INDEX OF SHEETS

SEE SHEET 2 FOR INDEX OF SHEETS SHEET 3 FOR LOCATION MAP

STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT

> FEDERAL AID PROJECT. BR 2022 (837), E+c.

CR 26320, ETC. LAMAR COUNTY, ETC.

LIMITS: CR 26320 AT MORRISON CREEK LIMITS: FM 2068 AT E FORK JERNIGAN CREEK

FOR THE CONSTRUCTION OF: BRIDGE REPLACEMENT CONSISTING OF: REPLACE EXISTING BRIDGE AND APPROACHES

ROAD NAME	LOCATION	CSJ	PROJECT NO.	COUNTY		ONING		LENGTH		LENGTH		LENGTH	DESIGN SPEED	ADT	ADT YEAR	FUNCTIONAL CLASSIFICATION
NOAD NAME	LOCATION	050	TROULET NO.	COONTI	BEGIN	END	FEET	MILES	FEET	MILES	FEET	MILES	MPH	AUT	ADT TEAN	CLASSIFICATION
FM 2068	E FORK JERNIGAN CREEK	1097-05-009	BR 2023(930)	DELTA	24+59	37+16	90	0.017	1167	0.221	1257	0.238	55 MPH ROADWAY	164 230	2021 2041	MINOR COLLECTOR
CR 26320	MORRISON CREEK	0901-29-092	BR 2022(837)	LAMAR	2+62	6+20	50	0.009	308	0.058	358	0.068	20 MPH ROADWAY	35 35	2021 2041	LOCAL

EXCEPTIONS: N/A EQUATIONS: N/A RAILROAD CROSSINGS: N/A

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF TRANSPORTATION, NOVEMBER 1, 2014 AND SPECIFICATION ITEMS LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION CONTRACTS (FORM FHWA 1273, JULY 5, 2022)

FHWA TEXAS				SHEET NO.
DIVISION				1
STATE	DISTRICT		COUNTY	
TEXAS	PAR	Lon	nar, E	tc.
CONTROL	SECTION	JOB	HIGHWAY	r NO.
0901	29	092,E†c.	CR, E	tc.

FINAL PLANS

LETTING DATE:

DATE CONTRACTOR BEGAN WORK:

DATE WORK WAS COMPLETED:

DATE WORK WAS ACCEPTED: ORIGINAL CONTRACT WORKING DAYS:

USED OF WORKING DAYS

NO. OF CHANGE ORDERS:

FINAL CONTRACT COST:

PERCENT OVER/UNDER RUN:

CONTRACTOR:

ICERTIFY THAT THIS PROJECT WAS BUILT IN ACCORDANCE WITH PLANS AND SPECIFICATIONS.

AREA ENGINEER

DATE

REQUIRED SIGNS SHALL BE IN ACCORDANCE WITH BC (1)- 21 THRU BC (12)- 21 AND THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES".

©2023 C Texos Department of Tran	sportation
SUBMITTED FOR LETTING:	March 31, 2023
Monte R. Retu	P.E.
DESIGN ENGINEER	
RECOMMENDED FOR LETTING:	3/31/2023
Docusigned by: Hamiel H. Jaylor; P.F.	
D3B5B88489E542FAREA ENGINEER	
APPROVED FOR LETTING:	3/31/2023
DocuSigned by: Nocl ParamananJham	

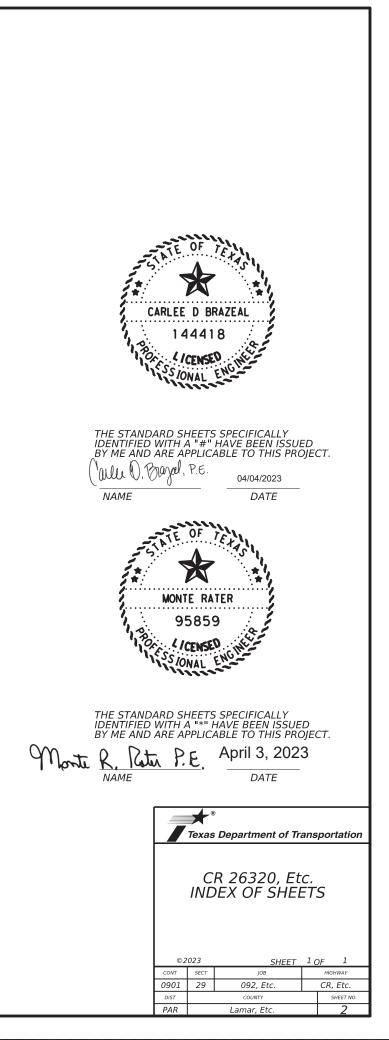
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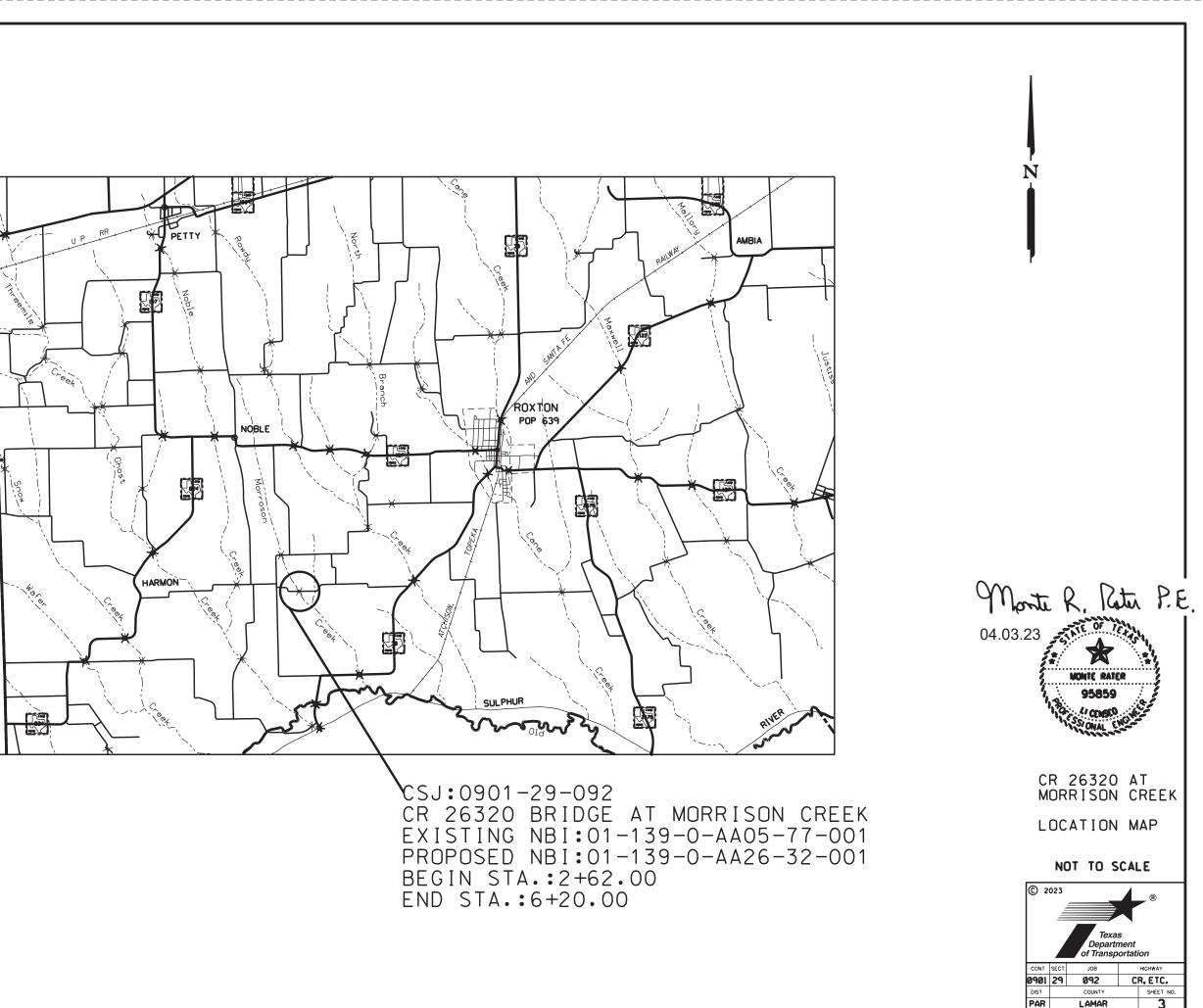
<u></u>	EET NO.	DESCRIPTION	SHEET NO.	DESCRIPTION
		GENERAL		
	1	TITLE SHEET	# 17.10	BRIDGE STANDARDS
	2	INDEX OF SHEETS	# 47-49	AIG-34
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	5,5A-5D	GENERAL NOTES	# 51	BAS-A
	6,6A	ESTIMATE & QUANTITY	* 52	APSB-24
			# 53-54	CSAB
			* 55-56	FD
		TRAFFIC CONTROL PLAN STANDARDS	# 57-58	IGD
#	7-18	BC (1)-21 THRU BC (12)-21	# 59-60	IGEB
	19	TREATMENT FOR VARIOUS EDGE CONDITIONS	# 60A	IGEB
#	20	WZ(RCD)-13	# 61-62	IGMS
#	20A	TCP(1-1) - 18	# 63-64	IGSD-24
#	20A 20B	TCP(1-2) - 18	# 65	IGSK
#	20B 20C	TPC(2-2) - 18	# 66-67	MEBR(C)
	200	1FG(2-2) - 18	# 68-71	PCP
			# 72	PCP-FAB
		ROADWAY DETAILS	* 73	PSB-5SB15
	21	TREE TRIMMING & BRUSH REMOVAL	* 74	PSBEB
			* 74	PSBRA
		ROADWAY DETAILS STANDARDS		PSBSD
				SIG-34
#	22	GF(31)-19	* 79	SPSB-24
#			# 80-81	SRR
	22A	GF(31)LS - 19	* 82-83	TYPE T631LS
#	22B	MBGF(TR) - 19	# 84-86	TYPE T223
#	23	SGT(12S)31-18		
#	24	SGT(15)31-20		
#	25	TE(HMAC)-11		PAVEMENT MARKINGS & DELINEATION DETAILS STAND
	25A	MISCELLANEOUS DETAILS	#	
	25B	HOTMIX LONGITUDINAL JOINT DETAIL	# 87	D&OM(1)-20
*	26	WF(2)-10	# 88	D&OM(2)-20
	27	WATER GAP	# 89	D&OM(5)-20
			# 90	D&OM(VIA)-20
		CR 26320 AT MORRISON CREEK	# 91	PM(1)-22
	28	TYPICAL SECTION	# 92	PM(2)-22
	29	ROAD CLOSURE PLAN		
	30	QUANTITY SUMMARIES		
	31	PLAN AND PROFILE		ENVIRONMENTAL ISSUES
	32	DRAINAGE AREA MAP	93-96	SWP3
	33	HYDRAULIC DATA	97-98	EPIC
	34	BRIDGE LAYOUT	99-100	SWP3 LAYOUT
	34 35	BRIDGE QUANTITIES AND BEARING SEAT ELEVATIONS		
	30	BRIDGE QUANTITIES AND BEARING SEAT ELEVATIONS		
				ENVIRONMENTAL ISSUES STANDARDS
		EM 2068 AT EAST FORK JERNIGAN CREEK	# 101	EC(1)-16
	36	TYPICAL SECTION	# 102	EC(2)-16
	36		# 103	EC(3)-16
	37	PAVEMENT CORE DATA		
	38	ROAD CLOSURE PLAN		
	39-40	QUANTITY SUMMARIES		
	41	PLAN AND PROFILE		
	42	DRAINAGE AREA MAP		
	43	HYDRAULIC DATA		
	44	BRIDGE LAYOUT		
	45	BRIDGE QUANTITIES AND BEARING SEAT ELEVATIONS		
		MBGF DETAILS FOR CULVERTS		

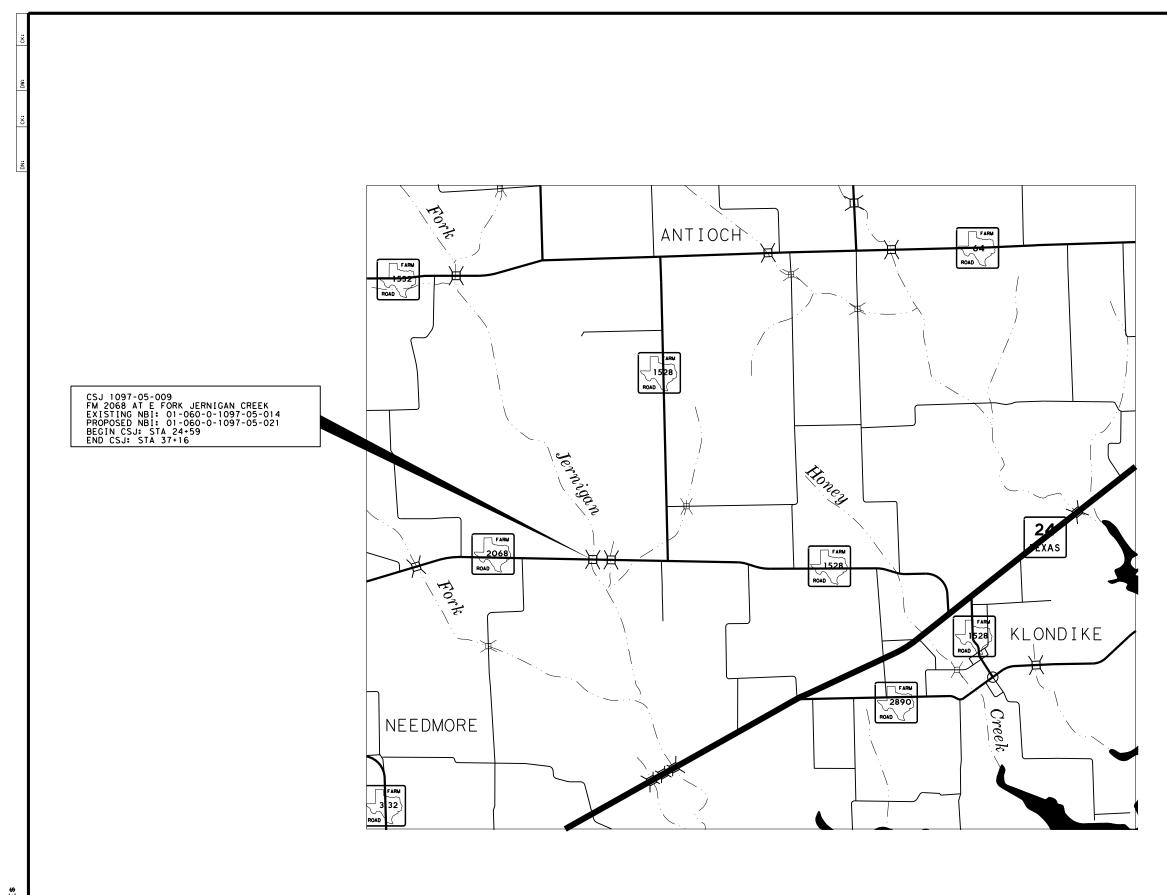
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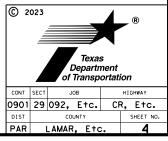
DATE:











Highway: CR, Etc.

Control: 0901-29-092, Etc.

Sheet:

GENERAL NOTES

General:

Contractor questions on this project are to be addressed to the following individual(s):

Paris Area Office Daniel Taylor, P.E. - Daniel.Taylor@txdot.gov Zachary Smith, P.E. - Zachary.Smith@txdot.gov

Contractor questions will be accepted through email, phone, and in person by the above individuals.

Questions may be submitted via the Letting Pre-Bid Q&A web page. This webpage can be accessed from the Notice to Contractors dashboard located at the following Address: https://tableau.txdot.gov/views/ProjectInformationDashboard/NoticetoContractors

All contractor questions will be reviewed by the Engineer. All questions and any corresponding responses that are generated will be posted through the same Letting Pre-Bid Q&A web page.

The Letting Pre-Bid Q&A web page for each project can be accessed by using the dashboard to navigate to the project you are interested in by scrolling or filtering the dashboard using the controls on the left. Hover over the blue hyperlink for the project you want to view the Q&A for and click on the link in the window that pops up.

On Contractor request, earthwork cross sections and construction timelines will be posted to TxDOT's Public FTP at the following Address:

https://ftp.dot.state.tx.us/pub/txdot-info/Pre-Letting%20Responses/

The site is organized by District, Project Type (Construction or Maintenance), Letting Date, CCSJ/Project Name.

Dispose of waste materials at an approved site. Furnish written approval from the property owner before disposal of waste materials.

Locate equipment a minimum of 30 feet from roadway when possible. Place signs and barricades as approved.

Stockpile sites for construction materials must be approved. Give at least 48 hours notification prior to stockpiling material.

Soil Sulfates Mitigation- The following notes are referenced hereafter by Items 110, 132 & 260:

County: Lamar, Etc.

Highway: CR, Etc.

Subgrade Sulfate Testing ~ Once proposed subgrade elevations are obtained TXDOT may test subgrade using an in-field continuous conductivity machine to determine where necessary soil samples will be collected for laboratory testing. Laboratory testing will determine necessary high sulfate mitigation techniques.

0 - 3,000 ppm of sulfates - no restrictions.

3.001 - 7.000 ppm of sulfates - In a single application, add the prescribed total amount of lime. Uniformly mix the lime into the soil being treated. Lightly compact the mixture to seal and minimize carbonation. Maintain moisture content above optimum. Three days after initial addition of lime, determine soluble sulfate concentration sampled at locations as directed using Tex-145-E, and if the sulfate measurement has been reduced to 3,000 ppm or less, then no additional lime or mellowing time is necessary; however, if sulfate measurement has not been reduced to 3,000 ppm or less, then add 4% additional lime. Uniformly mix the lime into the pretreated soil. Lightly compact the mixture to seal and minimize carbonation and mellow an additional 7 days while maintaining moisture content above optimum. Reprocess the soil-lime mixture to meet the gradation requirements in Item 260, Table 1, and compact it at the optimum moisture content. Greater than 7,000 ppm of sulfates - Do not bring this soil onto project. Remove or process as directed.

The Department will pay for additional lime treatment of material originating in TxDOT right-ofway when sulfate concentrations are greater than 3,000 ppm and such material is required to be used as shown in the plans. No additional compensation will be made for stabilizing and treating embankment material obtained outside TxDOT right-of-way which has a sulfate concentration exceeding 3,000 ppm.

Item 5 Control of the Work:

The responsibility for the construction surveying on this contract will be in accordance with Section 5.9.3, Method A.

Working days will be computed and charged in accordance with Article 8.3.1.4 Standard Work Week.

Right and left are determined based upon the forward direction of stationing in the specific control section.

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/business/resources/highway/bridge/bridge-publications.html#design. Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

Control: 0901-29-092, Etc.

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General Notes

Highway: CR, Etc.

Control: 0901-29-092, Etc.

Sheet:

Item 6 Control of Materials:

The existing bridges at FM 2068 at East Fork Jernigan Creek and CR 26320 at Morrison Creek has lead-containing paint. Provide a demolition plan to the Engineer three weeks in advance of lead paint disturbance to allow lead paint removal by TxDOT on-call contractor before Contractor bridge demolition.

To comply with the latest provisions of Build America, Buy America Act (BABA Act) of the Bipartisan Infrastructure Law, the contractor must submit a notarized original of the TxDOT Construction Material Buy America Certification Form for all items classified as construction materials. This form is not required for materials classified as a manufactured product.

Refer to the Buy America Material Classification Sheet for clarification on material categorization.

The Buy America Material Classification Sheet is located at the below link.

https://www.txdot.gov/business/resources/materials/buy-america-material-classification-sheet.html

Item 7 Legal Relations and Responsibilities:

No significant traffic generator events identified.

Item 8 Prosecution and Progress:

Before beginning work on this project submit in writing, for approval, a plan of construction operations outlining in detail a sequence of work to be followed.

Provide a Bar Chart progress schedule for this project.

Item 9 Measurement and Payment:

Items of work for the Monthly Estimate will be cut off on the 25th of each month. Items of work performed after the 25th will be processed and paid on the following month's estimate. Material On Hand (MOH) will cut off on the 20th of each month. Special circumstances will be considered on a case-by-case basis.

Item 100 Preparing Right of Way:

Remove all trees to ROW line on both sides of roadway. Remove underbrush and neatly trim trees and overhanging branches to produce a 60' vertical clear area within the limits of Prep ROW. Remove any trees or underbrush that interferes with any construction operation, including relocation of ditches or other drainage elements. Receive approval of equipment used to trim limbs. A boom axe will not be allowed. Remove all trimmed debris from the ROW or

County: Lamar, Etc.

Highway: CR, Etc.

mulch all debris and incorporate into the topsoil on State ROW to the satisfaction of the Engineer.

Item 110 Excavation:

Material below finished subgrade elevation suspected of containing sulfates will be tested in accordance with Tex-145-E by the Department. Treat subgrade material to the required depth and width in accordance with the Soil Sulfates Mitigation General Notes.

Before excavation operations the existing topsoil shall be salvaged in a manner to preserve the vigor of the existing Bermuda grass sod per Item 160.

Item 132 Embankment:

Test potential embankment sources using Tex-145-E to determine the presence and concentration of sulfates. Do not bring soil with greater than 3000 ppm sulfates into project.

Embankment sources containing sulfates that meet specification requirements may be used as fill material provided it is placed with at least one foot of separation from materials to be treated with lime, cement, or other calcium-based stabilizers. When soils are to be placed with less than one foot of separation from material to be treated with lime, cement, or other calcium-based stabilizers, process and treat such soils according to the Soil Sulfates Mitigation General Notes.

Excavation pits for project embankment made within 250 feet of State Right of Way must be approved.

Before embankment operations the existing topsoil shall be salvaged in a manner to preserve the vigor of the existing Bermuda grass sod per Item 160.

Item 164 Seeding for Erosion Control, 166 Fertilizer:

Apply fertilizer with a ratio of 3-1-2 (N-P-K) over the areas to be seeded. This work will not be paid for directly but will be considered subsidiary.

Item 168 Vegetative Watering:

Use water trucks equipped with a sprinkler system adequate to permit coverage of the entire seeded area from the roadbed. This equipment must be available to perform watering throughout the duration of vegetative establishment.

Water all seeded areas the day seed is applied. Thereafter, maintain the seeded areas in a wellwatered condition throughout the duration of vegetative establishment.

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Highway: CR, Etc.

Sheet:

Item 275 Cement Treatment (Road Mixed):

Microcracking is required where flexible base widths accept full roller width. When temperatures during curing period average below 60 degrees F, perform microcracking operations between 48 and 72 hours.

Subgrade, embankment or backfill suspected of containing sulfates will be tested in accordance with Tex-145-E by the Department. Subgrade, embankment or backfill material within one foot of any area to be treated using cement is subject to the following restriction:

Greater than 7,000 ppm sulfates – Do not treat with any cement or other calcium-based stabilizers. Material within one foot of any area to be treated with cement or other calcium-based stabilizers must be removed or processed as directed.

Item 400 Excavation and Backfill for Structures:

Excavation and backfill for bridge construction will be subsidiary to the project bid items.

Item 416 Drill Shaft Foundations:

One core hole per bent/abutment required.

Item 420 Concrete Structures:

Do not use membrane curing for structural elements.

Item 421 Hydraulic Cement Concrete:

Type A bridge expansion joints shall be subsidiary to Item 422.

Item 432 Riprap:

The Engineer may adjust placement of riprap in the field.

Filter fabric is required for stone riprap.

Bridge demolition waste concrete may be used for stone rip rap. Cut protruding rebar within 2" of concrete surface. Maximum waste concrete cobble size shall match proposed stone rip rap Dmax size.

Item 496 Removing Structure:

For County Road Bridge - The Contractor shall coordinate with the county commissioner for transferring salvageable materials such as beams, piling, and concrete riprap. The Contractor shall dispose of remaining materials.

County: Lamar, Etc.

Highway: CR, Etc.

Item 502 Barricades, Signs and Traffic Handling:

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

The following items will be required for flagger on this project:

- 2. Flaggers will be required at the intersection of all State maintained roadways.
- 3. Flaggers may be required at other high traffic generating intersections as deemed necessary by the Area Engineer.

The traffic control plan for this contract consists of the installation and maintenance of warning signs and other traffic control devices shown in the plans, specification data which may be included in the general notes, applicable provisions of the Texas Manual on Uniform Traffic Control Devices (TMUTCD), traffic control plan sheets included in the plans, standard BC sheets and Item 502 of the Standard Specifications.

Do not begin Item 502, Barricades, Signs, and Traffic Handling, on the roadway until both of the following conditions are met:

- 1. The work schedule is approved.
- commencement of roadway work bid items.

The final estimate will be withheld until all disturbed areas are covered with at least 70% perennial vegetative cover.

Correct all deficiencies within the time frame noted on the Traffic Control Device Inspection Form 599. Failure to make corrections within time frame specified may result in no payment for this Item for the month of the noted deficiency.

Control: 0901-29-092, Etc.

Sheet: 5B

1. Flaggers are required to wear a white hard hat while performing flagging operations.

2. No more than 5 workdays will pass between the beginning of Item 502 and the actual

Highway: CR, Etc.

Control: 0901-29-092, Etc.

Sheet:

Item 506 Temporary Erosion, Sedimentation & Environmental Controls:

The Temporary Erosion Control measures for this project will consist of using the following items, as directed:

- 1. Temporary Silt Fence
- 2. Rock Filter Dams: All rock filter dams shall be installed with 6:1 slopes regardless of their location on the project. Failure to do so will result in no payment for the dam.

Silt fences will remain the property of the Contractor upon completion of the project. The final estimate will not be released until all silt fences have been properly removed, or as directed and 70% establishment of vegetative cover is obtained.

Acquire approval for any change to the location of temporary sediment fence, as shown in the plans, prior to installation. Placement of erosion protection devices may be altered, as directed, to satisfy the requirements of the SW3P.

The pay item to remove rock filter dams will require only a partial removal after 70 percent perennial vegetation has been established and approved. When removing the rock filter dams, leave the lower layer of rock adjacent to the ground in place so as not to disturb the soil.

Refer to the SW3P sheet for the total disturbed area for the project.

The disturbed area in this project, all project locations in the Contract, and Contractor project specific locations (PSLs) within one mile of the project limits will further establish the authorization requirements for storm water discharges. The Department will obtain an authorization to discharge storm water from the Texas Commission on Environmental Quality (TCEQ) for the construction activities shown on the plans. Obtain any required authorization from the TCEQ for any Contractor PSLs for construction support activities on or off ROW. When the total area disturbed for all projects in the Contract and PSLs within one mile of the project limits exceeds five acres, provide a copy of the Contractors NOI for PSLs on the ROW (to the appropriate MS4 operator when on an off-system route).

Item 542 Removing Metal Beam Guard Fence:

Removed MBGF rail shall be retained by the Contractor.

Item 585 Ride Quality for Pavement Surfaces:

Use Surface Test Type A to evaluate ride quality of the final pavement surface on travel lanes and shoulders in accordance with Item 585, "Ride Quality for Pavement Surfaces."

County: Lamar, Etc.

Highway: CR, Etc.

Item 666 Reflectorized Pavement Markings:

been given by the Contractor. Lay out pilot lines for approval 24 hours prior to all final pavement marking applications.

counters prior to the beginning of striping operations.

paint film.

time.

zones.

Item 3076 Dense-Graded Hot-Mix Asphalt:

All surface mixes are to be SAC A.

The use of PG 64-22 asphalt is required.

Use a self-propelled wheel mounted MTV capable of receiving mix from the haul trucks, separate from the paver. It shall have a minimum storage capacity of approximately 25 tons. It shall be equipped with a pivoting discharge conveyor and shall completely and thoroughly remix the material prior to placement. The effectiveness of the MTV's remixing ability is subject to the approval of the Engineer. In addition, the paver shall have a surge storage insert with a minimum capacity of 20 tons.

submittal. After design submittal, continue producing the chosen design unless otherwise approved.

of contractor owned RAP will not be allowed if it consists primarily of siliceous aggregates.

A tack coat is required for all overlay areas and for all longitudinal joints unless otherwise directed.

Evaluation of the mixture for moisture susceptibility will be performed by using test method any time during production.

Control: 0901-29-092, Etc.

Sheet: 5C

- No stripe will be placed unless the inspector is present and at least 24 hours advance notice has
- Use equipment with footage counters capable of measuring the linear footage placed. Calibrate
- Reduce truck speed enough to ensure that the beads drop onto the stripe and do not roll in the
- Due to problems in traffic handling, do not place a dash center stripe and edge line at the same
- Contact the Engineer 7 days before pavement marking placement for re-establishment of no-pass

- Specify Hot Mix Asphalt Concrete (HMAC) or Warm Mix Asphalt (WMA) at the time of design
- RAP from contractor owned sources may be used if the RAP is fractionated. The course fraction
- TEX 530-C (boil test) and there shall be no evidence of stripping during design verification or at

Control: 0901-29-092, Etc.

Highway: CR, Etc.

Sheet: 5D

Item 3076 Dense-Graded Hot-Mix Asphalt (Cont.):

The maximum nighttime paved surface vertical differential will be limited to two inches. Prevent ponding of water on any travel ways that are exposed to traffic.

Perform all sampling for aggregate quality testing on stockpiles at the HMAC plant. Mixture sampling for QC/QA testing will typically be taken from the truck at the plant; however, the Engineer may direct that a sample be taken at any point or location of mixture during production, delivery or placement.

Preparation and construction of permanent / temporary transitions, terminations of mix courses and transitions to driveways and intersecting roadways is subsidiary to Item 341. This includes all labor, machinery, materials and incidentals to complete the work including planing, removal, hauling and stockpiling of materials and necessary clean-up.

