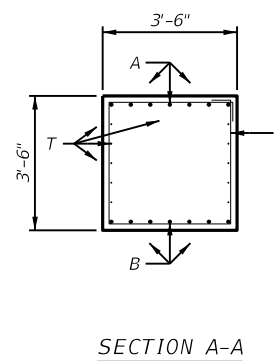
CALCULATED FOUNDATION LOAD:
 235 TONS/SHAFT



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ELEVATION
 LOOKING UPSTATION



SECTION A-A

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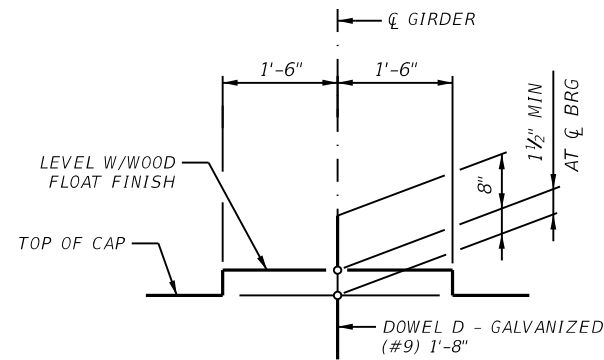
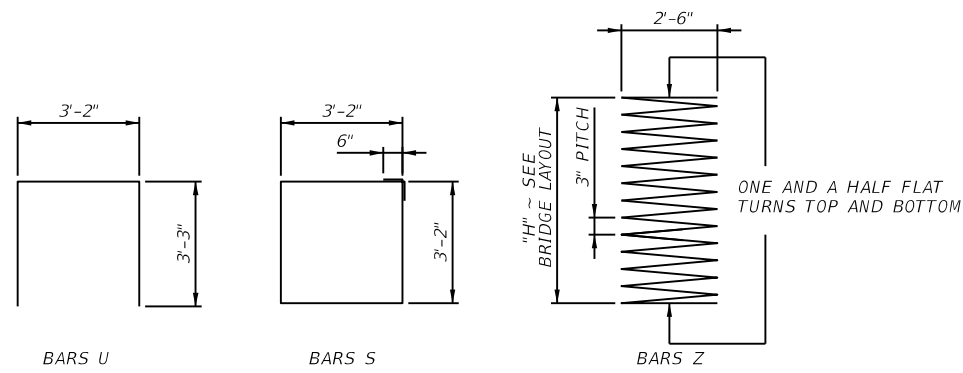
CR 118 AT COTTONWOOD CREEK

BENTS NO. 2 & 3

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 83 |

FILENAME: pw:\kn-pw-bent\kimley.com\kn-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\7. Bridge\CR118\CR118_BRG_BENT_DET
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BEARING SEAT DETAIL
 BEARING SURFACE MUST BE CLEAN AND FREE OF ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD

| TABLE OF ESTIMATED QUANTITIES BENT 2 & 3 (QUANTITIES PER BENT) | | | | | |
|---|-----|------|--------|--------|-------|
| BAR | NO. | SIZE | LENGTH | WEIGHT | |
| A | 7 | #11 | 35'-6" | 1,320 | |
| B | 7 | #11 | 34'-0" | 1,265 | |
| D | 4 | #9 | 1'-8" | 23 | |
| S | 38 | #5 | 13'-8" | 542 | |
| T | 10 | #5 | 34'-0" | 355 | |
| U | 2 | #5 | 9'-8" | 20 | |
| REINFORCING STEEL * | | | | LB | 3,525 |
| CLASS "C" CONCRETE (CAP) | | | | CY | 16.0 |

* FOR CONTRACTOR'S INFORMATION ONLY

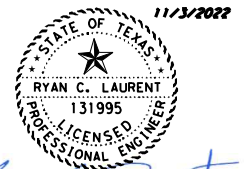
| TABLE OF ESTIMATED QUANTITIES | | | | | | | | | | |
|--|-----|-----|-----------|--------|--------|-----------|----------|----------------------------------|--------|--------------------|
| TABLE OF VARIABLE BENT COLUMN QUANTITIES | | | | | | | | ESTIMATED BENT COLUMN QUANTITIES | | |
| | COL | "H" | BARS V #9 | | | BARS Z #4 | | | REINF* | CL C CONC (COLUMN) |
| | | FT | NO. | LENGTH | WEIGHT | NO. | LENGTH | WEIGHT | LB | CY |
| BENT 2 | 1-3 | 4 | 30 | 6'-9" | 689 | 3 | 149'-3" | 299 | 988 | 3.2 |
| BENT 3 | 1-3 | 3 | 30 | 5'-9" | 587 | 3 | 117'-10" | 236 | 823 | 2.4 |
| TOTAL | | | | | | | | | 1,811 | 5.6 |

* FOR CONTRACTOR'S INFORMATION ONLY

| CONTROL ELEVATIONS | | | | | |
|--------------------|------------|----------|----------------|----------|----------|
| | TOP OF CAP | | TOP OF COLUMN* | | |
| | EL A | EL B | COL 1 | COL 2 | COL 3 |
| BENT 2 | 710.901' | 712.923' | 707.626' | 708.412' | 709.198' |
| BENT 3 | 710.088' | 712.054' | 706.807' | 707.571' | 708.336' |

* ELEVATIONS AT CL OF COLUMN

| BEARING SEAT ELEVATIONS | | | | | |
|-------------------------|----------|----------|----------|----------|----------|
| | EL A | EL B | EL C | EL D | EL E |
| BENT 2 (BK) | 711.166' | 711.602' | 712.038' | 712.474' | 712.911' |
| BENT 2 (FWD) | 711.194' | 711.630' | 712.066' | 712.502' | 712.938' |
| BENT 3 (BK) | 710.349' | 710.773' | 711.197' | 711.622' | 712.046' |
| BENT 3 (FWD) | 710.387' | 710.811' | 711.235' | 711.659' | 712.083' |



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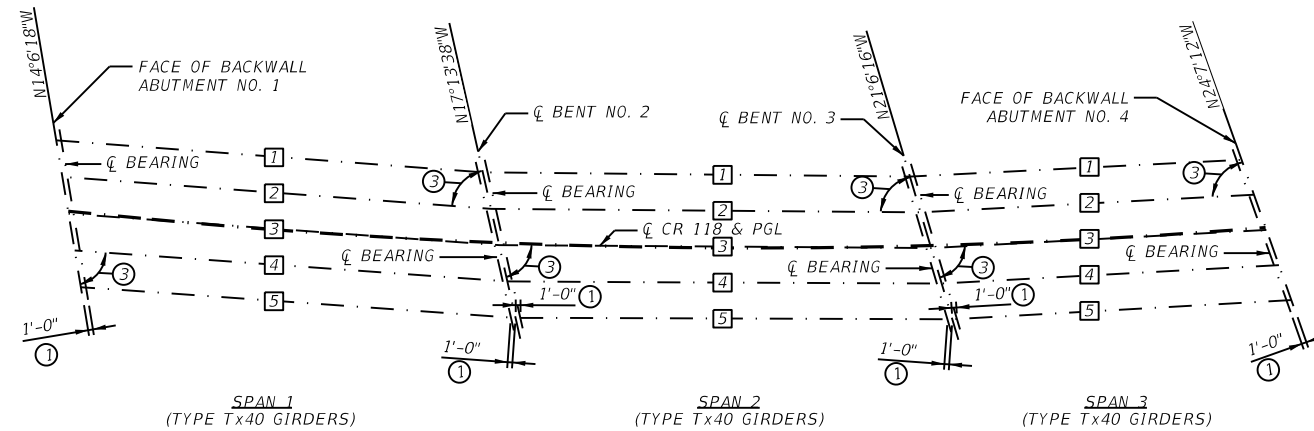
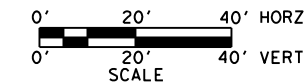
CR 118 AT COTTONWOOD CREEK

BENT DETAILS

SHEET 1 OF 1

| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
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| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | | COUNTY | SHEET NO. |
| | AUS | | WILLIAMSON | 84 |

FILENAME: \\kh-pw-bent\ky.com\kh-pw-01\Documents\01 Active Projects\TX-AUS-069284003 - Williamson County Bridges\DesignData\4 - Design\Plan Set\7. Bridge\CR118\CR118_BRG_FRAME
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FRAMING PLAN

NOTES:

- ① SEE IGEB STANDARD FOR ORIENTATION OF DIMENSION.
- ② BEAM LENGTHS SHOWN ARE BOTTOM OF BEAM LENGTHS WITH ADJUSTMENTS MADE FOR BEAM SLOPE.
- ③ GIRDER ANGLE (TYP).

| BENT REPORT | | | |
|---|------------|----------|--|
| BENT NO. 1 (S 14 6 9.01 E) | | | |
| DISTANCE BETWEEN STATION LINE AND BEAM 1, | | 15.529 L | |
| BEAM SPAC. | BEAM ANGLE | | |
| (C.L. BENT) | D M S | | |
| SPAN 1 BEAM 1 | 0.000 | 76 7 10 | |
| BEAM 2 | 7.765 | 76 11 29 | |
| BEAM 3 | 7.765 | 76 15 46 | |
| BEAM 4 | 7.765 | 76 20 1 | |
| BEAM 5 | 7.765 | 76 24 14 | |
| TOTAL | 31.058 | | |

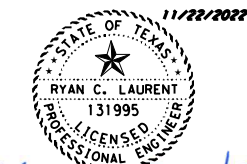
| BENT REPORT | | | |
|---|------------|----------|--|
| BENT NO. 2 (S 17 13 37.60 E) | | | |
| DISTANCE BETWEEN STATION LINE AND BEAM 1, | | 15.536 L | |
| BEAM SPAC. | BEAM ANGLE | | |
| (C.L. BENT) | D M S | | |
| SPAN 2 BEAM 1 | 0.000 | 76 45 48 | |
| BEAM 2 | 7.765 | 76 51 5 | |
| BEAM 3 | 7.765 | 76 56 19 | |
| BEAM 4 | 7.765 | 77 1 29 | |
| BEAM 5 | 7.765 | 77 6 35 | |
| TOTAL | 31.058 | | |

| BENT REPORT | | | |
|---|------------|----------|--|
| BENT NO. 3 (S 21 6 15.37 E) | | | |
| DISTANCE BETWEEN STATION LINE AND BEAM 1, | | 15.536 L | |
| BEAM SPAC. | BEAM ANGLE | | |
| (C.L. BENT) | D M S | | |
| SPAN 3 BEAM 1 | 0.000 | 76 19 57 | |
| BEAM 2 | 7.765 | 76 25 14 | |
| BEAM 3 | 7.765 | 76 30 28 | |
| BEAM 4 | 7.765 | 76 35 38 | |
| BEAM 5 | 7.765 | 76 40 44 | |
| TOTAL | 31.058 | | |

| BEAM REPORT | | | |
|---------------------|---------------|-----------------|---------|
| BEAM REPORT, SPAN 1 | | | |
| HORIZONTAL DISTANCE | TRUE DISTANCE | BEAM | |
| C-C BENT | C-C BRG. | BOT. BM. FLG. ② | SLOPE |
| BEAM 1 | 89.167 | 87.137 | -0.0047 |
| BEAM 2 | 89.576 | 87.546 | -0.0043 |
| BEAM 3 | 89.984 | 87.955 | -0.0040 |
| BEAM 4 | 90.392 | 88.363 | -0.0032 |
| BEAM 5 | 90.801 | 88.772 | -0.0025 |

| BEAM REPORT | | | |
|---------------------|---------------|-----------------|---------|
| BEAM REPORT, SPAN 2 | | | |
| HORIZONTAL DISTANCE | TRUE DISTANCE | BEAM | |
| C-C BENT | C-C BRG. | BOT. BM. FLG. ② | SLOPE |
| BEAM 1 | 88.968 | 86.968 | -0.0097 |
| BEAM 2 | 89.475 | 87.475 | -0.0098 |
| BEAM 3 | 89.982 | 87.982 | -0.0099 |
| BEAM 4 | 90.490 | 88.490 | -0.0099 |
| BEAM 5 | 90.998 | 88.998 | -0.100 |

| BEAM REPORT | | | |
|---------------------|---------------|-----------------|---------|
| BEAM REPORT, SPAN 3 | | | |
| HORIZONTAL DISTANCE | TRUE DISTANCE | BEAM | |
| C-C BENT | C-C BRG. | BOT. BM. FLG. ② | SLOPE |
| BEAM 1 | 69.203 | 67.159 | -0.0150 |
| BEAM 2 | 69.597 | 67.554 | -0.0151 |
| BEAM 3 | 69.992 | 67.949 | -0.0151 |
| BEAM 4 | 70.386 | 68.344 | -0.0152 |
| BEAM 5 | 70.781 | 68.739 | -0.0152 |



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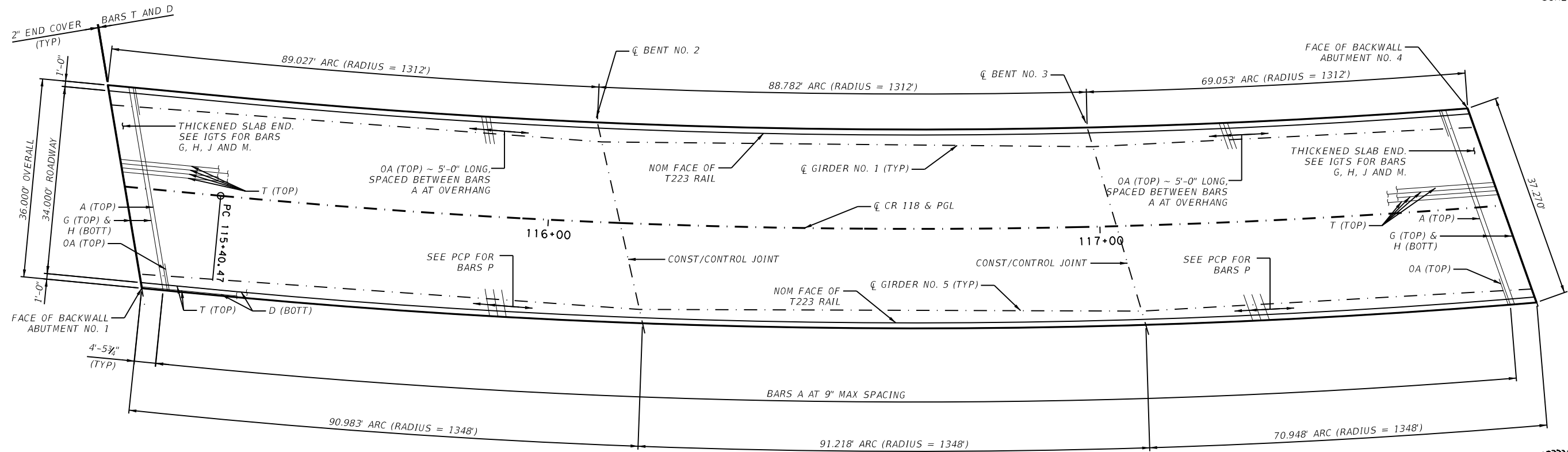
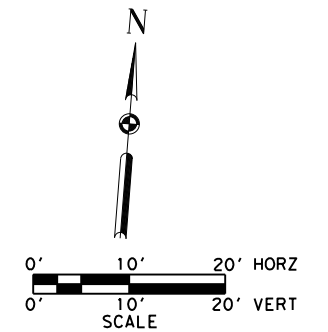
CR 118 AT COTTONWOOD CREEK

FRAMING PLAN

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 85 |

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PLAN

| SPAN NO. | "A" | "B" |
|----------|-------|-------|
| | FT | FT |
| 1 | 0.082 | 0.115 |
| 2 | 0.083 | 0.116 |
| 3 | 0.029 | 0.041 |

| | REINF CONCRETE SLAB | PRESTRESSED CONCRETE GIRDERS (Tx40) ① | TOTAL REINF STEEL * ② |
|--------|---------------------|---------------------------------------|-----------------------|
| | SF | LF | LB |
| SPAN 1 | 3,240 | 445.53 | 7,452 |
| SPAN 2 | 3,240 | 445.61 | 7,452 |
| SPAN 3 | 2,520 | 345.56 | 5,796 |

- * FOR CONTRACTOR'S INFORMATION ONLY
- ① GIRDER LENGTHS ARE MEASURED ALONG BOTTOM OF FLANGE.
 - ② REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.

| BAR | SIZE |
|-----|------|
| A | #4 |
| D | #4 |
| G | #4 |
| H | #4 |
| J | #4 |
| M | #4 |
| OA | #5 |
| P | #4 |
| T | #4 |

MATERIAL NOTES:

ALL CONCRETE SHALL BE CLASS S (HPC) CONCRETE, ($f'_c = 4,000$ PSI)

ALL REINFORCING SHALL BE EPOXY COATED, GRADE 60 REINFORCING STEEL.

PROVIDE BAR LAPOS, WHEN REQUIRE4D. UNCOATED ~ #4 - 1'-7".

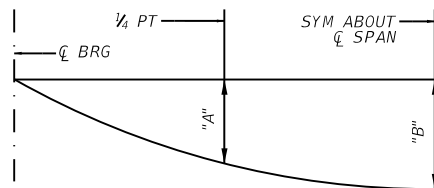
LAPS IN BARS A AND T SHALL BE STAGGERED AND ALTERNATED TO MAXIMIZE THE DISTANCE BETWEEN ADJACENT SPLICES.

NOTES:

1. DESIGNED ACCORDING TO AASHTO LFRD BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION (2020) AND CURRENT INTERIMS.
2. MULTI-SPAN UNITS, WITH SLAB CONTINUOUS OVER INTERIOR BENTS, MAY BE FORMED WITH THE DETAILS SHOWN ON THIS SHEET AND TXDOT STANDORD SHEET, IGCS.
3. SEE TXDOT STANDARD, IGTS, FOR THICKENED SLAB END DETAILS AND QUANTITY ADJUSTMENTS.
4. SEE TXDOT STANDARD, IGMS, FOR MISCELLANEOUS SLAB DETAILS.
5. SEE TXDOT STANDARD, PCP AND PCP-FAB, FOR PANEL DETAILS.
6. SEE TXDOT STANDARD, PMDF, FOR DETAILS AND QUANTITY ADJUSTMENT IF THIS OPTION IS USED.
7. SEE TXDOT STANDARD, SEJ-M, FOR SEALED EXPANSION JOINT DETAILS NOT SHOWN.
8. SEE TXDOT STANDARD, T223 RAIL DETAILS FOR RAIL ANCHORAGE IN SLAB.

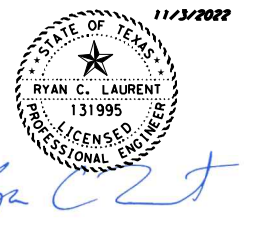
COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.

REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT BAR.



DEAD LOAD DEFLECTION DIAGRAM

CALCULATED DEFLECTIONS SHOWN ARE DUE TO THE CONCRETE SLAB ONLY ($E_c = 5000$ KSI). ADJUST VALUES AS REQUIRED FOR EXTERIOR GIRDERS AND IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERIFICATION.



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Texas Department of Transportation

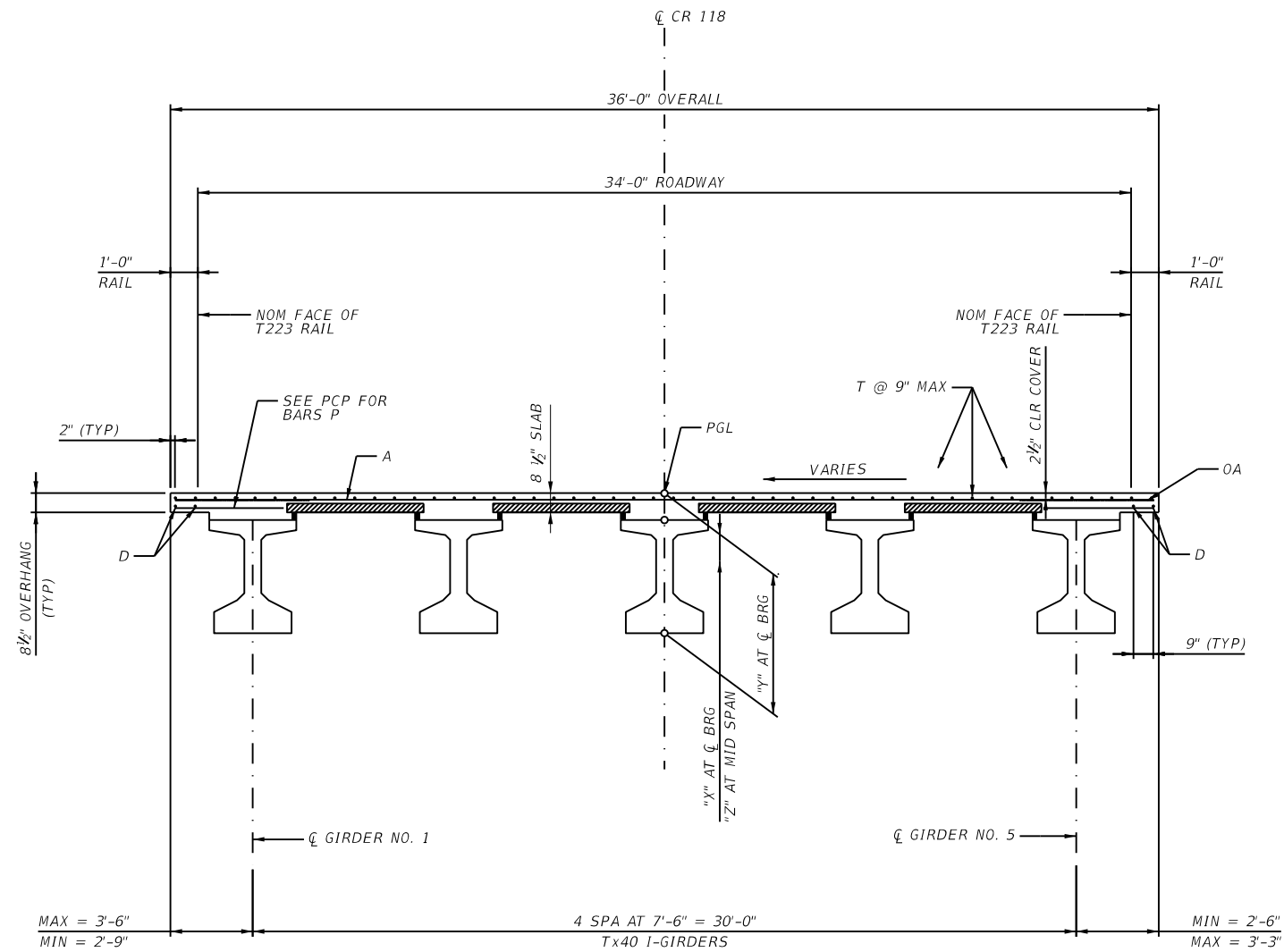
CR 118 AT COTTONWOOD CREEK

SLAB DETAILS

SHEET 1 OF 1

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
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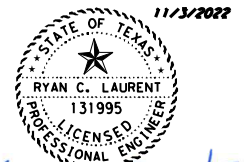
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PROPOSED BRIDGE TYPICAL SECTION

| TABLE OF SECTION DEPTHS | | | |
|-------------------------|------------------|------------------|--------------------|
| SPAN NO. | "X" AT CL BRG | "Y" AT CL BRG | "Z" AT MID SPAN |
| 1 | 12 3/4" | 4'-4 3/4" | 11" |
| 2 | 12 1/4" | 4'-4 1/4" | 10 1/4" |
| 3 | 11 1/2" | 4'-3 1/2" | 10 1/4" |

| TABLE OF VARYING OVERHANGS * | | |
|------------------------------|--------|--------|
| CL BENT | LEFT | RIGHT |
| 1 | 3.500' | 2.500' |
| 2 | 3.500' | 2.500' |
| 3 | 3.500' | 2.500' |
| 4 | 3.500' | 2.500' |



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CR 118 AT COTTONWOOD CREEK

BRIDGE TYPICAL
TRANSVERSE SECTION

SHEET 1 OF 1

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 87 |

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| STRUCTURE | DESIGNED GIRDERS | | | | | | | | DEPRESSED STRAND PATTERN | | CONCRETE | | OPTIONAL DESIGN | | | | LOAD RATING FACTORS | | | | |
|----------------------------|------------------|------------|-------------|------------------------|-----------|-----------|-----------------------|--------------------|--------------------------|--------|----------|-------|---|---|---|-------------------------------|---------------------|------------|------|-------------|------|
| | SPAN NO. | GIRDER NO. | GIRDER TYPE | PRESTRESSING STRANDS | | | | | | | | | DESIGN LOAD COMP STRESS (TOP ϵ) (SERVICE I) | DESIGN LOAD TENSILE STRESS (BOTT ϵ) (SERVICE III) | REQUIRED MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I) (kip-ft) | LIVE LOAD DISTRIBUTION FACTOR | | STRENGTH I | | SERVICE III | |
| | | | | NON-STD STRAND PATTERN | TOTAL NO. | SIZE (in) | STRGTH f_{pu} (ksi) | "e" \bar{e} (in) | "e" END (in) | Moment | Shear | Inv | | | | Opr | Inv | | | | |
| CR 118 AT COTTONWOOD CREEK | 1 | 1-5 | TX40 | | 30 | 0.6 | 270 | 14.40 | 8.40 | 6 | 28.5 | 5.000 | 6.500 | 3.498 | -3.857 | 4744 | 0.629 | 0.822 | 1.50 | 1.94 | 1.02 |
| | 2 | 1-5 | TX40 | | 30 | 0.6 | 270 | 14.40 | 8.40 | 6 | 30.5 | 5.000 | 6.500 | 3.444 | -3.815 | 4731 | 0.634 | 0.822 | 1.50 | 1.94 | 1.05 |
| | 3 | 1-5 | TX40 | | 16 | 0.6 | 270 | 15.16 | 14.27 | 4 | 28.5 | 4.100 | 5.000 | 2.087 | -2.419 | 3104 | 0.645 | 0.819 | 1.42 | 1.84 | 1.28 |

① Based on the following allowable stresses (ksi):

Compression = $0.65 f'_{ci}$

Tension = $0.24 \sqrt{f'_{ci}}$

Optional designs must likewise conform.

② Portion of full HL93.

DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to AASHTO Manual for Bridge Evaluation.

Optional designs for girders 120 feet or longer must have a calculated residual camber equal to or greater than that of the designed girder.

Prestress losses for the designed girders have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform.

FABRICATION NOTES:

Provide Class H concrete.

Provide Grade 60 reinforcing steel bars.

Use low relaxation strands, each pretensioned to 75 percent of f_{pu} .

Strand debonding must comply with Item 424.4.2.2.4. Full-length debonded strands are only permitted in positions marked Δ . Double wrap full-length debonded strands in outer most position of each row.

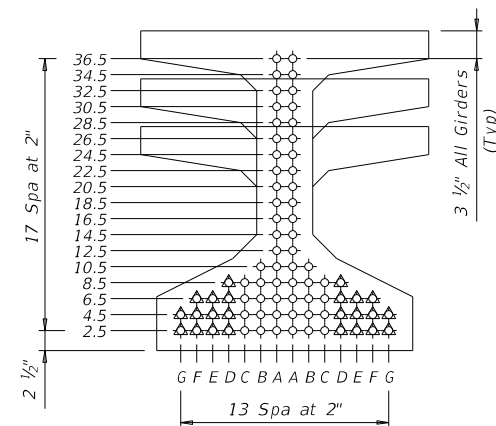
When shown on this sheet, the Fabricator has the option of furnishing either the designed girder or an approved optional design. All optional design submittals must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.

Seal cracks in girder ends exceeding 0.005" in width as directed by the Engineer. The fabricator is permitted to decrease the spacing of Bars R and S by providing additional bars to help limit crack width provided the decreased spacing results in no less than 1" clear between bars. The fabricator must take an approved corrective action if cracks greater than 0.005" form on a repetitive basis.

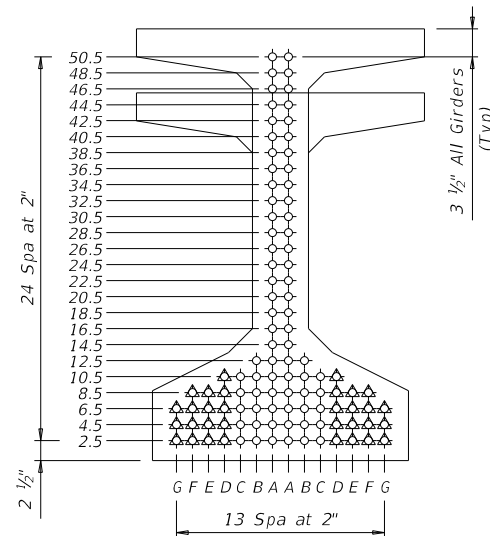
DEPRESSED STRAND DESIGNS:

Locate strands for the designed girder as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc., beginning each row in the "A" position and working outward until the required number of strands is reached. All strands in the "A" position must be depressed, maintaining the 2" spacing so that, at the girder ends, the upper two strands are in the position shown in the table.

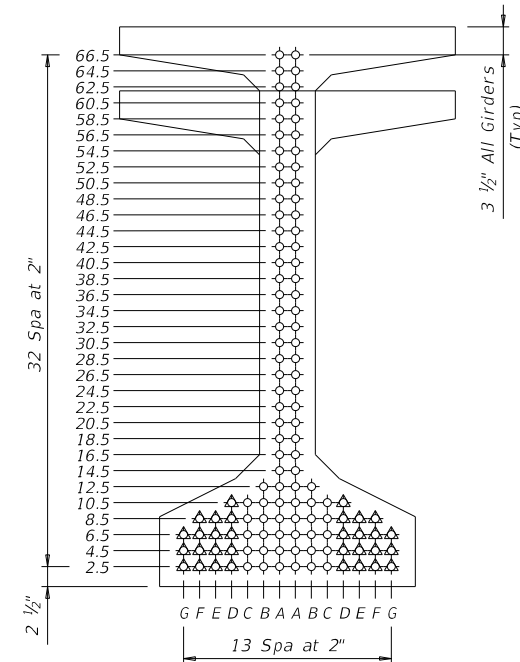
To complete this sheet input the girder designs in the table and the relative humidity under Design Notes. In all cases, remove this block. This sheet must be signed, sealed, and dated by a registered Professional Engineer.



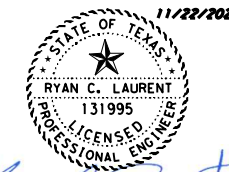
TYPE Tx28, Tx34 & Tx40



TYPE Tx46 & Tx54



TYPE Tx62 & Tx70



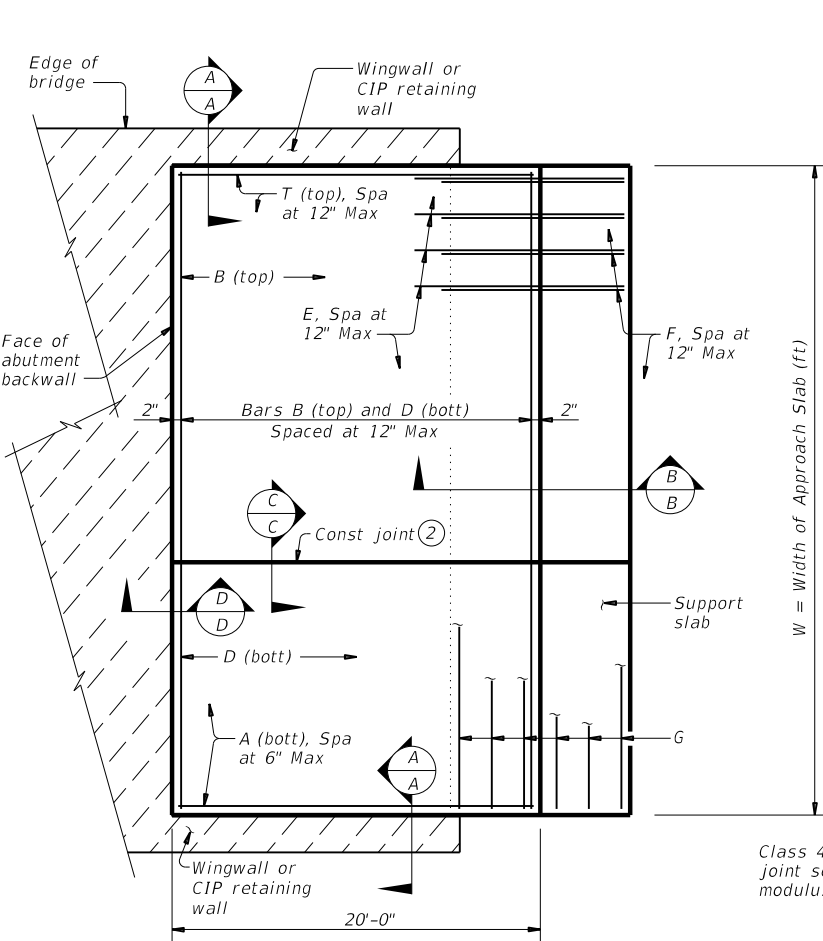
Ryan C. Laurent

HL93 LOADING

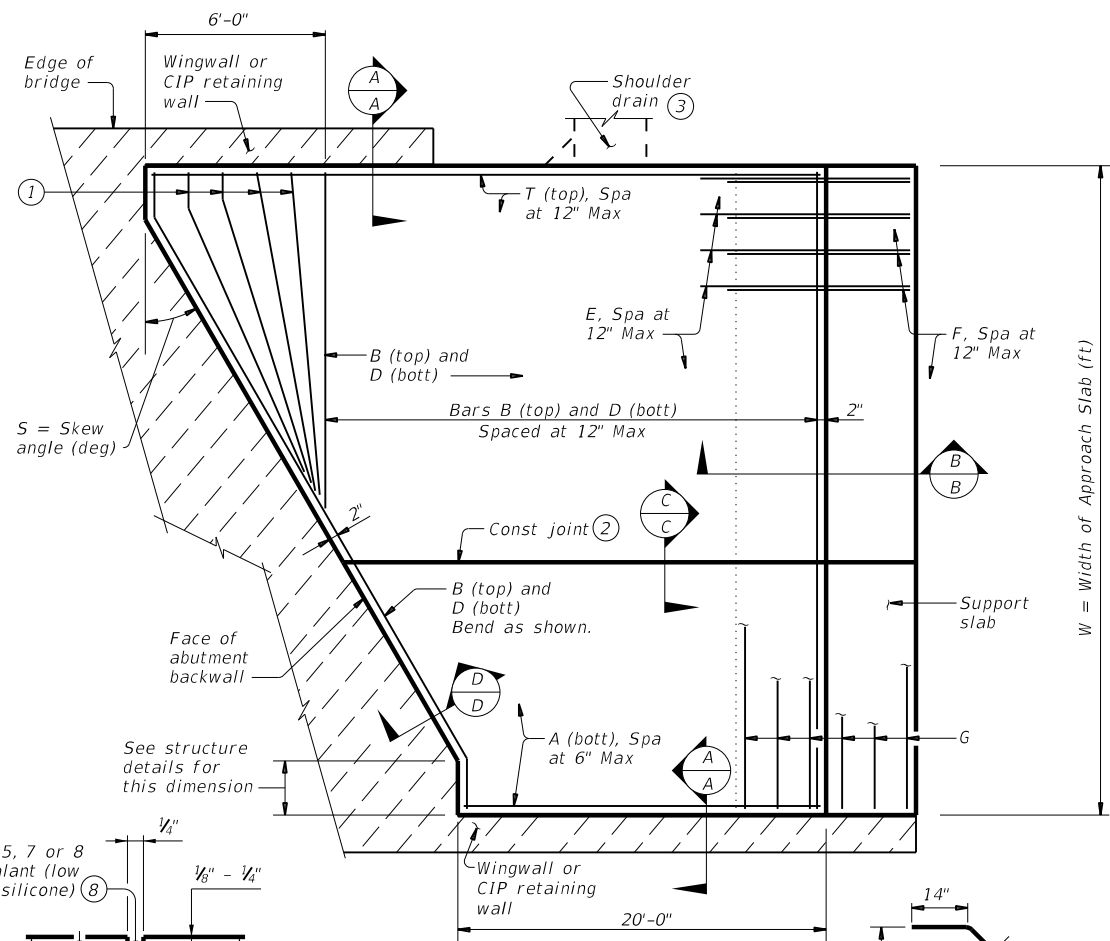
| | | | |
|--|-------------|-------------------------------------|---------|
| | | Bridge Division Standard | |
| PRESTRESSED CONCRETE I-GIRDER DESIGNS (NON-STANDARD SPANS) | | | |
| IGND(MOD) | | | |
| FILE: igndsts1-22.dgn | DN: TxDOT | CK: TxDOT | DW: EFC |
| 0914 05 | August 2017 | 204, ETC. | CR 118 |
| REVISIONS 10-19: Modified for depressed strands only. 3-22: Added Load Rating. | | COUNTY: WILLIAMSON SHEET NO.: 88 | |

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DATE: 11/3/2022 12:56:49 PM
 FILE: c:\pwwork\1\0222653\bascsteel-20 (1).dgn



PLAN
(Showing non-skewed approach slab.)



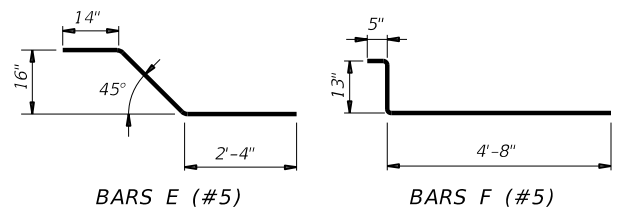
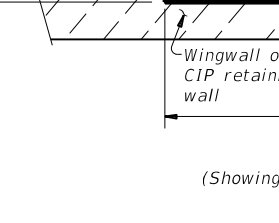
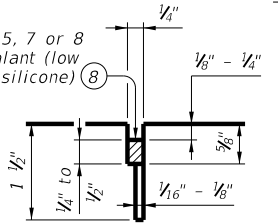
PLAN
(Showing skewed approach slab.)

| BAR TABLE | |
|-----------|------|
| BAR | SIZE |
| A | #8 |
| B | #5 |
| D | #5 |
| E | #5 |
| F | #5 |
| G | #5 |
| T | #5 |

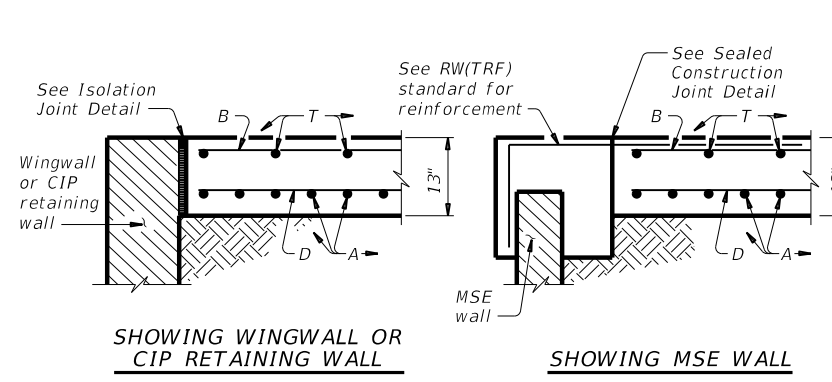
| APPROXIMATE QUANTITIES ④ | |
|------------------------------|--|
| Reinf steel weight = | 8.5 Lbs/SF of Approach Slab 18.4 Lbs/LF of Support Slab |
| Vol of Appr Slab Conc (CY) = | $1.057W - 0.008W \times T + 0.02W^2 \tan S$ (Includes Support Slab) |
| W = | Width of Approach Slab (ft) |
| T = | Conc Pavement Thickness (in) |
| S = | Skew Angle (deg) |

- ① Flare Bars B and D in this region (1'-6" Max Spa, 3" Min Spa). Minimum flared bar length = 2'-6". Bend bars as necessary.
- ② Provide longitudinal construction joints that align with longitudinal construction joints in the bridge slab with bridges built in stages. Other longitudinal construction joints must receive approval of the Engineer.
- ③ See details elsewhere in plans for shoulder drain location and details.
- ④ For Contractor's information only. Quantities shown are for one approach slab only.
- ⑤ On portion of support slab that supports the concrete pavement, adjust top surface elevation, if required, to accommodate concrete pavement thickness. Smooth trowel finish. Oil top of support slab with 60 grade oil and apply heavy coat of powdered graphite. Press down one layer of 30# roofing felt.
- ⑥ Multiple piece tie bars are acceptable at longitudinal construction joints provided minimum laps shown are achieved.
- ⑦ See details elsewhere in plans for required cross-slope.
- ⑧ Place in accordance with Item 438.
- ⑨ Provide backer rod that is 25% larger than joint opening and compatible with the sealant.
- ⑩ If bridge rail is present at the wingwall or CIP retaining wall, place 1/2" rebonded recycled tire rubber between concrete railing and top of approach slab as shown when concrete railing projects over the approach slab.

LONGITUDINAL SAW CUT JOINT DETAIL

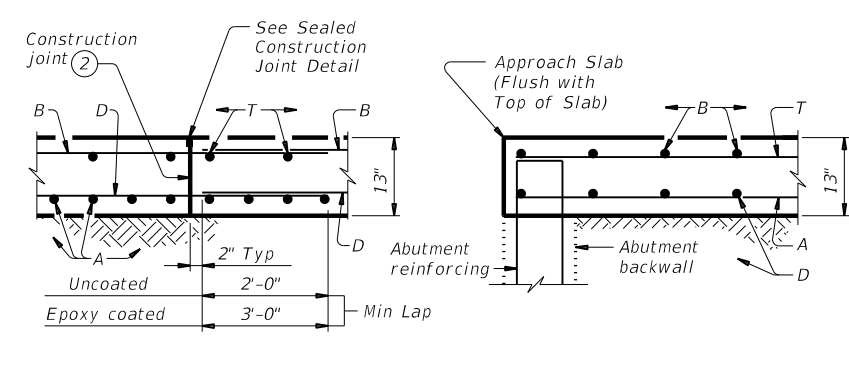


GENERAL NOTES:
 Construct approach slab in accordance with Item 422.
 Provide Class "S" concrete with a minimum compressive strength of 4,000 psi.
 Provide Grade 60 reinforcing steel.
 Provide longitudinal joints as shown on the Longitudinal Saw Cut Joint Detail at lane lines and shoulders when width between longitudinal construction joints or edges of approach slab exceeds 16 feet. Saw cut joints within 24 hours of concrete placement to a depth of 1 1/2" and seal in accordance with Item 438. Alternately, provide a controlled joint consisting of 1 1/2" vinyl or plastic joint former (Stress Cap, Zip Strip, Stress Lock, or equal as approved by the Engineer).
 Provide rebonded recycled tire rubber joint filler that meets the requirements of DMS-6310, "Joint Sealants and Fillers."
 Construct the subgrade or subbase away from the bridge for a minimum distance of 100 feet prior to the approach slab, unless otherwise indicated on the plans.
 Compact and finish the subgrade or foundation for the approach slab to the typical cross-section and to the lines and grades shown on the plans.
 Cure for 4 days using water or membrane curing per Item 422. All details shown herein are subsidiary to bridge approach slab.
 Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.



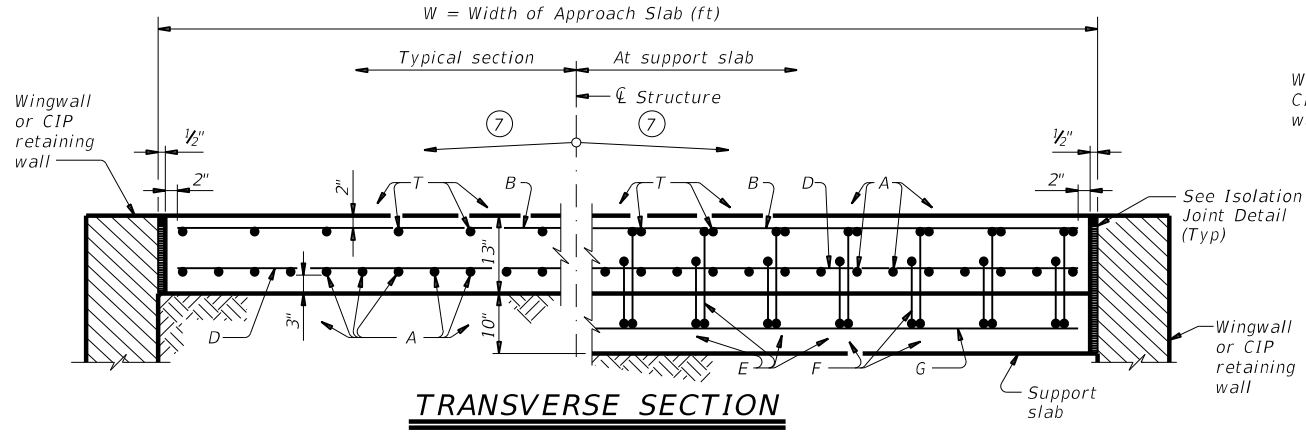
SECTION A-A

SECTION B-B

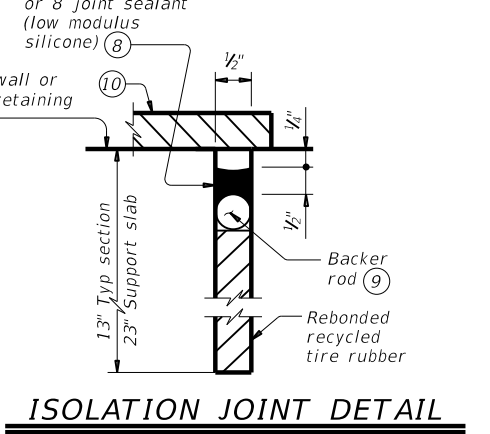


SECTION C-C

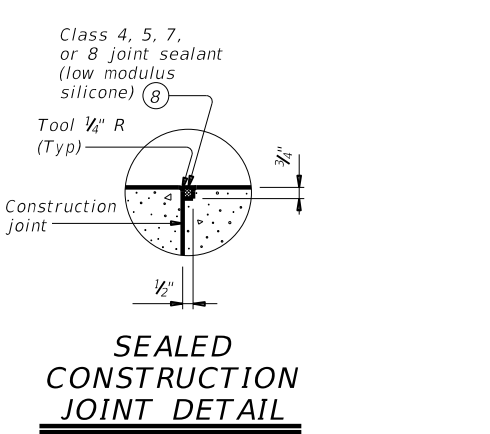
SECTION D-D



TRANSVERSE SECTION



ISOLATION JOINT DETAIL

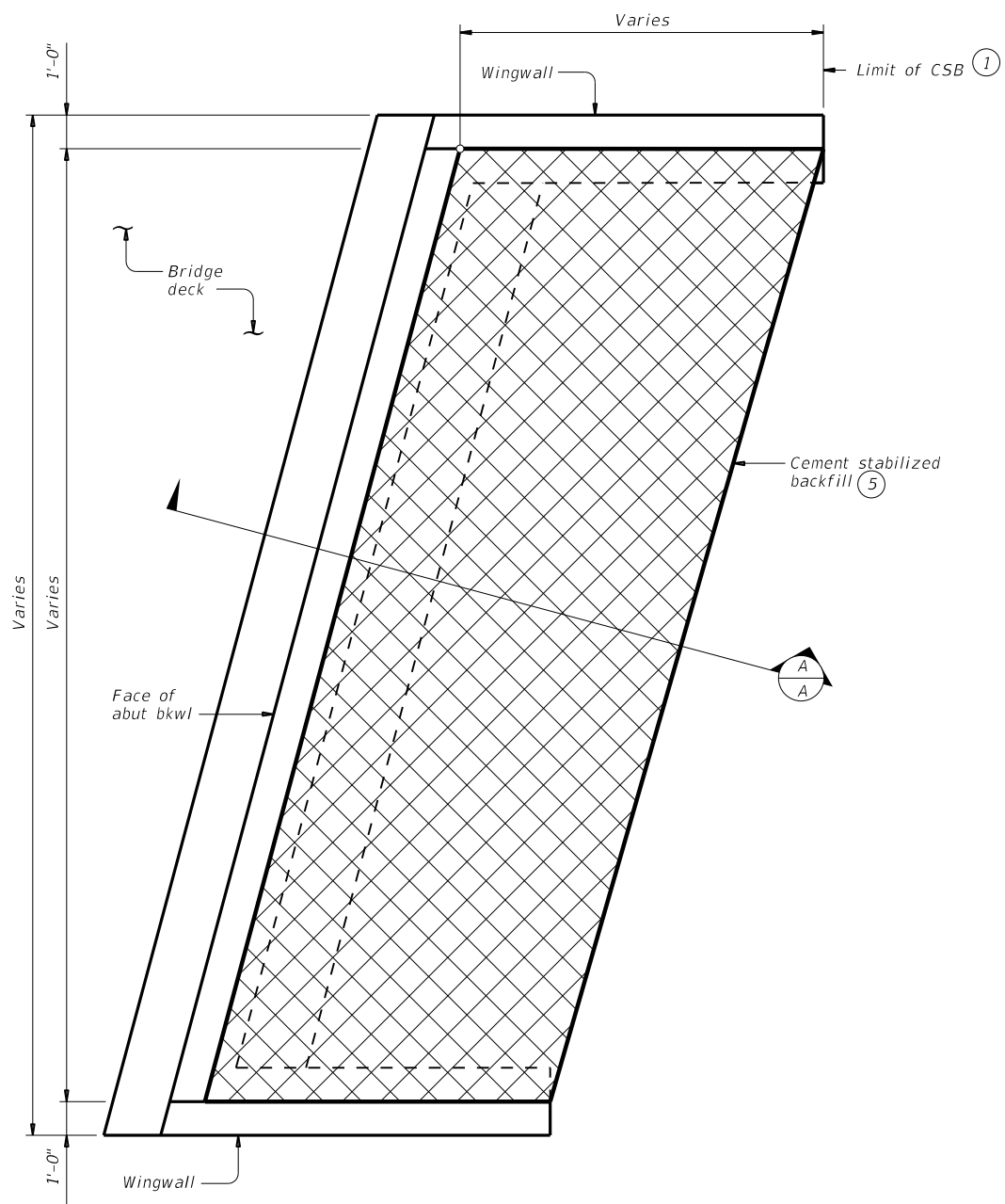


SEALED CONSTRUCTION JOINT DETAIL

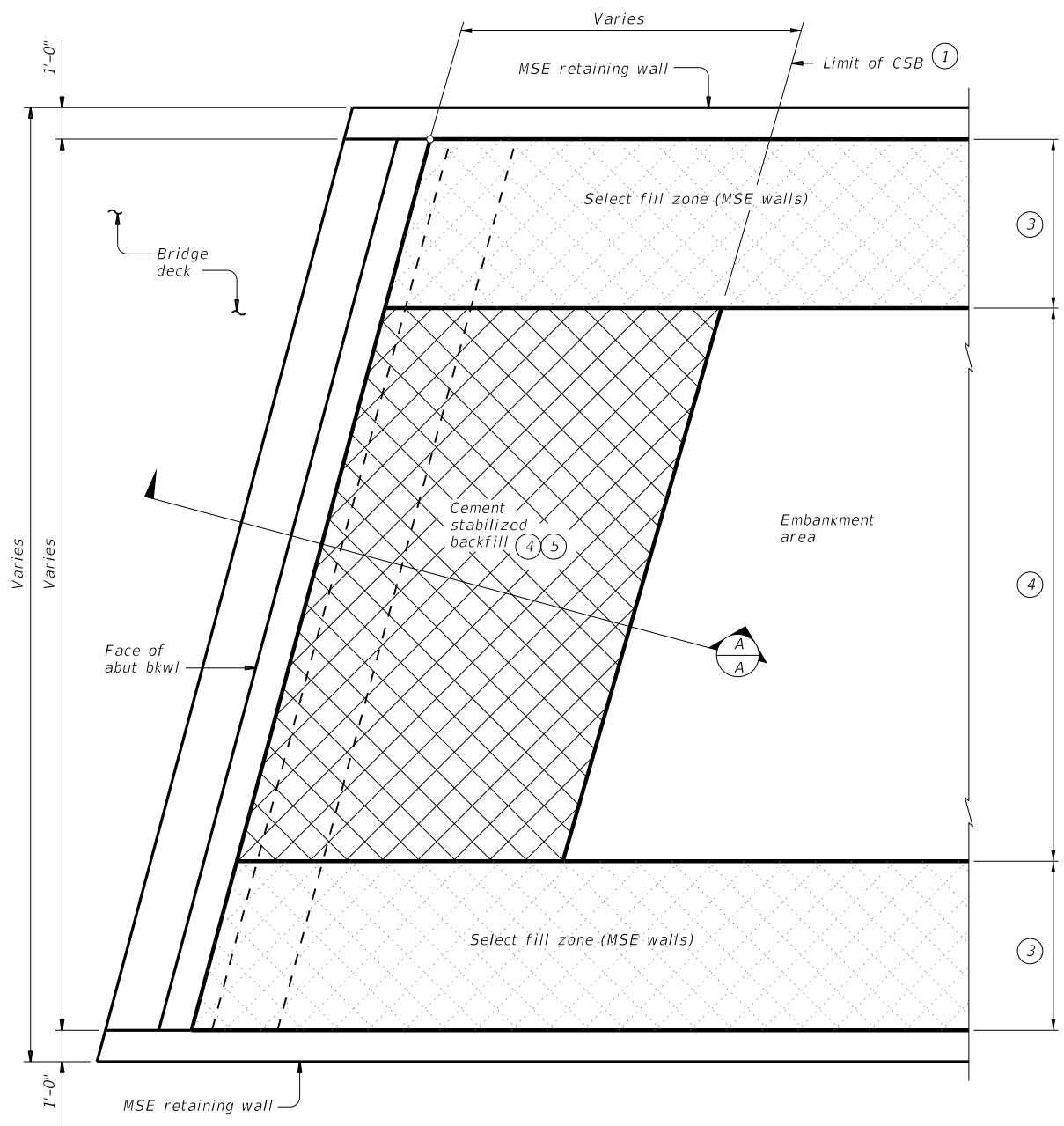
| | | | |
|---|-----------|---------------------------------|----------------|
| | | Bridge Division Standard | |
| BRIDGE APPROACH SLAB CONCRETE PAVEMENT | | | |
| BAS-C | | | |
| FILE: bascte1-20.dgn | DN: TxDOT | CK: TxDOT | OW: TxDOT |
| ©TxDOT April 2019 | CON: 0914 | SECT: 05 | JOB: 204, ETC. |
| REVISIONS | CR 118 | SHEET NO. | |
| 02-20: Removed stress relieving pad. | DIST: AUS | COUNTY: WILLIAMSON | 89 |

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DATE: 11/3/2022 12:56:55 PM
 FILE: c:\pwworkh\1\0222653\csabste1-20 (1).dgn



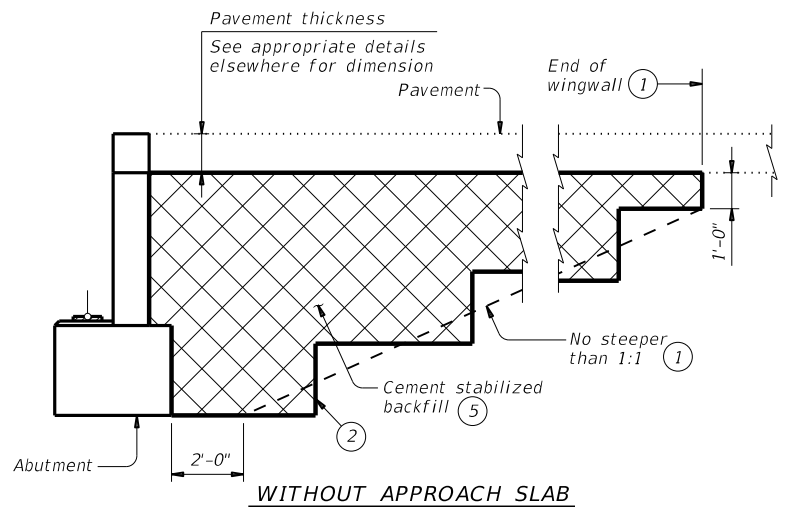
OPTION 1 ~ PLAN WITH WINGWALLS
 Cast-in-place retaining walls similar.



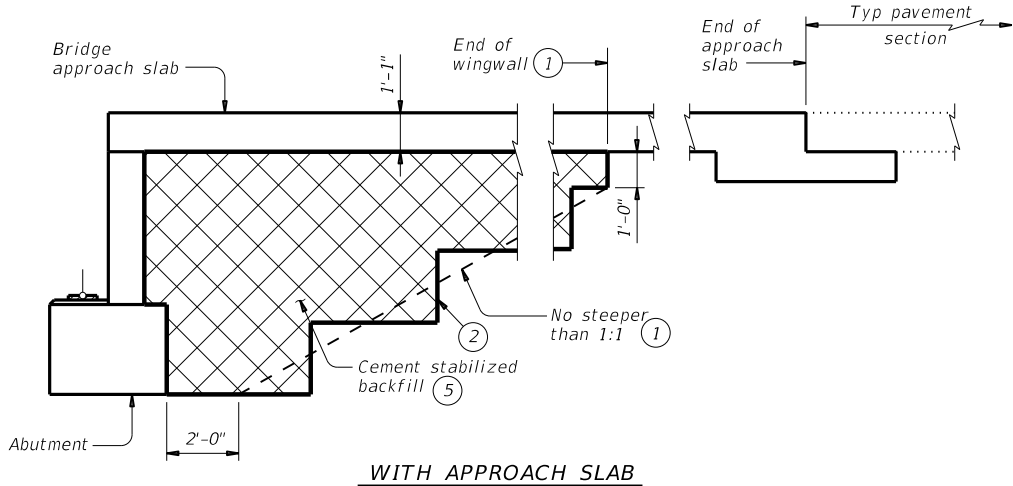
OPTION 1 ~ PLAN WITH MSE RETAINING WALLS

- 1 Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.
- 2 Bench backfill as shown with 12" (approximate) bench depths.
- 3 Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.
- 4 When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.
- 5 If shown in the plans flowable backfill can be used as a substitute for cement stabilized backfill with the following constraints:
 - a) If flowable backfill is to be placed over MSE backfill then a filter fabric will be placed over the flowable fill; and
 - b) Place flowable fill in lifts not exceeding 2 feet in height, place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowability).

GENERAL NOTES:
 See the Bridge Layout for selected Option. Option 2 is intended for new construction requiring high plasticity embankment fill with a plasticity index (PI) greater than 30 or pavement built in poor native soil. Poor soils are defined as high plasticity clays or expansive clays. Option 1 is intended for construction only requiring PI controlled embankment fill or excavation in competent soils/rocks in order to construct the abutment.
 Provide Cement Stabilized Backfill (CSB) meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments.
 If required elsewhere in the plans, provide Flowable Backfill meeting the requirements of Item 401, "Flowable Backfill", to the limits shown at bridge abutments.
 Details are drawn showing left forward skew. See Bridge Layout for actual skew direction.
 These details do not apply when Concrete Block retaining walls are used in lieu of wingwalls.



WITHOUT APPROACH SLAB

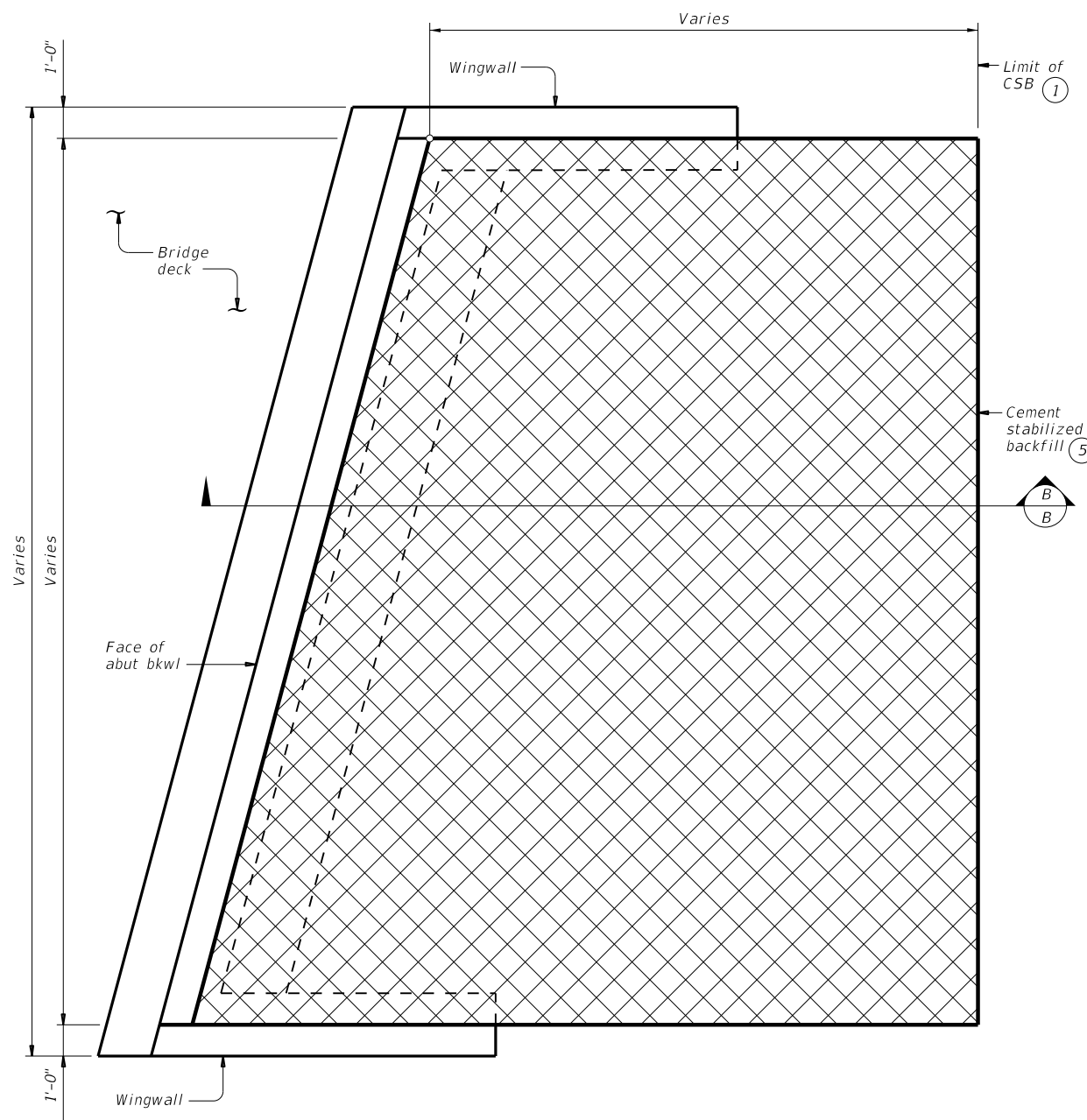


SECTION A-A
 WITH APPROACH SLAB
 (Showing BAS-C, BAS-A similar.)

| | | | |
|--|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| CEMENT STABILIZED ABUTMENT BACKFILL BRIDGE ABUTMENT | | | |
| CSAB | | | |
| FILE: csabste1-20.dgn | DN: TxDOT | CK: TxDOT | OW: TxDOT |
| ©TxDOT | APRIL 2019 | CONTRACT | SECTION |
| 0914 | 05 | 204, ETC. | CR 118 |
| 02-20: Added Option 2. | DIST | COUNTY | SHEET NO. |
| AUS | WILLIAMSON | | 90 |

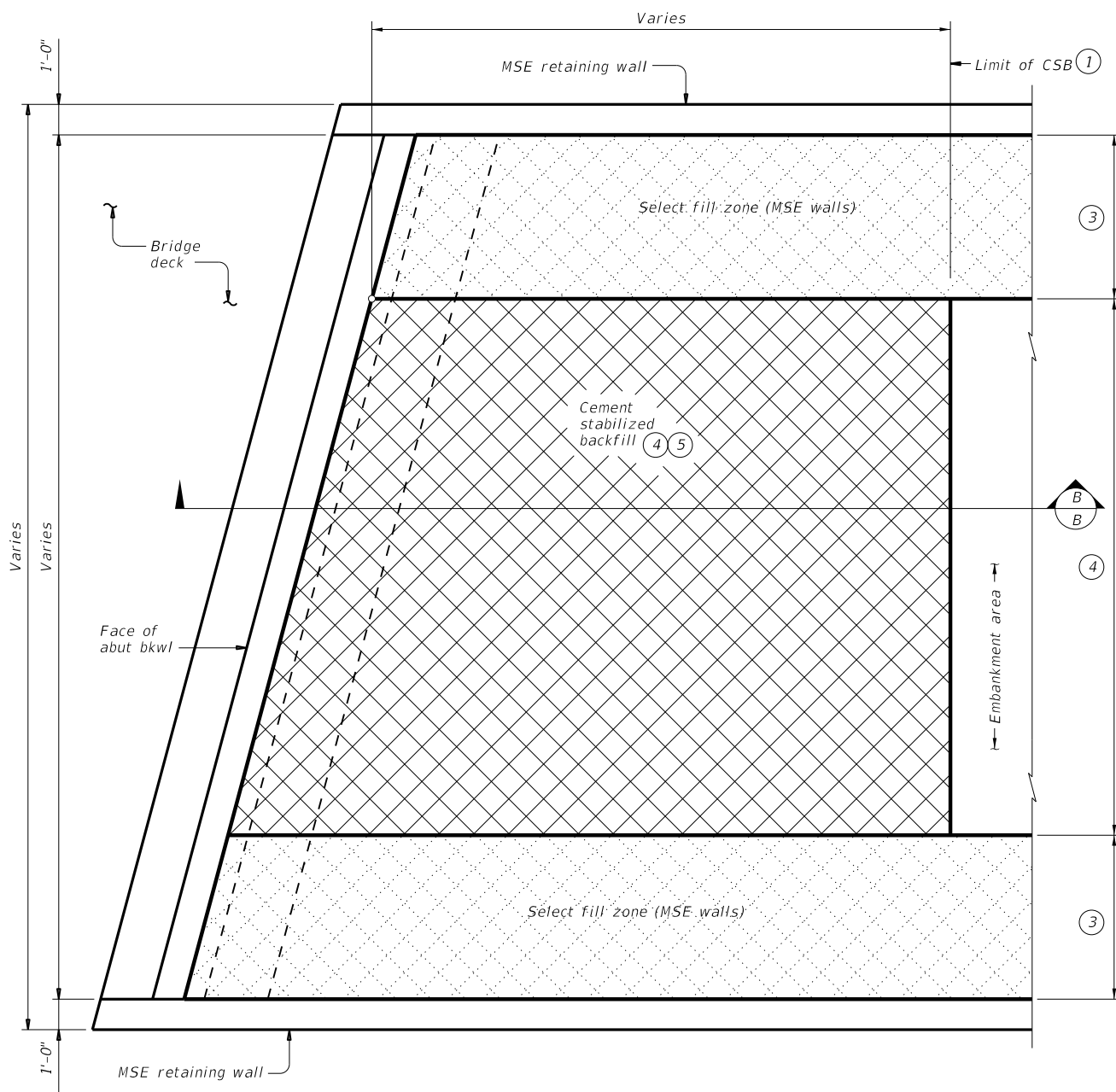
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DATE: 11/3/2022 12:56:56 PM
 FILE: c:\pwworking\10222653\csabste1-20 (1).dgn



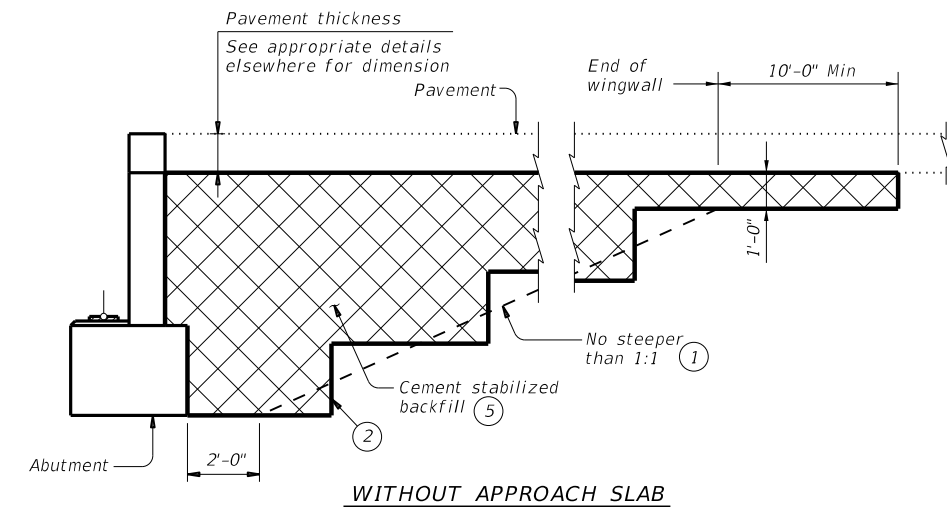
OPTION 2 ~ PLAN WITH WINGWALLS

Cast-in-place retaining walls similar.

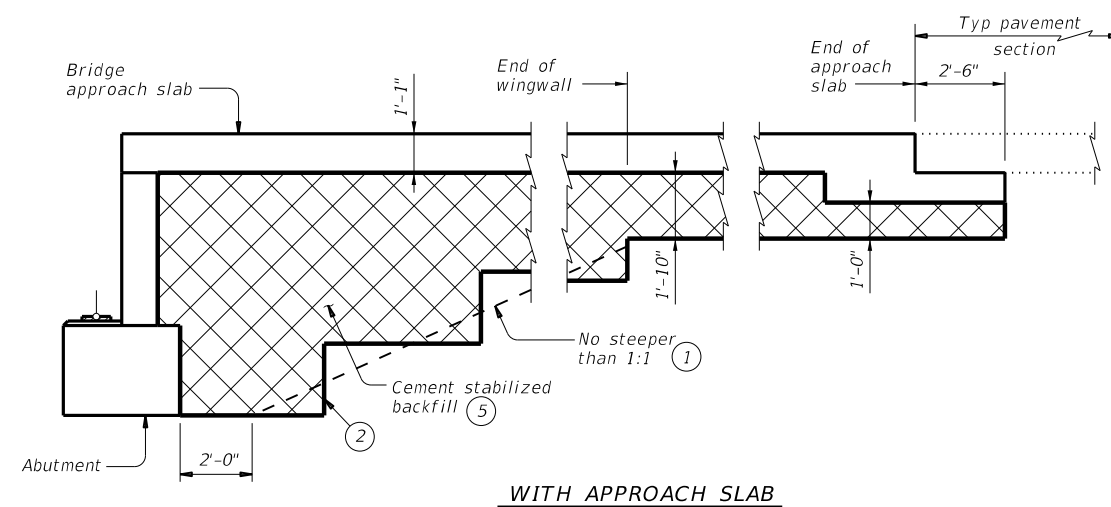


OPTION 2 ~ PLAN WITH MSE RETAINING WALLS

- ① Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.
- ② Bench backfill as shown with 12" (approximate) bench depths.
- ③ Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.
- ④ When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.
- ⑤ If shown in the plans flowable backfill can be used as a substitute for cement stabilized backfill with the following constraints:
 - a) If flowable backfill is to be placed over MSE backfill then a filter fabric will be placed over the MSE backfill prior to placement of the flowable fill; and
 - b) Place flowable fill in lifts not exceeding 2 feet in height, place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowability).



WITHOUT APPROACH SLAB



SECTION B-B

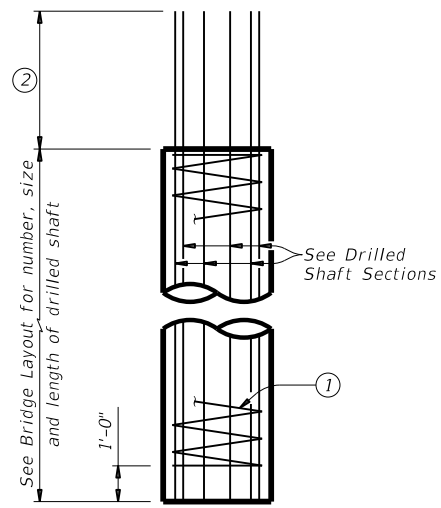
WITH APPROACH SLAB
 (Showing BAS-C, BAS-A similar.)

SHEET 2 OF 2

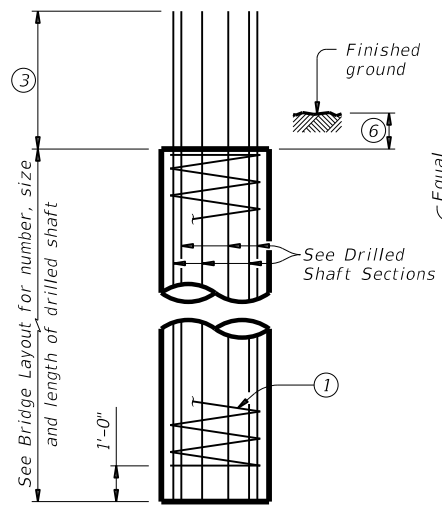
| | | | |
|--|-----------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| CEMENT STABILIZED ABUTMENT BACKFILL BRIDGE ABUTMENT | | | |
| CSAB | | | |
| FILE: csabste1-20.dgn | DN: TxDOT | CK: TxDOT | OW: TxDOT |
| ©TxDOT April 2019 | CONT SECT | JOB | HIGHWAY |
| 0914 05 | 204, ETC. | CR 118 | |
| 02-20: Added Option 2. | DIST | COUNTY | SHEET NO. |
| | AUS | WILLIAMSON | 91 |

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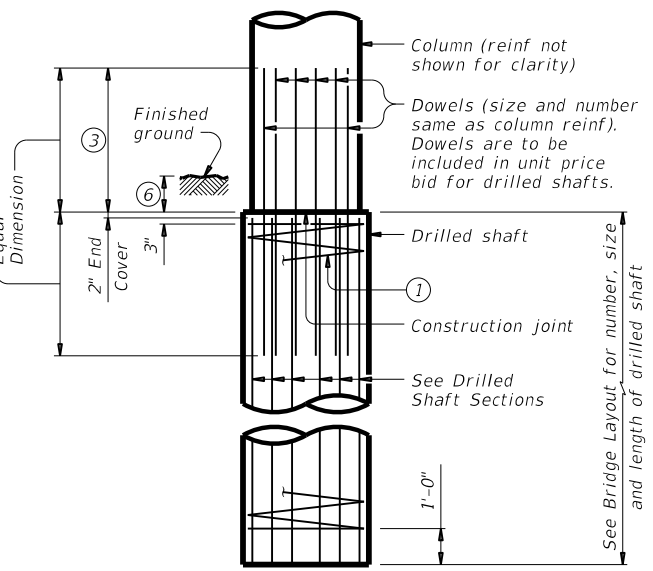
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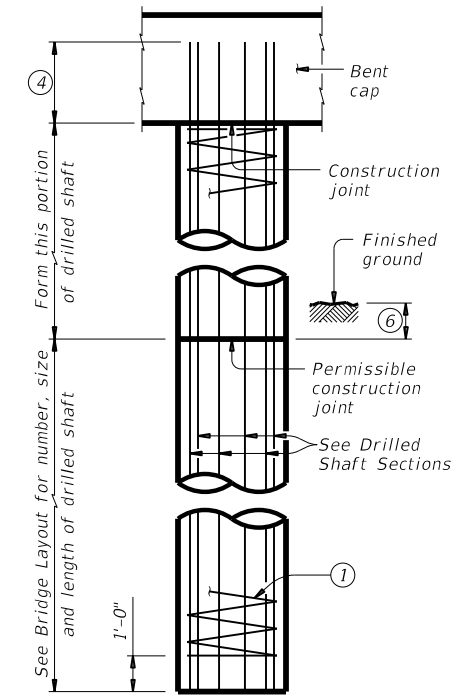
ABUTMENTS, WINGWALLS AND MULTI-DRILLED SHAFT FOOTINGS



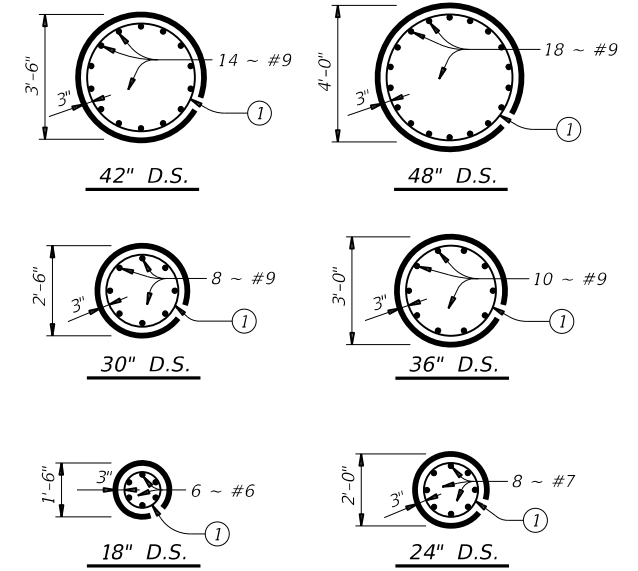
INTERIOR BENTS DRILLED SHAFT DIA EQUAL TO COLUMN DIA



INTERIOR BENTS DRILLED SHAFT DIA GREATER THAN COLUMN DIA



OPTIONAL INTERIOR BENT DRILLED SHAFT DETAIL

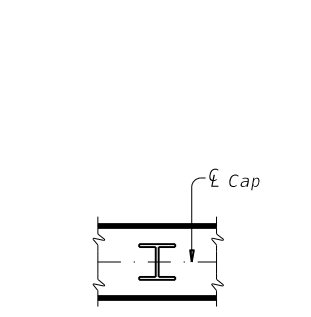


DRILLED SHAFT SECTIONS

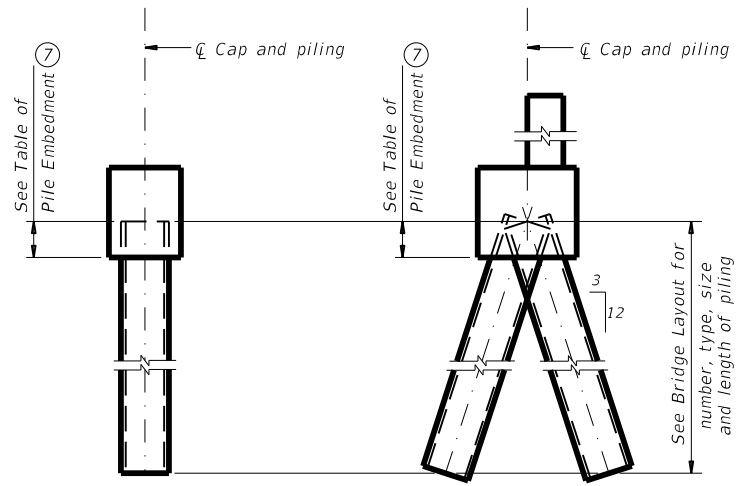
DRILLED SHAFT DETAILS

| TABLE OF PILE EMBEDMENT | |
|--|----------------------|
| Pile Type | Embedment Depth (Ft) |
| 16" Sq Concrete 18" Sq Concrete HP14 Steel HP16 Steel | 1'-0" |
| 20" Sq Concrete 24" Sq Concrete HP18 Steel | 1'-6" |

See Prestressed Concrete Piling (CP) standard for additional details on concrete pile embedment.