Item 3096 Asphalts, Oils, and Emulsions:

Provide 1L (1qt.) clean and dry screw top or friction-lid sampling cans as directed.

Furnish at least one sample of each type of asphalt used on the project for QA/QC purposes.

Item 6001 Portable Changeable Message Board:

Two (2) portable changeable message boards are required for advance warning.

Item 6185 Truck Mounted Attenuators:

Shadow vehicles with truck mounted attenuator (TMA) are required on the traffic control plan and TCP standards for this project. The contractor will be responsible for determining if one or more of these traffic control operations will be ongoing at the same time to determine the total number of TMAs needed for the project.



Estimate & Quantity Sheet

DISTRICT Paris HIGHWAY CR 2632, FM 2068 COUNTY Delta, Lamar

of Transportation					
٩LT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL
	100-6002	PREPARING ROW	STA	17.000	
	105-6038	REMOVING STAB BASE AND ASPH PAV (11")	SY	789.000	
	110-6001	EXCAVATION (ROADWAY)	CY	221.000	
	110-6002	EXCAVATION (CHANNEL)	CY	556.000	
	132-6003	EMBANKMENT (FINAL)(ORD COMP)(TY B)	CY	4,144.000	
	164-6009	BROADCAST SEED (TEMP) (WARM)	SY	8,610.000	
	164-6011	BROADCAST SEED (TEMP) (COOL)	SY	8,610.000	
	164-6015	STRAW/HAY MLCH SEED(PERM)(RURAL)(CLAY)	SY	16,760.000	
	164-6023	CELL FBR MLCH SEED(PERM)(RURAL)(CLAY)	SY	460.000	
	168-6001	VEGETATIVE WATERING	MG	105.000	
	247-6064	FL BS (CMP IN PLC)(TY A GR 4) (6")	SY	820.000	
	247-6096	FL BS (CMP IN PLC)(TY D GR 4)	TON	763.000	
	251-6280	REWORK BS MTL (TY A)(11")(ORD COMP)	STA	12.000	
	275-6001	CEMENT	TON	28.000	
	275-6003	CEMENT TREAT (NEW BASE) (6")	SY	3,582.000	
	316-6029	ASPH (RC-250)	GAL	1,003.000	
	316-6403	AGGR (TY-B GR-5 OR TY-L GR-5)	CY	27.000	
	400-6005	CEM STABIL BKFL	CY	105.000	
	416-6001	DRILL SHAFT (18 IN)	LF	172.000	
	416-6002	DRILL SHAFT (24 IN)	LF	108.000	
	416-6004	DRILL SHAFT (36 IN)	LF	344.000	
	420-6013	CL C CONC (ABUT)	CY	73.600	
	422-6001	REINF CONC SLAB	SF	3,240.000	
	422-6007	REINF CONC SLAB (SLAB BEAM)	SF	1,300.000	
	422-6015	APPROACH SLAB	CY	48.000	
	425-6012	PRESTR CONC SLAB BEAM (5SB15)	LF	247.500	
	425-6037	PRESTR CONC GIRDER (TX40)	LF	447.500	
	432-6033	RIPRAP (STONE PROTECTION)(18 IN)	CY	539.000	
	450-6006	RAIL (TY T223)	LF	240.000	
	450-6019	RAIL (TY T631LS)	LF	128.000	
	454-6003	ARMOR JOINT	LF	68.000	
	496-6009	REMOV STR (BRIDGE 0 - 99 FT LENGTH)	EA	2.000	
	500-6001	MOBILIZATION	LS	1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	MO	8.000	
	506-6001	ROCK FILTER DAMS (INSTALL) (TY 1)	LF	200.000	
	506-6011	ROCK FILTER DAMS (REMOVE)	LF	200.000	
	506-6020	CONSTRUCTION EXITS (INSTALL) (TY 1)	SY	250.000	
	506-6024	CONSTRUCTION EXITS (REMOVE)	SY	250.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	2,095.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	2,095.000	
	540-6002	MTL W-BEAM GD FEN (STEEL POST)	LF	900.000	



ESTIMATE & QUANTITY

DISTRICT	COUNTY	CCSJ	SHEET
Paris	Lamar	0901-29-092	6



CONTROLLING PROJECT ID 0901-29-092

Estimate & Quantity Sheet

DISTRICT Paris HIGHWAY CR 2632, FM 2068 COUNTY Delta, Lamar

ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL		
	542-6001	REMOVE METAL BEAM GUARD FENCE	LF	500.000			
	542-6003	REMOVE DOWNSTREAM ANCHOR TERMINAL	EA	8.000			
	544-6001	GUARDRAIL END TREATMENT (INSTALL)	EA	12.000			
	552-6003	WIRE FENCE (TY C)	LF	493.000			
	552-6008	WIRE FENCE (WATER GAP)	LF	40.000			
	658-6062	INSTL DEL ASSM (D-SW)SZ 1(BRF)GF2(BI)	EA	24.000			
	666-6305	RE PM W/RET REQ TY I (W)6"(BRK)(090MIL)	LF	2,514.000			
	666-6317	RE PM W/RET REQ TY I (Y)6"(BRK)(090MIL)	LF	310.000			
	3076-6016	D-GR HMA TY-C SAC-A PG64-22	TON	592.000			
	3084-6001	BONDING COURSE	GAL	179.000			
	5001-6002	GEOGRID BASE REINFORCEMENT (TY II)	SY	3,582.000			
	6001-6002	PORTABLE CHANGEABLE MESSAGE SIGN	EA	2.000			
	6185-6002	TMA (STATIONARY)	DAY	73.000			
	18	EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART)	LS	1.000			
		SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING)	LS	1.000			



ESTIMATE & QUANTITY

DISTRICT	COUNTY	CCSJ	SHEET
Paris	Lamar	0901-29-092	6A

BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

- The Barricade and Construction Standard Sheets (BC sheets) are intended 1. to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- The development and design of the Traffic Control Plan (TCP) is the 2. responsibility of the Engineer.
- The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop. sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown ON BC(2). THE OBEY WARNING SIGNS STATE LAW sign. STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES. CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, ČSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:

- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility" Apparel." or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

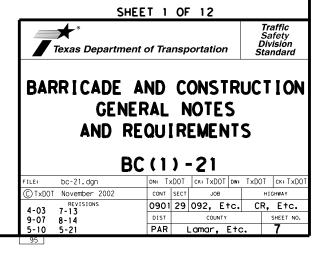
COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

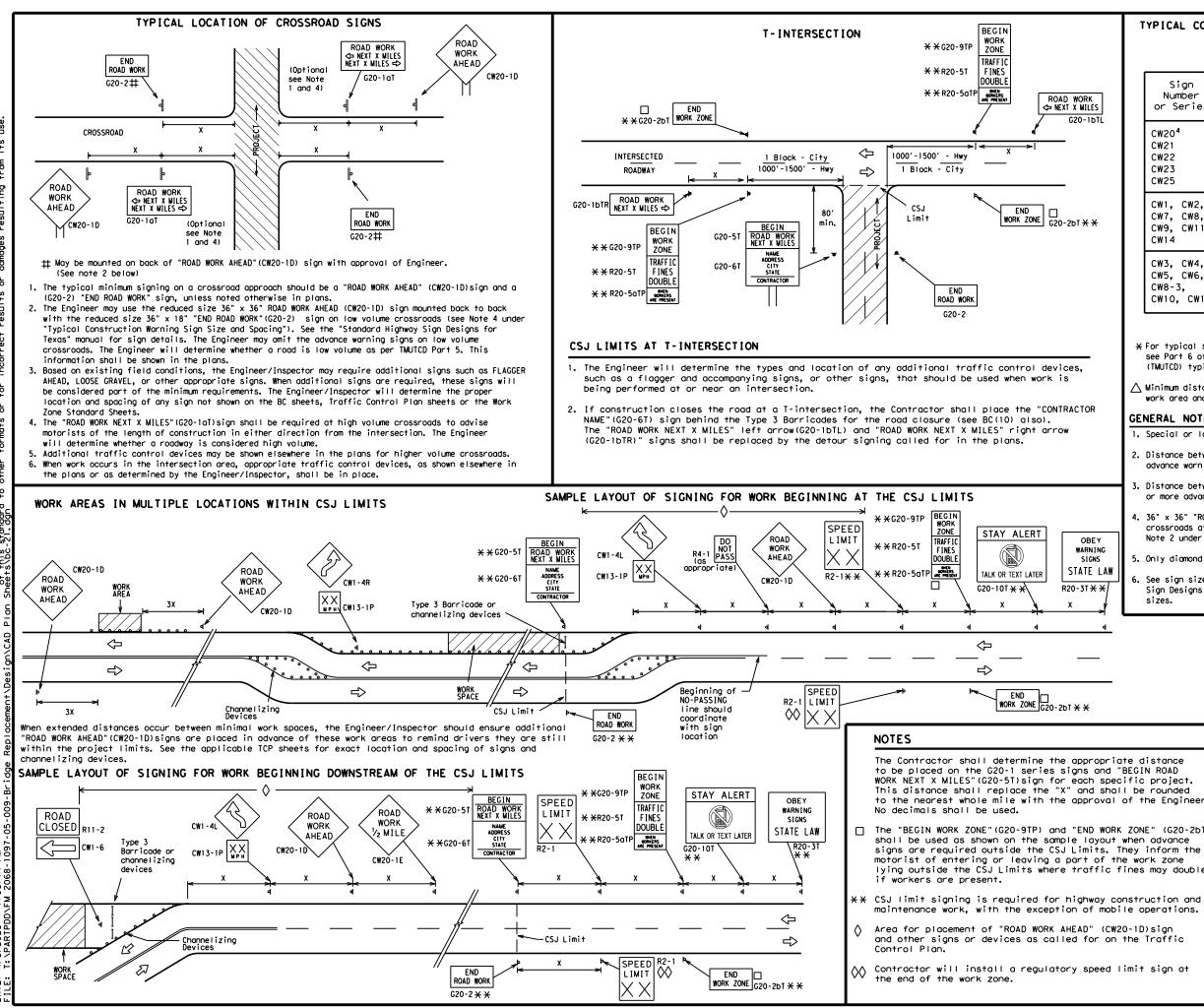
- 1. Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov				
COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD)				
DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS)				
MATERIAL PRODUCER LIST (MPL)				
ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)"				
STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD)				
TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD)				
TRAFFIC ENGINEERING STANDARD SHEETS				

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TYPICAL	CONSTRUCTION	WARNING	SIGN	SIZE	AND	SPACING ^{1,5,6}

SIZE

Sign Number or Series	Conventional Road	Expressway/ Freeway		
CW20 ⁴ CW21 CW22 CW23 CW25	48" × 48"	48" × 48"		
CW1, CW2, CW7, CW8, CW9, CW11, CW14	36" × 36"	48" × 48"		
CW3, CW4, CW5, CW6, CW8-3, CW10, CW12	48" × 48"	48" × 48"		

Posted Speed	Sign∆ Spacing "X"
MPH	Feet (Apprx.)
30	120
35	160
40	240
45	320
50	400
55	500 ²
60	600 ²
65	700 ²
70	800 ²
75	900 ²
80	1000 ²
*	* 3

SPACING

★ For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.

ightarrow Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

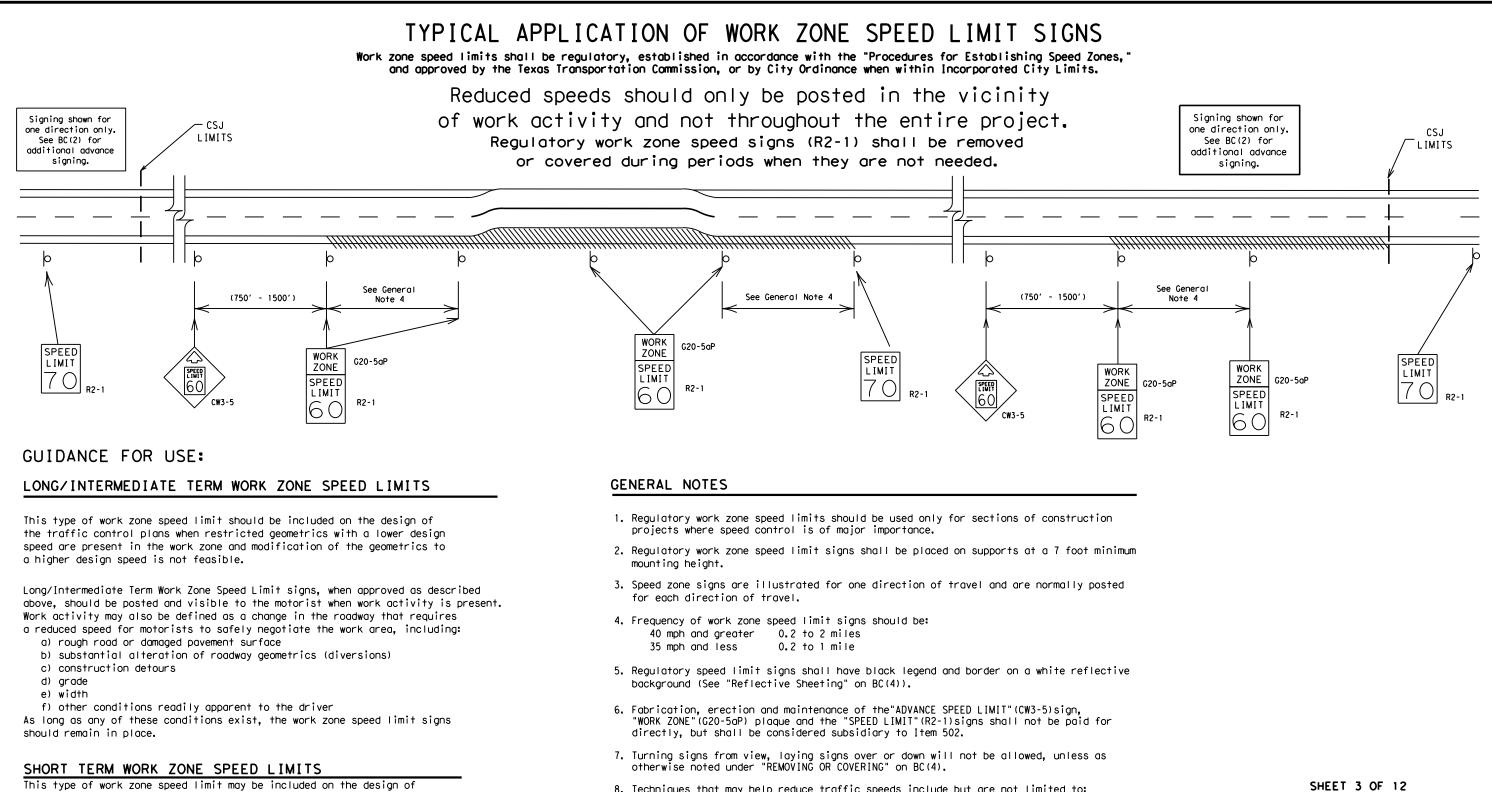
- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- 3. Distance between signs should be increased as required to have 1/2 mile or more advance warning.
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D)signs may be used on low volume crossroads at the discretion of the Engineer as per TMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.

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6. See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design sizes.

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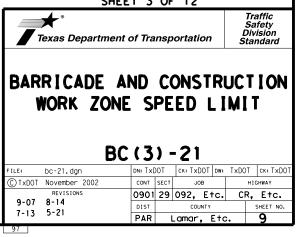
the traffic control plans when workers or equipment are not behind concrete barrier, when work activity is within 10 feet of the traveled way or actually in the traveled way.

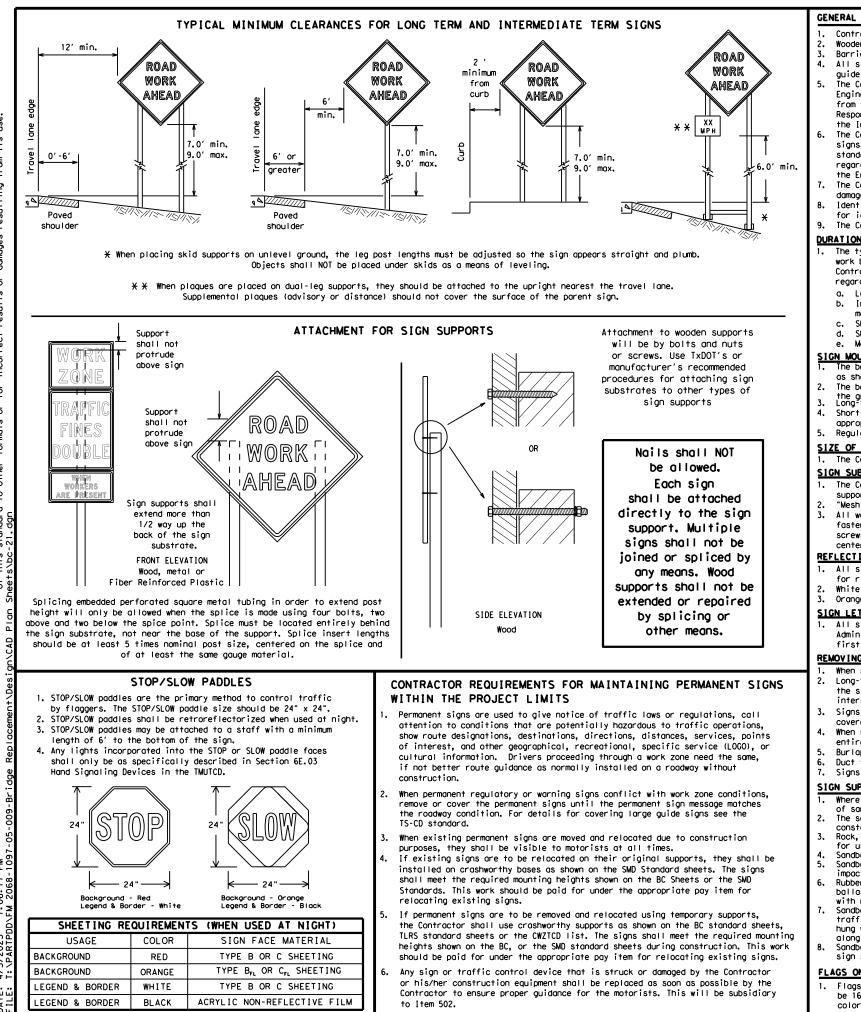
Short Term Work Zone Speed Limit signs should be posted and visible to the motorists only when work activity is present. When work activity is not present, signs shall be removed or covered. (See Removing or Covering on BC(4)).

- 8. Techniques that may help reduce traffic speeds include but are not limited to: A. Law enforcement.
 - B. Flagger stationed next to sign.
 - C. Portable changeable message sign (PCMS).
 - D. Low-power (drone) radar transmitter.
 - E. Speed monitor trailers or signs.
- 9. Speeds shown on details above are for illustration only. Work Zone Speed Limits should only be posted as approved for each project.
- 10. For more specific guidance concerning the type of work, work zone conditions and factors impacting allowable regulatory construction speed zone reduction see TxDOT form #1204 in the TxDOT e-form system.

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GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer. Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports
- guide the traveling public safely through the work zone.
- the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- the Engineer can verify the correct procedures are being followed.
- damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- for identification shall be 1 inch.
- The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

<u>DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)</u>

- regard to crashworthiness and duration of work requirements.
- a. Long-term stationary work that occupies a location more than 3 days.
- more than one hour.
- Short, duration work that occupies a location up to 1 hour.
- Mobile work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except as shown for supplemental plaques mounted below other signs.
- the ground. Long-term/Intermediate-term Signs may be used in Lieu of Short-term/Short Duration signing.
- Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to
- appropriate Long-term/Intermediate sign height.

SIZE OF SIGNS

The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

SIGN SUBSTRATES

- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave. centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- 1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300

SIGN LETTERS

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- intersections where the sign may be seen from approaching traffic. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely
- covered when not required.
- entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting. Burlap shall NOT be used to cover signs.
- Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a
- constant weight. Rock, concrete, iron, steel or other solid objects shall not be permitted
- for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used.
- Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list.
- Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

No warranty of any for the conversion m its use. Texas Engineering Practice Act". TxDDT assumes no responsibility t results or damages resulting fro of this standard is governed by the "Te by TxDOT for any purpose whatsoever. dard to other formats or for incorrect ISCLAIM The ind is f this

All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and

The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in

The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZICD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a guestion regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so

The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or

Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used

The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in

Intermediate-term stationary - work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting

Short-term stationary - daytime work that occupies a location for more than 1 hour in a single daylight period.

The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above

Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZICD lists each substrate that can be used on the different types and models of sign supports. All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6"

for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1). White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL}, shall be used for rigid signs with orange backgrounds.

Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of

Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any

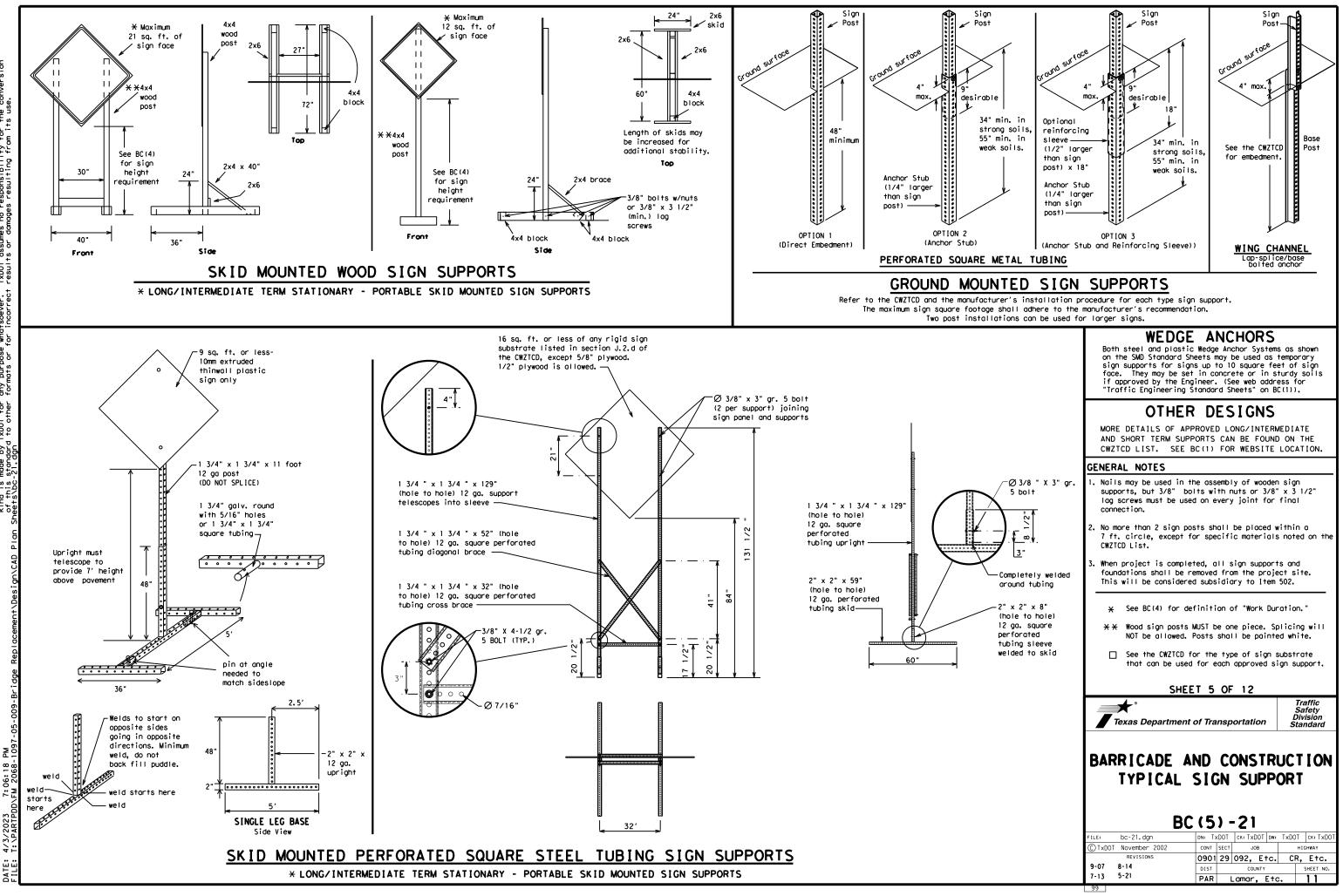
When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the

SHEET 4 OF 12

st Texas Department of Transportation Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

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WHEN NOT IN USE, REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

PORTABLE CHANGEABLE MESSAGE SIGNS

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- Messages on PCMS should contain no more than 8 words (about four to 2. eight characters per word), not including simple words such as "TO," "FOR, " "AT, " etc.
- 3. Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- 4. Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) 5. along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to 7. start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- Do not use the word "Danger" in message.
 Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together, Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- 15. PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

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RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES

(The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

Road/Lane/Ramp Closure List

FREEWAY CLOSED X MILE	FRONTAGE ROAD CLOSED	ROADWORK XXX FT	ROAD REPAIRS XXXX FT
ROAD CLOSED AT SH XXX	SHOULDER CLOSED XXX FT	FLAGGER XXXX FT	LANE NARROWS XXXX FT
ROAD CLSD AT FM XXXX	RIGHT LN CLOSED XXX FT	RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE
RIGHT X LANES CLOSED	RIGHT X LANES OPEN	MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT
CENTER LANE CLOSED	DAYTIME LANE CLOSURES	LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT
NIGHT LANE CLOSURES	I-XX SOUTH EXIT CLOSED	DETOUR X MILE	ROUGH ROAD XXXX FT
VARIOUS LANES CLOSED	EXIT XXX CLOSED X MILE	ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN
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XXXXXXXX BLVD CLOSED	₭ LANES SHIFT in Phase	1 must be used wit	h STAY IN LANE in Phos

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Action to Take/Effect on Travel List MERGE FORM RIGHT X LINES RIGHT DETOUR USE XXXXX NEXT RD EXIT X EXITS USE USE EXIT EXIT XXX I-XX NORTH STAY ON USE US XXX I-XX F SOUTH TO I-XX N TRUCKS WATCH USE FOR US XXX N TRUCKS WATCH EXPECT FOR DELAYS TRUCKS PREPARE EXPECT DELAYS ТΟ STOP REDUCE END SPEED SHOULDER XXX FT USE WATCH USE OTHER FOR ROUTES WORKERS STAY ĪΝ LANE

APPLICATION GUIDELINES

- 1. Only 1 or 2 phases are to be used on a PCMS. 2. The 1st phase (or both) should be selected from the
- "Road/Lane/Ramp Closure List" and the "Other Condition List".
- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- 4. A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- 5. If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate. 2. Roadway designations IH, US, SH, FM and LP can be interchanged as
- appropriate.
- be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- 5. ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary. 7. FT and MI. MILE and MILES interchanged as appropriate.
- 8. AT. BEFORE and PAST interchanged as needed.
- 9. Distances or AHEAD can be eliminated from the message if a
- location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

FULL MATRIX PCMS SIGNS

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under "PORTABLE CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the Engineer, it shall maintain the legibility/visibility requirement listed above
- When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and shall not substitute for, or replace that sign.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(7), for the some size arrow.

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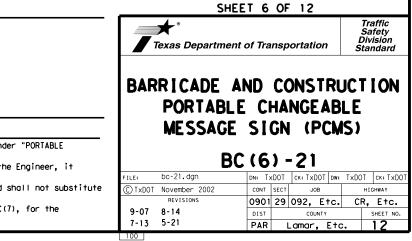
Phase 2: Possible Component Lists

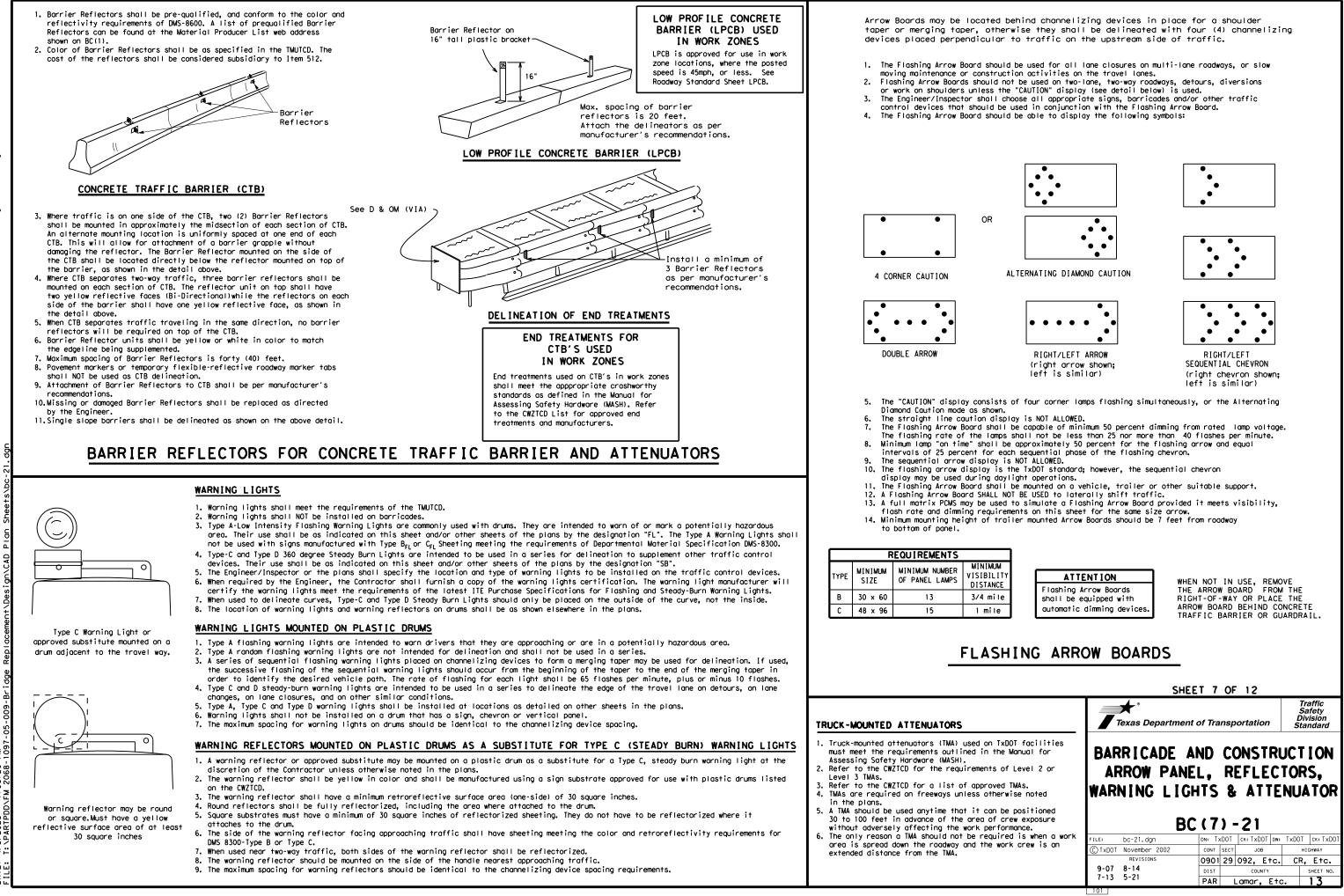


* * See Application Guidelines Note 6.

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EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can



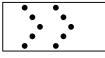


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GENERAL NOTES

- 1. For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 5. Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- 6. The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

- Pre-gualified plastic drums shall meet the following requirements:
- 1. Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- 3. Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

RETROREFLECTIVE SHEETING

- 1. The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

BALLAST

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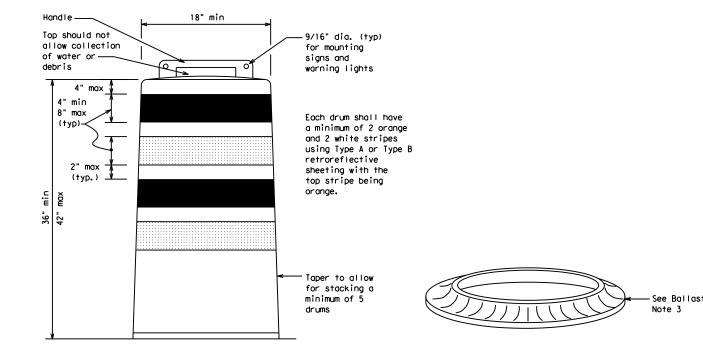
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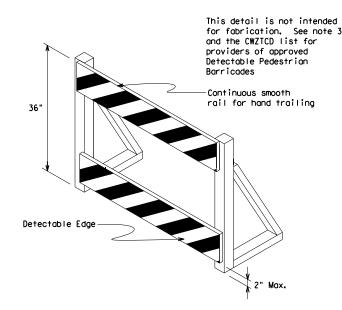
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- 1. Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- 2. Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- 5. When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.





DETECTABLE PEDESTRIAN BARRICADES

- 1. When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures. 2. Where pedestrians with visual disabilities normally use the
- closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- 3. Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian movements.
- 5, Warning lights shall not be attached to detectable pedestrian barricades.
- 6. Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.

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(Maximum Sign Dimension)

Chevron CW1-8, Opposing Traffic Lane

Divider, Driveway sign D70a, Keep Right

R4 series or other signs as approved

by Engineer



12" x 24" Vertical Panel mount with diagonals sloping down towards travel way

Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums

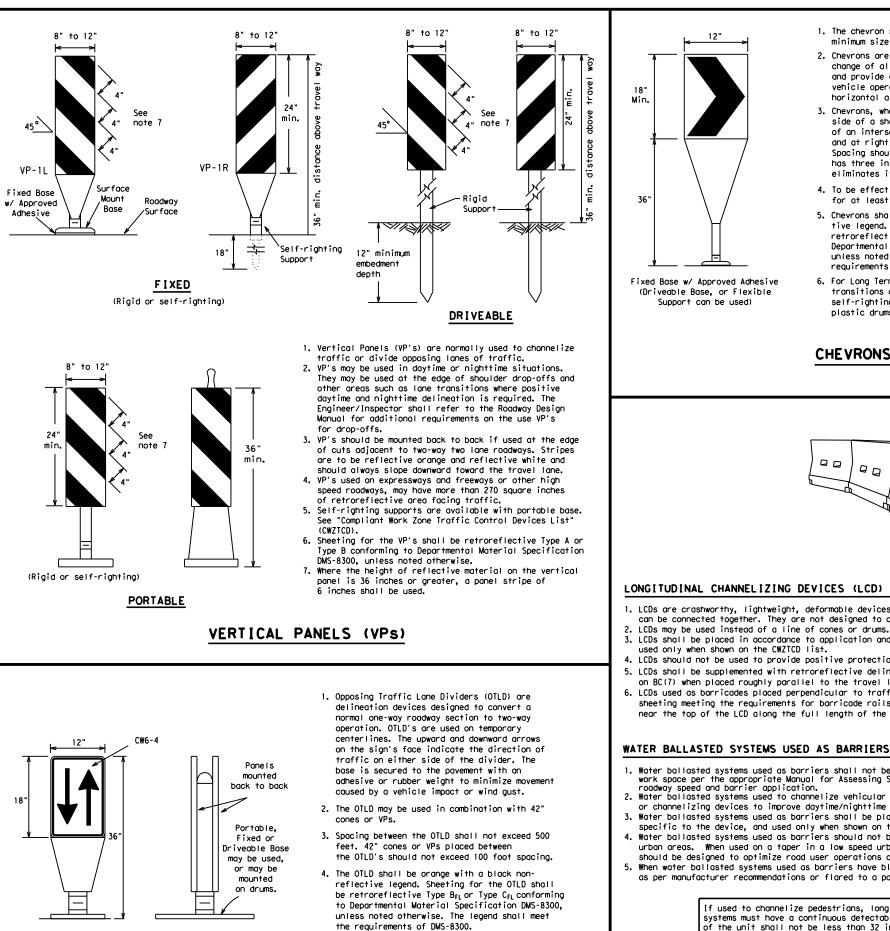
SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS

- 1. Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD.
- 2. Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL} Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
- 3. Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
- 4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
- 5. Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection.
- 6. Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts.
- 7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans.
- 8. R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.

Traffic Safety Division Standard BARR I CADE AND CONSTRUCTION CHANNEL IZING DEVICES BC (8) -21 FILE: Dc-21,dgn DNI: TXDOT CK: TXDOT DWI: TXDOT CK: TXDOT BC (8) -21 ONI: TXDOT CK: TXDOT DWI: TXDOT CK: TXDOT MICHIMAY FILE: Dc-21,dgn DNI: TXDOT CK: TXDOT DWI: TXDOT CK: TXDOT REVISIONS 4-03 8-14 O901 29 O901 29 O901 29 COUNTY SHEET NO. 4-03 8-14 O901 29 OPOT 29 COUNTY SHEET NO. 7-13 PAR Lamor, Etc. 14	SHEE	т 8	OF	12		
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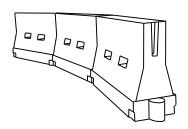
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OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

- 1. The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the out side of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type B_{FL} or Type C_{FL} conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- 6. For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS



LONGITUDINAL CHANNELIZING DEVICES (LCD)

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- 2. Water ballosted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings.
- 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.

Posted Speed	Formula	D	Minimur esirab er Lena X X	le gths	Spacin Channe	
		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent
30	2	150'	1651	180'	30′	60′
35	$L = \frac{WS^2}{60}$	205'	225'	245'	35′	70′
40	60	265'	295′	320'	40′	80′
45		450'	495′	540'	45′	90′
50		500'	550'	600'	50 <i>'</i>	100′
55	L=WS	550'	605′	660 <i>′</i>	55 <i>'</i>	110′
60	L - 11 S	600 <i>'</i>	660 <i>'</i>	720'	60 <i>'</i>	120′
65		650′	715′	780′	65 <i>'</i>	130'
70		700′	770′	840'	70′	140'
75		750'	825′	900'	75′	150'
80		800'	880′	960'	80 <i>'</i>	160'

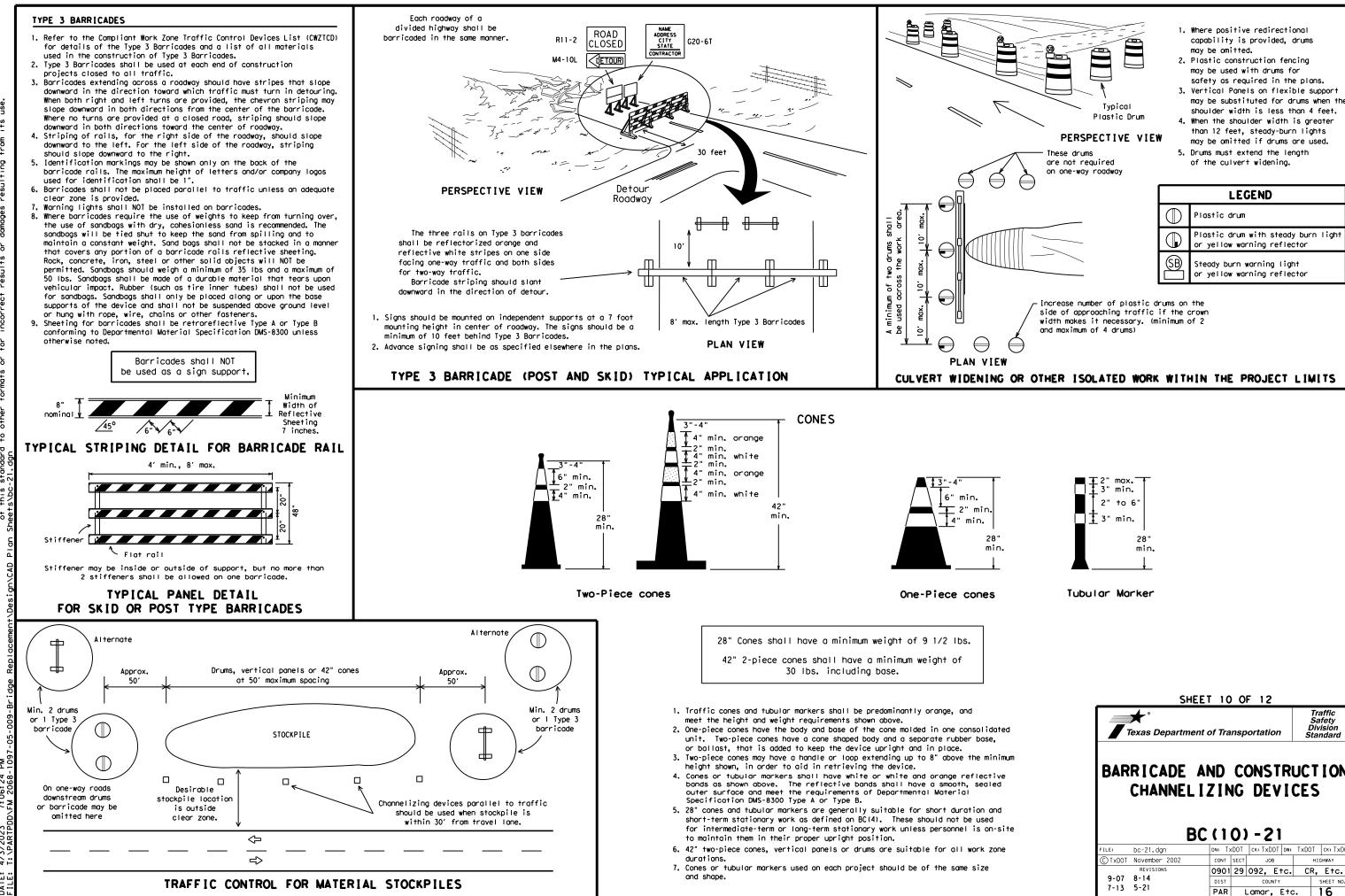
L=Length of Taper (FT.) W=Width of Offset (FT.) S=Posted Speed (MPH) SUGGESTED MAXIMUM SPACING OF

XX Taper lengths have been rounded off.

CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12 Traffic Safety Division Standard **st** Texas Department of Transportation BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

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9-07	8-14	DIST		COUNTY		SHEET NO.
7-13	5-21	PAR		Lamar, Et	с.	16

WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- 2. Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 3. Additional supplemental pavement marking details may be found in the plans or specifications.
- Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- 5. When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- 1. Raised pavement markers are to be placed according to the patterns on $\mathsf{BC}(\mathsf{12})$.
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

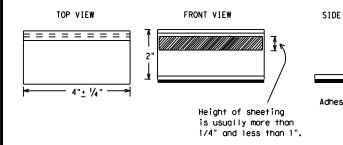
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- 8. Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10.Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECU TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARK TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guiden shall meet the requirements of DMS-8242.
- Tabs detailed on this sheet are to be inspected and accepted by Engineer or designated representative. Sampling and testing is r normally required, however at the option of the Engineer, either or "B" below may be imposed to assure quality before placement or roadway.
 - A. Select five (5) or more tabs at random from each lot or st and submit to the Construction Division, Materials and Pay Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix (5) tabs at 24 inch intervals on an asphaltic pavement in straight line. Using a medium size passenger vehicle or pi run over the markers with the front and rear tires at a sp of 35 to 40 miles per hour, four (4) times in each directi more than one (1) out of the five (5) reflective surfaces be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARK

- Raised pavement markers used as guidemarks shall be from the approduct list, and meet the requirements of DMS-4200.
- All temporary construction raised pavement markers provided on project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applie butyl rubber pad for all surfaces, or thermoplastic for concresurfaces.

Guidemarks shall be designated as:

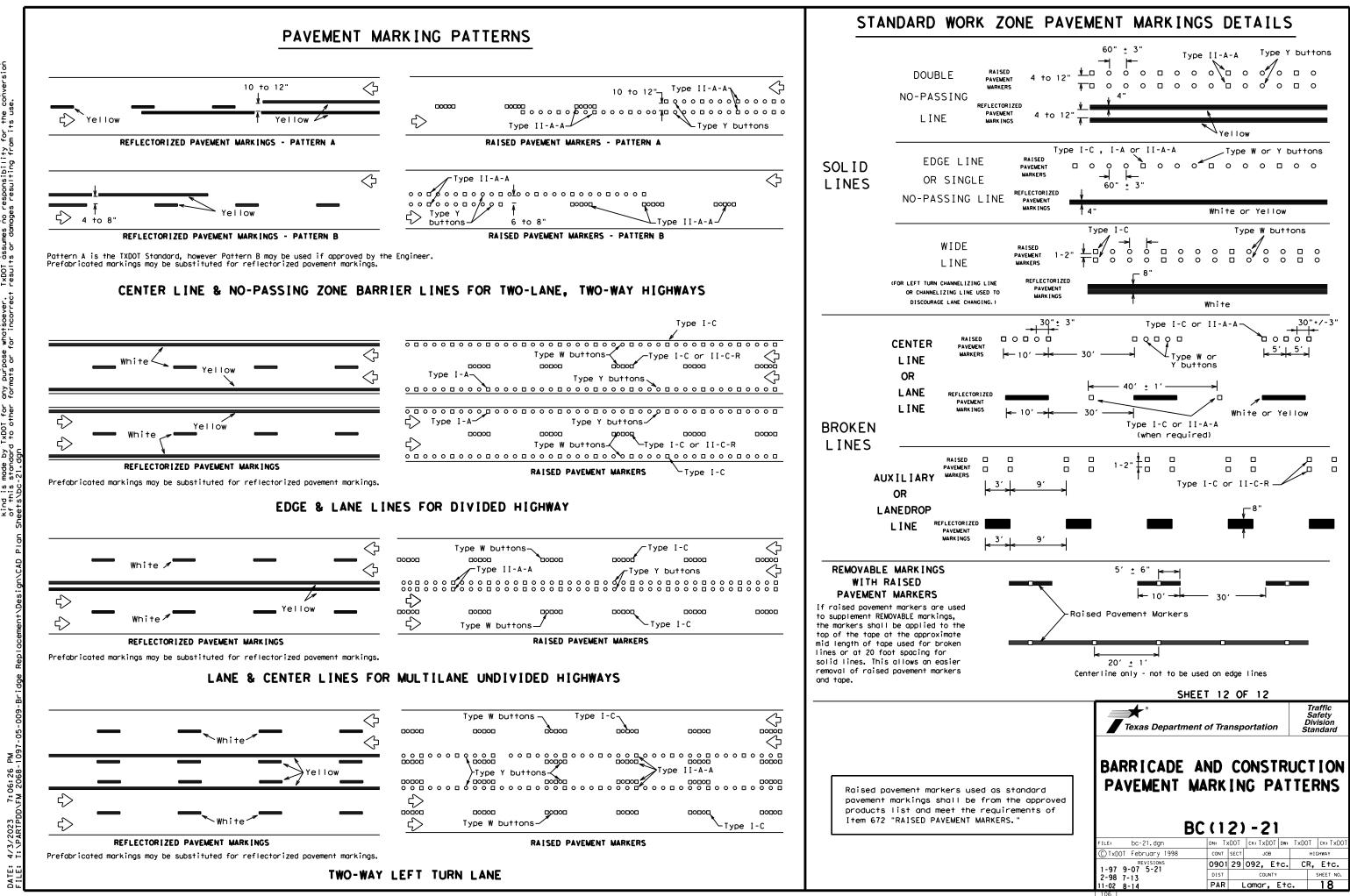
YELLOW - (two amber reflective surfaces with yellow body). WHITE - (one silver reflective surface with white body).

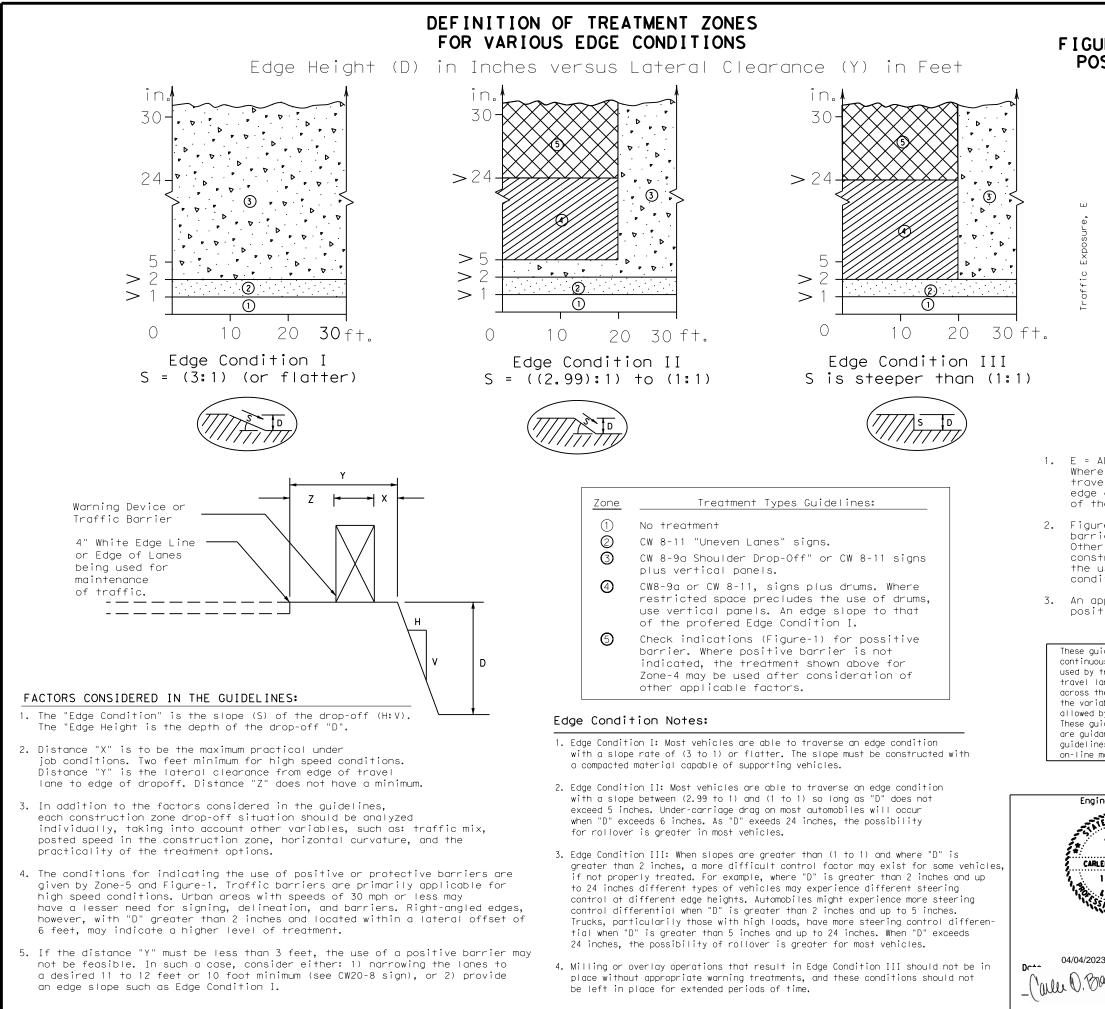
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	DEPARTMENTAL MATERIAL SPECIFICA	TIONS
	PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
		DMS-4300
w I-	EPOXY AND ADHESIVES BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6100 DMS-6130
	PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8130
	TEMPORARY REMOVABLE, PREFABRICATED	
	PAVEMENT MARKINGS	DMS-8241
	TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242
r F	list of prequalified reflective raised paveme on-reflective traffic buttons, roadway marker avement markings can be found at the Material eb address shown on BC(1).	tabs and othe
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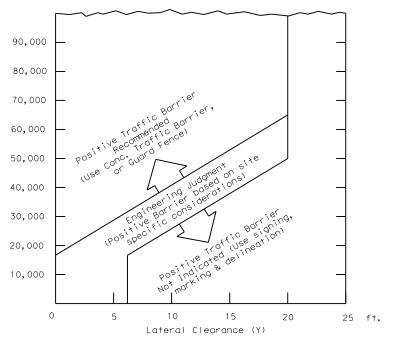


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2. Figure-1 provides a practical approach to the use of positive barriers for the protection of vehicles from pavement drop-offs. Other factors, such as the presence of heavy machinery, construction workers, or the mix and volume of traffic may make the use of positive barriers appropriate, even when the edge condition alone may not justify the use of a barrier. 3. An approved end treatment should be provided for any positive barrier end located within the clear zone.

These guidelines apply to temporary traffic control areas or work zones where continuous pavement edges or drop-offs exists parallel and adjacent to a lane used by traffic. The edge conditions may be present between shoulders and travel lanes, between adjacent or opposing travel lanes, at intermediate points across the width of the paved surface, or at the edge of pavement. Due to the variability in construction operations, tolerances in the variables may be allowed by the engineer. These guidelines do not apply to short term operations. These guidelines do not constitute a rigid standard or policy; rather, they are quidance to be used in conjunction with engineering judgement. These guidelines may be updated on the Design Division's on-line manuals.

FIGURE-1: CONDITIONS INDICATING USE OF POSITIVE BARRIER FOR ZONE 5 (I I)

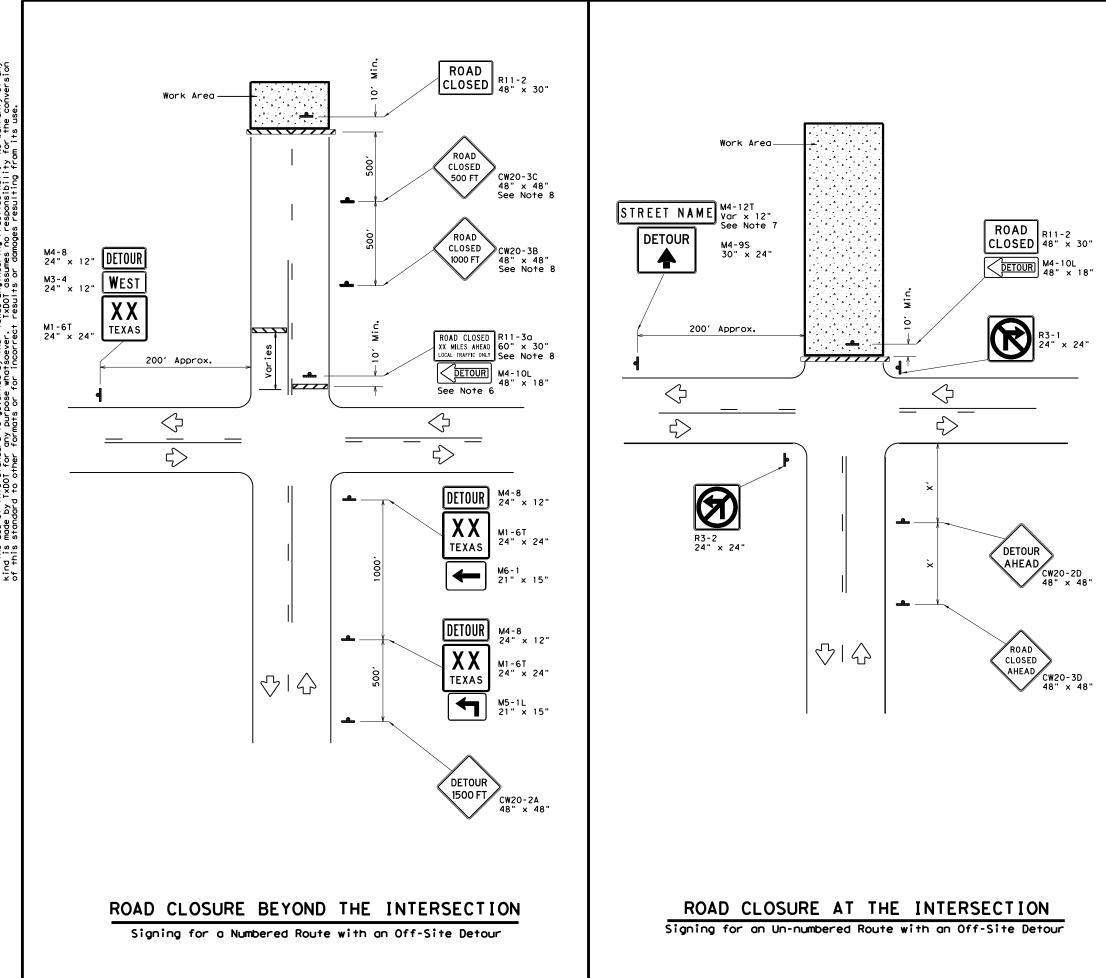


1. $E = ADT \times T$

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Where ADT is that portion of the average daily traffic volume traveling within 20 feet (generally two adjacent lanes) of the edge dropoff condition; and, T is the duration time in years of the dropoff condition.