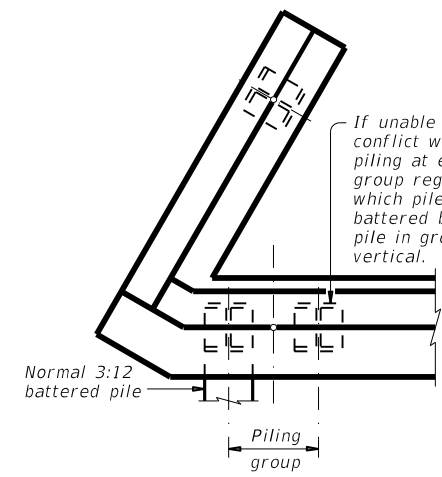


ORIENTATION OF STEEL H-PIILING



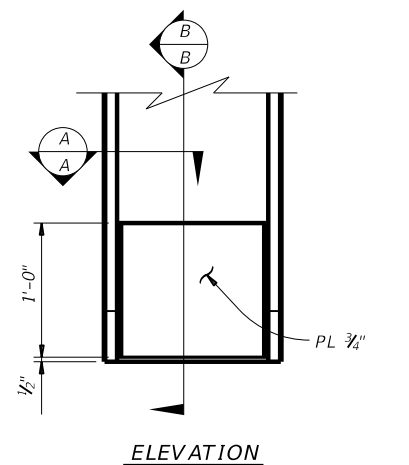
VERTICAL PILE BATTERED PILE

PIILING DETAILS
(Concrete or steel H)

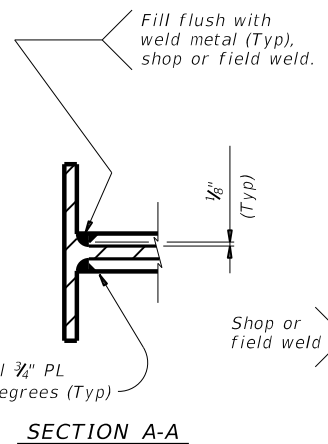


DETAIL "A"
(Showing plan view of a 30° skewed abutment)

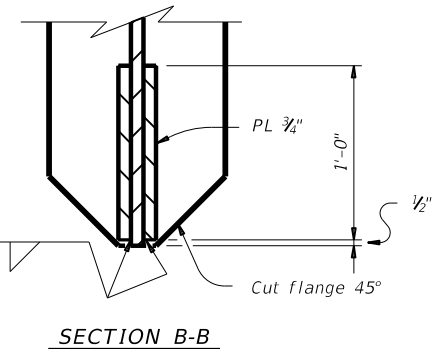
- ① #3 spiral at 6" pitch (one and a half flat turns top and bottom).
- ② Min extension into supported element:
#6 Bars = 1'-11"
#7 Bars = 2'-0"
#9 Bars = 2'-3"
- ③ Min lap with column reinf:
#7 Bars = 2'-11"
#9 Bars = 3'-9"
#11 Bars = 4'-8"
- ④ Min extension into supported element:
#6 Bars = 1'-11"
#7 Bars = 2'-3"
#9 Bars = 2'-9"
- ⑤ Drilled shafts may extend to the bottom of bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the drilled shaft diameter equals the column diameter. Obtain approval of the forming method above the ground line prior to construction. No adjustments in payment will be made if this option is used.
- ⑥ 1'-0" Min, unless shown otherwise on plans.
- ⑦ Or as shown on plans.



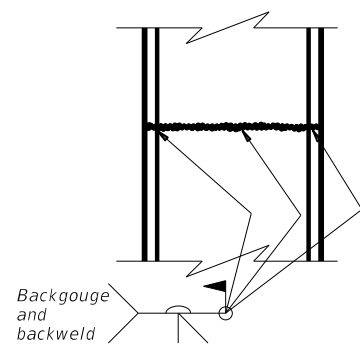
ELEVATION



SECTION A-A



SECTION B-B



SECTION THRU FLANGE OR WEB

STEEL H-PILE SPLICE DETAIL

Use when required.

STEEL H-PILE TIP REINFORCEMENT

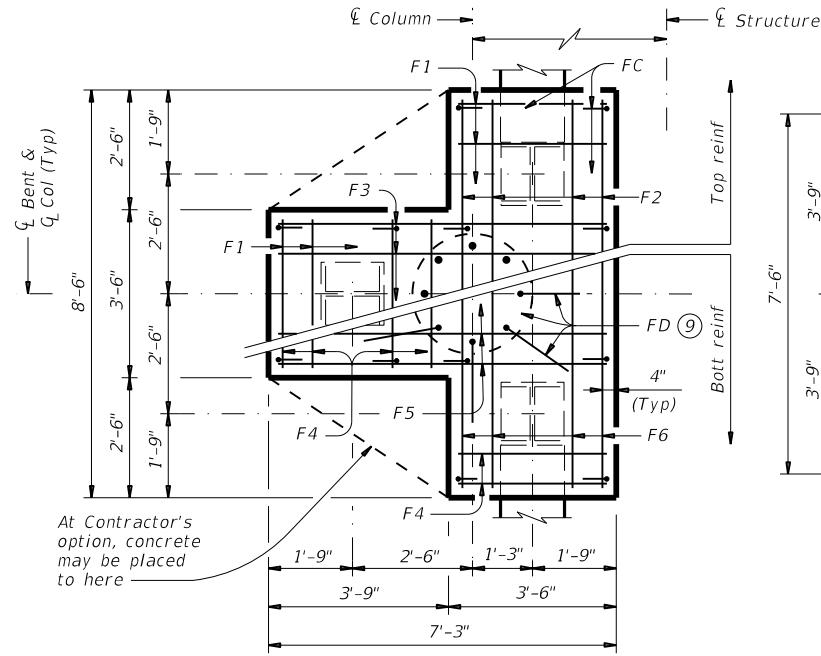
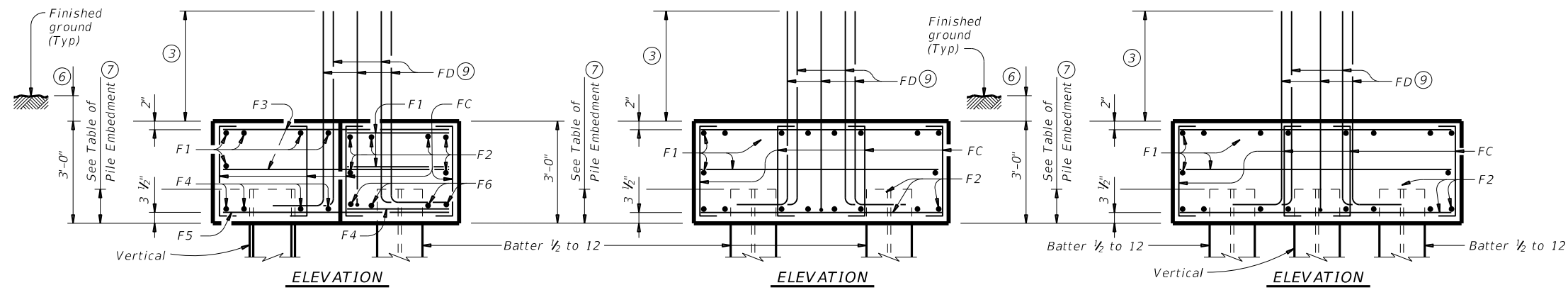
See Item 407 "Steel Piling" to determine when tip reinforcement is required and for options to the details shown.

SHEET 1 OF 2

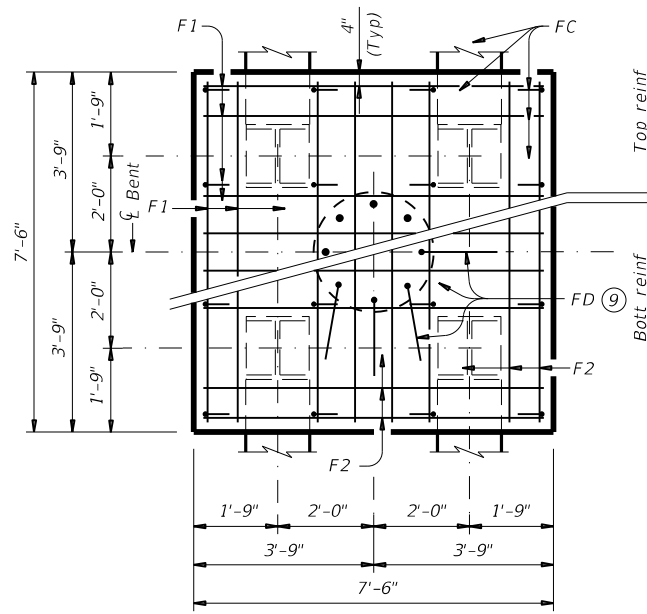
| | | | |
|---------------------------------------|------------|--------------------------|-----------|
| | | Bridge Division Standard | |
| COMMON FOUNDATION DETAILS | | | |
| FD | | | |
| FILE: fdstd01-20.dgn | DN: TxDOT | CK: TxDOT | DW: TxDOT |
| ©TxDOT | REVISIONS | CONTRACT | JOB |
| 01-20: Added #11 bars to the FD bars. | 0914 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 92 | |

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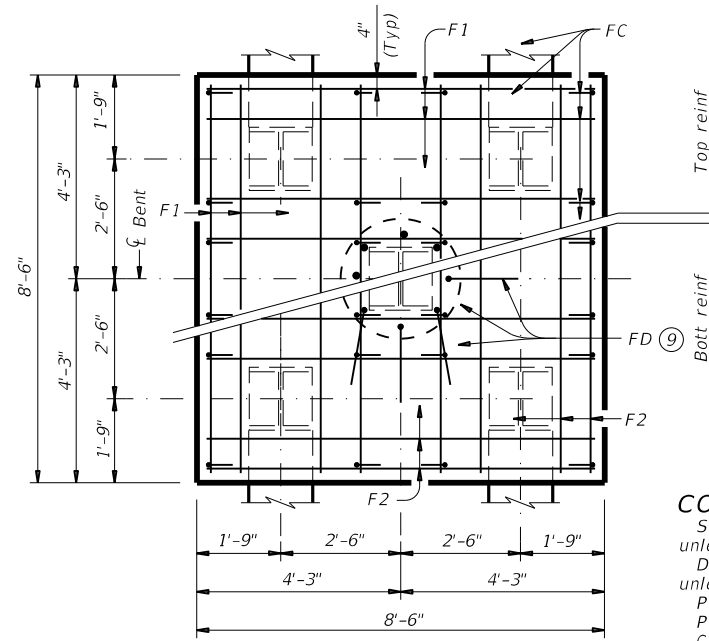
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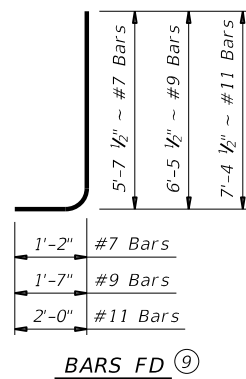
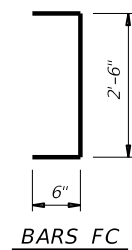
THREE PILE FOOTING^⑧
 For 36" Dia and smaller columns.



FOUR PILE FOOTING^⑧
 For 42" Dia and smaller columns.



FIVE PILE FOOTING^⑧
 For 42" Dia and smaller columns.



- ③ Min lap with column reinforcing:
 #7 Bars = 2'-11"
 #9 Bars = 3'-9"
 #11 Bars = 4'-8"
- ⑥ 1'-0" Min, unless shown otherwise on plans.
- ⑦ Or as shown on plans.
- ⑧ See Bridge Layout for type, size and length of piling.
- ⑨ Number and size of FD bars must match column reinforcing. Tie FD bars to the top of the bottom reinforcing mat.
- ⑩ Adjust FD quantity, size and weight as needed to match column reinforcing.

TABLE OF FOOTING QUANTITIES FOR 30" COLUMNS

| ONE 3 PILE FOOTING | | | | |
|--------------------|-----|------|---------|--------|
| Bar | No. | Size | Length | Weight |
| F1 | 11 | #4 | 3'- 2" | 23 |
| F2 | 6 | #4 | 8'- 2" | 33 |
| F3 | 6 | #4 | 6'- 11" | 28 |
| F4 | 8 | #9 | 3'- 2" | 86 |
| F5 | 4 | #9 | 6'- 11" | 94 |
| F6 | 4 | #9 | 8'- 2" | 111 |
| FC | 12 | #4 | 3'- 6" | 28 |
| FD ^⑩ | 8 | #9 | 8'- 1" | 220 |

| | | |
|--------------------|----|-----|
| Reinforcing Steel | Lb | 623 |
| Class "C" Concrete | CY | 4.8 |

| ONE 4 PILE FOOTING | | | | |
|--------------------|-----|------|--------|--------|
| Bar | No. | Size | Length | Weight |
| F1 | 20 | #4 | 7'- 2" | 96 |
| F2 | 16 | #8 | 7'- 2" | 306 |
| FC | 16 | #4 | 3'- 6" | 37 |
| FD ^⑩ | 8 | #9 | 8'- 1" | 220 |

| | | |
|--------------------|----|-----|
| Reinforcing Steel | Lb | 659 |
| Class "C" Concrete | CY | 6.3 |

| ONE 5 PILE FOOTING | | | | |
|--------------------|-----|------|--------|--------|
| Bar | No. | Size | Length | Weight |
| F1 | 20 | #4 | 8'- 2" | 109 |
| F2 | 16 | #9 | 8'- 2" | 444 |
| FC | 24 | #4 | 3'- 6" | 56 |
| FD ^⑩ | 8 | #9 | 8'- 1" | 220 |

| | | |
|--------------------|----|-----|
| Reinforcing Steel | Lb | 829 |
| Class "C" Concrete | CY | 8.0 |

CONSTRUCTION NOTES:

- See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise.
- Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile unless shown otherwise.
- Provide Class C Concrete ($f'_c = 3,600$ psi), unless shown otherwise.
- Provide Grade 60 reinforcing steel.
- Galvanize reinforcing if shown elsewhere in the plans.
- Provide bar laps for drilled shaft reinforcing, where required, as follows:
 Uncoated or galvanized (#6) ~ 2'-6"
 Uncoated or galvanized (#7) ~ 2'-11"
 Uncoated or galvanized (#9) ~ 3'-9"

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise.
 Reinforcing bar dimensions shown are out-to-out of bar.

DESIGNER NOTES:

- Do not use the drilled shaft details shown on this standard for retaining wall, noise wall, barrier, or sign foundations without structural evaluation.
- Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.
- Maximum allowable pile loads for the footings shown are:
 72 Tons/Pile with 24" Dia Columns
 80 Tons/Pile with 30" Dia Columns
 100 Tons/Pile with 36" Dia Columns
 120 Tons/Pile with 42" Dia Columns

SHEET 2 OF 2



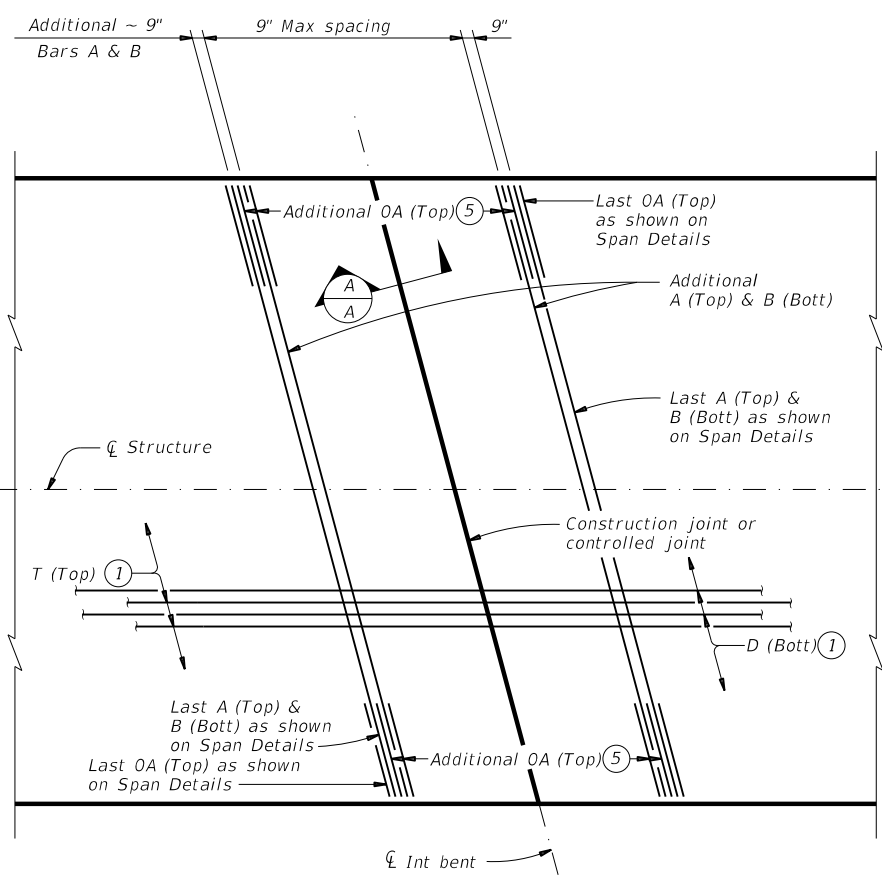
COMMON FOUNDATION DETAILS

FD

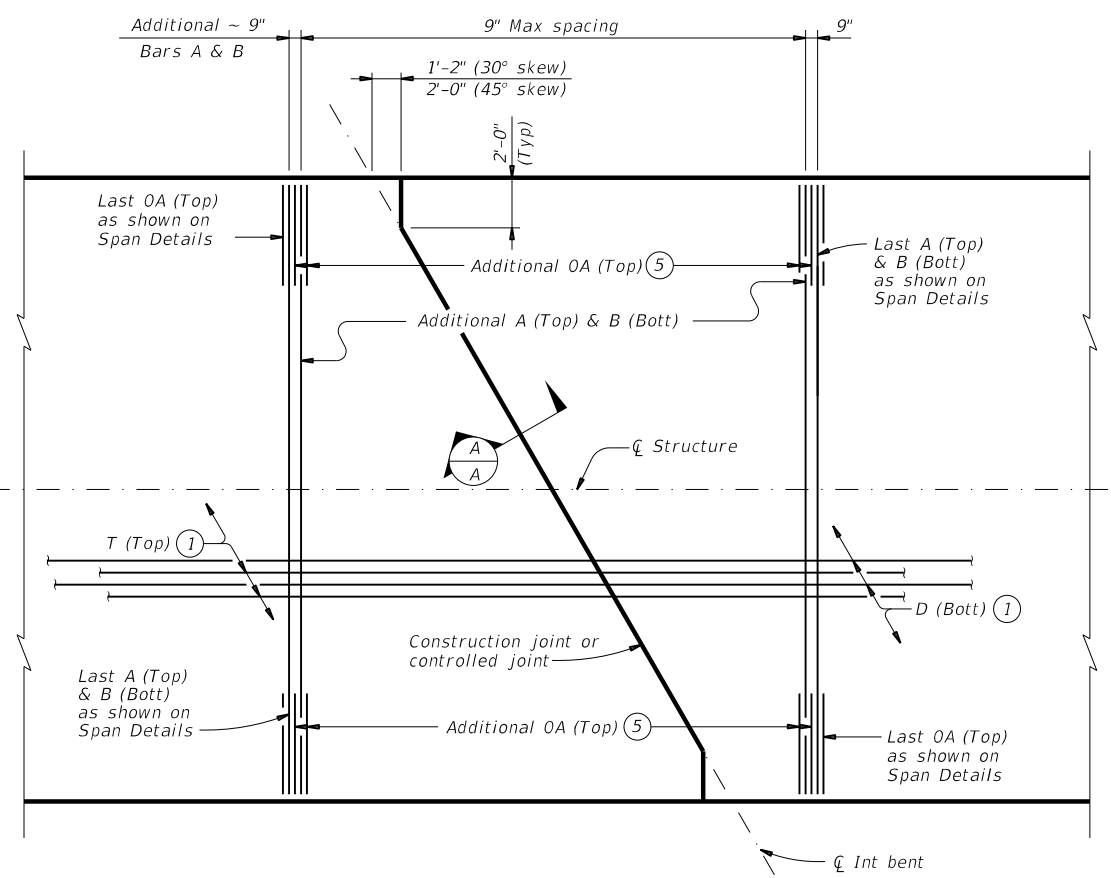
| | | | | |
|---------------------------------------|-----------|------------|-----------|-----------|
| FILE: fdstd01-20.dgn | DN: TxDOT | CK: TxDOT | DW: TxDOT | CR: TxDOT |
| ©TxDOT April 2019 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 01-20: Added #11 bars to the FD bars. | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 93 | |

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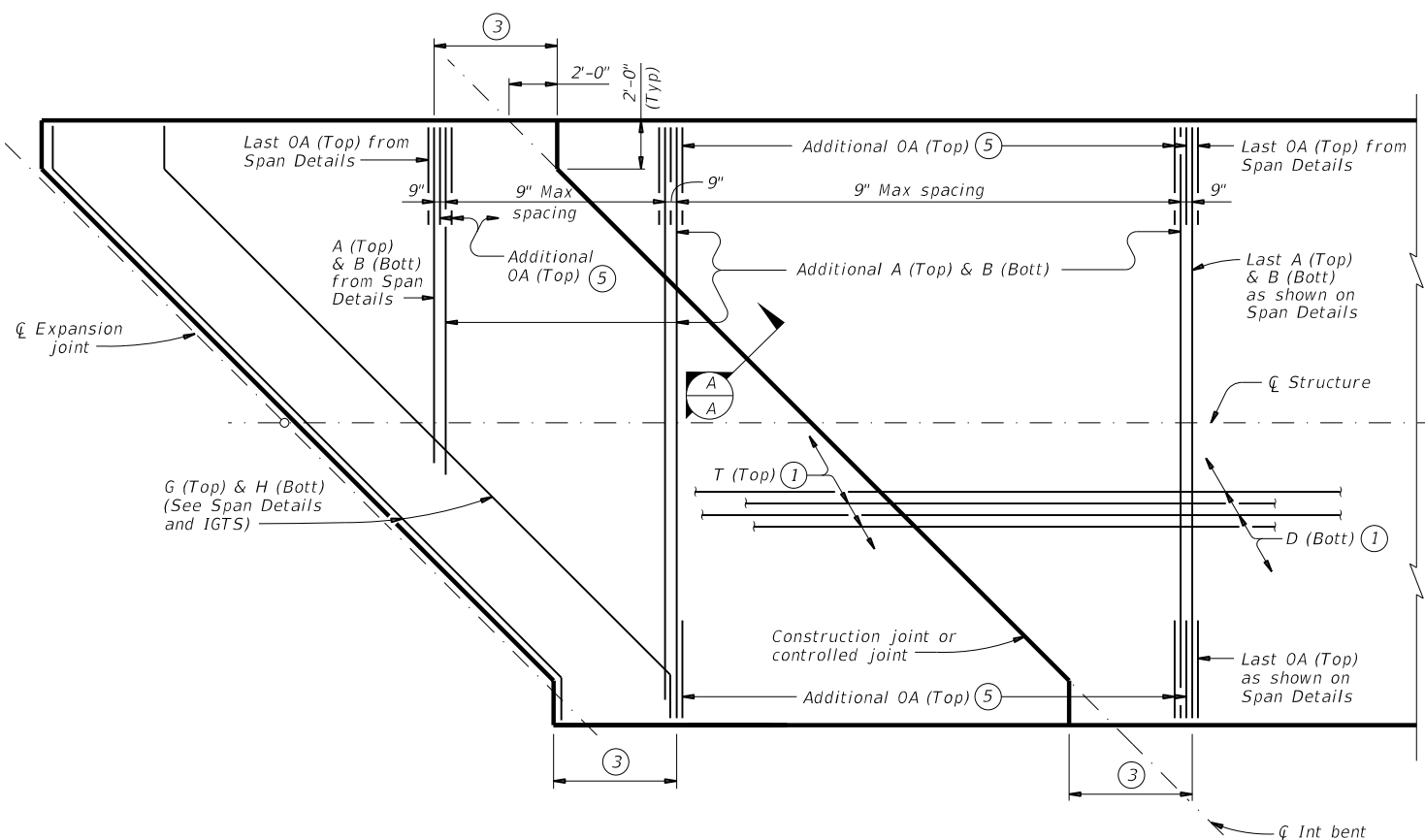
DATE: 11/3/2022 12:57:11 PM
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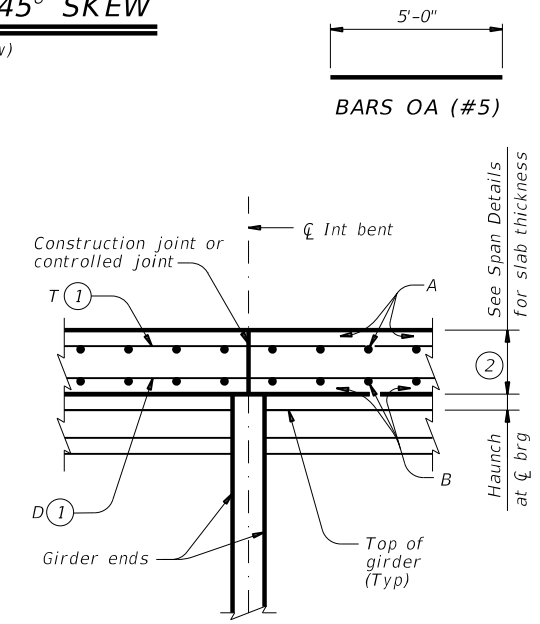
PLAN FOR 0° OR 15° SKEW
 (Showing 15° skew)



PLAN FOR 30° OR 45° SKEW
 (Showing 30° skew)



PLAN FOR 45° SKEW
 (Showing short span condition)



SECTION A-A
 Bars OA (Top) not shown for clarity.

- ① Top and bottom mats must be continuous through joint.
- ② Maintain a constant slab thickness over the bent.
- ③ 5'-4" as shown on Span Details.
- ④ Use these details when no full slab width bars A and B are shown on Span Details.
- ⑤ Bars OA (Top) at 9" Max spacing between Bars A (Top).
- ⑥ Values in table assume a temperature change of 70° F after erection when calculating thermal movement in one direction (not total).

TABLE OF ⑥ ALLOWABLE UNIT LENGTH

| Max Rdwy Grade, Percent | Unit Length Factor |
|-------------------------|--------------------|
| 0.00 | 4.1 |
| 1.00 | 3.9 |
| 2.00 | 3.7 |
| 3.00 | 3.5 |
| 4.00 | 3.3 |
| 5.00 | 3.1 |

Unit length must not exceed the length of the shortest end span times the Unit Length Factor shown in table or 400', whichever is less.

BAR TABLE

| BAR | SIZE |
|-----|------|
| A | #4 |
| B | #4 |
| D | #4 |
| T | #4 |
| OA | #5 |

The details shown on this sheet are applicable for two and three span units comprised of the same girder type. Units may be comprised of different span lengths. See "Table of Allowable Unit Length".

GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications.
 This standard is drawn showing right forward skew. See Bridge Layout for actual skew direction.

CONSTRUCTION NOTES:
 Where multi-span units are indicated on the Bridge Layout, the thickened slab end details and reinforcement shown on IGTS standard (Bars AA, G, H, J, K, and M) and on the Span Details will be omitted where slabs are continuous over interior bents. At these locations, the slab details and reinforcement will be as shown on this sheet or on PCP standard (if using this option).
 Thickened slab end reinforcement and details still apply at expansion joint locations (ends of units).
 See Span Details for remainder of slab reinforcement and details.

MATERIAL NOTES:
 Provide Grade 60 reinforcing steel.
 Provide Class "S" concrete (f'c = 4,000 psi).
 Provide Class "S" (HPC) if shown elsewhere on the plans.
 Provide bar laps, where required, as follows:
 Uncoated ~ #4 = 1'-7"
 Epoxy Coated ~ #4 = 2'-5"

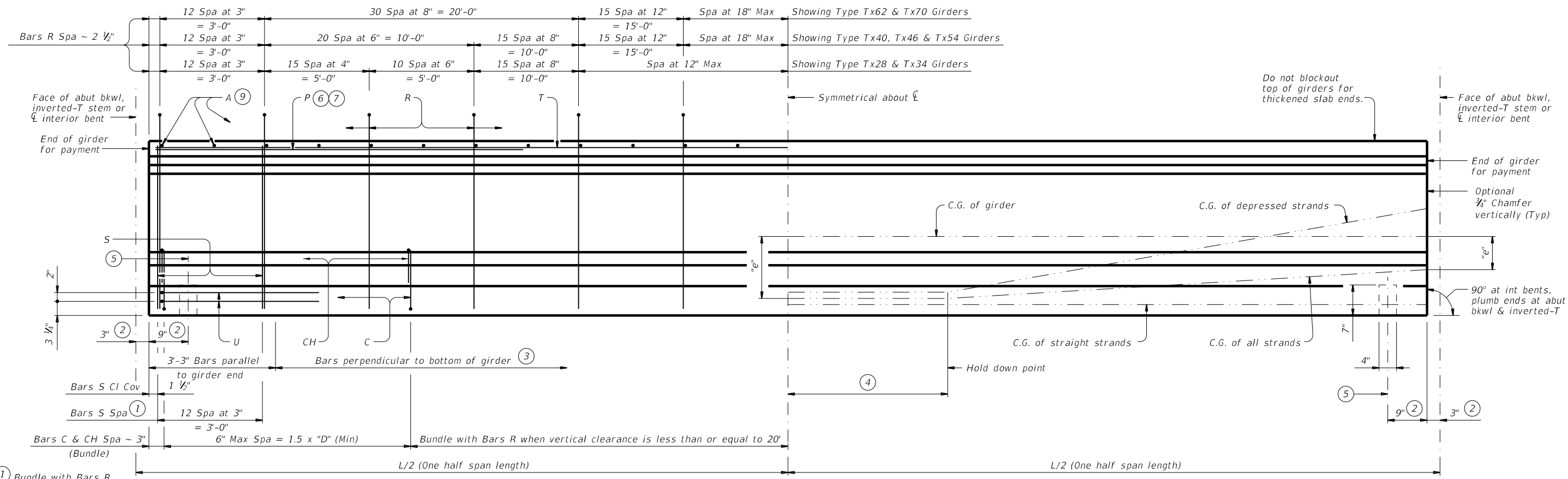
The details shown on this sheet are applicable for use only with the Prestressed Concrete I-Girder Standard Designs shown on standards IGSD-24, IGSD-28, IGSD-30, IGSD-32, IGSD-38, IGSD-40 and IGSD-44.

HL93 LOADING

| | | | |
|-----------------------------------|----------|--------------------------|-----------|
| | | Bridge Division Standard | |
| CONTINUOUS SLAB DETAILS | | | |
| PRESTR CONC I-GIRDER SPANS | | | |
| IGCS | | | |
| FILE: igs1sts-19.dgn | DN: JMH | CK: TxDOT | DW: JTR |
| ©TxDOT August 2017 | CONTRACT | SECTION | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| 10-19: Added bubble note 6. | DIST | COUNTY | SHEET NO. |
| | AUS | WILLIAMSON | 94 |

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- ① Bundle with Bars R.
- ② Measured along ξ Girder at interior bents; perpendicular to abutment bkwl or inverted-T stem.
- ③ The average of the top and bottom spacing of Bars R cannot exceed the required spacing.
- ④ L/20, but not less than 5'-0" (-0,+2').

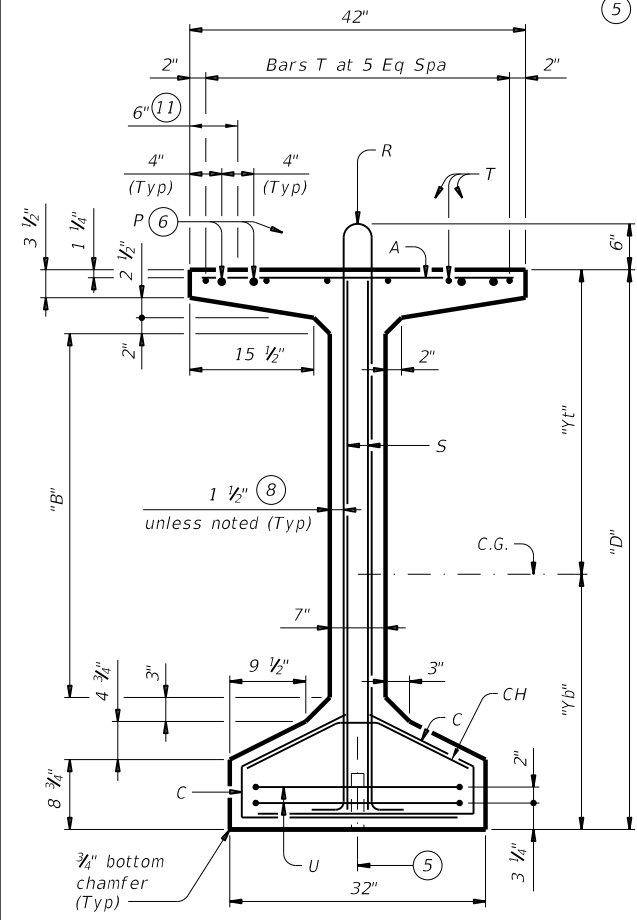
GIRDER ELEVATION

- ⑥ Bars P (#6 x 15'-0") required in Tx62 and Tx70 girders. At the fabricator's option bars larger than #6 may be used. When L is less than 50 ft, Bars P are to be the same length as Bars T.
- ⑦ Bars P (#6 x 15'-0") are only required in Tx28, Tx34, Tx40, Tx46, and Tx54 girders when "e" at girder ends exceeds 0.25 x "D". At the fabricator's option bars larger than #6 may be used. When L is less than 50 ft, Bars P are to be the same length as Bars T.
- ⑧ 1 3/8" Clear Cover to Bars S.
- ⑨ Space Bars A at 6" Max for girders requiring overhang bracket hangers. Space at 12" Max for all other girders. Tie to Bars R as necessary. See standard IGMS for "Deck Forming Notes".
- ⑩ Based on 155 pcf total weight of concrete and reinforcing steel.
- ⑪ Smooth trowel finish on the slab overhang side of exterior girder.

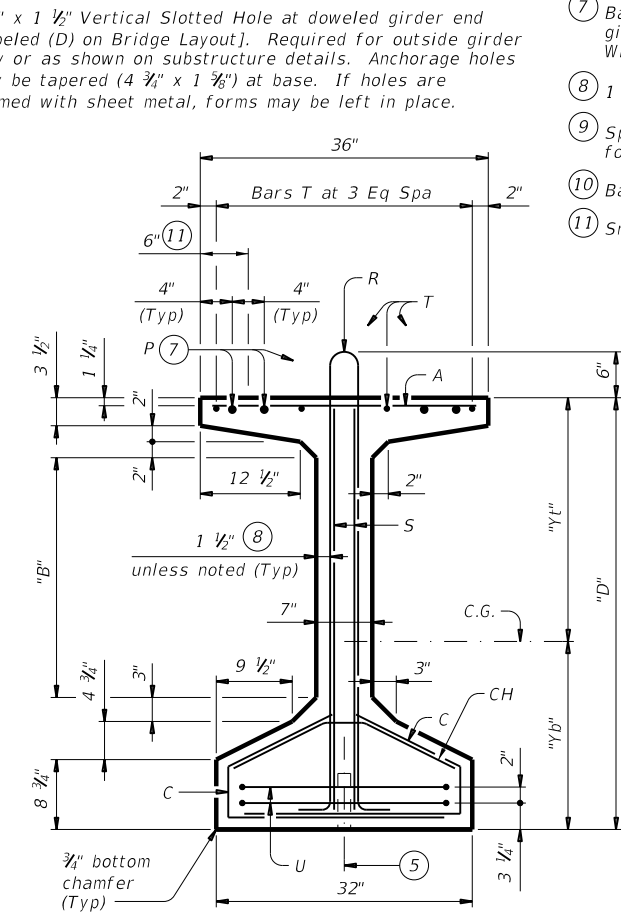
| GIRDER DIMENSIONS AND SECTION PROPERTIES | | | | | | | | |
|--|-------|---------|-------|-------|---------------------|---------------------|---------------------|-------------|
| Girder Type | "D" | "B" | "yt" | "yb" | Area | "Ix" | "Iy" | Weight (10) |
| | (in.) | (in.) | (in.) | (in.) | (in. ²) | (in. ⁴) | (in. ⁴) | (plf) |
| Tx28 | 28 | 6 | 15.02 | 12.98 | 585 | 52,772 | 40,559 | 630 |
| Tx34 | 34 | 12 | 18.49 | 15.51 | 627 | 88,355 | 40,731 | 675 |
| Tx40 | 40 | 18 | 21.90 | 18.10 | 669 | 134,990 | 40,902 | 720 |
| Tx46 | 46 | 22 | 25.90 | 20.10 | 761 | 198,089 | 46,478 | 819 |
| Tx54 | 54 | 30 | 30.49 | 23.51 | 817 | 299,740 | 46,707 | 880 |
| Tx62 | 62 | 37 1/2" | 33.72 | 28.28 | 910 | 463,072 | 57,351 | 980 |
| Tx70 | 70 | 45 1/2" | 38.09 | 31.91 | 966 | 628,747 | 57,579 | 1,040 |

GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications. Provide Class H concrete. Provide Grade 60 reinforcing steel. An equal area of deformed Welded Wire Reinforcement (WWR) (ASTM A1064) may be substituted for Bars A, C, R or T unless otherwise noted. It is permissible for bars or strands to come in contact with materials used in forming anchor holes.

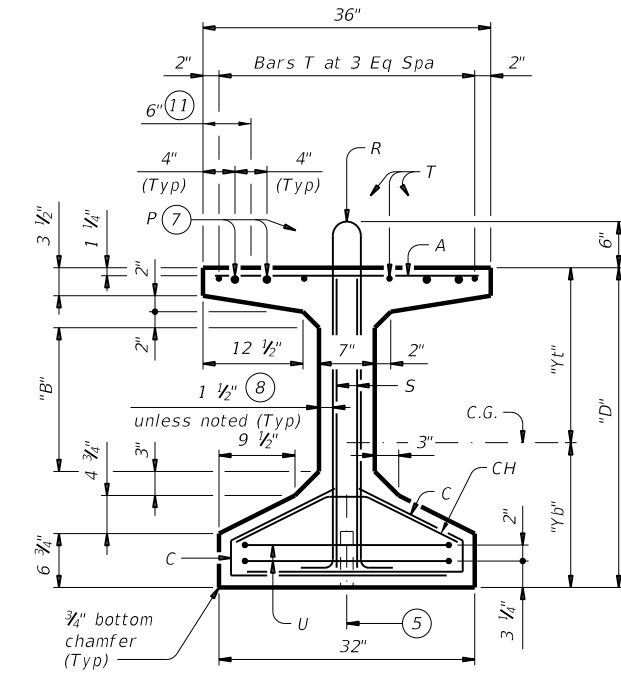
Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.



TYPE Tx62 & Tx70



TYPE Tx46 & Tx54



TYPE Tx28, Tx34 & Tx40



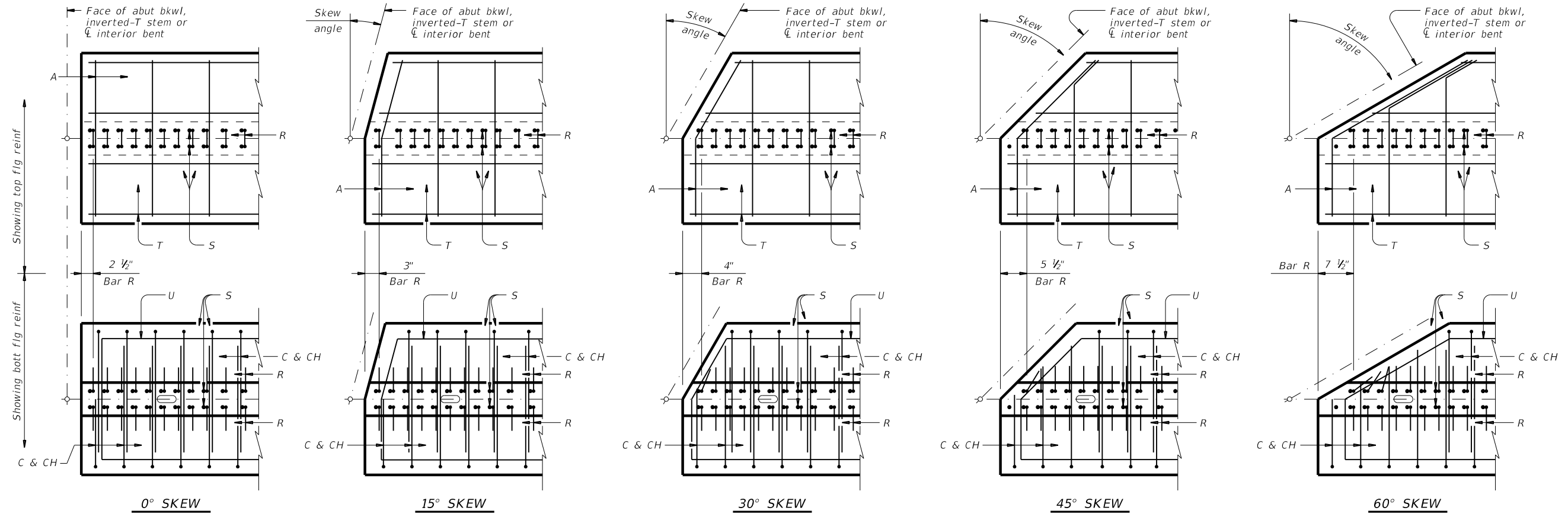
PRESTRESSED CONCRETE I-GIRDER DETAILS

IGD

| | | | | |
|---|-----------|------------|-----------|---------|
| FILE: igdstds1-19.dgn | DN: TxDOT | CK: JMH | DW: JTR | CK: TAR |
| ©TxDOT August 2017 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 10-19: Added Bars C and CH full length for VC < 20' | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 95 | |

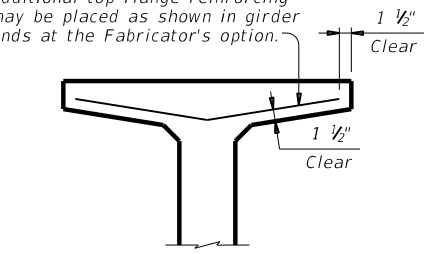
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DATE: 11/3/2022 12:57:18 PM
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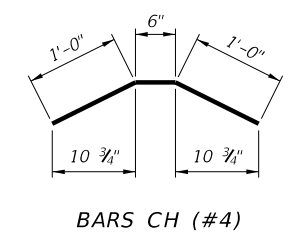


PLAN OF GIRDER ENDS (12)

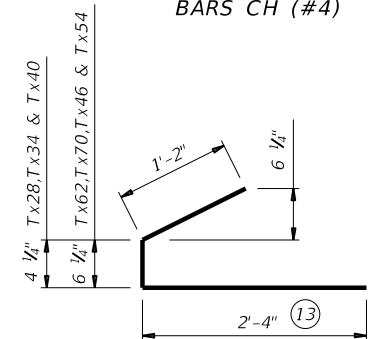
To control top flange cracking that may occur during form removal, additional top flange reinforcing may be placed as shown in girder ends at the Fabricator's option.



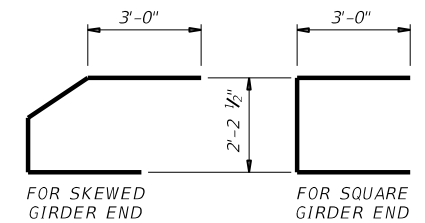
OPTIONAL TOP FLANGE REINFORCING DETAIL



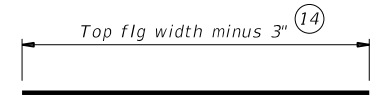
BARS CH (#4)



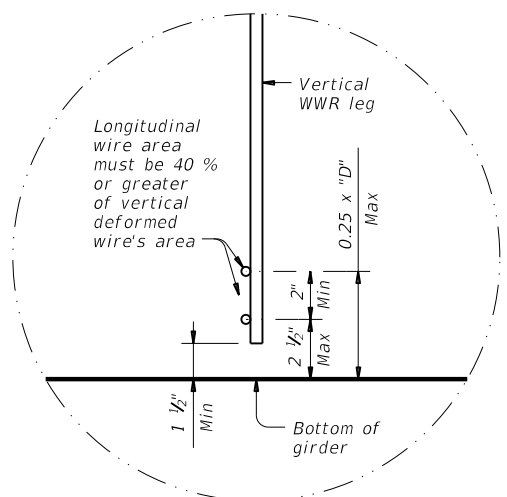
BARS C (#4)



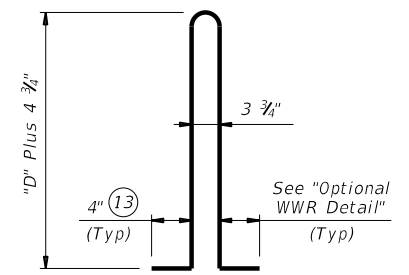
BARS U (#5)



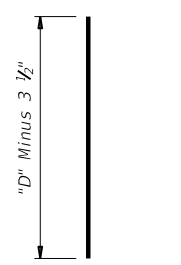
BARS A (#3)



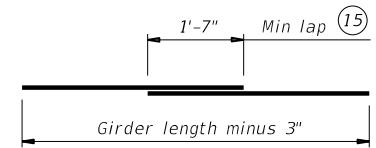
OPTIONAL WELDED WIRE REINFORCEMENT (WWR) DETAIL



BARS R (#4) (16)



BARS S (#6)



BARS T (#4)

- (12) Reinforcing patterns shown are provided as guides to determine reinforcement placement in skewed ends. Place Bars S as close to girder end as cover requirements permit, which may prevent them to be bundled with Bars R.
- (13) Bars may be cut or bent at skewed end as required.
- (14) Increase as necessary for bars at skewed end.
- (15) No portion of bar less than 10 ft.
- (16) For Welded Wire Reinforcement (WWR) option, area of Bars R may be reduced in proportion to the increase in reinforcement yield strength over 60 ksi. Yield strength of WWR is limited to 75 ksi.



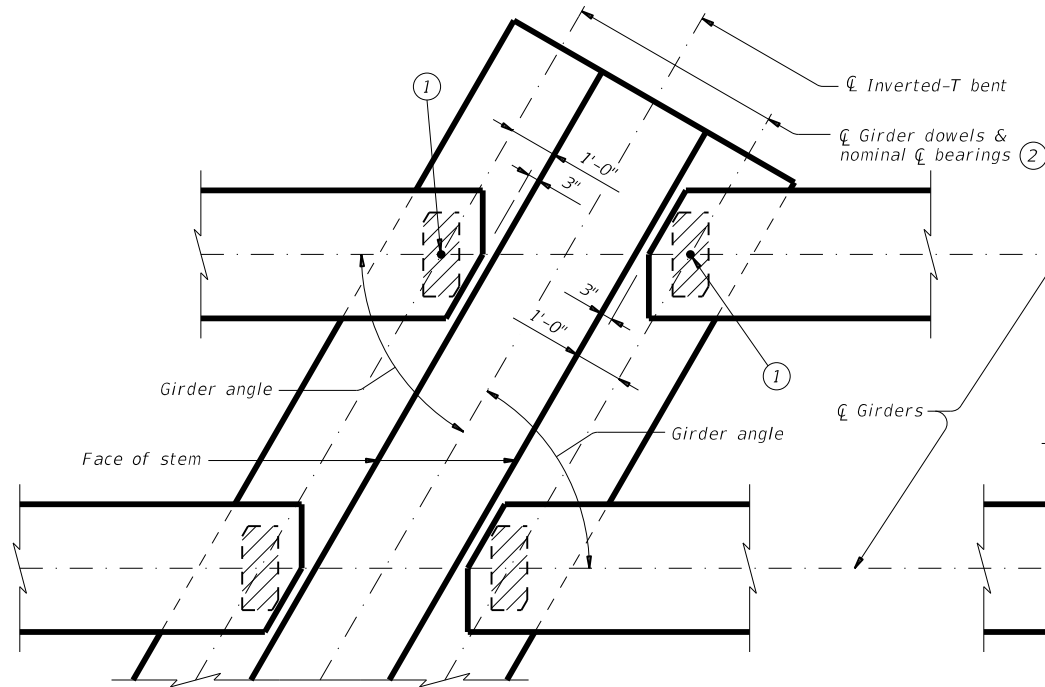
PRESTRESSED CONCRETE I-GIRDER DETAILS

IGD

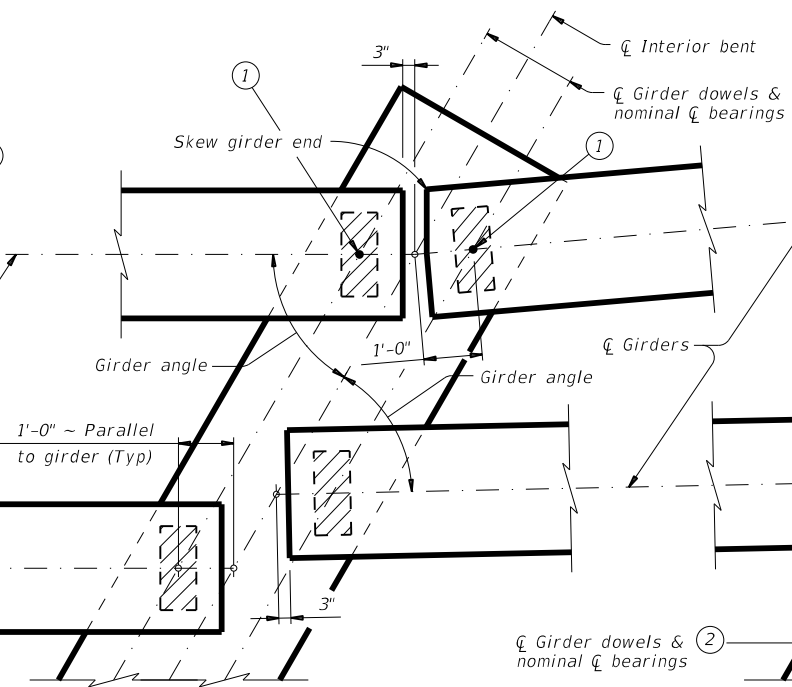
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| FILE: igdstds1-19.dgn | DN: TxDOT | CK: JMH | DW: JTR | CK: TAR |
| ©TxDOT August 2017 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 10-19: Added Bars C and CH full length for VC <= 20' | DIST | COUNTY | SHEET NO. | |
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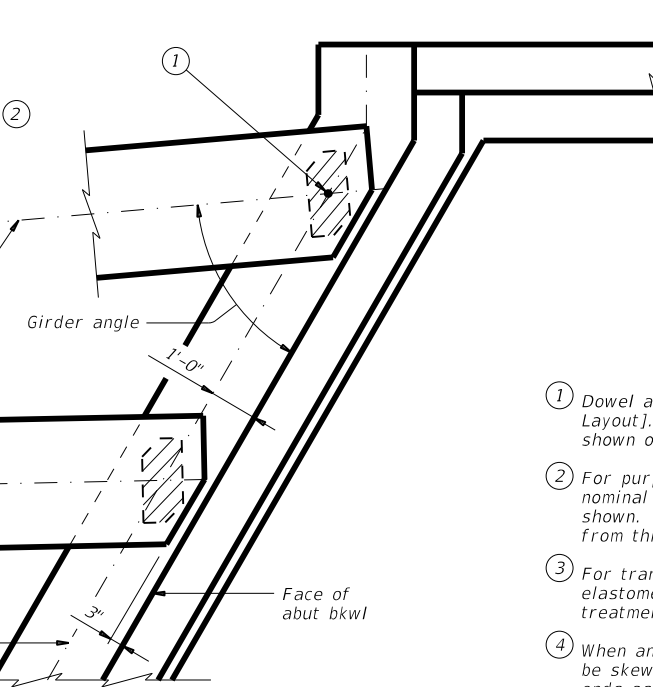
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AT INVERTED-T BENT W/SKEW

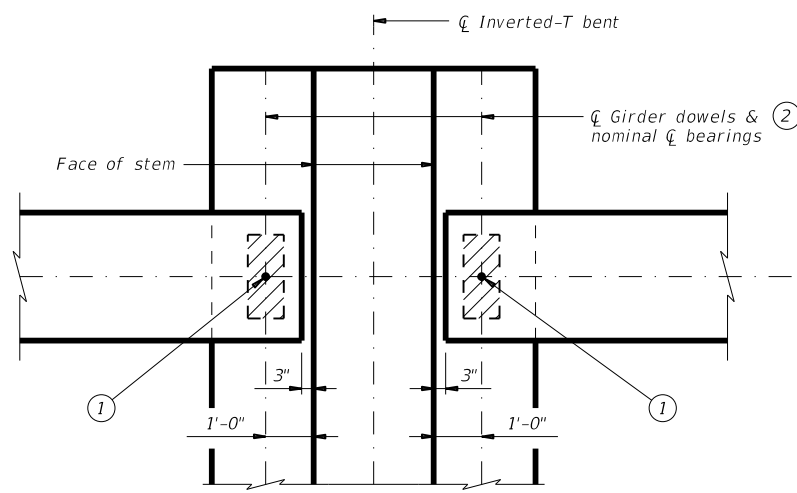


AT CONVENTIONAL INTERIOR BENT W/SKEW

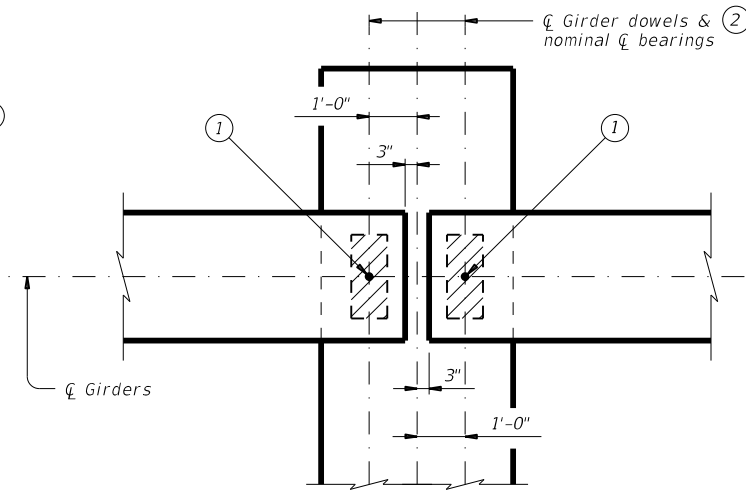


AT ABUTMENT W/SKEW³

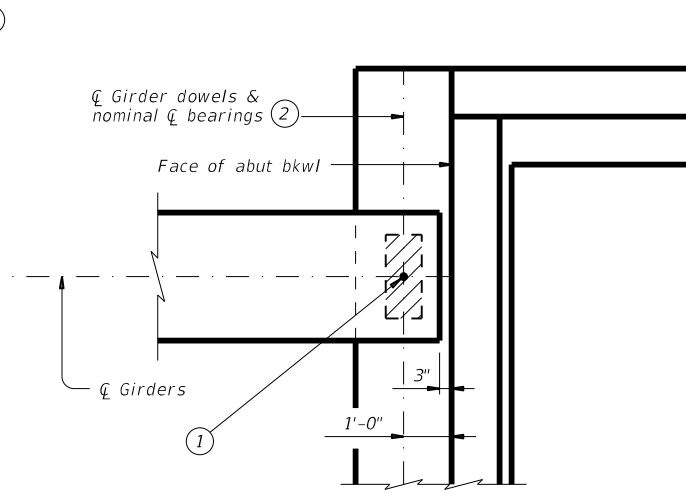
- ① Dowel at doweled girder end [labeled (D) on Bridge Layout]. Required for outside girder only or as shown on substructure details.
- ② For purposes of computing bearing seat elevations, nominal centerline of bearing must be defined as shown. The actual center of bearing pad may vary from this line.
- ③ For transition bents with backwall, girder and elastomeric bearings must receive the same treatment as shown for abutments.
- ④ When angle exceeds 0°, one or both girder ends must be skewed to maintain the clearance between girder ends as shown in view.
- ⑤ See Table of Bearing Pad Dimensions for bearing size. Girder end skew angles in Table not applicable for this situation. Table reflects girder conflicts of this type on radial bents only.



AT INVERTED-T BENT



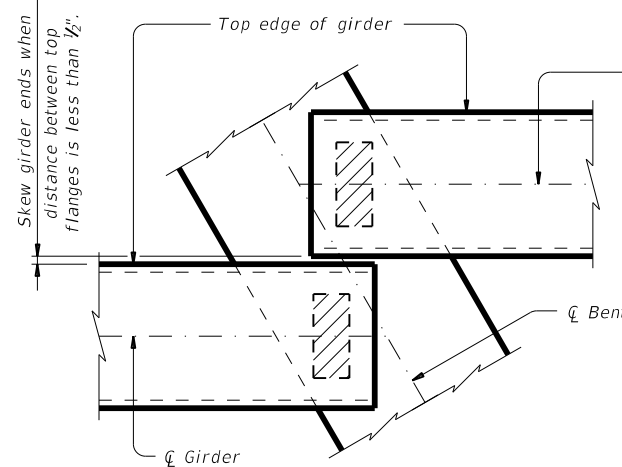
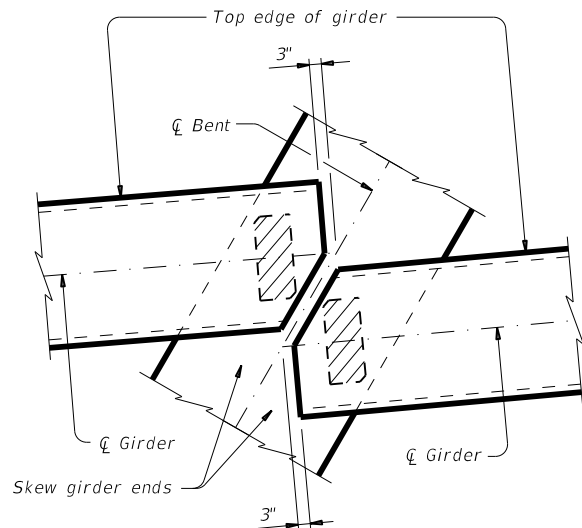
AT CONVENTIONAL INTERIOR BENT



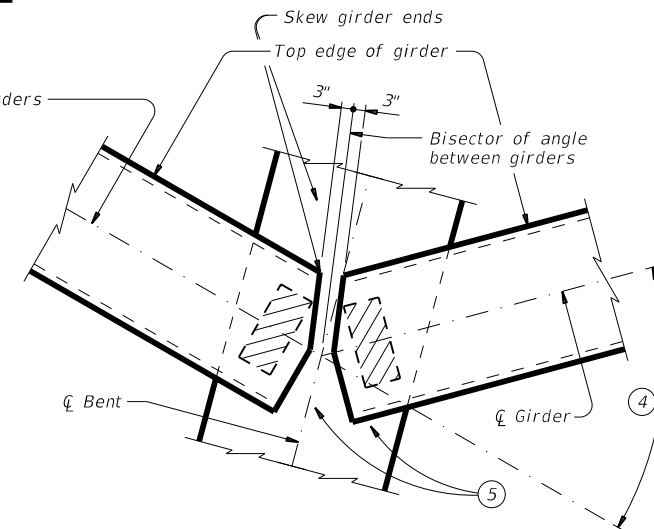
AT ABUTMENT³

GENERAL NOTES:
 These details accommodate skew angles up to 60°. Shop drawings for approval are required. A bearing layout which identifies location and orientation of all bearings must be developed by the bearing fabricator. Permanently mark each bearing in accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer. Cost of furnishing and installing elastomeric bearings, including beveled and embedded steel plates, must be included in unit price bid for "Prestressed Concrete Girders".

GIRDER END DETAILS



GIRDER CONFLICT DETAILS



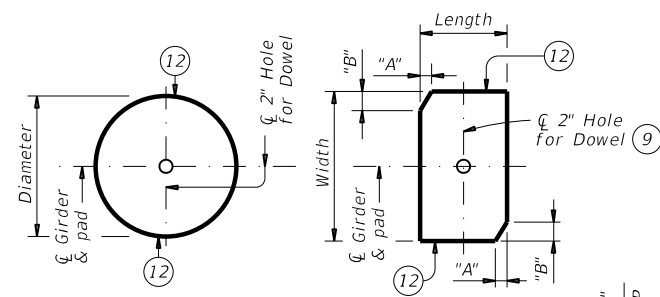
ELASTOMERIC BEARING AND GIRDER END DETAILS PRESTR CONCRETE I-GIRDERS

IGEB

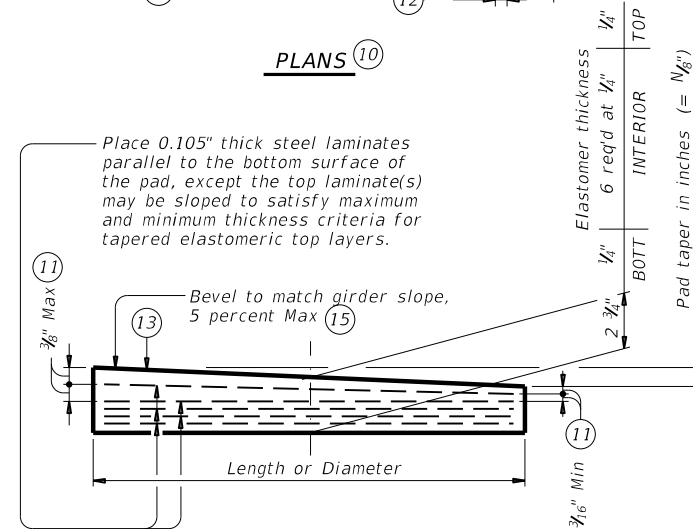
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| ©TxDOT August 2017 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | | SHEET NO. | |
| AUS | WILLIAMSON | | 97 | |

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PLANS (10)



ELEVATION

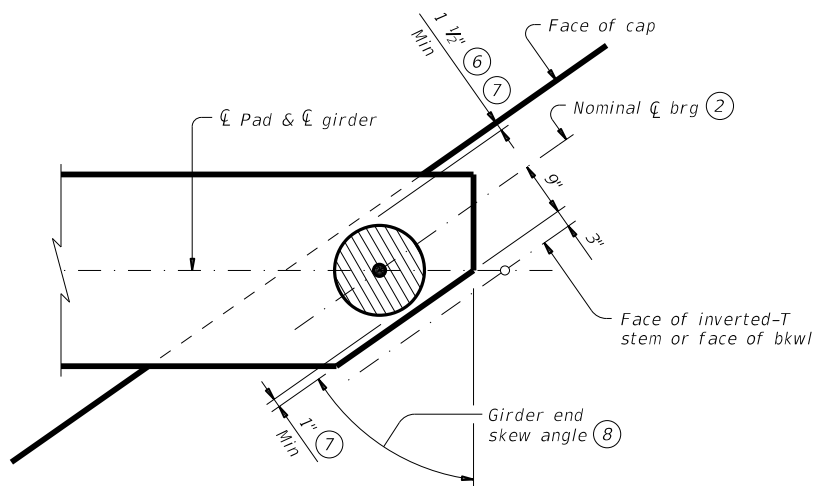
LAMINATED ELASTOMERIC BEARING PAD
 (50 DUROMETER)

TABLE OF MINIMUM SUBSTRUCTURE DIMENSIONS (14)

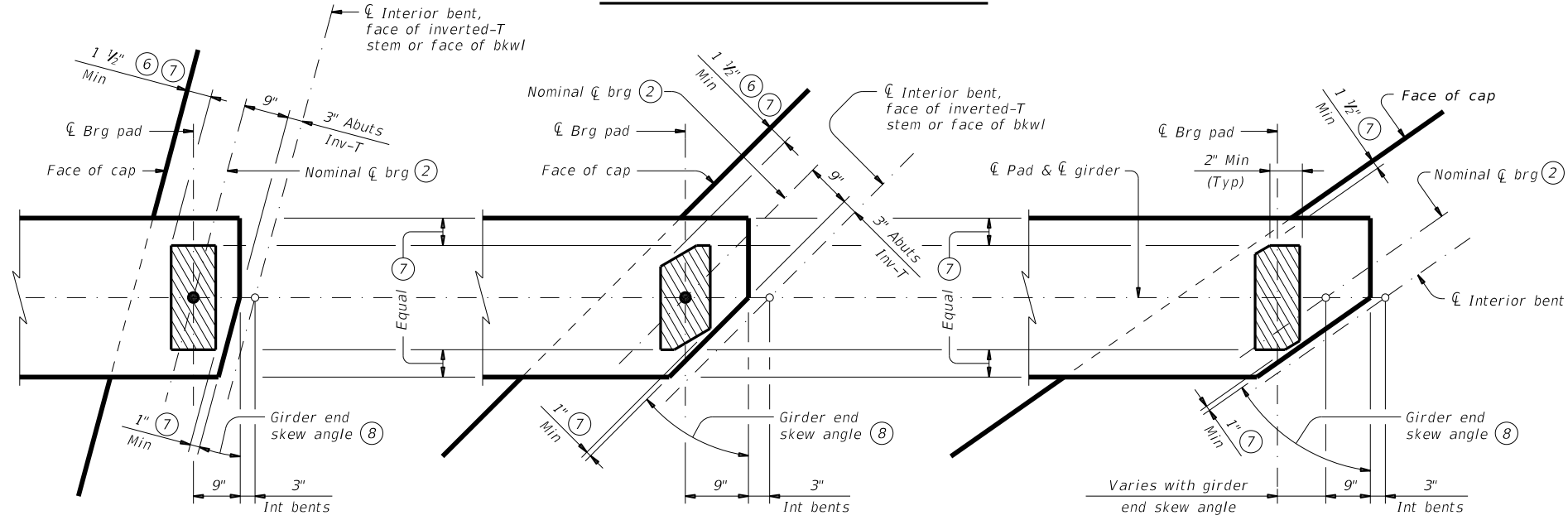
| Girder Type | Abutments | Int Bents | Inv-T Bents |
|----------------|-----------------------------|-------------------|--------------|
| | Face of Bkwl to Face of Cap | Overall Cap Width | Corbel Width |
| Tx28 thru Tx54 | 1'-9" | 3'-6" | 1'-10 1/2" |
| Tx62 & Tx70 | 2'-0" | 4'-0" | 2'-1 1/2" |

TABLE OF BEARING PAD DIMENSIONS

| Bent Type | Girder Type | Bearing Type (13) | Girder End Skew Angle Range | Pad Size Lgth x Wdth | Pad Clip Dimensions | |
|---|-------------------------------|-------------------|-----------------------------|----------------------|---------------------|--------|
| | | | | | "A" | "B" |
| ABUTMENTS, INVERTED-T AND TRANSITION BENTS WITH BACKWALLS | Tx28, Tx34, Tx40, Tx46 & Tx54 | G-1-"N" | 0° thru 21° | 8" x 21" | --- | --- |
| | | G-2-"N" | 21°+ thru 30° | 8" x 21" | 1 1/2" | 2 1/2" |
| | | G-3-"N" | 30°+ thru 45° | 9" x 21" | 4 1/2" | 4 1/2" |
| | | G-4-"N" | 45°+ thru 60° | 15" Dia | --- | --- |
| | Tx62 & Tx70 | G-5-"N" | 0° thru 21° | 9" x 21" | --- | --- |
| | | G-6-"N" | 21°+ thru 30° | 9" x 21" | 1 1/2" | 2 1/2" |
| | | G-7-"N" | 30°+ thru 45° | 10" x 21" | 4 1/2" | 4 1/2" |
| | | G-8-"N" | 45°+ thru 60° | 10" x 21" | 7 1/4" | 4 1/4" |
| CONVENTIONAL INTERIOR BENTS | Tx28, Tx34, Tx40, Tx46 & Tx54 | --- | --- | --- | --- | --- |
| | Tx62 & Tx70 | G-5-"N" | 0° thru 60° | 9" x 21" | --- | --- |
| CONVENTIONAL INTERIOR BENTS WITH SKEWED GIRDER ENDS (GIRDER CONFLICTS) (16) | Tx28, Tx34, Tx40, Tx46 & Tx54 | G-1-"N" | 0° thru 18° | 8" x 21" | --- | --- |
| | | G-2-"N" | 18°+ thru 30° | 8" x 21" | 1 1/2" | 2 1/2" |
| | | G-9-"N" | 30°+ thru 45° | 8" x 21" | 3" | 3" |
| | | G-10-"N" | 45°+ thru 60° | 9" x 21" | 6" | 3 1/2" |
| | Tx62 & Tx70 | G-5-"N" | 0° thru 18° | 9" x 21" | --- | --- |
| | | G-11-"N" | 18°+ thru 30° | 9" x 21" | --- | --- |
| G-12-"N" | 30°+ thru 45° | 9" x 21" | 1 1/2" | 1 1/2" | | |
| G-12-"N" | 45°+ thru 60° | 9" x 21" | 3" | 1 3/4" | | |



ROUND BEARINGS FOR SKEWED GIRDER ENDS AT FACE OF INVERTED-T STEM OR FACE OF BKWL



SKEWED GIRDER ENDS AT INT BENTS, FACE OF INVERTED-T STEM OR FACE OF BKWL

SKEWED GIRDER ENDS AT CONVENTIONAL INTERIOR BENTS (NO GIRDER DOWELS)

BEARING PAD PLACEMENT DIAGRAMS

- (2) For purposes of computing bearing seat elevations, nominal centerline of bearing must be defined as shown. The actual center of bearing pad may vary from this line.
- (6) 3" for inverted-T.
- (7) Place centerline pad as near nominal centerline bearing as possible between limits shown.
- (8) Girder end skew angle is equal to 90° minus the girder angle except at some conflicting girders.
- (9) Provide 2" dia hole only at locations required. See Substructure details for location.
- (10) See Table of Bearing Pad Dimensions for dimensions.
- (11) Maximum and minimum layer thicknesses shown are for elastomer only, on tapered layers.
- (12) Locate Permanent Mark here.
- (13) Indicate BEARING TYPE on all pads. For tapered pads, locate BEARING TYPE on the high side. The Fabricator must include the value of "N" (amount of taper in 1/8" increments) in this mark.
 Examples: N=0, (for 0" taper)
 N=1, (for 1/8" taper)
 N=2, (for 1/4" taper)
 (etc.)
 Fabricated pad top surface slope must not vary from plan girder slope by more than (0.0625" / Length or Dia) IN/IN.
- (14) Substructure dimensions must satisfy the minimums provided to accommodate the elastomeric bearings shown on this standard.
- (15) See sheet 3 of 3 for beveled plate use when slopes exceed 5 percent.
- (16) If girder end is skewed for a girder conflict at an interior bent and a beveled sole plate is required, use bearing type for abutments at this location. Location of bearing centerline is to be set as for abutments in this case.

HL93 LOADING SHEET 2 OF 3

Texas Department of Transportation
 Bridge Division Standard

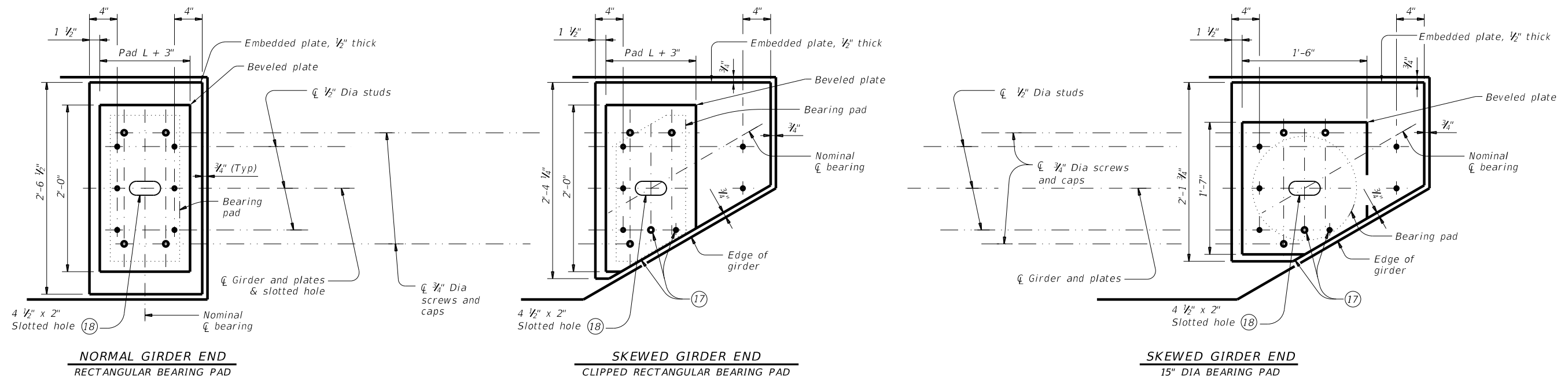
ELASTOMERIC BEARING AND GIRDER END DETAILS PRESTR CONCRETE I-GIRDERS

IGEB

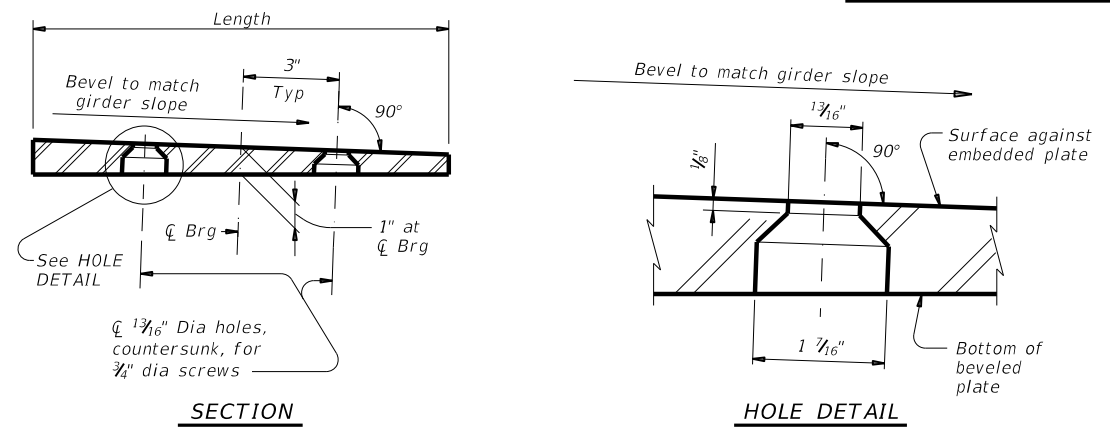
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| FILE: igebsts1-17.dgn | DN: AEE | CK: JMH | DW: JTR | CK: TxDOT |
| ©TxDOT August 2017 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 98 | |

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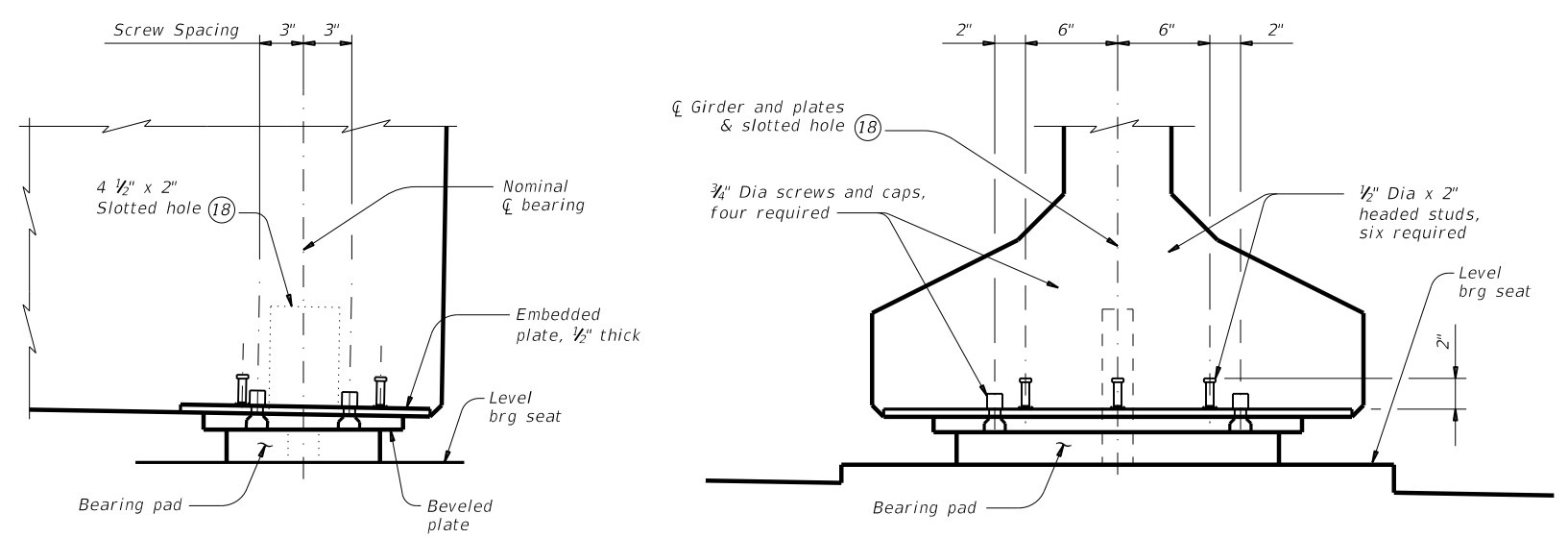
PLAN VIEW OF SOLE PLATE DETAILS



BEVELED PLATE DETAILS

- 17 Cut beveled and embedded plates to match girder end skew. Adjust location of screw and stud as shown when necessary.
- 18 Slotted hole is required at doweled girder end locations.

SOLE PLATE NOTES:
 Provide constant thickness elastomeric bearings with beveled and embedded steel sole plates in accordance with these details when the girder slope exceeds 5 percent or if otherwise required in the plans. Provide for all girders in the span.
 On the shop drawings, dimension sole plates to the nearest 1/16" based on required thickness at centerline of bearing and slope of girder. Thickness tolerance variation from the approved shop drawings is 1/16" +/-, except variation from a plane parallel to the theoretical top surface can not exceed 1/16" total. Bearing surface tolerances listed in Item 424 apply to embedded and beveled plates.
 Steel plate must conform to ASTM A36, A572 Gr 50, or A709 Gr 36 or Gr 50. Hot dip galvanize both the embedded plate and beveled sole plate after fabrication. Seal weld caps to embedded plate before galvanizing.
 When determining if relocation of screw holes and studs are necessary for skewed girder ends, minimum clearance from screw or stud centerline to plate edge is 1.25".
 Tap threads in the embedded plate only. Drill and tap prior to galvanizing.
 3/4" Dia screws must be electroplated, socket flat head countersunk cap screws conforming to ASTM F835. Electroplating must conform to ASTM B633, SC 2, Type I. Provide screws long enough to maintain a 3/4" minimum embedment into the embedded plate and galvanized cap. Provide galvanized steel caps (16 ga Min) with a nominal 1" inside diameter and deep enough to accommodate the screws, but not less than 1/2" deep or deeper than 1".
 Install beveled sole plates prior to shipping girders. Installed screw heads must not protrude below the bottom of the beveled plate.



GIRDER DETAILS

HL93 LOADING SHEET 3 OF 3

Texas Department of Transportation
 Bridge Division Standard

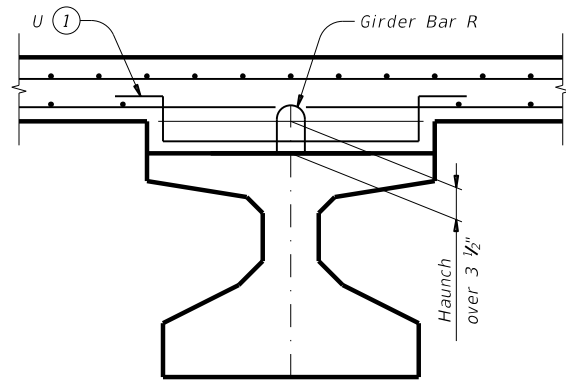
**ELASTOMERIC BEARING AND GIRDER END DETAILS
 PRESTR CONCRETE I-GIRDERS**

IGEB

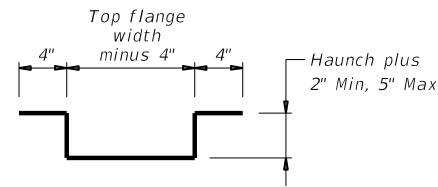
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| FILE: igbsts1-17.dgn | DN: AEE | CK: JMH | DW: JTR | CK: TxDOT |
| ©TxDOT August 2017 | CONT | SECT | JOB | HIGHWAY |
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| DIST | COUNTY | | SHEET NO. | |
| AUS | WILLIAMSON | | 99 | |

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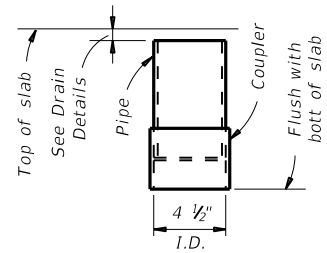
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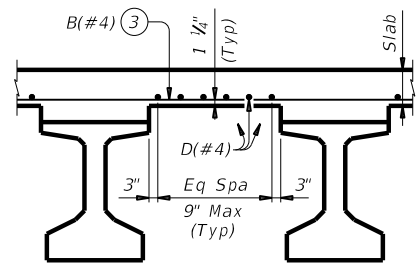
HAUNCH REINFORCING DETAIL



BARS U (#4)

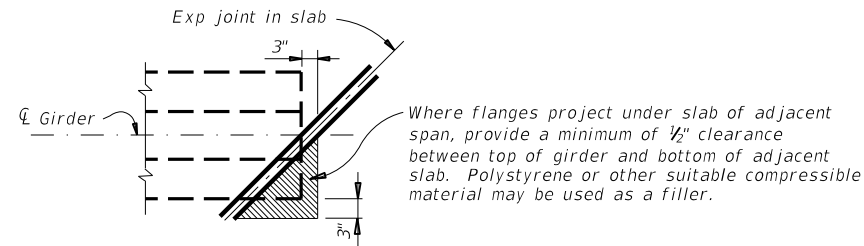


C-I-P DRAIN DETAIL (2)

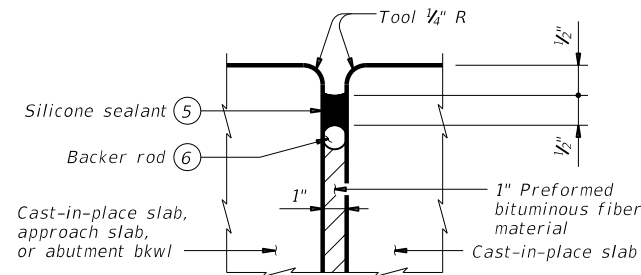


TYPICAL PART TRANSVERSE SLAB SECTION WITHOUT PCP (4)

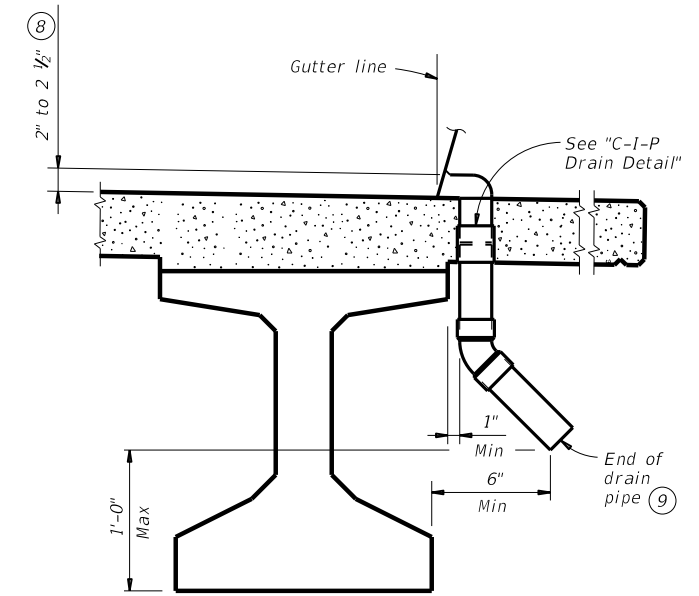
Top reinforcing steel not shown for clarity.



TREATMENT AT GIRDER END FOR SKEWED SPANS



TYPE A JOINT DETAIL (7)



DRAIN DETAIL (10)

GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications.
 Payment for Type A joint will be as per Item 454, "Bridge Expansion Joints."
 All other items (reinforcing steel, drains, etc.) shown on this sheet are subsidiary to other bid items.

Cover dimensions are clear dimensions, unless noted otherwise.
 Reinforcing bar dimensions shown are out-to-out of bar.

DECK FORMWORK NOTES:
 Overhang bracket hangers are limited to a safe working load of 3,600 lbs, applied to and along the axis of a coil rod at 45 degrees from vertical, regardless of higher loads permitted by hanger manufacturers. Do not place a hanger less than 12" from girder end. Space hangers accordingly.

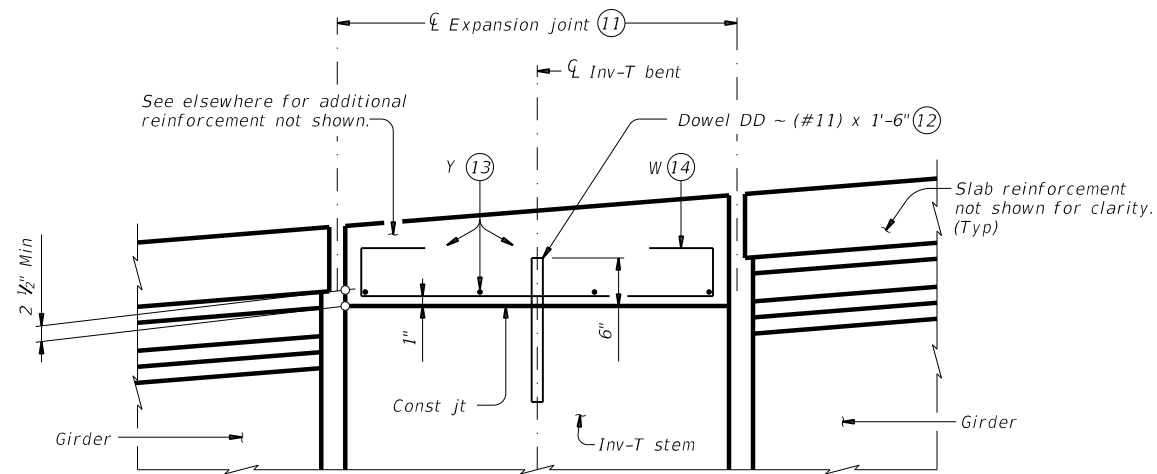
- (1) Space Bars U with girder Bars R in all areas where measured haunch exceeds 3 1/2".
- (2) Roughen outside of PVC with coarse rasp or equal to ensure bond with cast-in-place concrete.
- (3) Bars B(#4) spaced at 9" Max with 2" end cover. Overhang option, Contractor's may end alternating bars B(#4) at centerline outside girder.
- (4) Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows:
 Uncoated ~ #4 = 1'-7"
 Epoxy coated ~ #4 = 2'-5"
- (5) Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- (6) 1 1/4" backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- (7) The maximum distance between Type A expansion joints is 100'. See Bridge Layout for location of joints.
- (8) Drain entrance formed in rail or sidewalk.
- (9) Water may not be discharged onto girders.
- (10) All drain pipe and fittings to be 4" diameter (Sch 40) PVC. See Item 481 "Pipe for Drains" for pipe, connections and solvent welding. Bend reinforcing steel to clear PVC 1". Drain length and location is as directed by the Engineer. Drains are not permitted over roadways or railways, or within 10'-0" of bent caps. Degrease outside of exposed PVC, apply acrylic water base primer, then coat with same surface finishing material as used for outside girder face. Variations of the above designs, as required for the type of rail used and its location on the structure, may be installed with the approval and direction of the Engineer.

SHEET 1 OF 2

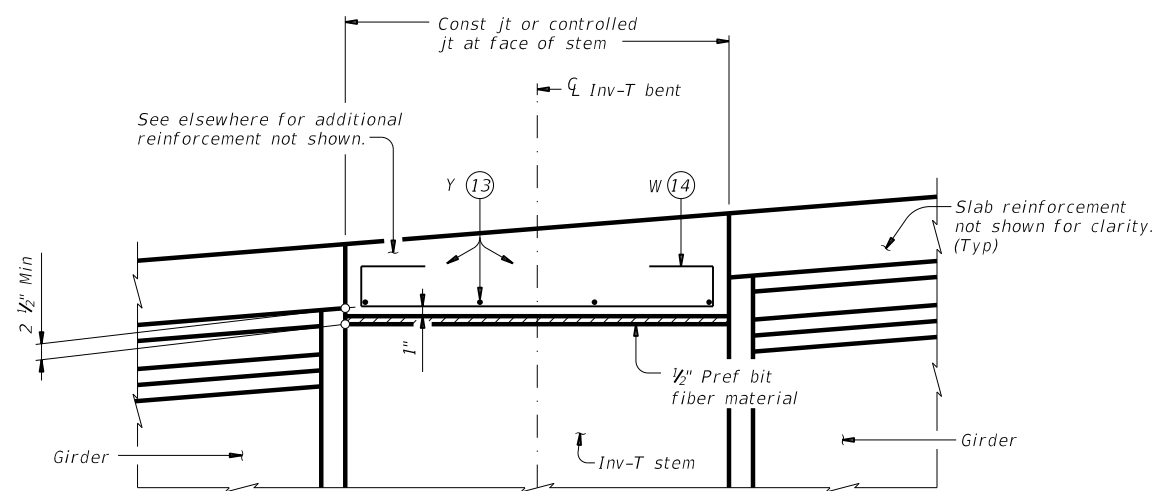
| | | | |
|---|-----------|--------------------------|-----------|
| | | Bridge Division Standard | |
| MISCELLANEOUS SLAB DETAILS PRESTR CONCRETE I-GIRDERS | | | |
| IGMS | | | |
| FILE: igssts1-19.dgn | DN: TxDOT | CK: TxDOT | DW: JTR |
| ©TxDOT August 2017 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| 10-19: Modified Note 7. Type A now a pay item. | DIST | COUNTY | SHEET NO. |
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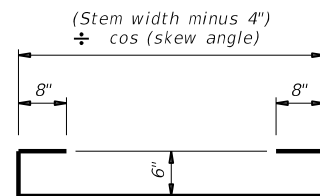
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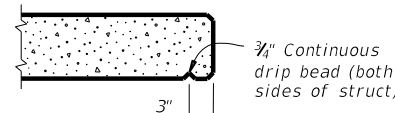
SHOWING EXPANSION JOINTS



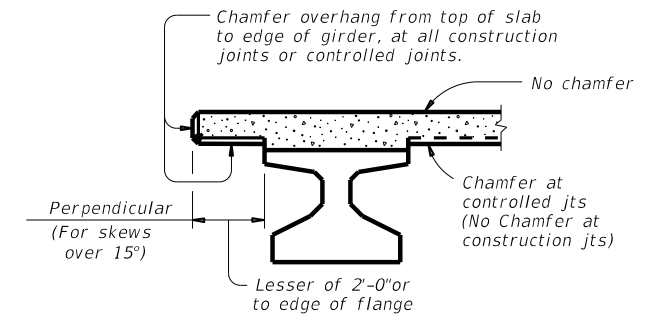
**SHOWING CONST JTS OR CONTROLLED JTS
 REINFORCEMENT OVER INV-T BENTS**



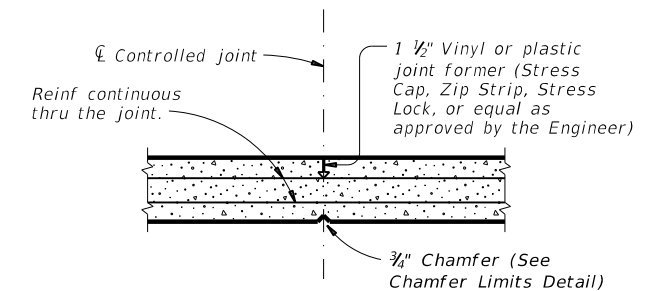
BARS W (#4)



DRIP BEAD DETAIL



CHAMFER LIMITS DETAIL (15)



CONTROLLED JOINT DETAIL

(Saw-cutting is not allowed)

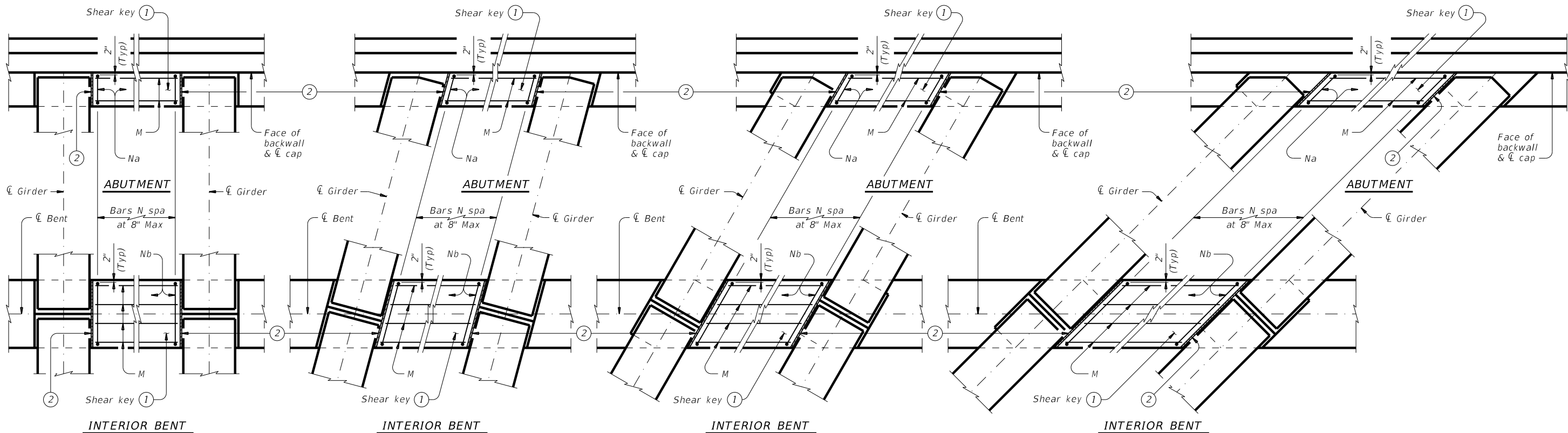
- (11) See Layout for joint type.
- (12) Dowels DD (#11) spaced at 5 Ft Max. See Inv-T bents for quantity and location.
- (13) Space Bars Y (#4) at 12" Max. Use 2" end cover. Number of Bars Y must satisfy spacing limit. Place parallel to bent.
- (14) Space Bars W at 12" Max (3" from end of cap). Tilt if necessary to maintain cover requirements. Place parallel to longitudinal slab reinforcement.
- (15) See Span details for type of joint and joint locations.

SHEET 2 OF 2

| | | | |
|---|-----------|--------------------------|-----------|
| | | Bridge Division Standard | |
| MISCELLANEOUS SLAB DETAILS PRESTR CONCRETE I-GIRDERS | | | |
| IGMS | | | |
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| 10-19: Modified Note 7. Type A now a pay item. | DIST | COUNTY | SHEET NO. |
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PARTIAL PLANS WITH NO SKEW

Showing shear keys on 3'-6" wide caps. 4'-0" caps similar.

PARTIAL PLANS WITH 15° SKEW

Showing shear keys on 3'-6" wide caps. 4'-0" caps similar.

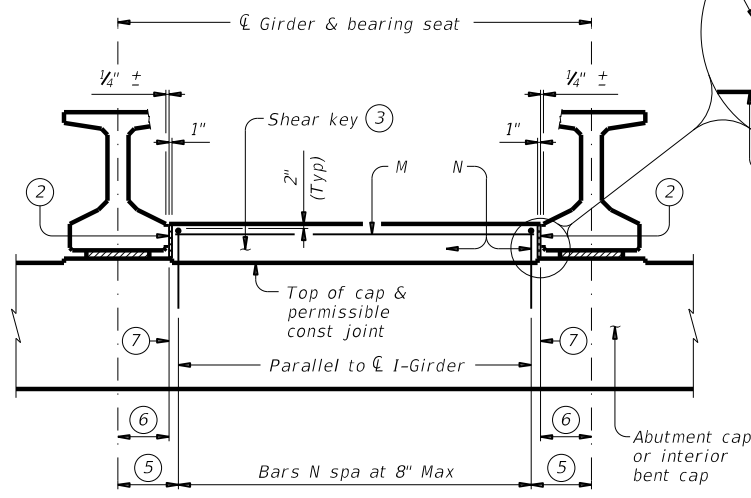
PARTIAL PLANS WITH 30° SKEW

Showing shear keys on 3'-6" wide caps. 4'-0" caps similar.

PARTIAL PLANS WITH 45° SKEW

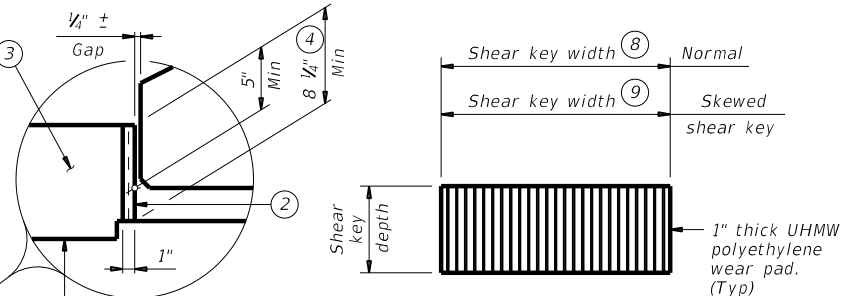
Showing shear keys on 3'-6" wide caps. 4'-0" caps similar.

- ① Place shear keys on the upstream side of structure between outside girder and next adjacent girder, unless shown otherwise on plans.
- ② UHMW polyethylene wear pad. (Typ)
- ③ Leave a 1/4" gap plus or minus between girder and face of wear pad. Cast wear pad with shear key, smooth side facing girder. Care must be taken to keep concrete from flowing under girder. Slope top of shear keys in accordance with Item 420.4.9, "Treatment and Finishing of Horizontal Surfaces."
- ④ Measure at higher bearing seat elevation forward or back. Dimension based on typical bearing pad and bearing seat. Increase as necessary to maintain 5" overlap.
- ⑤ With No Skew = 1'-8 1/4", measured along \bar{C} cap. With Skew = 1'-8 1/4" \div Cos Skew, measured along \bar{C} cap.
- ⑥ With No Skew = 1'-4 1/4", measured along \bar{C} cap. With Skew = 1'-4 1/4" \div Cos Skew, measured along \bar{C} cap.
- ⑦ Face of UHMW polyethylene wear pad. Smooth side of pad facing girder.
- ⑧ Abutments = 1/2 Cap width. Interior bents = Cap width.
- ⑨ Abutments = 1/2 Cap width \div Cos Skew. Interior bents = Cap width \div Cos Skew.

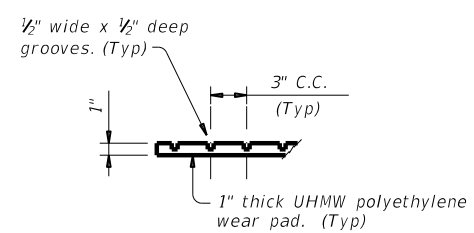


PARTIAL ELEVATION OF ABUTMENT OR INTERIOR BENT CAP

Showing shear key with girder Type Tx46. Other I-Girder types similar.

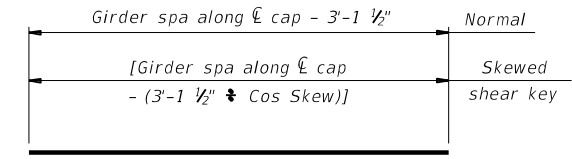


ELEVATION

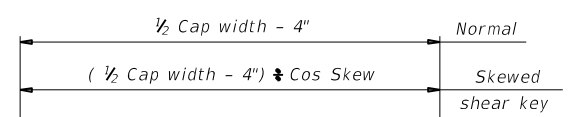


PART SECTION

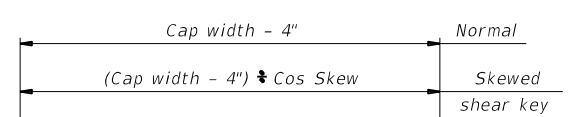
ULTRA HIGH MOLECULAR WEIGHT (UHMW) POLYETHYLENE WEAR PAD DETAILS



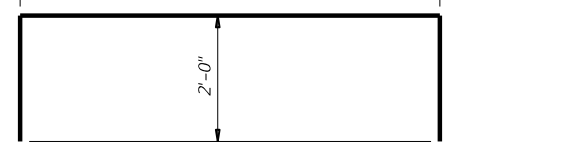
BARS M (#5)



BARS Na (#5) (For abutments)



BARS Nb (#5) (For interior bents)

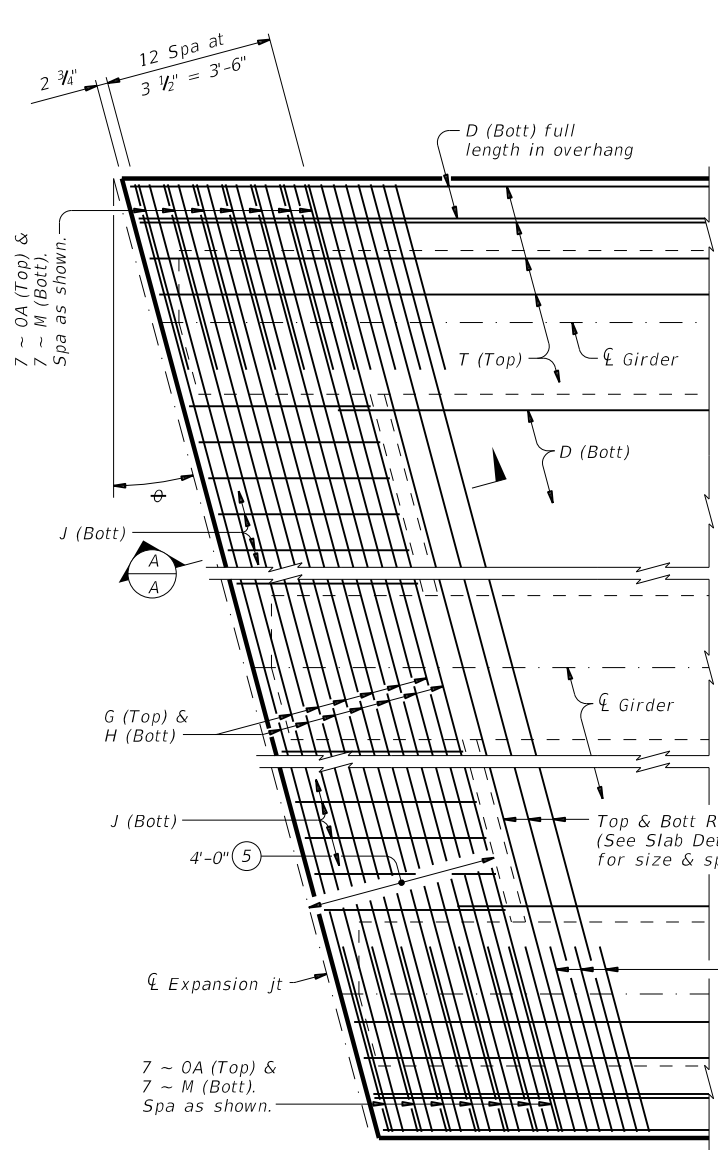


CONSTRUCTION NOTES:
 Provide Class "C" concrete ($f'_c = 3,600$ psi). Provide Class "C" (HPC) if shown elsewhere on the plans.
 Provide Grade 60 reinforcing steel.
 Provide epoxy coated reinforcing steel for shear key if abutment or interior bent reinforcing steel is epoxy coated.
 Provide Ultra High Molecular Weight (UHMW) polyethylene wear pads in accordance with ASTM D6712.

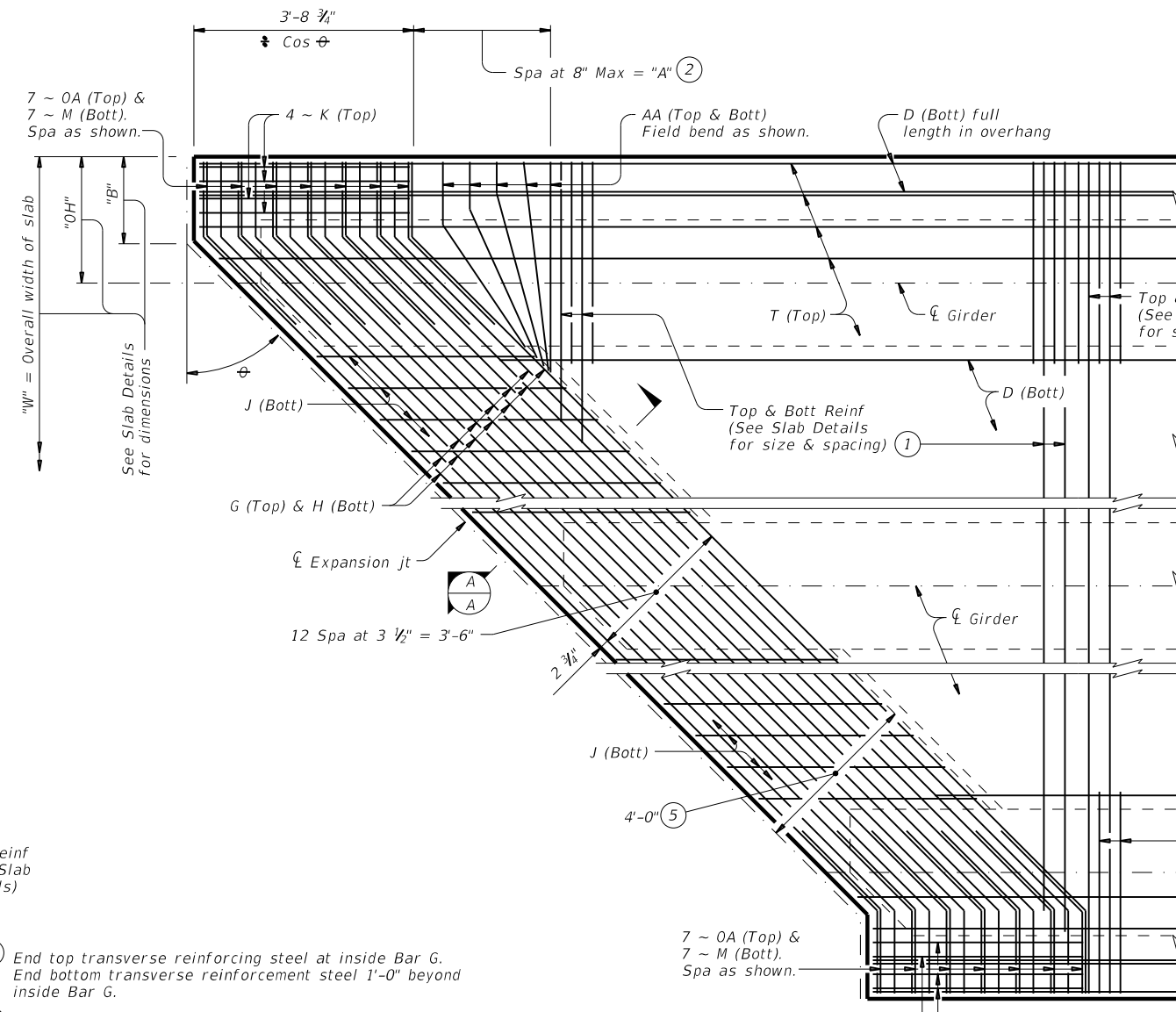
GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications. Details showing skew are drawn showing right forward skew. See Bridge Layout for actual skew direction.
 These details are limited to bridges skewed 45 degrees and less. This standard is only applicable for I-Girders.
 Modify details for bearing conditions, and girder spacing not shown on this standard. Details do not account for sole plate or pedestal bearing seat.
 Include shear key concrete in abutment or bent concrete for payment.
 UHMW polyethylene wear pads are subsidiary to Class "C" concrete.
 Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

| | | | |
|--|-----------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| SHEAR KEY DETAILS PRESTR CONCRETE I-GIRDERS | | | |
| IGSK | | | |
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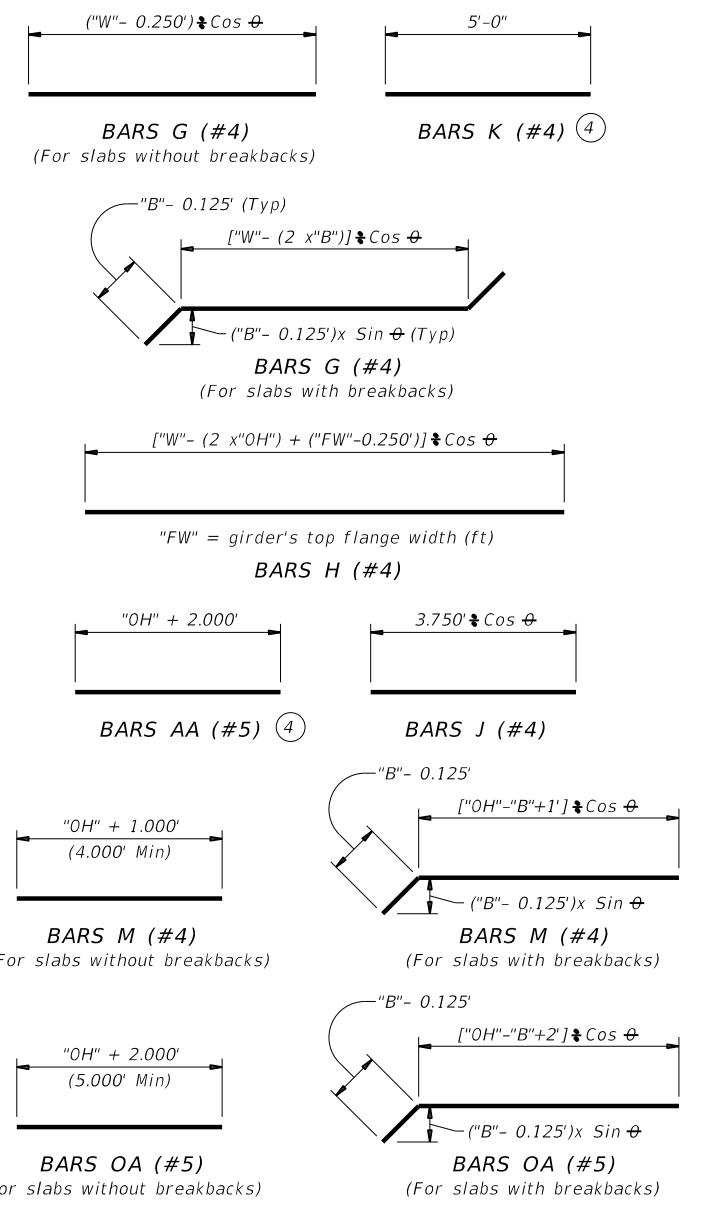


PARTIAL PLAN FOR SLABS WITHOUT BREAKBACK



PARTIAL PLAN FOR SLABS WITH BREAKBACK

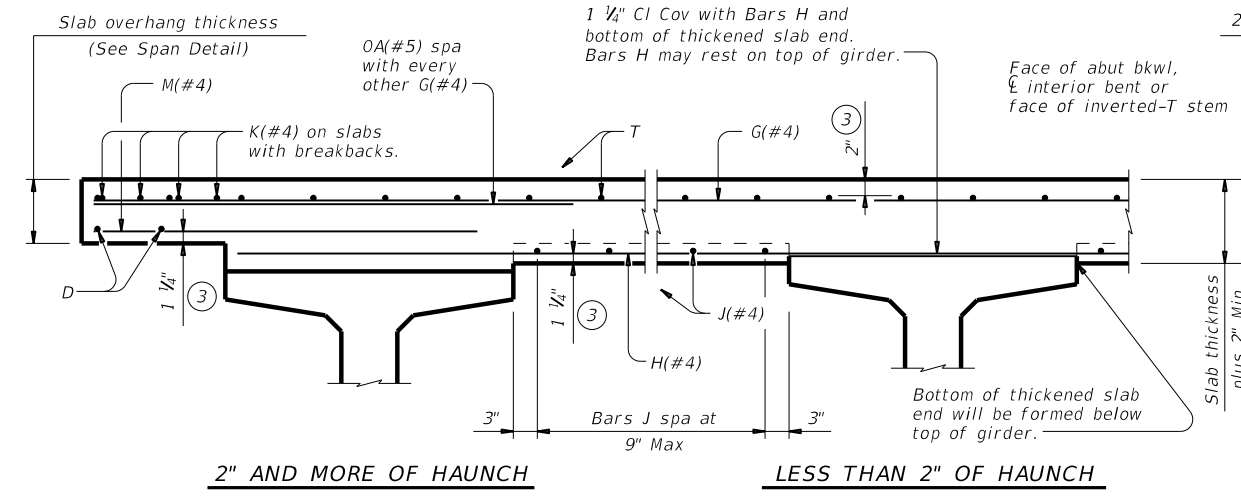
- ① End top transverse reinforcing steel at inside Bar G. End bottom transverse reinforcement steel 1'-0" beyond inside Bar G.
- ② "A" = ("OH" + 2.333' - "B") x Tan ϕ
- ③ Provide clear cover as indicated unless otherwise shown on Span Details.
- ④ Only required on slabs with breakbacks.
- ⑤ Thickened slab end dimensioned perpendicular to face of bkwl, centerline interior bent or face of inverted-T stem.



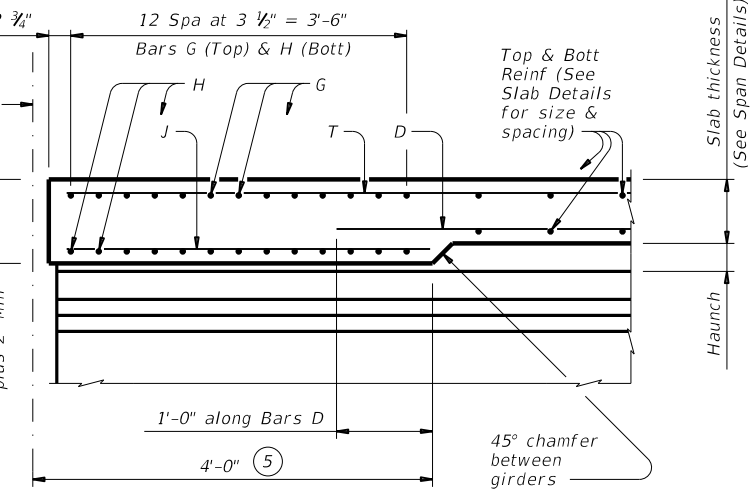
GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications. These details are restricted to Prestressed Concrete I-Girder Spans. These details are to be used in conjunction with the Span Details and PCP standard (if prestressed concrete panels are used). When Option 2 from PCP standard is used, provide Bars AA, G, K and OA in the slab.

MATERIAL NOTES:
 Provide Grade 60 reinforcing steel. If slab reinforcing steel is shown on the Slab Details to be epoxy coated, then Bars AA, G, K, H, J, M and OA must be epoxy coated. Provide bar laps, where required, as follows:
 Uncoated ~ #4 = 1'-7"
 Epoxy Coated ~ #4 = 2'-5"

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.



TYPICAL TRANSVERSE SECTION
 (Showing Prestressed Conc I-Girders at ϕ Brg)



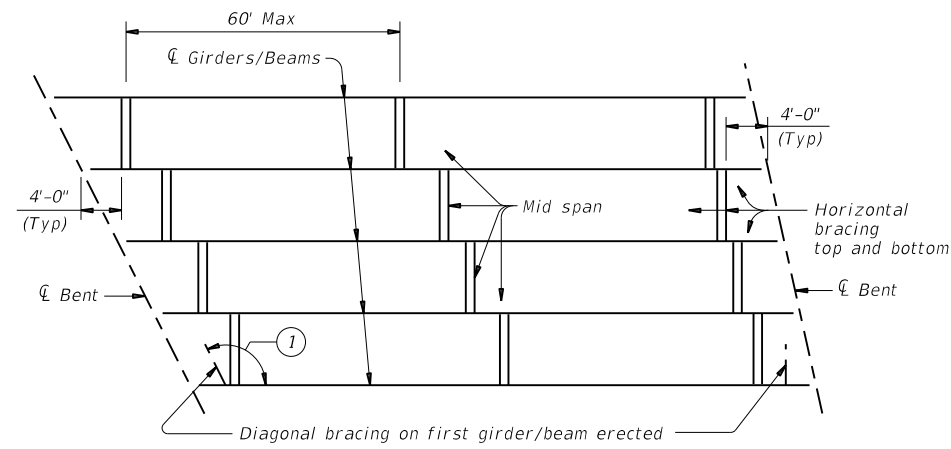
SECTION A-A
 (Showing with 2" and more of haunch)

| | | | |
|--|------------|--------------------------|---------|
| HL93 LOADING | | Bridge Division Standard | |
| THICKENED SLAB END DETAILS | | | |
| PRESTRESSED CONCRETE I-GIRDER SPANS | | | |
| IGTS | | | |
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| REVISIONS | 0914 05 | 204, ETC. | CR 118 |
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| AUS | WILLIAMSON | 103 | |

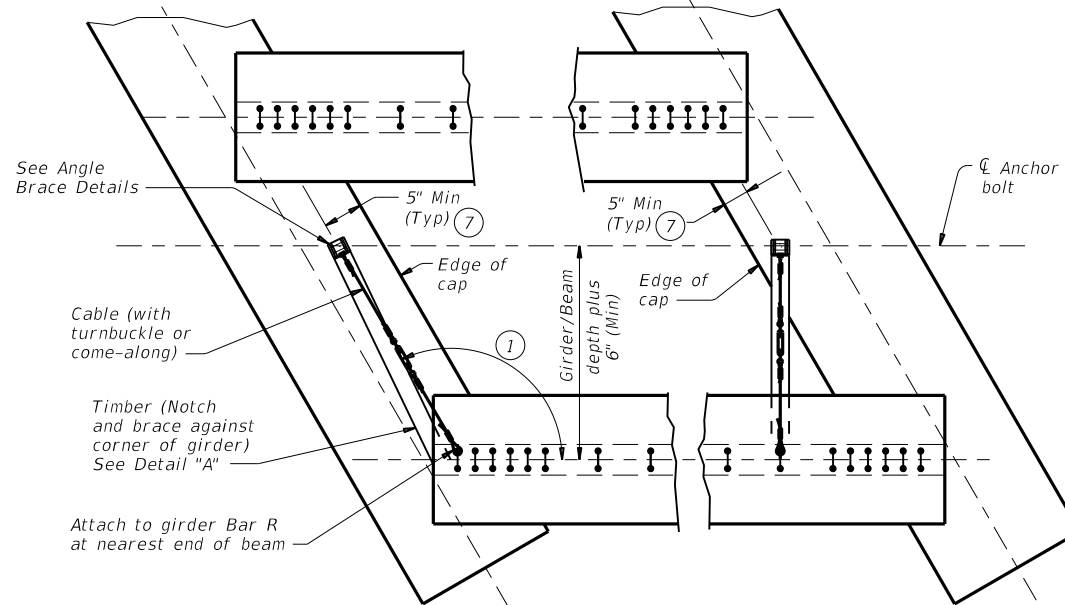
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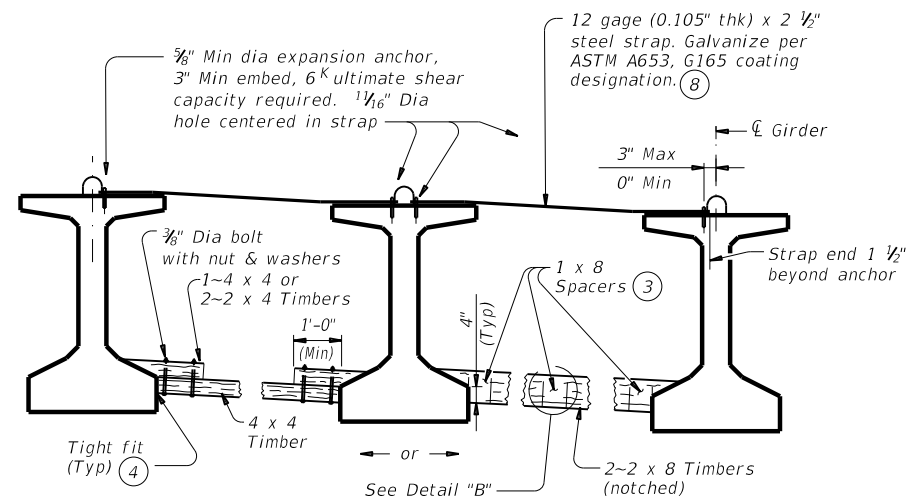
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ERECTION BRACING

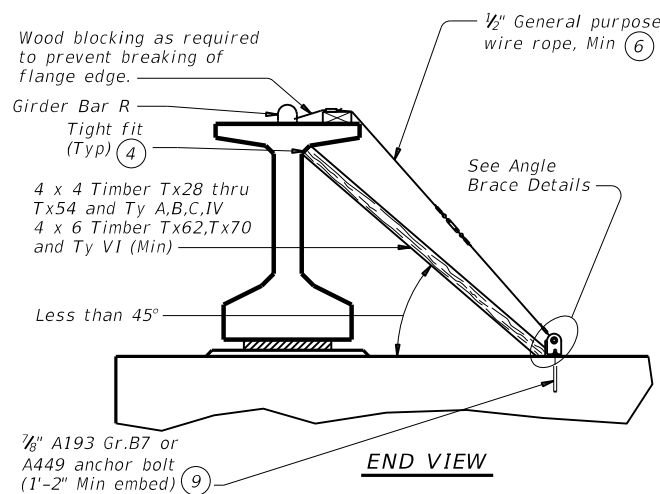


PLAN



FOR ERECTION BRACING, OPTION 1

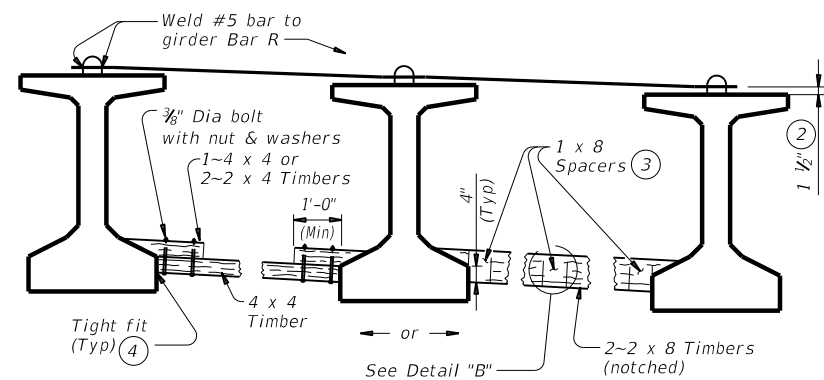
(This option is not allowed when slab is formed with PMDF or plywood.)



END VIEW

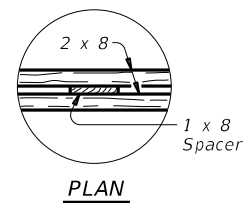
DIAGONAL BRACING DETAILS

(To be used on both ends of the first girder/beam erected in the span in each phase.)



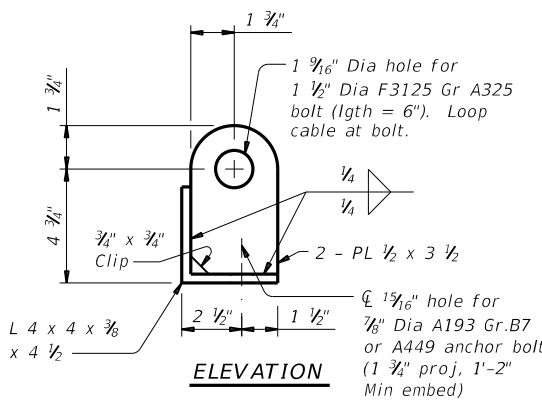
FOR ERECTION BRACING, OPTION 2

HORIZONTAL BRACING DETAILS

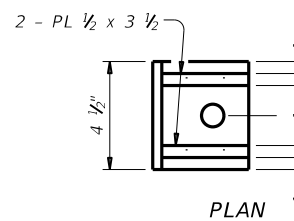


PLAN

DETAIL "B"



ELEVATION



PLAN

ANGLE BRACE DETAILS

HAULING & ERECTION:

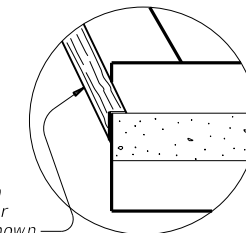
The Contractor's attention is directed to the possible lateral instability of prestressed concrete girders and beams over 130' long, especially during hauling and erection. The use of the following methods to improve stability is encouraged: Locate lifting devices at the maximum practical distance from girder ends; use external lateral stiffening devices during hauling and erection; lift with vertical lines using two machines; and take care in handling to minimize inertial and impact forces.

ERECTION BRACING:

Erection bracing details shown are considered the minimum for fulfilling the bracing requirements of Item 425. Required erection bracing must be placed immediately after erection of each girder and remain in place until additional bracing as required for slab placement is in place. This standard is needed in all cases to meet requirements for Slab Placement Bracing.

PHASED CONSTRUCTION:

Place erection and slab placement bracing for all girders in a phase as shown in these details. For phases after first, also place erection and slab placement bracing between outer girder of completed phase and adjacent girder of current phase. When the phase construction joint is between girders, top bracing can be omitted.



DETAIL "A"

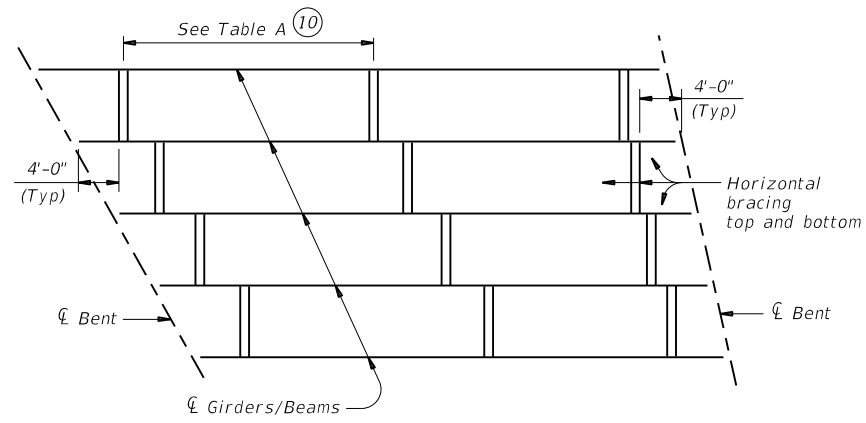
- 1 If angle shown exceeds 120 degrees, move diagonal brace to other side of girder/beam and place square to girder/beam. This may prevent exterior girder from being erected first.
- 2 Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R (See Sheet 2 of 2).
- 3 Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- 4 Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- 5 Pressure treated landscape timbers can not be used.
- 6 All hardware used with cable must be able to develop a minimum 25 kips breaking strength. Use thimbles at all loops in cable. Install cable clamps with saddles bearing against the live end and U-bolts bearing against the dead end.
- 7 It is acceptable to tie anchor bolts to cap reinforcement.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- 9 Anchor bolt may be drilled and epoxied in place. Provide 25k minimum pullout. Core drill hole.

SHEET 1 OF 2

| | | | |
|---|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| MINIMUM ERECTION AND BRACING REQUIREMENTS PRESTRESSED CONCRETE I-GIRDERS AND I-BEAMS | | | |
| MEBR(C) | | | |
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| ©TxDOT August 2017 | CONTRACT | SECTION | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 104 | |

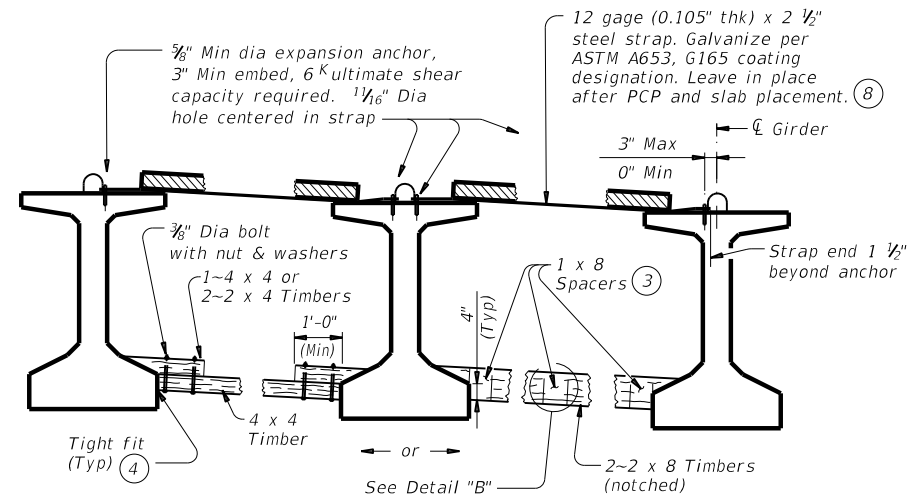
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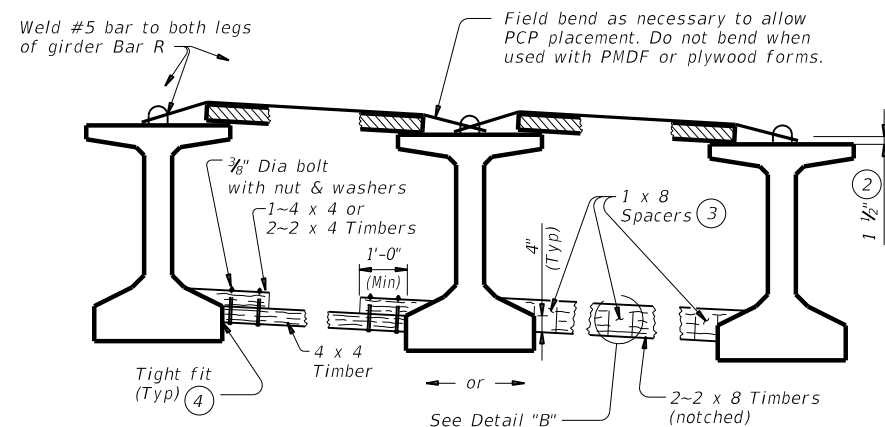
SLAB PLACEMENT BRACING

| TABLE A | | | | |
|---------------------|--------------------------------------|--------------------------------------|--|--------------------------------------|
| Girder or Beam Type | OPTION 1-RIGID BRACING (STEEL STRAP) | | OPTION 2-FLEXIBLE BRACING (NO. 5 OVER PCP) | |
| | Maximum Bracing Spacing | | Maximum Bracing Spacing | |
| | Slab Overhang less than 4'-0" (11) | Slab Overhang 4'-0" and greater (11) | Slab Overhang less than 4'-0" (11) | Slab Overhang 4'-0" and greater (11) |
| Tx28 | 1/4 points | 1/4 points | Tx28 | 1/4 points |
| Tx34 | 1/4 points | 1/4 points | Tx34 | 1/4 points |
| Tx40 | 1/4 points | 1/4 points | Tx40 | 1/4 points |
| Tx46 | 1/4 points | 1/4 points | Tx46 | 1/4 points |
| Tx54 | 1/4 points | 1/4 points | Tx54 | 1/4 points |
| Tx62 | 1/4 points | 1/4 points | Tx62 | 1/4 points |
| Tx70 | 1/4 points | 1/4 points | Tx70 | 1/4 points |
| A | 1/8 points | 1/8 points | A | 2.0 ft |
| B | 1/8 points | 1/8 points | B | 3.0 ft |
| C | 1/8 points | 1/8 points | C | 4.5 ft |
| IV | 1/4 points | 1/4 points | IV | 1/4 points |
| VI | 1/4 points | 1/4 points | VI | 1/4 points |



FOR SLAB PLACEMENT BRACING, OPTION 1 - RIGID

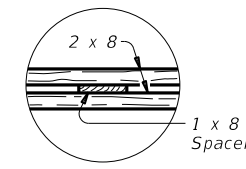
(Showing slab formed with PCP. This option is not allowed when slab is formed with PMDF or plywood.)



FOR SLAB PLACEMENT BRACING, OPTION 2 - FLEXIBLE

(Showing slab formed with PCP.)

HORIZONTAL BRACING DETAILS (5)



- (2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R.
- (3) Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- (4) Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- (8) Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- (10) Bracing spacing (1/4 and 1/8 points) measured between first and last typical brace location.
- (11) Measure slab overhang from centerline of girder or beam. When overhang varies in span, determine bracing spacing based on largest overhang.

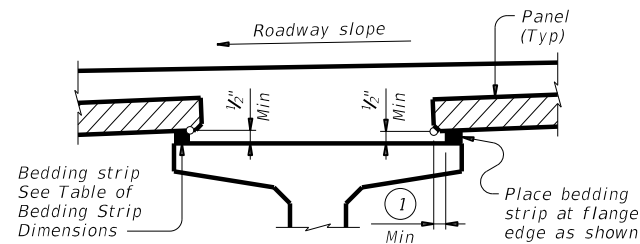
SLAB PLACEMENT BRACING:
 The details for slab placement bracing are considered minimum for fulfilling the requirements of Specification Items 422 and 425. Required slab placement bracing must remain in place until slab concrete has attained a compressive strength of 3000 psi.

GENERAL NOTES:
 Bracing details for spans longer than 150' are not provided. The Contractor must submit proposed bracing details for such conditions to the Engineer for approval prior to erection. Systems equal to or better than those shown may be used provided details of such systems are submitted to and approved by the Engineer prior to erection. Use of these systems or details does not relieve the Contractor of the responsibility for the adequacy of the bracing and the safety of the structure. Removal of bracing for short periods of time to align girders and beams is permissible. All turn-buckles, come-alongs, anchors and other connections must be capable of developing the full strength of the cable shown. Furnish anchor bolts and nuts in accordance with Item 449, "Anchor Bolts".

| | | | |
|---|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| MINIMUM ERECTION AND BRACING REQUIREMENTS PRESTRESSED CONCRETE I-GIRDERS AND I-BEAMS | | | |
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| AUS | WILLIAMSON | 105 | |

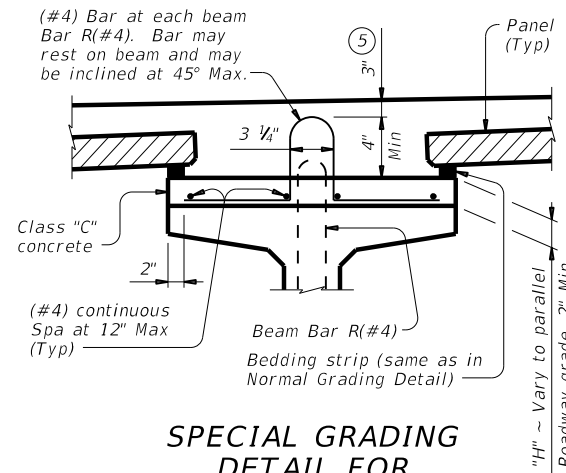
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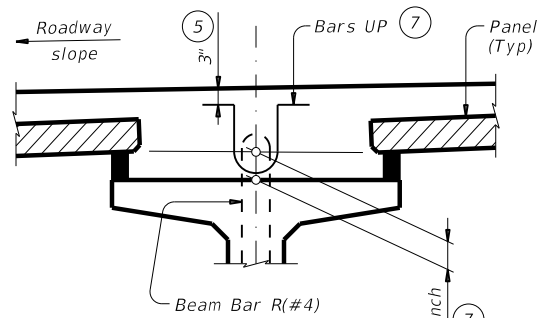
NORMAL GRADING DETAIL ③

Showing prestressed concrete I-girders. (Other beam types similar)



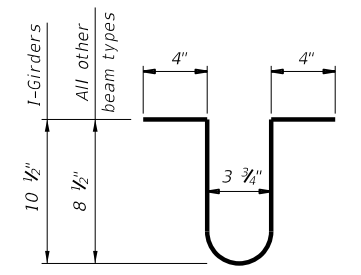
SPECIAL GRADING DETAIL FOR CONCRETE BEAMS

Showing prestressed concrete I-girders. (Other beam types similar)

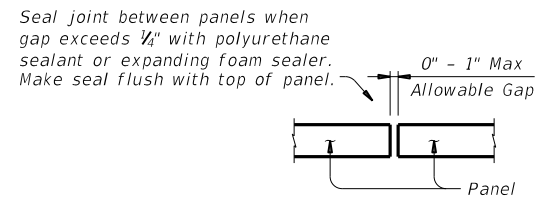


HAUNCH REINFORCING DETAIL

Showing prestressed concrete I-girders. (Other beam types similar)

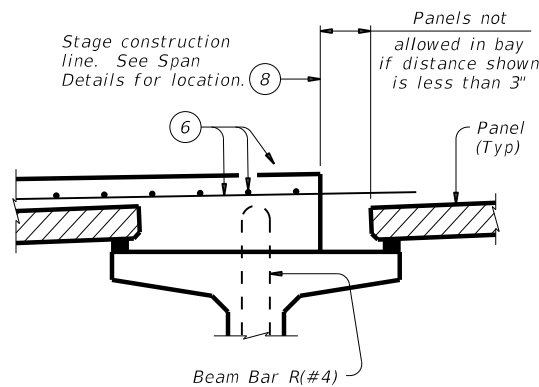


BARS UP (#4) ⑦

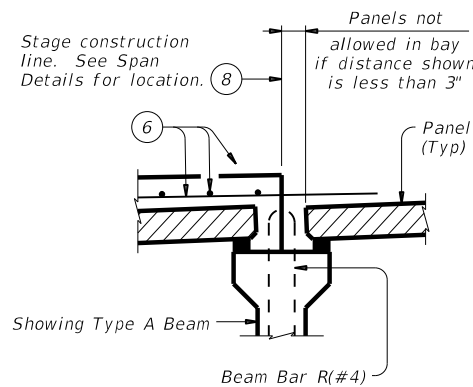


PANEL JOINTS

(Panel reinforcing not shown for clarity. The gap cannot be considered as a panel fabrication tolerance. Adjust panel placement to minimize joint openings.)



PRESTR CONC I-GIRDERS



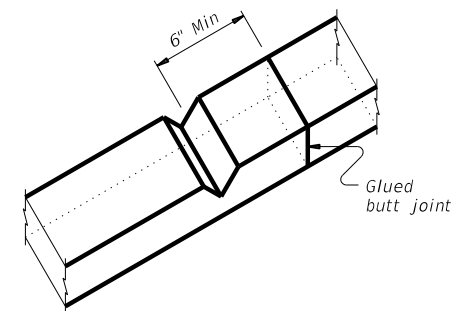
PRESTR CONC I-BEAMS

STAGE CONSTRUCTION LIMITATIONS

(Other beam types similar)

| WIDTH | HEIGHT ④ | |
|----------|----------|----------|
| | Min | Max |
| 1" (Min) | 1/2" | 2" |
| 1 1/4" | 1/2" | 2 1/2" |
| 1 1/2" | 1/2" | 3" |
| 1 3/4" | 1/2" | 3 1/2" |
| 2" | 1/2" | 4" |
| 2 1/4" | 1/2" | 4 1/2" ② |
| 2 1/2" | 1/2" | 5" ② |
| 2 3/4" | 1/2" | 5 1/2" ② |
| 3" (Max) | 1/2" | 6" ② |

- ① 2" Min for I-girders, 1 1/2" Min for all other beam types.
- ② Allowed for I-girders, not allowed on other beam types.
- ③ To reduce the quantity of cast-in-place concrete, bedding strip thickness may be increased in 1/4" increments. Bedding strips must be comprised of one layer. Bond bedding strips to the beams with an adhesive compatible with bedding strips. Bedding strips over 2.5" high may need to be bonded to panels. The same thickness strip must be used under any one panel edge and the maximum change in thickness between adjacent panels is 1/4". Alternatively, bedding strips may be cut to grade. Panels may be supported by an alternate method, using a commercial product, if approved by the Engineer of Bridge Design, Bridge Division. If bedding strips exceed 6" high for I-Girders, 4" high for all other beam types, use Special Grading Detail for Concrete Beams or submit an alternate method to the Bridge Division for approval.
- ④ Height must not exceed twice the width.
- ⑤ Provide clear cover as indicated unless otherwise shown on Span Details.
- ⑥ See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- ⑦ Space Bars UP(#4) with Beam Bars R(#4) in all areas where measured haunch exceeds 3 1/2" with I-girders, and 3" for all other beam types. Epoxy coating for Bars UP is not required.
- ⑧ Do not locate construction joints on top of a panel.
- ⑨ Butt adjacent bedding strips together with adhesive. Cut v-notches, approx 1/4" deep, in the top of the bedding strips at 8" o.c..



BEDDING STRIP DETAIL ⑨

CONSTRUCTION NOTES:
 Erected panels must bear uniformly on bedding strips of extruded polystyrene placed along top flange edges. Placing panels to minimize joint openings is recommended. If additional blocking is needed, special grading details for supporting the panels and extra reinforcing between beam and slab will be considered subsidiary to deck construction. Bars U, shown on PCP-FAB, may be bent over or cut off if necessary. Care must be taken to ensure proper cleaning of construction debris and consolidation of concrete material under the edges of the panels. Bedding strips must be placed at beam flange edges so that adequate space is provided for the mortar to flow a minimum of 1 1/2" under the panels as the slab concrete is placed. To allow the proper amount of mortar to flow between beam and panel, the minimum vertical opening must be at least 1/2". Roadway cross-slope reduces the opening available for entry of the mortar. Bedding strips varying in thickness across the beam are therefore required. For clear span between U-beams less than or equal to 18", see Permissible Slab Forming Detail on Miscellaneous Slab Detail sheets, UBMS.

MATERIAL NOTES:
 Provide Grade 60 reinforcing steel in the cast-in-place slab. See Table of Reinforcing Steel for size and spacing of reinforcement. If the top and bottom layer of reinforcing steel is shown on the Span Details to be epoxy coated, then the D, E, P, & Z bars must be epoxy coated. Provide bar Laps, where required, as follows:
 Uncoated ~ #4 = 1'-7"
 Epoxy Coated ~ #4 = 2'-5"

GENERAL NOTES:
 Designed according to AASHTO LRFD Bridge Design Specifications. Panel placement may follow either Option 1 or Option 2 except Option 1 must be used if the skew exceeds 45 degrees. Use of Prestressed Concrete Panels is not permitted for horizontally curved steel plate or tub girders. See Span Details for other possible restrictions on their use. These details are to be used in conjunction with the Span Details, PCP-FAB and other applicable standard drawings. When panel support (bedding strips) deviates from what is shown herein, provide details signed and sealed by a professional Engineer. Any additional reinforcing or concrete required on this standard is considered subsidiary to the bid item "Reinforced Concrete Slab".

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

HL93 LOADING SHEET 1 OF 4

Texas Department of Transportation **Bridge Division Standard**

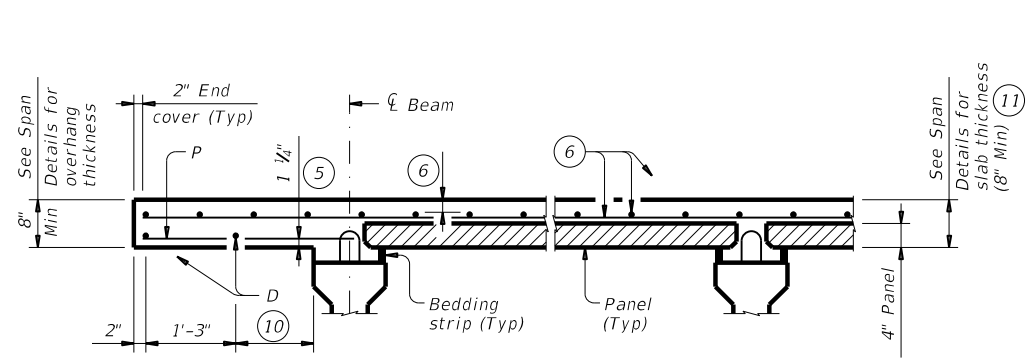
PRESTRESSED CONCRETE PANELS DECK DETAILS

PCP

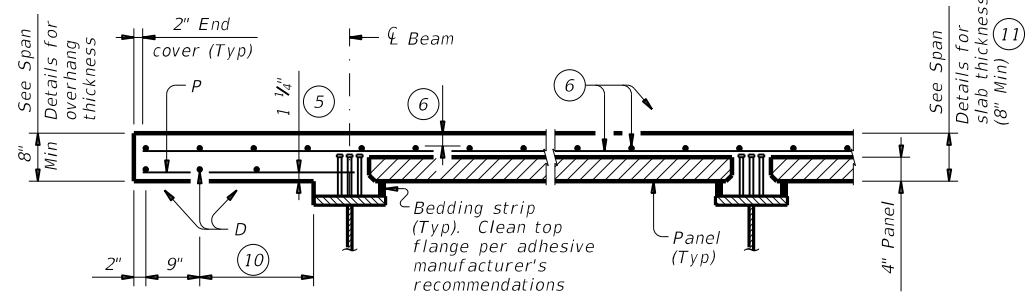
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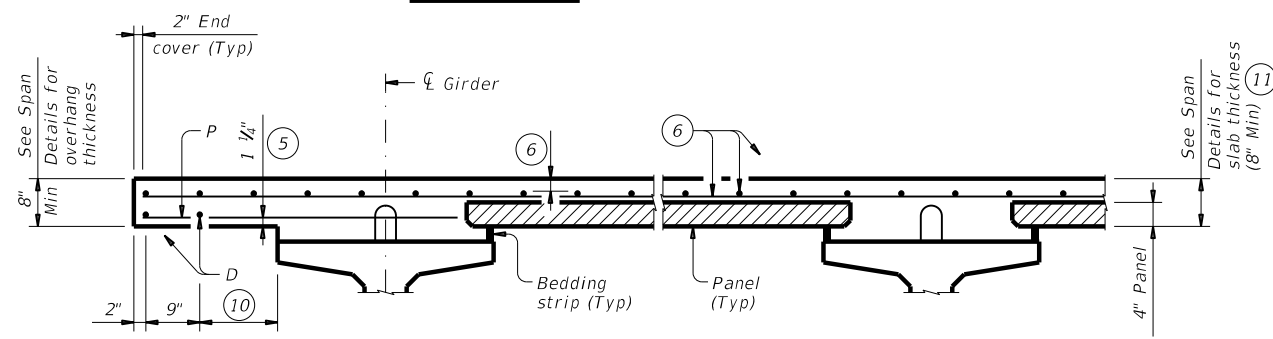
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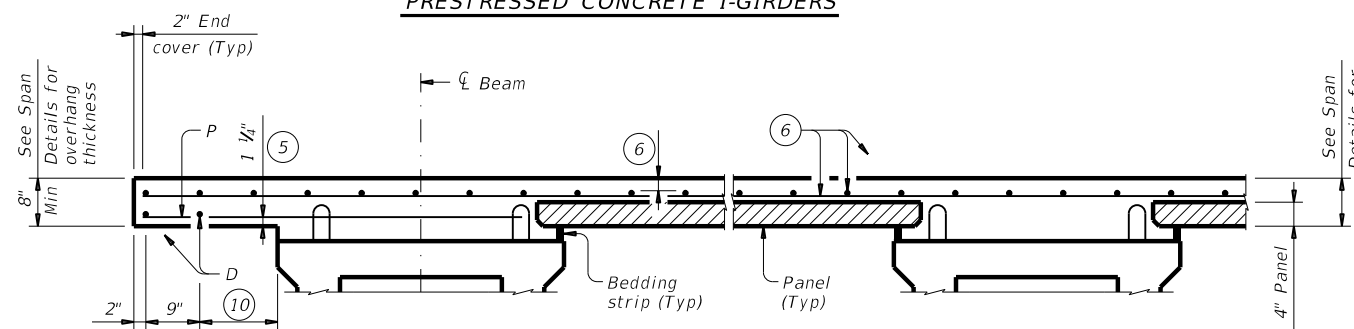
PRESTRESSED CONCRETE I-BEAMS



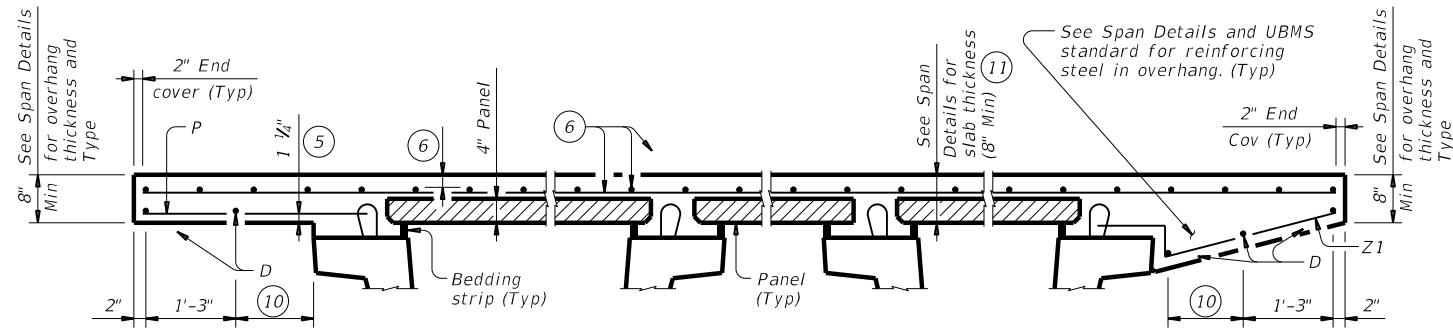
STEEL BEAMS



PRESTRESSED CONCRETE I-GIRDERS



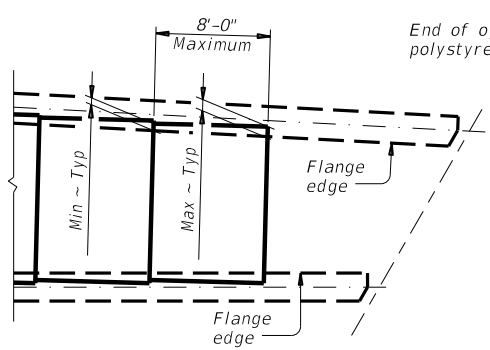
PRESTRESSED CONCRETE X-BEAMS



NORMAL OVERHANG WITH PRESTR CONC U-BEAMS

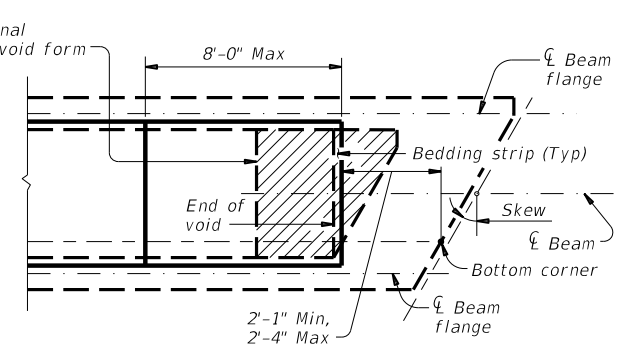
TYPICAL PART TRANSVERSE SECTIONS

SLOPED OVERHANG WITH PRESTR CONC U-BEAMS



AT FLARED BEAMS OR GIRDERS

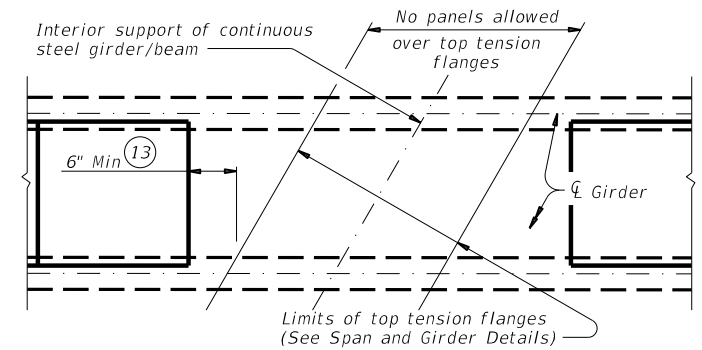
See PCP-FAB standard for Min and Max dimensions based on beam/girder type.



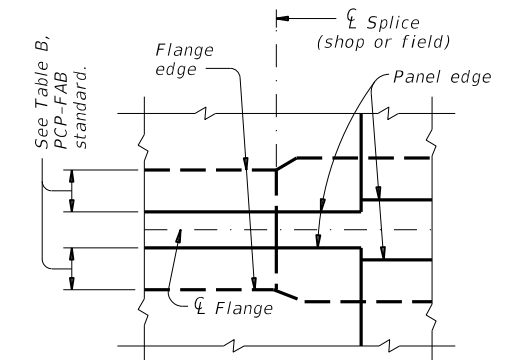
OVER CONC U-BEAMS

PART PLANS OF PANEL PLACEMENT

- 5 Provide clear cover as indicated unless otherwise shown on Span Details.
- 6 See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- 9 Butt adjacent bedding strips together with adhesive. Cut v-notches, approx 1/4" deep, in the top of the bedding strips at 8' o.c..
- 10 Equally space additional bar if more than 1'-3" Max.
- 11 The actual thickness constructed may exceed the slab thickness shown on the Span Details but the extra thickness may be no more than 2" (1" for prestressed concrete U-beams and steel beams). Bearing seat elevations or finished grade may be adjusted.
- 12 Field adjust Bars Z1(#4) to match actual slope of slab overhangs. Width of slab overhang will vary along span with curved slab edges. Adjust Bar Z1(#4) dimensions to maintain proper cover. Bars Z2(#4) are located at Inverted-Tee stems only.
- 13 Location of concrete placement sequence boundaries and bolted field splices should be considered by the contractor in determining panel limits.



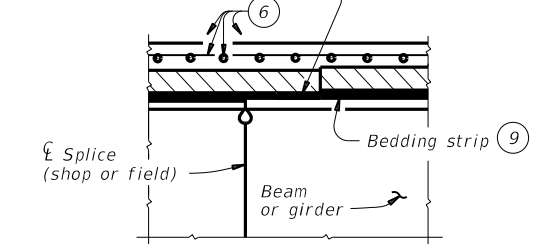
AT INT SUPPORTS OF CONTINUOUS STEEL GIRDERS



PLAN AT SPLICE

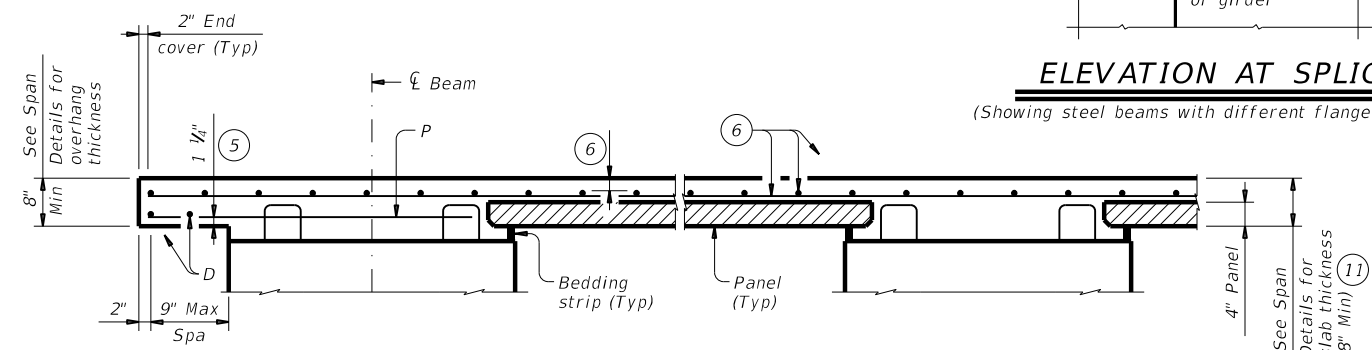
(Showing steel beams with flange width transition)

Cut bedding strip to adjust for difference in flange thickness.