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LEGEND					
<u>~ ~ ~ ~ ~</u>	Type 3 Barricade				
4	Sign				

Posted Speed X	Minimum Sign Spacing "X" Distance
30	120'
35	160′
40	240′
45	320′
50	400′
55	500'
60	600′
65	700′
70	800′
75	900'

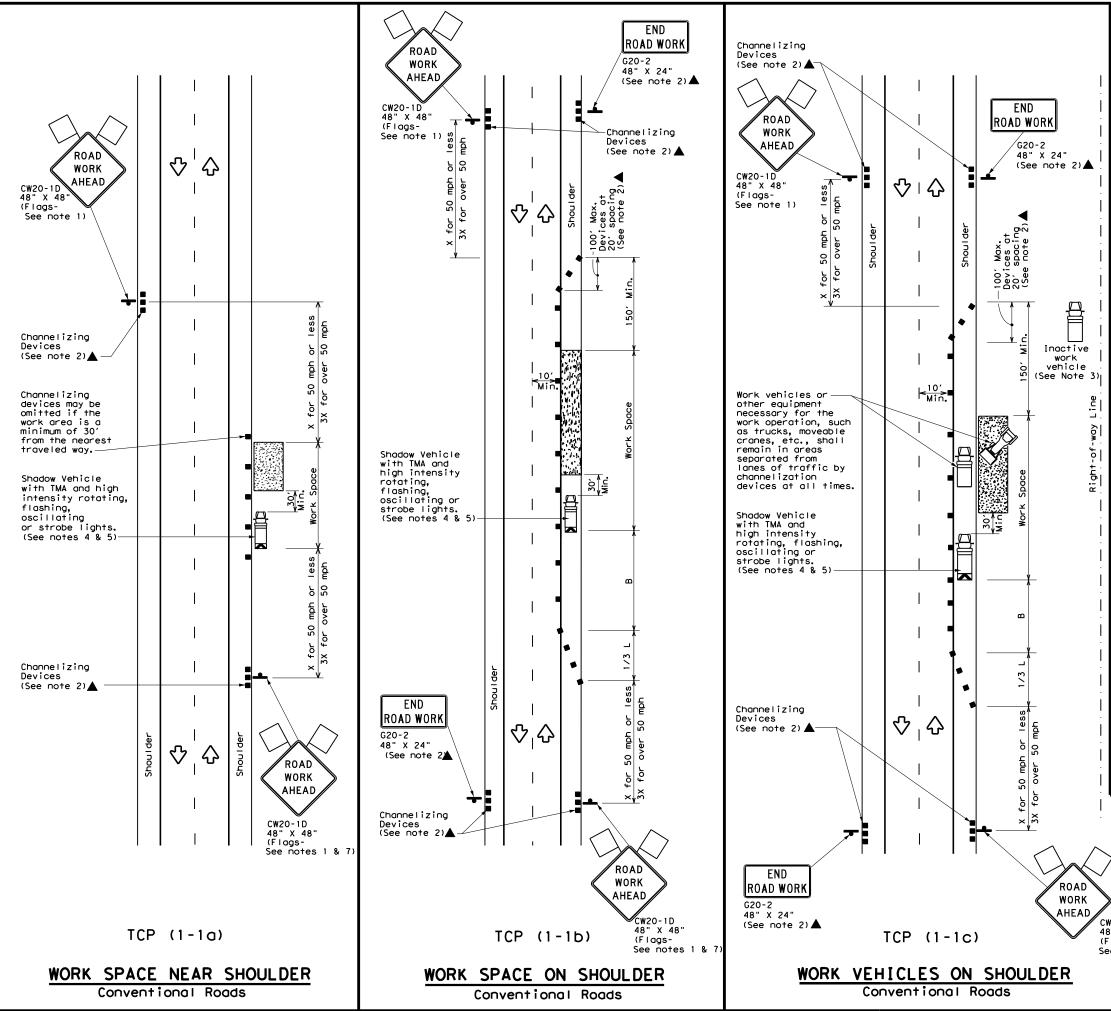
* Conventional Roads Only

GENERAL NOTES

- 1. This sheet is intended to provide details for temporary work zone road closures. For permanent road closure details see the D&OM standards.
- 2. Barricades used shall meet the requirements shown on Barricade and Construction Standard BC(10) and listed on the Compliant Work Zone Traffic Control Devices list (CWZTCD).
- 3. Stockpiled materials shall not be placed on the traffic side of barricades.
- 4. Barricades at the road closure should extend from pavement edge to pavement edge.
- 5. Detour signing shown is intended to illustrate the type of signing that is appropriate for numbered routes or un-numbered routes as labeled. It does not indicate the full extent of detour signing required. Detour routes should be signed as shown elsewhere in the plans.
- 6. If the road is open for a significant distance beyond the intersection or there are significant origin/destination points beyond the intersection, the signs and barricades at this location should be located at the edge of the traveled way.
- 7. The Street Name (M4-12T) sign is to be placed above the DETOUR (M4-9S) sign.
- 8. For urban areas where there is a shorter distance between the intersection and the actual closure location, the ROAD CLOSED XX MILES AHEAD (R11-3a) sign may be replaced with a ROAD CLOSED TO THRU TRAFFIC (R11-4) sign. If adequate space does not exist between the intersection and the closure a single ROAD CLOSED AHEAD (CW20-3D) sign spaced as per the table above may replace the ROAD CLOSED 1000 FT (CW20-3B) and ROAD CLOSED 500 FT (CW20-3C) signs.
- 9. Signs and barricades shown shall be subsidiary to Item 502. Locations where these details will be required shall be as shown elsewhere in the plans.

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	LEGEND					
<u>e 7 7 7 8</u>	Type 3 Barricade		Channelizing Devices			
₽	Heavy Work Vehicle	K	Truck Mounted Attenuator (TMA)			
	Trailer Mounted Flashing Arrow Board		Portable Changeable Message Sign (PCMS)			
4	Sign	2	Traffic Flow			
$\langle \rangle$	Flag	۵ ₀	Flagger			

Speed	Formula	D	Minimur esirab er Lena X X	le	Špacir Channe		Minimum Sign Spacing "x"	Suggested Longitudina। Buffer Space
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"
30		150'	165′	180'	30′	60'	120'	90'
35	$L = \frac{WS^2}{60}$	205'	225′	245'	35′	70′	160'	120′
40	60	265′	295'	320'	40′	80′	240'	155′
45		450'	495′	540′	45′	90′	320′	195′
50		500'	550ʻ	600′	50 <i>'</i>	100'	400′	240′
55	L=WS	550'	605 <i>'</i>	660 <i>'</i>	55′	110'	500 <i>'</i>	295′
60	L - # 5	600 <i>'</i>	660'	720'	60′	120'	600 <i>'</i>	350′
65		650 <i>'</i>	715′	780′	65 <i>'</i>	130'	700′	410′
70		700'	770'	840 <i>'</i>	70'	140'	800'	475′
75		750'	825′	900′	75′	150'	900′	540′

* Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

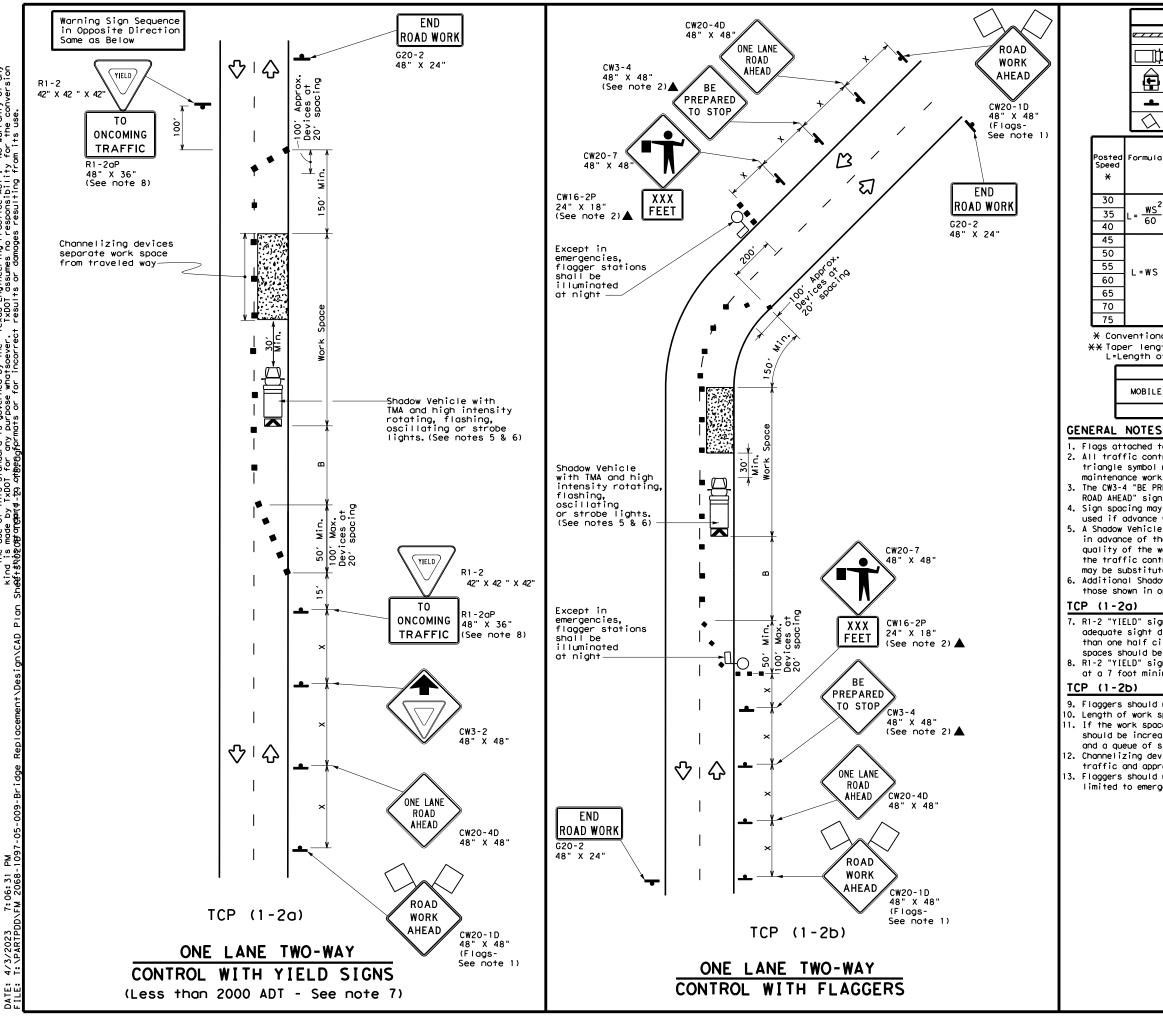
		TYPICAL U	JSAGE	
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY
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GENERAL NOTES

1. Flags attached to signs where shown are REQUIRED.

- All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved by the Engineer.
- Inactive work vehicles or other equipment should be parked near the right-of-way line and not parked on the paved shoulder.
- 4. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.
- Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect wider work spaces.
 See TCP(5-1) for shoulder work on divided highways, expressways and
- freeways. 7. CW21-5 "SHOULDER WORK" signs may be used in place of CW20-1D
- "ROAD WORK AHEAD" signs for shoulder work on conventional roadways.

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Formula	D	Minimur esirab er Len X X	le	Spac S Channe	ed Maxim ing of elizing vices	um	Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space	Stopping Sight Distance
	10' Offset	11' Offset	12' Offset	On a Taper	On a Tangen	+	Distance	"В"	
2	150'	165′	180'	30′	60'		120′	90′	200′
$L = \frac{WS^2}{60}$	205'	225'	245'	35′	70'		160'	120'	250 <i>'</i>
60	265'	295'	320'	40'	80'		240'	155'	305′
	450 <i>'</i>	495′	540'	45′	90'		320'	195'	360'
	500'	550ʻ	600'	50'	100'		400′	240'	425'
L=₩S	550'	605 <i>'</i>	660'	55'	110'		500 <i>'</i>	295'	495′
- "3	600'	660′	720'	60′	120'		600 <i>'</i>	350'	570'
	650 <i>'</i>	715′	780′	65′	130'		700′	410′	645′
	700′	770'	840'	70'	140'		800′	475′	730'
	750'	825′	900'	75'	150'		900′	540'	820'

X Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

		TYPICAL L	ISAGE	
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY
	1	1		

1. Flags attached to signs where shown are REQUIRED.

2, All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved by the Engineer.

3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4D "ONE LANE ROAD AHEAD" sign, but proper sign spacing shall be maintained.

4. Sign spacing may be increased or an additional CW20-1D "ROAD WORK AHEAD" sign may be used if advance warning ahead of the flagger or R1-2 "YIELD" sign is less than 1500 feet. 5. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.

6. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect wider work spaces.

 R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work spaces should be no longer than one half city block. In rural areas on roadways with less than 2000 ADT, work spaces should be no longer than 400 feet.

8. R1-2 "YIELD" sign with R1-20P "TO ONCOMING TRAFFIC" plaque shall be placed on a support at a 7 foot minimum mounting height.

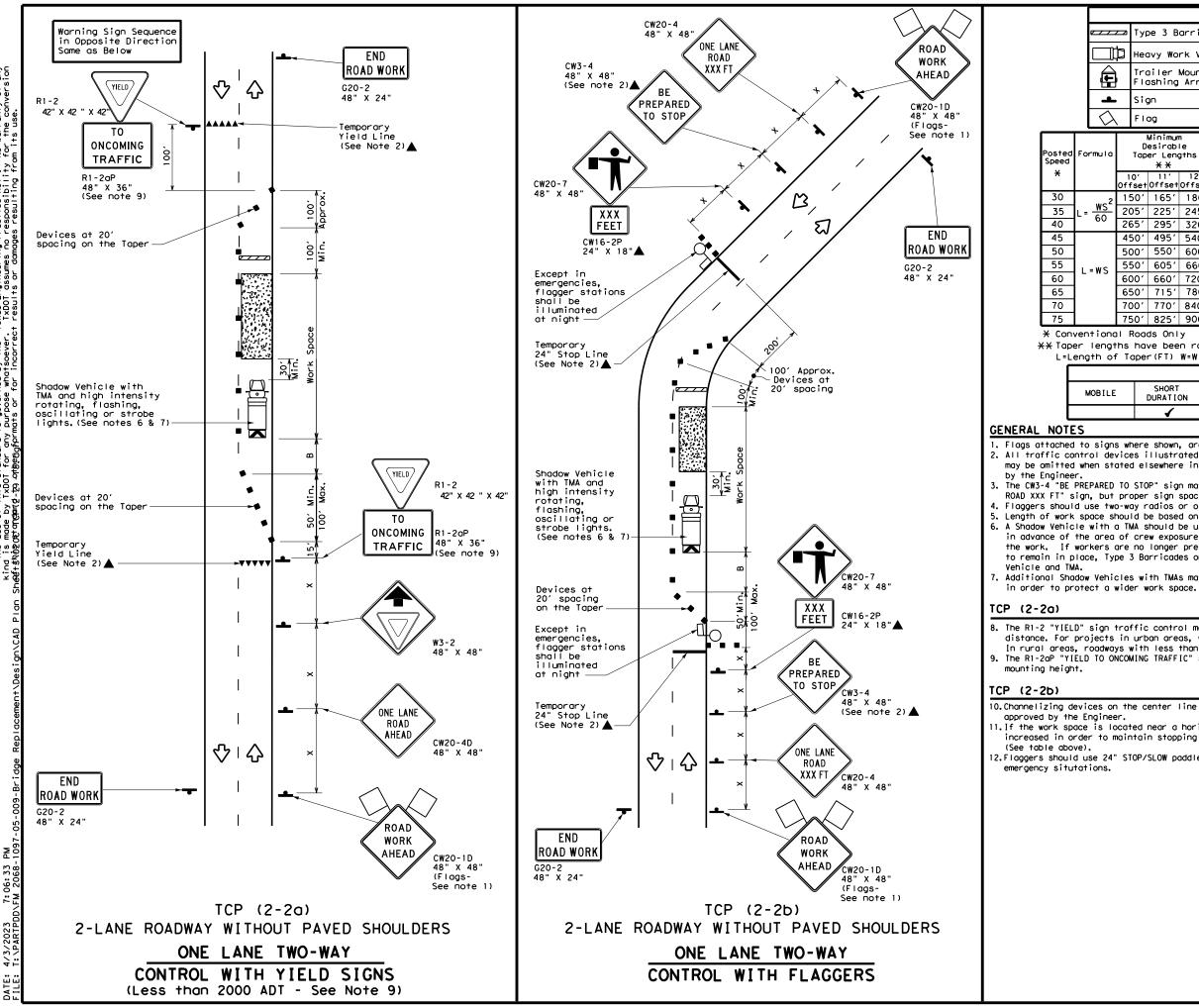
9. Flaggers should use two-way radios or other methods of communication to control traffic. 10. Length of work space should be based on the ability of flaggers to communicate. 11. If the work space is located near a horizontal or vertical curve, the buffer distances

should be increased in order to maintain adequate stopping sight distance to the flagger and a queue of stopped vehicles (see table above).

12. Channelizing devices on the center-line may be omitted when a pilot car is leading traffic and approved by the Engineer.

3. Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to emergency situations.

TRAFFIC CONTROL PLAN ONE-LANE TWO-WAY TRAFFIC CONTROL TRAFFIC CONTROL TCP (1-2) - 18 FILE: tcp1-2-18. dgn DN: CK: © TXDOT December 1985 CONT SECT JOB HIGHMAY 4-90 4-98 O9201 29 O92. Etc. CR. Etc. CR. Etc. 1-97 2-18 PAR Lamor, Etc. 20B	Texas Department	of Tra	nsp	ortati	on	Ор L	Traffic perations Division tandard
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	1-97 2-18	PAR	L	.amar,	, Etc		20B



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2		D	Minimum esirabl er Leng X X	le			'n	Minimum Sign Spacing "x"	Suggested Longitudinal Buffer Space	Stopping Sight Distance
		0' set	11' Offset	12' Offset	On a Taper	On a Tangen	t	Distance	"B"	
2	15	50'	165'	180′	30′	60′		120'	90'	200'
-	20)51	225′	245'	35′	70′		160'	120'	250 <i>'</i>
	26	551	295′	320'	40'	80′		240′	1551	305′
	45	50'	495′	540'	45'	90′		320′	195′	360′
	50)0ʻ	550'	600′	50 <i>'</i>	100′		400′	240′	425′
	55	50'	605′	660 <i>'</i>	55 <i>'</i>	110′		500 <i>'</i>	295 <i>'</i>	495′
	60)0 <i>'</i>	660'	720′	60′	120′		600′	350'	570′
	65	50'	715′	780′	65 <i>'</i>	130'		700′	410′	645′
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	75	601	825'	900'	75'	150′		900'	540 <i>′</i>	820′

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

		TYPICAL U	ISAGE	
E	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY
	1	√	4	

1. Flags attached to signs where shown, are REQUIRED. 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved

3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4 "ONE LANE ROAD XXX FT" sign, but proper sign spacing shall be maintained. 4. Flaggers should use two-way radios or other methods of communication to control traffic. 5. Length of work space should be based on the ability of flaggers to communicate. 6. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow

7. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown

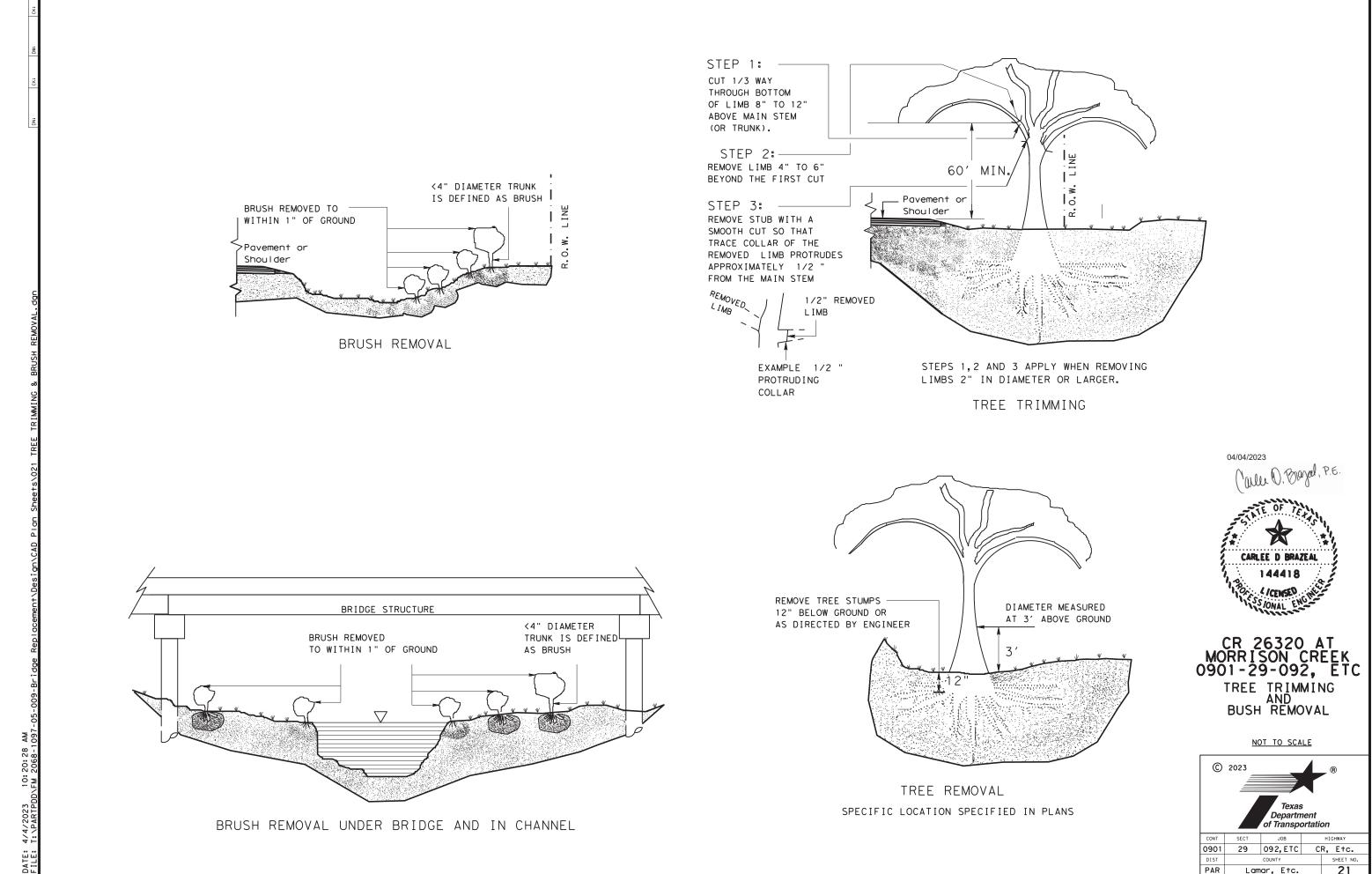
8. The R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work space should be no longer than one half city block. In rural areas, roadways with less than 2000 ADT, work space should be no longer than 400 feet. 9. The R1-2aP "YIELD TO ONCOMING TRAFFIC" sign shall be placed on a support at a 7 foot minimum

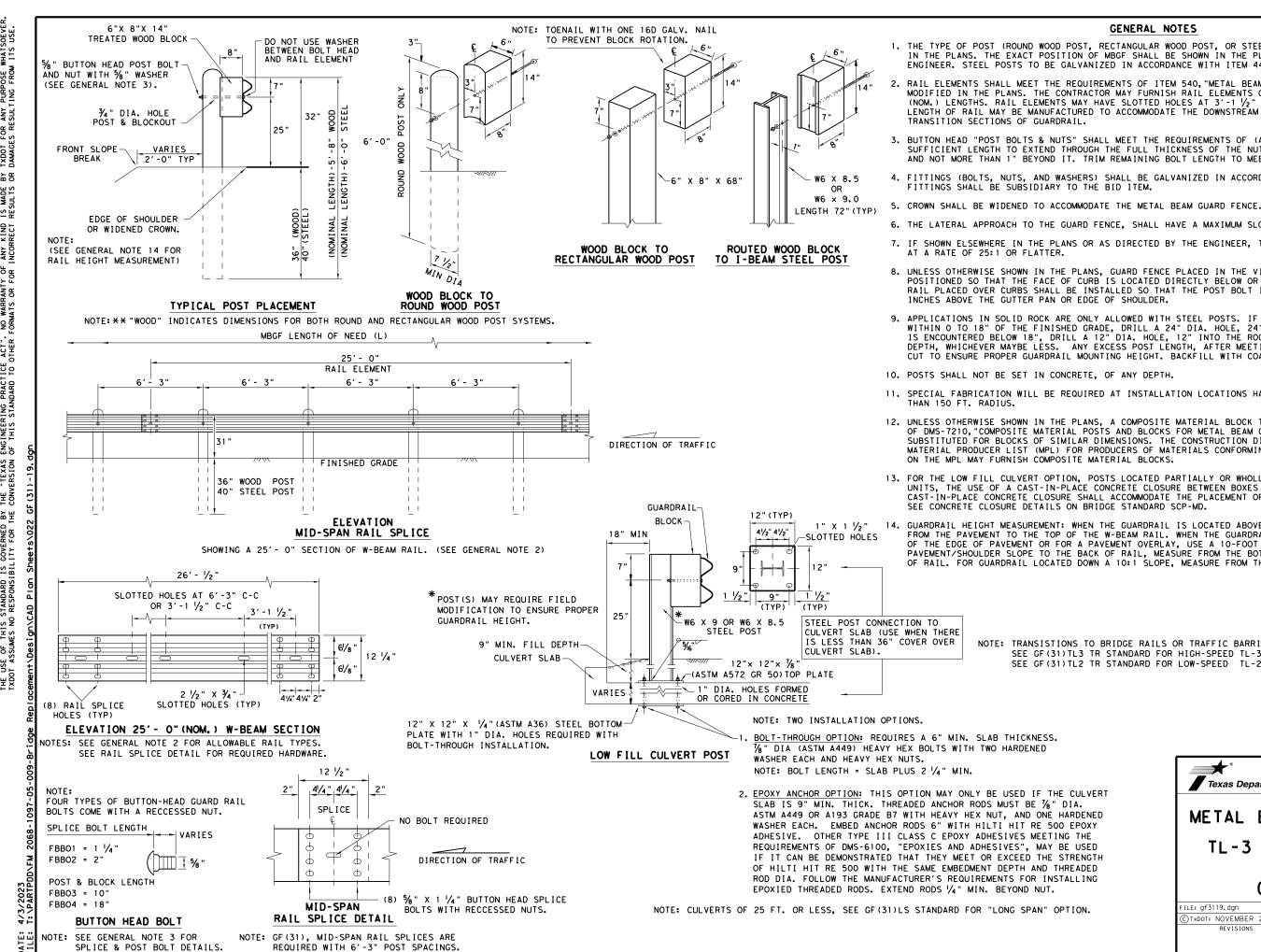
10.Channelizing devices on the center line may be omitted when a pilot car is leading traffic and

11. If the work space is located near a horizontal or vertical curve, the buffer distances should be increased in order to maintain stopping sight distance to the flagger and a queue of stopped vehicles.

12.Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to

Texas Department	t of Tra	nsp	ortati	on	0µ 1	Traffic perations Division tandard
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GENERAL NOTES

1. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST, OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. THE EXACT POSITION OF MBGF SHALL BE SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER, STEEL POSTS TO BE GALVANIZED IN ACCORDANCE WITH ITEM 445. "GALVANIZING.

RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED IN THE PLANS. THE CONTRACTOR MAY FURNISH RAIL ELEMENTS OF 25'- 0", OR 12'- 6" (NOM.) LENGTHS. RAIL ELEMENTS MAY HAVE SLOTTED HOLES AT $3'-1 \frac{1}{2}$ " C-C OR 6'-3" C-C. A SPECIAL LENGTH OF RAIL MAY BE MANUFACTURED TO ACCOMMODATE THE DOWNSTREAM ANCHOR TERMINAL (DAT) AND THE

3. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 5/4" WASHER (FWC16g) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING BOLT LENGTH TO MEET REQUIRED LENGTH.

4. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING. FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.

6. THE LATERAL APPROACH TO THE GUARD FENCE, SHALL HAVE A MAXIMUM SLOPE OF 1V:10H.

7. IF SHOWN ELSEWHERE IN THE PLANS OR AS DIRECTED BY THE ENGINEER, THE GUARD FENCE MAY BE FLARED

8. UNLESS OTHERWISE SHOWN IN THE PLANS. GUARD FENCE PLACED IN THE VICINITY OF CURBS SHALL BE POSITIONED SO THAT THE FACE OF CURB IS LOCATED DIRECTLY BELOW OR BEHIND THE FACE OF THE RAIL. RAIL PLACED OVER CURBS SHALL BE INSTALLED SO THAT THE POST BOLT IS LOCATED APPROXIMATELY 25

9. APPLICATIONS IN SOLID ROCK ARE ONLY ALLOWED WITH STEEL POSTS. IF SOLID ROCK IS ENCOUNTERED WITHIN 0 TO 18" OF THE FINISHED GRADE, DRILL A 24" DIA. HOLE, 24" INTO THE ROCK. IF SOLID ROCK IS ENCOUNTERED BELOW 18", DRILL A 12" DIA. HOLE, 12" INTO THE ROCK OR TO THE STANDARD EMBEDMENT DEPTH, WHICHEVER MAYBE LESS. ANY EXCESS POST LENGTH, AFTER MEETING THESE DEPTHS, MAY BE FIELD CUT TO ENSURE PROPER GUARDRAIL MOUNTING HEIGHT. BACKFILL WITH COARSE AGGREGATE MATERIAL.

11. SPECIAL FABRICATION WILL BE REQUIRED AT INSTALLATION LOCATIONS HAVING A CURVATURE OF LESS

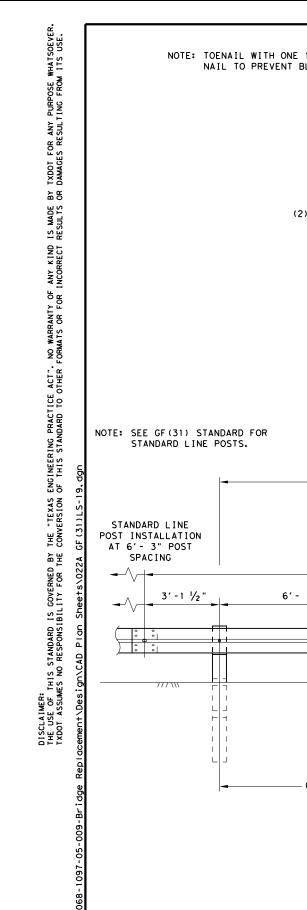
12. UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. THE CONSTRUCTION DIVISION, TXDOT MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210 ONLY PRODUCERS

13. FOR THE LOW FILL CULVERT OPTION, POSTS LOCATED PARTIALLY OR WHOLLY BETWEEN PRECAST BOX CULVERT UNITS, THE USE OF A CAST-IN-PLACE CONCRETE CLOSURE BETWEEN BOXES IS REQUIRED. THE LENGTH OF THE CAST-IN-PLACE CONCRETE CLOSURE SHALL ACCOMMODATE THE PLACEMENT OF THE LOW FILL CULVERT OPTION. SEE CONCRETE CLOSURE DETAILS ON BRIDGE STANDARD SCP-MD.