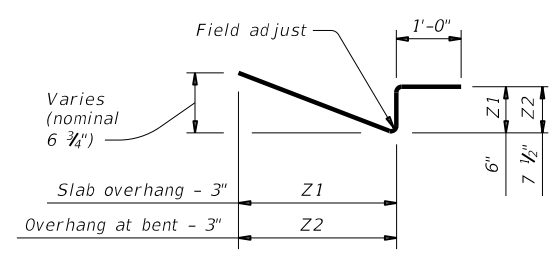
ELEVATION AT SPLICE

(Showing steel beams with different flange thickness)



PRESTRESSED CONCRETE SPREAD SLAB BEAMS

Bars P over exterior beams are still required when no overhang is used. In this case, only one Bar D, 2" from slab edge, is required.



BARS Z (#4)

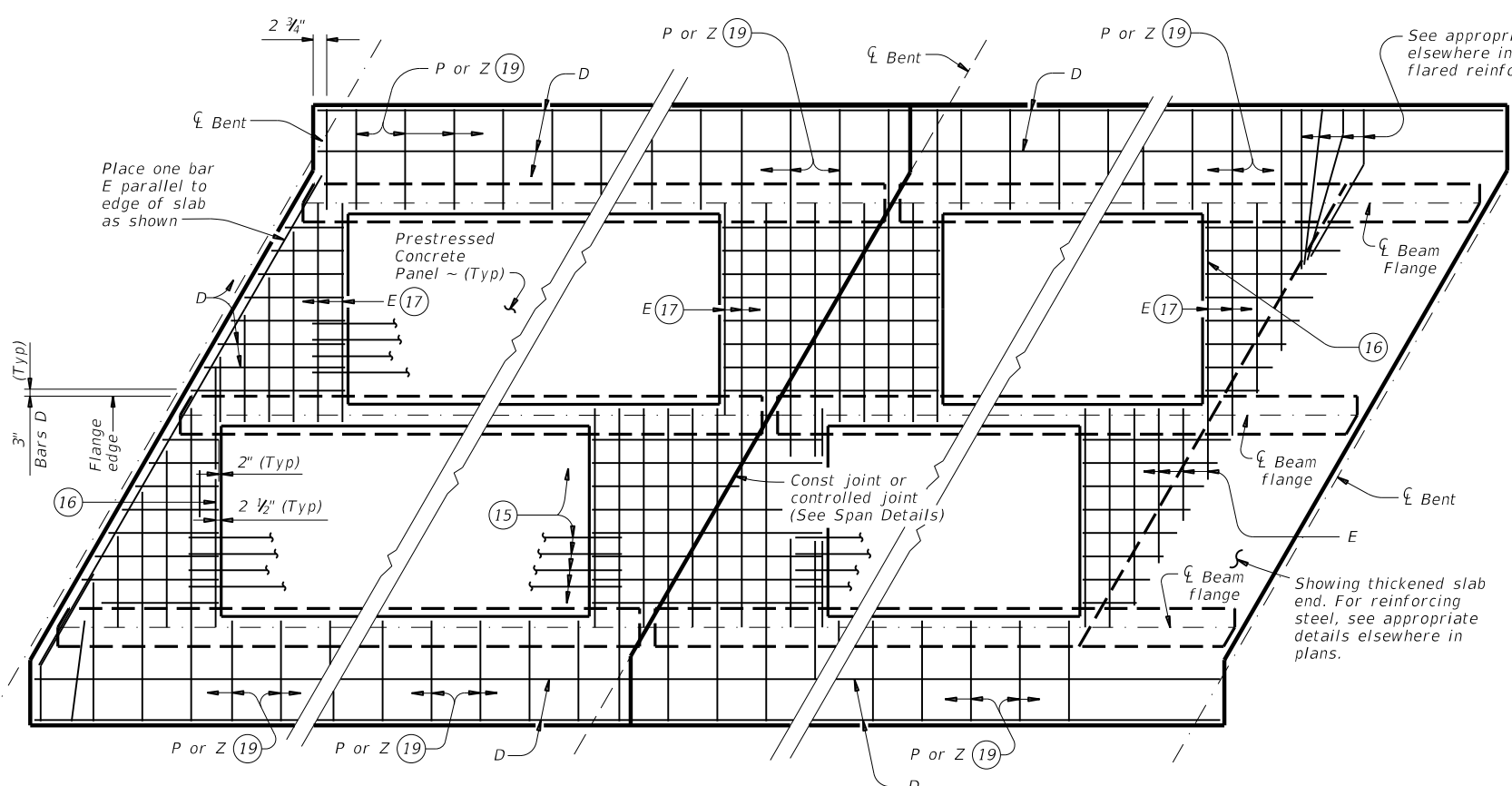
PRESTRESSED CONCRETE PANELS DECK DETAILS

PCP

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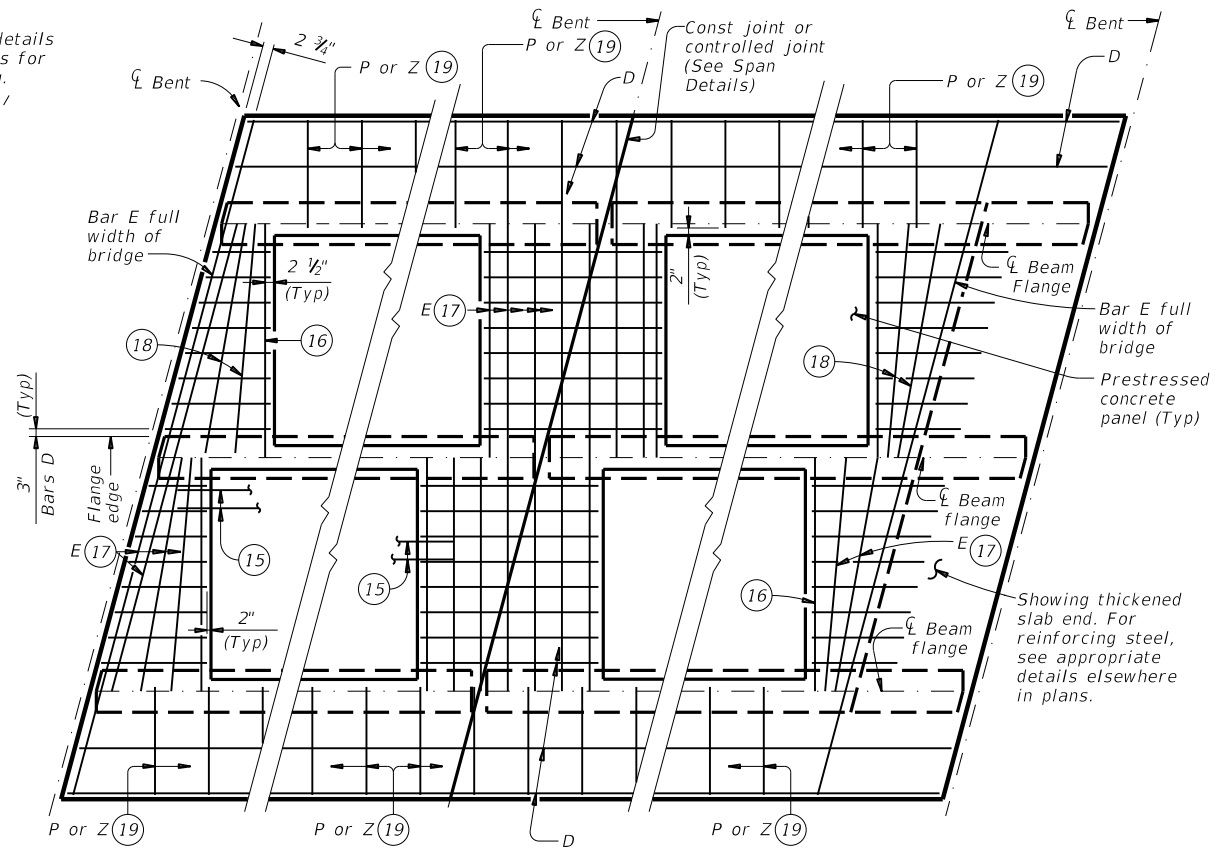
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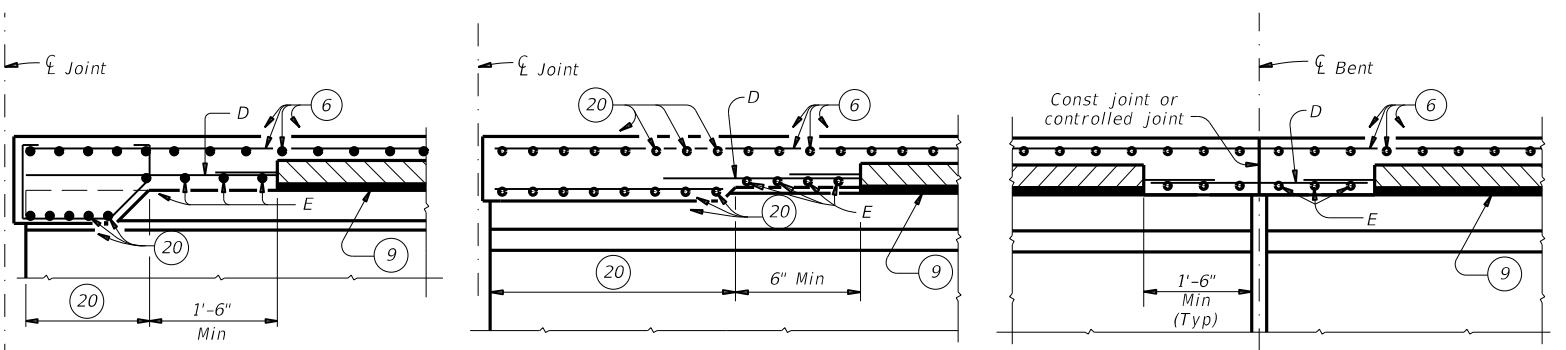
AT ALL SPAN ENDS UNLESS NOTED OTHERWISE
 AT INTERIOR BENTS
 AT THICKENED END SLABS

OPTION 1 ~ PLAN OF SLABS WITH NORMAL REINFORCEMENT

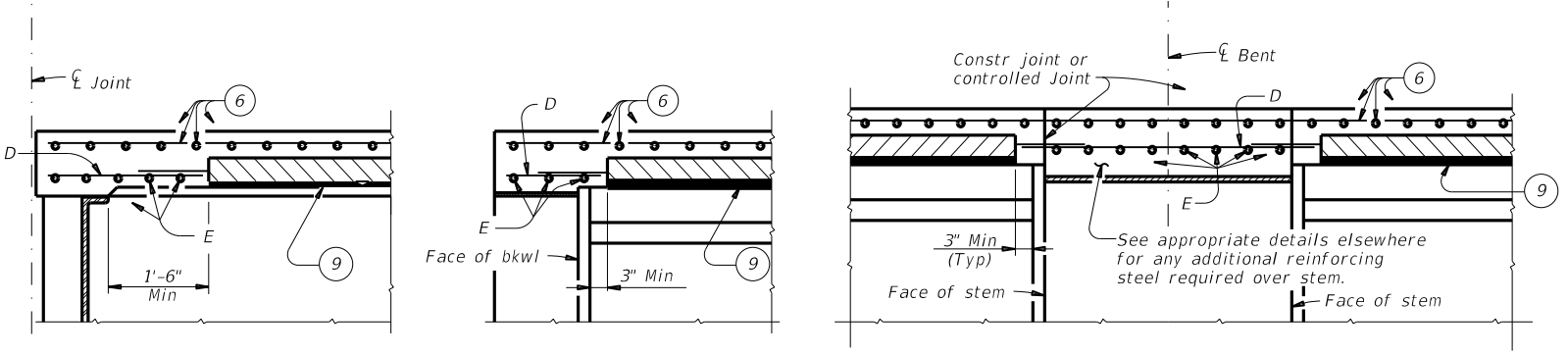


AT ALL SPAN ENDS UNLESS NOTED OTHERWISE
 AT INTERIOR BENTS
 AT THICKENED END SLABS

OPTION 1 ~ PLAN OF SLABS WITH SKEWED REINFORCEMENT



AT THICKENED SLAB ENDS FOR PRESTR CONC U-BMS
 AT THICKENED SLAB ENDS FOR PRESTR CONC I-BMS AND STEEL BMS
 AT SLAB CONTINUOUS OVER CONVENTIONAL INTERIOR BENTS FOR ALL SIMPLE SPAN BMS



AT CONVENTIONAL END DIAPHRAGMS FOR STEEL BMS
 AT SLAB OVER ABUTMENT BACKWALL FOR ALL BMS
 AT SLAB CONTINUOUS OVER INVERTED-T BENTS FOR ALL BMS

OPTION 1 ~ ELEVATIONS AT BEAM ENDS

- 6 See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- 9 Butt adjacent bedding strips together with adhesive. Cut v-notches, approx 1/4" deep, in the top of the bedding strips at 8' o.c.
- 14 Max Spacing as listed unless otherwise shown.
- 15 At connection with cast-in-place slab, extend longitudinal panel reinforcement. See PCP-FAB for details.
- 16 Maintain one Bar E(#4) parallel to panel ends (Typ).
- 17 Bars E(#4) not continuous over beam flanges must overlap beam flange 6" Min.
- 18 Add flared Bars E(#4) (Min Spa = 6", Max Spa = 12") as required at panel ends.
- 19 Where possible, Bars E(#4) may be extended into overhangs to replace Bars P(#4). Bars Z(#4) are required for sloped overhangs with U-Beams.
- 20 See appropriate thickened slab end details for reinforcing and limits of thickened slab end.

| TABLE OF REINFORCING STEEL (14) | | |
|---------------------------------|------|---------------|
| BAR | SIZE | Max Spa (in.) |
| D | #4 | 9 |
| E | #4 | 9 |
| P | #4 | 18 |
| UP | #4 | ~ |
| Z | #4 | 18 |

HL93 LOADING SHEET 3 OF 4

Texas Department of Transportation
 Bridge Division Standard

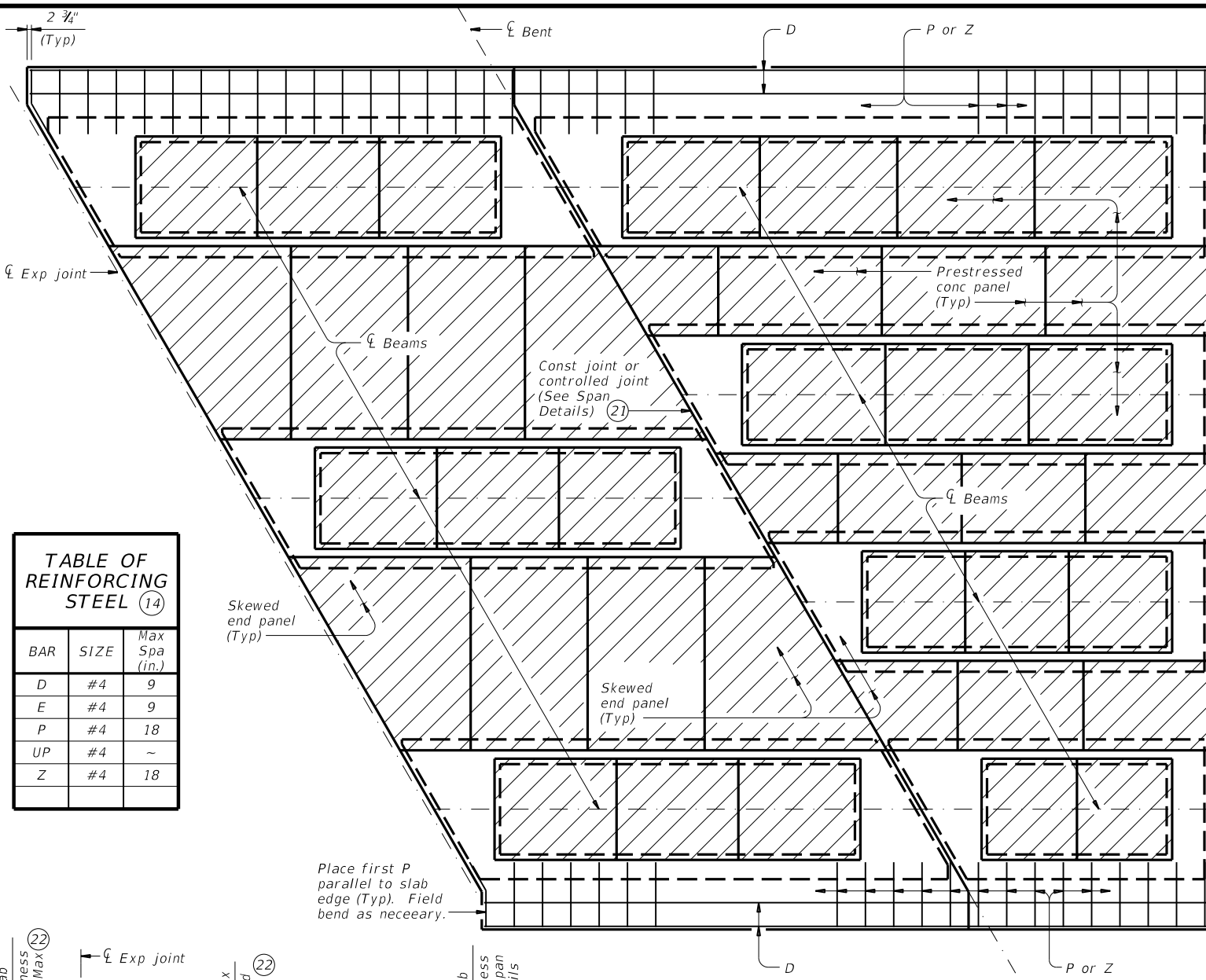
PRESTRESSED CONCRETE PANELS DECK DETAILS

PCP

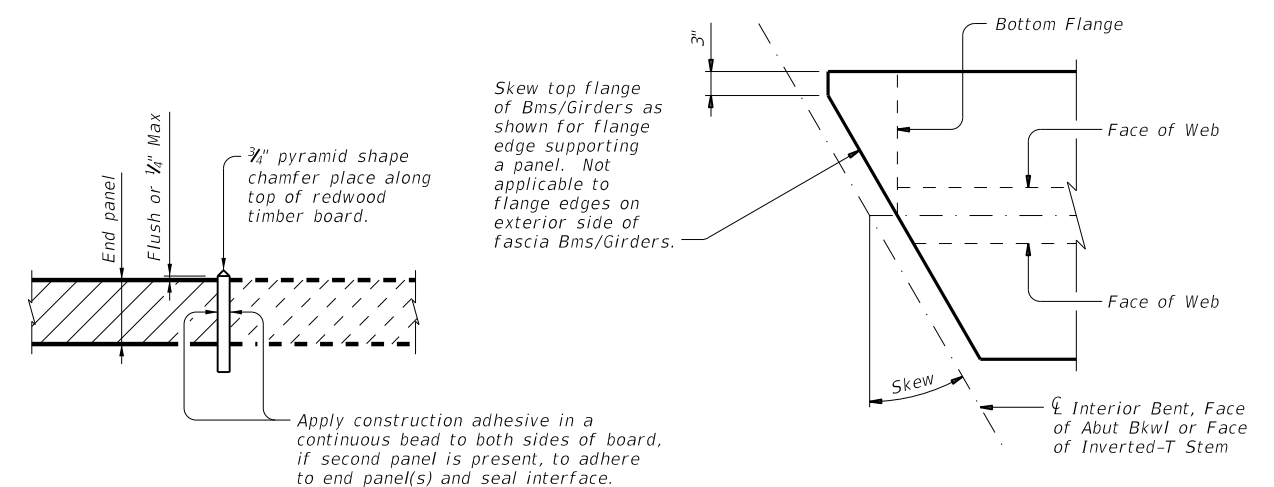
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| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | | SHEET NO. | |
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| TABLE OF REINFORCING STEEL (14) | | |
|---------------------------------|------|---------------|
| BAR | SIZE | Max Spa (in.) |
| D | #4 | 9 |
| E | #4 | 9 |
| P | #4 | 18 |
| UP | #4 | ~ |
| Z | #4 | 18 |



ELEVATION EXAMPLE OF END PANEL AND TIMBER BOARD (23)

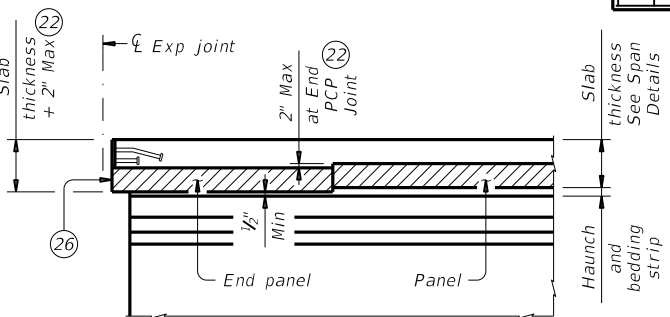
See "Option 2 ~ Elevation At Beam Ends".

OPTION 2 ~ SHOWING MODIFICATION TO BEAM/GIRDER TOP FLANGE FOR SKEWS OVER 5°

Showing I-Bm/I-Girder, U-Bms and Steel Bms similar.

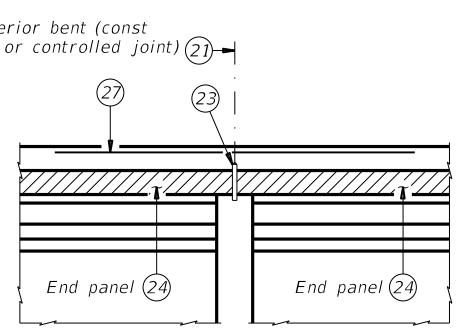
OPTION 2 ~ PLAN OF SLAB

(Showing U-Beams; other beams similar)



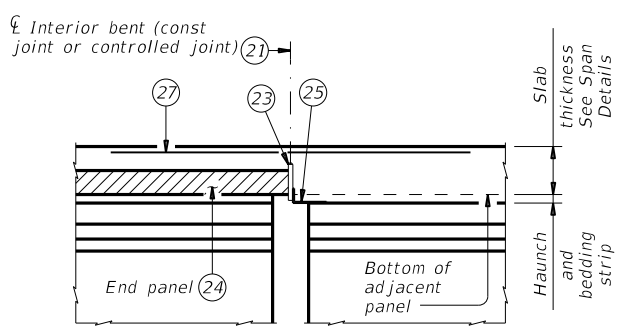
JOINTS (BETWEEN BEAMS/GIRDERS OR AT INV-T STEM)

For SEJ-B, SEJ-M, SEJ-S(0), AJ, and Type A expansion joints only.



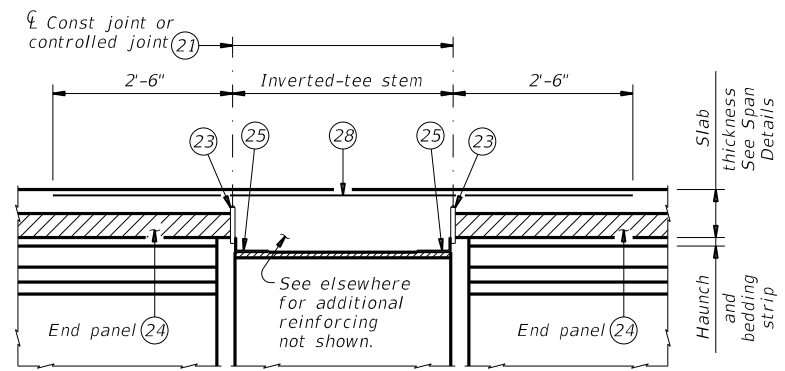
CONVENTIONAL INTERIOR BENT

Panel against panel between beams/girders.



CONVENTIONAL INTERIOR BENT

Panel against beam/girder end in adjacent span.



INVERTED-T BENT

Panels against inverted-tee stem

OPTION 2 ~ ELEVATIONS AT BEAM ENDS (6)

- (6) See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- (14) Max Spacing as listed unless otherwise shown.
- (21) 1 1/2" Vinyl or plastic joint former at controlled joints (Stress Cap, Zip Strip, Stress Lock, or equal as approved by the Engineer.)
- (22) End panel may be set up to 2" lower to accommodate expansion joint hardware, provided bedding strip is not less than 1/2" thick.
- (23) 3/4" thick redwood timber board, leave in place. Redwood timber board placed flush with top of panel or within 1/4" Max above panel. Place 3/4" pyramid shape chamfer along top of timber board. See "Elevation Example of End Panel and Timber Board". Place straight, within 1/4" of centerline of bent or face of inverted-tee, across bridge width and end board at exterior flange edge of fascia beams/girders. Do not extend into overhang.
- (24) Place panel within 1/2" of 3/4" thick board.
- (25) Permanent galvanized steel sheet form. Removable formwork is acceptable.
- (26) Place end panel within 1/2" of expansion joint opening. End panel cannot encroach on required expansion joint opening.
- (27) Place additional (#4) bar 5'-0" in length between every slab bars T. Center (#4) bar on joint.
- (28) Place additional (#4) bar continuous 2'-6" beyond each side of Inverted-T Stem between every slab bars T.

SPECIAL OPTION 2 CONSTRUCTION NOTES:

When Option 2 is chosen bottom mat of thickened end slab reinforcing is not required. Use the same top mat as shown on the Thickened Slab End Details sheet.
 Placing panels adjacent to expansion joints and bent centerlines prior to completing interior panel placement is recommended. Saw cutting panels to fit is acceptable when approved by the Engineer. Minimum distance from a saw cut edge to a panel strand is 1 1/2".
 Do not extend the longitudinal panel reinforcement into the cast-in-place slab.
 Top flanges of beams and girders on skewed bridges must be modified as shown on this drawing. The Contractor is responsible for coordinating this modification with the beam fabricator prior to submitting shop drawings for approval.
 Fabricator may optionally skew the whole end. When electing to skew whole end, girder end details and bearing type at conventional interior bent must be changed to use condition at abutment. Fabricator must coordinate change in bearing type, bearing centerline location, and dowel location with Engineer and Contractor. Show appropriate changes on girder and bearing shop drawings.
 Bending of anchor studs of expansion joints shown on standards AJ, SEJ-B, SEJ-M, and SEJ-S(0) is permissible if necessary to clear top of end panels. The Contractor is responsible for coordinating modifications with the joint fabricator. Submit shop drawings for approval when modifications to expansion joint hardware are made.
 Bedding strips under skewed end panels must conform to the requirements of Item 422 except their minimum compressive strength must be 60 psi.
 Provide Bars AA, G, K and OA from standard IGTS in the slab.

HL93 LOADING SHEET 4 OF 4

Texas Department of Transportation Bridge Division Standard

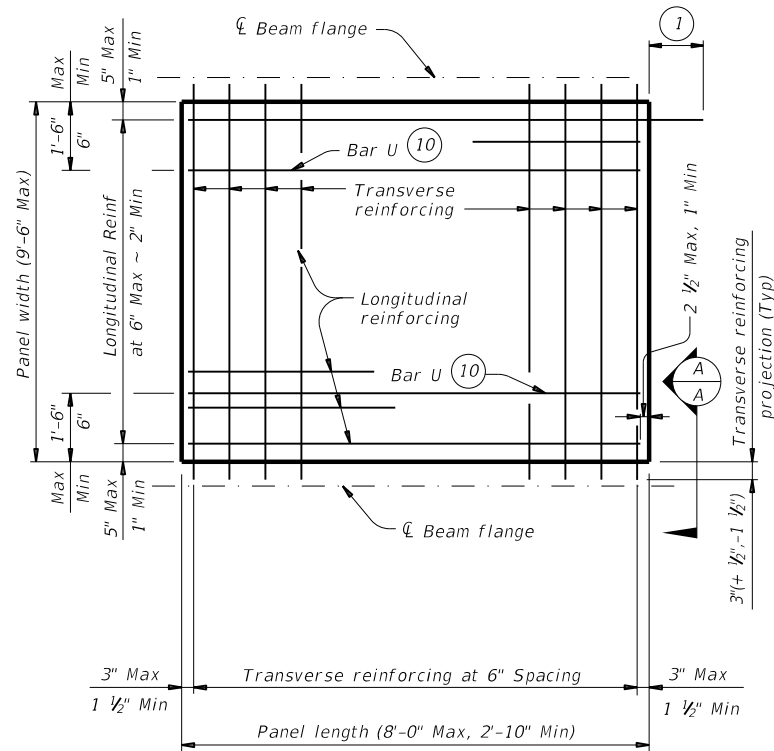
PRESTRESSED CONCRETE PANELS DECK DETAILS

PCP

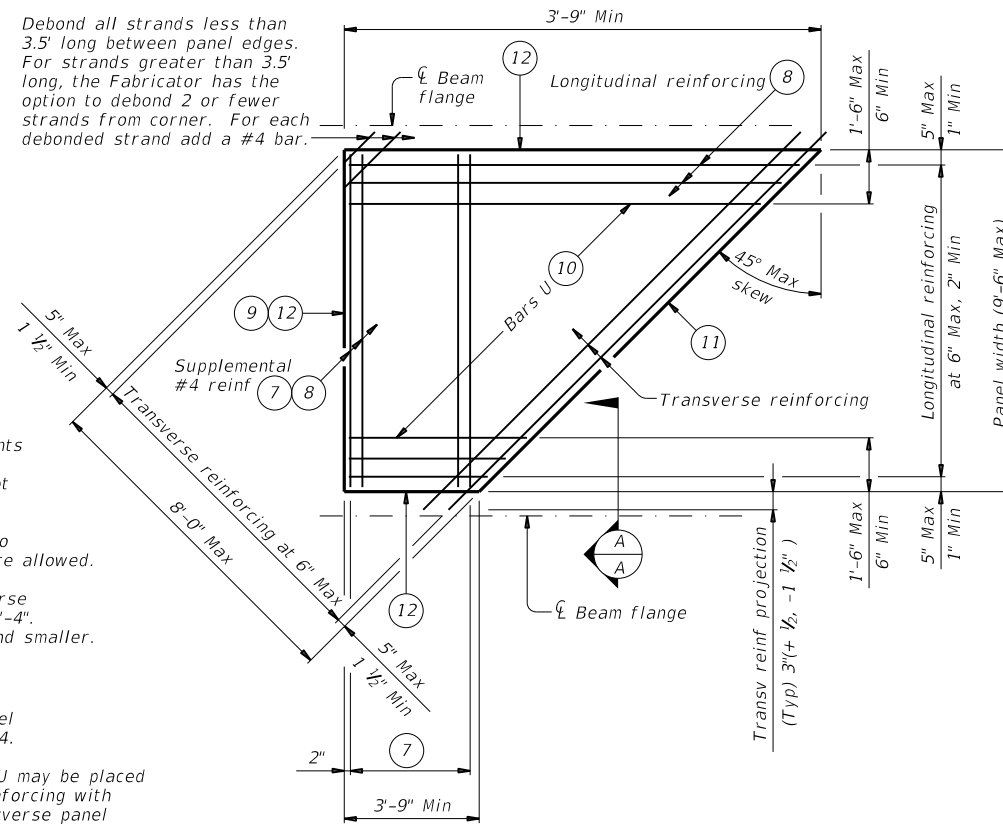
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| DIST | COUNTY | | SHEET NO. | |
| AUS | WILLIAMSON | | 109 | |

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TYPICAL NON-SKEWED PANEL PLAN



TYPICAL SKEWED END PANEL PLAN

(Only to be used with details shown elsewhere in the plans.)

- 1 At connection with cast-in-place slab, extend longitudinal panel reinforcement 1'-0" (+2", -0") past panel end. Alternatively, provide (#3) x 2'-0" dowels at 6" Max Spacing and extend dowels 1'-0" past panel end.
- 2 Four loops required per panel.
- 3 Four loops required per panel. 3/8" or 1/2" strands may be used.
- 4 Normal dimensions must be used on spans with parallel beams. Maximum and Minimum dimensions apply only to spans with flared beams.
- 5 See Normal Grading Detail on PCP standard for lap requirements and bedding strip dimensions. Some laps shown in tables cannot utilize all bedding strip widths.
- 6 One Splice allowed per panel. No more than two sheets of WWR are allowed.
- 7 Provide (#4) bars under transverse reinforcing, 10 Spaces at 4" = 3'-4". Omit for 5 degree (1:12) skew and smaller.
- 8 End Cover 2 1/2" Max, 1" Min.
- 9 Recess strands on indicated panel edge in accordance with Item 424.
- 10 At the fabricator's option, Bars U may be placed parallel to transverse panel reinforcing with horizontal legs in plane of transverse panel reinforcing.
- 11 Use length of indicated panel edge as panel width for purpose of determining type of transverse reinforcing.
- 12 Timber form work permissible this edge.

| TABLE A (4) (5) | | | | TABLE B (4) (5) | | | |
|-----------------|--------------|-----------|-----------|------------------|--------------|-----------|-----------|
| Beam Type | Normal (In.) | Min (In.) | Max (In.) | Top Flange Width | Normal (In.) | Min (In.) | Max (In.) |
| A | 3 | 2 1/2 | 3 1/2 | 11" to 12" | 2 3/4 | 2 1/2 | 2 3/4 |
| B | 3 | 2 1/2 | 3 1/2 | Over 12" to 15" | 3 1/4 | 3 | 3 1/4 |
| C | 4 | 3 | 4 1/2 | Over 15" to 18" | 4 | 3 | 4 3/4 |
| IV | 6 | 4 | 7 1/2 | Over 18" | 5 | 3 1/2 | 6 1/4 |
| VI | 6 1/2 | 4 1/2 | 8 1/2 | | | | |
| U40 - 54 | 5 1/2 | 5 1/2 | 7 | | | | |
| Tx28-70 | 6 | 5 | 7 1/2 | | | | |
| XB20 - 40 | 4 | 3 | 4 1/2 | | | | |
| XSB12 - 15 | 4 | 3 | 4 1/2 | | | | |

GENERAL NOTES:

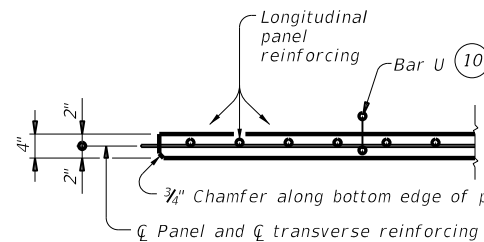
Provide Class H concrete for panels. Release strength $f'_{ci}=3,500$ psi. Minimum 28 day strength $f'_{c}=5,000$ psi.
 Provide 3/4" chamfer along bottom edge of panel on beam side. Do not use epoxy-coated reinforcing steel bar or strand in panels. Remove laitance from top panel surface. Finish top of panel to a roughness between a No. 6 and No. 9 concrete surface profile, inclusive, as specified by the International Concrete Repair Institute (ICRI).
 Shop drawings for the fabrication of panels will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.
 A panel layout which identifies location of each panel must be developed by the Fabricator. Permanently mark each panel in accordance with the panel layout. A copy of the layout is to be provided to the Engineer.

TRANSVERSE PANEL REINFORCEMENT:

For panel widths over 5', use 3/8" or 1/2" Dia (270k) prestressing strands with a tension of 14.4 kips per strand.
 For panel widths over 3'-6" up to and including 5', use 3/8" or 1/2" Dia (270k) prestressing strands with a tension of 14.4 kip per strand. Optionally, (#4) Grade 60 reinforcing bars may be used in lieu of prestressed strands.
 For panel widths up to 3'-6", use (#4) Grade 60 reinforcing bars (prestressed strands alone are not allowed).
 Place transverse panel reinforcement at panel centroid and space at 6" Max.

LONGITUDINAL PANEL REINFORCEMENT:

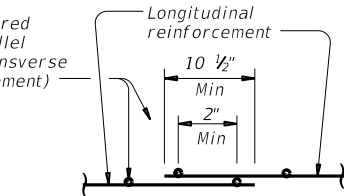
Any of the following options may be used for longitudinal panel reinforcement:
 1. (#3) Grade 60 reinforcing steel at 6" Max Spacing. No splices allowed.
 2. 3/8" Dia prestressing strands at 4 1/2" Max Spacing (unstressed). No splices allowed.
 3. 1/2" Dia prestressing strands at 6" Max Spacing (unstressed). No splices allowed.
 4. Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) providing 0.22 sq in per foot of panel width. Wires larger than D11 not permitted. Provide transverse wires to ensure proper handling of reinforcing. One splice per panel is allowed. See WWR Splice Detail.
 No combination of longitudinal reinforcement options in a panel is allowed. Place longitudinal panel reinforcement above or below transverse panel reinforcement. Must be placed above transverse panel reinforcement for skewed end panels with supplemental (#4) reinforcement.



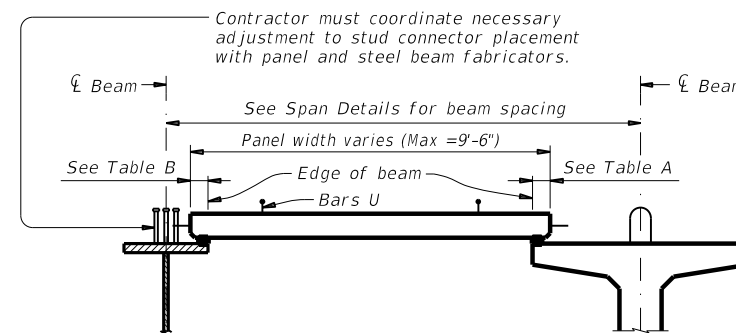
SECTION A-A

(Not showing supplemental #4 bars for skewed end panels.)

No splice required for wires parallel to strands (transverse panel reinforcement)

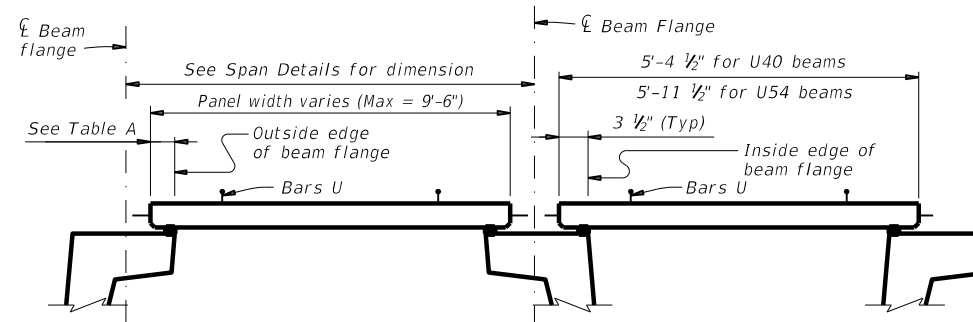


WELDED WIRE REINFORCEMENT (WWR) SPLICE DETAIL



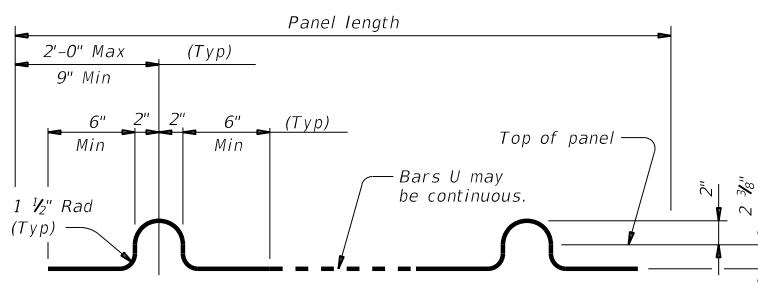
STEEL BEAMS

PRESTRESSED CONCRETE BEAMS OR GIRDERS
 Typ unless noted otherwise

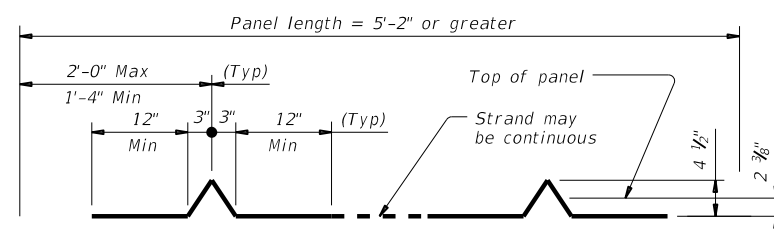


PRESTRESSED CONCRETE U-BEAMS

TYPICAL SECTIONS FOR DETERMINING PANEL WIDTH



BARS U (#3)



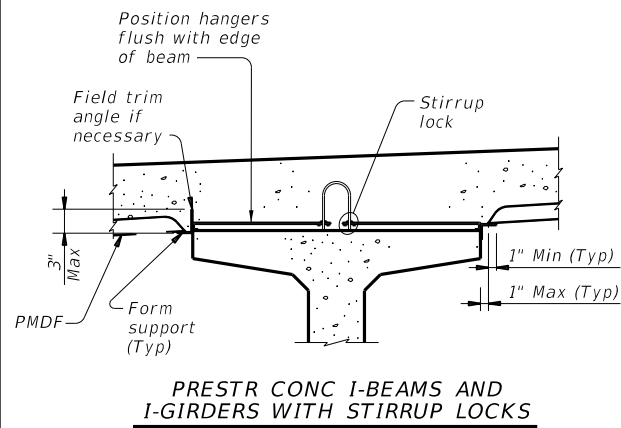
OPTIONAL STRAND FOR BARS U

HL93 LOADING

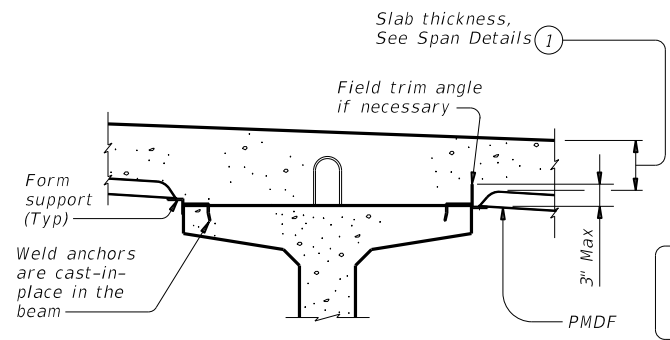
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| | | Bridge Division Standard | |
| PRESTRESSED CONCRETE PANEL FABRICATION DETAILS | | | |
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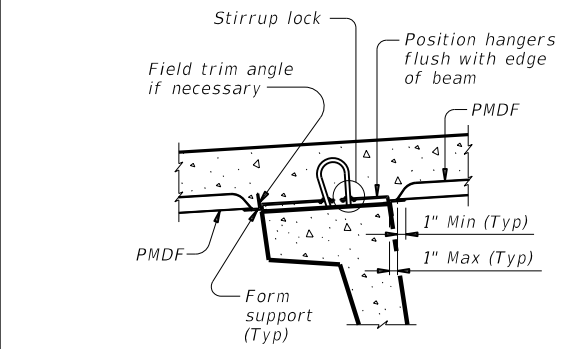
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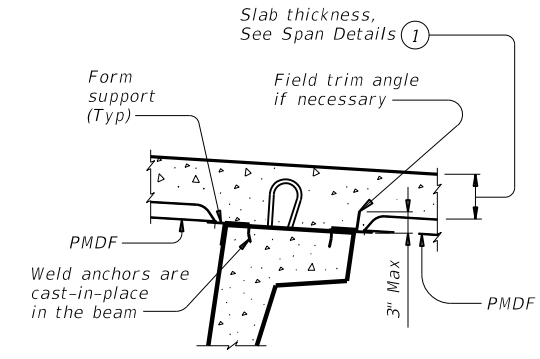
PRESTR CONC I-BEAMS AND I-GIRDERS WITH STIRRUP LOCKS



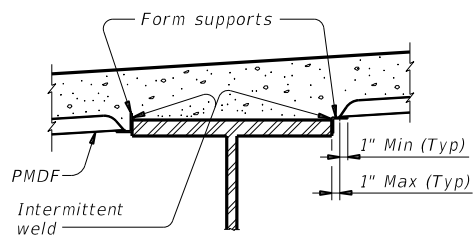
PRESTR CONC I-BEAMS AND I-GIRDERS WITH WELD ANCHORS



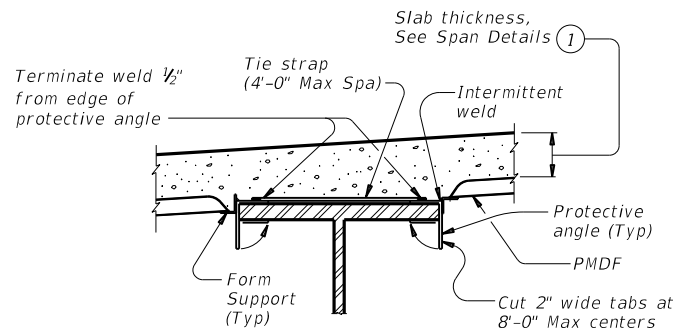
U-BEAMS WITH STIRRUP LOCKS



U-BEAMS WITH WELD ANCHORS

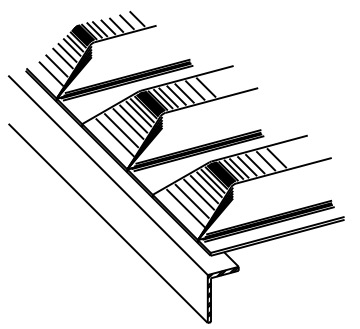


STEEL BEAMS AT COMPRESSION FLANGES

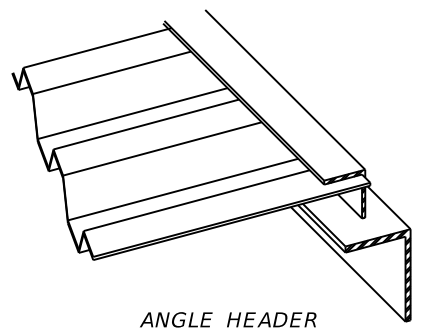


STEEL BEAMS AT TENSION FLANGES

TYPICAL TRANSVERSE SECTIONS



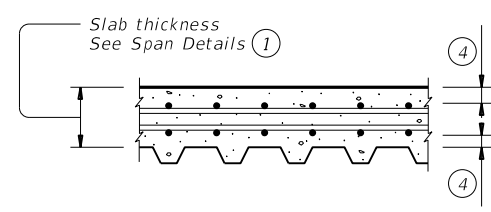
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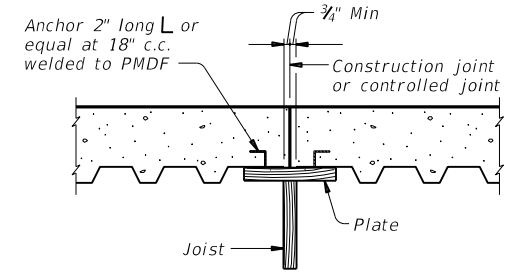
ANGLE HEADER

NOTE: This type is to be used for skewed ends only.

TYPES OF END CLOSURES



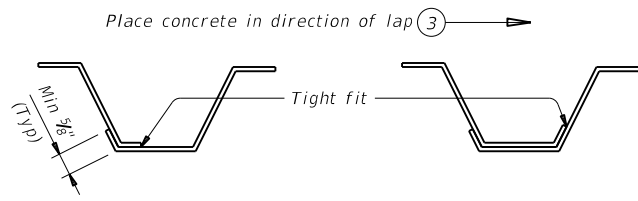
TYP LONGITUDINAL SLAB SECTION



Note: In spans where PMD forms are used, timber forms must be used at construction joints. Adequate provision must be made to support edge of metal form and to provide anchorage of metal form to slab concrete where joined to wood forms.

SECTION THRU CONSTRUCTION JOINT

FOR PRESTR CONC U-BEAM AND STEEL GIRDER BRIDGES:
 Unless shown elsewhere in the plans, size, spacing, and orientation of bottom mat of slab reinforcement must match the top mat of reinforcing shown on the span details except all bottom mat bars are to be #5. Bottom mat reinforcement and additional concrete is subsidiary to Item 422 "Concrete Superstructures."
FOR PRESTR CONC TX-GIRDER BRIDGES:
 See Miscellaneous Slab Details, Prestr Concrete I-Girders (IGMS) standard sheet for bottom mat reinforcing.



SIDE LAP DETAILS

- 1 Slab thickness minus 5/8" if corrugations match reinforcing bars.
- 2 Welding of form supports to tension flanges will not be permitted. Other methods of providing wind hold down resistance for PMDF in tension flange zones will be considered. At least one layer of sheet metal must be provided between the flange and the weld joint.
- 3 The direction of concrete placement will be such that the upper layer of the form overlap is loaded first.
- 4 See Span details for cover requirements.

GENERAL NOTES:

Steel for Permanent Metal Deck Forms (PMDF) and support angles shall conform to ASTM A653, structural steel (SS), with coating designation G165. Steel must have a minimum yield strength of 33 ksi. Minimum thickness of PMDF is 20 gage and that of support angles and protective angles is 12 gage.
 Submit two copies of forming plans for PMDF to the Engineer. These plans must show all essential details of proposed form sheets, closures, fasteners, supports, connectors, special conditions and size and location of welds. These plans must clearly show areas of tension flanges for steel beams and provisions for protecting the tension flanges from welding notch effects by inclusion of separating sheet metal or other positive method. These plans must be designed, signed, and sealed by a licensed professional engineer. Department approval of these plans is not required, but the Department reserves the right to require modifications to the plans. The Contractor is responsible for the adequacy of these plans.
 The details and notes shown on this standard are to be used as a guide in preparation of the forming plans.
 All material, labor, tools and incidentals necessary to form a bridge deck with Permanent Metal Deck Forms is considered subsidiary to Item 422, "Concrete Superstructures".

DESIGN NOTES:
 As a minimum, PMDF and support angles must be designed for the dead load of the form, reinforcement and concrete plus 50 psf for construction loads. Flexural stresses due to these design loads must not exceed 75 percent of the yield strength of the steel. Allowable stress for weld metal must be 12,400 psi.
 Maximum deflection under the weight of forms, reinforcement and concrete or 120 psf, whichever is greater, shall not exceed the following:

- 1/180 of the form design span, but not more than 0.50", for design spans of 10' or less.
- 1/240 of the form design span, but not more than 0.75", for all design spans of railroad overpass bridge spans fully or partially over railroad right-of-way, and for all bridge spans of railroad underpass structures.
- 1/240 of the form design span, but not more than 0.75", for all design spans of railroad overpass bridge spans fully or partially over railroad right-of-way, and for all bridge spans of railroad underpass structures.

The form design span must not be less than the clear distance between beam flanges, measured parallel to the form flutes, minus 2".

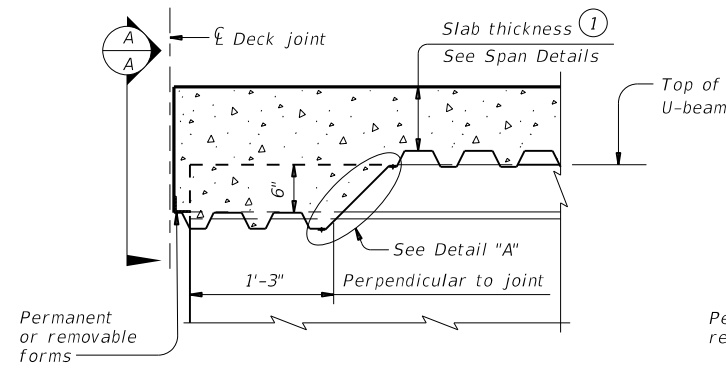
CONSTRUCTION NOTES:

Form sheets must not be permitted to rest directly on the top of beam flanges. Form sheets must be securely fastened to form supports and must have a minimum bearing length of one inch at each end. Form supports must be placed in direct contact with beam flanges.
 All attachments must be made by permissible welds, screws, bolts, clips or other means shown on the the forming plans. All sheet metal assembly screws must be installed with torque-limiting devices to prevent stripping. Only welds or bolts must be used to support vertical loads.
 Welding and welds must be in accordance with the provisions of Item 448, "Structural Field Welding", pertaining to fillet welds. All welds must be made by a qualified welder in accordance with Item 448.
 All permanently exposed form metal, where the galvanized coating has been damaged, must be thoroughly cleaned and repaired in accordance with Item 445, "Galvanizing". Minor heat discoloration in areas of welds need not be touched up.
 Flutes must line up uniformly across the entire width of the structure where main reinforcing steel is located in the flute.
 Construction joints will not be permitted unless shown on the plans. The location of and forming details for any construction joint used must be shown on the forming plans. Forms below a construction joint must be removed after curing of the slab.
 A sequence for uniform vibration of concrete must be approved by the Engineer prior to concrete placement. Attention must be given to prevent damage to the forms, yet provide proper vibration to prevent voids or honeycomb in the flutes and at headers and/or construction joints.

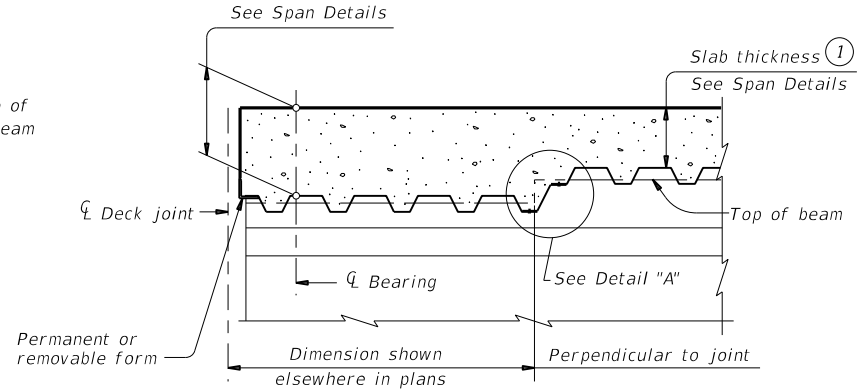
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| | | Bridge Division Standard | |
| PERMANENT METAL DECK FORMS | | | |
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| ©TxDOT April 2019 | CON: 0914 | SECT: 05 | JOB: 204, ETC. |
| 02-20: Modified box note by adding steel beams/girders and subsidiary | DIST: AUS | COUNTY: WILLIAMSON | SHEET NO: 111 |
| 12-21: Updated max deflection for RR. | | | |

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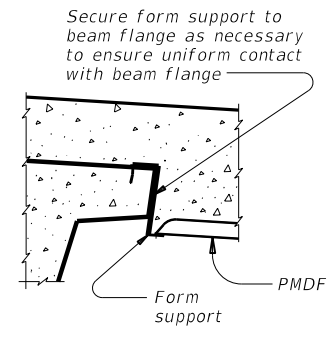
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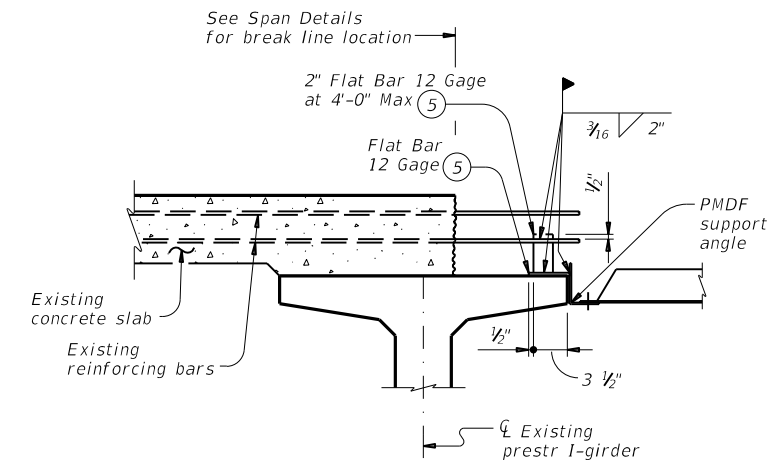
AT THICKENED SLAB END FOR U-BEAMS



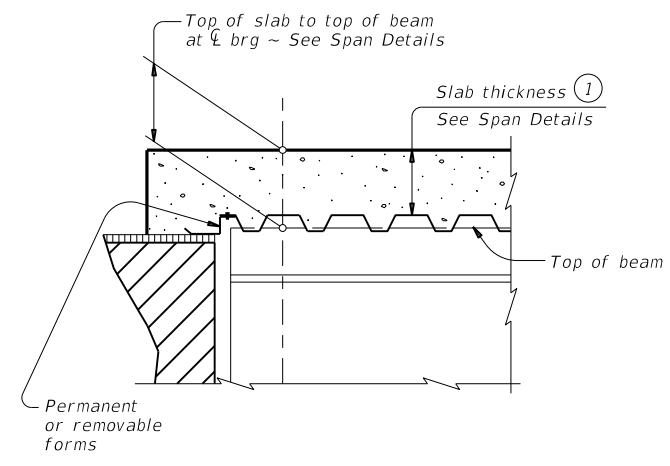
AT THICKENED SLAB END FOR PRESTRESSED I-BEAMS, I-GIRDERS AND STEEL BEAMS
 Showing I-beam block-out. No block-out for I-girders or steel beams.



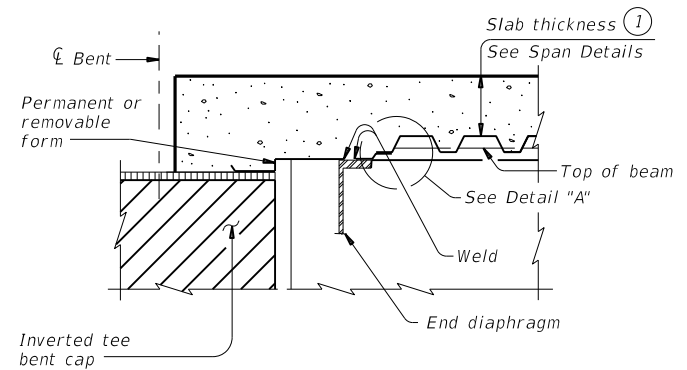
SECTION A-A



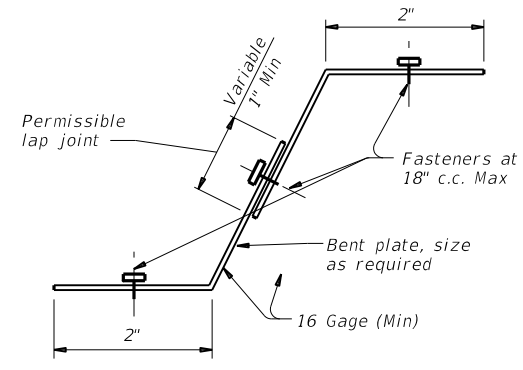
SHOWING PRESTRESSED CONCRETE I-BEAMS, I-GIRDERS AND U-BEAMS



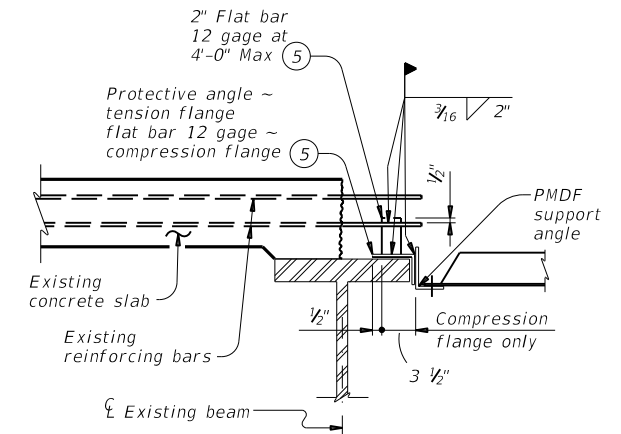
AT SLAB OVER ABUT BKWL OR INV TEE STEM FOR CONC BEAMS WITHOUT THICKENED SLAB END



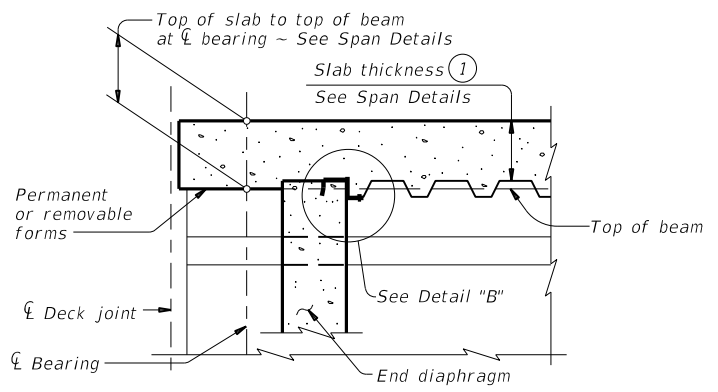
AT SLAB OVER INV TEE STEM FOR STEEL BEAMS WITHOUT THICKENED SLAB END



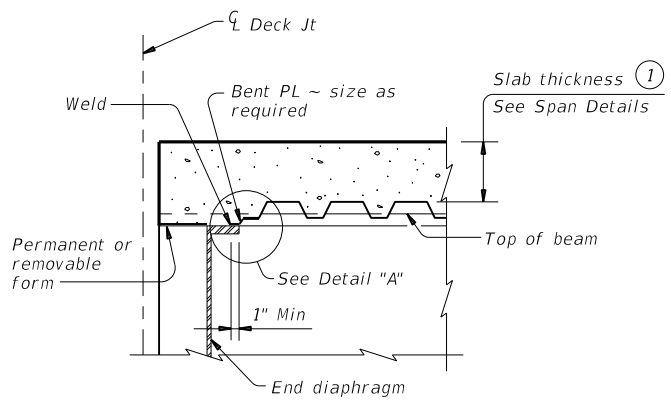
DETAIL "A"



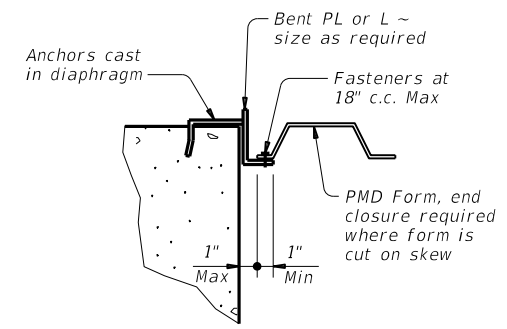
SHOWING STEEL BEAMS



AT CONC END DIAPHRAGM FOR PRESTRESSED I-BEAMS AND STEEL BEAMS



AT END DIAPHRAGM FOR STEEL BEAMS WITHOUT THICKENED SLAB END



DETAIL "B"

- ① Slab thickness minus 5/8" if corrugations match reinforcing bars
- ⑤ Minimum yield stress of 12 gage bars shall be 40 ksi

DETAILS AT ENDS OF BEAMS

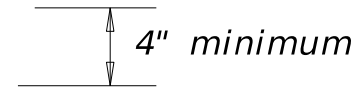
WIDENING DETAILS

SHEET 2 OF 2

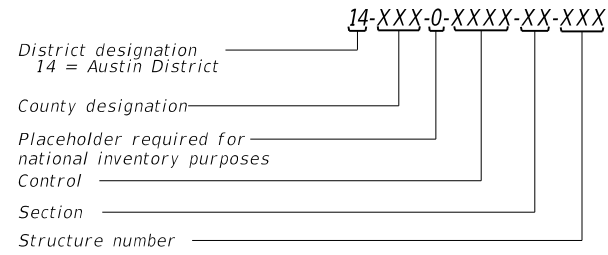
| | | | |
|---|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| PERMANENT METAL DECK FORMS | | | |
| PMDF | | | |
| FILE: pmdfstel-21.dgn | DN: TxDOT | CK: TxDOT | OW: TxDOT |
| ©TxDOT April 2019 | CONT | SECT | JOB |
| REVISIONS 02-20: Modified box note by adding steel beams/girders and subsidiary 12-21: Updated max deflection for RR. | | 0914 05 204, ETC. | CR 118 |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 112 | |

14-XXX-0-XXXX-XX-XXX

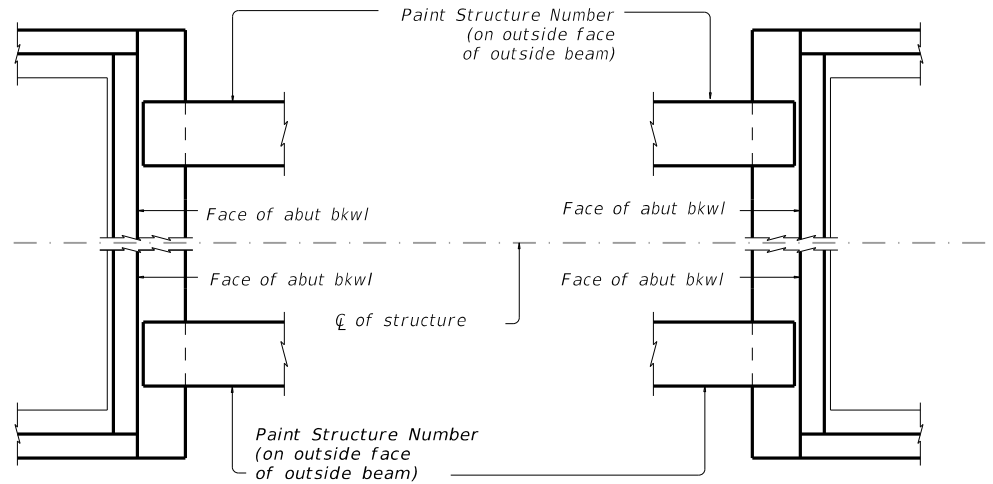
District designation County designation Placeholder Control Section Structure number



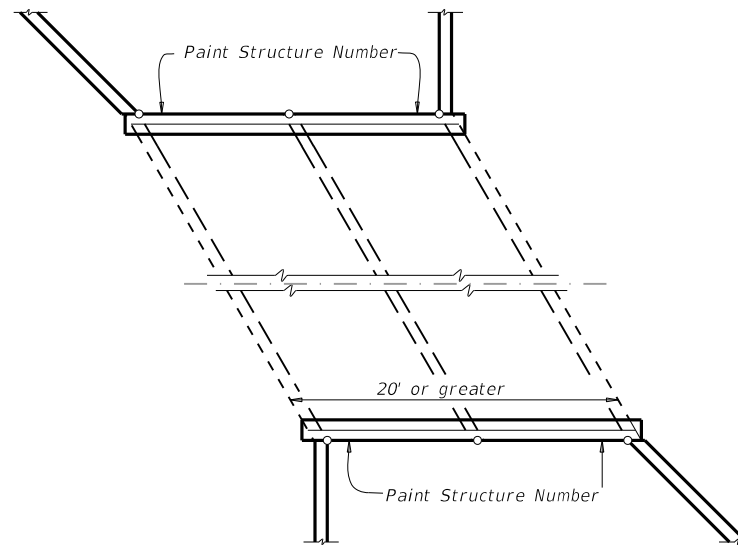
PAINTED STRUCTURE NUMBER LEGEND



- 011 = Bastrop
- 016 = Blanco
- 027 = Burnet
- 028 = Caldwell
- 087 = Gillespie
- 106 = Hays
- 144 = Lee
- 150 = Llano
- 157 = Mason
- 227 = Travis
- 246 = Williamson



AT BRIDGE LOCATIONS



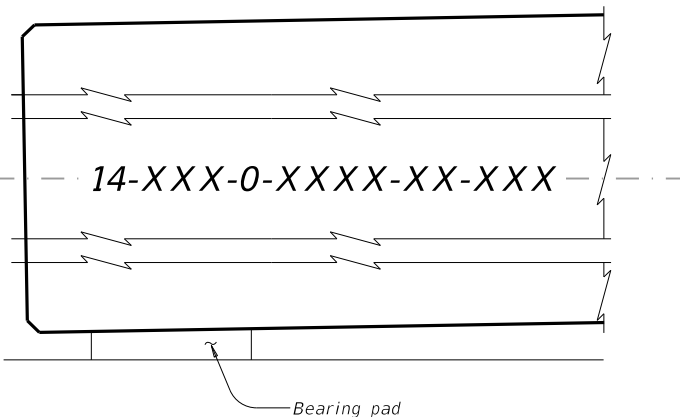
AT CULVERT LOCATIONS

GENERAL NOTES:

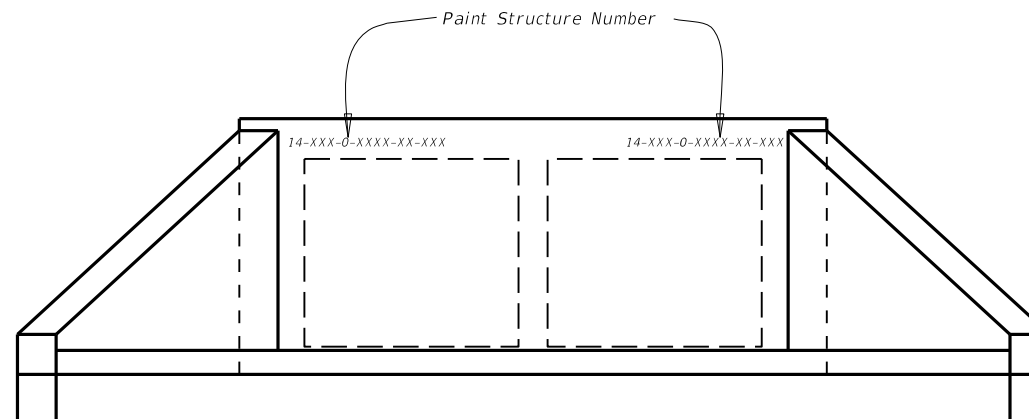
Permanently mark each structure with the painted structure number in accordance with the plans.
 Each Structure shall have 4 (four) Structure numbers painted per structure.
 Painting structure number work will not be measured or paid for directly but will be considered subsidiary to other pertinent items.

MATERIAL:

Provide black, lead free, CFC free, and CFHC free paint that is water proof, weather resistant, and dries instantly on all surfaces without smearing, smudging, or rippling



ELEVATION VIEW DETAIL



ELEVATION VIEW DETAIL

DATE: 11/3/2022 12:58:29 PM
FILE: c:\pw\knl\d0222653\psn-19.dgn



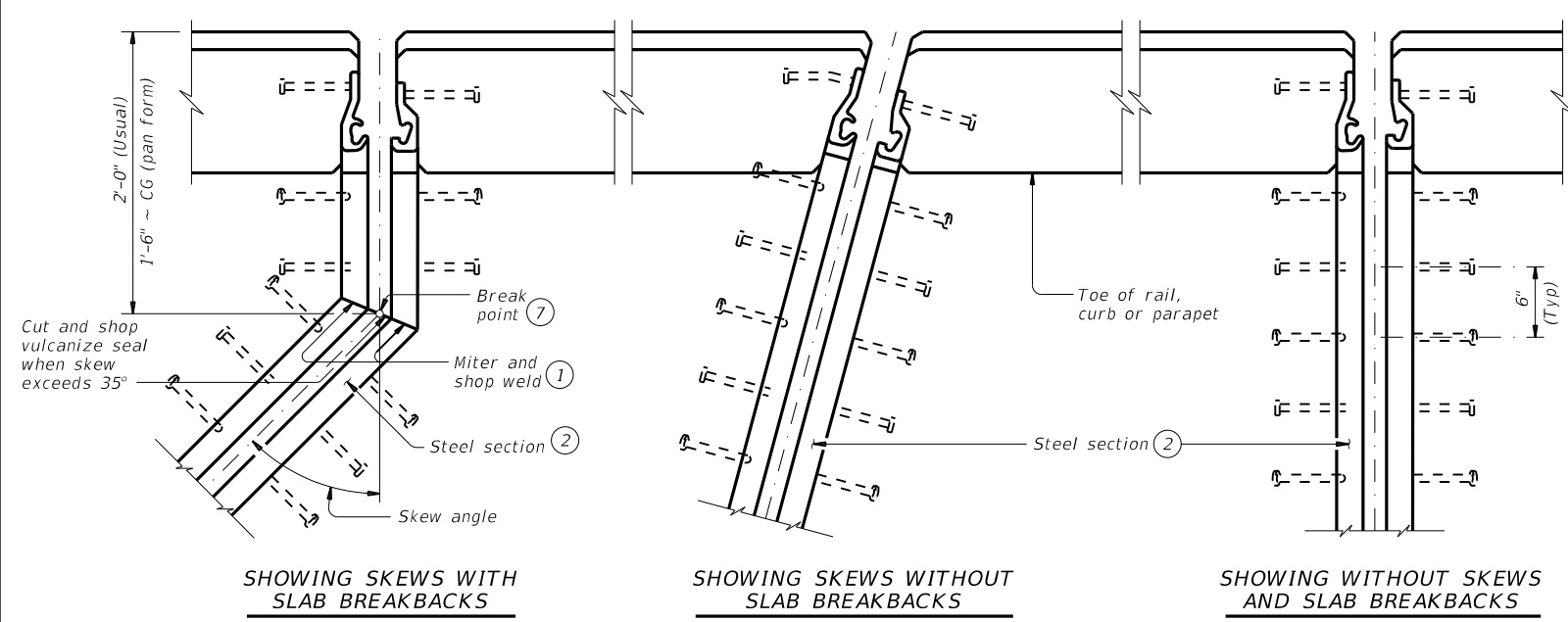
PAINTING STRUCTURE NUMBERS

PSN-19 (AUS)

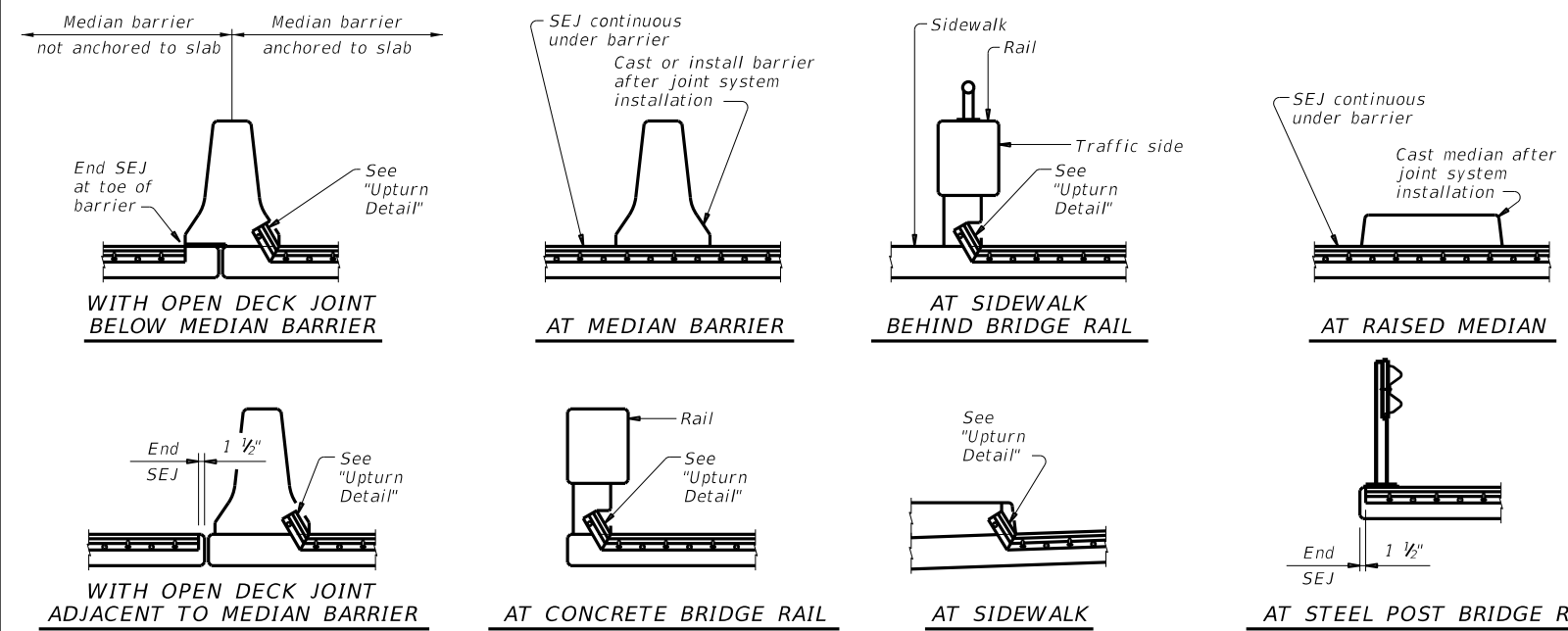
| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 113 |

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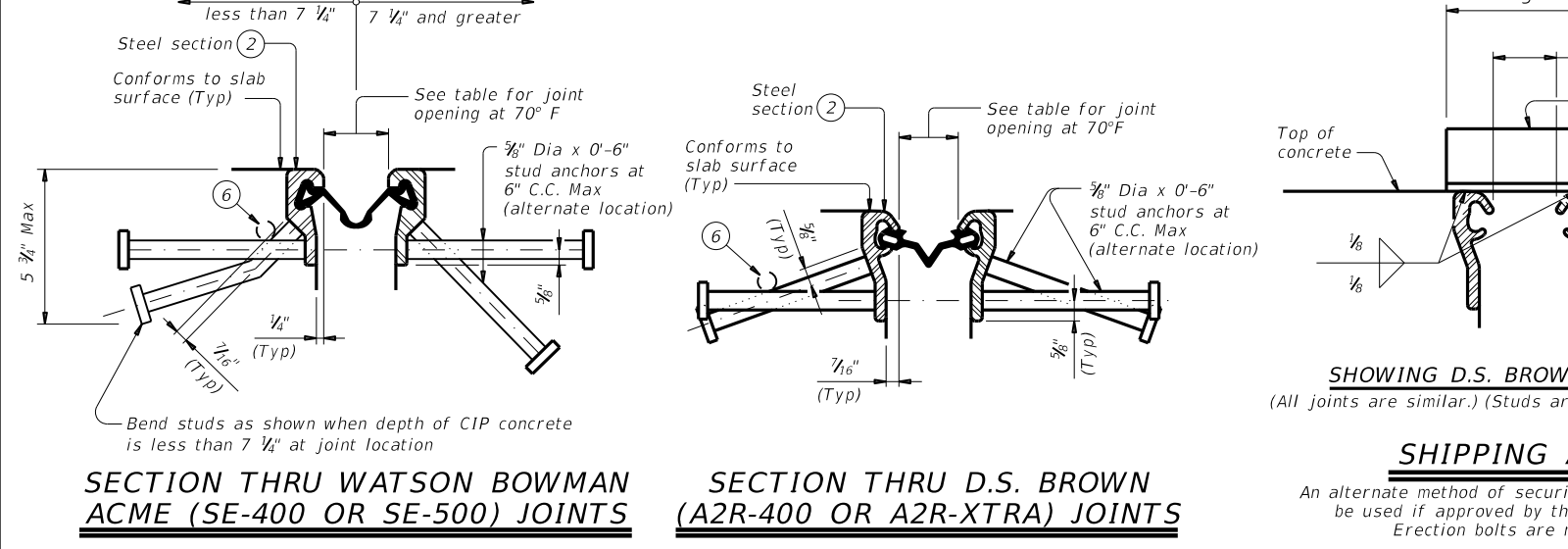
DATE: 11/23/2022 12:58:34 PM
 FILE: c:\pwworking\dot222653\sejmste1-19 (1).dgn



PLANS OF END CONDITIONS



TYPICAL SECTIONS

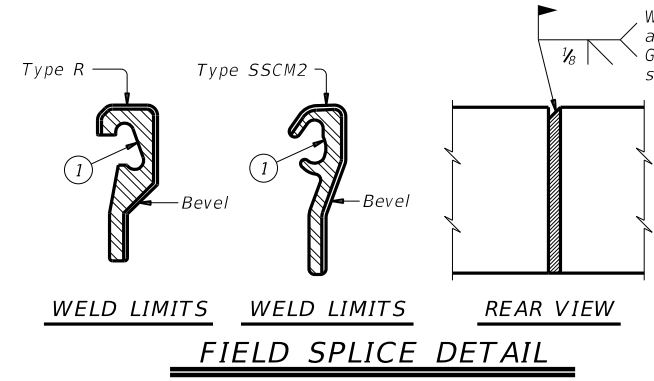


| TABLE OF SEALED EXPANSION JOINT INFORMATION | | | | | |
|---|-----------------|------------|-----------------|-----------|-----------------|
| MANUFACTURER | STEEL SECTION ② | STRIP SEAL | | | |
| | | 4" JOINT | | 5" JOINT | |
| | | Seal Type | Joint Opening ③ | Seal Type | Joint Opening ③ |
| D.S. Brown | Type SSCM2 | A2R-400 | 1 3/4" | A2R-XTRA | 2" |
| Watson Bowman Acme | Type R | SE-400 | 1 3/4" | SE-500 | 2" |

| SKEW (deg) | JOINT SIZE | |
|------------|------------|------|
| | 4" | 5" |
| 0 | 4.0" | 5.0" |
| 15 | 4.0" | 5.0" |
| 30 | 3.5" | 4.3" |
| 45 | 2.8" | 3.5" |

DESIGN NOTES:
 Joints installed on a skew have reduced ability to accommodate longitudinal movement. Use table values to determine the correct joint size for skewed installations. For other skews over 25 degrees, calculate reduced movement range by multiplying joint size by cosine (skew).