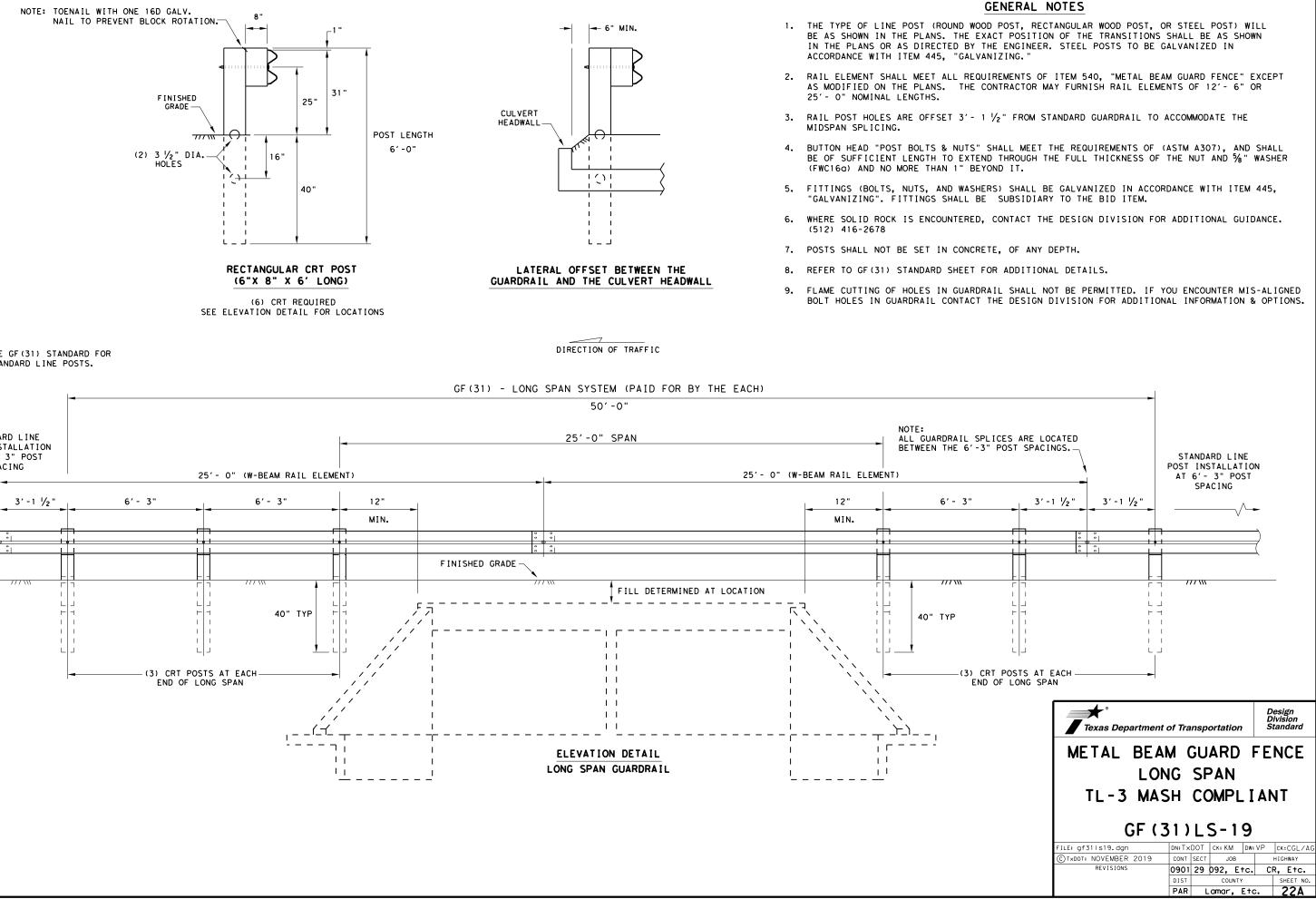
14. GUARDRAIL HEIGHT MEASUREMENT: WHEN THE GUARDRAIL IS LOCATED ABOVE PAVEMENT, MEASURE THE HEIGHT S FROM THE PAVEMENT TO THE TOP OF THE W-BEAM RAIL. WHEN THE GUARDRAIL IS LOCATED UP TO 2 FT. OFF OF THE EDGE OF PAVEMENT OR FOR A PAVEMENT OVERLAY, USE A 10-FOOT STRAIGHTEDGE TO EXTEND THE PAVEMENT/SHOULDER SLOPE TO THE BACK OF RAIL, MEASURE FROM THE BOTTOM OF STRAIGHTEDGE TO THE TOP OF RAIL. FOR GUARDRAIL LOCATED DOWN A 10:1 SLOPE, MEASURE FROM THE NOMINAL TERRAIN.

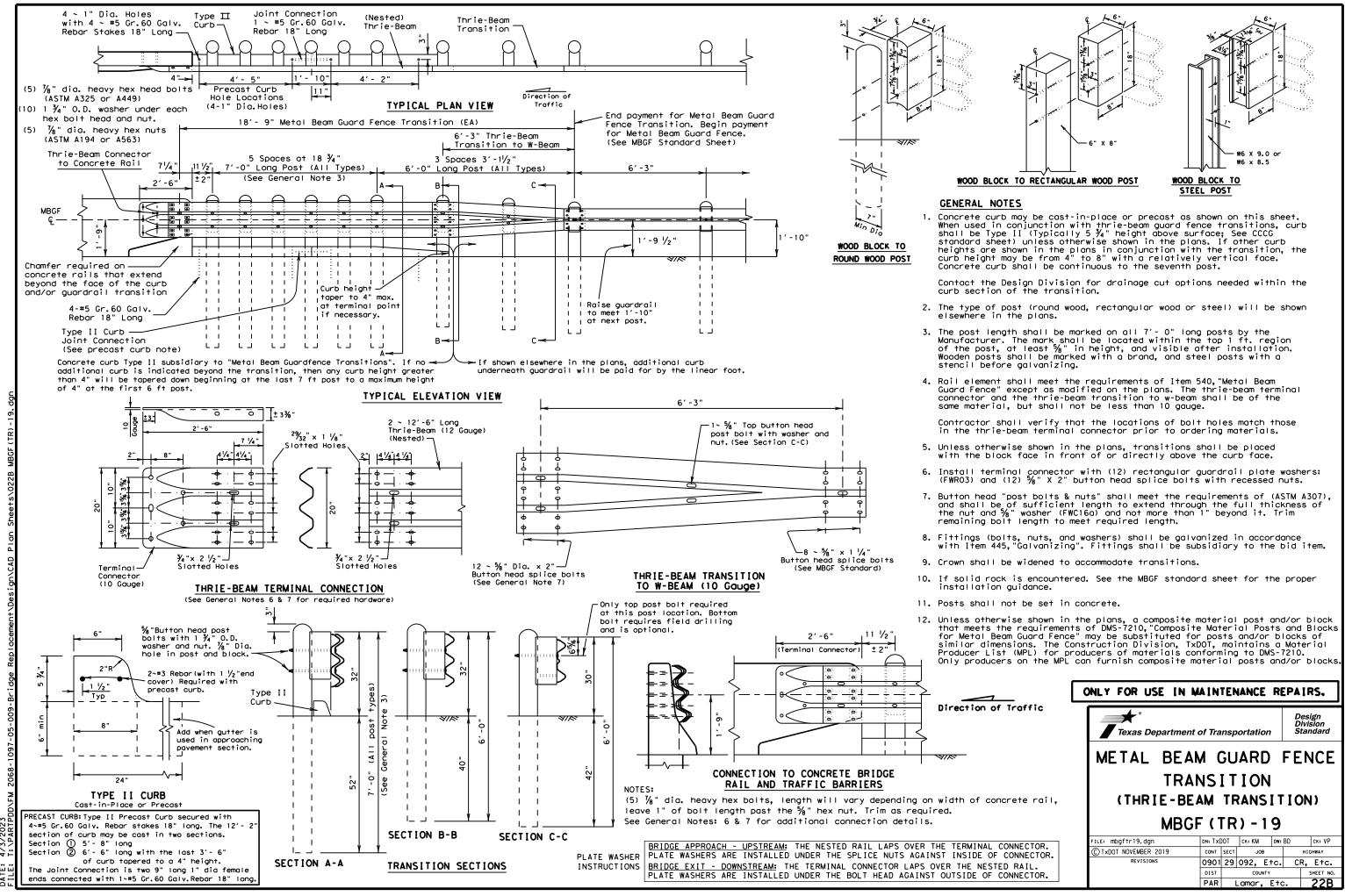
> NOTE: TRANSISTIONS TO BRIDGE RAILS OR TRAFFIC BARRIERS. SEE GF (31) TL3 TR STANDARD FOR HIGH-SPEED TL-3 TRANSITIONS. SEE GF (31) TL2 TR STANDARD FOR LOW-SPEED TL-2 TRANSITIONS.

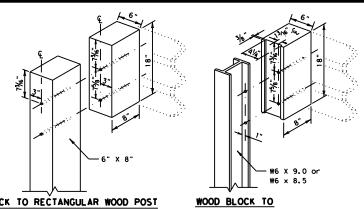




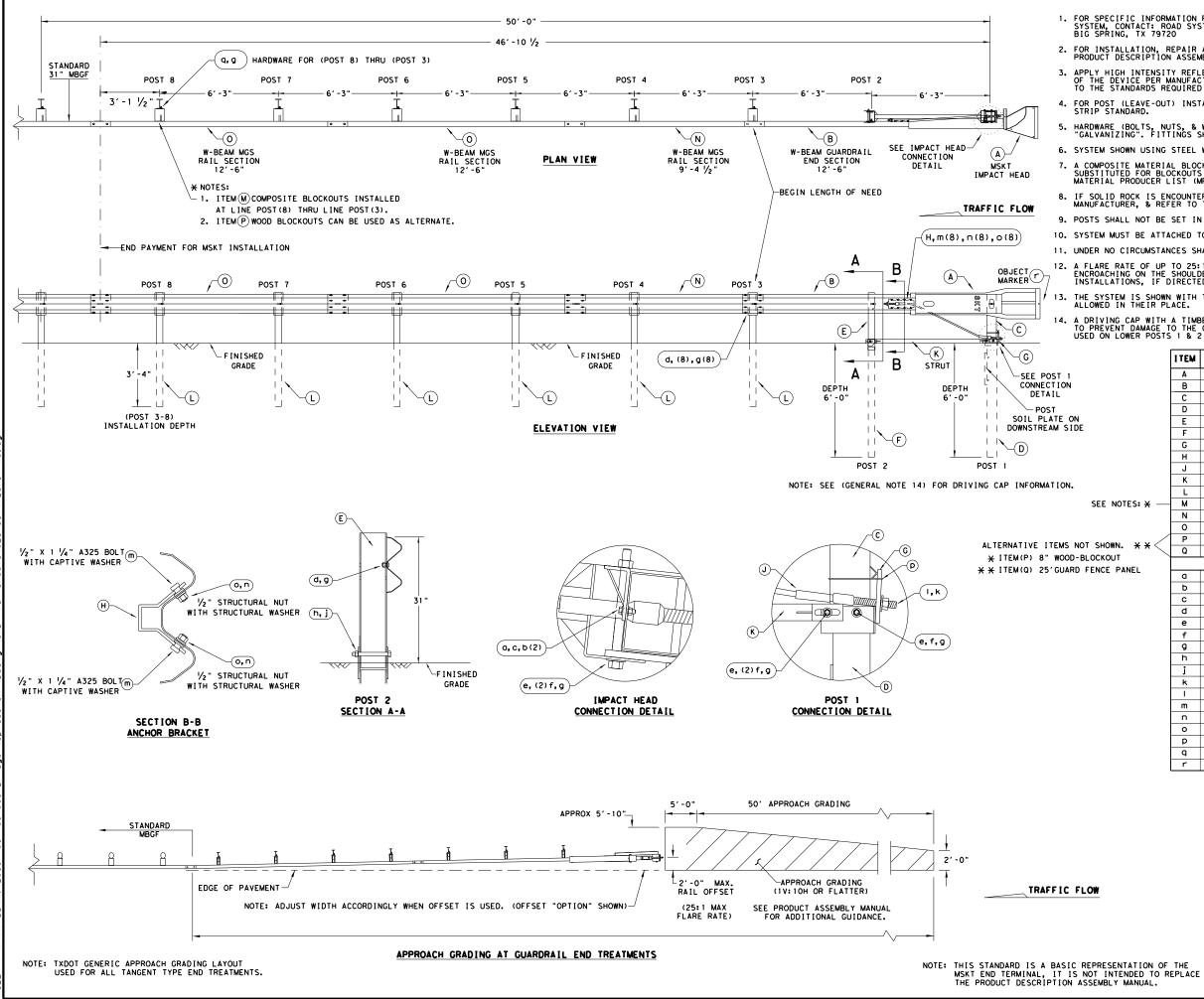
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GENERAL NOTES

FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: ROAD SYSTEMS, INC. (432)263-2435. 3616 OLD HOWARD COUNTY AIRPORT, BIG SPRING, TX 79720

2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE: MSKT END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL (PUBLICATION~062717).

3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.

FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

5. HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POSTS WITH COMPOSITE BLOCKOUTS.

7. A COMPOSITE MATERIAL BLOCKOUTS THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

8. IF SOLID ROCK IS ENCOUNTERED IN THE AREA OF (POST 1) AND / OR (POST 2) CONTACT THE MANUFACTURER, & REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE 9. POSTS SHALL NOT BE SET IN CONCRETE.

10. SYSTEM MUST BE ATTACHED TO STANDARD 31" MBGF.

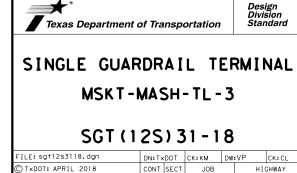
11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE MSKT SYSTEM BE CURVED.

12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.

13. THE SYSTEM IS SHOWN WITH TWO 12'-6" MBGF PANELS, ONE 25'-0" MBGF PANEL IS ALSO ALLOWED IN THEIR PLACE.

A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POSTS 3-8 TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST. SPECIAL DRIVING CAP TO BE USED ON LOWER POSTS 1 & 2 TO PREVENT DAMAGE TO THE WELDED PLATES.

	ITEM	QTY	MAIN SYSTEM COMPONENTS	I TEM NUMBERS	
	Α	1	MSKT IMPACT HEAD	MS3000	
	В	1	W-BEAM GUARDRAIL END SECTION, 12 Ga.	SF1303	
	С	1	POST 1 - TOP (6" X 6" X 1/8" TUBE)	MTPHP1A	
	D	1	POST 1 - BOTTOM (6' W6X15)	MTPHP1B	
	Е	1	POST 2 - ASSEMBLY TOP	UHP2A	
	F	1	POST 2 - ASSEMBLY BOTTOM (6' W6X9)	HP2B	
	G	1	BEARING PLATE	E750	
	н	1	CABLE ANCHOR BOX	S760	
	J	1	BCT CABLE ANCHOR ASSEMBLY	E770	
	К	1	GROUND STRUT	MS785	
	L	6	W6×9 OR W6×8.5 STEEL POST	P621	
NOTES: X	м	6	COMPOSITE BLOCKOUTS	CBSP-14	
	N	1	W-BEAM MGS RAIL SECTION (9'-4 1/2")	G12025	
	0	2	W-BEAM MGS RAIL SECTION (12'-6")	G1203A	
	Р	6	WOOD BLOCKOUT 6" X 8" X 14"	P675	
wn. ** $<$	Q	1	W-BEAM MGS RAIL SECTION (25'-0")	G1209	
UT	SMALL HARDWARE				
PANEL	a	2	5%5" x 1" HEX BOLT (GRD 5)	B5160104A	
	b	4	% " WASHER	W0516	
	c	2	5% " HEX NUT	N0516	
	d	25	% Dio. × 1 ¼ SPLICE BOLT (POST 2)	B580122	
	e	2	5% " Dig. × 9" HEX BOLT (GRD A449)	B580904A	
	f	3	5% " WASHER	W050	
	g	33	%" Dio. H.G.R NUT	N050	
	ĥ	1	3/4" Dio. × 8 1/2" HEX BOLT (GRD A449)	B340854A	
	i	1	34" Dig. HEX NUT	N030	
	k	2	1 ANCHOR CABLE HEX NUT	N100	
		2	1 ANCHOR CABLE WASHER	W100	
	m	8	$\frac{1}{2}$ " x 1 $\frac{1}{4}$ " A325 BOLT WITH CAPTIVE WASHER		
		8	1/2" STRUCTURAL NUTS	NO12A	
	0	8	$1 \frac{1}{16}$ " 0.D. x $\frac{3}{6}$ " I.D. STRUCTURAL WASHERS	W012A	
	P	0	BEARING PLATE RETAINER TIE	CT-100ST	
	q	6	% × 10" H.G.R. BOLT	B581002	
	r	1	98 X 10 H.G.R. BOLT OBJECT MARKER 18" X 18"	E3151	
	L_'	<u> </u>	ODJECI MARNER IO A IO	E3121	
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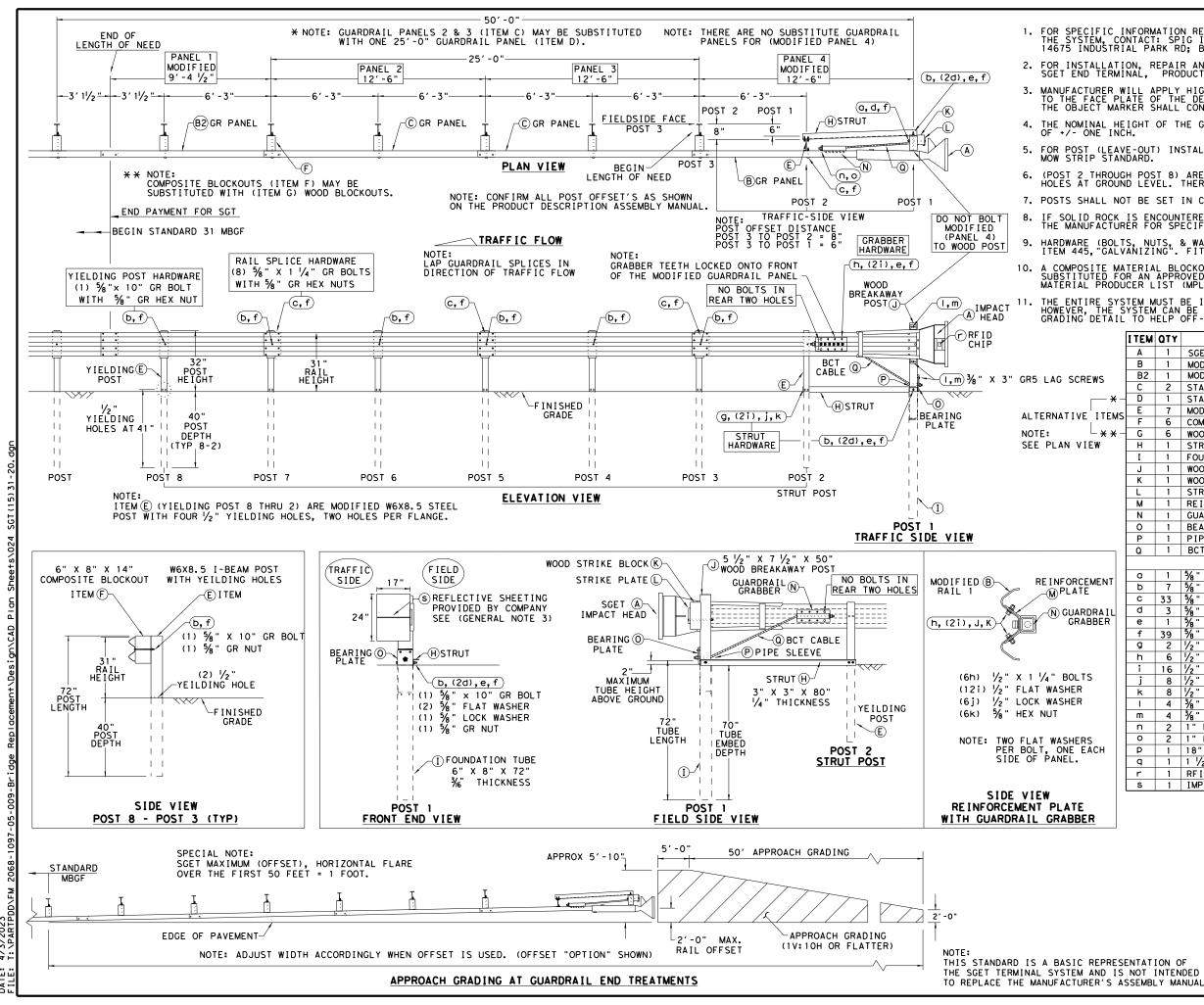
SHEET NO

23

COUNTY

PAR Lamar, Etc.

REVISIONS



DATE: FIIF:

1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202

2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL.

3. MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER' TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD. 4. THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.

5. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

6. (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS. 7. POSTS SHALL NOT BE SET IN CONCRETE.

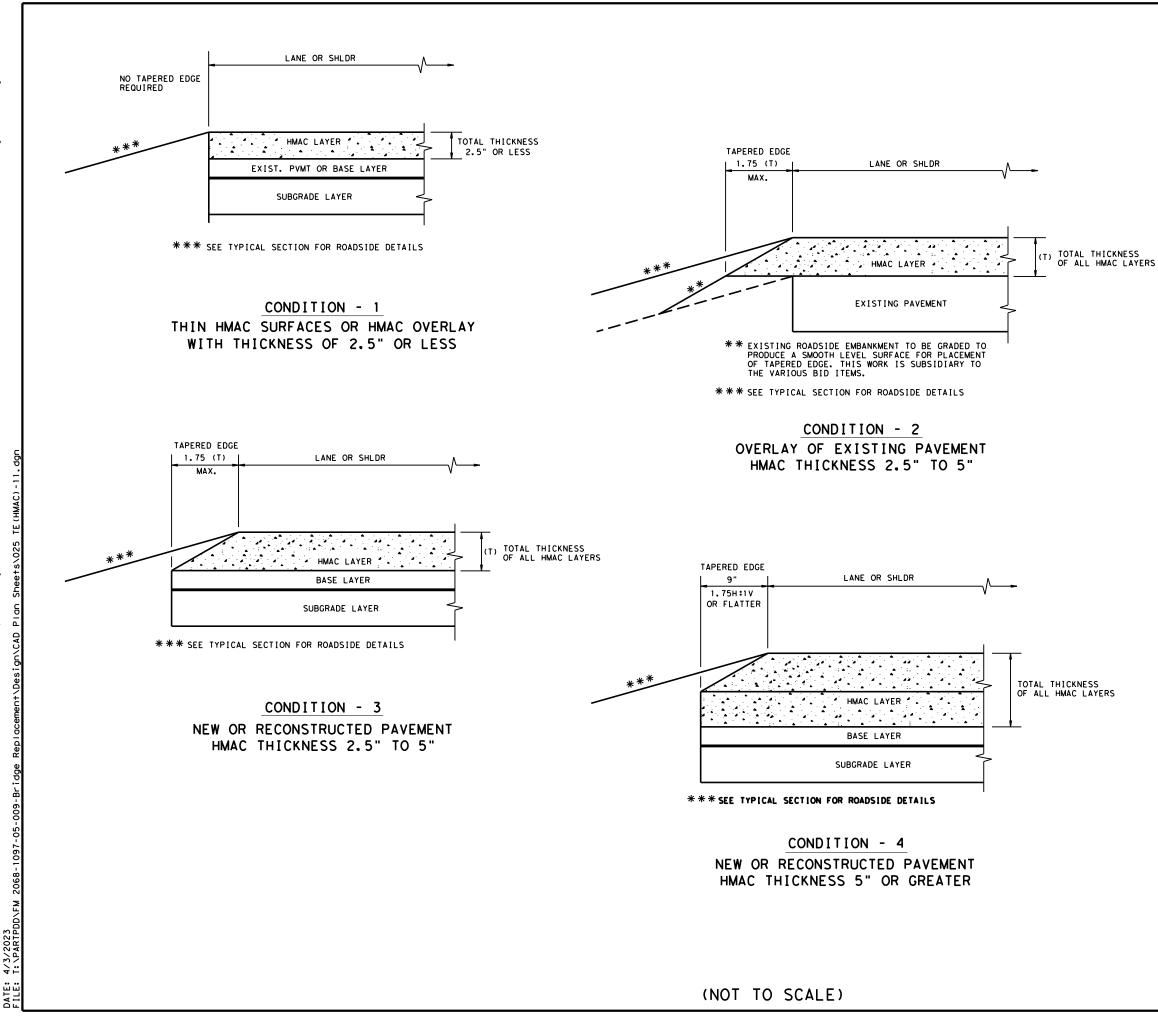
IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.

HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. 10. A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.

	ITEM	QTY	MAIN SYSTEM COMPONENTS	ITEM #
	Α	1	SGET IMPACT HEAD	SIH1A
	В	1	MODIFIED GUARDRAIL PANEL 12'-6" 12GA	126SPZGF
ľ	B2	1	MODIFIED GUARDRAIL PANEL 9'-4 1/2" 12GA	GP94
	С	2	STANDARD GUARDRAIL PANEL 12'-6" 12GA	GP126
×-	D	1	STANDARD GUARDRAIL PANEL 25'-0" 12GA	GP25
	Ε	7	MODIFIED YIELDING I-BEAM POST W6×8.5	YP6MOD
MS	F	6	COMPOSITE BLOCKOUT 6" X 8" X 14"	CB08
* -	G	6	WOOD BLOCKOUT 6" X 8" X 14"	WBO8
	Н	1	STRUT 3" X 3" X 80" x 1/4" A36 ANGLE	STR80
	I	1	FOUNDATION TUBE 6" X 8" X 72" × 3/6"	FNDT6
	J	1	WOOD BREAKAWAY POST 5 1/2" × 7 1/2" × 50"	WBRK50
ľ	К	1	WOOD STRIKE BLOCK	WSBLK14
	L	1	STRIKE PLATE 1/4" A36 BENT PLATE	SPL T8
	M	1	REINFORCEMENT PLATE 12 GA. GR55	REPLT17
	N	1	REINFORCEMENT PLATE 12 GA. GR55 GUARDRAIL GRABBER 2 $\frac{1}{2}$ " X 2 $\frac{1}{2}$ " X 16 $\frac{1}{2}$ "	GGR17
	0	1	BEARING PLATE 8" X 8 % X 5% X 436	BPLT8
	P	1	PIPE SLEEVE 4 1/4" X 2 3/8" O.D. (2 1/8" I.D.)	
_	Q	1	BCT CABLE 3/4" X 81" LENGTH	CBL81
	ŭ		SMALL HARDWARE	CDLOI
	a	1	% " X 12" GUARDRAIL BOLT 307A HDG	12GRBL T
r	b	7	% X 10" GUARDRAIL BOLT 307A HDG	10GRBLT
	c	33	% X 1 /4 GR SPLICE BOLTS 307A HDG	1 GRBL T
.	d		% " FLAT WASHER F436 A325 HDG	58FW436
L	e	3	⅓ LAT WASHER F436 A325 HDG	
	f		% GUARDRAIL HEX NUT HDG	58LW 58HN563
	g	39 2	1/2" X 2" STRUT BOLT A325 HDG	28LT
	9 h		1/2 X 2 STROT BOLT A325 HDG	
	i	6	$\frac{72}{2}$ FLAT WASHER F436 A325 HDG	125BLT
		16		12FWF436
	j	8		12LW
	k	8	½" HEX NUT A563 HDG ¾" X 3" HEX LAG SCREW GR5 HDG	12HN563
		4		38LS
	m	4	3/8" FLAT WASHER F436 A325 HDG	38FW844
	n	2	1" FLAT WASHER F436 A325 HDG	1FWF436
	0	2	1" HEX NUT A563DH HDG	1HN563
	p	1	18" TO 24" LONG ZIP TIE RATED 175-200LB	ZPT18
	q	1	1 ¹ / ₂ " X 4" SCH-40 PVC PIPE	PSPCR4
	r	1	RFID CHIP RATED MIL-STD-810F	RF ID810F
l	S	1	IMPACT HEAD REFLECTIVE SHEETING	RS30M
				Decide
				Design Division
-			Texas Department of Transportation	Standard
			SPIG INDUSTRY, LI	C
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			SINGLE GUARDRAIL TER	MINAI
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PAR Lamar, Etc. 24



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Engineering Practice Act". of this standard to other

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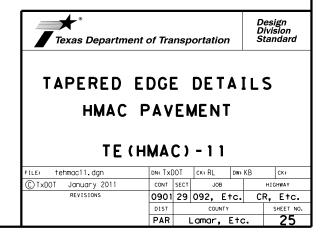
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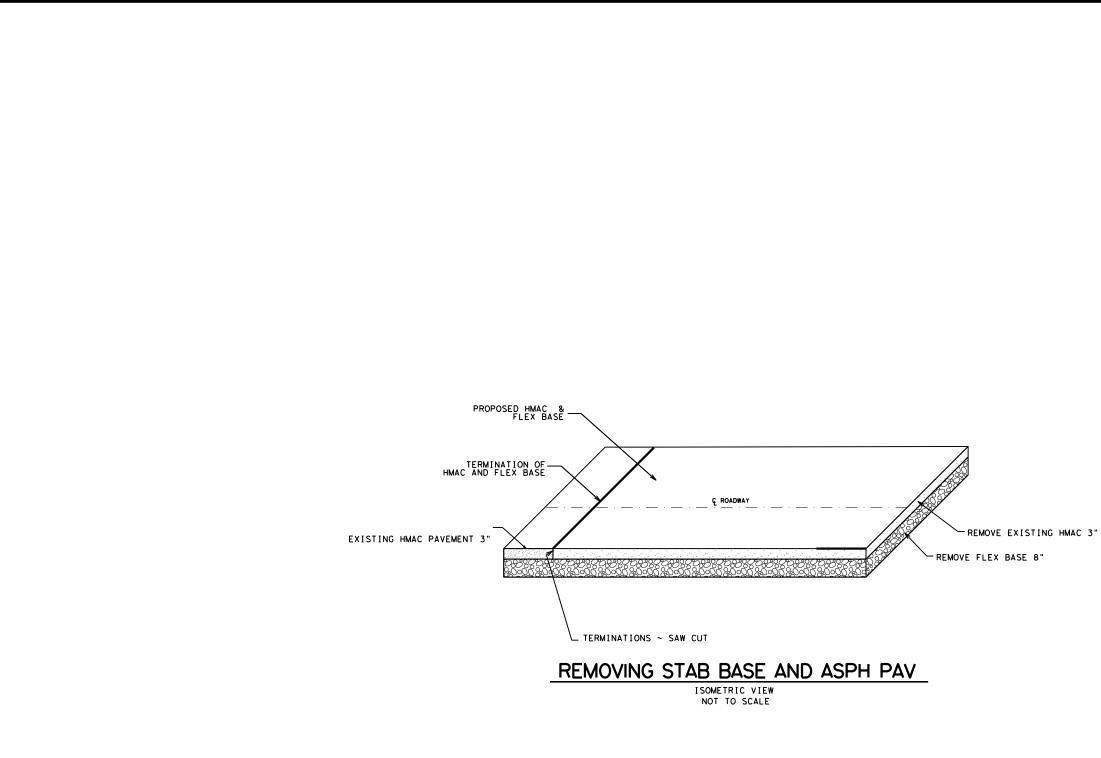
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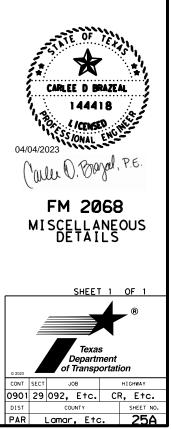
DISCLAIMER: The use of t T×DOT assume

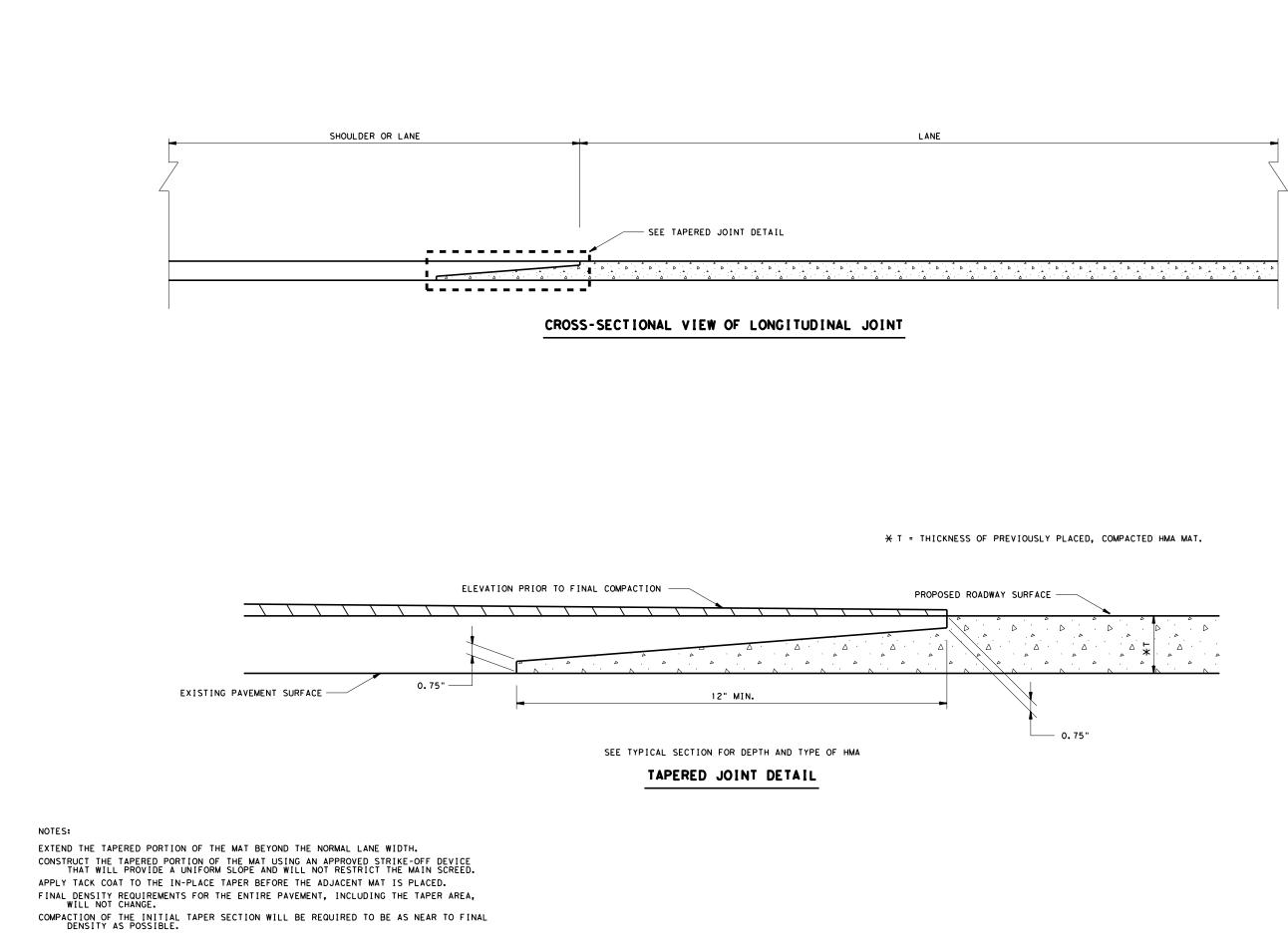
GENERAL NOTES

- 1. UNLESS OTHERWISE SHOWN IN THE PLANS, A VERTICAL EDGE IS PERMISSIBLE FOR HMAC PLACED GREATER THAN 5" BELOW THE EDGE OF PAVEMENT AND FOR THICKNESS OF HMAC LESS THAN 2.5"
- 2. FOR FURTHER INFORMATION REGARDING THE ROADSIDE AND PAVEMENT DETAILS, SEE TYPICAL SECTIONS.
- 3. PAYMENT FOR TAPERED EDGE WILL BE IN ACCORDANCE WITH APPLICABLE ITEMS IN THE CONTRACT.
- 4. THE SLOPE OF THE TAPERED EDGE SHALL BE 1.75H:1V OR FLATTER.
- 5. THE TAPERED EDGE SHALL BE PRODUCED BY USE OF A SCREED ATTACHMENT CAPABLE OF PRODUCING A SMOOTH COMPACTED SURFACE. ADDITIONAL COMPACTING EFFORT BEHIND THE SCREED IS NOT REQUIRED.

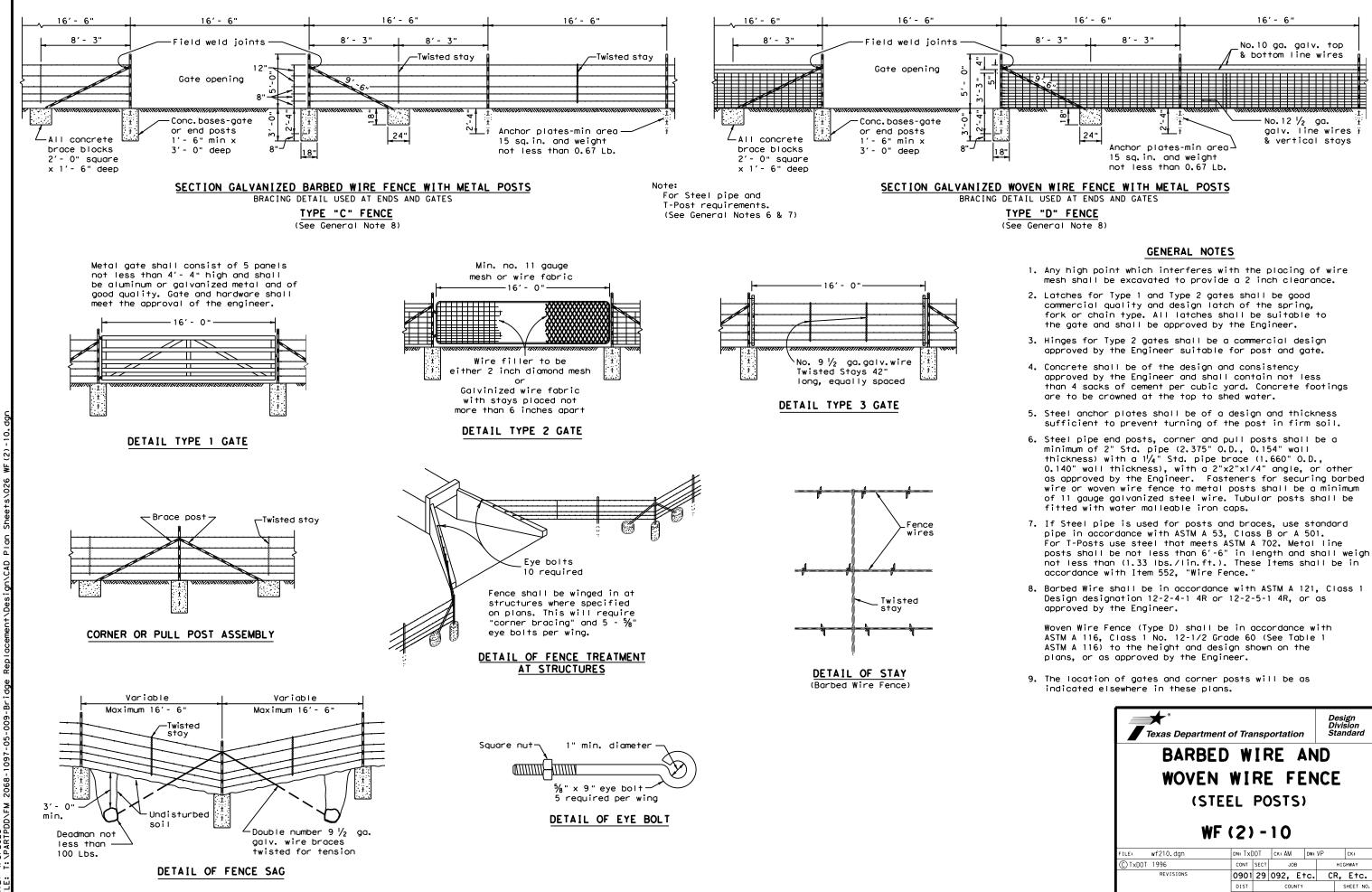






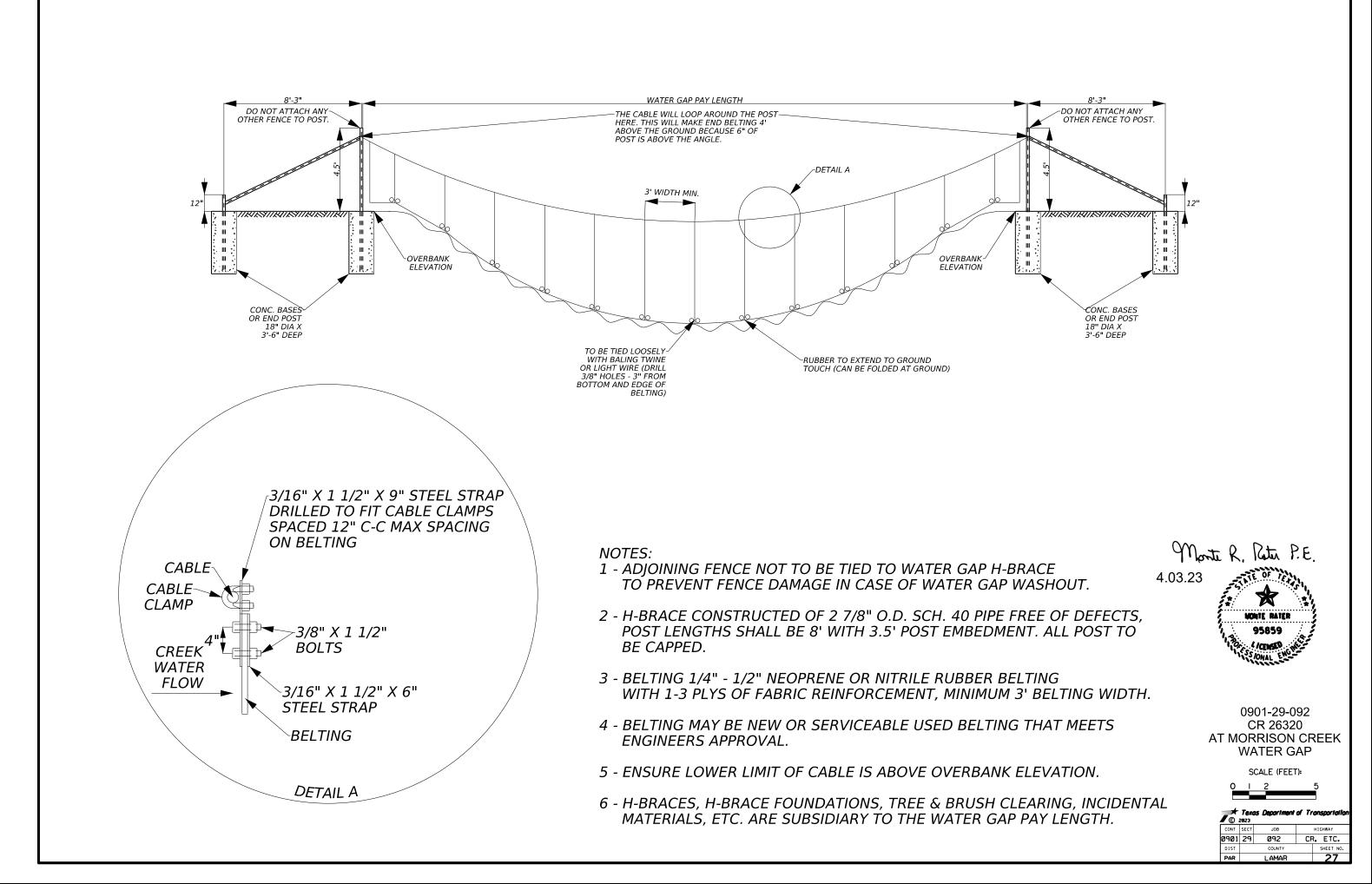


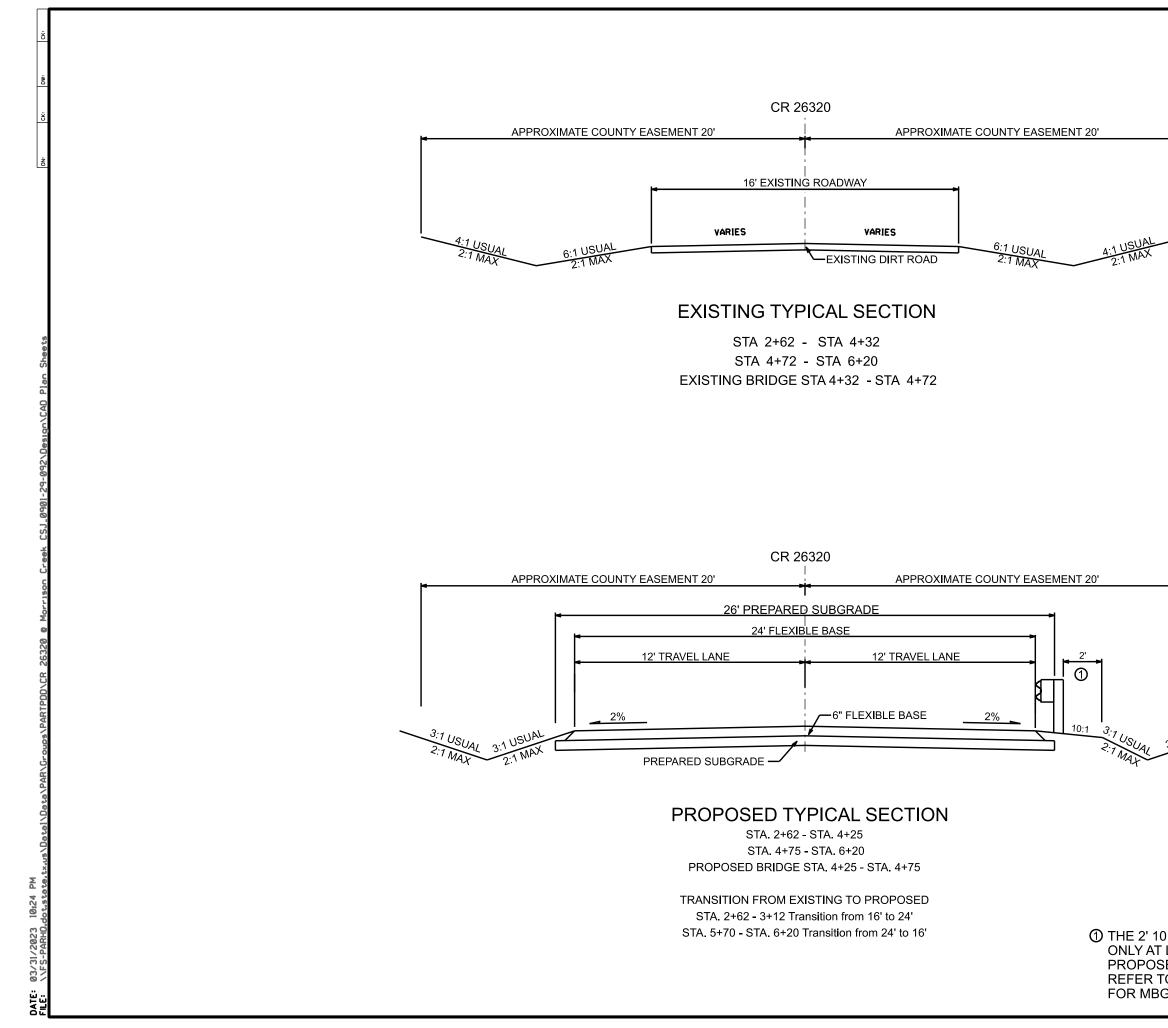




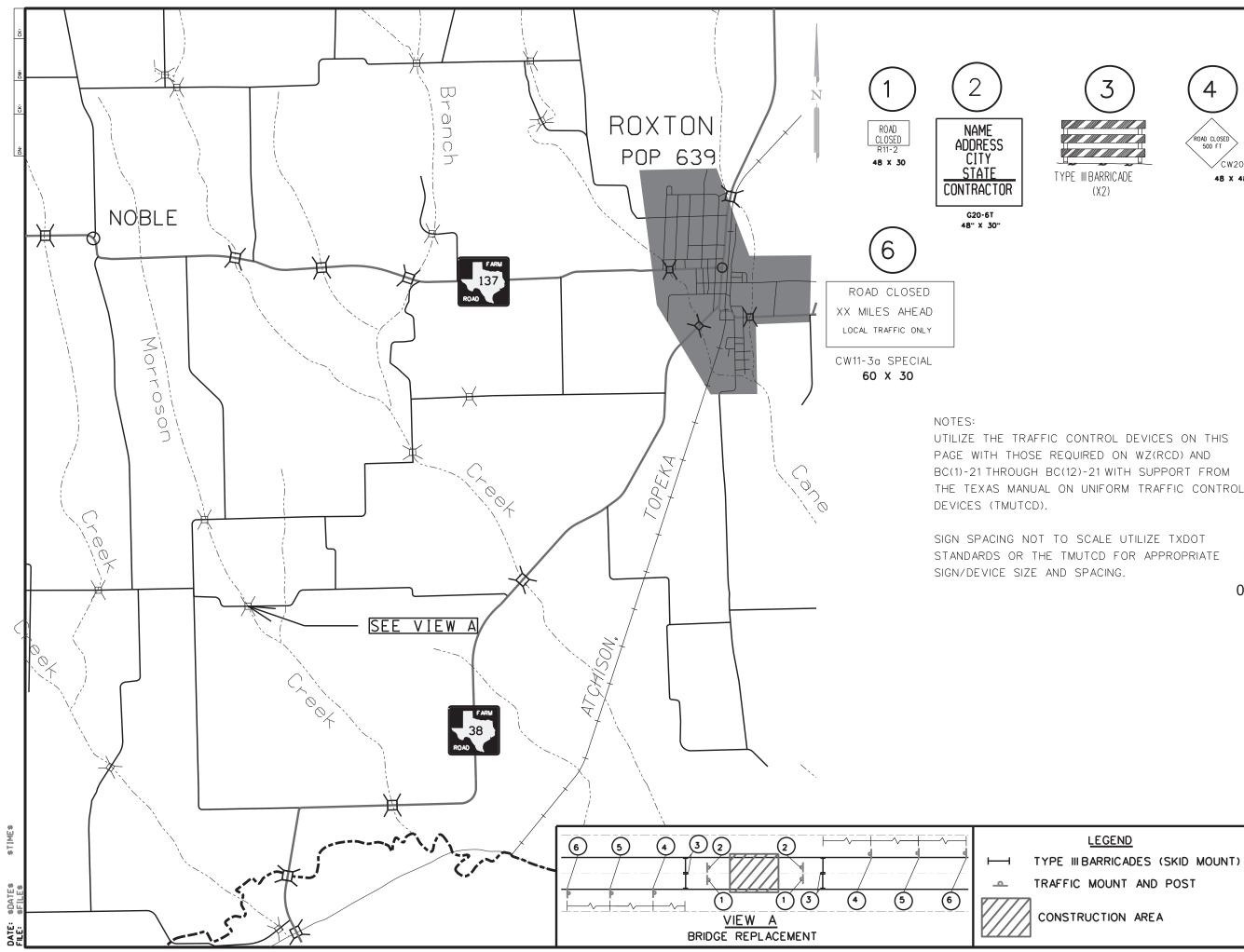
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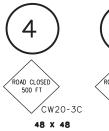
	Texas Department of Transportation							
BARBED WIRE AND								
	WOVE	N WIR	RE F	EN	ICE			
(STEEL POSTS)								
			• •					
	٧	/F (2)	-10					
FILE:	wf210.dgn	/F (2)		Dw:	VP	Ск:		
		DN: TxDO		-		CK: HIGHWAY		
	wf210.dgn	DN: TxDO	ск: АМ	B				
	wf210.dgn DT 1996	DN: TXDOT CONT SE	ск: АМ ст јс 9 092,	B		HIGHWAY		

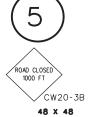


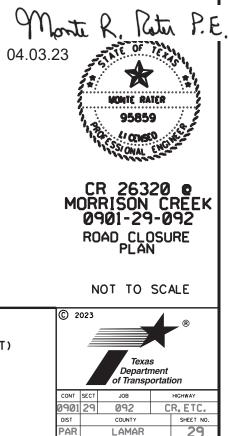


April 1, 20 3:1 USUAL 2:1 MAX		R. Rote A NONITE MATER 95859	
	AT MC	0901-29-0 CR 2632 ORRISON PICAL SEC	0 CREEK
0:1 SLOPE WILL BE REQUIRED I LOCATIONS WHERE MBGF IS SED ADJACENT TO THE ROADWAY.		SCALE (FE)	ET): 5 of Transportation
TO PLAN AND PROFILE SHEETS		ECT JOB	HIGHWAY
GF LOCATIONS.	DIST	COUNTY	SHEET NO.
	PAR	LAMAR	28









TYPE III BARRICADES (SKID MOUNT) TRAFFIC MOUNT AND POST

:MQ
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		; 			100 6002	110 6001	110 6002	132 6003	247 6064	540 6002	544 6001	658 6Ø62
LOCA	ATION	LENGTH	EXISTING WIDTH	PROPOSED WIDTH	PREPARING ROW	EXCAVATION (ROADWAY)	EXCAVATION (CHANNEL)	EMBANKMENT (FINAL)(ORD COMP)(TY B)	FL BS (CMP IN PLC) (TY A GR 4)	MTL W-BEAM GD FEN (STEEL POST)	GUARDRAIL END TREATMENT (INSTALL)	INSTL DEL ASSM (D-SW)S 1(BRF)GF2(BI)
FROM	то	LF	LF	LF	STA	CY	CY	CY	SY	LF	EA	EA
2.02	2.12	5.0	10	24	25	10			100	100		10
2+62	3+12	50	16	24	0.5	10		0	133	100	4	12
3+12	4+25	113	16	24	1.13	35	190	18	301			
4+25	4+75	50	16	24	0.5		106		050			
4+75	5+70	95	16	24	0.95	25		53	253			
5+70	6+20	50	16	24	0.5	32		5	133			
			CSJ 0901-2	9-092 TOTALS	4	102	106	76	820	100	4	12

• AVERAGE WIDTH

SUMMARY OF LANDSCAPE	ITEMS						
			164 6009	164 6011	164 6023	168 6001	
LOCATION	LT/RT WIDTHS	BROADCAST SEED (TEMP) (WARM)	BROADCAST SEED (TEMP) (COOL)	CELL FBR MLCH SEED(PERM)(RU RAL)(CLAY)	VEGETATIVE WATERING (2)	FERTILIZER 3-1-2 3)	
			SY	SY	SY	MG	LBS
2+62 - 4+25	LT	8**	72	72	144	1	15
2+62 - 4+25	RT	8**	72	72	144	1	15
4+75 - 6+20	LT	8**	43	43	86	1	9
4+75 - 6+20	RT	8**	43	43	86	1	9
	CSJ 0901	-29-092 TOTALS	230	230	460	4	48

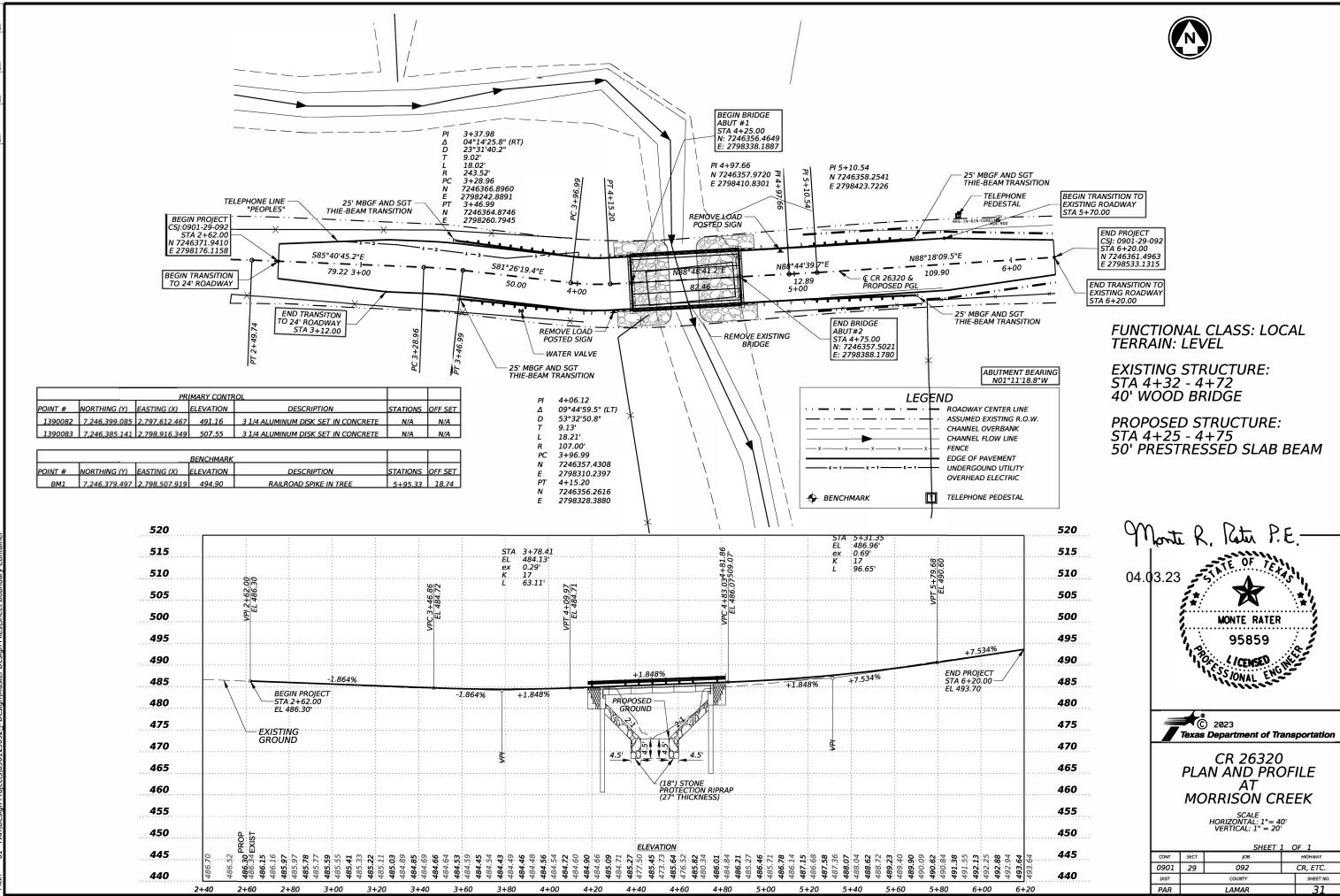
SUMMARY OF REMOVAL ITEMS								
		496						
		6009						
LOCA	TION	REMOV STR (BRIDGE Ø - 99 FT LENGTH)						
FROM	TO	EA						
4+32	1							
CSJ 696	01-29-092 TOTALS	1						

 (2) WATERING BASED ON 2 APPLICATION, 0.5' RAINFAALL EQUIVALENT = 0.003 MG/SY/CYCLE
 (3) FOR CONTRACTOR'S INFORMATION ONLY: 2 CYCLES AT 50 LBS NITROGEN PER ACRE AT 21-7-14 (NPK) ANALYSIS = 0.0492 LBS/SY/CYCLE ** AVERAGE WIDTH

UMMARY OF EROSION CO	INTROL ITEMS						
		506	506	506	506	506	506
		6001	6011	6020	6024	6038	6039
LOCATION	LOCATION LT/RT		ROCK FILTER DAMS (REMOVE)	CONSTRUCTION EXITS (INSTALL) (TY 1)	CONSTRUCTION EXITS (REMOVE)	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDM ⁻ CONT FENCE (REMOVE)
		LF	LF	SY	SY	LF	LF
2+62 - 4+25	LT					75	75
2+62 - 4+25	RT						
4+25	LT	10	10				
4+25	RT	10	10				
4+75	LT	10	10				
4+75	RT	10	10				
4+75 - 6+20	LT						
4+75 - 6+20	RT						
2+62 - 6+20				100	100		
PROJECT TOTALS		40	40	100	100	75	75

	SUMMARY OF WIRE FENCE ITEMS							
			552 6003	552 6008				
	LOCATION		WIRE FENCE (TY C)	WIRE FENCE (WATER GAP)				
Beain	END	RT/LT	LF	LF				
-								
2+62	4+36	RT	174					
2+62	4+36	LT	174					
4+35	4+75	RT		40				
4+75	6+20	RT	145					
	CSJ 0901-29	-092 TOTALS	493	40				

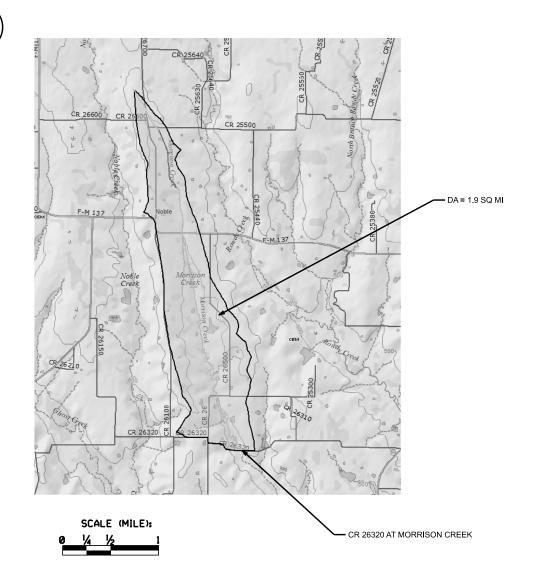




03/3







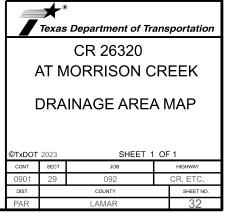
HYDROLOGIC METHOD

DRAINAGE AREAS WERE DETERMINED BY SURVEY DATA, USGS TOPOGRAPHIC MAPS, DIGITAL ELEVATION MODELS AND FIELD OBSERVATIONS.