- Remove all burrs which will be in contact with seal prior to making splice.
- Shape of steel section shown is typical. Variations in sections must be approved by the Engineer.
- These openings are also the recommended minimum installation openings.
- Reduce for sidewalk or parapet heights less than 6".
- Other conditions affecting the joint profile should be noted elsewhere.
- Move transverse bars that are in conflict with SEJ studs, in either the bridge slab or approach slab, to rest at the junction of the studs.
- See Span details for location of break point.
- Align shipping angle perpendicular to joint.



FABRICATION NOTES:
 Temporarily shop assemble corresponding sections of sealed expansion joints (SEJ), check for fit, and match mark for shipment. Secure corresponding sections together for shipment with shipping angle. Do not use erection bolts.

The seal must be continuous and included in the price bid for sealed expansion joint. Ship steel sections in convenient lengths of 10'-0" Min and 24'-0" Max unless necessary for staged construction or widenings. One shop splice is permitted in each shipping length provided no piece is less than 2'-0" long and sufficient studs are added to limit the stud to shop splice distance to 2" Min and 4" Max.

Weld studs in accordance with AWS D1.1. Butt weld all shop and field splices and grind smooth areas in contact with seal. Make all necessary field splice joint preparations in the shop.

Paint the entire steel section with System II or IV primer in accordance with Item 446, "Field Cleaning and Painting Steel", unless required to galvanize when shown in the plans. Provide galvanizing in accordance with Item 445, "Galvanizing". Provide paints in accordance with Item 446.2. Prepare steel and apply paint in accordance with Item 446.7.3 and 446.7.4.

Shop drawings for the fabrication of sealed expansion joints will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.

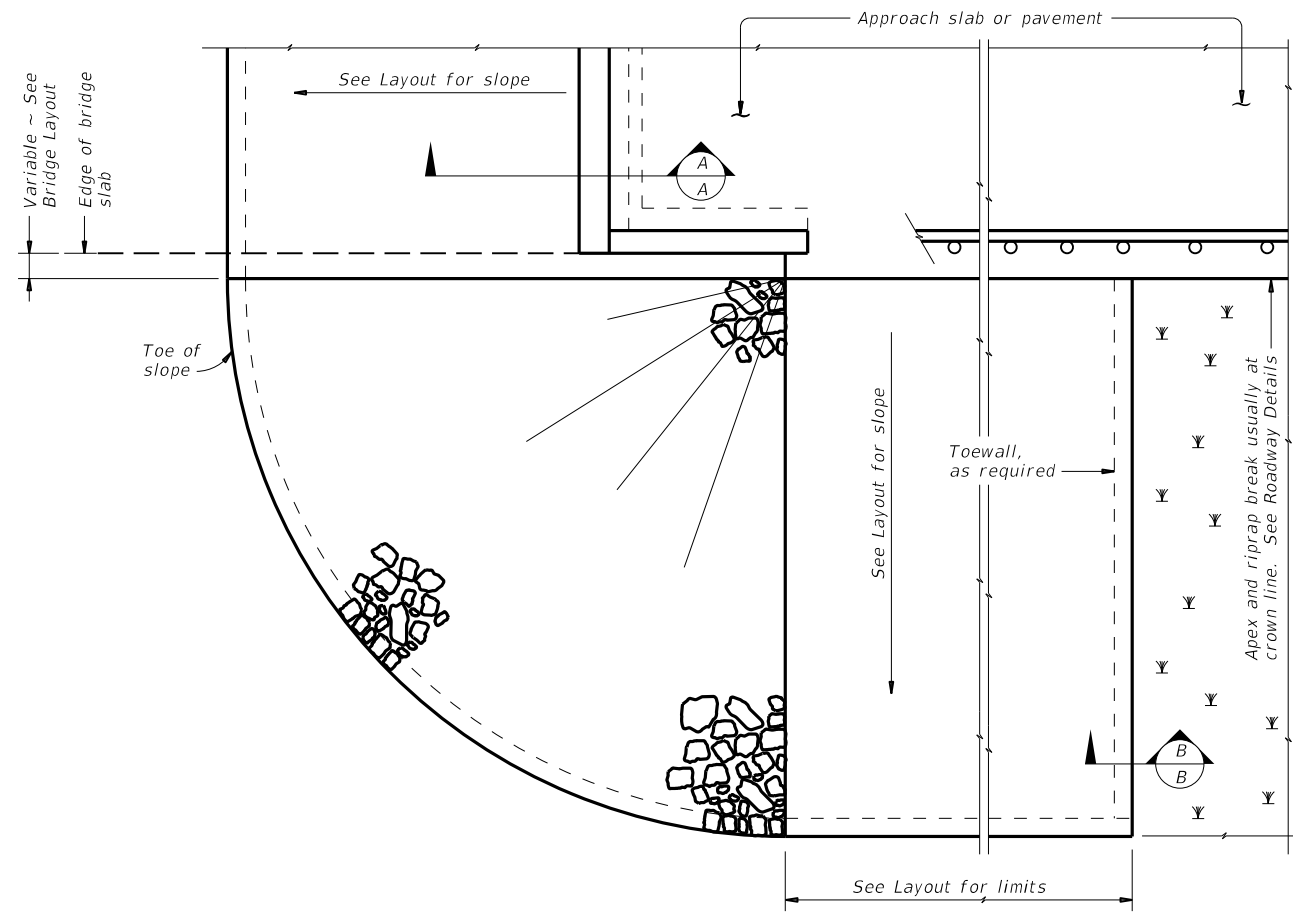
CONSTRUCTION NOTES:
 Secure the sealed expansion joint in position and place to the proper grade and alignment by welding braces to adjacent reinforcing steel, to prestressed beam stirrups, or to anchors cast in concrete diaphragms. Include cost of temporary bracing in the price bid for sealed expansion joint. Remove shipping angle immediately after each joint half is secured in place. Grind smooth, and touch up with organic zinc-rich paint. Clean and prepare seal cavity for seal installation as per the Manufacturer's installation procedures.

GENERAL NOTES:
 Provide sealed expansion joints in the size and at locations shown on the plans. Minimum slab and overhang thickness required for the use of SEJ-M is 6 1/2".

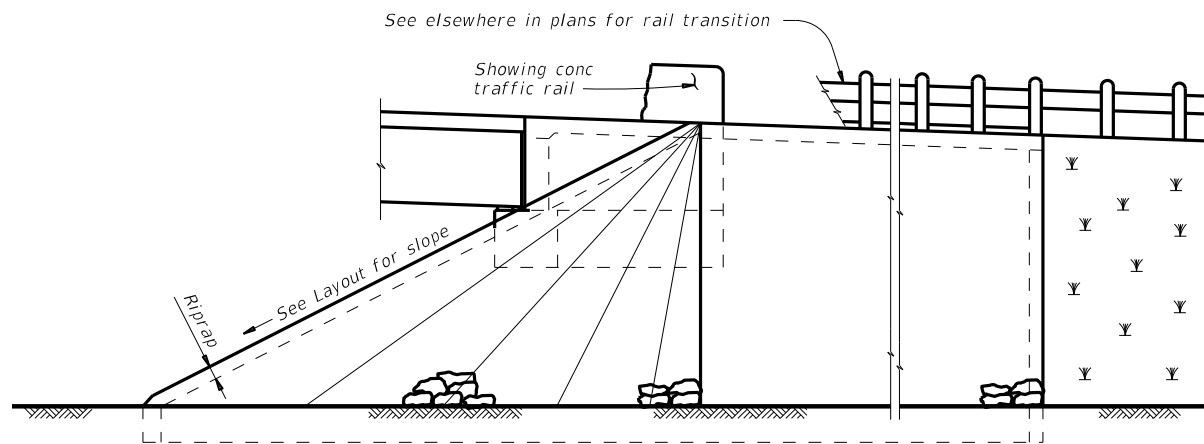
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|--|------------|---------------------------------|---------|
| | | Bridge Division Standard | |
| SEALED EXPANSION JOINT TYPE M WITHOUT OVERLAY | | | |
| SEJ-M | | | |
| FILE: sejmste1-19.dgn | DN: TxDOT | CK: TxDOT | DW: JTR |
| ©TxDOT April 2019 | CONTRACT | SECTION | JOB |
| REVISIONS | 0914 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 114 | |

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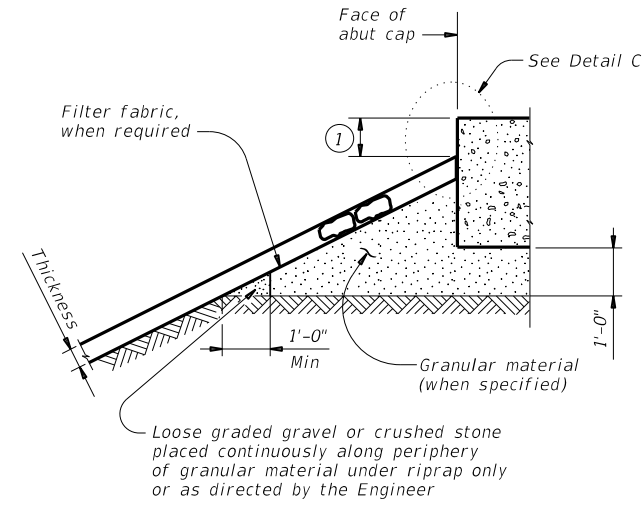
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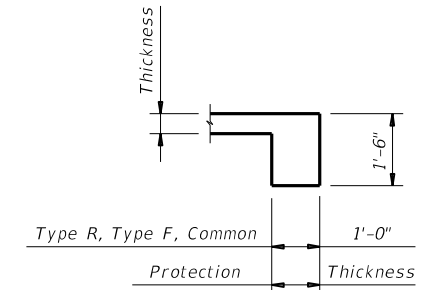
PLAN



ELEVATION

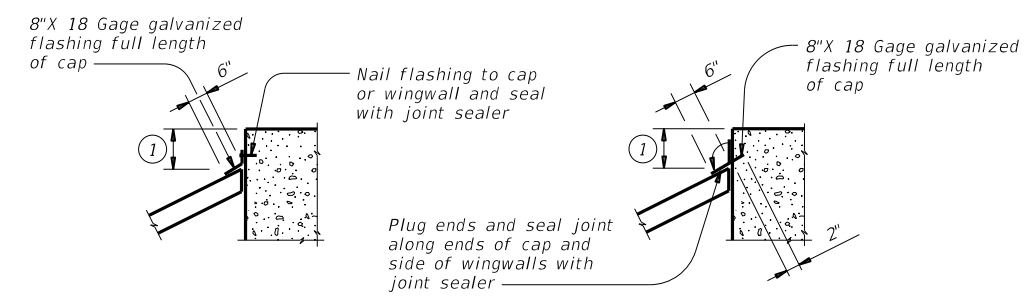


SECTION A-A AT CAP



SECTION B-B

Provide toewall when shoulder drain is located adjacent to limits of stone riprap. Omit toewall when thickness of protection riprap is greater than 18".



CAP OPTION A

CAP OPTION B

DETAIL C

① Top of cap to top of riprap dimension varies as directed by the Engineer. Provide 9" Min for beam/slab type bridges and 1'-6" for slab span, box beam, or slab beam bridges.

GENERAL NOTES:
 Refer to Item 432, "Riprap" for stone size and gradation, and construction details. See Layout for limits and thickness of riprap specified.
 See elsewhere in plans for locations and details of shoulder drains.

SHEET 1 OF 2

| | | | |
|-----------------------|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| <h2>STONE RIPRAP</h2> | | | |
| <h3>SRR</h3> | | | |
| FILE: srrstdel-19.dgn | DN: AES | CK: JGD | DW: BWH |
| ©TxDOT April 2019 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | | SHEET NO. |
| AUS | WILLIAMSON | | 115 |

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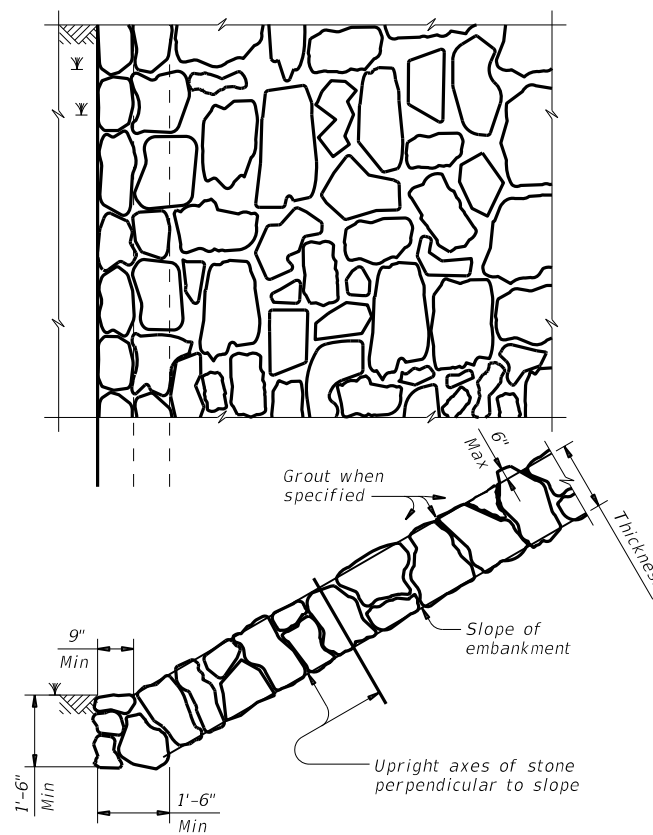


FIGURE 1 ~ TYPE R STONE RIPRAP
 dry or grouted

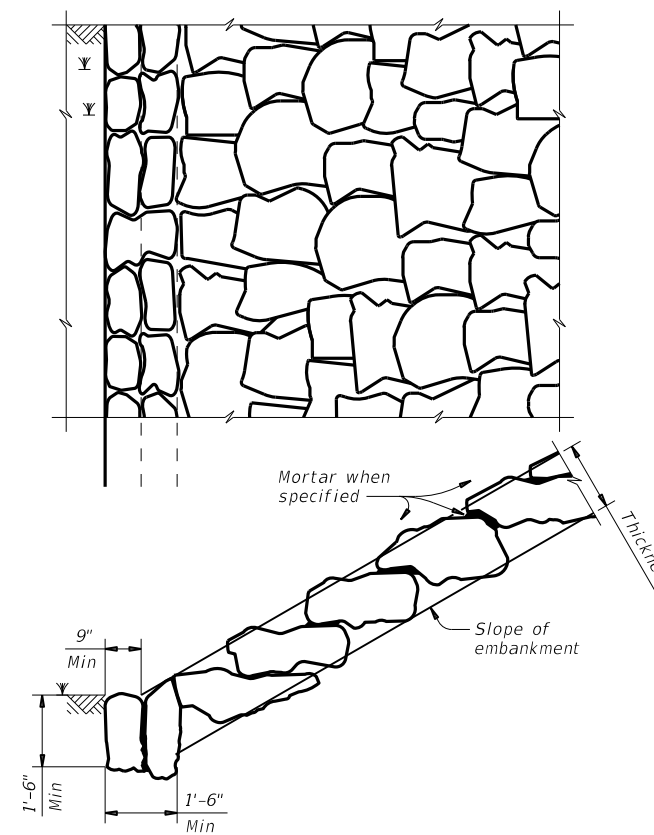


FIGURE 2 ~ TYPE F STONE RIPRAP
 dry or mortared

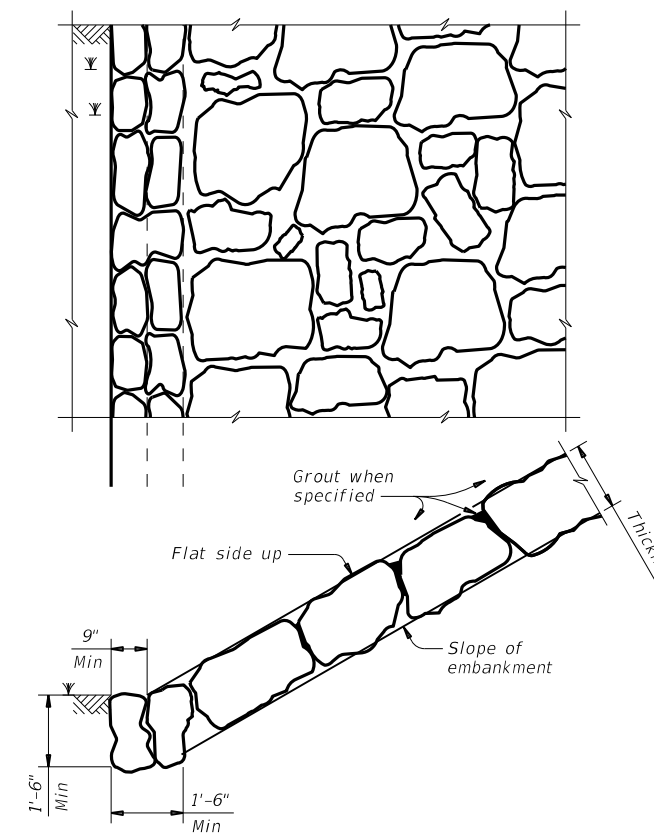


FIGURE 3 ~ TYPE F STONE RIPRAP
 grouted

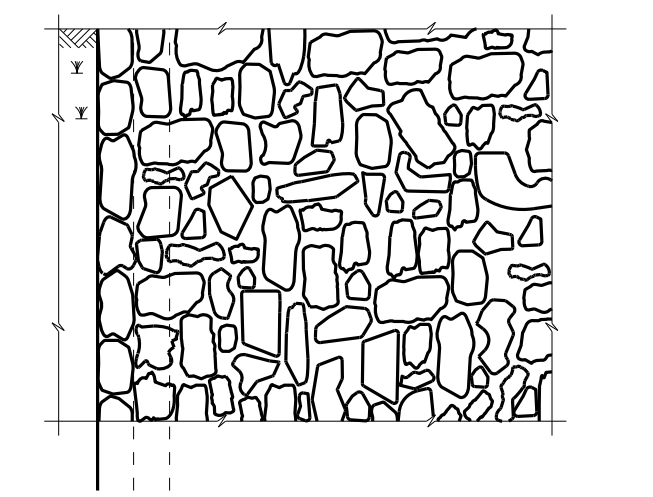


FIGURE 4 ~ COMMON STONE RIPRAP
 dry or grouted

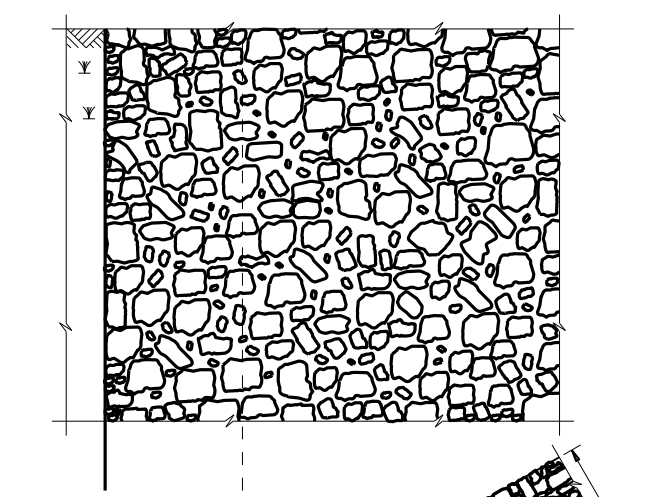
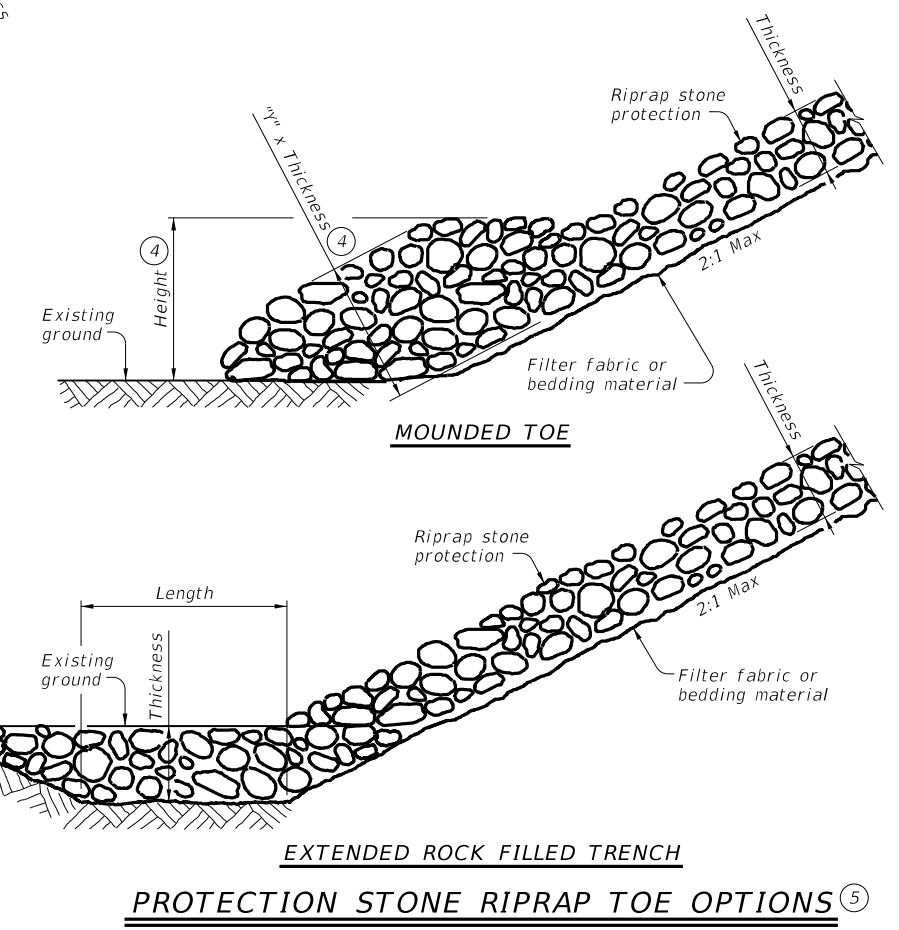


FIGURE 5 ~ PROTECTION STONE RIPRAP

- ② Provide bedding material instead of filter fabric if shown elsewhere in plans. See Layout for thickness of bedding material.
- ③ Minimum toe depth is the larger of the maximum scour depth or 2 times the riprap thickness.
- ④ "Y" and Height need to be defined. See layout or detail sheet for values if this option is used.
- ⑤ List Stone Protection as size (XX inch) and thickness (YY inch) on the layout.
 Example: Riprap (Stone Protection) XX inch, Thickness = YY inch.



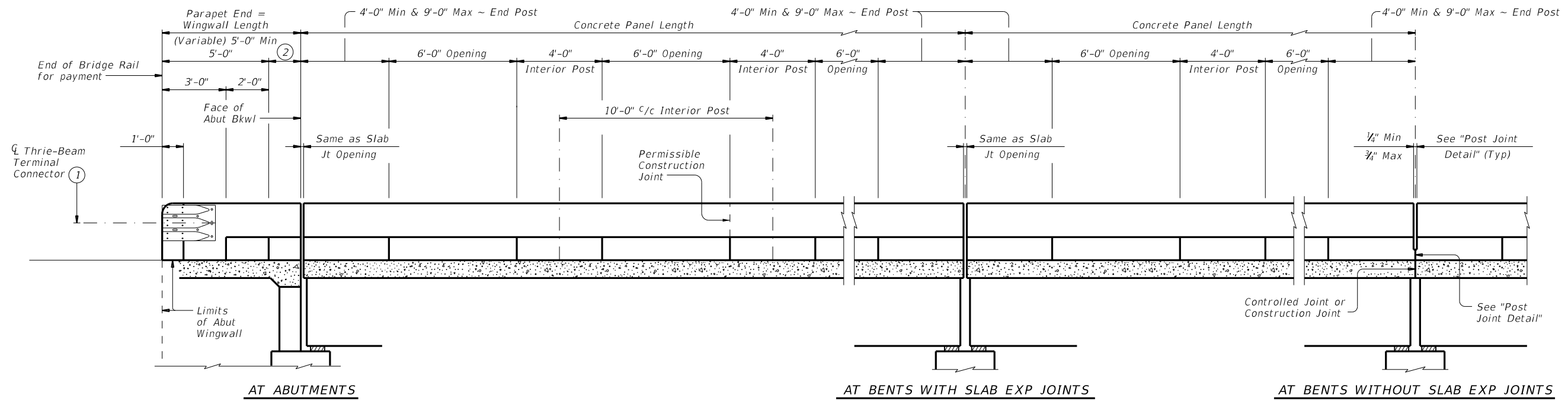
PROTECTION STONE RIPRAP TOE OPTIONS

SHEET 2 OF 2

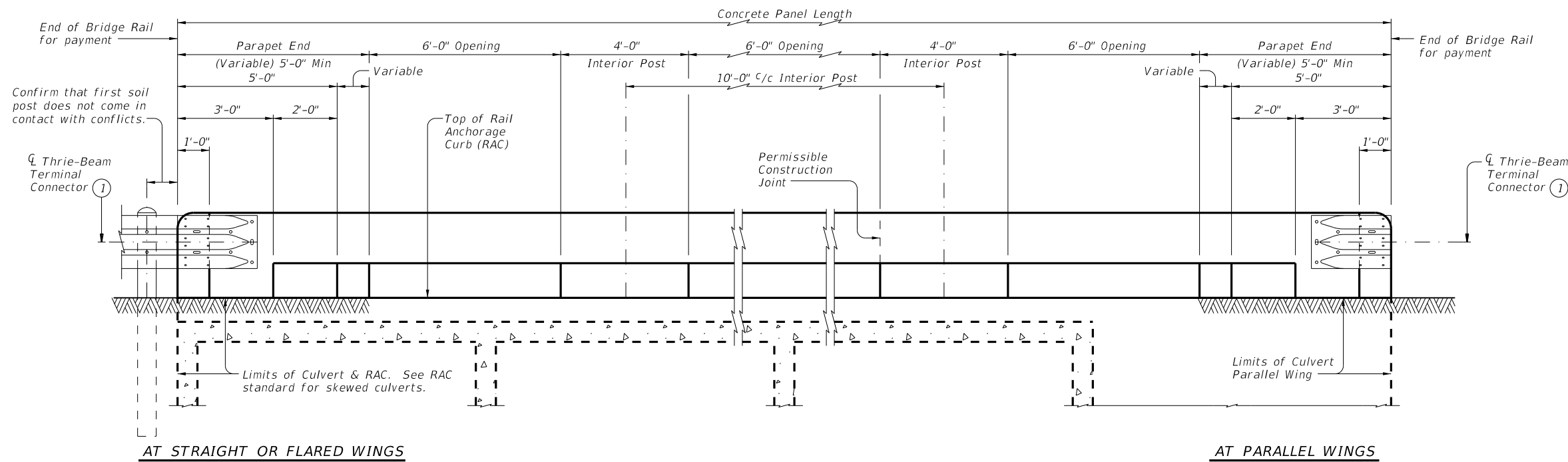
| | | | |
|-----------------------|------------|---------------------------------|---------|
| | | Bridge Division Standard | |
| <h2>STONE RIPRAP</h2> | | | |
| <h3>SRR</h3> | | | |
| FILE: srrstde1-19.dgn | DN: AES | CK: JGD | DW: BWH |
| ©TxDOT April 2019 | CONT SECT | JOB | HIGHWAY |
| REVISIONS | 0914 05 | 204, ETC. | CR 118 |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 116 | |

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ROADWAY ELEVATION OF RAIL ON BRIDGE



ROADWAY ELEVATION OF RAIL ON BOX CULVERTS

Showing 0° skew culvert. Skewed culverts similar. See RAC standard for details not shown. Vertical joints in concrete rail are not required, unless shown elsewhere.

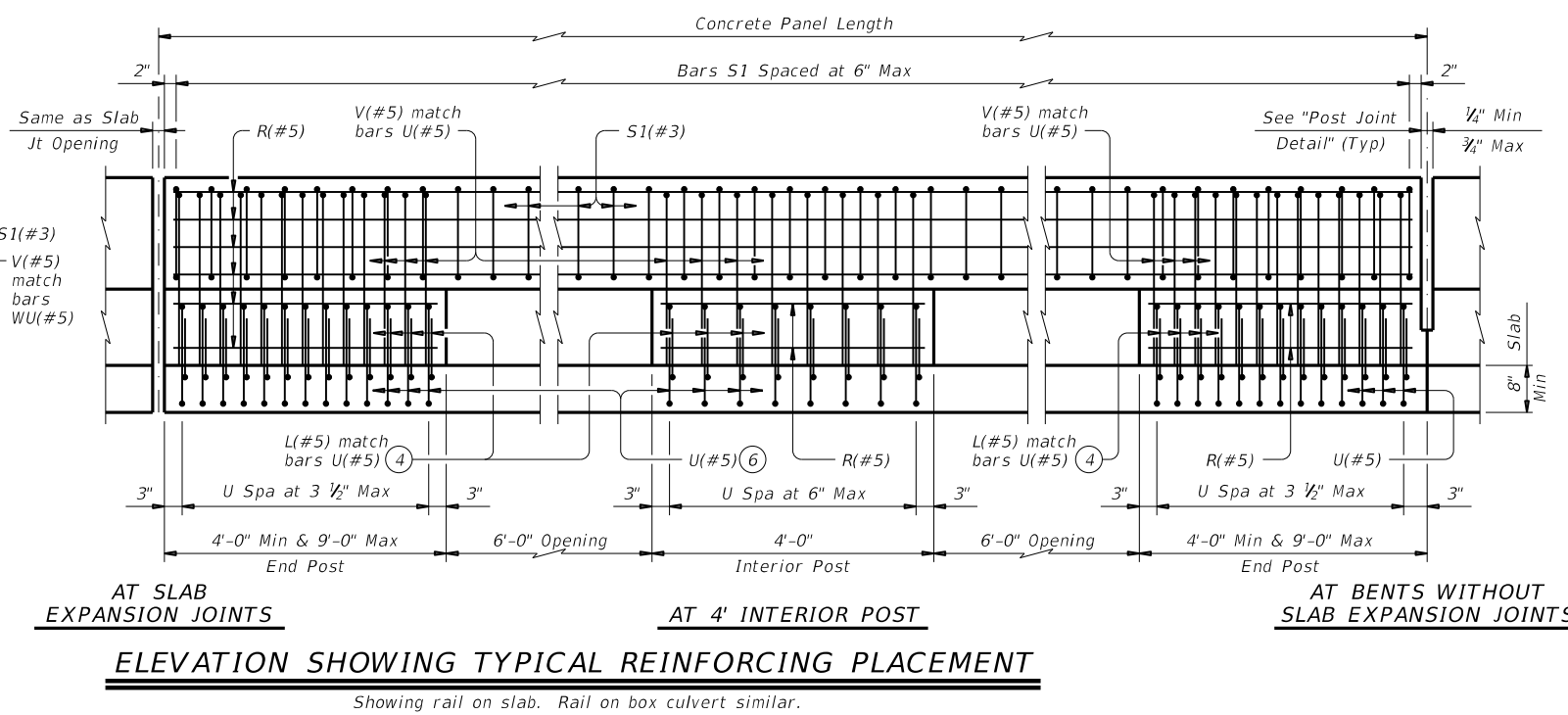
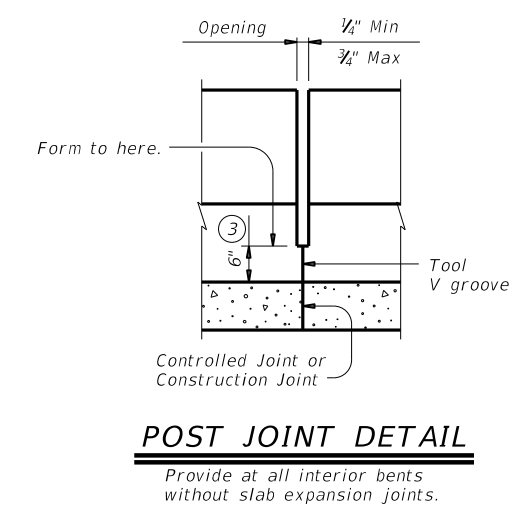
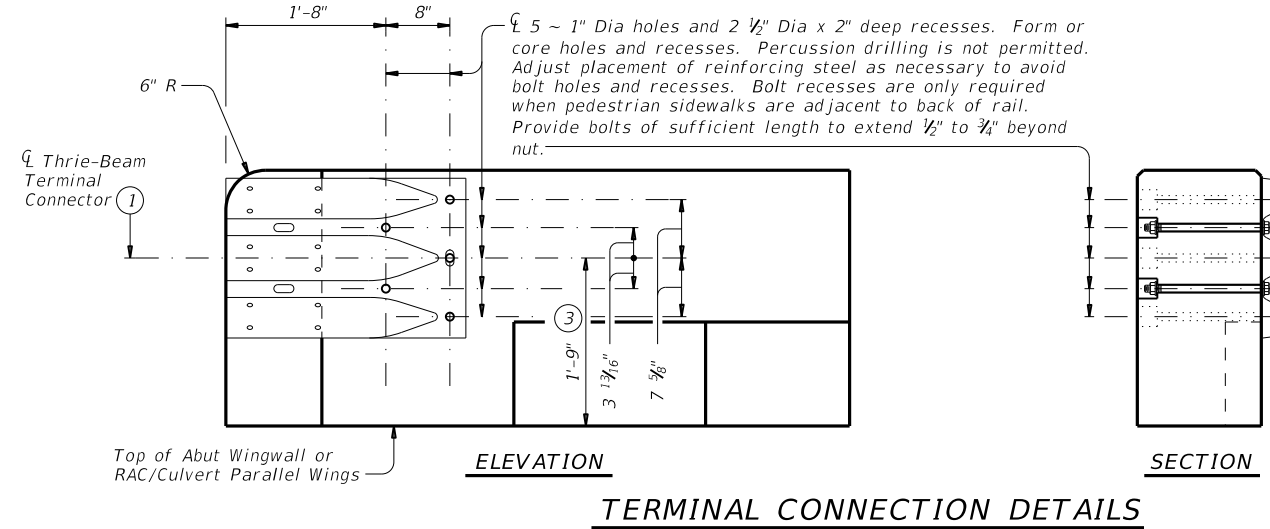
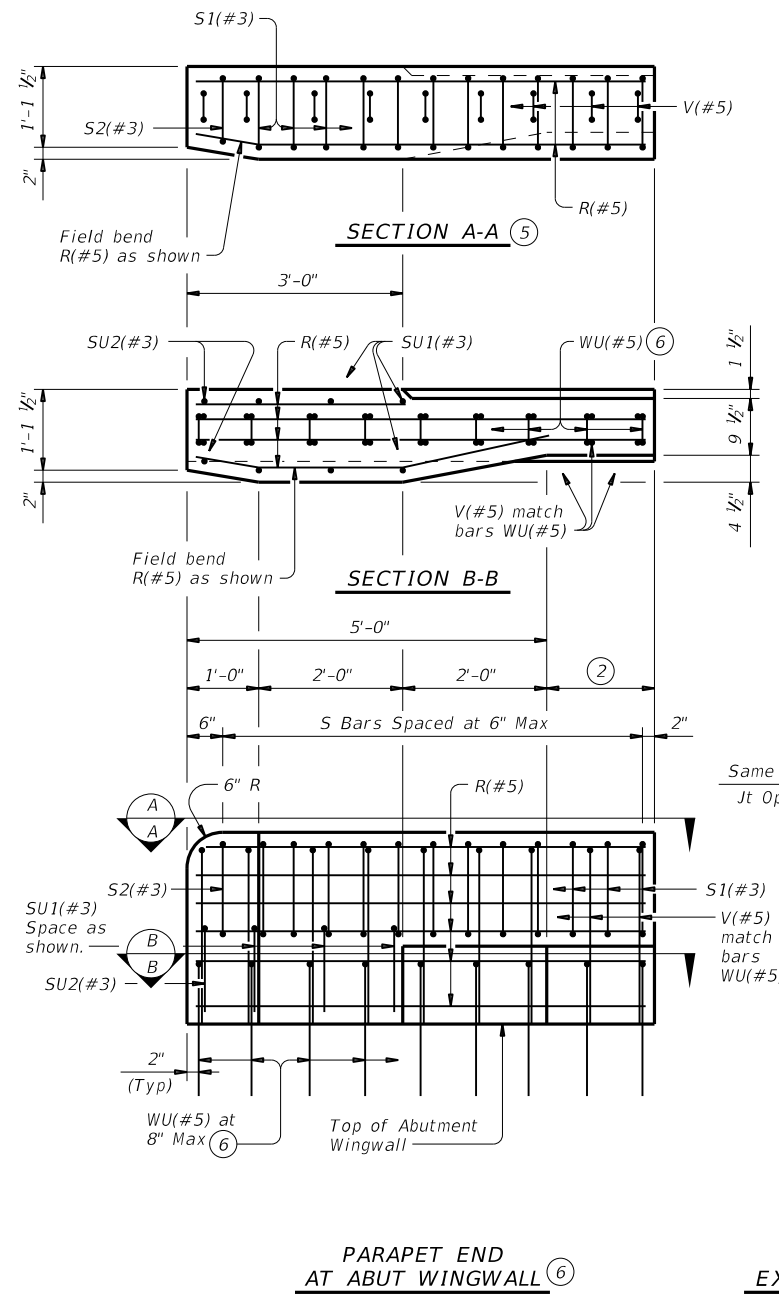
- ① Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.
- ② Wingwall Length minus 5'-0" (Varies)

SHEET 1 OF 3

| | | | | | |
|-----------------------|-----------|------------|-----------|--------------------------|--|
| | | | | Bridge Division Standard | |
| <h2>TRAFFIC RAIL</h2> | | | | | |
| <h3>TYPE T223</h3> | | | | | |
| FILE: r1std005-19.dgn | DN: TxDOT | CK: TxDOT | DW: JTR | CK: AES | |
| ©TxDOT September 2019 | CONT | SECT | JOB | HIGHWAY | |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | |
| | DIST | COUNTY | SHEET NO. | | |
| | AUS | WILLIAMSON | 117 | | |

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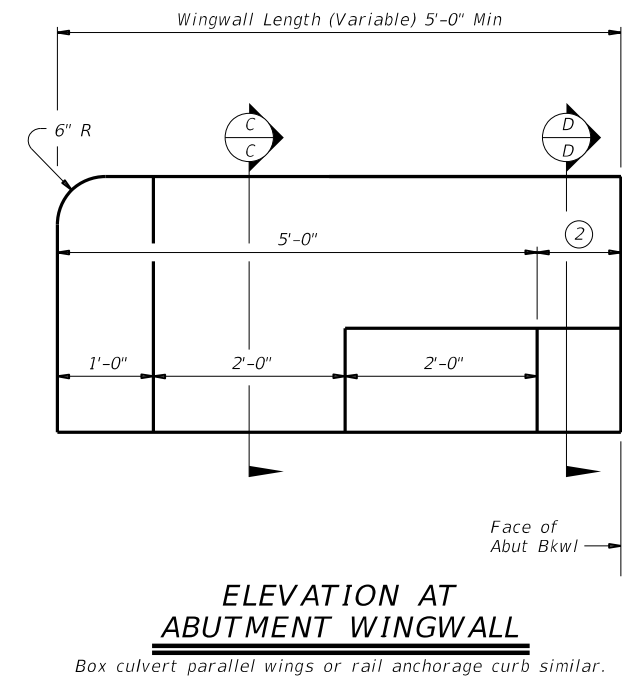
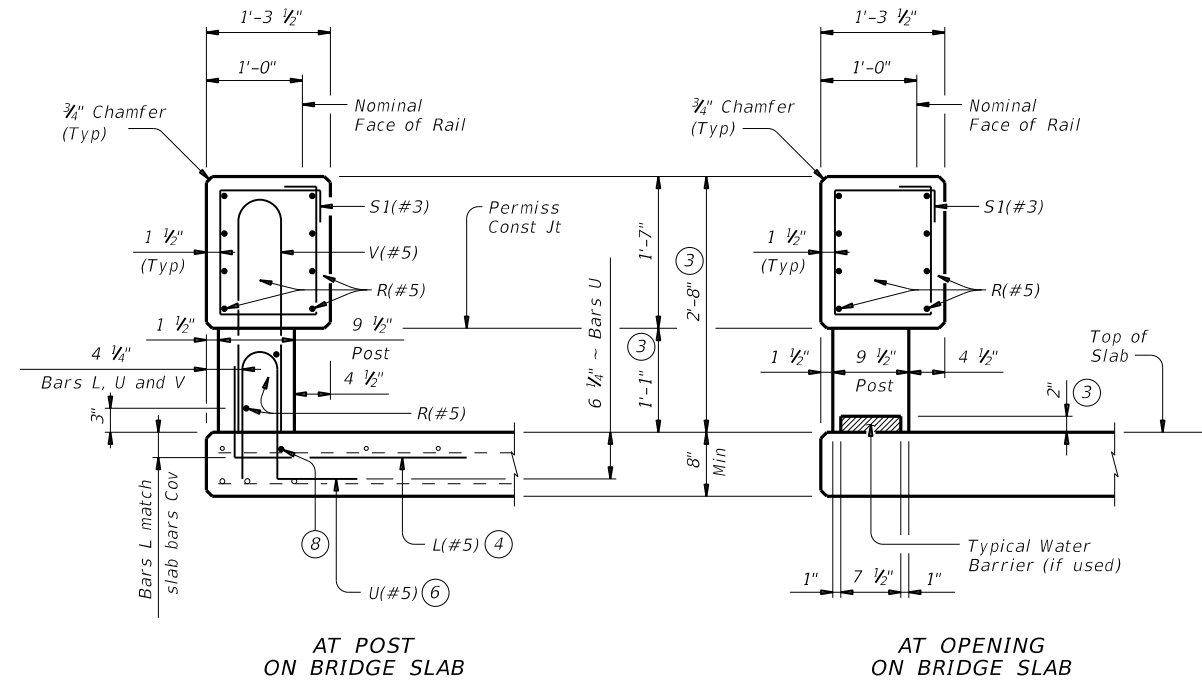
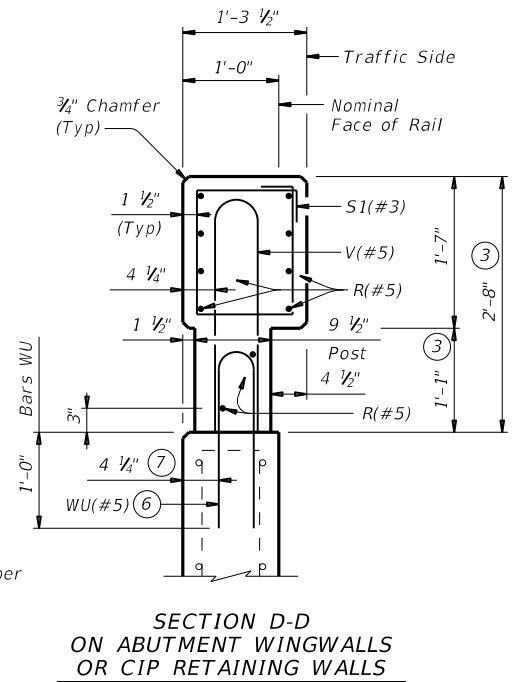
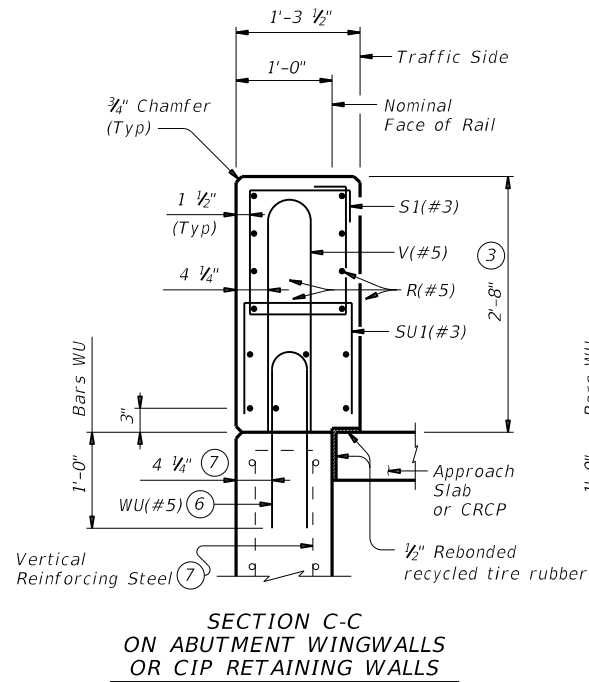
- ① Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.
- ② Wingwall Length minus 5'-0" (Varies)
- ③ Increase 2" for structures with overlay.
- ④ Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.
- ⑤ Bars SU1(#3), SU2(#3) and WU(#5) not shown for clarity.
- ⑥ Substitute Bars U(#5) for Bars WU(#5) when parapel end is located on anchorage curb over culvert top slab. Use Bars WU(#5) in culvert parallel wings.

SHEET 2 OF 3

| | | | |
|-----------------------|------------|--------------------------|-----------|
| | | Bridge Division Standard | |
| TRAFFIC RAIL | | | |
| TYPE T223 | | | |
| FILE: r1std005-19.dgn | DN: TxDOT | CK: TxDOT | DW: JTR |
| ©TxDOT September 2019 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 118 | |

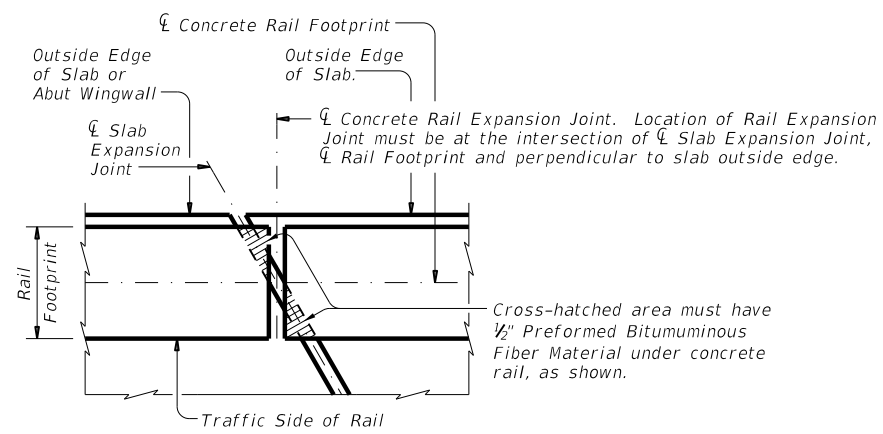
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SECTIONS THRU RAIL
 Sections on box culverts similar.

- ② Wingwall Length minus 5'-0" (Varies)
- ③ Increase 2" for structures with overlay.
- ④ Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.
- ⑥ Substitute Bars U(#5) for Bars WU(#5) when parapet end is located on anchorage curb over culvert top slab. Use Bars WU(#5) in culvert parallel wings.
- ⑦ When vertical reinforcing has closer clear cover over horizontal reinforcing in abutment wingwalls on traffic side of wall, move the horizontal wingwall/retaining wall reinforcing to the inside of Bars WU where bars conflict.
- ⑧ Top longitudinal slab bar may be adjusted laterally 3" plus or minus to tie reinforcing.
- ⑨ At the Contractor's option, Bars V may be replaced by extending Bars U to 2'-5 1/4" above the roadway surface without overlay.



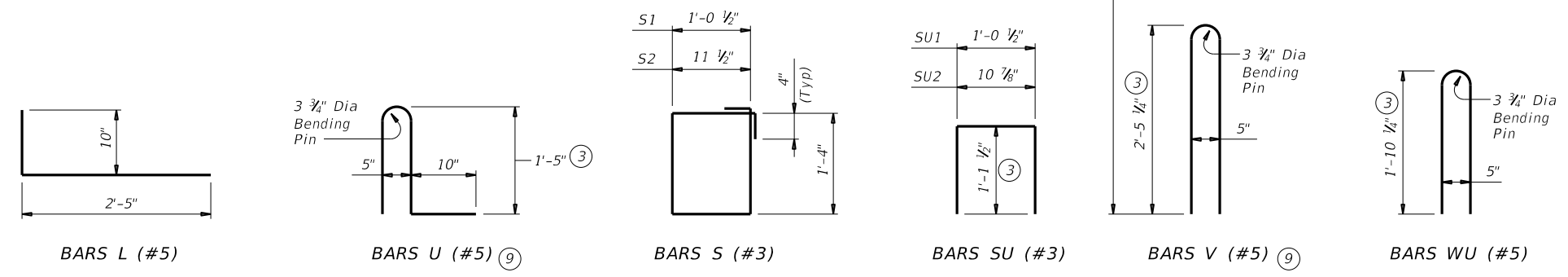
PLAN OF RAIL AT EXPANSION JOINTS
 Example showing Slab Expansion Joints without breakbacks.

CONSTRUCTION NOTES:
 Face of rail and parapet must be vertical transversely unless otherwise shown in the plans or approved by the Engineer.
 Provide water barriers at openings draining onto undercrossing roadways and sidewalks. They may be cast-in-place or precast in convenient lengths and bonded to the bridge deck with an approved epoxy cement.
 Chamfer all exposed corners.

MATERIAL NOTES:
 Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere.
 Provide Grade 60 reinforcing steel.
 Epoxy coat or galvanize all reinforcing steel if slab bars are epoxy coated or galvanized.
 Deformed Welded Wire Reinforcing (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars U, V, and WU unless noted otherwise. Provide the same laps as required for reinforcing bars.
 Provide bar laps, where required, as follows:
 Uncoated or galvanized ~ #5 = 2'-0"
 Epoxy coated ~ #5 = 3'-0"

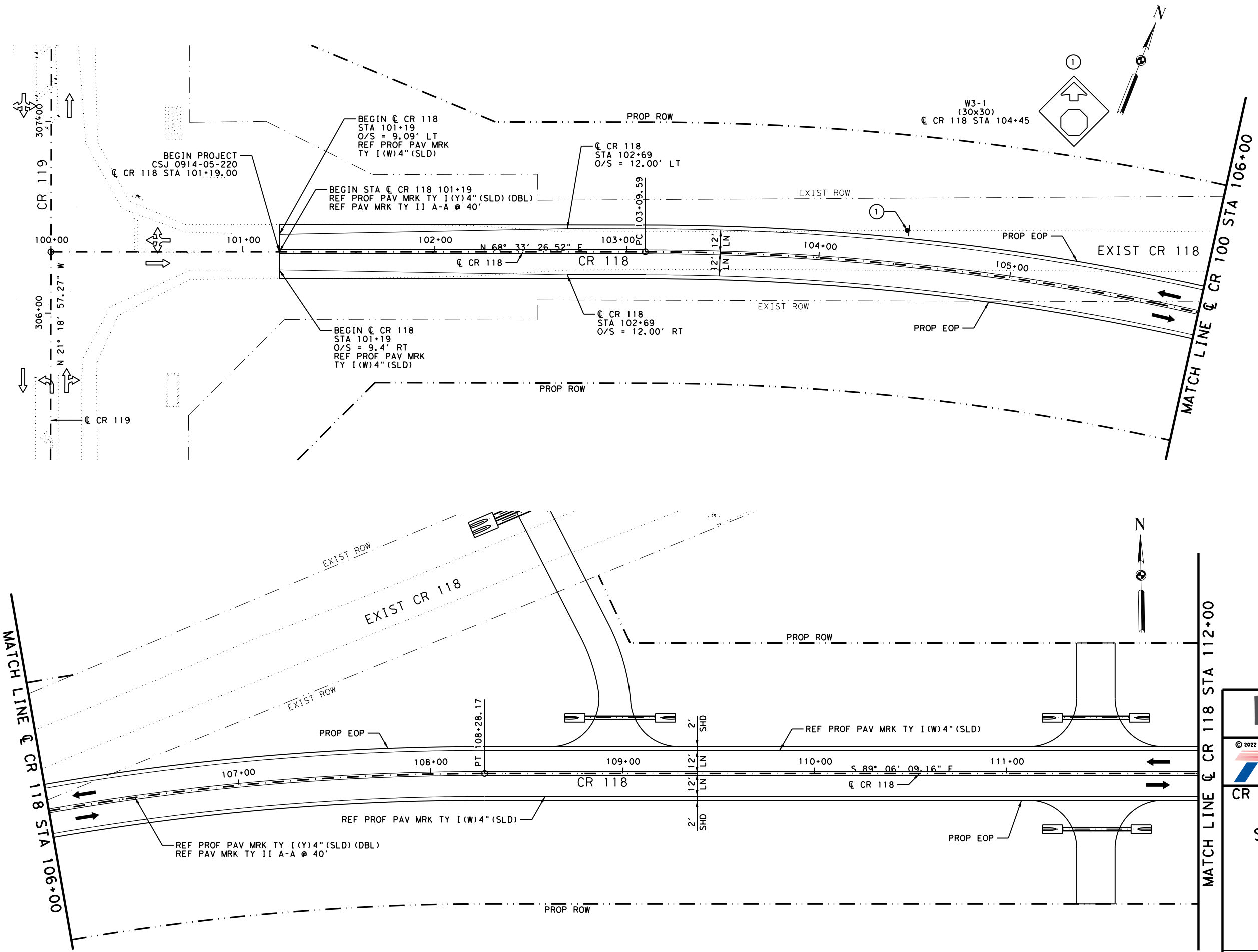
GENERAL NOTES:
 This rail has been evaluated by full-scale crash test to meet MASH TL-3 criteria. This rail can be used for speeds of 50 mph and greater when a TL-3 rated guard fence transition is used. When a TL-2 rated guard fence transition is used, this rail can be used for speeds of 45 mph and less.
 Do not use this railing on bridges with expansion joints providing more than 5" movement.
 Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details elsewhere in plans for these modifications.
 Shop drawings are not required for this rail.
 Average weight of railing with no overlay is 358 plf.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

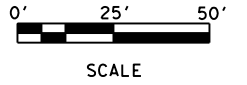


| | | | |
|--|------------|---------------------------------|-----------|
| | | Bridge Division Standard | |
| <h1>TRAFFIC RAIL</h1> <h2>TYPE T223</h2> | | | |
| FILE: r1std005-19.dgn | DN: TxDOT | CK: TxDOT | DW: JTR |
| ©TxDOT September 2019 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 119 | |

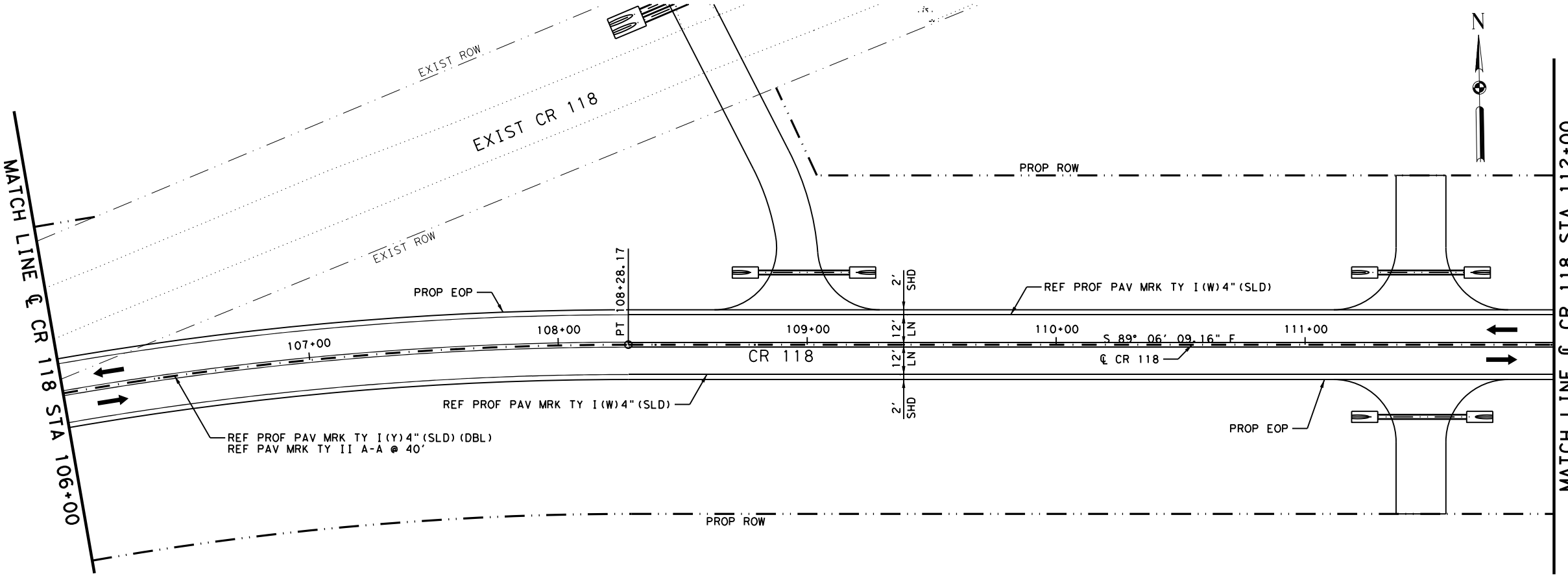
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- LEGEND**
- PROPOSED SMALL SIGN
 - EXIST SIGN TO BE REMOVED
 - DIRECTION OF TRAFFIC
 - TY 2 (OM-2) OBJECT MARKER
 - EXISTING SIGN
 - DEL ASSM (D-SW) SZ 1 (BRF) GF-2 (BI)
 - DEL ASSM (D-SW) SZ (BRF) CTB (BI)
 - PROPOSED SIGN
- NOTES:**
- REFER TO REMOVAL SHEETS FOR EXISTING SIGN AND OBJECT MARKER REMOVAL.



TJN
 11/3/2022



Kimley»Horn F-928

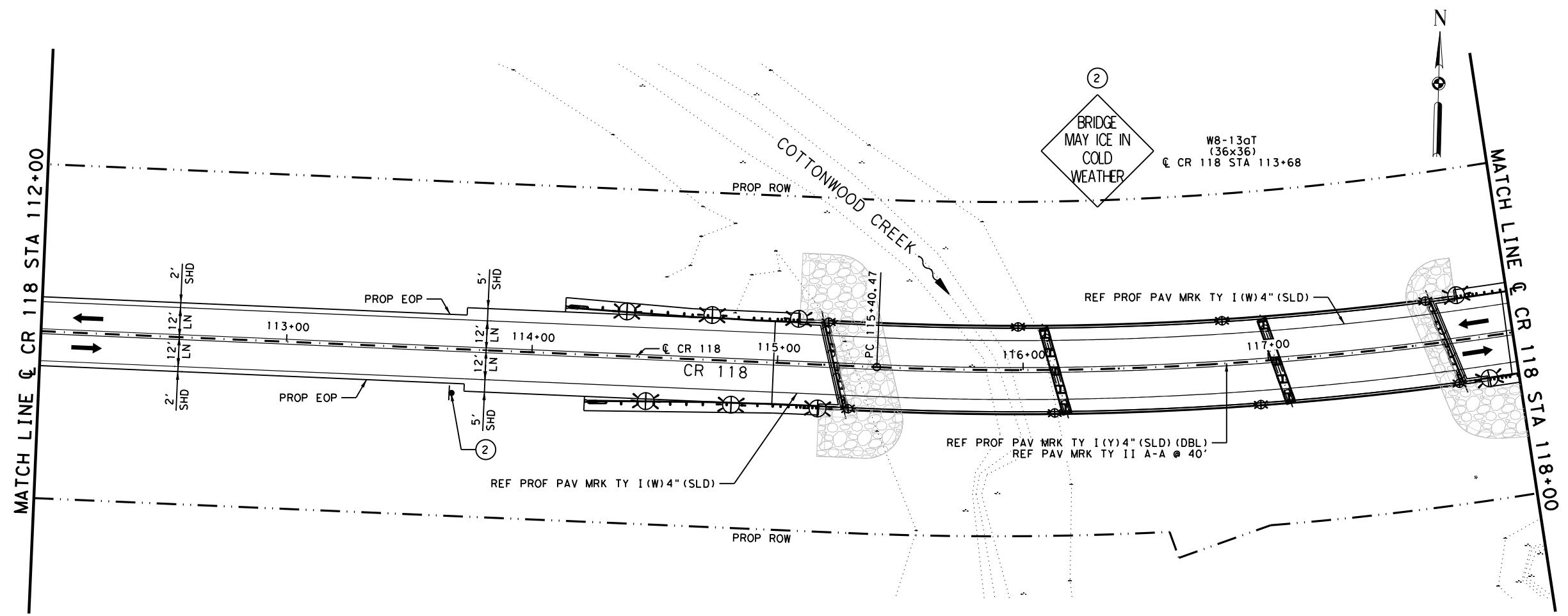
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CR 118 AT COTTONWOOD CREEK
SIGNING & PAVEMENT MARKING LAYOUT

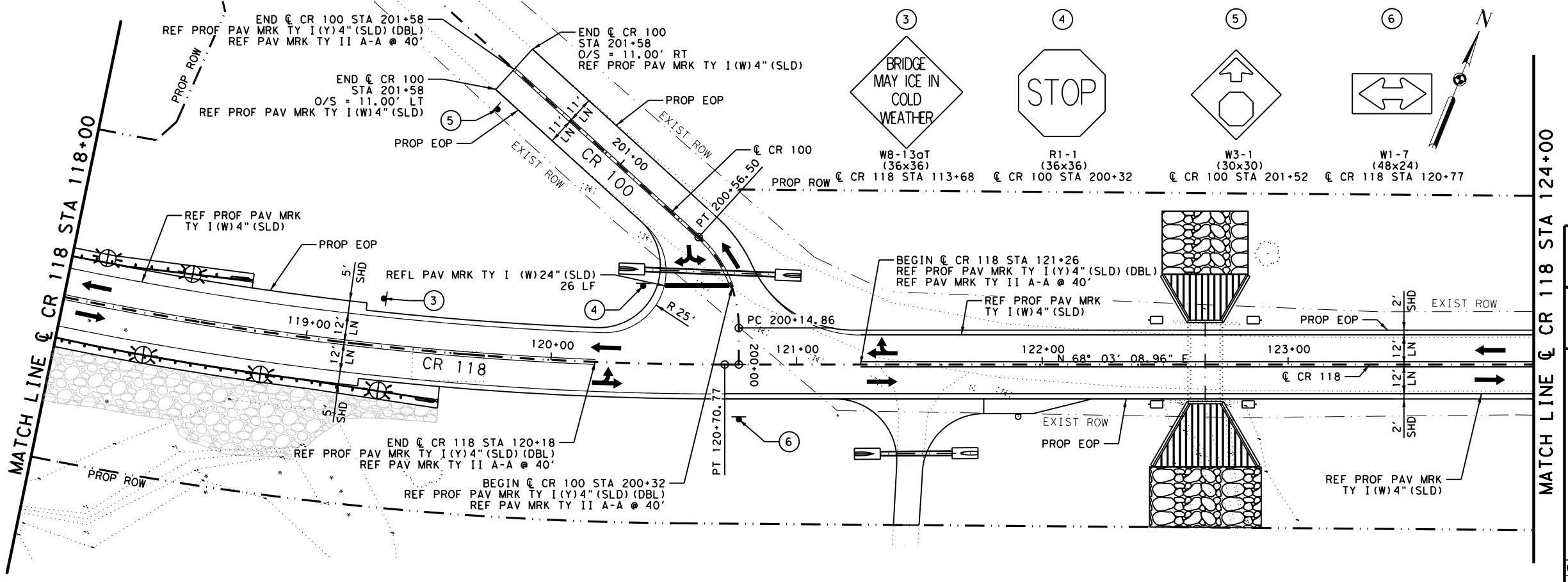
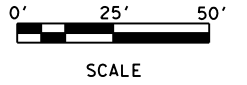
SHEET 1 OF 3

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 120 |

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- LEGEND**
- (*) PROPOSED SMALL SIGN
 - (R#) EXIST SIGN TO BE REMOVED
 - DIRECTION OF TRAFFIC
 - TY 2 (OM-2) OBJECT MARKER
 - ⋯ EXISTING SIGN
 - ⊗ DEL ASSM (D-SW) SZ 1 (BRF) GF-2 (BI)
 - ⊗ DEL ASSM (D-SW) SZ (BRF) CTB (BI)
 - ▲ PROPOSED SIGN
- NOTES:**
- REFER TO REMOVAL SHEETS FOR EXISTING SIGN AND OBJECT MARKER REMOVAL.



TJN
 11/3/2022

Kimley»Horn F-928

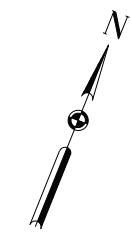
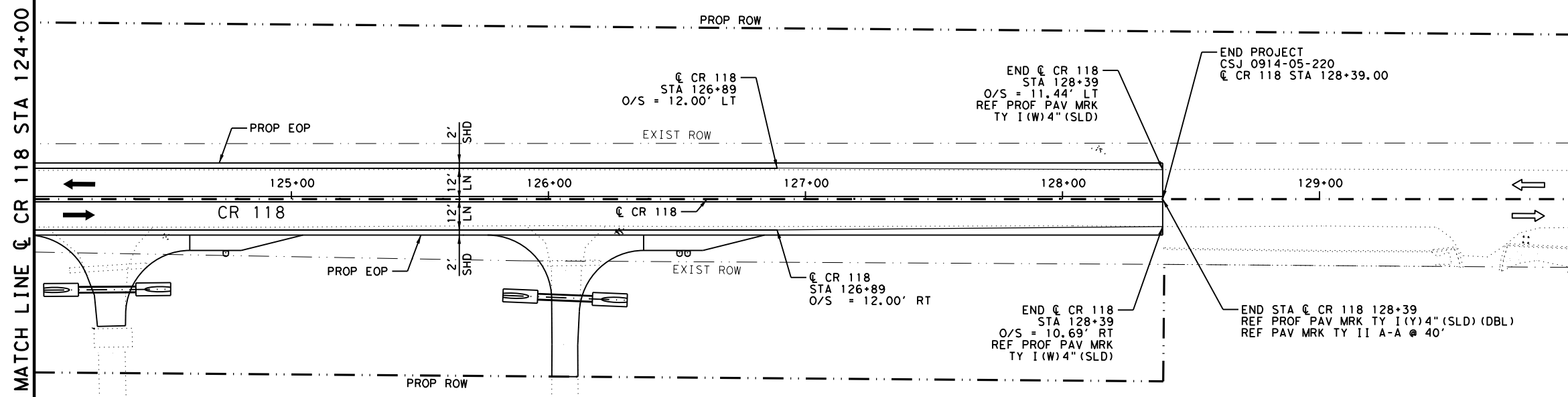
Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
SIGNING & PAVEMENT MARKING LAYOUT

SHEET 2 OF 3

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 121 |

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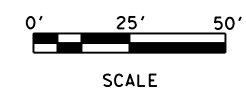


LEGEND

- PROPOSED SMALL SIGN
- EXIST SIGN TO BE REMOVED
- DIRECTION OF TRAFFIC
- TY 2 (OM-2) OBJECT MARKER
- EXISTING SIGN
- DEL ASSM (D-SW) SZ 1 (BRF) GF-2 (BI)
- DEL ASSM (D-SW) SZ (BRF) CTB (BI)
- PROPOSED SIGN

NOTES:

1. REFER TO REMOVAL SHEETS FOR EXISTING SIGN AND OBJECT MARKER REMOVAL.



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 11/3/2022

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CR 118 AT COTTONWOOD CREEK

SIGNING & PAVEMENT MARKING LAYOUT

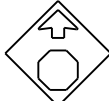



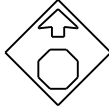

SHEET 3 OF 3

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 122 |

SUMMARY OF SMALL SIGNS

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DATE:
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| PLAN SHEET NO. | SIGN NO. | SIGN NOMENCLATURE | SIGN | DIMENSIONS | FLAT ALUMINUM (TYPE A) | EXAL ALUMINUM (TYPE G) | SM RD SGN ASSM TY XXXXX (X) XX (X-XXXX) | | | | BRIDGE MOUNT CLEARANCE SIGNS (See Note 2) | |
|----------------|----------|-------------------|---|------------|------------------------|------------------------|---|-------|-------------|----------------------|--|---|
| | | | | | | | POST TYPE | POSTS | ANCHOR TYPE | MOUNTING DESIGNATION | | |
| | | | | | | | | | | PREFABRICATED | | 1EXT or 2EXT = # of Ext BM = Extruded Wind Beam WC = 1.12 #/ft Wing Channel EXAL= Extruded Alum Sign Panels |
| 1 | 1 | W3-1 |  | 30 x 30 | X | | 10BWG | 1 | SA | P | | |
| 2 | 2 | W8-13aT |  | 36 x 36 | X | | 10BWG | 1 | SA | P | | |
| 2 | 3 | W8-13aT |  | 36 x 36 | X | | 10BWG | 1 | SA | P | | |
| 2 | 4 | R1-1 |  | 36 x 36 | X | | 10BWG | 1 | SA | P | | |
| 2 | 5 | W3-1 |  | 30 x 30 | X | | 10BWG | 1 | SA | P | | |
| 2 | 6 | W1-7 |  | 48 x 24 | X | | 10BWG | 1 | SA | T | | |
| | | | | | | | | | | | | |
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| | | | | | | | | | | | | |

| ALUMINUM SIGN BLANKS THICKNESS | |
|--------------------------------|-------------------|
| Square Feet | Minimum Thickness |
| Less than 7.5 | 0.080" |
| 7.5 to 15 | 0.100" |
| Greater than 15 | 0.125" |

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website:
<http://www.txdot.gov/>

- NOTE:**
- Sign supports shall be located as shown on the plans, except that the Engineer may shift the sign supports, within design guidelines, where necessary to secure a more desirable location or to avoid conflict with utilities. Unless otherwise shown on the plans, the Contractor shall stake and the Engineer will verify all sign support locations.
 - For installation of bridge mount clearance signs, see Bridge Mounted Clearance Sign Assembly (BMCS) Standard Sheet.
 - For Sign Support Descriptive Codes, see Sign Mounting Details Small Roadside Signs General Notes & Details SMD(GEN).


Traffic Operations Division Standard

SUMMARY OF SMALL SIGNS

SOSS

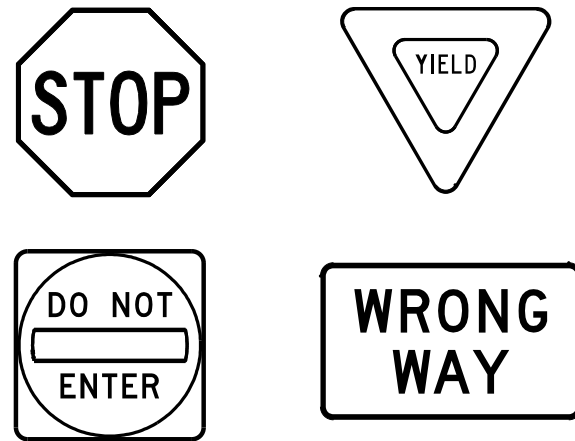
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| © TxDOT May 1987 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204,220 | CR 118 |
| 4-16 | DIST | COUNTY | SHEET NO. | |
| 8-16 | AUSTIN | WILLIAMSON | 123 | |

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REQUIREMENTS FOR RED BACKGROUND REGULATORY SIGNS

(STOP, YIELD, DO NOT ENTER AND WRONG WAY SIGNS)



REQUIREMENTS FOR FOUR SPECIFIC SIGNS ONLY

| SHEETING REQUIREMENTS | | |
|-----------------------|-------|----------------------|
| USAGE | COLOR | SIGN FACE MATERIAL |
| BACKGROUND | RED | TYPE B OR C SHEETING |
| BACKGROUND | WHITE | TYPE B OR C SHEETING |
| LEGEND & BORDERS | WHITE | TYPE B OR C SHEETING |
| LEGEND | RED | TYPE B OR C SHEETING |

REQUIREMENTS FOR WHITE BACKGROUND REGULATORY SIGNS

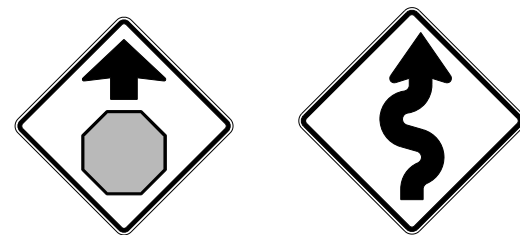
(EXCLUDING STOP, YIELD, DO NOT ENTER AND WRONG WAY SIGNS)



TYPICAL EXAMPLES

| SHEETING REQUIREMENTS | | |
|-----------------------------|------------|-----------------------------|
| USAGE | COLOR | SIGN FACE MATERIAL |
| BACKGROUND | WHITE | TYPE A SHEETING |
| BACKGROUND | ALL OTHERS | TYPE B OR C SHEETING |
| LEGEND, BORDERS AND SYMBOLS | BLACK | ACRYLIC NON-REFLECTIVE FILM |
| LEGEND, BORDERS AND SYMBOLS | ALL OTHER | TYPE B OR C SHEETING |

REQUIREMENTS FOR WARNING SIGNS



TYPICAL EXAMPLES

| SHEETING REQUIREMENTS | | |
|-----------------------|--------------------|--|
| USAGE | COLOR | SIGN FACE MATERIAL |
| BACKGROUND | FLOURESCENT YELLOW | TYPE B _{FL} OR C _{FL} SHEETING |
| LEGEND & BORDERS | BLACK | ACRYLIC NON-REFLECTIVE FILM |
| LEGEND & SYMBOLS | ALL OTHER | TYPE B OR C SHEETING |

REQUIREMENTS FOR SCHOOL SIGNS



TYPICAL EXAMPLES

| SHEETING REQUIREMENTS | | |
|-----------------------------|--------------------------|--|
| USAGE | COLOR | SIGN FACE MATERIAL |
| BACKGROUND | WHITE | TYPE A SHEETING |
| BACKGROUND | FLOURESCENT YELLOW GREEN | TYPE B _{FL} OR C _{FL} SHEETING |
| LEGEND, BORDERS AND SYMBOLS | BLACK | ACRYLIC NON-REFLECTIVE FILM |
| SYMBOLS | RED | TYPE B OR C SHEETING |

GENERAL NOTES

- Signs to be furnished shall be as detailed elsewhere in the plans and/or as shown on sign tabulation sheet. Standard sign designs and arrow dimensions can be found in the "Standard Highway Sign Designs for Texas" (SHSD).
- Sign legend shall use the Federal Highway Administration (FHWA) Standard Highway Alphabets (B, C, D, E, Emod or F).
- Lateral spacing between letters and numerals shall conform with the SHSD, and any approved changes thereto. Lateral spacing of legend shall provide a balanced appearance when spacing is not shown.
- Black legend and borders shall be applied by screening process or cut-out acrylic non-reflective black film to background sheeting, or combination thereof.
- White legend and borders shall be applied by screening process with transparent colored ink, transparent colored overlay film to white background sheeting or cut-out white sheeting to colored background sheeting, or combination thereof.
- Colored legend shall be applied by screening process with transparent colored ink, transparent colored overlay film or colored sheeting to background sheeting, or combination thereof.
- Sign substrate shall be any material that meets the Departmental Material Specification requirements of DMS-7110 or approved alternative.
- Mounting details for roadside mounted signs are shown in the "SMD series" Standard Plan Sheets.