THE PEAK FLOWS WERE CALCULATED USING THE NRCS UNIT HYDROGRAPH METHOD WITH THE FREQUENCY STORM PRECIPITATION MODEL IN HEC-HMS 4.10. THE 2018 NOAA ATLAS 14 PRECIPITATION DEPTHS WERE USED TO TABULATE RAINFALL AMOUNTS.

NRCS Method						
Frequency (yrs)	Volume (in)	Flow (cfs)				
5 year	3.19	1132				
10 year	4.03	1403				
25 year	5.2	1761				
50 year	6.11	2028				
100 year	7.07 2298					
Lag (min)	Lag (min) 82.5					
Time Interval (min)	terval (min) 15					
Curve Number 82.6						

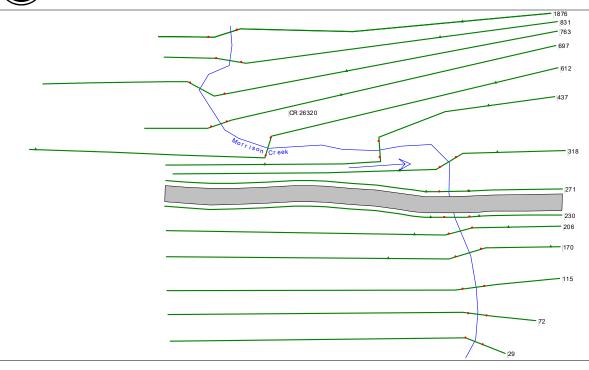




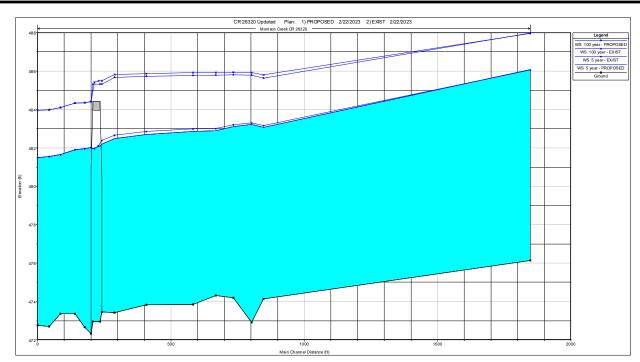
					EXISTING	PROPOSED
				LOW CHORD (FT)	483.01	482.97
			LOWEST RO/	AD ELEVATION (FT)	484.46	484.42
		HEC-RAS	5 YEAR FLOOD	EVENT		
RIVER STATION	EXISTING WATER SURFACE ELEVATION (FT)	PROPOSED WATER SURFACE ELEVATION (FT)	DIFFERENCE (FT)	EXISTING CHANNEL VELOCITY (FT/S)	PROPOSED CHANNEL VELOCITY (FT/S)	DIFFERENCE (FT/S)
1876	486.06	486.07	0.01	4.48	4.47	-0.01
876	483.16	483.07	-0.09	5.70	5.84	0.14
831	483.30	483.22	-0.08	3.64	3.70	0.06
763	483.20	483.11	-0.09	3.83	3.92	0.09
697	483.00	482.89	-0.11	5.00	5.19	0.19
612	482.99	482.86	-0.13	3.89	4.12	0.23
437	482.86	482.70	-0.16	3.56	3.79	0.23
318	482.66	482.49	-0.17	4.03	4.20	0.17
271	482.38	482.20	-0.18	5.03	5.22	0.19
250			BRIDGE			
230	482.02	482.02	0.00	4.93	4.93	0.00
206	481.96	481.96	0.00	5.04	5.04	0.00
170	481.90	481.90	0.00	4.86	4.86	0.00
115	481.65	481.65	0.00	5.62	5.62	0.00
72	481.55	481.55	0.00	5.70	5.70	0.00
29	481.50	481.50	0.00	5.38	5.38	0.00

	HEC-RAS 100 YEAR FLOOD EVENT								
RIVER STATION	EXISTING WATER SURFACE ELEVATION (FT)	PROPOSED WATER SURFACE ELEVATION (FT)	DIFFERENCE (FT)	EXISTING CHANNEL VELOCITY (FT/S)	PROPOSED CHANNEL VELOCITY (FT/S)	DIFFERENCE (FT/S)			
1876	487.95	487.98	0.03	5.62	5.59	-0.03			
876	485.80	485.64	-0.16	5.89	6.18	0.29			
831	485.92	485.78	-0.14	4.17	4.34	0.17			
763	485.95	485.81	-0.14	3.15	3.29	0.14			
697	485.92	485.78	-0.14	3.52	3.67	0.15			
612	485.91	485.77	-0.14	2.75	2.88	0.13			
437	485.87	485.73	-0.14	2.45	2.56	0.11			
318	485.82	485.67	-0.15	2.84	2.97	0.13			
271	485.49	485.32	-0.17	5.11	5.27	0.16			
250			BRIDGE						
230	484.41	484.43	0.02	6.40	6.36	-0.04			
206	484.36	484.36	0.00	6.49	6.49	0.00			
170	484.32	484.32	0.00	6.11	6.11	0.00			
115	484.09	484.09	0.00	6.89	6.89	0.00			
72	483.99	483.99	0.00	7.08	7.08	0.00			
29	483.96	483.96	0.00	6.62	6.62	0.00			

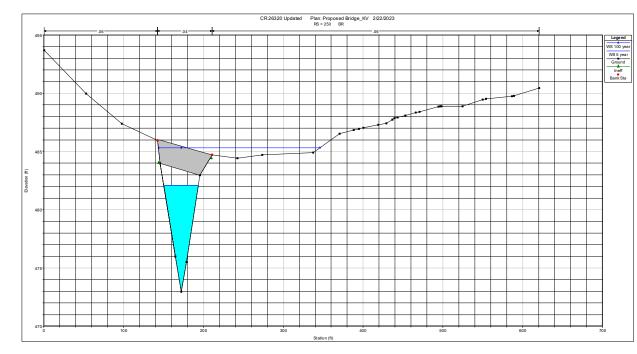




CROSS-SECTION LAYOUT



WATER SURFACE PROFILES



SECTION AT UPSTREAM BRIDGE FACE RIVER STA. 250

NOTES:

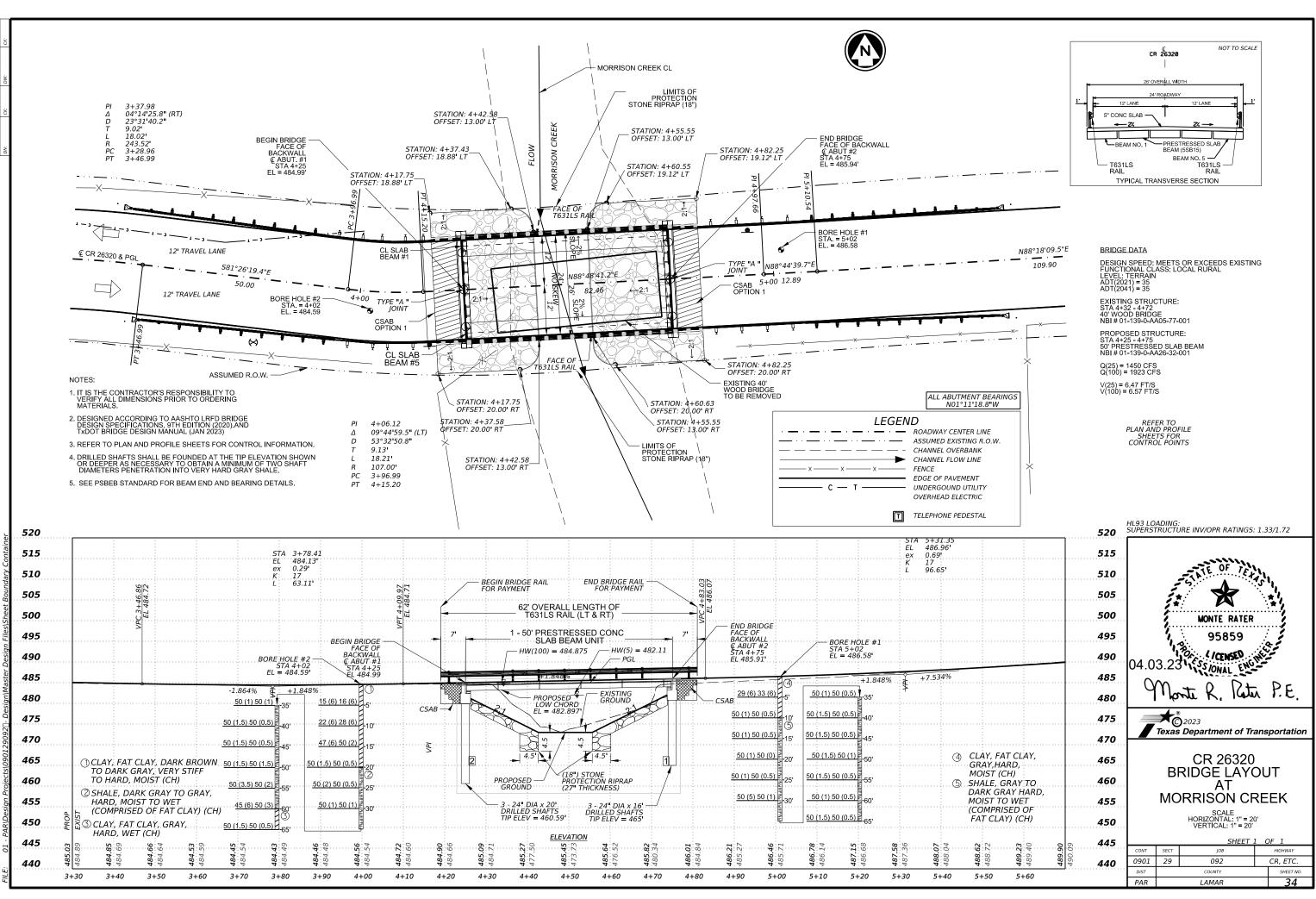
1. THE EXISTING AND PROPOSED WATER SURFACE ELEVATIONS WERE COMPUTED USING HEC-RAS 6.2.

2. NORMAL DEPTH COMPUTATIONS WERE USED FOR UPSTREAM AND DOWNSTREAM BOUNDARY CONDITIONS. A SLOPE OF 0.002 WAS UTILIZED FOR THE EXISTING AND PROPOSED UPSTREAM AND DOWNSTREAM CONDITIONS.

3. THIS SITE LIES IN FEMA FLOOD ZONE X, (FIRM PANEL NO. 48277C.)



Texas Department of Transportation								
CR 26320 AT MORRISON CREEK HYDRAULIC DATA								
©TxDOT 2023 SHEET 1 OF 1								
CONT SECT JOB HIGHWAY								
0901	0901 29 092 CR, ETC.							
DIST		COUNTY		SHEET NO.				
PAR		LAMAR 33						



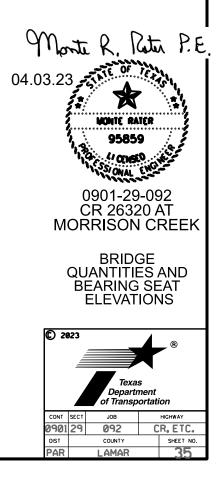
SUMMARY OF MORRISON CREEK BRI	IDGE ITEMS						
	400 6005	416 6002	42Ø 6Ø13	422 6007	425 6Ø12	432 6Ø33	450 6019
LOCATION NBI # 01-139-0-AA26-32-001	CEM STABIL BKFL	DRILL SHAFT (24 IN)	CL C CONC (ABUT)	REINF CONC SLAB (SLAB BEAM)		RIPRAP (STONE PROTECTION)(18 IN)	RAIL (TY T631LS)
	СҮ	LF	СҮ	SF	LF	СҮ	LF
STA 4+25 - 4+75	35	108	20.4	1300	247.5	137	128
CSJ 0901-29-092 TOTALS	35	1Ø8	20.4	1300	247.5	137	128

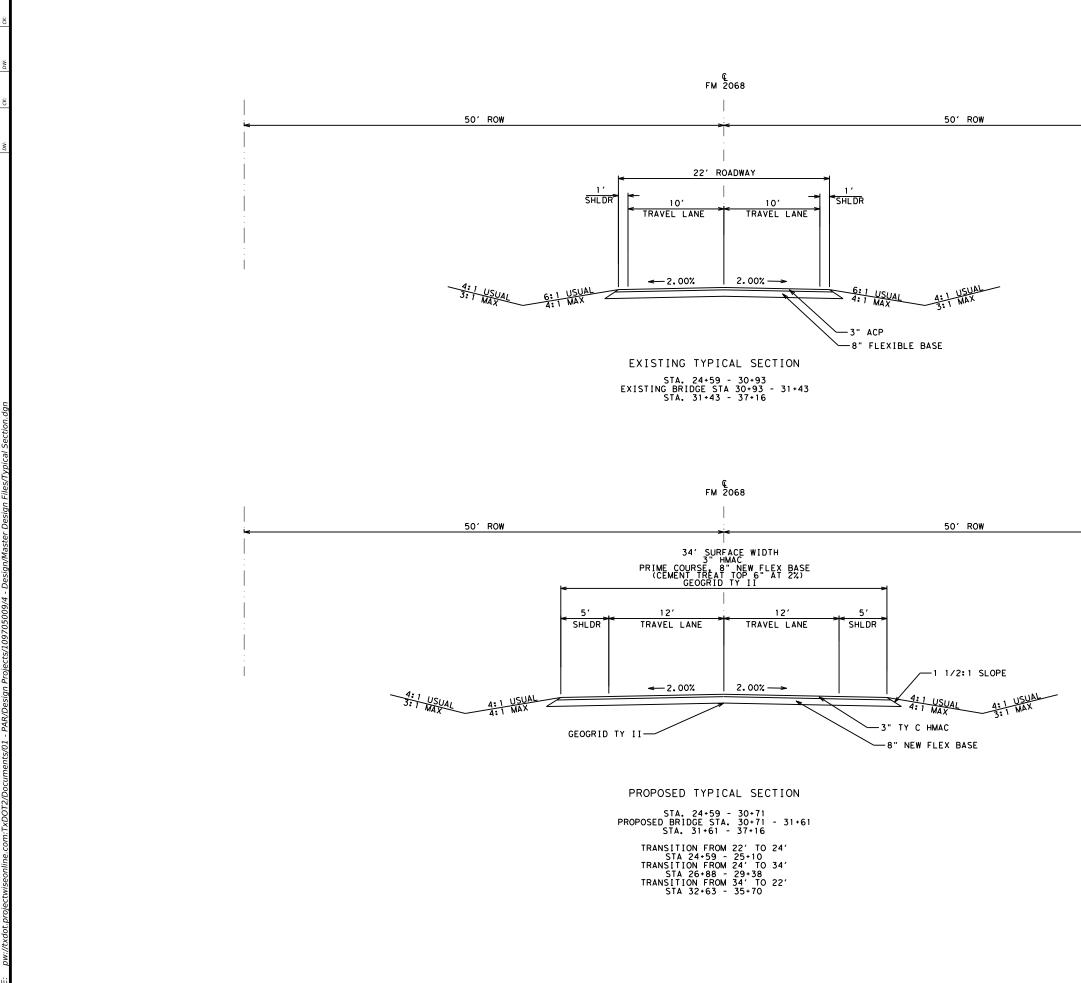
CAP ELEVATIONS (FT)

ABUT 1 (FWD)	STEP 1 (RIGHT) 482.743	(LT.SIDE)	TEP 3 (RT.SIDE) 482.951	(LT.SIDE)	TEP 4 (RT.SIDE) 482.951
ABUT 2 (BK)	STEP 1 (RIGHT) 483.638	(LT.SIDE)	FEP 3 (RT.SIDE) 483.846	(LT.SIDE)	TEP 4 (RT.SIDE) 483.846

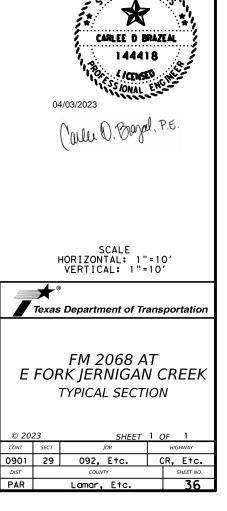
STEP 6 (LEFT) 482.743

STEP 6 (LEFT) 483.638





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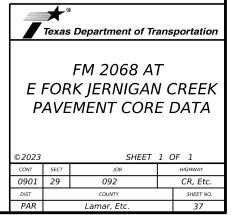


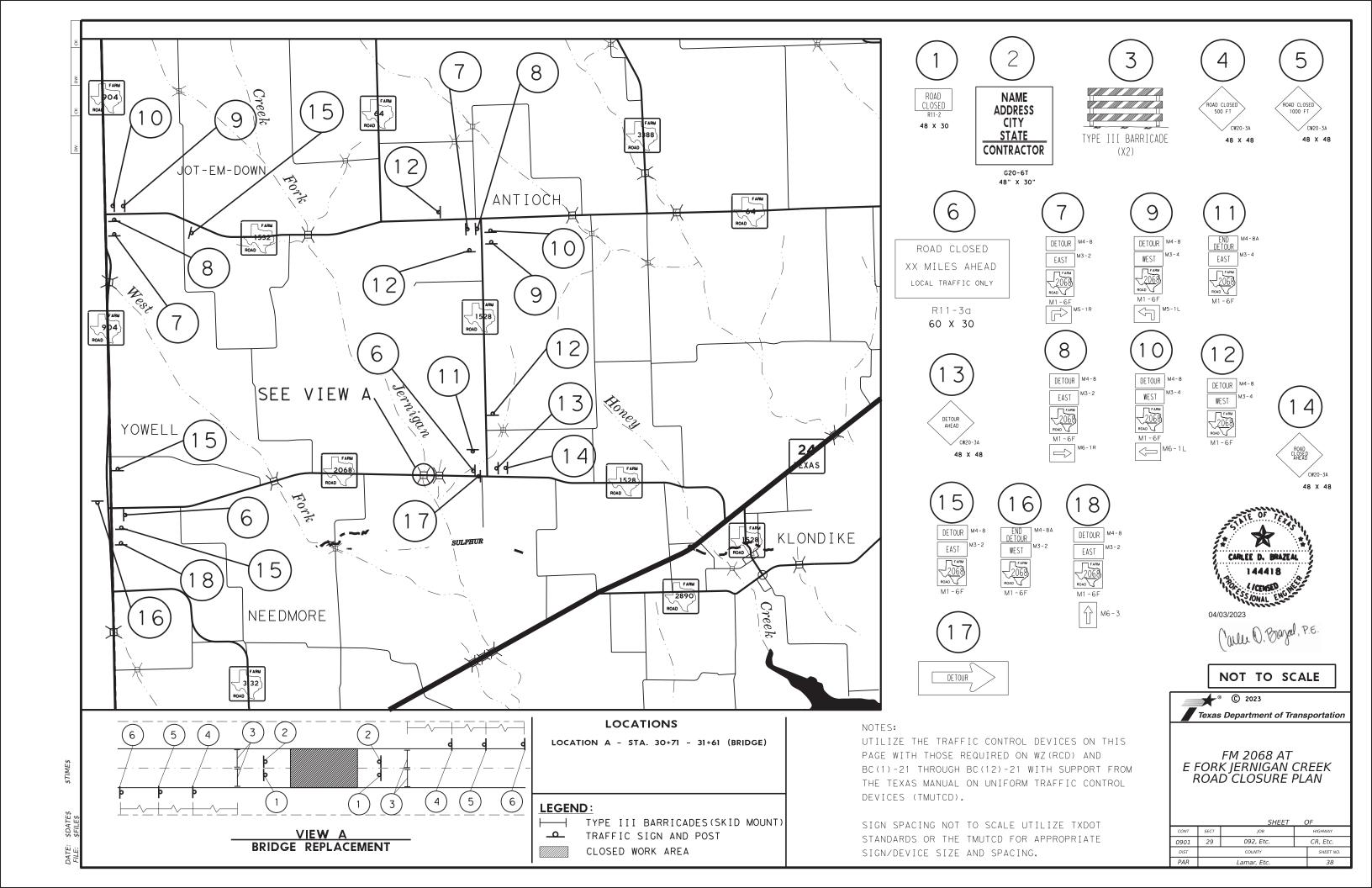
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-	F	PAVEMENT	CORE DATA	
	C - 01 (EB)		APPROX. 2.91 MI EAST 33.344616, -95.810415	
		CORES PROV	IDED BY EST, 2022	

\$TIME\$

FM 904





[SUMMARY OF	F ROADWAY	ITEMS													
					100 6002	110 6001	110 6002	132 6003	275 6003	3076 6016	422 6Ø15	251 628Ø	275 6001	3084 6001	316 6029	316 64Ø3
	LOCA	ATION	LENGTH	WIDTH	PREPARING Row	I IN	EXCAVATIO N (CHANNEL)	EMBANKMEN T (FINAL)(ORD COMP)(TY B)	CEMENT TREAT (NEW BASE)(6")	D-GR HMA TY-C SAC-A PG64-22 (1)	APPROACH SLAB	REWORK BS MTL (TY A)(11")(ORD COMP)	CEMENT (2)	BONDING COURSESE	ASPH (RC-25Ø)	AGGR (TY-B GR-5 OR TY-L GR-5)
	FROM	ТО	LF	LF	STA	СҮ	СҮ	СҮ	SY	TON	СҮ	STA	TON	GAL	GAL	СҮ
	24+59	25+1Ø	51	23	1	57		2	130	22		1	1	6	36	1
	25+1Ø	26+88	178	24	2	16		263	475	78		2	4	24	133	3
	26+88	29+38	250	28	3			884	778	128		3	6	39	218	6
	29+38	3Ø+71	133	34	1			9Ø4	502	83		1	4	25	141	4
	30+71	31+61	90	34	1		450				48					
	31+61	32+63	102	34	1			956	385	64		1	3	19	108	3
	32+63	35+70	3Ø7	28	3	25		965	955	158		3	7	48	267	7
	35+70	37+16	146	22	1	21		94	357	59		1	3	18	100	3
_																
		CSU	J 1097-05-1	009 TOTALS	13	119	450	4068	3582	592	48	12	28	179	1003	27

* AVERAGE WIDTH (1) BASED ON 110 LBS/SY/IN

<u>PRIME COURSE:</u> ASPH: RC-250 @ 0.28 GAL/SY AGGR: GR 5 OR MOD 5 B OR L @ 1:140 CY/SY

(2) CEMENT BASED ON AN ASSUMED DRY COMPACTED UNTI WEIGHT OF 110 LBS/CF @ 2% BY WEIGHT

JMMARY OF	F GUARD FE	NCE ITEMS						SUMMARY O	F ROADWAY	ITEMS				
			540 6002	544 6001	542 6001	542 6003	658 6Ø62					5001 6002	105 6038	247 6Ø96
LOCA	TION	LT/RT	MTL W-BEAM GD FEN (STEEL POST)	GUARDRAIL END TREATMENT (INSTALL)	REMOVE METAL BEAM GUARD FENCE	REMOVE DOWNSTRE AM ANCHOR TERMINAL	INSTL DEL ASSM (D-SW)SZ 1(BRF)GF 2(BI)	LOCA	ηTΙΟΝ	LENGTH	WIDTH	GEOGRID BASE REINFORC EMENT (TY II)	REMOVING STAB BASE AND ASPH PAV (11")	FL BS (CMP IN PLC)(TY I GR 4)
FROM	ТО	-	LF	EA	LF	EA	EA	FROM	ТО	LF	LF	SY	SY	TON
								24+59	25+10	51	23	130		53
25+63	25+86	LT	200	2	100	2	3	25+10	26+88	178	24	475		192
25+63	25+86	RT	200	2	100	2	3	26+88	29+38	250	28	778		315
30+71	31+61	LT	200	2	150	2	3	29+38	3Ø+71	133	34	502		2Ø3
30+71	31+61	RT	200	2	150	2	3	30+71	31+61	90	34			
								31+61	32+63	102	34	385		156
								32+63	35+70	307	28	955		387
								35+70	37+16	146	22	357		145
<u> </u>	J 1097-05-1	, 1010 1010	800	8	500	8	12	24+59	26+33	174	24		464	
000			1 000		000			35+94	37+16	122	24		325	
									CS	J 1097-05-I	209 TOTALS	3582	789	763

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FM 2068 AT E FORK JERNIGAN CREEK QUANTITY SUMMARIES

©2023		SHEET	1 0	DF 2		
CONT	SECT	JOB		HIGHWAY		
0901	29 092, Etc.			CR, Etc.		
DIST	COUNTY			SHEET NO.		
PAR	Lamar, Etc. 39					

- úi	4/3/2023	4:02:38 PM
	- +- p	and the det are instanting and an arm TVDOTO (Decrements //
	DW://LXUUL.D	I UECLWISEUTITIE. CUTI: LXDUIZ/DUCUTIETLIS/

DATE

(2) WATERING BASED ON 2 APPLICATION, Ø.5" RAINFALL EQUIVALENT = Ø.ØØ28 MG/SY/CYCLE (3) FOR CONTRACTOR'S INFORMATION ONNLY: 2 CYCLES AT 50 LBS NITROGEN PER ACRE AT 21-7-14 (NPK) ANALYSIS = 0.0492 LBS/SY/CYCLE

5Ø6

6Ø11

ROCK

FILTER

DAMS

(REMOVE)

LF

2Ø

2Ø

2Ø

2Ø

2Ø

2Ø

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160

506

6001

ROCK

FILTER

DAMS

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FENCE

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96

96

463

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FENCE

(REMOVE)

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CONSTRUCT

ION EXITS

(INSTALL)

(TY 1)

SY

150

150

5Ø6

6Ø24

CONSTRUCT

ION EXITS

(REMOVE)

SY

SUMMARY OF LANDSCAPE ITEMS						
	164 6009	164 6Ø11	164 6Ø15	168 6001		
LOCATION	BROADCAST SEED (TEMP) (WARM)	BROADCAST SEED (TEMP) (COOL)	STRAW/HAY MLCH SEED (PERM) (RURAL) (CLAY)	VEGETATIVE WATERING (2)	FERTILIZER (3)	
	SY	SY	SY	MG	LBS	
24+59 - 37+16	8380	838Ø	1676Ø	1Ø1	1650	
CSJ 1097-05-009	8380	838Ø	1676Ø	1Ø1	1650	

SUMMARY OF RE	EMOVAL ITE	MS			
		496 6009			
LOCA	LOCATION				
FROM	ТО	EA			
30+93	31+43	1			
CSJ 1097-05-0	009 TOTALS	1			

	CSJ 1097-05-009	2514	31Ø
	SUMMARY OF WORKZONE	TRAFFIC CC	DNTROL ITEMS
		6001 6002	6185 6002
	LOCATION	PORTABLE CHANGEAB LE MESSAGE SIGN	TMA (STATION ARY)
		EA	DAY
	24+59 - 37+16	2	73
150			
150	CSJ 1097-05-009	2	73