ALUMINUM SIGN BLANKS THICKNESS

| Square Feet | Minimum Thickness |
|-----------------|-------------------|
| Less than 7.5 | 0.080 |
| 7.5 to 15 | 0.100 |
| Greater than 15 | 0.125 |

DEPARTMENTAL MATERIAL SPECIFICATIONS

| | |
|----------------------|----------|
| ALUMINUM SIGN BLANKS | DMS-7110 |
| SIGN FACE MATERIALS | DMS-8300 |

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

<http://www.txdot.gov/>



TYPICAL SIGN REQUIREMENTS

TSR(4) - 13

| | | | | | | | | | |
|-----------|--------------|------|------------|-----------|---------|-----|-------|-----|-------|
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| © TxDOT | October 2003 | CONT | SECT | JOB | HIGHWAY | | | | |
| REVISIONS | | 0914 | 05 | 204, ETC. | CR 118 | | | | |
| 12-03 | 7-13 | DIST | COUNTY | SHEET NO. | | | | | |
| 9-08 | | AUS | WILLIAMSON | 124 | | | | | |

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| REFLECTOR UNIT SIZES FOR DELINEATORS AND OBJECT MARKERS | | | | DELINEATORS | | | | D & OM DESCRIPTIVE CODES | |
|--|--------|--------|--------|--|--------|--|--------|--------------------------|--|
| DEVICE | SIZE 1 | SIZE 2 | SIZE 3 | SIZE 4 | SINGLE | | DOUBLE | | INSTL DEL ASSM (D-XX)SZ X (XXXX)XXX(XX) NUMBER OF REFLECTORS S = Single D = Double COLOR OF REFLECTORS W = White Y = Yellow R = Red REFLECTOR UNIT SIZE 1 or 2 TYPE OF POST OR DELINEATOR WC = Wing Channel Post YFLX = Yellow Flexible Post WFLX = White Flexible Post BRFL = Barrier Reflector TYPE OF MOUNT GND = Embedded (drivable or set in concrete) CTB = Concrete Barrier Mount GF1 or GF2 = Guard Fence Attachment SRF = Surface Mount DIRECTION If Required BI = Bi-Directional BR = Bi-Directional with red on back |
| | | | | | | | | | |
| SHEETING: Yellow, White or Red Type B or C reflective sheeting | | | | SHEETING: Yellow, White or Red Type B or C Reflective Sheeting | | | | | |
| POST TYPE: WC, YFLX, WFLX | | | | MOUNT TYPE: GND, SRF | | | | | |

| OBJECT MARKERS | | | | | | | | D & OM DESCRIPTIVE CODES | |
|---------------------------------------|---------------|---|-------|-------|---|-------|-------|---|---|
| DEVICE | Type 1 (OM-1) | Type 2 (OM-2) | | | Type 3 (OM-3) | | | Type 4 (OM-4) | INSTL OM ASSM (OM-XX) (XXXX)XXX(XX) TYPE OF OBJECT MARKER 1, 2, 3, or 4 NUMBER OF REFLECTORS OR DIRECTION X = 3-Size 2 reflector unit (Type 2 only) Y = 1-Size 3 reflector unit (Type 2 only) Z = 3-Size 1 or 1-Size 4 reflector unit(s) (Type 2 only) L = Left Side (Type 3 Object Marker only) R = Right Side (Type 3 Object Marker only) C = Center (Type 3 Object Marker only) TYPE OF POST WC = Wing Channel Post WFLX = White Flexible Post TWT = Thin Walled Tubing TYPE OF MOUNT GND = Embedded (drivable) SRF = Surface Mount WAS = Wedge Anchor Steel WAP = Wedge Anchor Plastic DIRECTION If Required BI = Bi-Directional |
| | | OM-1 | OM-2X | OM-2Y | OM-2Z | OM-3L | OM-3R | OM-3C | |
| SHEETING: Yellow-Type B or C Sheeting | | SHEETING: Yellow - Type B or C Sheeting | | | SHEETING: Alternating acrylic black and retroreflective yellow - Type B _{FL} or C _{FL} Sheeting | | | SHEETING: Red -Type B _{FL} or C _{FL} Sheeting | |

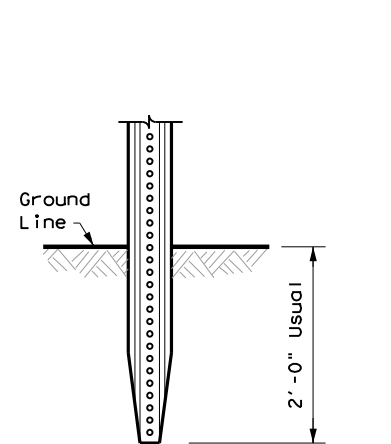
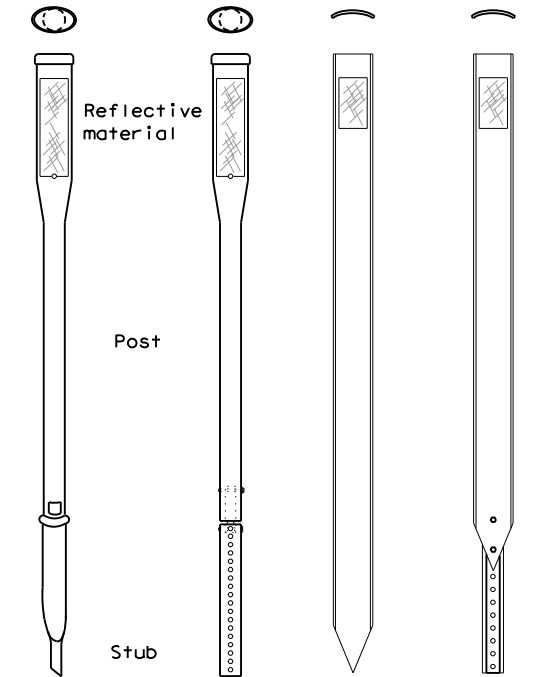
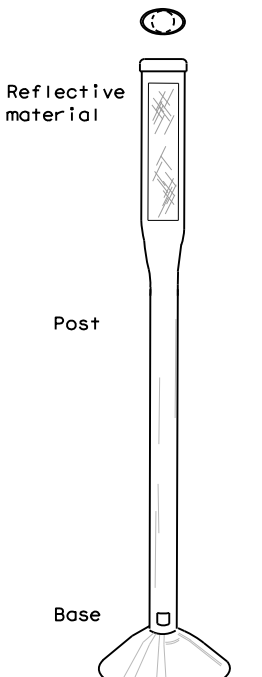
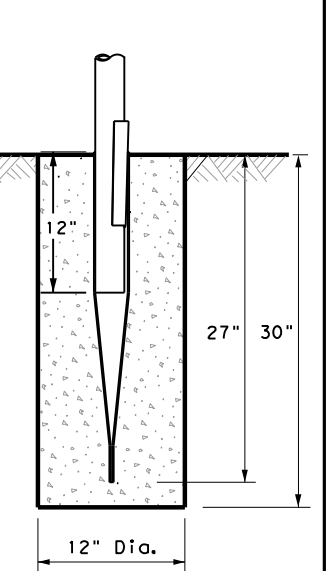
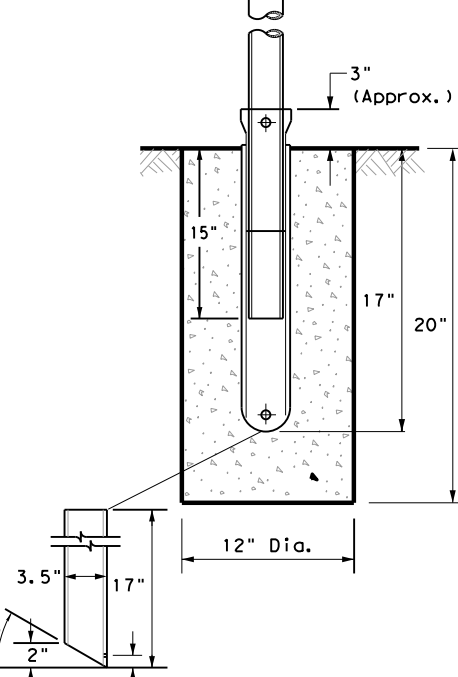
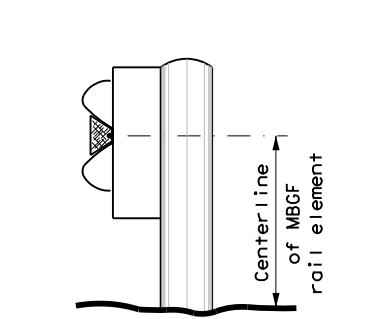
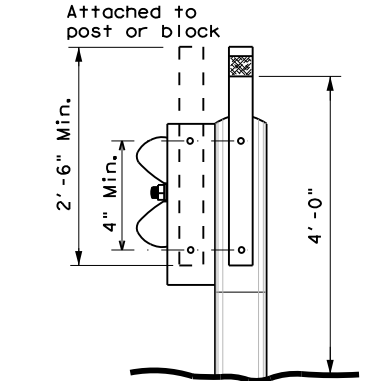
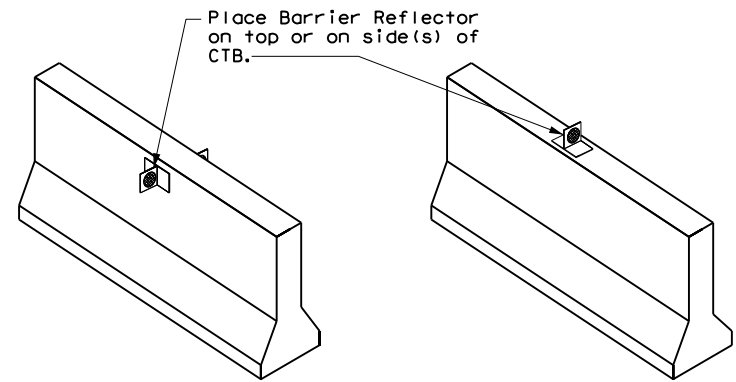
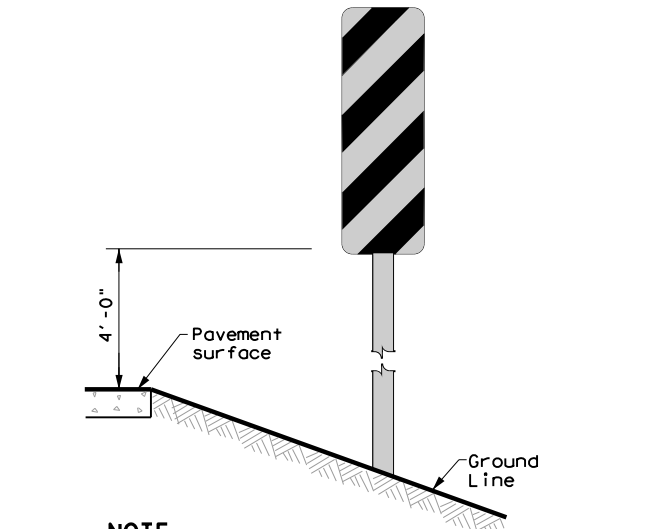
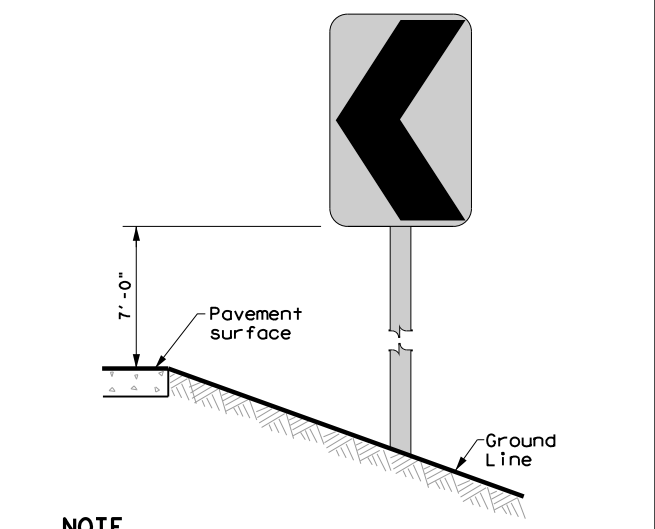
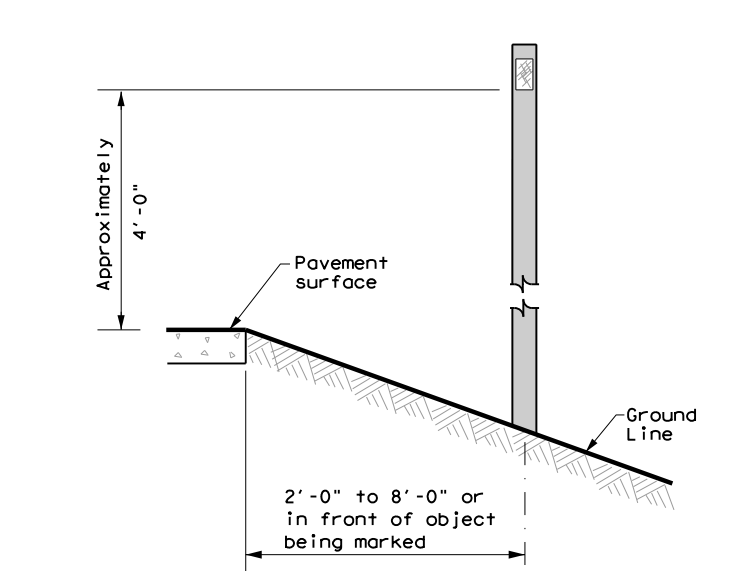

| DEPARTMENTAL MATERIAL SPECIFICATIONS | |
|--|----------|
| FLEXIBLE DELINEATOR & OBJECT MARKER POSTS (EMBEDDED & SURFACE MOUNT TYPES) | DMS-4400 |
| SIGN FACE MATERIALS | DMS-8300 |
| DELINEATORS, OBJECT MARKERS AND BARRIER REFLECTORS | DMS-8600 |

| BARRIER REFLECTORS (BRF) | | | CHEVRONS | | | | ONE DIRECTION LARGE ARROW | | NOTE: Delineator and object marker substrates and sign substrates shall be 0.080" Aluminum sign blank to conform to ASTM B-209 Alloy 6061-T6 or approved alternative. |
|--|--|--|--|--|--|--|--|--|--|
| DEVICE: GF1, GF2, CTB SHEETING: Yellow, White, Red NOTE: 1. Barrier reflectors shall meet the requirements of DMS 8600. 2. Approved Barrier Reflectors are listed on the "Barrier Reflectors" Material Producer List at: www.txdot.gov. | DEVICE: W1-8 SIZE (W x L): 18"x 24" (Conventional), 24"x 30" (Conventional Oversize), 30"x 36" (Expressway), 36" x 48" (Freeway) MOUNTING HEIGHT: 4'-0" or 7'-0", 7'-0" Only | | | | DEVICE: W1-6 SIZE (W x L): 48" x 24" (Conventional), 60" x 30" (Expressway & Freeway) MOUNTING HEIGHT: 7'-0" | | Texas Department of Transportation Traffic Safety Division Standard | | |
| SHEETING: Yellow, White, Red | | | NOTE: 1. CHEVRON (W1-8) signs and ONE DIRECTION LARGE ARROW (W1-6) Signs shall be installed per Sign Mounting Details (SMD) Standard Sheets and paid under Item 644 (Small Roadside Sign Assemblies). 2. When there is a need to increase conspicuity, the Texas version of the ONE DIRECTION LARGE ARROW sign (W1-9T) may be used instead of the ONE DIRECTION LARGE ARROW (W1-6). | | | | | | |

| DELINEATOR & OBJECT MARKER MATERIAL DESCRIPTION | | | | D & OM(1)-20 | |
|---|-----------|------------|-----------|--------------|--|
| FILE: dom1-20.dgn | DN: TXDOT | CK: TXDOT | DW: TXDOT | CR: TXDOT | |
| © TXDOT August 2004 | CONT | SECT | JOB | HIGHWAY | |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | |
| 10-09 3-15 | DIST | COUNTY | SHEET NO. | | |
| 4-10 7-20 | AUS | WILLIAMSON | 125 | | |

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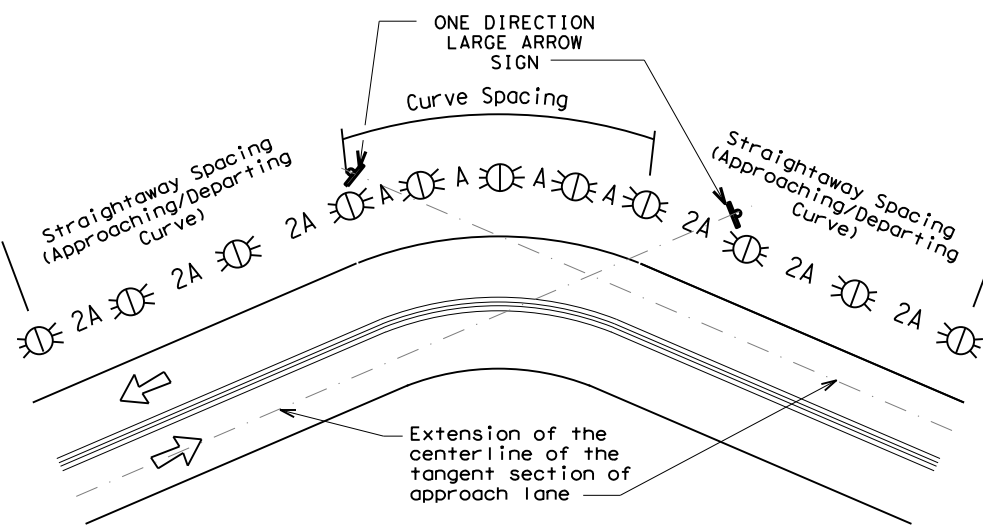
| POST TYPE AND SUPPORT FOUNDATION DETAILS | | | | TYPE OF BARRIER MOUNTS | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|---|--|--|--|--|--|--|--|-------------------|-----------|-----------|-----------|-----------|---------------------|------|------|-----|---------|-----------|------|----|-----------|--------|------------|------|--------|--|-----------|-----------|-----|------------|--|-----|
| WING CHANNEL (WC) | FLEXIBLE POSTS (YFLX, WFLX) | | WEDGE ANCHOR SYSTEMS | | GUARD FENCE ATTACHMENT | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| GND | GND | SRF | WAS | WAP | GF 1 | | | | | | | | | | | | | | | | | | | | | | | | | | | |
|  <p style="text-align: center;">2'-0" Usual</p> |  <p style="text-align: center;">Post</p> <p style="text-align: center;">Stub</p> |  <p style="text-align: center;">Post</p> <p style="text-align: center;">Base</p> |  <p style="text-align: center;">12" Dia.</p> <p style="text-align: center;">27" 30"</p> |  <p style="text-align: center;">3" (Approx.)</p> <p style="text-align: center;">15" 17" 20"</p> <p style="text-align: center;">12" Dia.</p> <p style="text-align: center;">3.5" 17" 1" 2" 30°</p> |  <p style="text-align: center;">Centerline of MBCF rail element</p> |  <p style="text-align: center;">Attached to post or block</p> <p style="text-align: center;">2'-6" Min. 4" Min. 4'-0"</p> | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | EMBEDDED | | SURFACE MOUNT | | STEEL | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| NOTES 1. Embedded Wing Channel (WC) post option may be used for Type 2 Object Markers and Delineators only. 2. 1.12 lbs/ft steel per ASTM A 1011 SS Gr. 50, or ASTM A499. | | NOTES 1. See "Flexible Delineator and Object Marker Posts" Material Producer List for approved devices. 2. Install per manufacturer's recommendations. 3. Post length may vary to meet field conditions. 4. When using yellow delineators with flexible posts to separate opposing direction of travel, such as centerline or median use, the flexible posts shall be yellow. | | NOTE 1. Install per manufacturer's recommendations. | | CONCRETE TRAFFIC BARRIER (CTB)  <p style="text-align: center;">Place Barrier Reflector on top or on side(s) of CTB.</p> | | | | | | | | | | | | | | | | | | | | | | | | | | |
| TYPES 1,3, AND 4 OBJECT MARKERS AND CHEVRONS | | CHEVRONS AND ONE DIRECTION LARGE ARROW SIGN | | DELINEATORS AND TYPE 2 OBJECT MARKERS | | GENERAL NOTES | | | | | | | | | | | | | | | | | | | | | | | | | | |
|  <p style="text-align: center;">4'-0" Pavement surface Ground Line</p> | |  <p style="text-align: center;">7'-0" Pavement surface Ground Line</p> | |  <p style="text-align: center;">Approximately 4'-0" Pavement surface Ground Line</p> <p style="text-align: center;">2'-0" to 8'-0" or in front of object being marked</p> | | <ol style="list-style-type: none"> 1. Place delineators on a section of roadway at a consistent distance from the edge of pavement. 2. Where a restriction prevents consistent placement from the pavement edge, place the affected object markers in line with the innermost edge of the obstruction. 3. When Type 2 object markers and delineators are more than 8'-0" from the edge of the pavement, it may not be possible to maintain a height of approximately 4'-0". If this is the case, place the object marker or delineator as close to the desired height as possible. 4. Install all delineators, object markers and barrier reflectors in accordance with the manufacturer's recommendation. 5. Barrier reflectors should be installed a minimum of 18 inches above the edge of the pavement surface. 6. Diagonal stripes on Type 3 object markers shall slope down toward the intended travel lane. | | | | | | | | | | | | | | | | | | | | | | | | | | |
| NOTE Mounting at 4 feet to the bottom of the chevron is permitted for chevrons that will not exceed a height of 6'-6" to the top of the chevron (sizes 24" x 30" and smaller) | | NOTE Chevrons 30" x 36" and larger shall be mounted at a height of 7' to the bottom of the chevron. Chevron sign and ONE DIRECTION LARGE ARROW sign (W1-9T) shall be installed per SMD standard sheets and paid under item 644. | | See general notes 1, 2 and 3. | |  <p style="text-align: center;">Texas Department of Transportation</p> <p style="text-align: right;">Traffic Safety Division Standard</p> <h2 style="text-align: center;">DELINEATOR & OBJECT MARKER INSTALLATION</h2> <h3 style="text-align: center;">D & OM(2)-20</h3> <table border="1" style="width: 100%; border-collapse: collapse; font-size: small;"> <tr> <td>FILE: dom2-20.dgn</td> <td>DN: TxDOT</td> <td>CK: TxDOT</td> <td>DW: TxDOT</td> <td>CR: TxDOT</td> </tr> <tr> <td>© TxDOT August 2004</td> <td>CONT</td> <td>SECT</td> <td>JOB</td> <td>HIGHWAY</td> </tr> <tr> <td>REVISIONS</td> <td>0914</td> <td>05</td> <td>204, ETC.</td> <td>CR 118</td> </tr> <tr> <td>10-09 3-15</td> <td>DIST</td> <td>COUNTY</td> <td></td> <td>SHEET NO.</td> </tr> <tr> <td>4-10 7-20</td> <td>AUS</td> <td>WILLIAMSON</td> <td></td> <td>126</td> </tr> </table> | | FILE: dom2-20.dgn | DN: TxDOT | CK: TxDOT | DW: TxDOT | CR: TxDOT | © TxDOT August 2004 | CONT | SECT | JOB | HIGHWAY | REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | 10-09 3-15 | DIST | COUNTY | | SHEET NO. | 4-10 7-20 | AUS | WILLIAMSON | | 126 |
| FILE: dom2-20.dgn | DN: TxDOT | CK: TxDOT | DW: TxDOT | CR: TxDOT | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| © TxDOT August 2004 | CONT | SECT | JOB | HIGHWAY | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 10-09 3-15 | DIST | COUNTY | | SHEET NO. | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| 4-10 7-20 | AUS | WILLIAMSON | | 126 | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

| Amount by which Advisory Speed is less than Posted Speed | Curve Advisory Speed | |
|--|--|---|
| | Turn (30 MPH or less) | Curve (35 MPH or more) |
| 5 MPH & 10 MPH | • RPMs | • RPMs |
| 15 MPH & 20 MPH | • RPMs and One Direction Large Arrow sign | • RPMs and Chevrons; or • RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons. |
| 25 MPH & more | • RPMs and Chevrons; or • RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons | • RPMs and Chevrons |

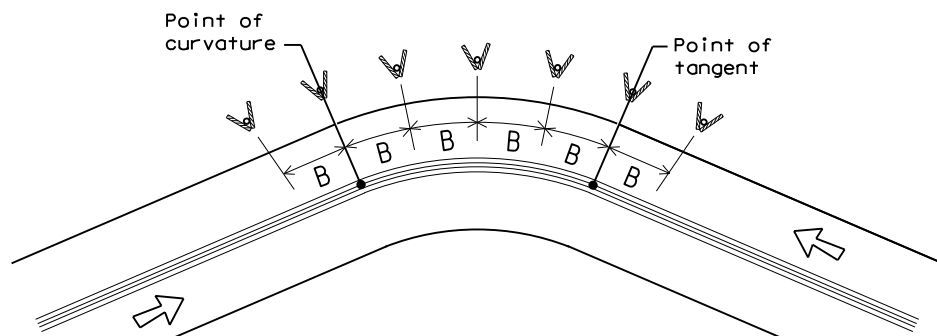
SUGGESTED SPACING FOR DELINEATORS ON HORIZONTAL CURVES



NOTE

ONE DIRECTION LARGE ARROW (W1-6) sign should be located at approximately and perpendicular to the extension of the centerline of the tangent section of approach lane.

SUGGESTED SPACING FOR CHEVRONS ON HORIZONTAL CURVES



NOTE

At least one chevron pair is installed beyond the point of tangent in tangent section.

DELINEATOR AND CHEVRON SPACING

| WHEN DEGREE OF CURVE OR RADIUS IS KNOWN | | | | |
|---|-----------------|------------------|-------------------------|--------------------------|
| Degree of Curve | FEET | | | |
| | Radius of Curve | Spacing in Curve | Spacing in Straightaway | Chevron Spacing in Curve |
| | | A | 2A | B |
| 1 | 5730 | 225 | 450 | — |
| 2 | 2865 | 160 | 320 | — |
| 3 | 1910 | 130 | 260 | 200 |
| 4 | 1433 | 110 | 220 | 160 |
| 5 | 1146 | 100 | 200 | 160 |
| 6 | 955 | 90 | 180 | 160 |
| 7 | 819 | 85 | 170 | 160 |
| 8 | 716 | 75 | 150 | 160 |
| 9 | 637 | 75 | 150 | 120 |
| 10 | 573 | 70 | 140 | 120 |
| 11 | 521 | 65 | 130 | 120 |
| 12 | 478 | 60 | 120 | 120 |
| 13 | 441 | 60 | 120 | 120 |
| 14 | 409 | 55 | 110 | 80 |
| 15 | 382 | 55 | 110 | 80 |
| 16 | 358 | 55 | 110 | 80 |
| 19 | 302 | 50 | 100 | 80 |
| 23 | 249 | 40 | 80 | 80 |
| 29 | 198 | 35 | 70 | 40 |
| 38 | 151 | 30 | 60 | 40 |
| 57 | 101 | 20 | 40 | 40 |

Curve delineator approach and departure spacing should include 3 delineators spaced at 2A. This spacing should be used during design preparation or when the degree of curve is known.

DELINEATOR AND CHEVRON SPACING

| WHEN DEGREE OF CURVE OR RADIUS IS NOT KNOWN | | | |
|---|------------------|-------------------------|--------------------------|
| Advisory Speed (MPH) | Spacing in Curve | Spacing in Straightaway | Chevron Spacing in Curve |
| | A | 2xA | B |
| 65 | 130 | 260 | 200 |
| 60 | 110 | 220 | 160 |
| 55 | 100 | 200 | 160 |
| 50 | 85 | 170 | 160 |
| 45 | 75 | 150 | 120 |
| 40 | 70 | 140 | 120 |
| 35 | 60 | 120 | 120 |
| 30 | 55 | 110 | 80 |
| 25 | 50 | 100 | 80 |
| 20 | 40 | 80 | 80 |
| 15 | 35 | 70 | 40 |

If the degree of curve is not known, delineator spacing may be determined based on the Advisory Speed of the curve. Use the delineator curve spacing for each Advisory Speed (MPH).

DELINEATOR AND OBJECT MARKER APPLICATION AND SPACING

| CONDITION | REQUIRED TREATMENT | MINIMUM SPACING |
|--|---|---|
| Frwy./Exp. Tangent | RPMs | See PM-series and FPM-series standard sheets |
| Frwy./Exp. Curve | Single delineators on right side | See delineator spacing table |
| Frwy/Exp. Ramp | Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4)) | 100 feet on ramp tangents Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves) |
| Acceleration/Deceleration Lane | Double delineators (see Detail 3 on D&OM(4)) | 100 feet (See Detail 3 on D & OM (4)) |
| Truck Escape Ramp | Single red delineators on both sides | 50 feet |
| Bridge Rail (steel or concrete) and Metal Beam Guard Fence | Bi-Directional Delineators when undivided with one lane each direction Single Delineators when multiple lanes each direction | Equal spacing (100' max) but not less than 3 delineators |
| Concrete Traffic Barrier (CTB) or Steel Traffic Barrier | Barrier reflectors matching the color of the edge line | Equal spacing 100' max |
| Cable Barrier | Reflectors matching the color of the edge line | Every 5th cable barrier post (up to 100' max) |
| Guard Rail Terminus/Impact Head | Divided highway - Object marker on approach end Undivided 2-lane highways - Object marker on approach and departure end | Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6) |
| Bridges with no Approach Rail | Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail | See D & OM(5) |
| Reduced Width Approaches to Bridge Rail | Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge | Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) |
| Culverts without MBGF | Type 2 Object Markers | See Detail 2 on D & OM(4) |
| Crossovers | Double yellow delineators and RPMs | See Detail 1 on D & OM (4) |
| Pavement Narrowing (lane merge) on Freeways/Expressway | Single delineators adjacent to affected lane for full length of transition | 100 feet |

NOTES

- Unless indicated otherwise, the delineator or barrier reflector color shall conform to the color of the pavement edge line on the side of the road where the delineators or barrier reflectors are placed.
- Barrier reflectors may be used to replace required delineators.
- Single red delineators may be mounted on the back side of delineator posts for wrong way driver applications

LEGEND

| | |
|--|---------------------------|
| | Bi-directional Delineator |
| | Delineator |
| | Sign |



DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

D & OM(3)-20

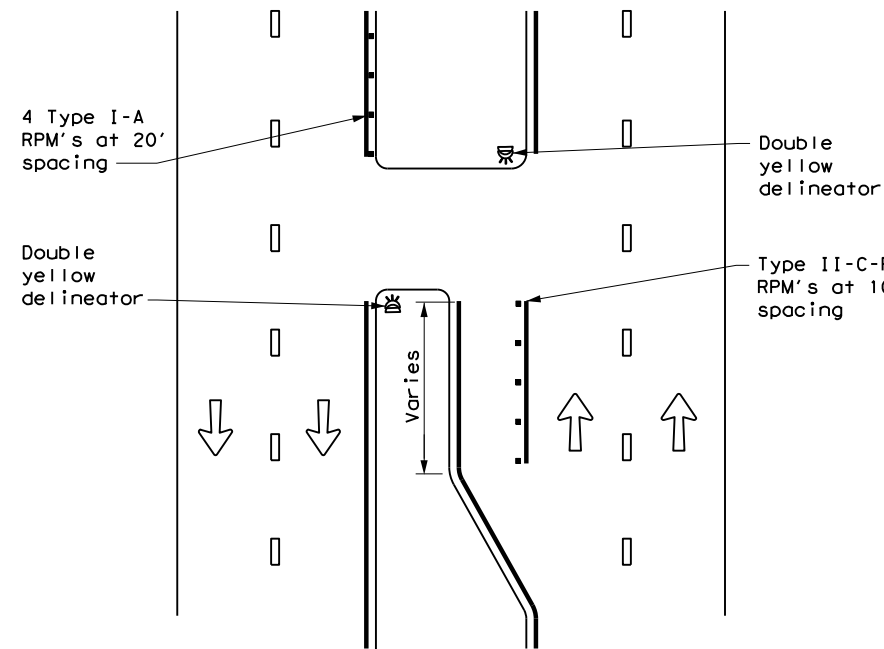
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| © TXDOT August 2004 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 3-15 8-15 | DIST | COUNTY | SHEET NO. | |
| 8-15 7-20 | AUS | WILLIAMSON | 127 | |

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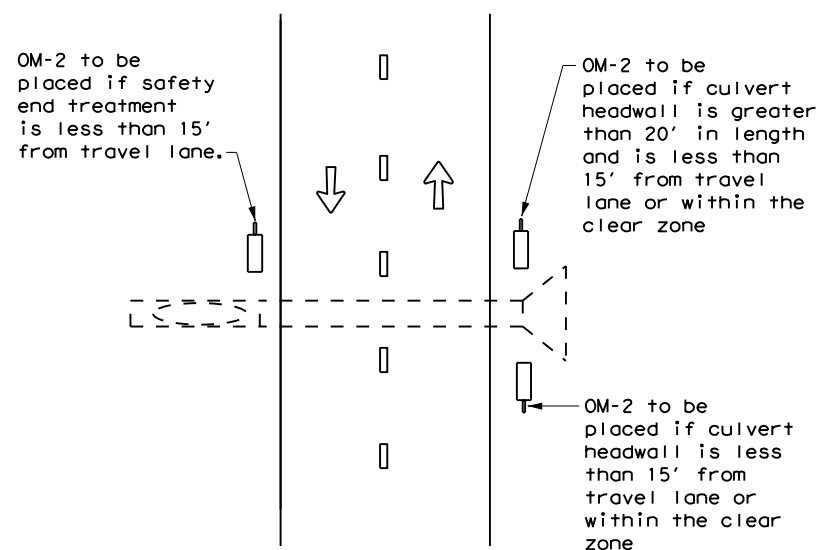
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CROSSOVERS



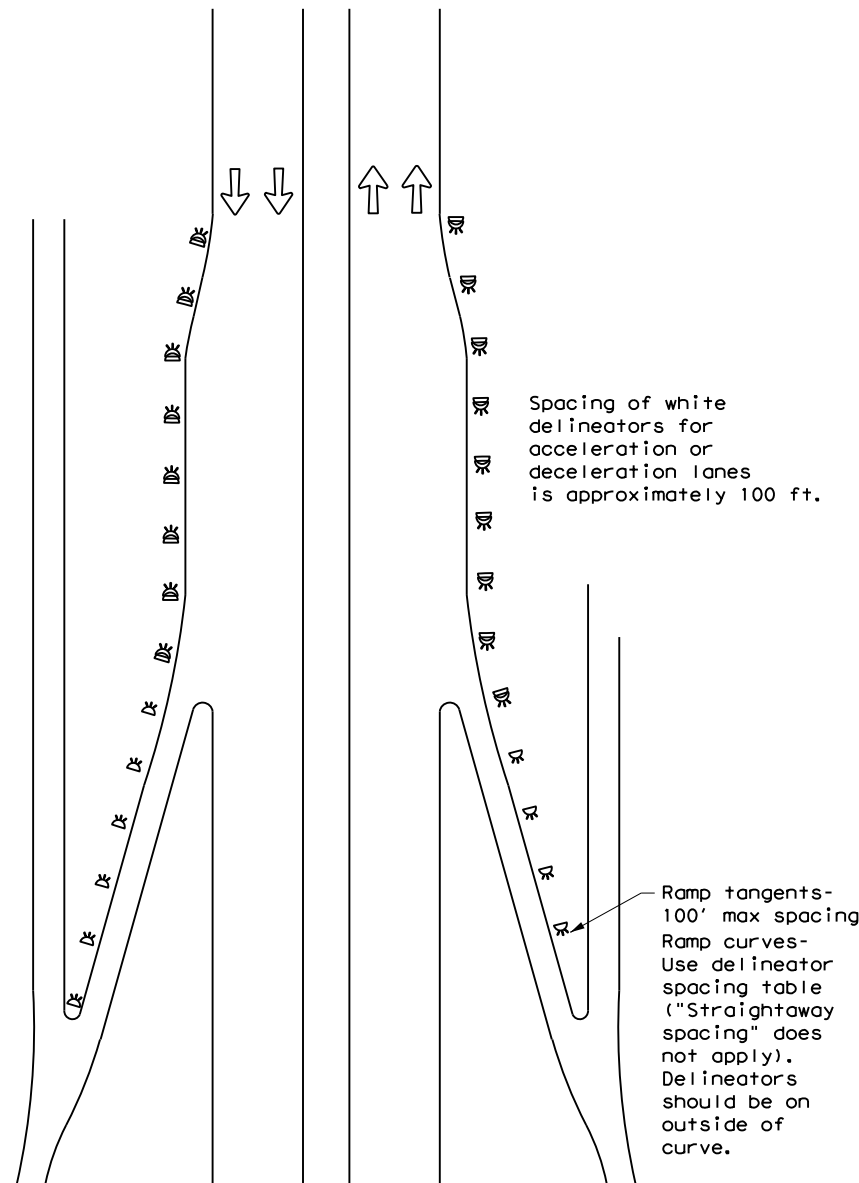
DETAIL 1

FOR CULVERTS WITHOUT MBGF



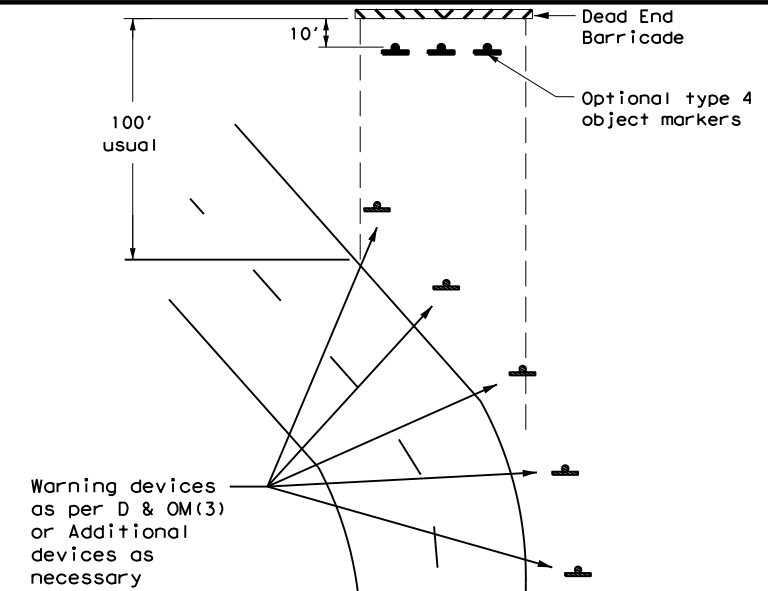
DETAIL 2

FREEWAY DELINEATION FOR RAMPS AND ACCELERATION/DECELERATION LANES



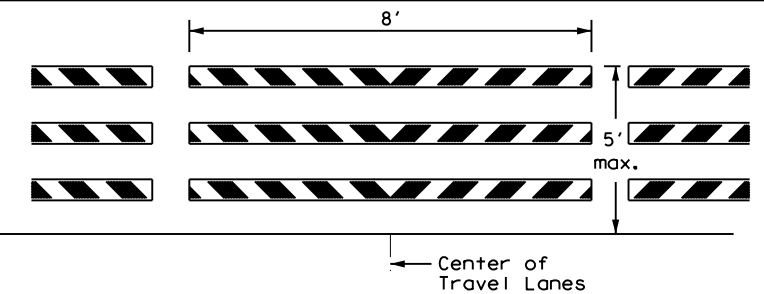
DETAIL 3

TYPICAL APPLICATION OF DEAD END BARRICADE



DETAIL 4

TYPICAL DEAD END BARRICADE INSTALLATION



NOTES

- Barricade striping shall be red and white reflective sheeting for all permanent road closures.
- Barricade striping is red and white sloping toward the center of the roadway.
- Type 3 Barricade Supports should be anchored to soil or pavement as described in compliant Work Zone Traffic Control Devices List, section D.2.f and D.2.g.

DETAIL 5

| LEGEND | |
|--------|--------------------------|
| | Bidirectional Delineator |
| | Delineator |
| | OM-3 |
| | Barricade |
| | Sign |
| | OM-2 |
| | Double Delineator |



DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

D & OM(4) -20

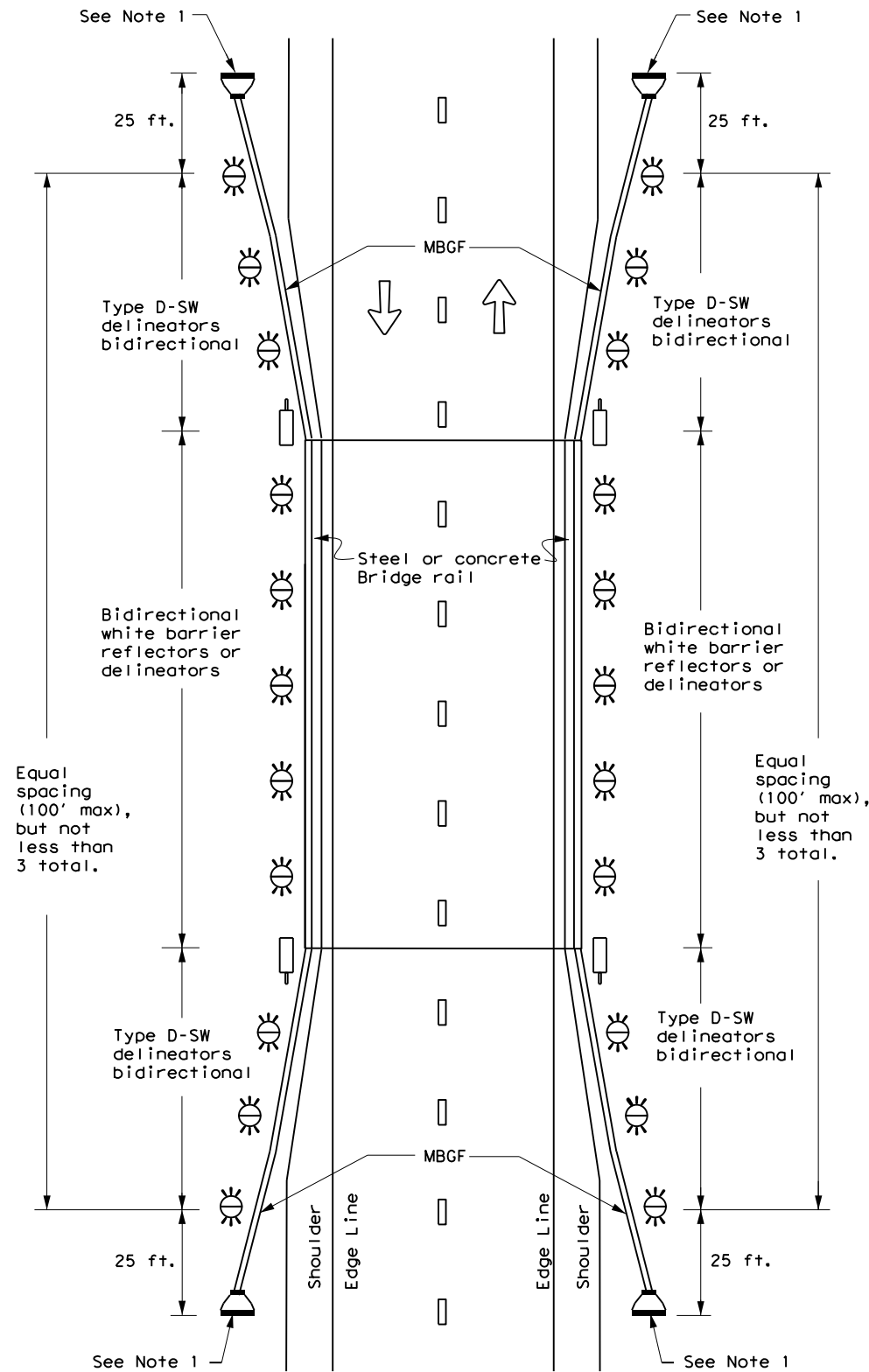
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| 3-15 | DIST | COUNTY | SHEET NO. | |
| 7-20 | AUS | WILLIAMSON | 128 | |

**TWO-WAY, TWO LANE ROADWAY
WITH REDUCED WIDTH APPROACH RAIL**

**TWO-WAY, TWO LANE ROADWAY
WITH METAL BEAM GUARD FENCE (MBGF)**

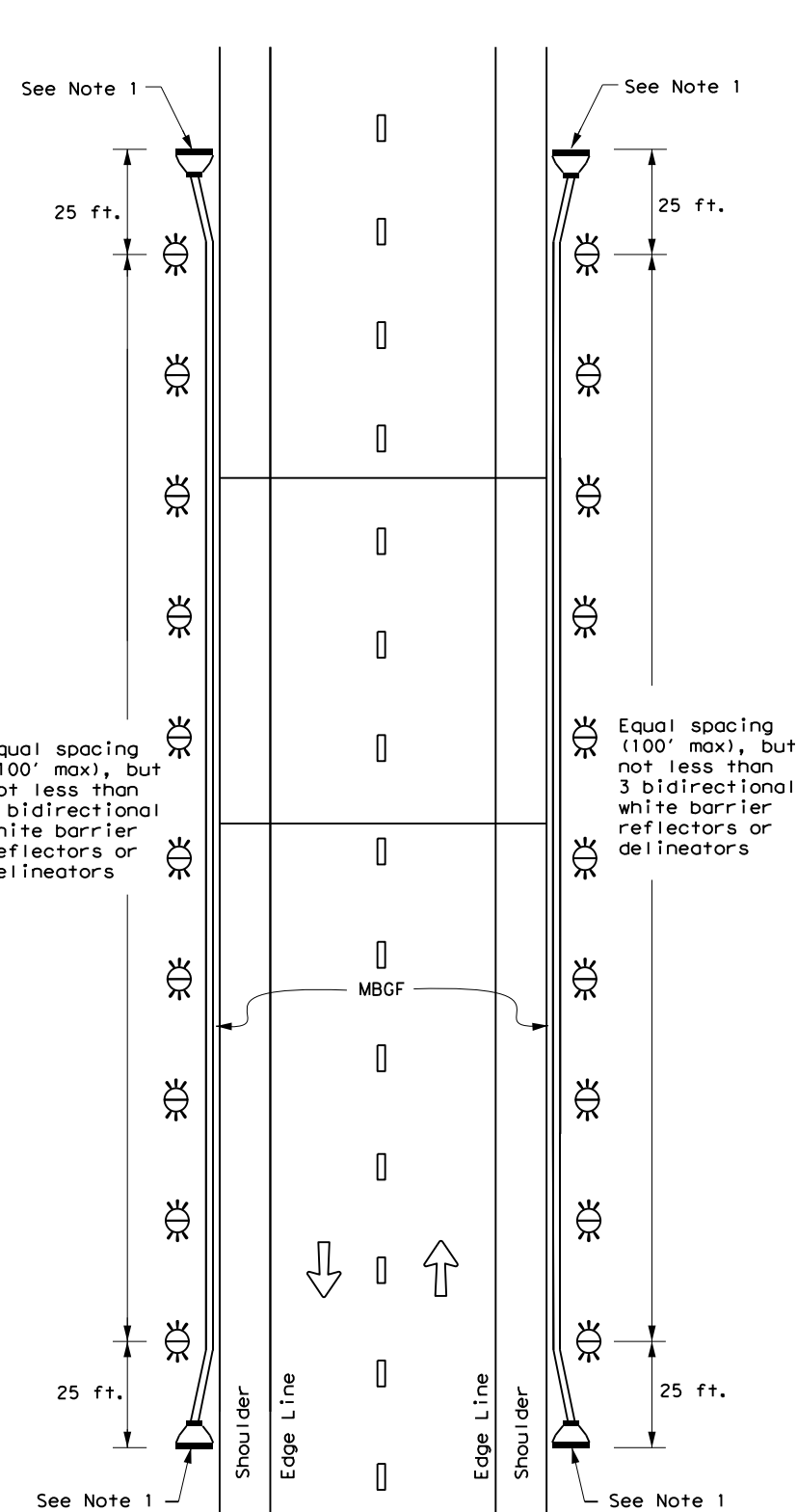
**TWO-WAY, TWO LANE ROADWAY
BRIDGE WITH NO APPROACH RAIL**

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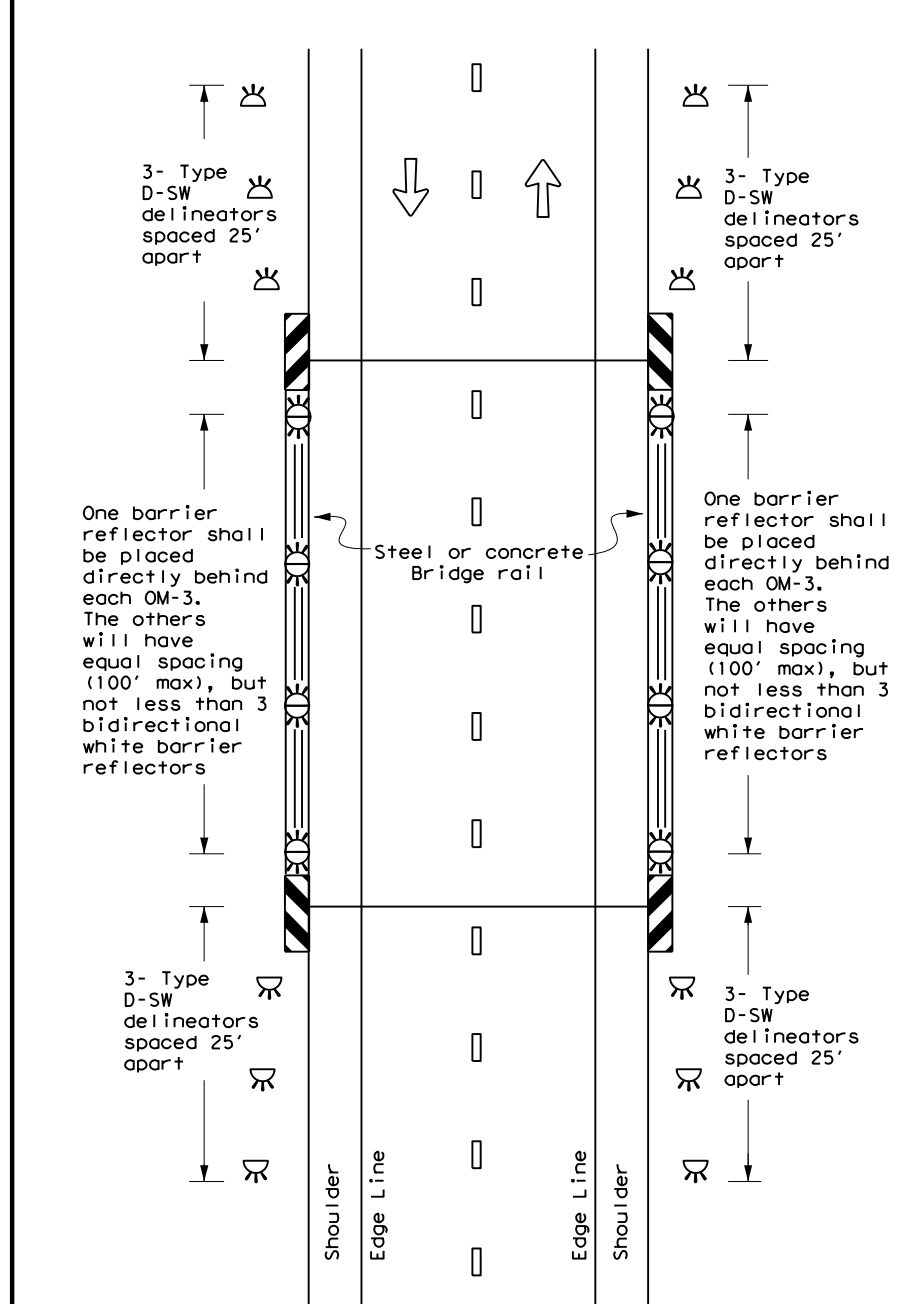
NOTE:

1. Terminal ends require reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end.



NOTE:

1. Terminal ends require reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end.



| LEGEND | |
|--------|--------------------------|
| | Bidirectional Delineator |
| | Delineator |
| | OM-3 |
| | OM-2 |
| | Terminal End |
| | Traffic Flow |

Texas Department of Transportation
Traffic Safety Division Standard

DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

D & OM(5)-20

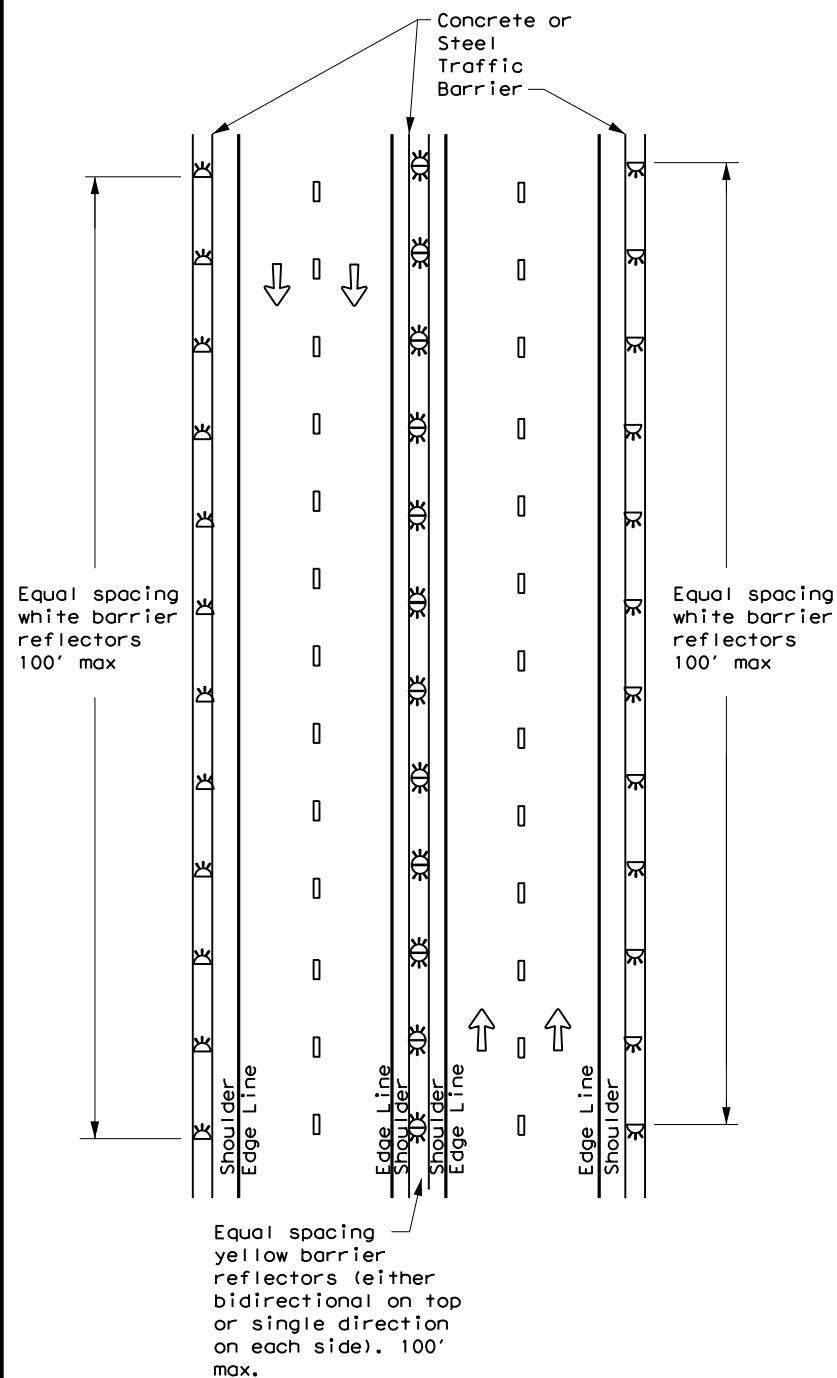
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| © TxDOT August 2015 | CONT | SECT | JOB | HIGHWAY |
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| DIST | COUNTY | SHEET NO. | | |
| AUS | WILLIAMSON | 129 | | |

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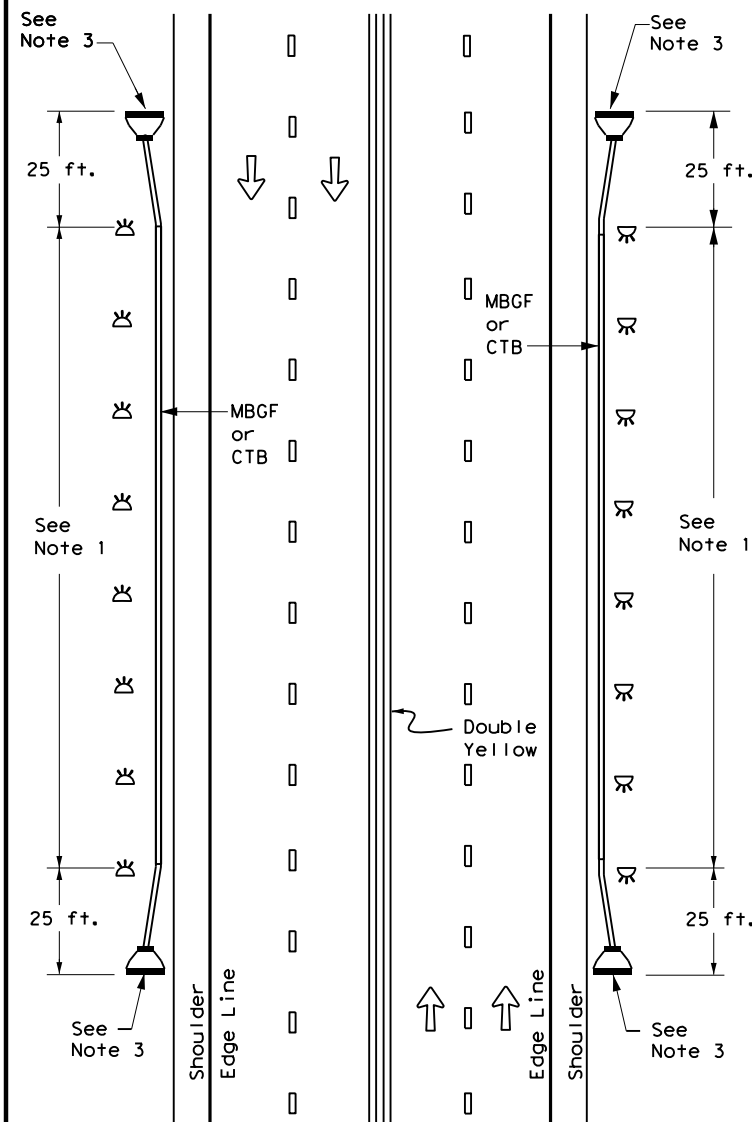
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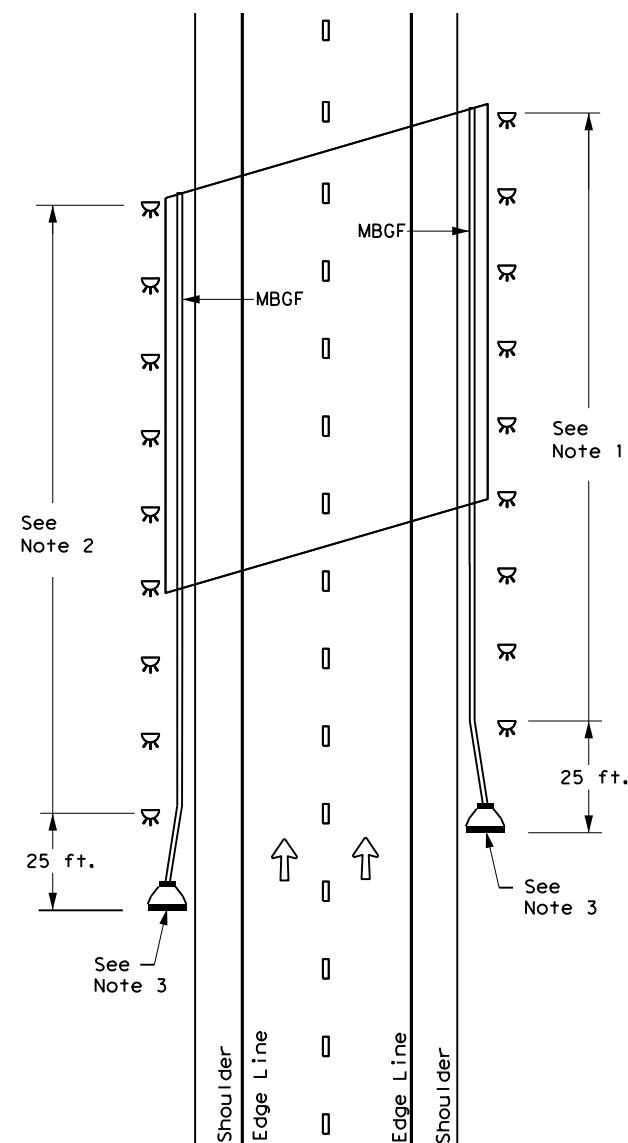
CONTINUOUS CONCRETE OR STEEL BARRIER



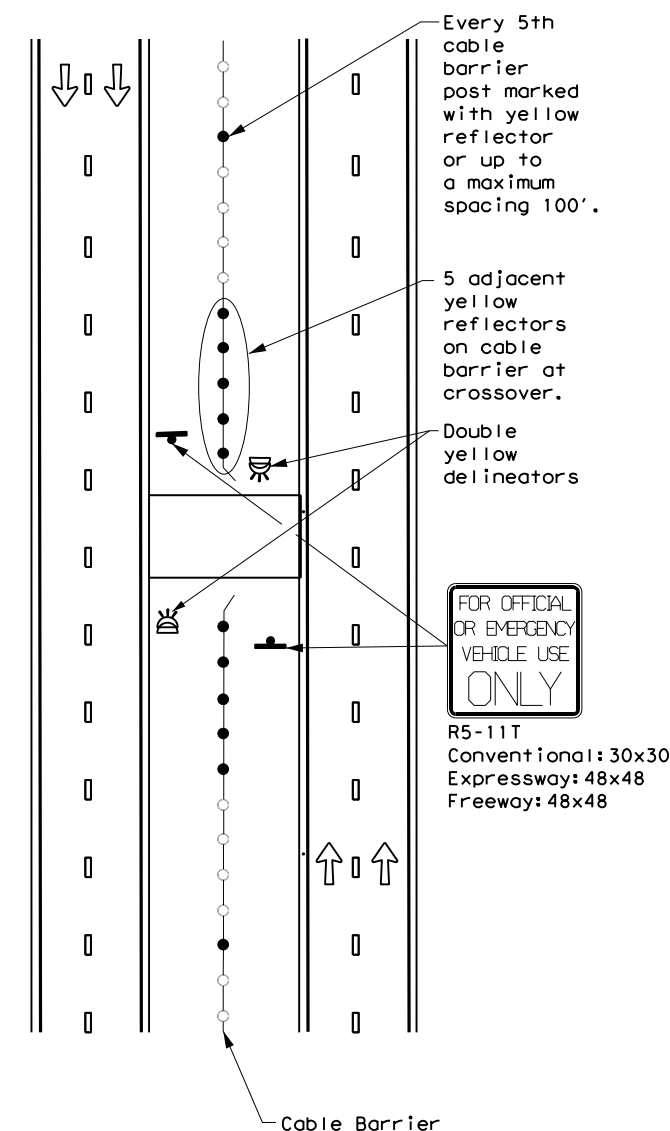
MULTI-LANE UNDIVIDED, TWO-WAY ROADWAY WITH METAL BEAM GUARD FENCE (MBGF)



DIVIDED ROADWAY WITH METAL BEAM GUARD FENCE (MBGF)



EMERGENCY CROSSOVER



NOTES

1. Equal spacing (100' max), but not less than 3 single directional white barrier reflectors or delineators. On Continuous Barrier, equal spacing (100' max.)
2. Equal spacing (100' max), but not less than 3 single directional yellow barrier reflectors or delineators.
3. Terminal ends require reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end.

LEGEND

| | |
|--|--------------------------|
| | Bidirectional Delineator |
| | Delineator |
| | OM-3 |
| | OM-2 |
| | Terminal End |
| | Traffic Flow |

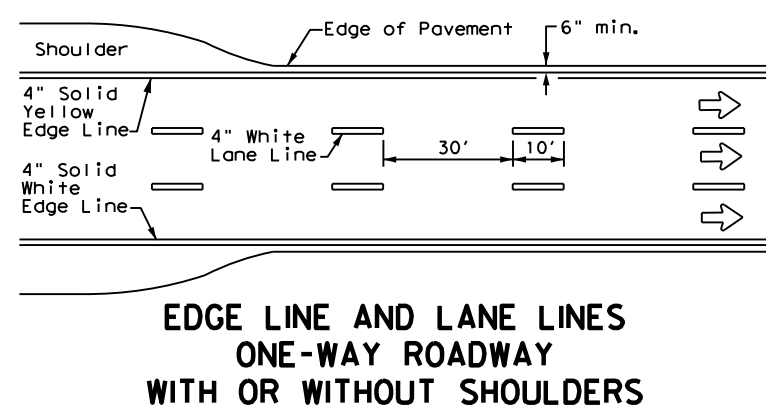
Texas Department of Transportation
 Traffic Safety Division Standard

DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

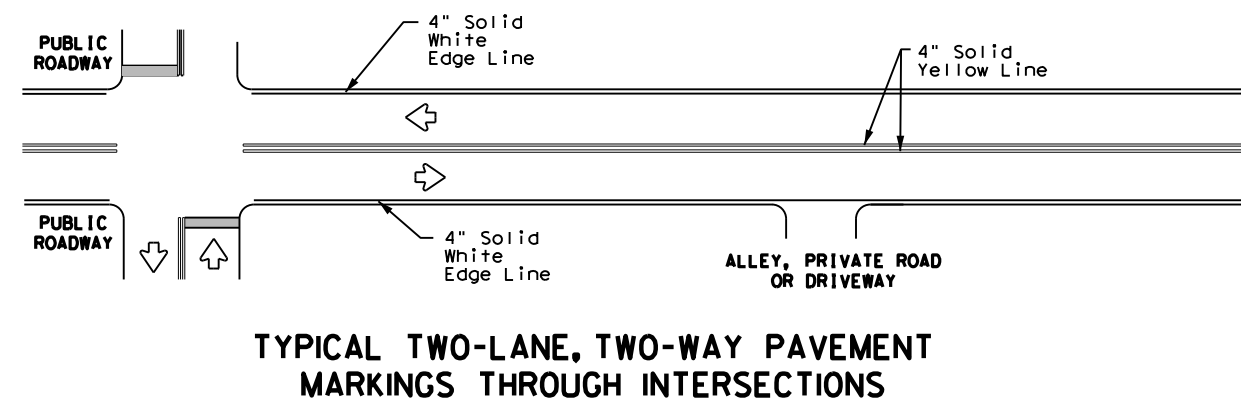
D & OM(6)-20

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| | DIST | COUNTY | SHEET NO. | |
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**EDGE LINE AND LANE LINES
ONE-WAY ROADWAY
WITH OR WITHOUT SHOULDERS**

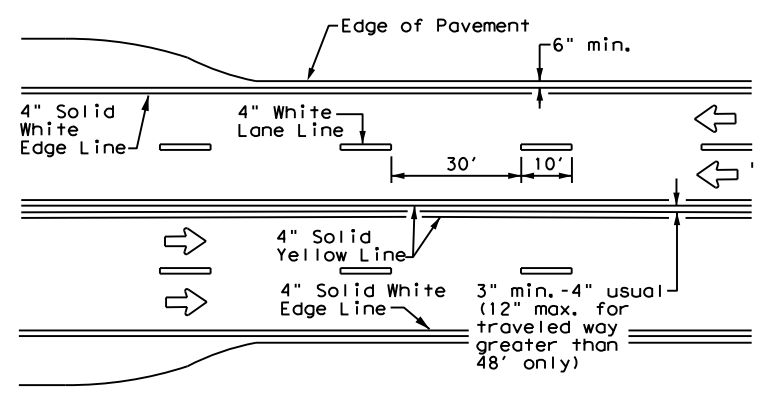


**TYPICAL TWO-LANE, TWO-WAY PAVEMENT
MARKINGS THROUGH INTERSECTIONS**

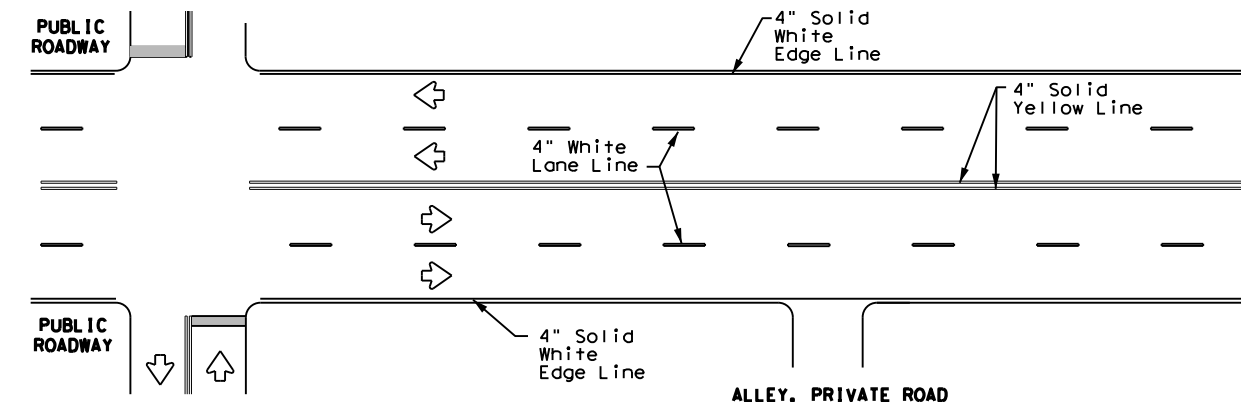
- GENERAL NOTES**
1. Edgeline striping shall be as shown in the plans or as directed by the Engineer. The edgeline should not be placed less than 6 inches from the edge of pavement. This distance may vary due to pavement raveling or other conditions. Edgelines are not required in curb and gutter sections of roadways.
 2. The traveled way includes only that portion of the roadway used for vehicular travel. It does not include the parking lanes, sidewalks, berms and shoulders. The traveled ways shall be measured from the inside of edgeline to the inside of edgeline of a two lane roadway.

| MATERIAL SPECIFICATIONS | |
|---|----------|
| PAVEMENT MARKERS (REFLECTORIZED) | DMS-4200 |
| EPOXY AND ADHESIVES | DMS-6100 |
| BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS | DMS-6130 |
| TRAFFIC PAINT | DMS-8200 |
| HOT APPLIED THERMOPLASTIC | DMS-8220 |
| PERMANENT PREFABRICATED PAVEMENT MARKINGS | DMS-8240 |

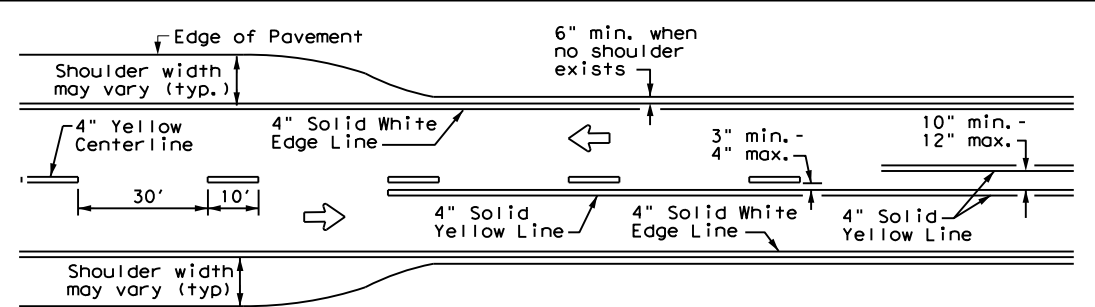
All pavement marking materials shall meet the required Departmental Material Specifications as specified by the plans.



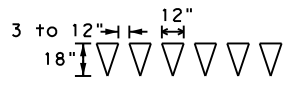
**CENTERLINE AND LANE LINES
FOUR LANE TWO-WAY ROADWAY
WITH OR WITHOUT SHOULDERS**



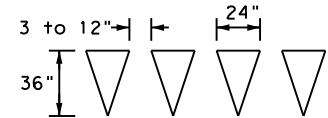
**TYPICAL MULTI-LANE, TWO-WAY PAVEMENT
MARKINGS THROUGH INTERSECTIONS**



**TWO LANE TWO-WAY ROADWAY
WITH OR WITHOUT SHOULDERS**

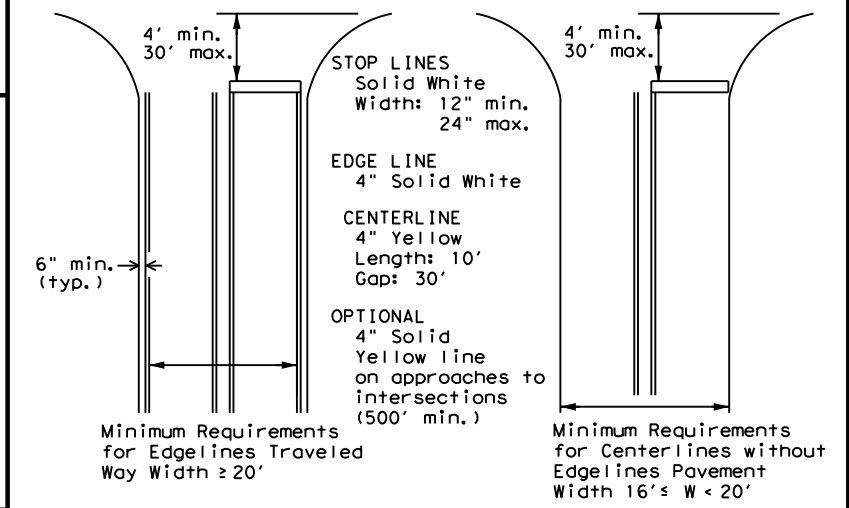


For posted speed on road being marked equal to or less than 40 MPH.



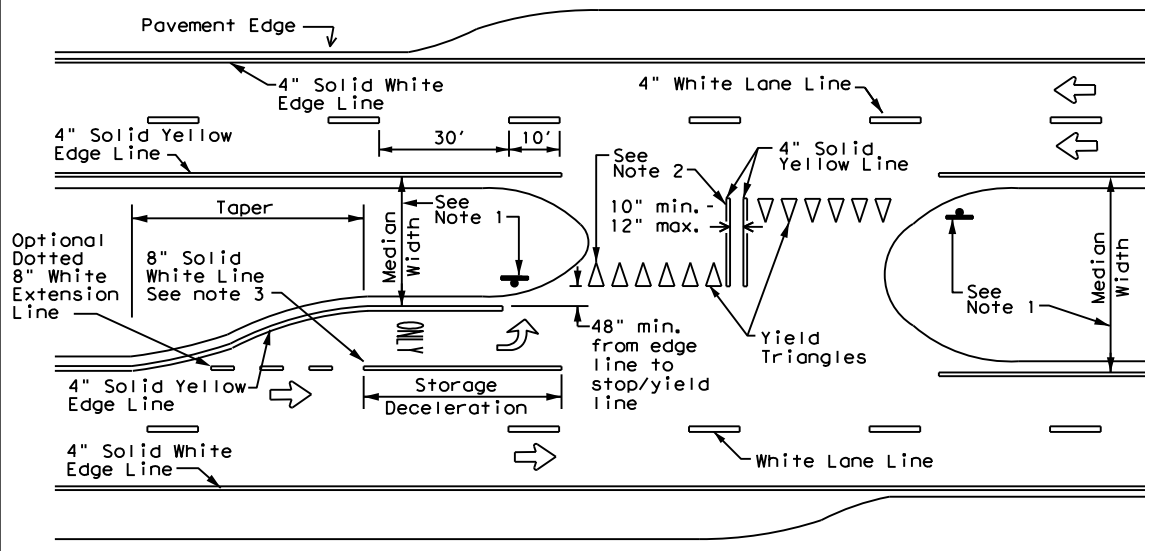
For posted speed on road being marked equal to or greater than 45 MPH.

YIELD LINES



**GUIDE FOR PLACEMENT OF STOP LINES,
EDGE LINE & CENTERLINE**
Based on Traveled Way and Pavement Widths
for Undivided Highways

- NOTES**
1. Where divided highways are separated by median widths at the median opening itself of 30 feet or more, median openings shall be signed as two separate intersections. Each median opening has two width measurements, with one measurement for each approach. The narrow median width will be the controlling width to determine if signs are required. Yield signs are the typical intersection control. Stop signs are optional as determined by the Engineer.
 2. Install median striping (double yellow centerlines and stop bars/yield triangles) when a 50' or greater median centerline can be placed. Stop bars shall only be used with stop signs. Yield triangles shall only be used with yield signs.
 3. Length of turn bays, including taper, deceleration, and storage lengths shall be as shown on the plans or as directed by the Engineer.



FOUR LANE DIVIDED ROADWAY CROSSOVERS



**TYPICAL STANDARD
PAVEMENT MARKINGS**

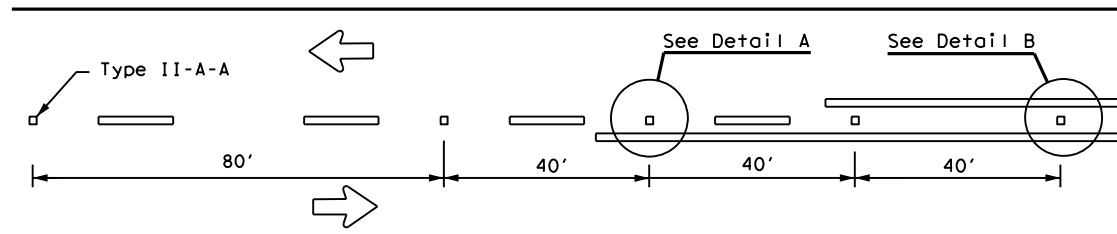
PM(1) - 20

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| © TxDOT November 1978 | CONT | SECT | JOB | HIGHWAY |
| 8-95 3-03 REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 5-00 2-12 | DIST | COUNTY | SHEET NO. | |
| 8-00 6-20 | AUS | WILLIAMSON | 131 | |

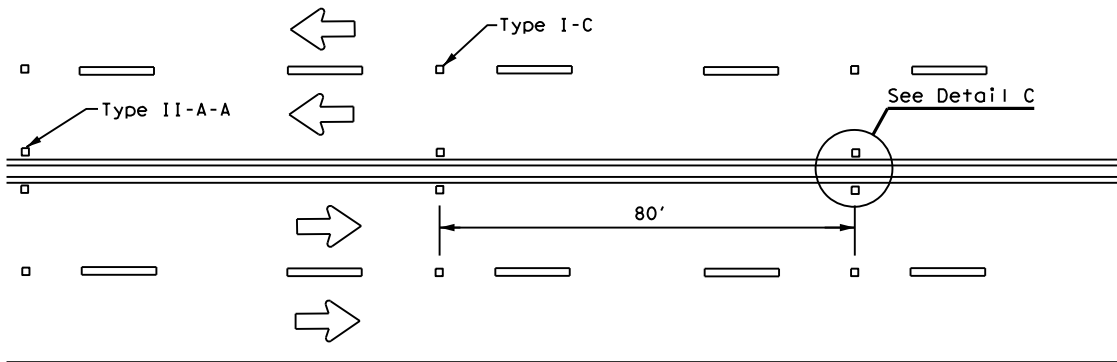
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REFLECTIVE RAISED PAVEMENT MARKERS FOR VEHICLE POSITIONING GUIDANCE

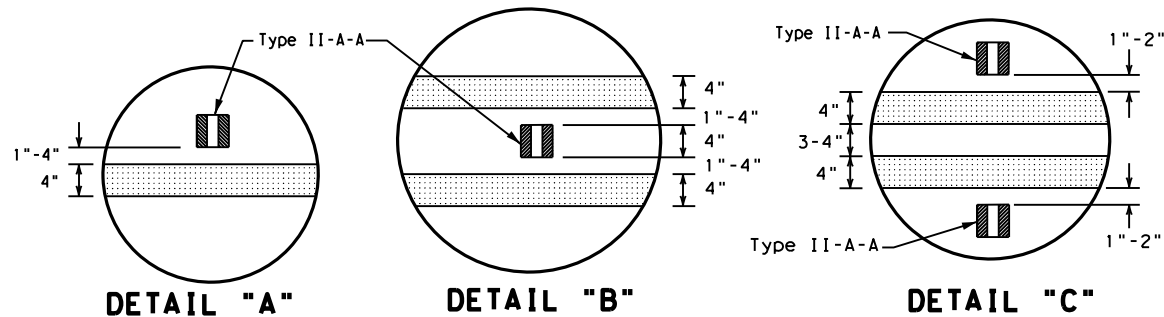
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CENTERLINE FOR ALL TWO LANE ROADWAYS



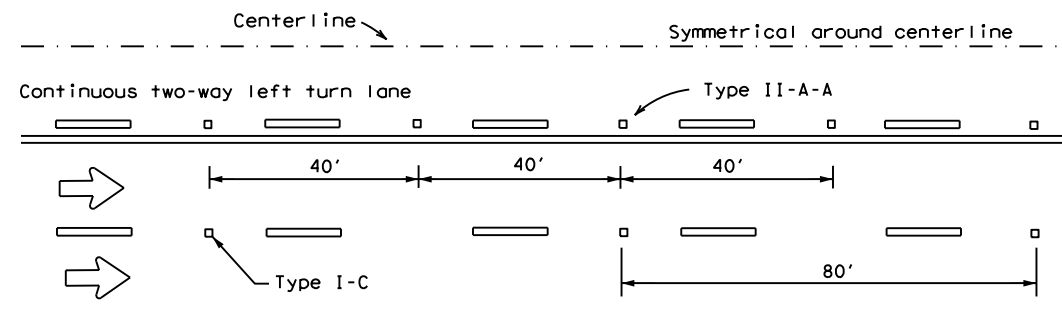
**CENTERLINE & LANE LINES
FOR FOUR LANE TWO-WAY HIGHWAYS**



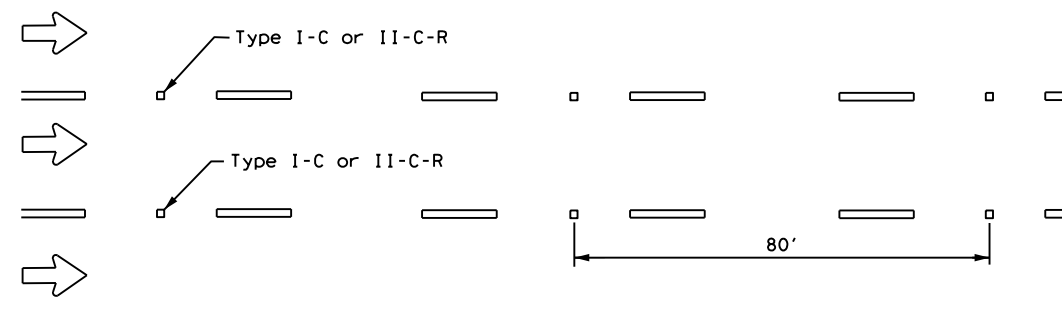
DETAIL "A"

DETAIL "B"

DETAIL "C"



CENTERLINE AND LANE LINES FOR TWO-WAY LEFT TURN LANE

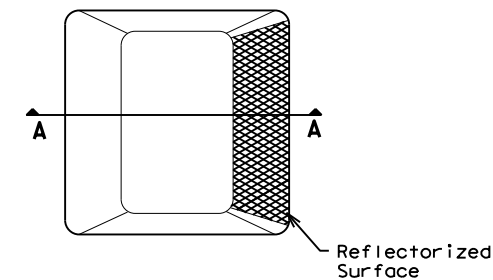


LANE LINES FOR ONE-WAY ROADWAY (NON-FREEWAY FACILITIES)

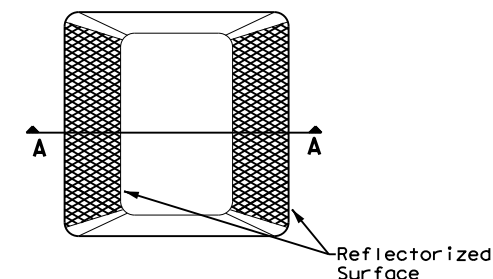
Raised pavement markers Type II-C-R shall have clear face toward normal traffic and red face toward wrong-way traffic.

| MATERIAL SPECIFICATIONS | |
|---|----------|
| PAVEMENT MARKERS (REFLECTORIZED) | DMS-4200 |
| EPOXY AND ADHESIVES | DMS-6100 |
| BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS | DMS-6130 |
| TRAFFIC PAINT | DMS-8200 |
| HOT APPLIED THERMOPLASTIC | DMS-8220 |
| PERMANENT PREFABRICATED PAVEMENT MARKINGS | DMS-8240 |

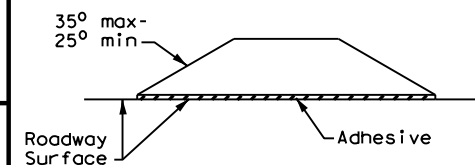
All pavement marking materials shall meet the required Departmental Material Specifications as specified by the plans.



Type I (Top View)



Type II (Top View)



SECTION A

RAISED PAVEMENT MARKERS

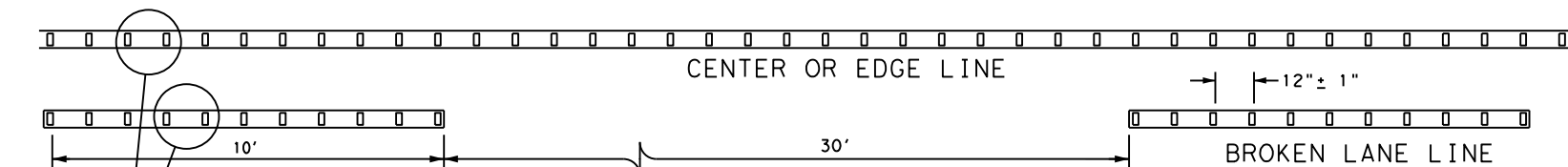


POSITION GUIDANCE USING RAISED MARKERS REFLECTORIZED PROFILE MARKINGS PM(2) - 20

| | | | | |
|---------------------|------|------------|-----------|---------|
| FILE: pm2-20.dgn | DN: | CK: | DW: | CK: |
| © TxDOT April 1977 | CONT | SECT | JOB | HIGHWAY |
| 4-92 2-10 REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| 5-00 2-12 | DIST | COUNTY | SHEET NO. | |
| 8-00 6-20 | AUS | WILLIAMSON | 132 | |

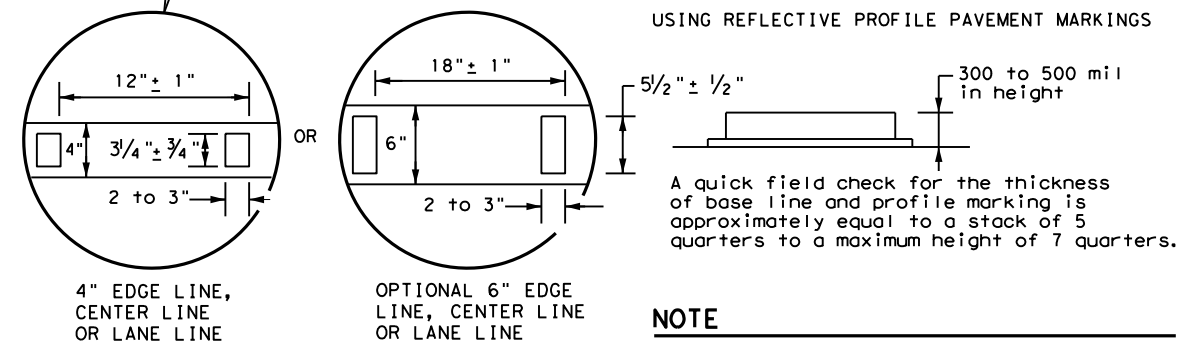
GENERAL NOTES

- All raised pavement markers placed in broken lines shall be placed in line with and midway between the stripes.
- On concrete pavements the raised pavement markers should be placed to one side of the longitudinal joints.



REFLECTORIZED PROFILE PATTERN DETAIL

USING REFLECTIVE PROFILE PAVEMENT MARKINGS



NOTE

Profile markings shall not be placed on roadways with a posted speed limit of 45 MPH or less.

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SIGN SUPPORT DESCRIPTIVE CODES

(Descriptive Codes correspond to project estimate and quantities sheets)

SM RD SGN ASSM TY XXXXX(X)XX(X-XXXX)

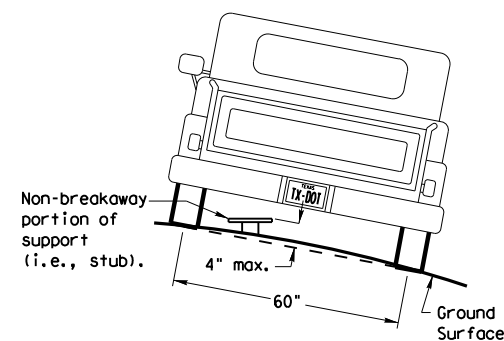
Post Type
 FRP = Fiberglass Reinforced Plastic Pipe (see SMD(FRP))
 TWT = Thin-Walled Tubing (see SMD(TWT))
 10BWG = 10 BWG Tubing (see SMD(SLIP-1) to (SLIP-3))
 S80 = Schedule 80 Pipe (see SMD(SLIP-1) to (SLIP-3))

Number of Posts (1 or 2)

Anchor Type
 UA = Universal Anchor - Concreted (see SMD(FRP) and (TWT))
 UB = Universal Anchor - Bolted down (see SMD(FRP) and (TWT))
 WS = Wedge Anchor Steel - (see SMD(TWT))
 WP = Wedge Anchor Plastic (see SMD(TWT))
 SA = Slipbase - Concreted (see SMD(SLIP-1) to (SLIP-3))
 SB = Slipbase - Bolted Down (see SMD(SLIP-1) to (SLIP-3))

Sign Mounting Designation
 P = Prefab. "Plain" (see SMD(SLIP-1) to (SLIP-3), (TWT), (FRP))
 T = Prefab. "T" (see SMD(SLIP-1) to (SLIP-3), (TWT))
 U = Prefab. "U" (see SMD(SLIP-1) to (SLIP-3))
 IF REQUIRED
 1EXT or 2EXT = Number of Extensions (see SMD(SLIP-1) to (SLIP-3), (TWT))
 BM = Extruded Wind Beam (see SMD(SLIP-1) to (SLIP-3))
 WC = 1.12 #/ft Wing Channel (see SMD(SLIP-1) to (SLIP-3))
 EXAL = Extruded Aluminum Sign Panels (see SMD(SLIP-3))

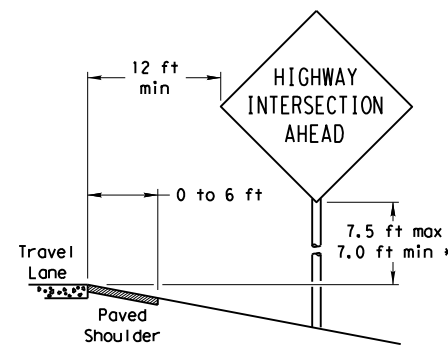
REQUIRED CLEARANCE FOR BREAKAWAY SUPPORT



To avoid vehicle undercarriage snagging, any substantial remains of a breakaway support, when it is broken away, should not project more than 4 inches above a 60-inch chord (i.e., typical space between wheel paths).

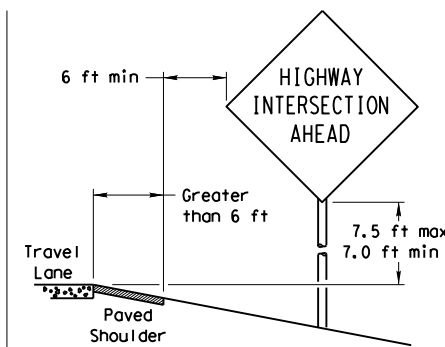
SIGN LOCATION

PAVED SHOULDERS



LESS THAN 6 FT. WIDE

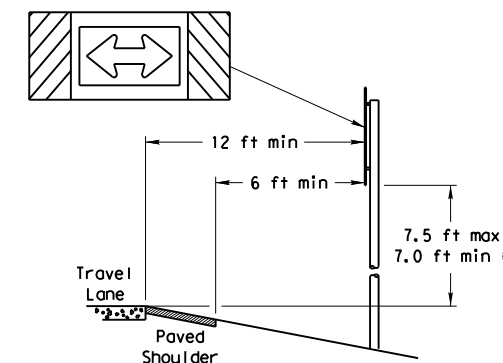
When the shoulder is 6 ft. or less in width, the sign must be placed at least 12 ft. from the edge of the travel lane.



GREATER THAN 6 FT. WIDE

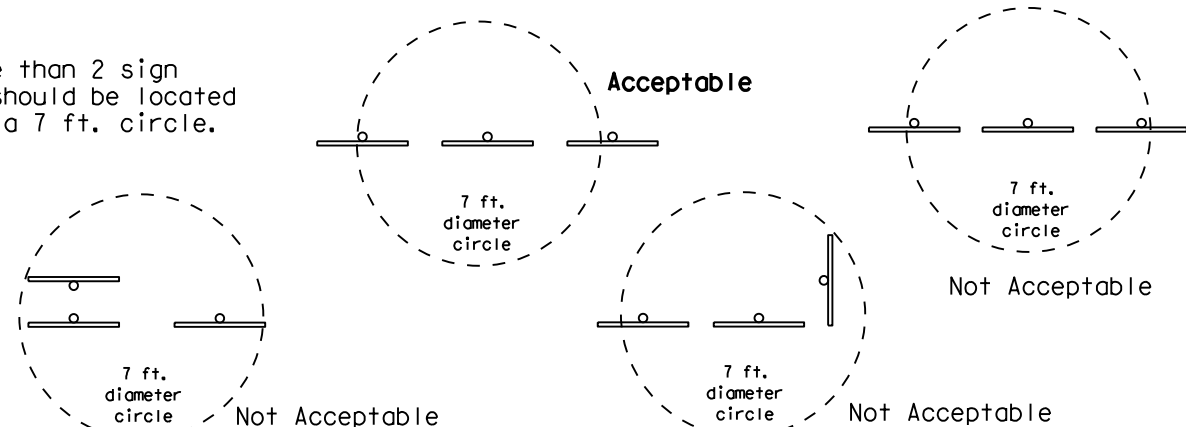
When the shoulder is greater than 6 ft in width, the sign must be placed at least 6 ft. from the edge of the shoulder.

T-INTERSECTION

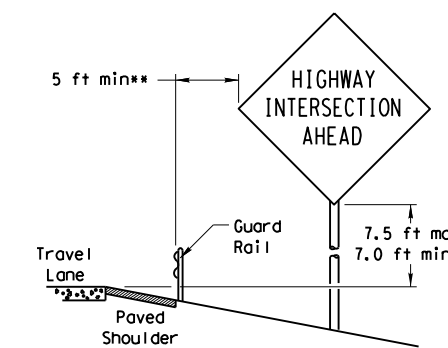


When this sign is needed at the end of a two-lane, two way roadway, the right edge of the sign should be in line with the centerline of the roadway. Place as close to ROW as practical.

No more than 2 sign posts should be located within a 7 ft. circle.

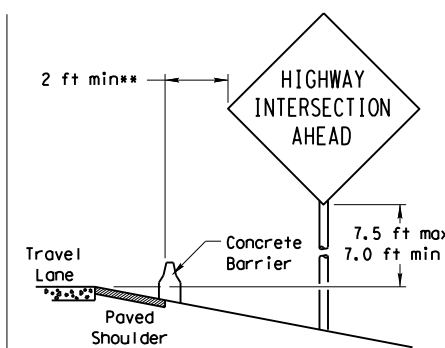


BEHIND BARRIER

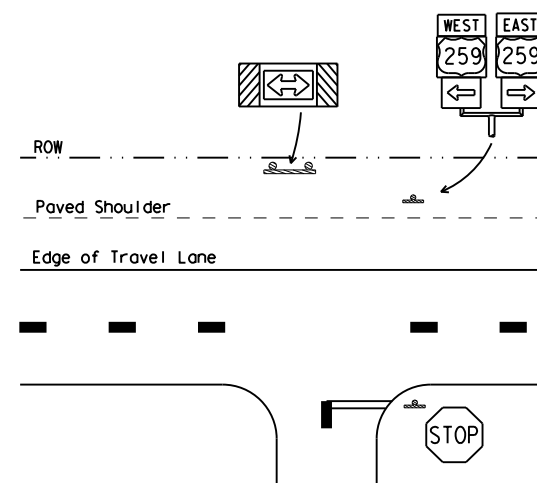


BEHIND GUARDRAIL

**Sign clearance based on distance required for proper guard rail or concrete barrier performance.



BEHIND CONCRETE BARRIER



* Signs shall be mounted using the following condition that results in the greatest sign elevation:

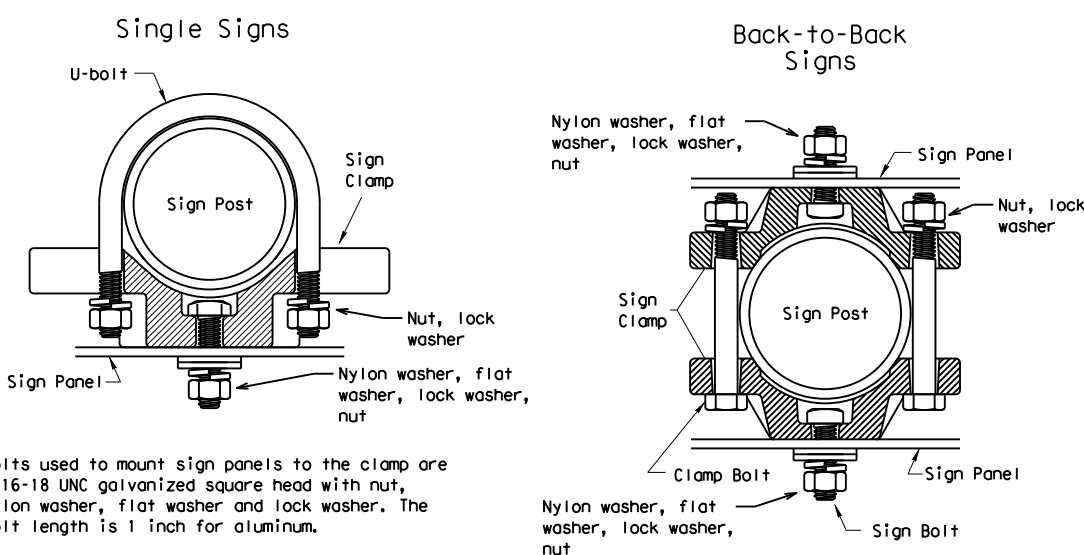
- (1) a minimum of 7 to a maximum of 7.5 feet above the edge of the travel lane or
- (2) a minimum of 7 to a maximum of 7.5 feet above the grade at the base of the support when sign is installed on the backslope.

The maximum values may be increased when directed by the Engineer.

See the Traffic Operations Division website for detailed drawings of sign clamps, Triangular Slipbase System components and Wedge Anchor System components.

The website address is:
<http://www.txdot.gov/publications/traffic.htm>

TYPICAL SIGN ATTACHMENT DETAIL



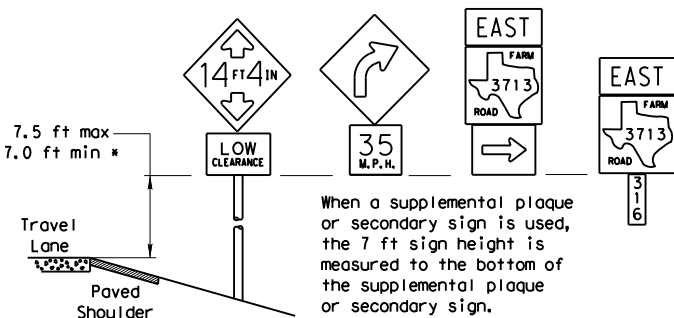
Bolts used to mount sign panels to the clamp are 5/16-18 UNC galvanized square head with nut, nylon washer, flat washer and lock washer. The bolt length is 1 inch for aluminum.

When two sign clamps are used to mount signs back-to-back, use a 5/16-18 UNC galvanized hex head per ASTM A307 with nut and helical-spring lock washer. The approximate bolt lengths for various post sizes and sign clamp types are given in the table at right. The bolt length may need to be adjusted depending upon field conditions.

Sign clamps may be either the specific size clamp or the universal clamp.

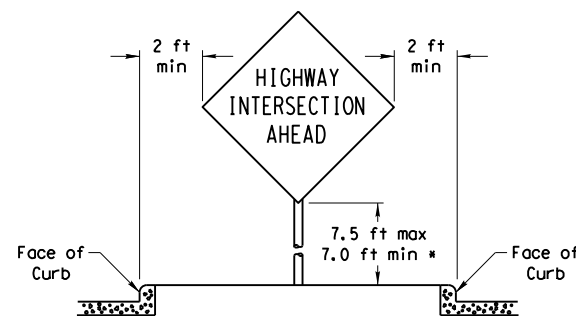
| Pipe Diameter | Approximate Bolt Length | |
|----------------|-------------------------|-----------------|
| | Specific Clamp | Universal Clamp |
| 2" nominal | 3" | 3 or 3 1/2" |
| 2 1/2" nominal | 3 or 3 1/2" | 3 1/2 or 4" |
| 3" nominal | 3 1/2 or 4" | 4 1/2" |

SIGNS WITH PLAQUES

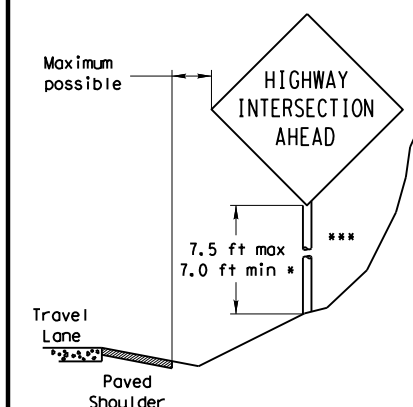


When a supplemental plaque or secondary sign is used, the 7 ft sign height is measured to the bottom of the supplemental plaque or secondary sign.

CURB & GUTTER OR RAISED ISLAND



RESTRICTED RIGHT-OF-WAY (When 6 ft min. is not possible.)



Right-of-way restrictions may be created by rocks, water, vegetation, forest, buildings, a narrow island, or other factors.

In situations where a lateral restriction prevents the minimum horizontal clearance from the edge of the travel lane, signs should be placed as far from the travel lane as practical.

*** Post may be shorter if protected by guardrail or if Engineer determines the post could not be hit due to extreme slope.



SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS GENERAL NOTES & DETAILS

SMD(GEN)-08

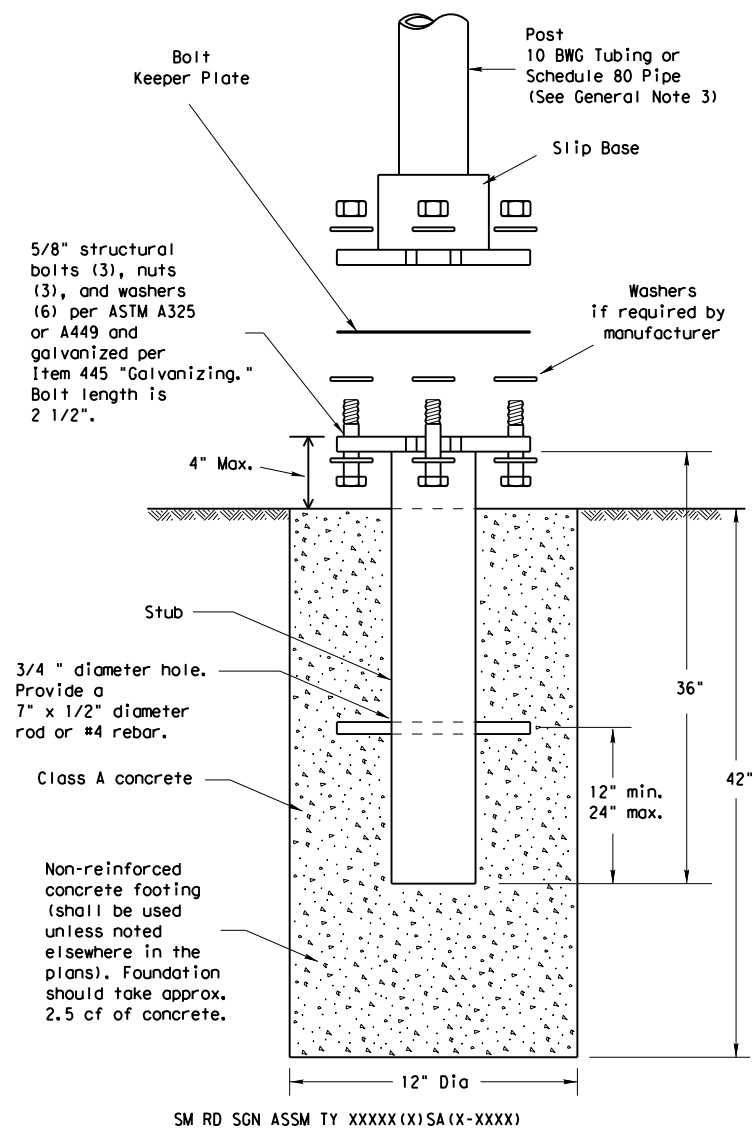
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| © TxDOT July 2002 | DN: TxDOT | CK: TxDOT | DW: TxDOT | CK: TxDOT | |
| 9-08 | REVISIONS | CONTRACT | SECTION | JOB | HIGHWAY |
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TRIANGULAR SLIPBASE INSTALLATION GENERAL REQUIREMENTS



NOTE

There are various devices approved for the Triangular Slipbase System. Please reference the Material Producer List for approved slip base systems. http://www.txdot.gov/business/producer_list.htm The devices shall be installed per manufacturers' recommendations. Installation procedures shall be provided to the Engineer by Contractor.

GENERAL NOTES:

- Slip base shall be permanently marked to indicate manufacturer. Method, design, and location of marking are subject to approval of the TxDOT Traffic Standards Engineer.
- Material used as post with this system shall conform to the following specifications:
 - 10 BWG Tubing (2.875" outside diameter)
 - 0.134" nominal wall thickness
 - Seamless or electric-resistance welded steel tubing or pipe
 - Steel shall be HSLAS Gr 55 per ASTM A1011 or ASTM A1008
 - Other steels may be used if they meet the following:
 - 55,000 PSI minimum yield strength
 - 70,000 PSI minimum tensile strength
 - 20% minimum elongation in 2"
 - Wall thickness (uncoated) shall be within the range of 0.122" to 0.138"
 - Outside diameter (uncoated) shall be within the range of 2.867" to 2.883"
 - Galvanization per ASTM A123 or ASTM A653 G210. For precoated steel tubing (ASTM A653), recoat tube outside diameter weld seam by metallizing with zinc wire per ASTM B833.
 - Schedule 80 Pipe (2.875" outside diameter)
 - 0.276" nominal wall thickness
 - Steel tubing per ASTM A500 Gr C
 - Other seamless or electric-resistance welded steel tubing or pipe with equivalent outside diameter and wall thickness may be used if they meet the following:
 - 46,000 PSI minimum yield strength
 - 62,000 PSI minimum tensile strength
 - 21% minimum elongation in 2"
 - Wall thickness (uncoated) shall be within the range of 0.248" to 0.304"
 - Outside diameter (uncoated) shall be within the range of 2.855" to 2.895"
 - Galvanization per ASTM A123
- See the Traffic Operations Division website for detailed drawings of sign clamps and Texas Universal Triangular Slipbase System components. The website address is: <http://www.txdot.gov/publications/traffic.htm>
- Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.

ASSEMBLY PROCEDURE

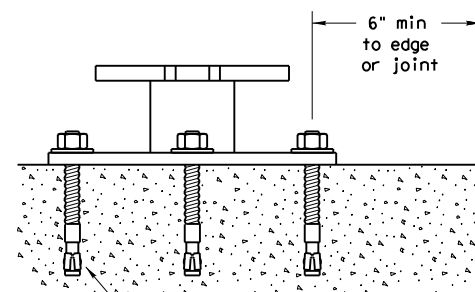
Foundation

- Prepare 12-inch diameter by 42-inch deep hole. If solid rock is encountered, the depth of the foundation may be reduced such that it is embedded a minimum of 18 inches into the solid rock.
- The Engineer may permit batches of concrete less than 2 cubic yards to be mixed with a portable, motor-driven concrete mixer. For small placements less than 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Concrete shall be Class A.
- Push the pipe end of the slip base stub into the center of the concrete. Rotate the stub back and forth while pushing it down into the concrete to assure good contact between the concrete and stub. Continue to work the stub into the concrete until it is between 2 to 4 inches above the ground.
- Plumb the stub. Allow a minimum of 4 days to set, unless otherwise directed by the Engineer.
- The triangular slipbase system is multidirectional and is designed to release when struck from any direction.

Support

- Cut support so that the bottom of the sign will be 7 to 7.5 feet above the edge of the travelway (i.e., edge of the closest lane) when slip plate is below the edge of pavement or 7 to 7.5 feet above slip plate when the slip plate is above the edge of the travelway. The cut shall be plumb and straight.
- Attach sign to support using connections shown. When multiple signs are installed on the same support, ensure the minimum clearance between each sign is maintained. See SMD(SLIP-2) for clearances based on sign types.

CONCRETE ANCHOR



Concrete anchor consists of 5/8" diameter stud bolt with UNC series bolt threads on the upper end. Heavy hex nut per ASTM A563, and hardened washer per ASTM F436. The stud bolt shall have a minimum yield and ultimate tensile strength of 50 and 75 KSI, respectively. Nuts, bolts and washers shall be galvanized per Item 445, "Galvanizing." Adhesive type anchors shall have stud bolts installed with Type III epoxy per DMS-6100, "Epoxyes and Adhesives." Adhesive anchors may be loaded after adequate epoxy cure time per the manufacturer's recommendations. Top of bolt shall extend at least flush with top of the nut when installed. The anchor, when installed in 4000 psi normal-weight concrete with a 5 1/2" minimum embedment, shall have a minimum allowable tension and shear of 3900 and 3100 psi, respectively.

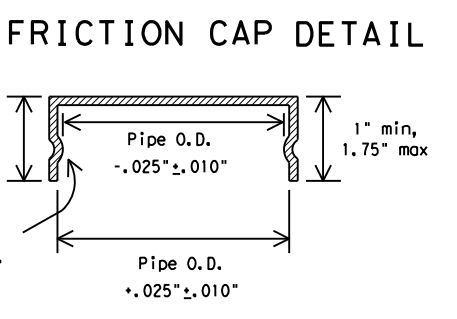
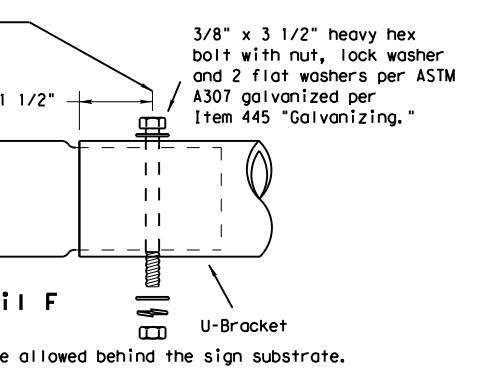
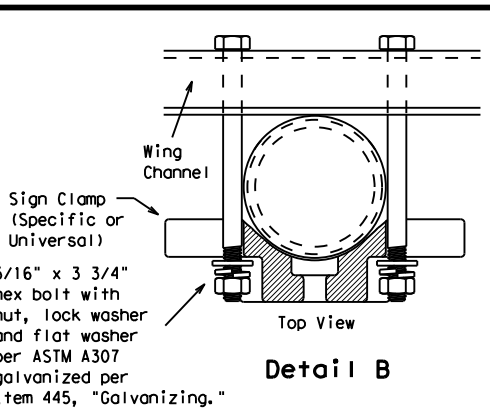
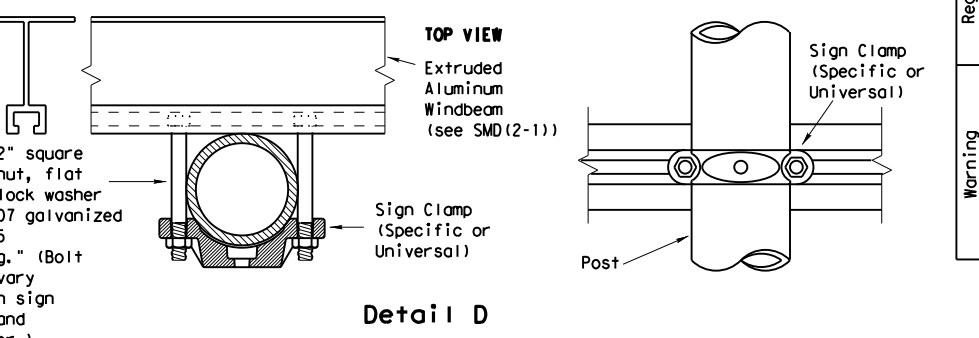
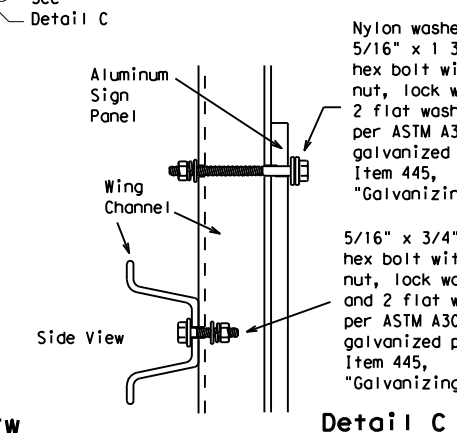
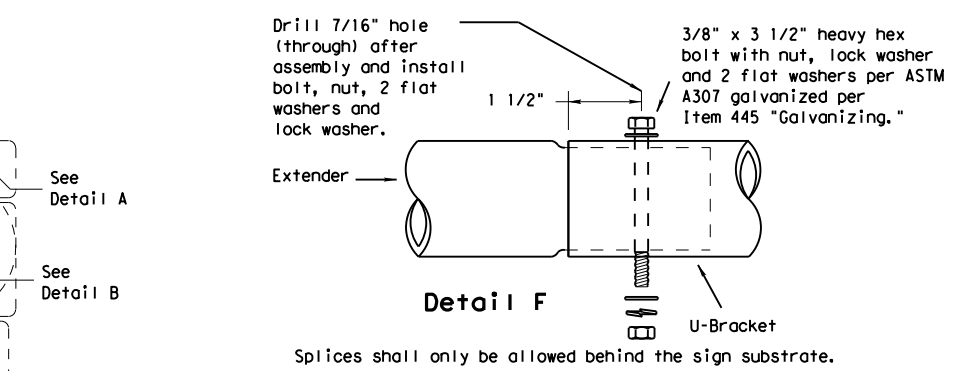
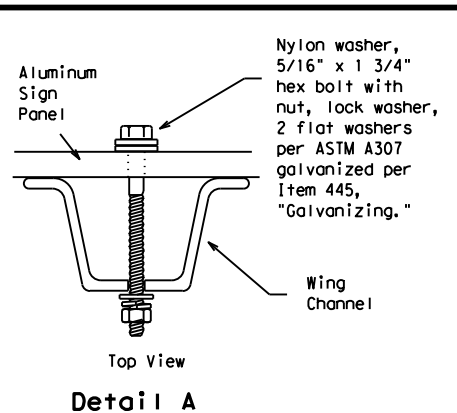
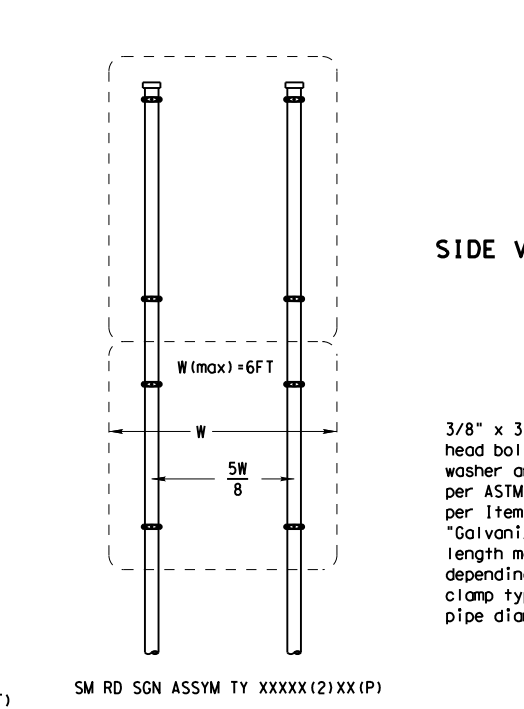
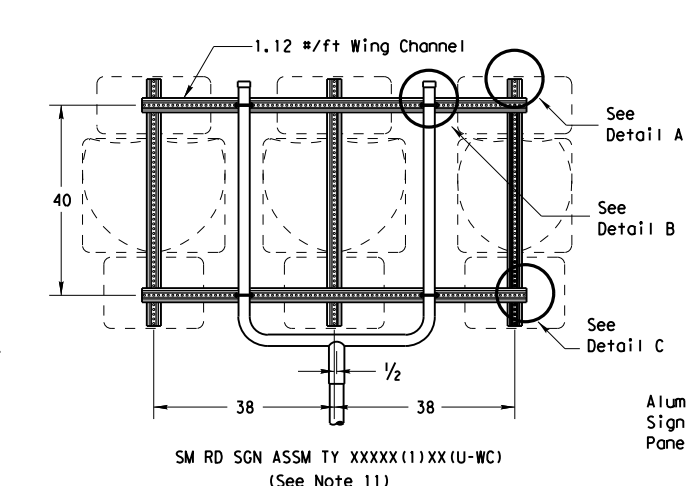
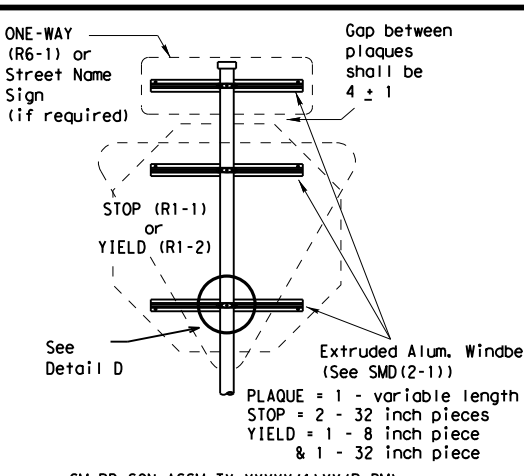
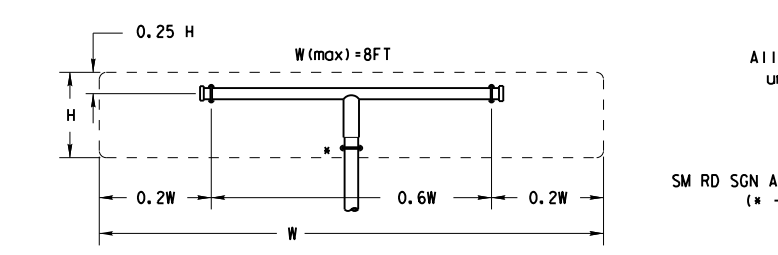
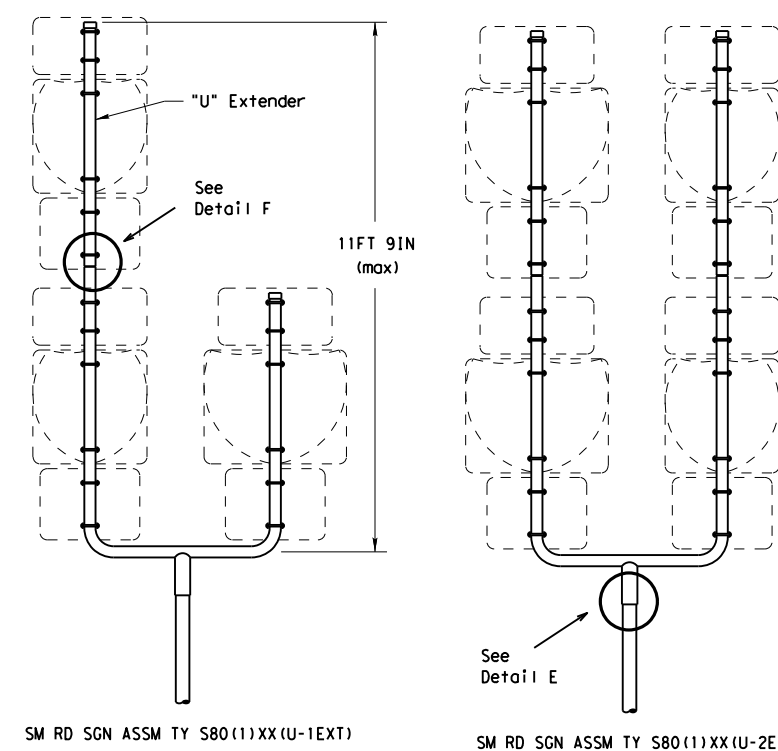
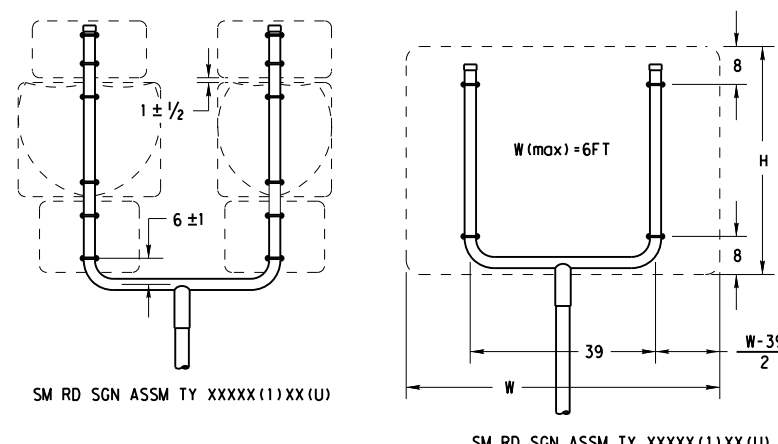
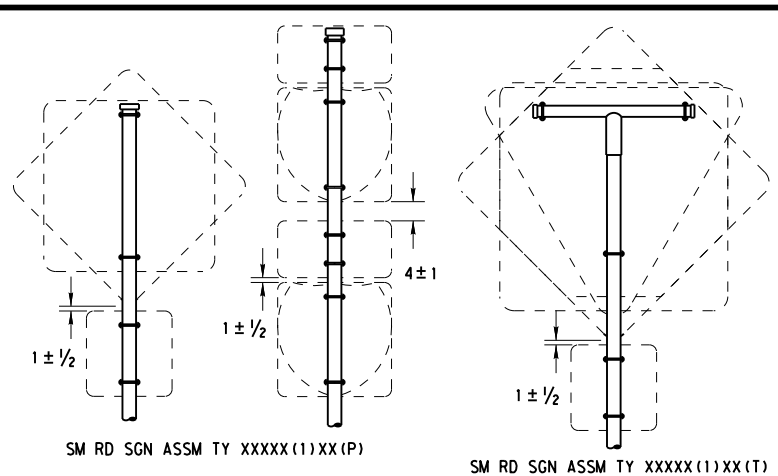
 Texas Department of Transportation
Traffic Operations Division

SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM SMD(SLIP-1)-08

| | | | | | |
|-------------------|-----------|-----------|------------|-----------|-----------|
| © TxDOT July 2002 | | DN: TxDOT | CK: TxDOT | DW: TxDOT | CR: TxDOT |
| 9-08 | REVISIONS | CONT | SECT | JOB | HIGHWAY |
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| | | DIST | COUNTY | SHEET NO. | |
| | | AUS | WILLIAMSON | 134 | |

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All dimensions are in english unless detailed otherwise.

SM RD SGN ASSM TY XXXX(1)XX(T) (* - See Note 12)

GENERAL NOTES:

- | SIGN SUPPORT | # OF POSTS | MAX. SIGN AREA |
|--------------|------------|----------------|
| 10 BWG | 1 | 16 SF |
| 10 BWG | 2 | 32 SF |
| Sch 80 | 1 | 32 SF |
| Sch 80 | 2 | 64 SF |
- The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.
- Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.
- Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.
- Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.
- For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of greater height.
- When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly" connected to each other except through the sign panel. This will allow each support to act independently when impacted by an errant vehicle.
- Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.
- Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing."
- Additional route markers may be added vertically, provided the total sign area does not exceed the maximum allowable amount per Note 1.
- Additional sign clamp required on the "T-bracket" post for 24 inch height signs. Place the clamp 3 inches above bottom of sign when possible.
- Post open ends shall be fitted with Friction Caps.
- Sign blanks shall be the sizes and shapes shown on the plans.

| REQUIRED SUPPORT | |
|--|---------------------|
| SIGN DESCRIPTION | SUPPORT |
| 48-inch STOP sign (R1-1) | TY 10BWG(1)XX(T) |
| | TY 10BWG(1)XX(P-BM) |
| 60-inch YIELD sign (R1-2) | TY 10BWG(1)XX(T) |
| | TY 10BWG(1)XX(P-BM) |
| 48x16-inch ONE-WAY sign (R6-1) | TY 10BWG(1)XX(T) |
| 48x16-inch ONE-WAY sign (R6-1) | TY 10BWG(1)XX(P-BM) |
| 36x48, 48x36, and 48x48-inch signs | TY 10BWG(1)XX(T) |
| 48x60-inch signs | TY S80(1)XX(T) |
| 48x48-inch signs (diamond or square) | TY 10BWG(1)XX(T) |
| 48x60-inch signs | TY S80(1)XX(T) |
| 48-inch Advance School X-ing sign (S1-1) | TY 10BWG(1)XX(T) |
| 48-inch School X-ing sign (S2-1) | TY 10BWG(1)XX(T) |
| Large Arrow sign (W1-6 & W1-7) | TY 10BWG(1)XX(T) |

Friction caps may be manufactured from hot rolled or cold rolled steel sheets. The minimum sheet metal thickness shall be 24 gauge for all cap sizes. The rim edges shall be reasonably straight and smooth. Caps shall be sized and formed in such a manner as to produce a drive-on friction fit and have no tendency to rock when seated on the pipe. The depth shall be sufficient to give positive protection against entrance of rainwater. They shall be free of sharp creases or indentations and show no evidence of metal fracture. Caps shall have an electrodeposited coating of zinc in accordance with the requirements of ASTM B633 Class FE/ZN 8.

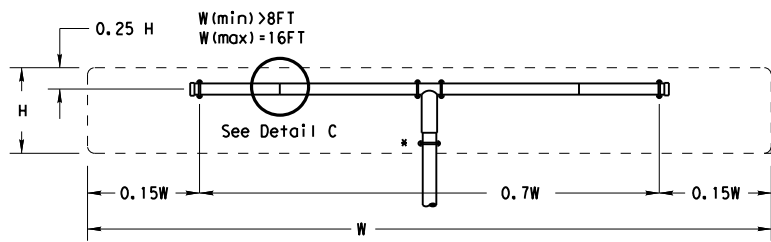


SIGN MOUNTING DETAILS
 SMALL ROADSIDE SIGNS
 TRIANGULAR SLIPBASE SYSTEM
 SMD(SLIP-2)-08

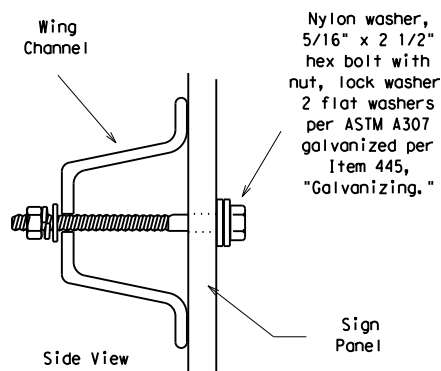
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| © TxDOT July 2002 | DN: TXDOT | CK: TXDOT | DW: TXDOT | CR: TXDOT |
| 9-08 | REVISIONS | CON: 0914 | SECT: 05 | JOB: 204, ETC. |
| | | DIST: AUS | COUNTY: WILLIAMSON | CR 118 |
| | | | | SHEET NO. 135 |

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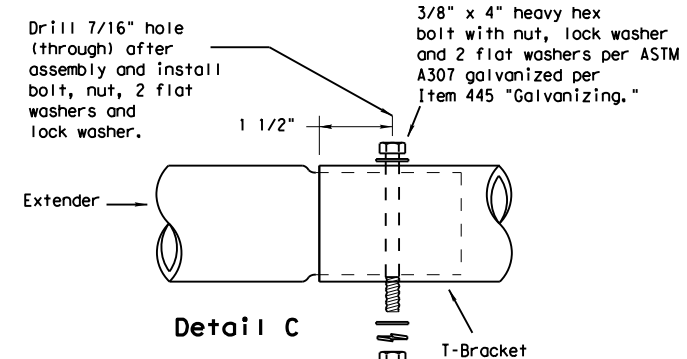
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SM RD SGN ASSM TY XXXX(1)XX(T-2EXT)
(* - See Note 12)

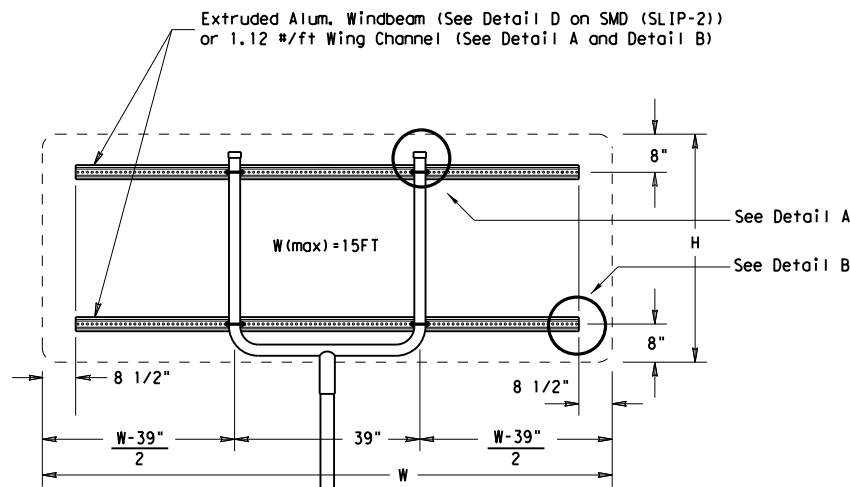


Detail B

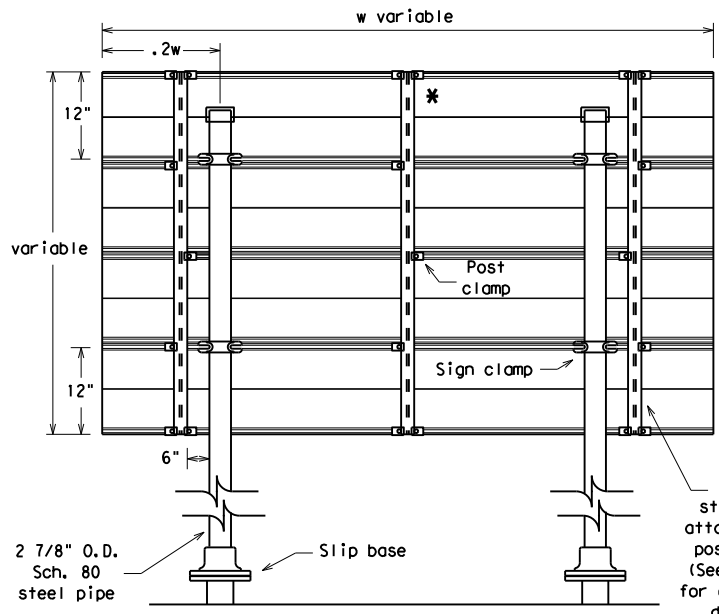


Detail C

Splices shall only be allowed behind the sign substrate.

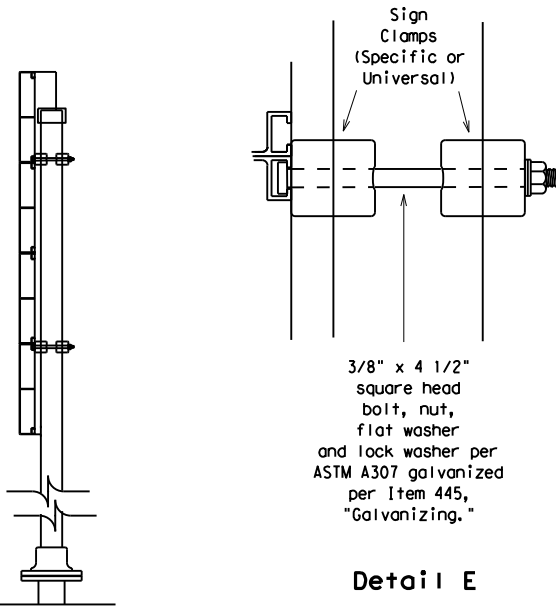


SM RD SGN ASSM TY XXXX(1)XX(U-XX)

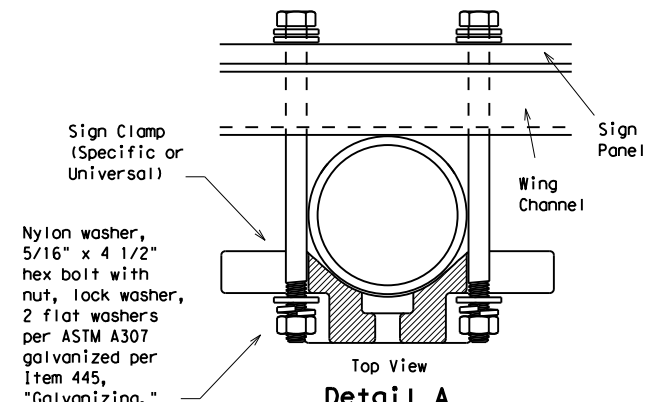


Typical Sign Mount

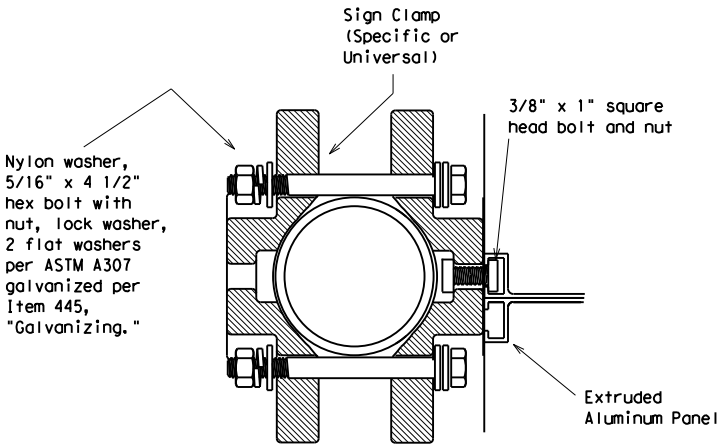
SM RD SGN ASSM TY S80(2)XX(IP-EXAL)
* Additional stiffener placed at approximate center of signs when sign width is greater than 10'.



Detail E

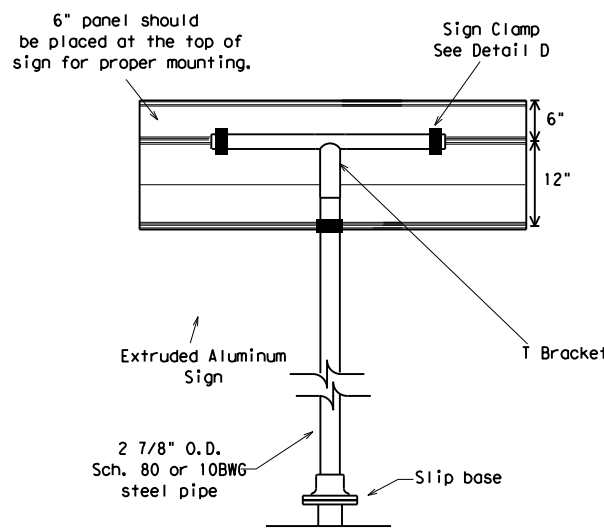


Detail A



Detail D

EXTRUDED ALUMINUM SIGN WITH T BRACKET



Extruded Aluminum Sign With T Bracket

Use Extruded Alum. Windbeam as stiffeners See SMD (2-1) for additional details
See Detail E for clamp installation

GENERAL NOTES:

- | SIGN SUPPORT | # OF POSTS | MAX. SIGN AREA |
|--------------|------------|----------------|
| 10 BWG | 1 | 16 SF |
| 10 BWG | 2 | 32 SF |
| Sch 80 | 1 | 32 SF |
| Sch 80 | 2 | 64 SF |
- The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.
- Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.
- Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.
- Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.
- For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of greater height.
- When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly" connected to each other except through the sign panel. This will allow each support to act independently when impacted by an errant vehicle.
- Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.
- Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing."
- Sign blanks shall be the sizes and shapes shown on the plans.
- Additional sign clamp required on the "T-bracket" post for 24 inch high signs. Place the clamp 3 inches above bottom of sign when possible.
- Post open ends shall be fitted with Friction Caps.

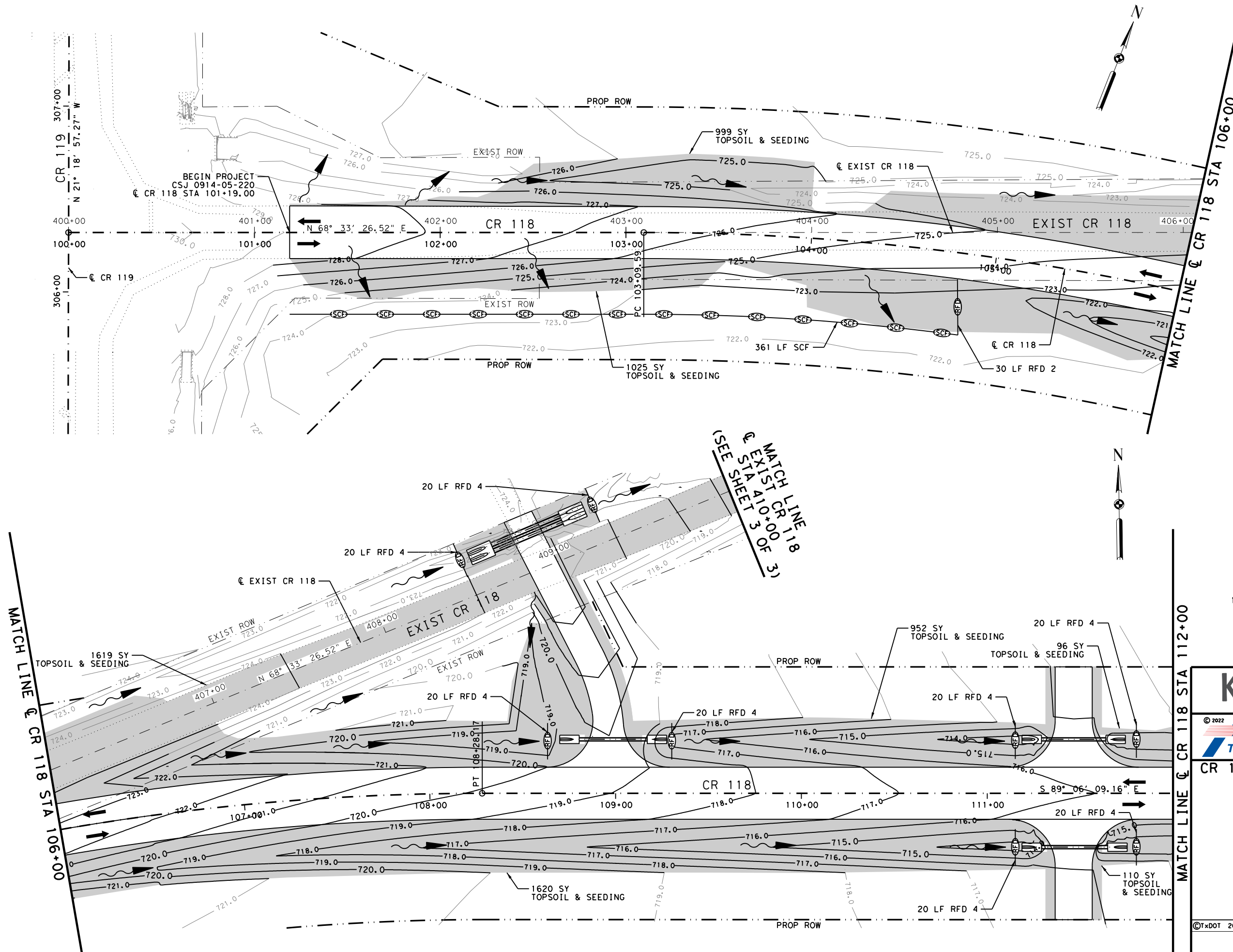
| | | REQUIRED SUPPORT | |
|------------|--|------------------|---|
| | | SIGN DESCRIPTION | SUPPORT |
| Regulatory | 48-inch STOP sign (R1-1) | | TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM) |
| | 60-inch YIELD sign (R1-2) | | TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM) |
| | 48x16-inch ONE-WAY sign (R6-1) | | TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM) |
| | 36x48, 48x36, and 48x48-inch signs | | TY 10BWG(1)XX(T) |
| | 48x60-inch signs | | TY S80(1)XX(T) |
| Warning | 48x48-inch signs (diamond or square) | | TY 10BWG(1)XX(T) |
| | 48x60-inch signs | | TY S80(1)XX(T) |
| | 48-inch Advance School X-ing sign (S1-1) | | TY 10BWG(1)XX(T) |
| | 48-inch School X-ing sign (S2-1) | | TY 10BWG(1)XX(T) |
| | Large Arrow sign (W1-6 & W1-7) | | TY 10BWG(1)XX(T) |

Texas Department of Transportation
Traffic Operations Division

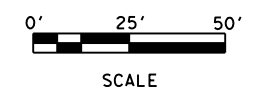
SIGN MOUNTING DETAILS
SMALL ROADSIDE SIGNS
TRIANGULAR SLIPBASE SYSTEM
SMD(SLIP-3)-08

| | | | | | |
|-------------------|-----------|-----------|------------|-----------|-----------|
| © TxDOT July 2002 | | DN: TxDOT | CK: TxDOT | DW: TxDOT | CK: TxDOT |
| 9-08 | REVISIONS | CONT | SECT | JOB | HIGHWAY |
| | | 0914 | 05 | 204, ETC. | CR 118 |
| | | DIST | COUNTY | | SHEET NO. |
| | | AUS | WILLIAMSON | | 136 |

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- LEGEND**
- TEMPORARY SEDIMENT CONTROL FENCE
 - FLOW DIRECTION
 - TEMPORARY ROCK FILTER DAM (TY 2, 3 & 4)
 - TRAFFIC DIRECTION
 - xxx.x- PROPOSED CONTOURS
 - xxx.x- EXISTING CONTOURS
 - PROPOSED SOIL RETENTION BLANKETS, TOPSOIL, & SEEDING
- NOTES:**
1. ALL SW3P MEASURES ARE TO BE PLACED WITHIN TXDOT RIGHT OF WAY AND AS SHOWN IN STANDARDS EC(1)-EC(2).
 2. SILT FENCE AND ROCK FILTER DAMS, ONCE INSTALLED, SHALL REMAIN IN PLACE THROUGHOUT ALL PHASES OF CONSTRUCTION, OR AS DIRECTED.



TJN
 11/3/2022

 TREY NEAL
 106194
 LICENSED PROFESSIONAL ENGINEER

(SEE SHEETS 2 & 3)
 MATCH LINE CR 118 STA 111+00

Kimley»Horn F-928

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 Texas Department of Transportation

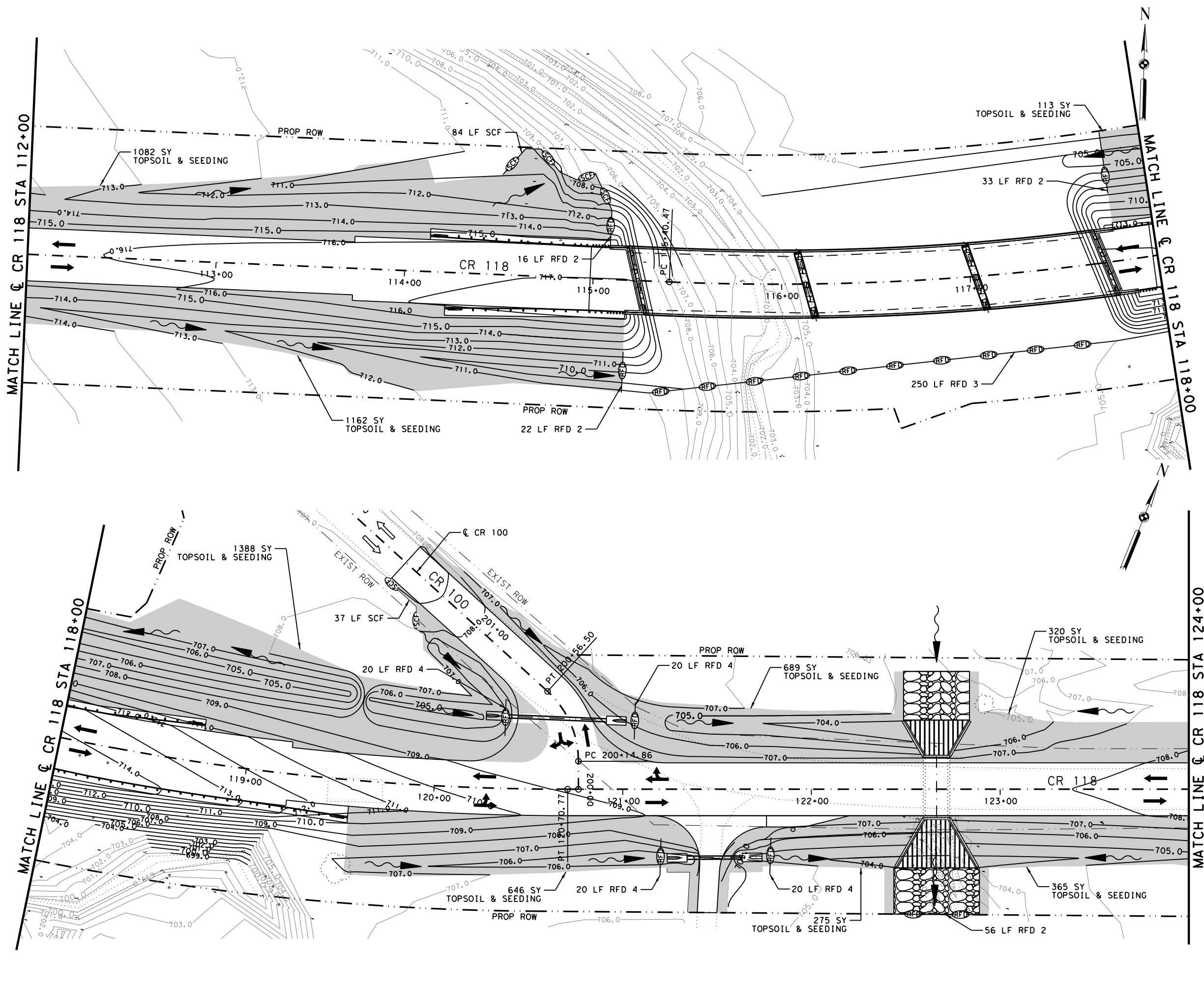
CR 118 AT COTTONWOOD CREEK

EROSION CONTROL LAYOUT

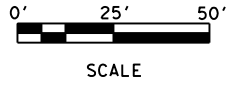
SHEET 1 OF 3

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TXDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 137 |

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- LEGEND**
- TEMPORARY SEDIMENT CONTROL FENCE
 - FLOW DIRECTION
 - TEMPORARY ROCK FILTER DAM (TY 2, 3 & 4)
 - TRAFFIC DIRECTION
 - xxx.x- PROPOSED CONTOURS
 - xxx.x- EXISTING CONTOURS
 - PROPOSED SOIL RETENTION BLANKETS, TOPSOIL, & SEEDING
- NOTES:**
1. ALL SW3P MEASURES ARE TO BE PLACED WITHIN TXDOT RIGHT OF WAY AND AS SHOWN IN STANDARDS EC(1)-EC(2).
 2. SILT FENCE AND ROCK FILTER DAMS, ONCE INSTALLED, SHALL REMAIN IN PLACE THROUGHOUT ALL PHASES OF CONSTRUCTION, OR AS DIRECTED.



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 11/3/2022

 TREY NEAL
 106194
 LICENSED PROFESSIONAL ENGINEER

Kimley»Horn F-928

Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK
 EROSION CONTROL LAYOUT

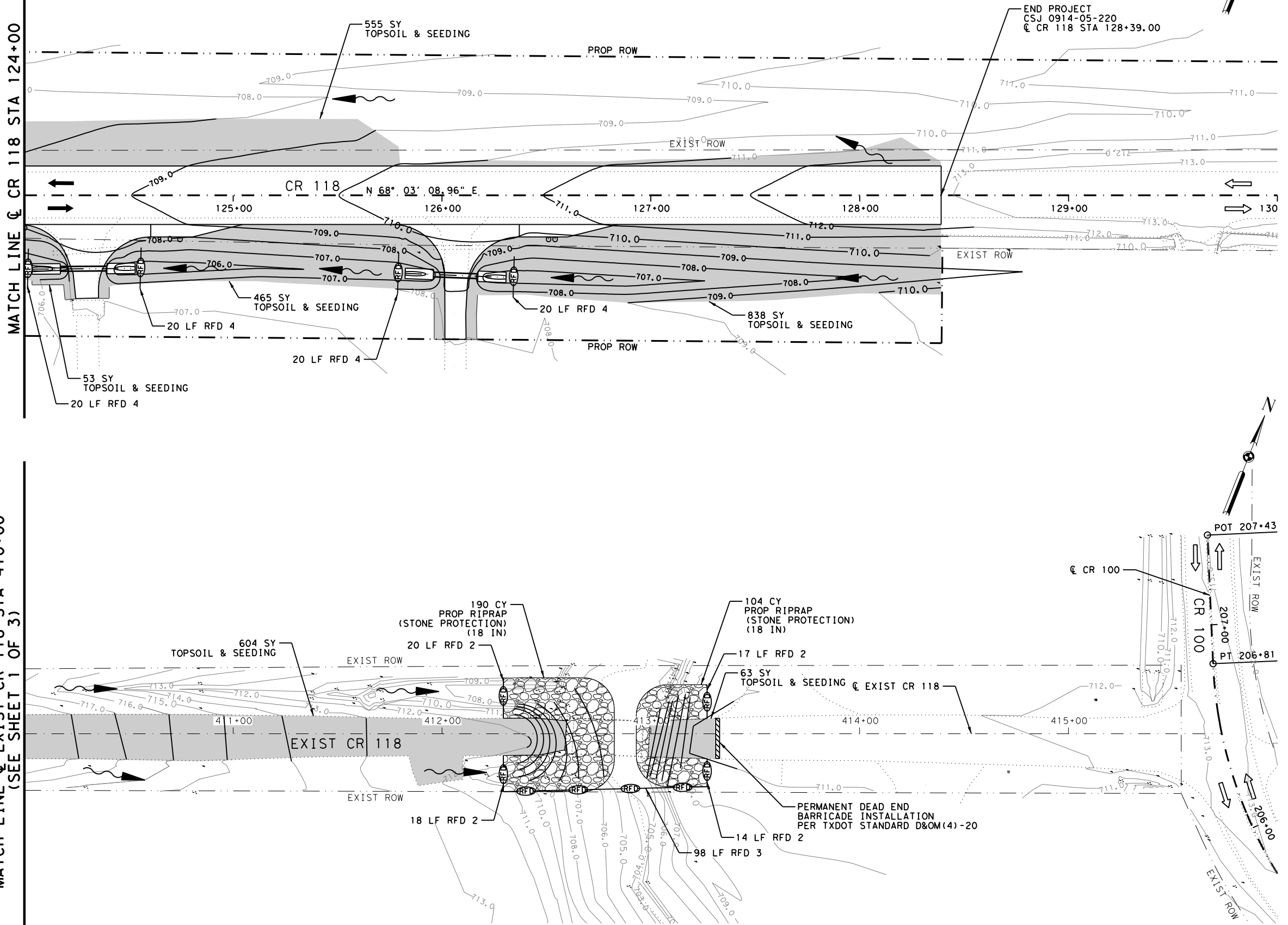
SHEET 2 OF 3

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TxDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 138 |

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MATCH LINE @ CR 118 STA 124+00

MATCH LINE @ EXIST CR 118 STA 410+00
(SEE SHEET 1 OF 3)

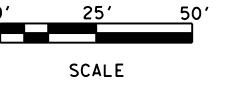


LEGEND

- TEMPORARY SEDIMENT CONTROL FENCE
- FLOW DIRECTION
- TEMPORARY ROCK FILTER DAM (TY 2, 3 & 4)
- TRAFFIC DIRECTION
- xxx.x- PROPOSED CONTOURS
- xxx.x- EXISTING CONTOURS
- PROPOSED SOIL RETENTION BLANKETS, TOPSOIL, & SEEDING

NOTES:

1. ALL SW3P MEASURES ARE TO BE PLACED WITHIN TXDOT RIGHT OF WAY AND AS SHOWN IN STANDARDS EC(1)-EC(2).
2. SILT FENCE AND ROCK FILTER DAMS, ONCE INSTALLED, SHALL REMAIN IN PLACE THROUGHOUT ALL PHASES OF CONSTRUCTION, OR AS DIRECTED.



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Texas Department of Transportation

CR 118 AT COTTONWOOD CREEK

EROSION CONTROL LAYOUT

SHEET 3 OF 3

| | | | | |
|-------------|------|------------|-----------|-----------|
| ©TXDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | | SHEET NO. |
| | AUS | WILLIAMSON | | 139 |

STORMWATER POLLUTION PREVENTION PLAN (SWP3):

This SWP3 has been developed in accordance with the TPDES Construction General Permit TXR150000 (CGP). The Texas Department of Transportation (TxDOT) ensures that project specifications include adequate best management practices (BMPs) for this project.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept in the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans and the projects environmental permits, issues, and commitments (EPICs). A copy of the CGP is included in Attachment 2.12 of the SWP3 binder.

1.0 SITE/PROJECT DESCRIPTION

1.1 PROJECT CONTROL SECTION JOB (CSJ):

0914-05-204, ETC.

1.2 PROJECT LIMITS:

From: CR 118 AT COTTONWOOD CREEK

To:

1.3 PROJECT COORDINATES:

BEGIN: (Lat) N 30.5836976, (Long) W 97.5692092

END: (Lat) N 30.581599, (Long) W 97.562144

1.4 TOTAL PROJECT AREA (Acres): 11.68

1.5 TOTAL AREA TO BE DISTURBED (Acres): 6.15

1.6 NATURE OF CONSTRUCTION ACTIVITY:

BRIDGE REPLACEMENT AND NEW ROAD

1.7 MAJOR SOIL TYPES:

| Soil Type | Description |
|---|-----------------------------|
| AUSTIN SILTY CLAY 1 TO 3% SLOPES | 7.4% |
| BRANYON CLAY 1 TO 3% SLOPES | 2.3% |
| BRANYON-KRUM COMPLEX 1 TO 3% SLOPES | 15.1% |
| HOUSTON BLACK CLAY 0 TO 1% SLOPES | 16.6% |
| HOUSTON BLACK CLAY 1 TO 3% SLOPES | 41.6% |
| TINN CLAY 0 TO 1% SLOPES | 16.9% FREQUENTLY FLOODED |

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

- PSLs determined during preconstruction meeting
- PSLs determined during construction
- No PSLs planned for construction

| Type | Sheet #s |
|------|----------|
| | |
| | |
| | |
| | |
| | |
| | |

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.5.)

- Mobilization
- Install sediment and erosion controls
- Blade existing topsoil into windrows, prep ROW, clear and grub
- Remove existing pavement
- Grading operations, excavation, and embankment
- Excavate and prepare subgrade for proposed pavement widening
- Remove existing culverts, safety end treatments (SETs)
- Remove existing metal beam guard fence (MBGF), bridge rail
- Install proposed pavement per plans
- Install culverts, culvert extensions, SETs
- Install mow strip, MBGF, bridge rail
- Place flex base
- Rework slopes, grade ditches
- Blade windrowed material back across slopes
- Revegetation of unpaved areas
- Achieve site stabilization and remove sediment and erosion control measures

- Other: _____
- Other: _____
- Other: _____

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- Sediment laden stormwater from stormwater conveyance over disturbed area
- Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- Solvents, paints, adhesives, etc. from various construction activities
- Transported soils from offsite vehicle tracking
- Construction debris and waste from various construction activities
- Contaminated water from excavation or dewatering pump-out water
- Sanitary waste from onsite restroom facilities
- Trash from various construction activities/receptacles
- Long-term stockpiles of material and waste
- Other: _____
- Other: _____
- Other: _____

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

| Tributaries | Classified Waterbody |
|-------------|----------------------|
| | |
| | |
| | |
| | |
| | |
| | |
| | |

* Add (*) for impaired waterbodies with pollutant in ().

1.12 ROLES AND RESPONSIBILITIES: TxDOT

- Development of plans and specifications
- Submit Notice of Intent (NOI) to TCEQ (≥5 acres)
- Post Construction Site Notice
- Submit NOI/CSN to local MS4
- Perform SWP3 inspections
- Maintain SWP3 records and update to reflect daily operations
- Complete and submit Notice of Termination to TCEQ
- Maintain SWP3 records for 3 years
- Other: _____
- Other: _____
- Other: _____

1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

- Day To Day Operational Control
- Submit Notice of Intent (NOI) to TCEQ (≥5 acres)
- Post Construction Site Notice
- Submit NOI/CSN to local MS4
- Maintain schedule of major construction activities
- Install, maintain and modify BMPs
- Complete and submit Notice of Termination to TCEQ
- Maintain SWP3 records for 3 years
- Other: _____
- Other: _____
- Other: _____

1.14 LOCAL MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) OPERATOR COORDINATION:

| MS4 Entity |
|------------|
| |
| |
| |
| |
| |
| |

STORMWATER POLLUTION PREVENTION PLAN (SWP3)



| | | | | |
|-------------------|---------------|------------|-------------|-----------|
| FED. RD. DIV. NO. | PROJECT NO. | | | SHEET NO. |
| 6 | BR 2020 (733) | | | 140 |
| STATE | STATE DIST. | COUNTY | | |
| TEXAS | AUS | WILLIAMSON | | |
| CONT. | SECT. | JOB | HIGHWAY NO. | |
| 0914 | 05 | 204 | CR 118 | |

STORMWATER POLLUTION PREVENTION PLAN (SWP3):

2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:

T / P

- Protection of Existing Vegetation
- Vegetated Buffer Zones
- Soil Retention Blankets
- Geotextiles
- Mulching/ Hydromulching
- Soil Surface Treatments
- Temporary Seeding
- Permanent Planting, Sodding or Seeding
- Biodegradable Erosion Control Logs
- Rock Filter Dams/ Rock Check Dams
- Vertical Tracking
- Interceptor Swale
- Riprap
- Diversion Dike
- Temporary Pipe Slope Drain
- Embankment for Erosion Control
- Paved Flumes
- Other: _____
- Other: _____
- Other: _____
- Other: _____

2.2 SEDIMENT CONTROL BMPs:

T / P

- Biodegradable Erosion Control Logs
- Dewatering Controls
- Inlet Protection
- Rock Filter Dams/ Rock Check Dams
- Sandbag Berms
- Sediment Control Fence
- Stabilized Construction Exit
- Floating Turbidity Barrier
- Vegetated Buffer Zones
- Vegetated Filter Strips
- Other: _____
- Other: _____
- Other: _____
- Other: _____

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

Sediment control BMPs requiring design capacity calculations (See SWP3 Attachment 1.3.):

T / P

- Sediment Trap
 - Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
 - 3,600 cubic feet of storage per acre drained
- Sedimentation Basin
 - Not required (<10 acres disturbed)
 - Required (>10 acres) and implemented.
 - Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
 - 3,600 cubic feet of storage per acre drained
 - Required (>10 acres), but not feasible due to:
 - Available area/Site geometry
 - Site slope/Drainage patterns
 - Site soils/Geotechnical factors
 - Public safety
 - Other: _____

2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

| Type | Stationing | |
|------|------------|----|
| | From | To |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.4 OFFSITE VEHICLE TRACKING CONTROLS:

- Excess dirt/mud on road removed daily
- Haul roads dampened for dust control
- Loaded haul trucks to be covered with tarpaulin
- Stabilized construction exit
- Other: _____
- Other: _____
- Other: _____
- Other: _____

2.5 POLLUTION PREVENTION MEASURES:

- Chemical Management
- Concrete and Materials Waste Management
- Debris and Trash Management
- Dust Control
- Sanitary Facilities
- Other: _____
- Other: _____
- Other: _____
- Other: _____

2.6 VEGETATED BUFFER ZONES:

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

| Type | Stationing | |
|------|------------|----|
| | From | To |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |
| | | |

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- Fire hydrant flushings
- Irrigation drainage
- Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- Potable water sources
- Springs
- Uncontaminated groundwater
- Water used to wash vehicles or control dust
- Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3 .

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.

STORMWATER POLLUTION PREVENTION PLAN (SWP3)

| | | | |
|-------------------|---------------|------------|-------------|
| FED. RD. DIV. NO. | PROJECT NO. | | SHEET NO. |
| 6 | BR 2020 (733) | | 140A |
| STATE | STATE DIST. | COUNTY | |
| TEXAS | AUS | WILLIAMSON | |
| CONT. | SECT. | JOB | HIGHWAY NO. |
| 0914 | 05 | 204 | CR 118 |

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I. STORMWATER POLLUTION PREVENTION-CLEAN WATER ACT SECTION 402

TPDES TXR 150000: Stormwater Discharge Permit or Construction General Permit required for projects with 1 or more acres disturbed soil. Projects with any disturbed soil must protect for erosion and sedimentation in accordance with Item 506.

List MS4 Operator(s) that may receive discharges from this project. They may need to be notified prior to construction activities.

-
- No Action Required Required Action

Action No.

- Prevent stormwater pollution by controlling erosion and sedimentation in accordance with TPDES Permit TXR 150000
- Comply with the SW3P and revise when necessary to control pollution or required by the Engineer.
- Post Construction Site Notice (CSN) with SW3P information on or near the site, accessible to the public and TCEQ, EPA or other inspectors.
- When Contractor project specific locations (PSL's) increase disturbed soil area to 5 acres or more, submit NOI to TCEQ and the Engineer.

II. WORK IN OR NEAR STREAMS, WATERBODIES AND WETLANDS CLEAN WATER ACT SECTIONS 401 AND 404

USACE Permit required for filling, dredging, excavating or other work in any water bodies, rivers, creeks, streams, wetlands or wet areas.

The Contractor must adhere to all of the terms and conditions associated with the following permit(s):

- No Permit Required
- Nationwide Permit 14 - PCN not Required (less than 1/10th acre waters or wetlands affected)
- Nationwide Permit 14 - PCN Required (1/10 to <1/2 acre, 1/3 in tidal waters)
- Individual 404 Permit Required
- Other Nationwide Permit Required: NWP# _____

Required Actions: List waters of the US permit applies to, location in project and check Best Management Practices planned to control erosion, sedimentation and post-project TSS.

- COTTONWOOD CREEK - NWP #14
-
-
-

The elevation of the ordinary high water marks of any areas requiring work to be performed in the waters of the US requiring the use of a nationwide permit can be found on the Bridge Layouts.

Best Management Practices:

| Erosion | Sedimentation | Post-Construction TSS |
|--|--|--|
| <input checked="" type="checkbox"/> Temporary Vegetation | <input checked="" type="checkbox"/> Silt Fence | <input type="checkbox"/> Vegetative Filter Strips |
| <input type="checkbox"/> Blankets/Matting | <input checked="" type="checkbox"/> Rock Berm | <input type="checkbox"/> Retention/Irrigation Systems |
| <input type="checkbox"/> Mulch | <input type="checkbox"/> Triangular Filter Dike | <input type="checkbox"/> Extended Detention Basin |
| <input type="checkbox"/> Sodding | <input type="checkbox"/> Sand Bag Berm | <input type="checkbox"/> Constructed Wetlands |
| <input type="checkbox"/> Interceptor Swale | <input type="checkbox"/> Straw Bale Dike | <input type="checkbox"/> Wet Basin |
| <input type="checkbox"/> Diversion Dike | <input type="checkbox"/> Brush Berms | <input type="checkbox"/> Erosion Control Compost |
| <input type="checkbox"/> Erosion Control Compost | <input type="checkbox"/> Erosion Control Compost | <input type="checkbox"/> Mulch Filter Berm and Socks |
| <input type="checkbox"/> Mulch Filter Berm and Socks | <input type="checkbox"/> Mulch Filter Berm and Socks | <input type="checkbox"/> Compost Filter Berm and Socks |
| <input type="checkbox"/> Compost Filter Berm and Socks | <input type="checkbox"/> Compost Filter Berm and Socks | <input checked="" type="checkbox"/> Vegetation Lined Ditches |
| | <input type="checkbox"/> Stone Outlet Sediment Traps | <input type="checkbox"/> Sand Filter Systems |
| | <input type="checkbox"/> Sediment Basins | <input type="checkbox"/> Grassy Swales |

III. CULTURAL RESOURCES

Refer to TxDOT Standard Specifications in the event historical issues or archeological artifacts are found during construction. Upon discovery of archeological artifacts (bones, burnt rock, flint, pottery, etc.) cease work in the immediate area and contact the Engineer immediately.

- No Action Required Required Action

Action No.

-
-
-
-

IV. VEGETATION RESOURCES

Preserve native vegetation to the extent practical. Contractor must adhere to Construction Specification Requirements Specs 162, 164, 192, 193, 506, 730, 751, 752 in order to comply with requirements for invasive species, beneficial landscaping, and tree/brush removal commitments.

- No Action Required Required Action

Action No.

- Comply with Executive Order 13112 on Invasive Species if and when applicable.
- See the special provisions for vegetation in Item 7 of the general notes.
- See the special provisions for water quality in Item 7 of the general notes.
-

V. FEDERAL LISTED, PROPOSED THREATENED, ENDANGERED SPECIES, CRITICAL HABITAT, STATE LISTED SPECIES, CANDIDATE SPECIES AND MIGRATORY BIRDS.

- No Action Required Required Action

Action No.

- See the special provisions for terrestrial amphibians and reptiles in Item 7 of the general notes.
- See the special provisions for aquatic amphibians and reptiles in Item 7 of the general notes.
-
-

If any of the listed species are observed, cease work in the immediate area, do not disturb species or habitat and contact the Engineer immediately. The work may not remove active nests from bridges and other structures during nesting season of the birds associated with the nests. If caves or sinkholes are discovered, cease work in the immediate area, and contact the Engineer immediately.

LIST OF ABBREVIATIONS

| | |
|---|---|
| BMP: Best Management Practice | SPCC: Spill Prevention Control and Countermeasure |
| CGP: Construction General Permit | SW3P: Storm Water Pollution Prevention Plan |
| DSHS: Texas Department of State Health Services | PCN: Pre-Construction Notification |
| FHWA: Federal Highway Administration | PSL: Project Specific Location |
| MOA: Memorandum of Agreement | TCEQ: Texas Commission on Environmental Quality |
| MOU: Memorandum of Understanding | TPDES: Texas Pollutant Discharge Elimination System |
| MS4: Municipal Separate Stormwater Sewer System | TPWD: Texas Parks and Wildlife Department |
| MBTA: Migratory Bird Treaty Act | TxDOT: Texas Department of Transportation |
| NOT: Notice of Termination | T&E: Threatened and Endangered Species |
| NWP: Nationwide Permit | USACE: U.S. Army Corps of Engineers |
| NOI: Notice of Intent | USFWS: U.S. Fish and Wildlife Service |

VI. HAZARDOUS MATERIALS OR CONTAMINATION ISSUES

General (applies to all projects):

Comply with the Hazard Communication Act (the Act) for personnel who will be working with hazardous materials by conducting safety meetings prior to beginning construction and making workers aware of potential hazards in the workplace. Ensure that all workers are provided with personal protective equipment appropriate for any hazardous materials used. Obtain and keep on-site Material Safety Data Sheets (MSDS) for all hazardous products used on the project, which may include, but are not limited to the following categories: Paints, acids, solvents, asphalt products, chemical additives, fuels and concrete curing compounds or additives. Provide protected storage, off bare ground and covered, for products which may be hazardous. Maintain product labelling as required by the Act.

Maintain an adequate supply of on-site spill response materials, as indicated in the MSDS. In the event of a spill, take actions to mitigate the spill as indicated in the MSDS, in accordance with safe work practices, and contact the District Spill Coordinator immediately. The Contractor shall be responsible for the proper containment and cleanup of all product spills.

Contact the Engineer if any of the following are detected:

- * Dead or distressed vegetation (not identified as normal)
- * Trash piles, drums, canister, barrels, etc.
- * Undesirable smells or odors
- * Evidence of leaching or seepage of substances

Does the project involve any bridge class structure rehabilitation or replacements (bridge class structures not including box culverts)?

- Yes No

If "No", then no further action is required.

If "Yes", then TxDOT is responsible for completing asbestos assessment/inspection.

Are the results of the asbestos inspection positive (is asbestos present)?

- Yes No

If "Yes", then TxDOT must retain a DSHS licensed asbestos consultant to assist with the notification, develop abatement/mitigation procedures, and perform management activities as necessary. The notification form to DSHS must be postmarked at least 15 working days prior to scheduled demolition.

If "No", then TxDOT is still required to notify DSHS 15 working days prior to any scheduled demolition.

In either case, the Contractor is responsible for providing the date(s) for abatement activities and/or demolition with careful coordination between the Engineer and asbestos consultant in order to minimize construction delays and subsequent claims.

Any other evidence indicating possible hazardous materials or contamination discovered on site. Hazardous Materials or Contamination Issues Specific to this Project:

- No Action Required Required Action

Action No.

-
-
-

VII. OTHER ENVIRONMENTAL ISSUES

(includes regional issues such as Edwards Aquifer District, etc.)

- No Action Required Required Action

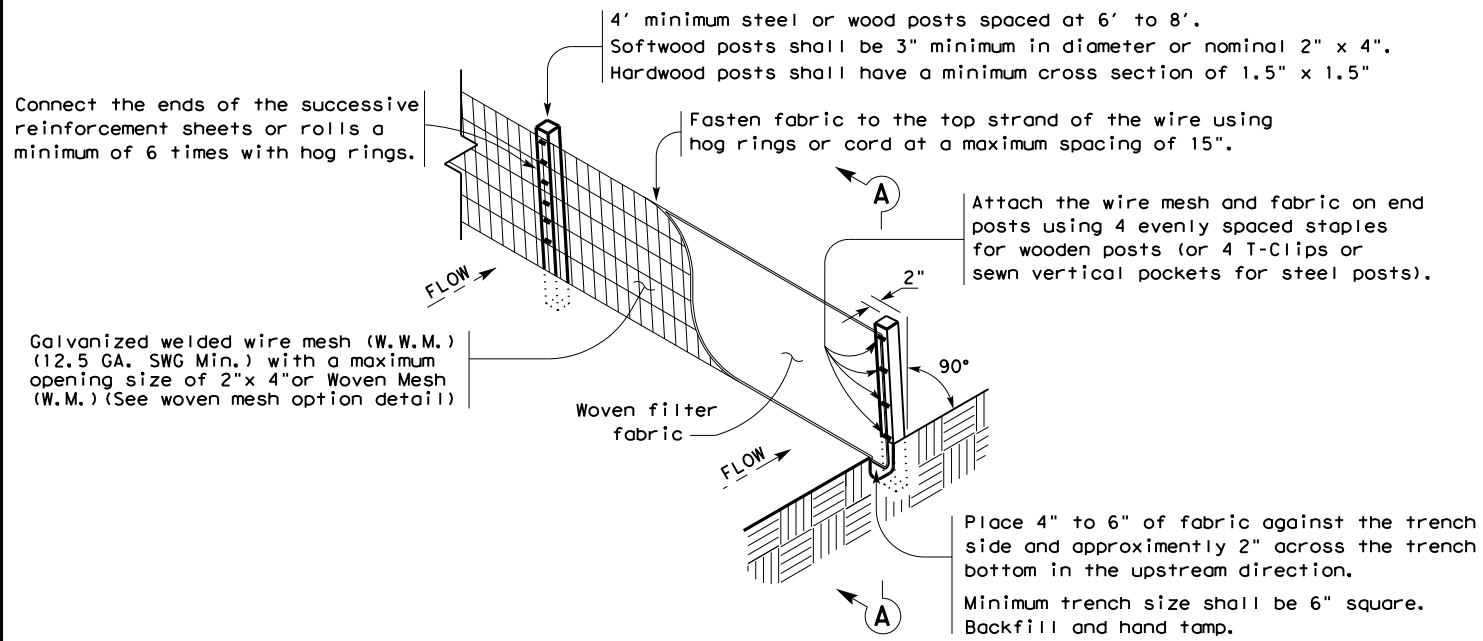
Action No.

- The project is located in a Federal Emergency Management Agency mapped floodplain. Notify the local floodplain administrator as necessary and comply with all applicable rules and regulations regarding hydraulic design of the project.
-
-

| Texas Department of Transportation | | | | Design Division Standard | |
|---|-----------|------------|-----------|--------------------------|--|
| ENVIRONMENTAL PERMITS, ISSUES AND COMMITMENTS EPIC | | | | | |
| FILE: epic.dgn | DN: TxDOT | CK: RG | DW: VP | CK: AR | |
| ©TxDOT: February 2015 | CONT | SECT | JOB | HIGHWAY | |
| 12-12-2011 (DS) REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | |
| 05-07-14 ADDED NOTE SECTION IV. | DIST | COUNTY | | SHEET NO. | |
| 01-23-2015 SECTION I (CHANGED ITEM 1122 TO ITEM 506, ADDED GRASSY SWALES. | AUS | WILLIAMSON | | 141 | |

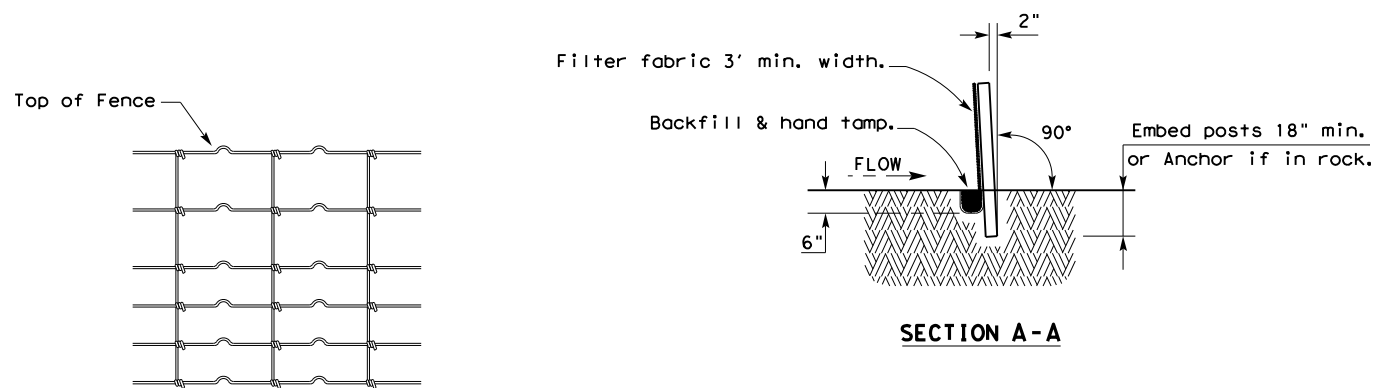
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10/18/2022
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TEMPORARY SEDIMENT CONTROL FENCE

SCF



HINGE JOINT KNOT WOVEN MESH (OPTION) DETAIL

Galvanized hinge joint knot woven mesh (12.5 GA. SWG Min.) requires a minimum of five horizontal wires spaced at a maximum of 12 inches apart and all vertical wires spaced at a maximum of 12 inches apart.

SEDIMENT CONTROL FENCE USAGE GUIDELINES

A sediment control fence may be constructed near the downstream perimeter of a disturbed area along a contour to intercept sediment from overland runoff. A 2 year storm frequency may be used to calculate the flow rate to be filtered.

Sediment control fence should be sized to filter a maximum flow through rate of 100 GPM/FT². Sediment control fence is not recommended to control erosion from a drainage area larger than 2 acres.

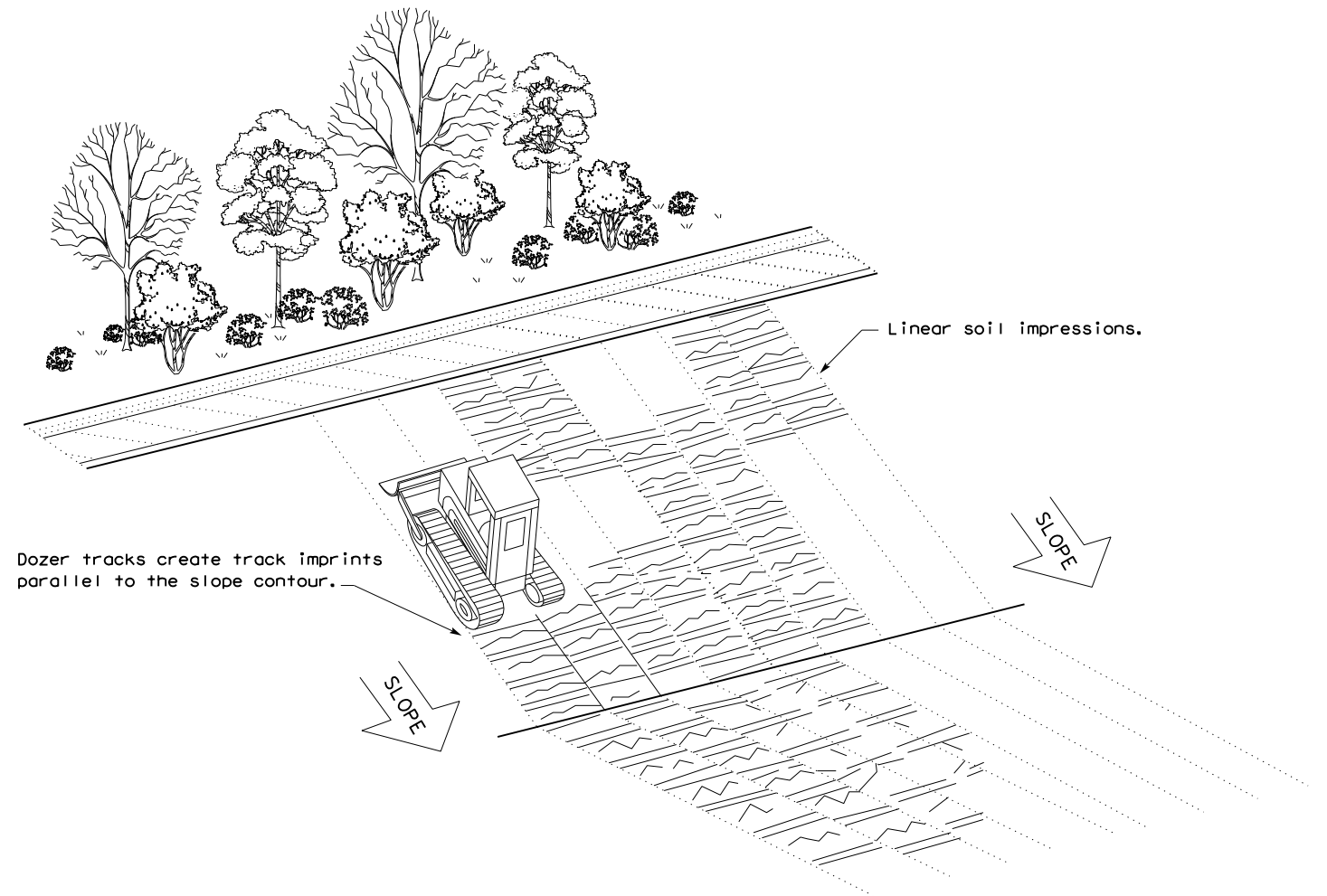
LEGEND

Sediment Control Fence

SCF

GENERAL NOTES

1. Vertical tracking is required on projects where soil distributing activities have occurred unless otherwise approved.
2. Perform vertical tracking on slopes to temporarily stabilize soil.
3. Provide equipment with a track undercarriage capable of producing linear soil impressions measuring a minimum of 12" in length by 2" to 4" in width by 1/2" to 2" in depth.
4. Do not exceed 12" between track impressions.
5. Install continuous linear track impressions where the minimum 12" length impressions are perpendicular to the slope or direction of water flow.

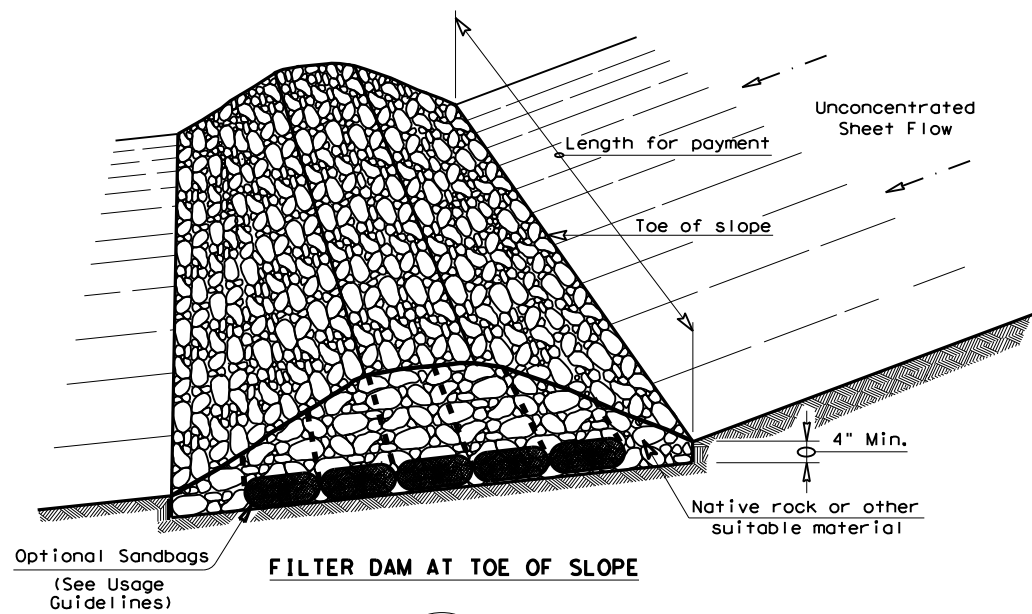


VERTICAL TRACKING

| | | | | | |
|--|-----------|------------|-----------|--------------------------|--|
| | | | | Design Division Standard | |
| TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES FENCE & VERTICAL TRACKING EC(1)-16 | | | | | |
| FILE: ec116 | DN: TxDOT | CK: KM | DW: VP | DN/CK: LS | |
| © TxDOT: JULY 2016 | CONT | SECT | JOB | HIGHWAY | |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 | |
| | DIST | COUNTY | SHEET NO. | | |
| | AUS | WILLIAMSON | 142 | | |

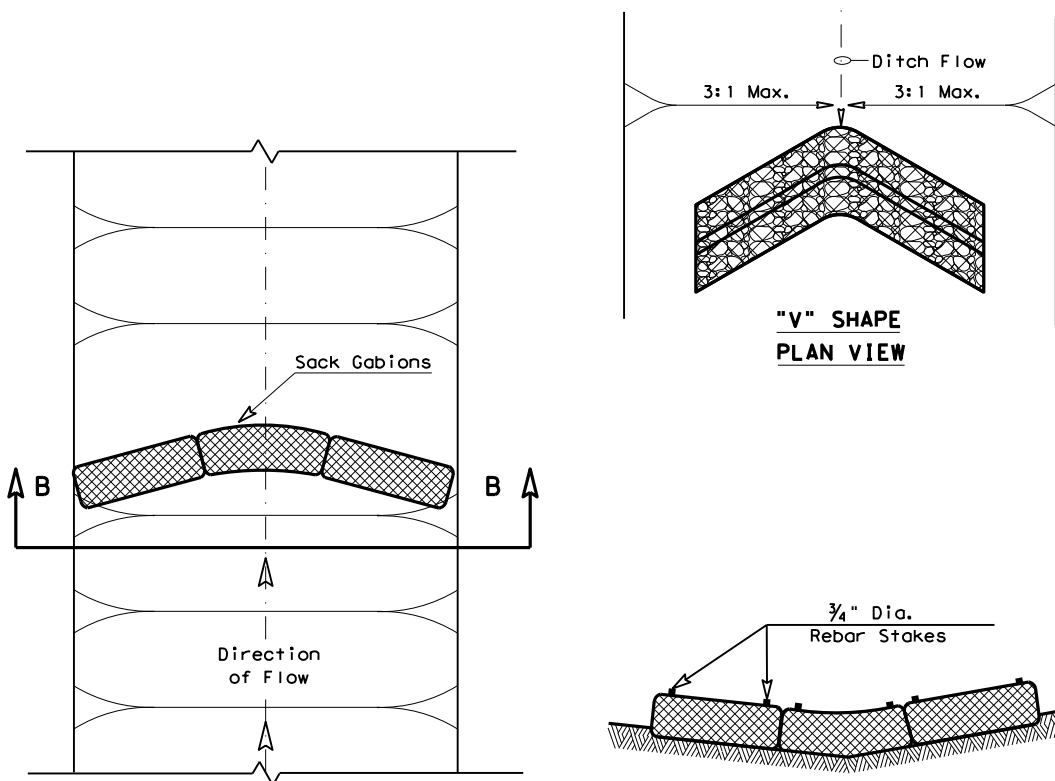
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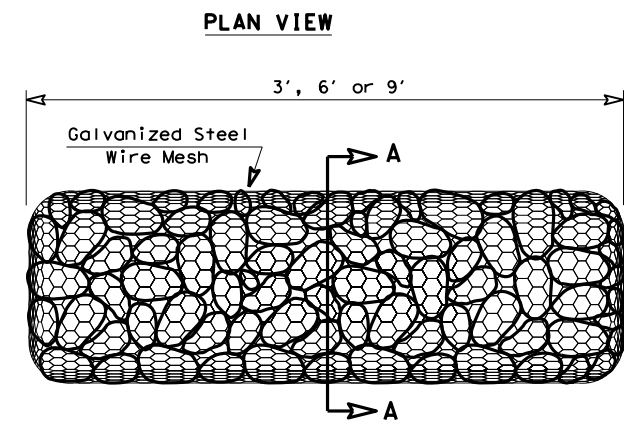


FILTER DAM AT TOE OF SLOPE

(RFD1)

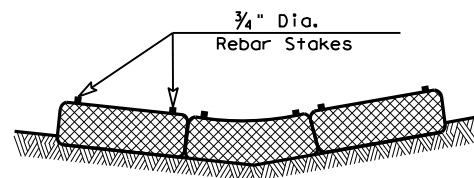


"V" SHAPE PLAN VIEW

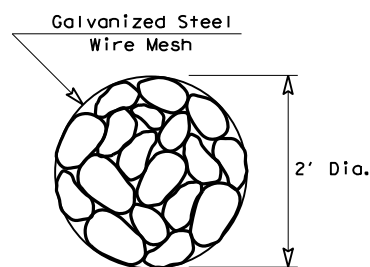


TYPE 4 (SACK GABIONS)

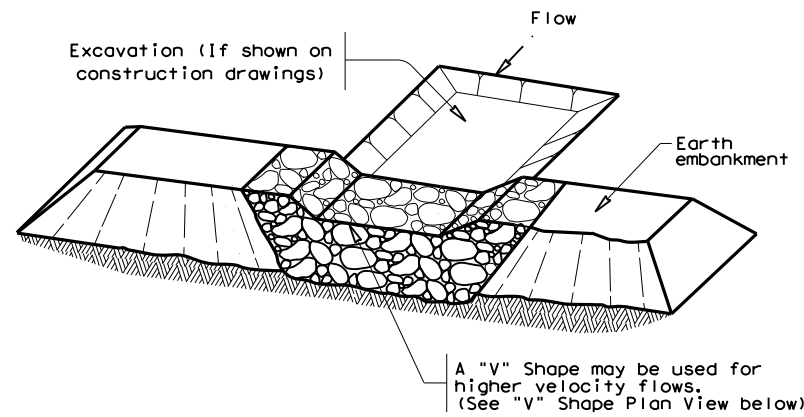
(RFD4)



SECTION B-B

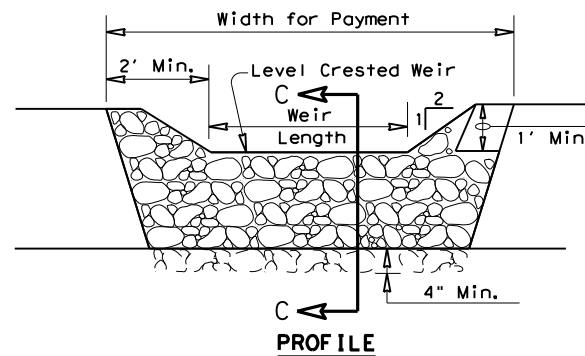


SECTION A-A

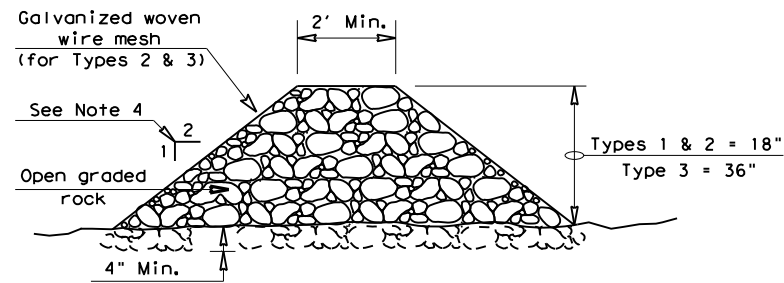


FILTER DAM AT SEDIMENT TRAP

(RFD1) OR (RFD2)



PROFILE



SECTION C-C

ROCK FILTER DAM USAGE GUIDELINES

Rock Filter Dams should be constructed downstream from disturbed areas to intercept sediment from overland runoff and/or concentrated flow. The dams should be sized to filter a maximum flow through rate of 60 GPM/FT² of cross sectional area. A 2 year storm frequency may be used to calculate the flow rate.

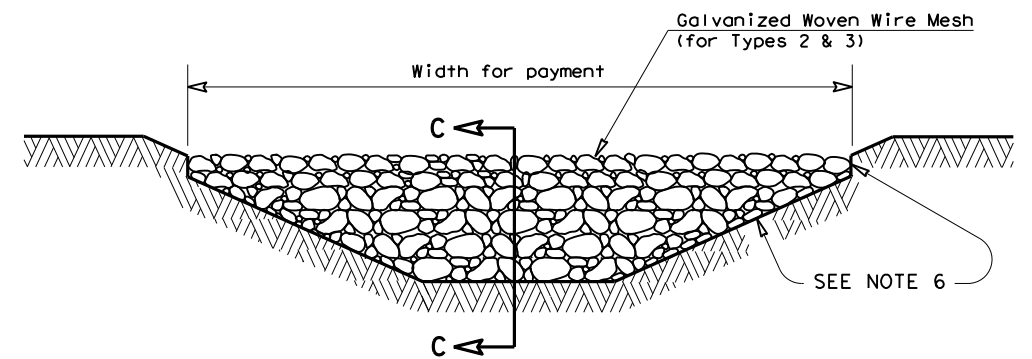
Type 1 (18" high with no wire mesh) (3" to 6" aggregate): Type 1 may be used at the toe of slopes, around inlets, in small ditches, and at dike or swale outlets. This type of dam is recommended to control erosion from a drainage area of 5 acres or less. Type 1 may not be used in concentrated high velocity flows (approximately 8 Ft/Sec or more) in which aggregate wash out may occur. Sandbags may be used at the embedded foundation (4" deep min.) for better filtering efficiency of low flows if called for on the plans or directed by the Engineer.

Type 2 (18" high with wire mesh) (3" to 6" aggregate): Type 2 may be used in ditches and at dike or swale outlets.

Type 3 (36" high with wire mesh) (4" to 8" aggregate): Type 3 may be used in stream flow and should be secured to the stream bed.

Type 4 (Sack gabions) (3" to 6" aggregate): Type 4 May be used in ditches and smaller channels to form an erosion control dam.

Type 5: Provide rock filter dams as shown on plans.



FILTER DAM AT CHANNEL SECTIONS

(RFD1) OR (RFD2) OR (RFD3)

GENERAL NOTES

1. If shown on the plans or directed by the Engineer, filter dams should be placed near the toe of slopes where erosion is anticipated, upstream and/or downstream at drainage structures, and in roadway ditches and channels to collect sediment.
2. Materials (aggregate, wire mesh, sandbags, etc.) shall be as indicated by the specification for "Rock Filter Dams for Erosion and Sedimentation Control".
3. The rock filter dam dimensions shall be as indicated on the SW3P plans.
4. Side slopes should be 2:1 or flatter. Dams within the safety zone shall have sideslopes of 6:1 or flatter.
5. Maintain a minimum of 1' between top of rock filter dam weir and top of embankment for filter dams at sediment traps.
6. Filter dams should be embedded a minimum of 4" into existing ground.
7. The sediment trap for ponding of sediment laden runoff shall be of the dimensions shown on the plans.
8. Rock filter dam types 2 & 3 shall be secured with 20 gauge galvanized woven wire mesh with 1" diameter hexagonal openings. The aggregate shall be placed on the mesh to the height & slopes specified. The mesh shall be folded at the upstream side over the aggregate and tightly secured to itself on the downstream side using wire ties or hog rings. For in stream use, the mesh should be secured or staked to the stream bed prior to aggregate placement.
9. Sack Gabions should be staked down with 3/4" dia. rebar stakes, and have a double-twisted hexagonal weave with a nominal mesh opening of 2 1/2" x 3 1/4".
10. Flow outlet should be onto a stabilized area (vegetation, rock, etc.).
11. The guidelines shown hereon are suggestions only and may be modified by the Engineer.

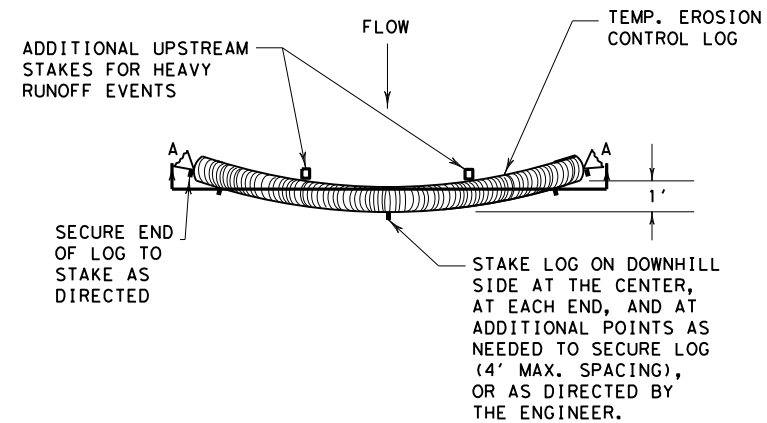
PLAN SHEET LEGEND

- Type 1 Rock Filter Dam (RFD1)
- Type 2 Rock Filter Dam (RFD2)
- Type 3 Rock Filter Dam (RFD3)
- Type 4 Rock Filter Dam (RFD4)

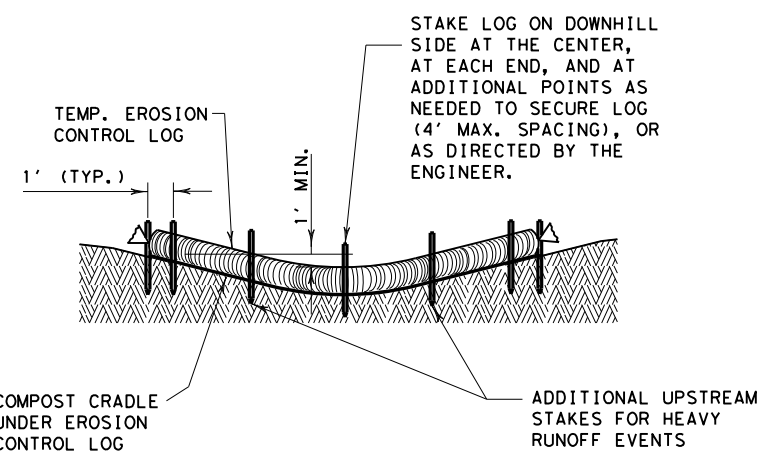
| | | | |
|---|-----------|---------------------------------|-----------|
| | | Design Division Standard | |
| TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES ROCK FILTER DAMS EC(2) - 16 | | | |
| FILE: ec216 | DN: TxDOT | CK: KM | DW: VP |
| © TxDOT: JULY 2016 | CONT | SECT | JOB |
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PLAN VIEW

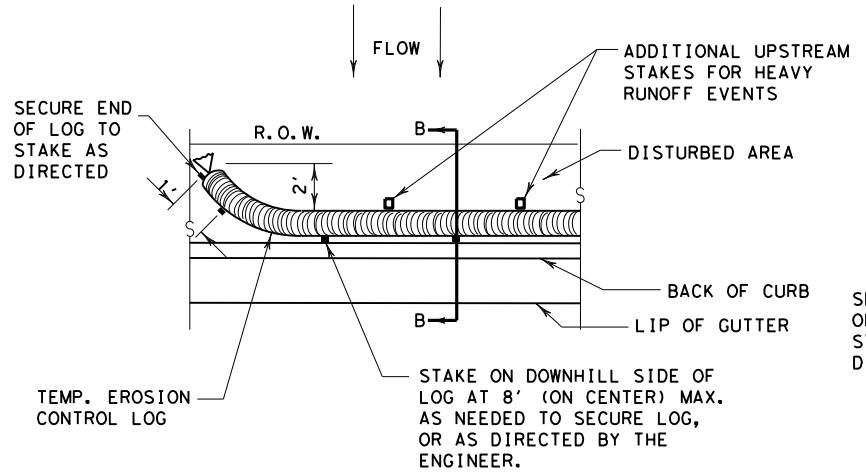


SECTION A-A
 EROSION CONTROL LOG DAM

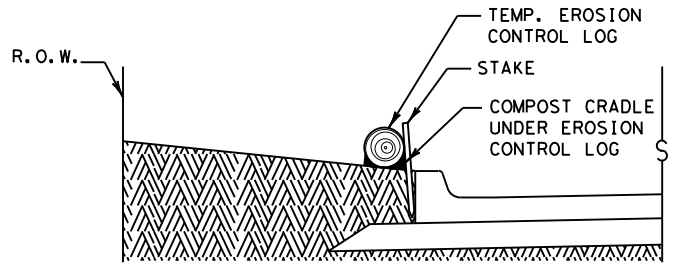
CL-D

LEGEND

- CL-D EROSION CONTROL LOG DAM
- CL-BOC EROSION CONTROL LOG AT BACK OF CURB
- CL-ROW EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY
- CL-SST EROSION CONTROL LOGS ON SLOPES STAKE AND TRENCHING ANCHORING
- CL-SSL EROSION CONTROL LOGS ON SLOPES STAKE AND LASHING ANCHORING
- CL-DI EROSION CONTROL LOG AT DROP INLET
- CL-CI EROSION CONTROL LOG AT CURB INLET
- CL-GI EROSION CONTROL LOG AT CURB & GRATE INLET



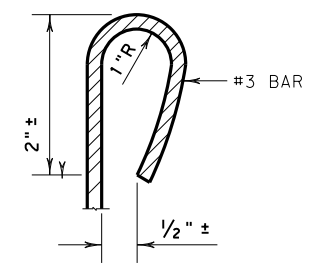
PLAN VIEW



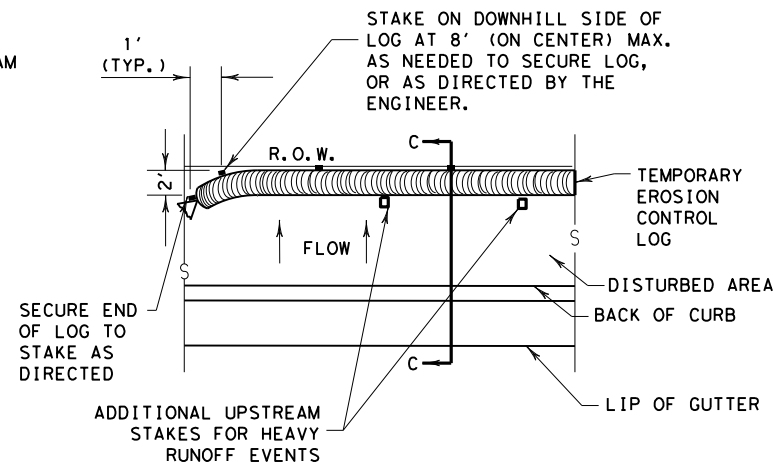
SECTION B-B

EROSION CONTROL LOG AT BACK OF CURB

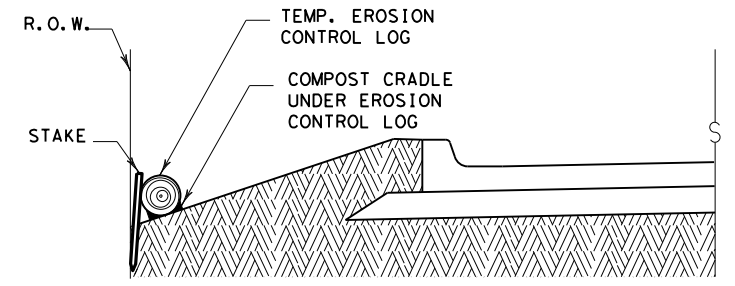
CL-BOC



REBAR STAKE DETAIL



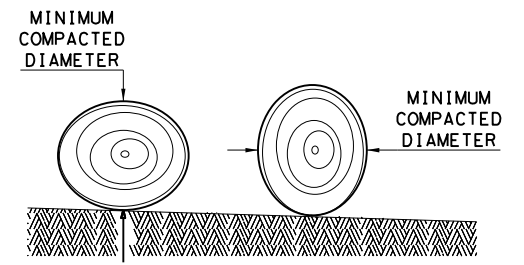
PLAN VIEW



SECTION C-C

EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY

CL-ROW



DIAMETER MEASUREMENTS OF EROSION CONTROL LOGS SPECIFIED IN PLANS

SEDIMENT BASIN & TRAP USAGE GUIDELINES

An erosion control log sediment trap may be used to filter sediment out of runoff draining from an unstabilized area.

Log Traps: The drainage area for a sediment trap should not exceed 5 acres. The trap capacity should be 1800 CF/Acre (0.5" over the drainage area).

Control logs should be placed in the following locations:

1. Within drainage ditches spaced as needed or min. 500' on center
2. Immediately preceding ditch inlets or drain inlets
3. Just before the drainage enters a water course
4. Just before the drainage leaves the right of way
5. Just before the drainage leaves the construction limits where drainage flows away from the project.

The logs should be cleaned when the sediment has accumulated to a depth of 1/2 the log diameter.

Cleaning and removal of accumulated sediment deposits is incidental and will not be paid for separately.

GENERAL NOTES:

1. EROSION CONTROL LOGS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, OR AS DIRECTED BY THE ENGINEER.
2. LENGTHS OF EROSION CONTROL LOGS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND AS REQUIRED FOR THE PURPOSE INTENDED.
3. UNLESS OTHERWISE DIRECTED, USE BIODEGRADABLE OR PHOTODEGRADABLE CONTAINMENT MESH ONLY WHERE LOG WILL REMAIN IN PLACE AS PART OF A VEGETATIVE SYSTEM. FOR TEMPORARY INSTALLATIONS, USE RECYCLABLE CONTAINMENT MESH.
4. FILL LOGS WITH SUFFICIENT FILTER MATERIAL TO ACHIEVE THE MINIMUM COMPACTED DIAMETER SPECIFIED IN THE PLANS WITHOUT EXCESSIVE DEFORMATION.
5. STAKES SHALL BE 2" X 2" WOOD OR #3 REBAR, 2'-4' LONG, EMBEDDED SUCH THAT 2" PROTRUDES ABOVE LOG, OR AS DIRECTED BY THE ENGINEER.
6. DO NOT PLACE STAKES THROUGH CONTAINMENT MESH.
7. COMPOST CRADLE MATERIAL IS INCIDENTAL & WILL NOT BE PAID FOR SEPARATELY.
8. SANDBAGS USED AS ANCHORS SHALL BE PLACED ON TOP OF LOGS & SHALL BE OF SUFFICIENT SIZE TO HOLD LOGS IN PLACE.
9. TURN THE ENDS OF EACH ROW OF LOGS UPSLOPE TO PREVENT RUNOFF FROM FLOWING AROUND THE LOG.
10. FOR HEAVY RUNOFF EVENTS, ADDITIONAL UPSTREAM STAKES MAY BE NECESSARY TO KEEP LOG FROM FOLDING IN ON ITSELF.

SHEET 1 OF 3

Texas Department of Transportation
 Design Division Standard

TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

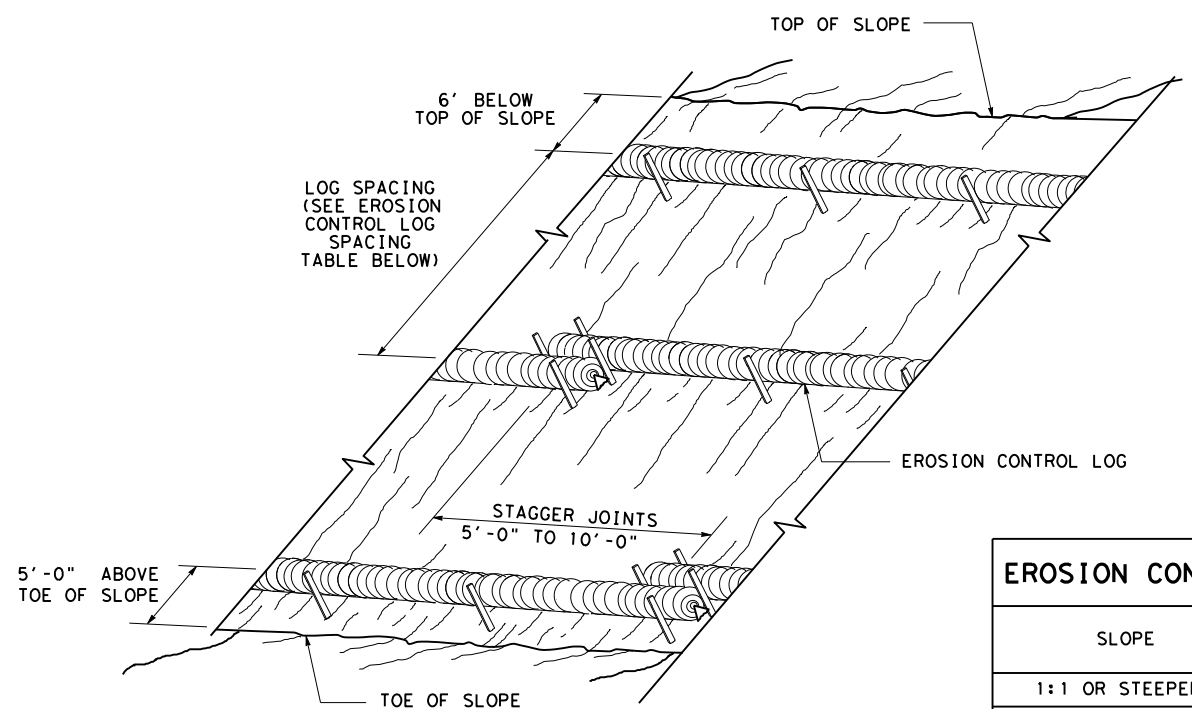
EROSION CONTROL LOG

EC (9) - 16

| | | | | |
|--------------------|-----------|------------|-----------|---------|
| FILE: ec916 | DN: TxDOT | CK: KM | DW: LS/PT | CK: LS |
| © TxDOT: JULY 2016 | CONT | SECT | JOB | HIGHWAY |
| REVISIONS | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST | COUNTY | SHEET NO. | |
| | AUS | WILLIAMSON | 144 | |

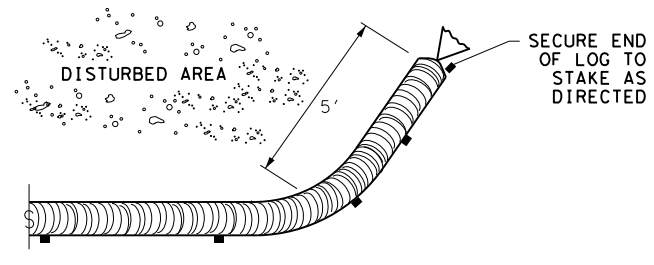
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**EROSION CONTROL LOGS ON SLOPES
 STAKE AND TRENCHING ANCHORING**

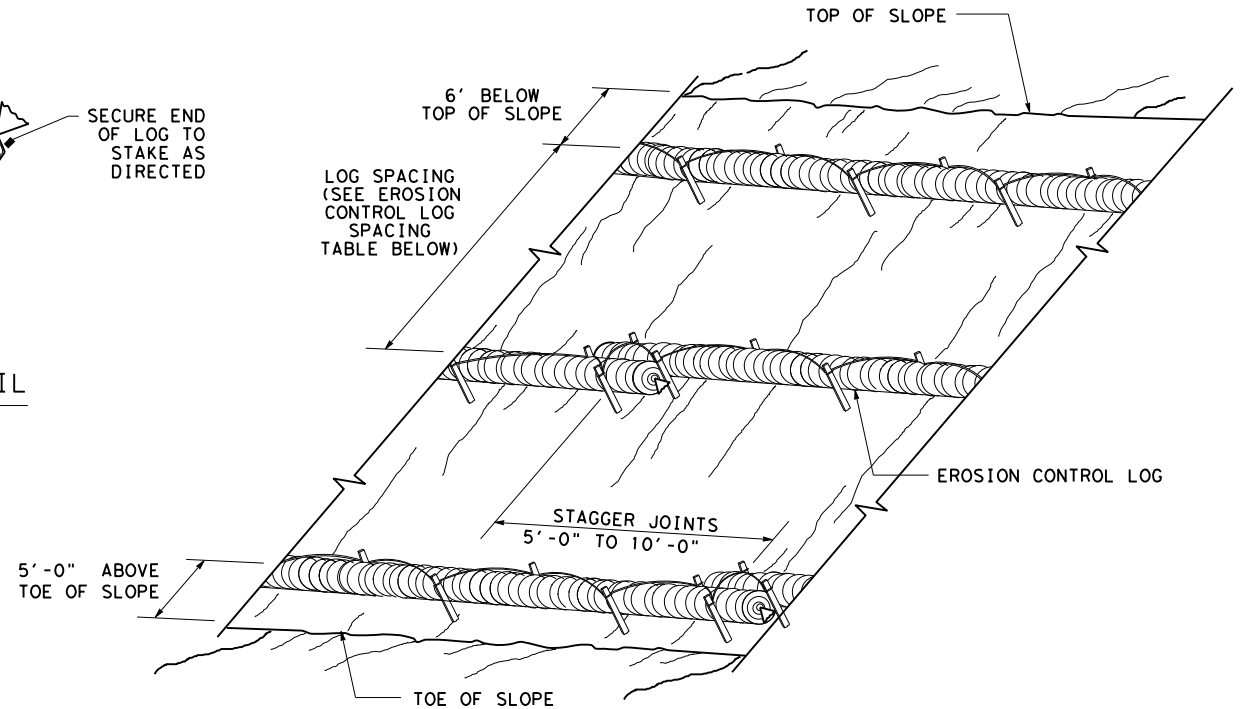
CL-SST



END SECTION RAP DETAIL

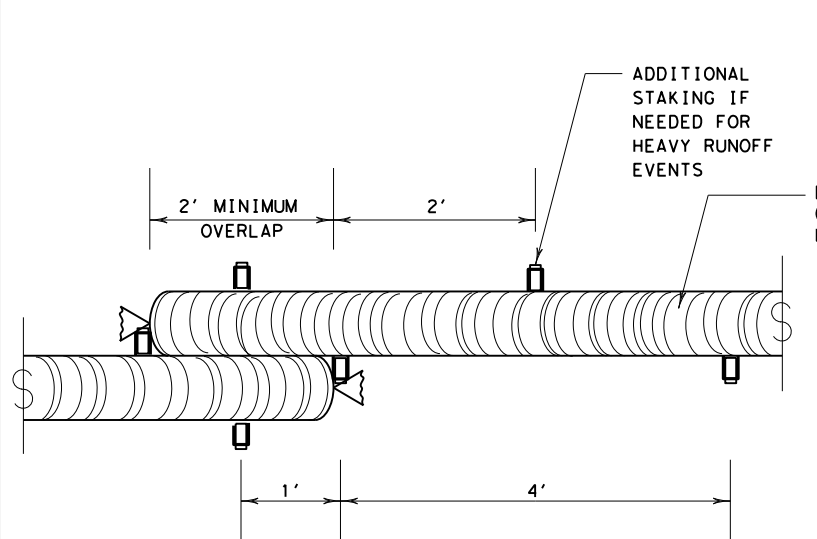
| SLOPE | LOG DIAMETER | | | |
|----------------|--------------|-----|-----|-----|
| | 6" | 8" | 12" | 18" |
| 1:1 OR STEEPER | 5' | 10' | 15' | 20' |
| 2:1 | 10' | 20' | 30' | 40' |
| 3:1 | 15' | 30' | 45' | 60' |
| 4:1 OR FLATTER | 20' | 40' | 60' | 80' |

* ADJUSTMENTS CAN BE MADE FOR SOIL TYPE:
 SOFT, LOAMY SOILS-ADJUST ROWS CLOSER TOGETHER;
 HARD, ROCKY SOILS- ADJUST ROWS FARTHER APART



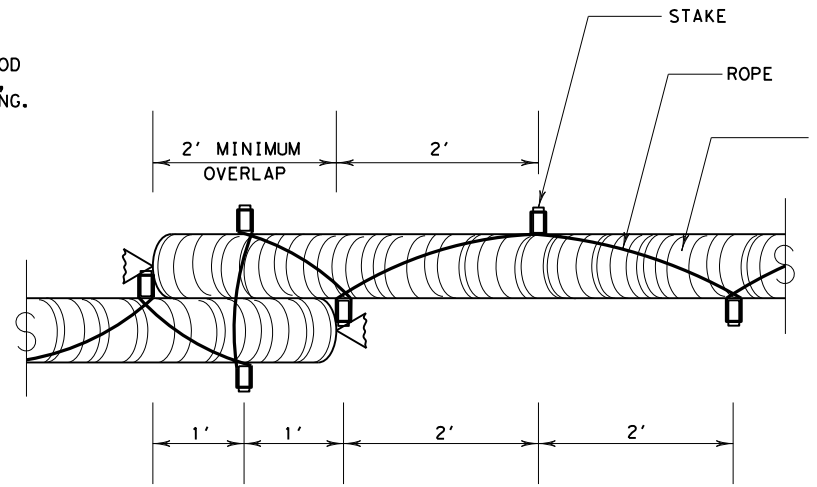
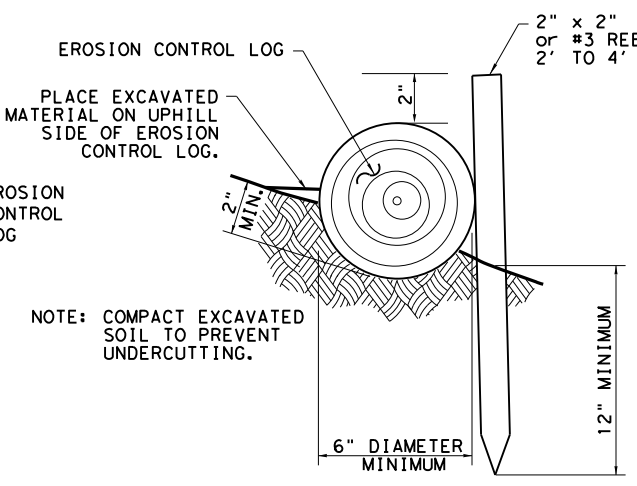
**EROSION CONTROL LOGS ON SLOPES
 STAKE AND LASHING ANCHORING**

CL-SSL



STAKE AND TRENCHING ANCHORING DETAIL

CL-SST

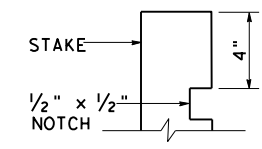


STAKE AND LASHING ANCHORING DETAIL

CL-SSL

| LOG DIAMETER | DEPTH |
|--------------|-------|
| 6" | 2" |
| 8" | 3" |
| 12" | 4" |
| 18" | 5" |

TRENCH DEPTH TABLE



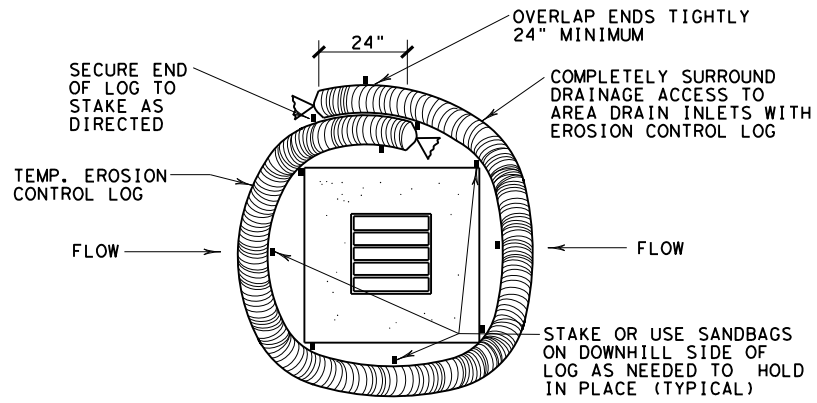
STAKE NOTCH DETAIL

SHEET 2 OF 3

| | | | |
|--|------------|--------------------------|-----------|
| | | Design Division Standard | |
| TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES EROSION CONTROL LOG EC(9) - 16 | | | |
| FILE: ec116 | DN: TxDOT | CK: KM | DW: LS/PT |
| © TxDOT: JULY 2016 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | SHEET NO. | |
| AUS | WILLIAMSON | 145 | |

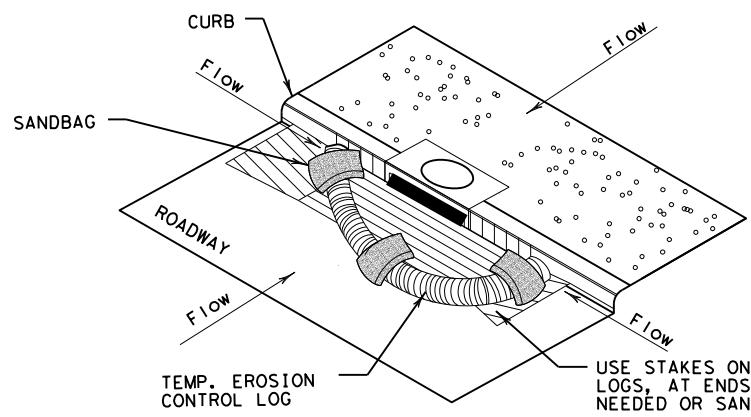
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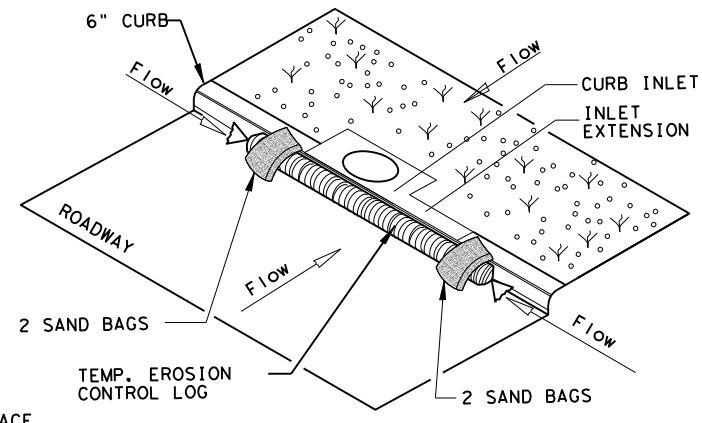
EROSION CONTROL LOG AT DROP INLET

CL-DI



EROSION CONTROL LOG AT CURB INLET

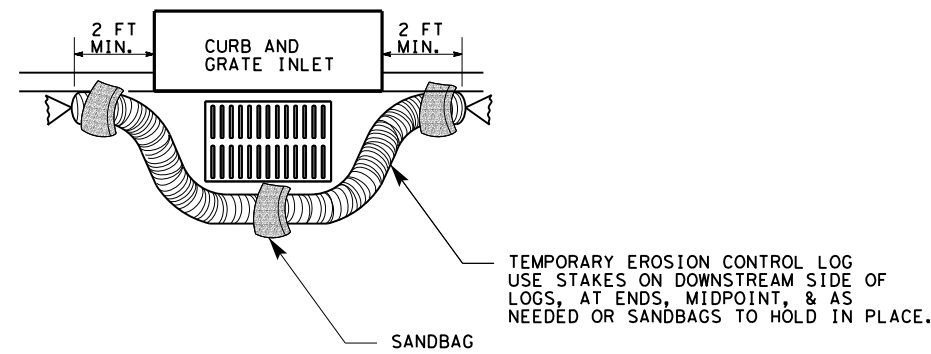
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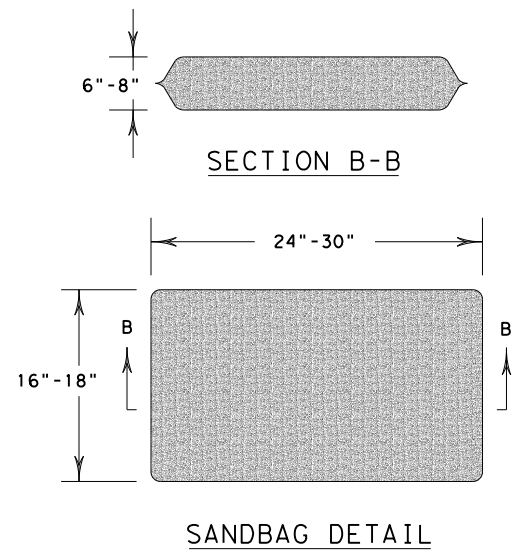
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NOTE:
 EROSION CONTROL LOGS USED AT CURB INLETS SHOULD ONLY BE USED IF THEY WILL NOT IMPEDE TRAFFIC OR FLOOD THE ROADWAY OR WHEN THE STORM SEWER SYSTEM IS NOT FULLY FUNCTIONAL.



EROSION CONTROL LOG AT CURB & GRADE INLET

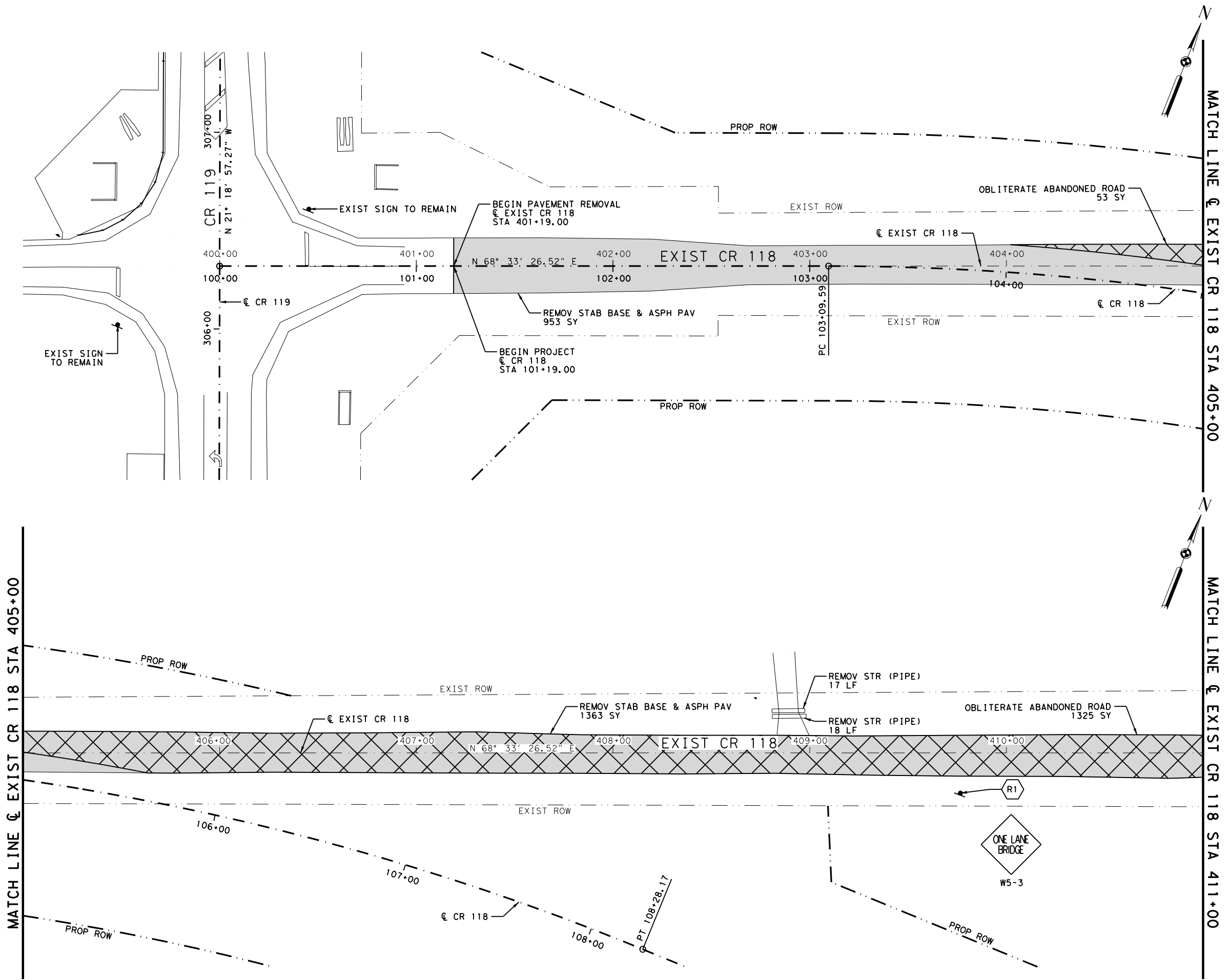
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


SHEET 3 OF 3

| | | | |
|---|------------|---------------------------------|-----------|
| | | <i>Design Division Standard</i> | |
| TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES EROSION CONTROL LOG EC (9) - 16 | | | |
| FILE: ec916 | DN: TxDOT | CK: KM | DW: LS/PT |
| © TxDOT: JULY 2016 | CONT | SECT | JOB |
| REVISIONS | 0914 | 05 | 204, ETC. |
| DIST | COUNTY | | SHEET NO. |
| AUS | WILLIAMSON | | 146 |

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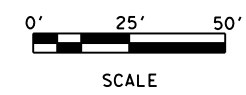


LEGEND


-  OBLITERATE ABANDONED ROAD
-  REMOVE STAB BASE & ASPH PAV
-  EXISTING SIGN
-  EXISTING SIGN TO BE REMOVED
-  EXISTING OBJECT MARKER
-  EXISTING OBJECT MARKER TO BE REMOVED

NOTES:

1. REMOVAL OF EXISTING GRAVEL DRIVEWAYS IS SUBSIDIARY TO ITEM 0110 600T EXCAVATION.



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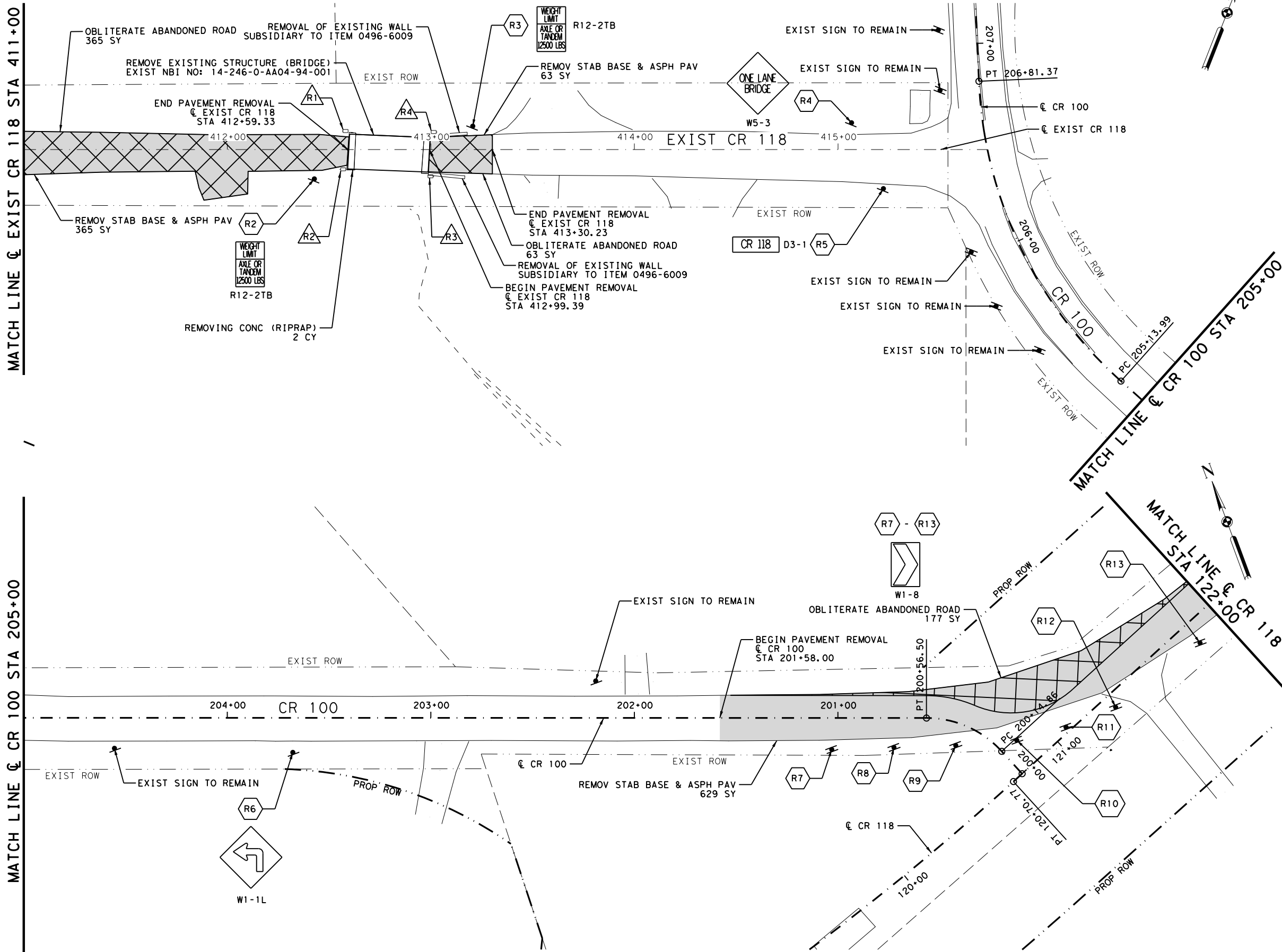
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 Texas Department of Transportation

**CR 118 AT COTTONWOOD CREEK
 REMOVAL LAYOUT**

SHEET 1 OF 3

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 147 |

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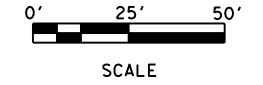


LEGEND

- OBLITERATE ABANDONED ROAD
- REMOVE STAB BASE & ASPH PAV
- EXISTING SIGN
- EXISTING SIGN TO BE REMOVED
- EXISTING OBJECT MARKER
- EXISTING OBJECT MARKER TO BE REMOVED

NOTES:

- REMOVAL OF EXISTING GRAVEL DRIVEWAYS IS SUBSIDIARY TO ITEM 0110 6001 EXCAVATION.



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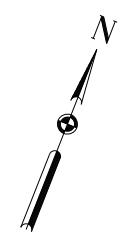
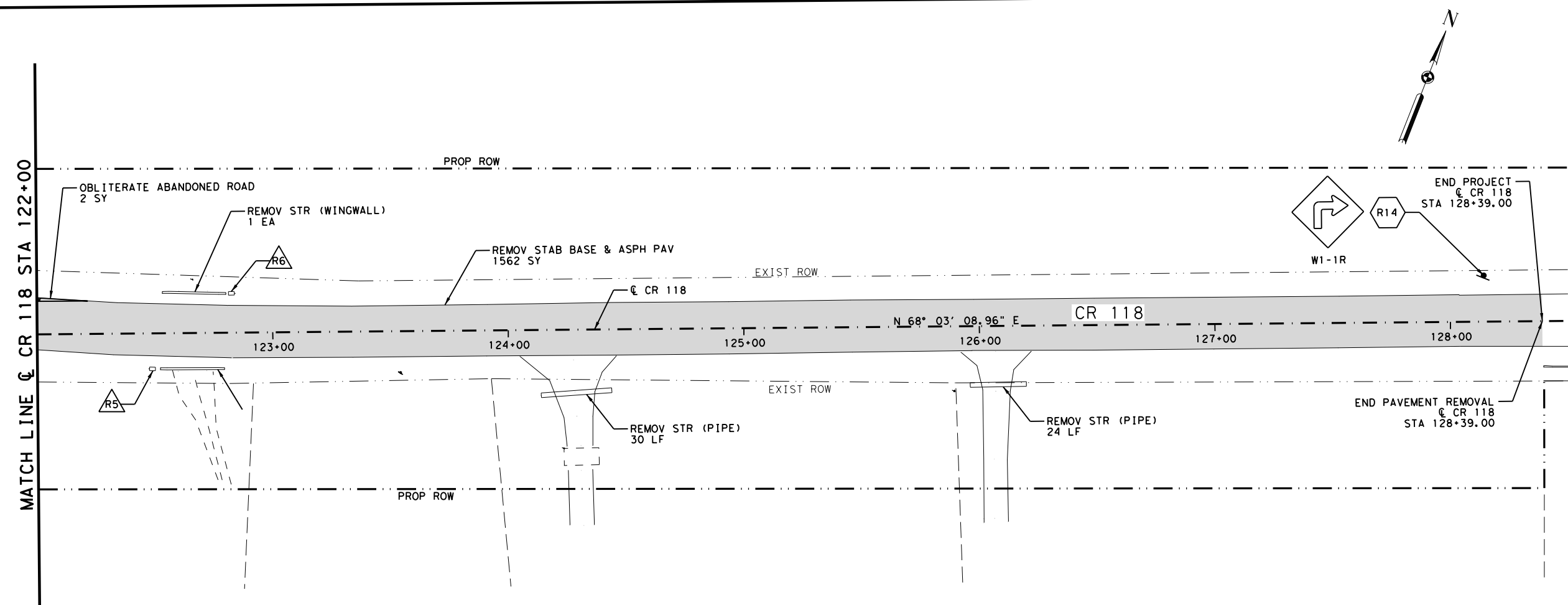
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CR 118 AT COTTONWOOD CREEK
REMOVAL LAYOUT

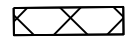





SHEET 2 OF 3

| CONT | SECT | JOB | HIGHWAY |
|------|------|------------|-----------|
| 0914 | 05 | 204, ETC. | CR 118 |
| DIST | | COUNTY | SHEET NO. |
| AUS | | WILLIAMSON | 148 |

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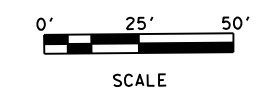



LEGEND

-  OBLITERATE ABANDONED ROAD
-  REMOVE STAB BASE & ASPH PAV
-  EXISTING SIGN
-  EXISTING SIGN TO BE REMOVED
-  EXISTING OBJECT MARKER
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NOTES:

1. REMOVAL OF EXISTING GRAVEL DRIVEWAYS IS SUBSIDIARY TO ITEM 0110 6001 EXCAVATION.



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 11/3/2022


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CR 118 AT COTTONWOOD CREEK

REMOVAL LAYOUT

SHEET 3 OF 3

| | | | | |
|-------------|----------------|------|-----------|-----------|
| ©TXDOT 2022 | CONT | SECT | JOB | HIGHWAY |
| | 0914 | 05 | 204, ETC. | CR 118 |
| | DIST COUNTY | | | SHEET NO. |
| | AUS WILLIAMSON | | | 149 |