SUMMARY

SUMMARY OF EROSION CONTROL ITEMS

LOCATION

24+59 - 25+55

24+59 - 25+55

25+55

25+55

26+00

26+00

26+00 - 30+63

26+00 - 30+63

30+71

30+71

31+61

31+61

31+69 - 36+20

31+69 - 36+20

30+71 - 31+61

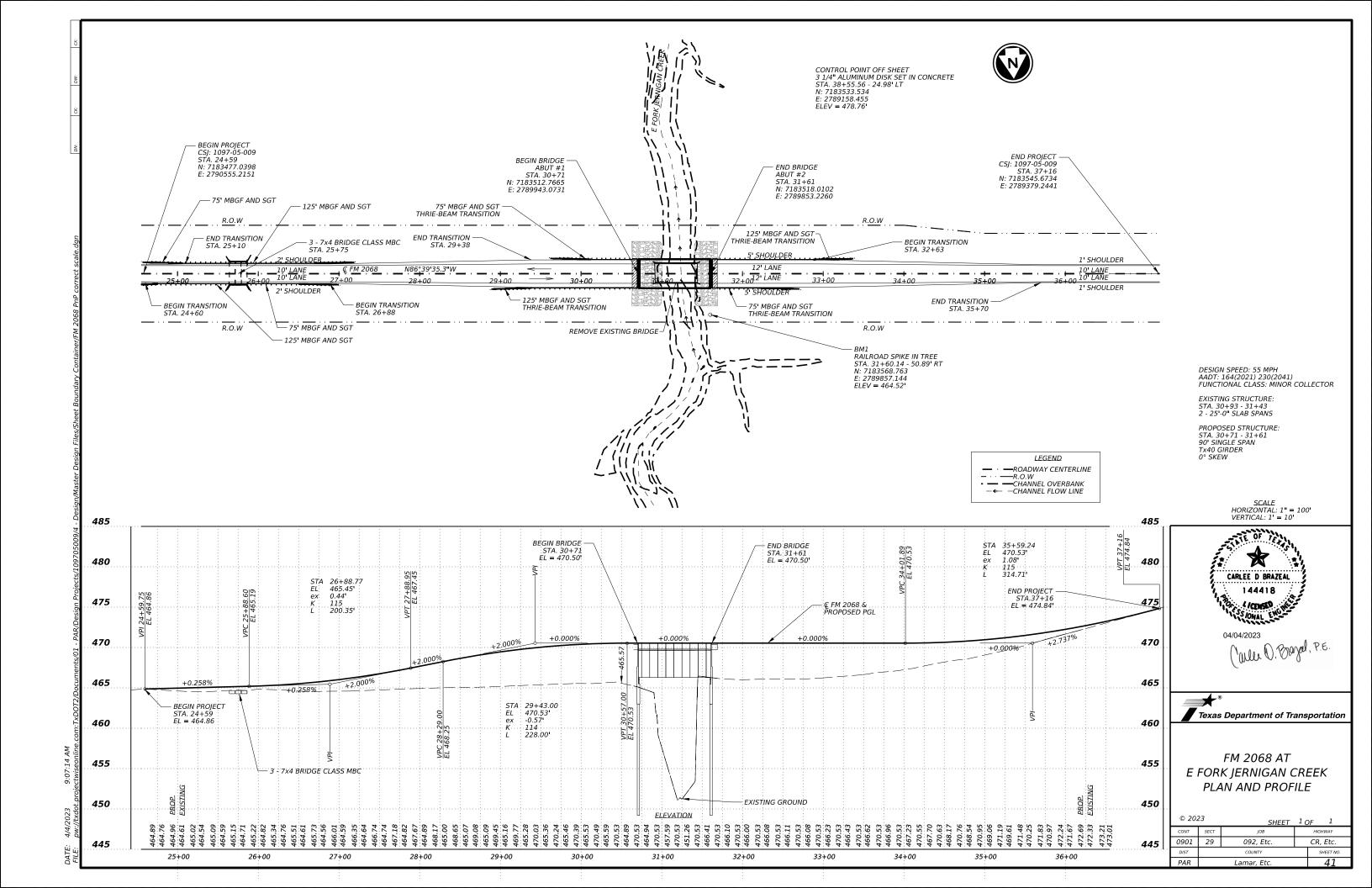
PROJECT TOTALS

ARY OF PAVEMENT	MARKING II	EMS
LOCATION	666	666
	6305	6317
	RE PM W/RET REQ TY I (W)6"(BR K)(Ø9ØMIL)	RE PM W/RET REQ TY I (Y)6"(BR K)(Ø90MIL)
	LF	LF
24+59-37+16	2514	31Ø
SJ 1097-05-009	2514	310



FM 2068 AT E FORK JERNIGAN CREEK QUANTITY SUMMARIES

©2023	SHEET 2 OF 2					
CONT	SECT	JOB		HIGHWAY		
0901	29 092, Etc.			CR, Etc.		
DIST	COUNTY			SHEET NO.		
PAR	Lamar, Etc.			40		



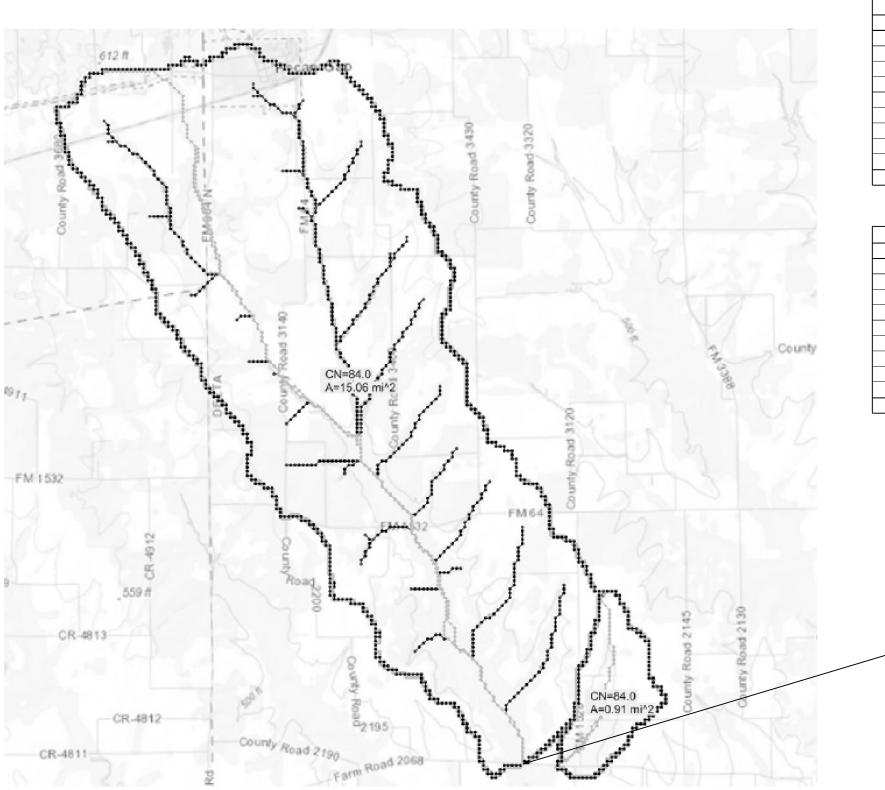


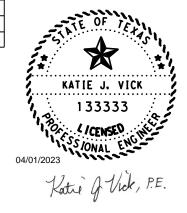
DRAINAGE AREAS WERE DETERMINED BY SURVEY DATA, USGS TOPOGRAPHIC MAPS, DIGITAL ELEVATION MODELS AND FIELD OBSERVATIONS. THE PEAK FLOWS WERE DETERMINED USING THE FREQUENCY STORM METHOD MODELED IN HECHMS 4.8. THE 2018 NOAA ATLAS 14 DEPTHS WERE USED TO TABULATE RAINFALL AMOUNTS.

	HEC-HMS					
Recurrence	Flow (cfs)					
2 year	3710					
5 year	5402					
10 year	6721					
25 year	8473					
50 year	9807					
100 year	11170					
Lag (min)	181.00					
Time Interval (min)	30.00					
RCN (AMC II)	84.0					
DA (sq mi)	15.06					

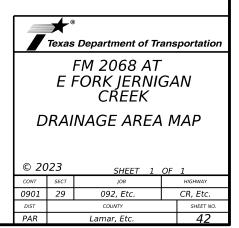
HEC-HMS					
Recurrence	Flow (cfs)				
2 year	529				
5 year	753				
10 year	922				
25 year	1142				
50 year	1302				
100 year	1464				
Lag (min)	55.5				
Time Interval (min)	10.00				
RCN (AMC II)	84.0				
DA (sq mi)	0.91				

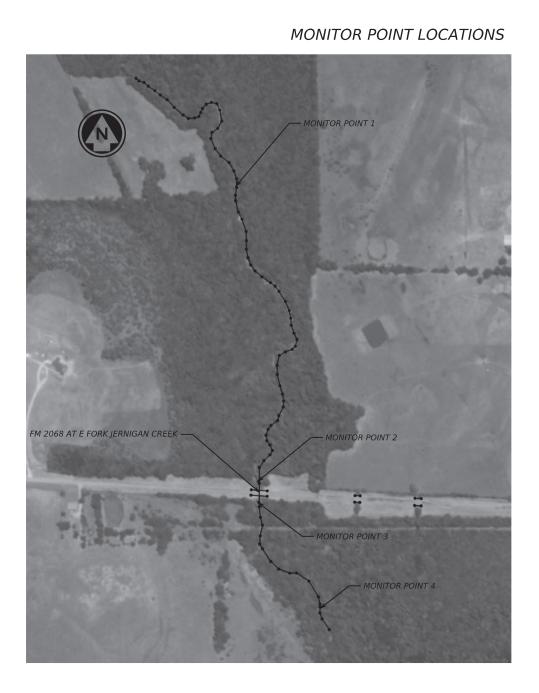
- FM 2068 AT E FORK JERNIGAN CREEK BRIDGE LOCATION



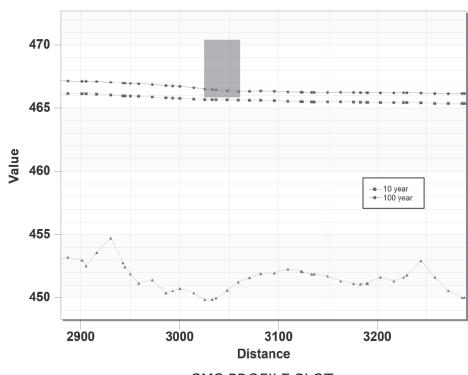


SCALE (FEET):						
0	30	1500				

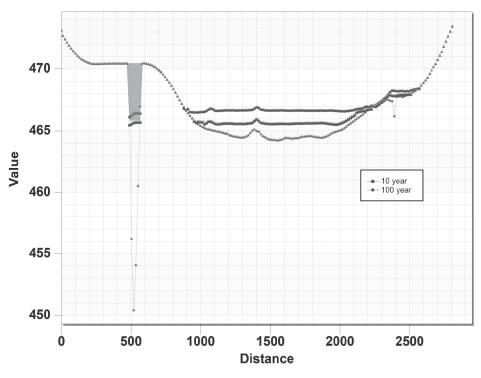




LOW CHORD (FT) 465.87								
L	OWEST ROAD ELEVATION (FT)	463.13						
	10 YEAR FLOOD EVENT							
MONITOR	PROPOSED WATER SURFACE	PROPOSED CHANNEL						
LOCATION	ELEVATION (FT)	VELOCITY (FT/S)						
1	469.05	4.3						
2 465.82		5.03						
	BRIDGE							
3 465.59		5.1						
4	464.67	4.1						
	100 YEAR FLOOD EVEN	νT						
MONITOR	PROPOSED WATER SURFACE	PROPOSED CHANNEL						
LOCATION	ELEVATION (FT)	VELOCITY (FT/S)						
1	469.98	4.1						
2	466.8	5.03						
	BRIDGE							
3	466.33	6.15						
4	465.5	4.11						



SMS PROFILE PLOT

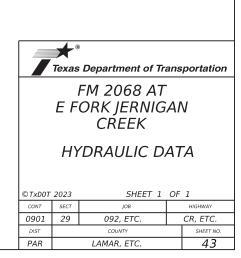


SMS BRIDGE CROSS-SECTION

NOTES:

- 1. THE PROPOSED WATER SURFACE ELEVATIONS WERE COMPUTED USING SMS 13.2.10.
- 2. DELTA COUNTY DOES NOT PARTICIPATE IN FEMA.
- 3. THE SURVEYED VERTICAL DATUM IS NAV88. NO ADJUSTMENTS TO VERTICAL DATUM WERE MADE.

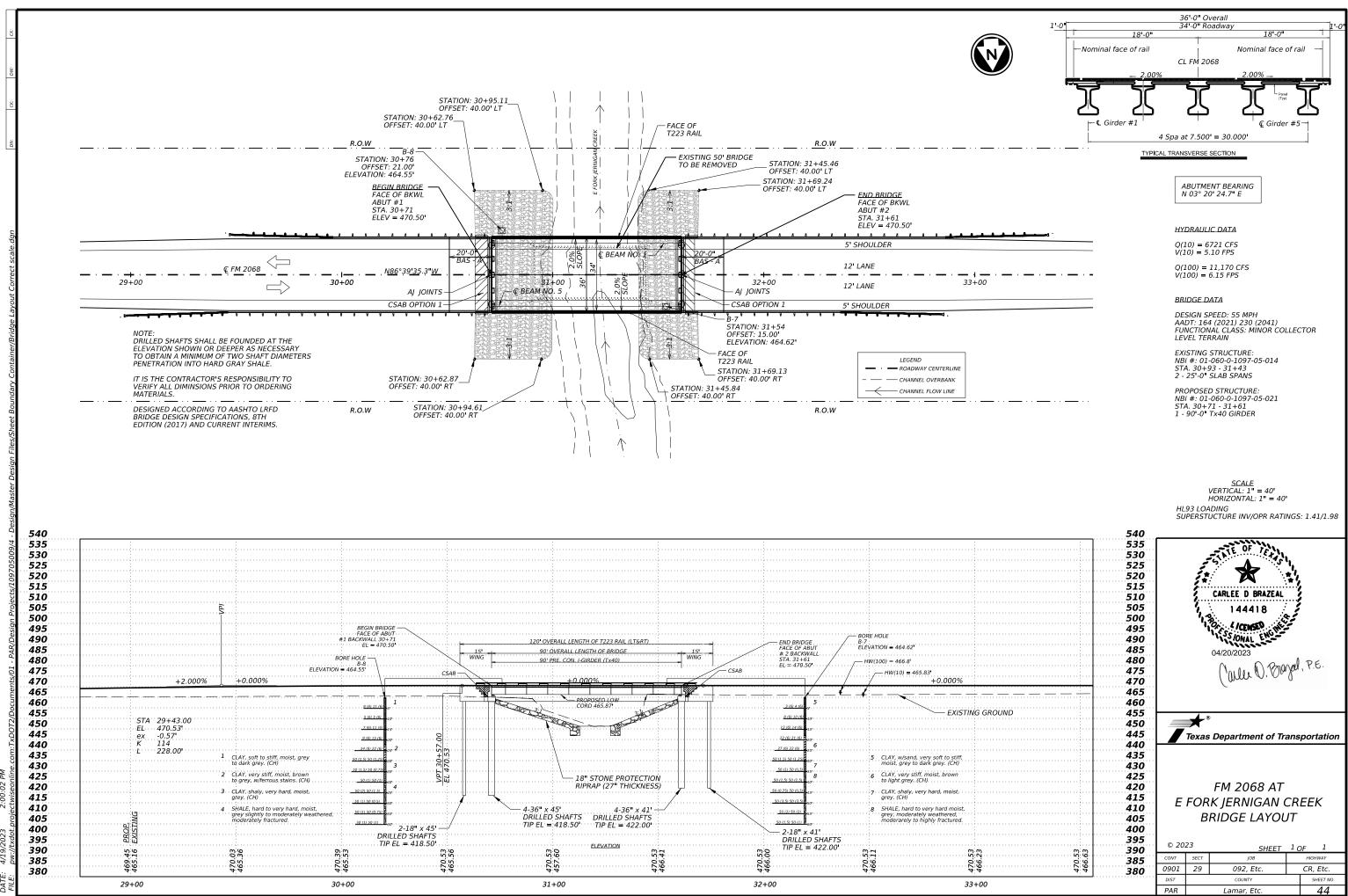




KATIE J. VICK

133333

04/01/2023 Katie & Vick, P.E.



00:0 4/1

SUMMARY OF E FORK JERNIGAN CREEK BRIDGE ITEMS									
	400 6005	416 6004	42Ø 6Ø13	422 6001	425 6Ø37	432 6Ø33	45Ø 6ØØ6	416 6001	454 6003
LOCATION NBI# 01-060-0-1097-05-021	CEM STABIL BKFL	DRILL Shaft (36 IN)	CL C CONC (ABUT)	REINF Conc Slab	PRESTR CONC GIRDER (TX4Ø)	RIPRAP (STONE PROTECTI ON)(18 IN)	RAIL (TY T223)	DRILL Shaft (18 IN)	ARMOR Joint
	СҮ	LF	СҮ	SF	LF	СҮ	LF	LF	LF
STA 30+71 - 31+61	7Ø	344	53.2	324Ø	447.5	4Ø2	24Ø	172	68
CSJ 1097-05-009 TOTALS	7Ø	344	53.2	324Ø	447.5	4Ø2	24Ø	172	68

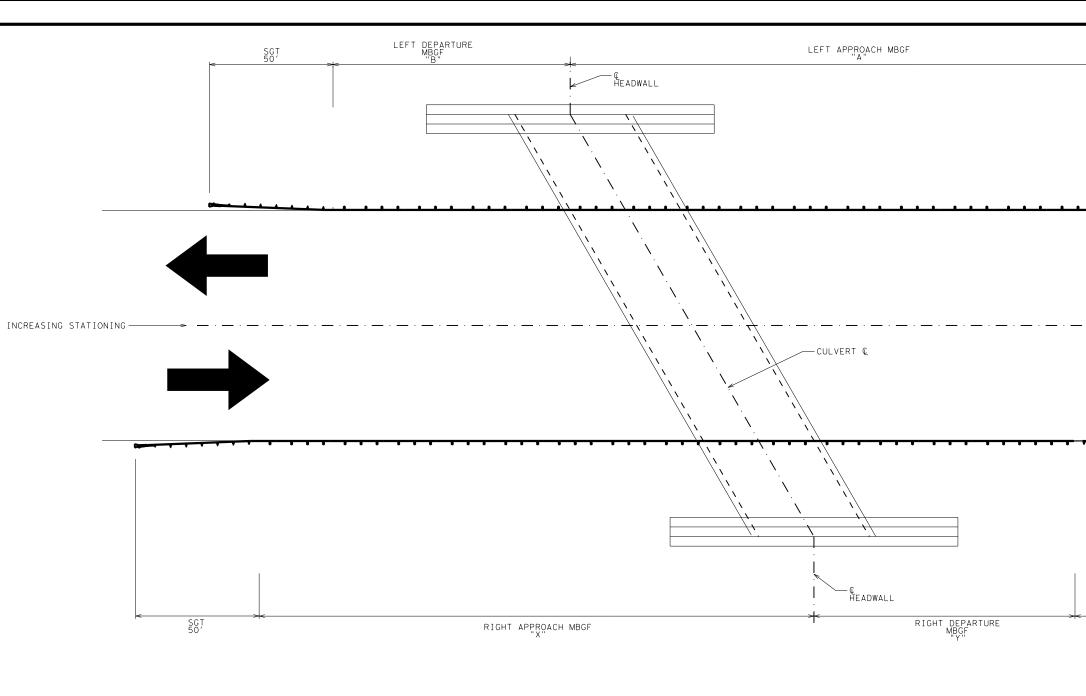
(1) 0.4 CY ADDED FOR SHEAR KEY

Bearing Seat Elevations

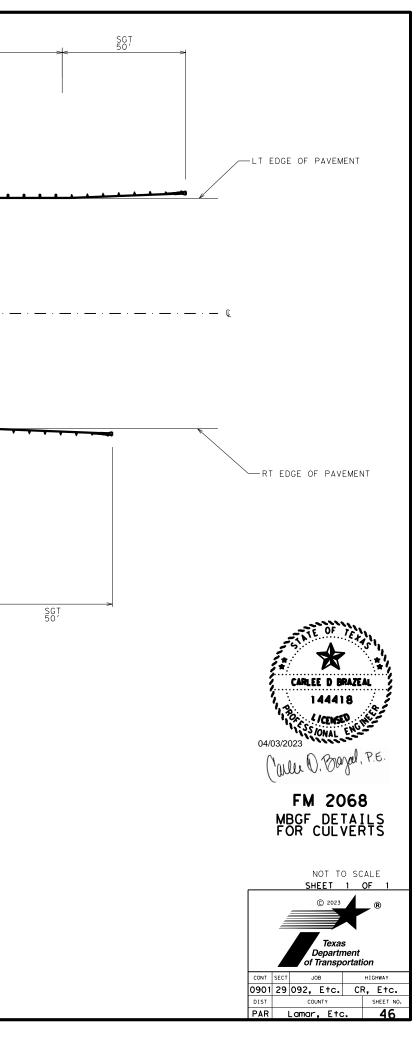
ABUT 1 (FWD)	 GIRDER 2 465.618	 GIRDER 4 465.618	GIRDER 5 465.468
ABUT 1 (BK)	 GIRDER 2 465.618	 GIRDER 4 465.618	GIRDER 5 465.468

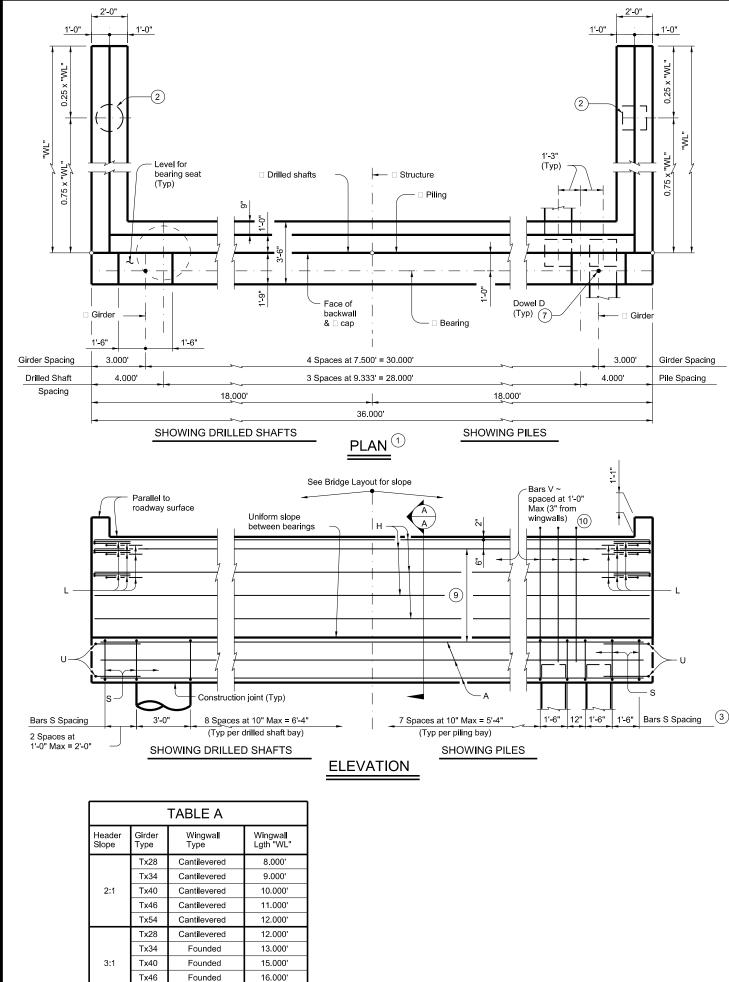


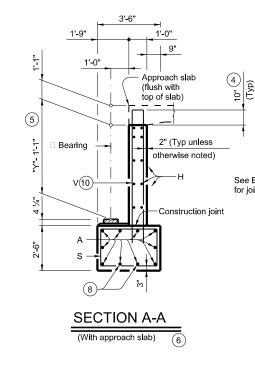
Texas Department of Transportation							
FM 2068 AT E FORK JERNIGAN CREEK BRIDGE QUANTITIES AND BEARING SEAT ELEVATIONS							
CONT	SECT	JOB	HIGHWAY				
0901	29	092, Etc. CR, E		CR, Etc.			
DIST		COUNTY		SHEET NO.			
PAR		Lamar, Etc. 45					



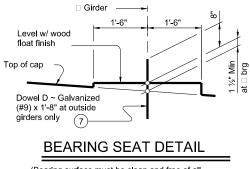
BRIDGE CULVERTS	CULVERT START	CULVERT END	A	В	x	Y
CSJ 1097-05-009						
DRAIN	STA. 25+63.5	STA. 25+86.5	125	75	125	75







- (1) See Table A for variable dimensions based on header slope and girder type.
- (2) See Table A to determine if this wingwall foundation is required.
- (3) For piling larger than 16" adjust Bars S spacing as required to avoid piling.
- (4) Increase as required to maintain 3" from finished grade.
- 5 See Span details for "Y" value.
- 6 See Bridge Layout to determine if approach slab is present.
- (7) Omit Dowels D at end of multi-span unit. Adjust reinforcing steel total accordingly.
- (8) With pile foundations, move Bars A shown to clear piles.
- (9) Spacing based on girder type: Tx28 ~ 3 spaces at 1'-0" Max Tx34 ~ 3 spaces at 1'-0" Max Tx40 ~ 4 spaces at 1'-0" Max Tx46 ~ 4 spaces at 1'-0" Max Tx54 ~ 5 spaces at 1'-0" Max
- (10) Field bend as needed to clear piles.



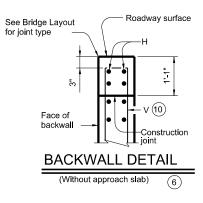
(Bearing surface must be clean and free of all loose material before placing bearing pad.)

Tx54

Founded

18.000'

TABLE OF FOUNDATION LOADS



Span Length	All Girder Types					
Ft	Tons/Shaft	Tons/Pile				
40	61	53				
45	65	56				
50	69	58				
55	73	60				
60	77	62				
65	81	64				
70	84	65				
75	88	67				
80	92	69				
85	96	71				
90	99	73				
95	103	75				
100	107	77				
105	110	79				
110	114	81				
115	117	82				
120	121	84				
125	125	86				

MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi).

- Provide Class C (HPC) concrete if shown elsewhere in the plans.
- Provide Grade 60 reinforcing steel. Galvanize dowel bars D.

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

See Bridge Layout for header slope and foundation type, size and length. See Common Foundation Details (FD) standard sheet

See Common Foundation Details (FD) standard sheet for all foundation details and notes.

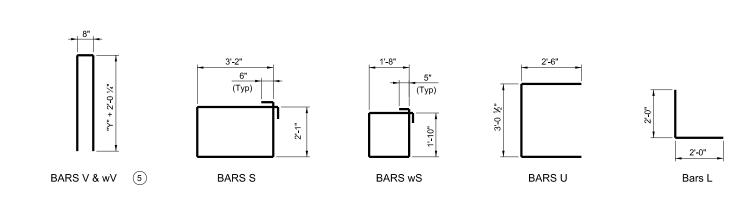
See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment details, if applicable.

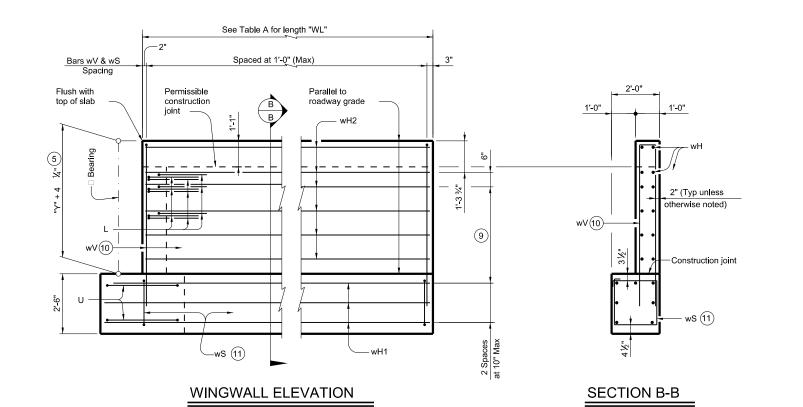
See applicable rail details for rail anchorage in wingwalls.

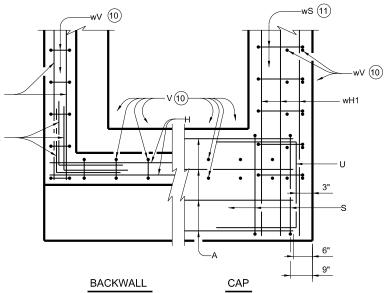
These abutment details may be used with standard SIG-34 only.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

HL93	L93 LOADING SHEET 1 OF 3							
Texas Department of Transportation				D	ridge ivision tandard	d		
ABUTMENTS								
	TYPE TX28	в тн	IRL	J TX54	4			
	PRESTR CO	DNC	I-(GIRDE	ER	S		
	34' R	OAI	DΜ	/AY				
			A	IG-3	4			
FILE:	IG-AIG3400-23.dgn	DN: TA	२	ск: VC	DW:	SFS	ск: Т/	٩R
C TxDOT	January 2023	CONT	SECT	JOB			HIGHWAY	
	REVISIONS	0901	29	092, E	tc.	C	R, Etc	с.
		DIST		COUNT	Y		SHEET N	νο.
		PAR	l	_amar.	E+c		47	







wH2

- 9 Spacing by Tx28 ~ 3 s Tx34 ~ 3 s Tx40 ~ 4 s Tx46 ~ 4 s Tx54 ~ 5 s

- (1) Adjust as required to avoid piling.

CORNER DETAILS

5 See Span details for "Y" value.

based on girder type:
spaces at 1'-0" Max

(10) Field bend as needed to clear piles.

HL93 LOADING SHE					ET 2 O	F 3		
Texas Department of Transportation				1	Bridge Division Standard			
ABUTMENTS								
TYPE TX28 THRU TX54								
PRESTR CO	ONC	I-C	GIRDE	ER	s			
34' R	OA	ΟW	ΆΥ					
AIG-34								
FILE: IG-AIG3400-23.dgn	DN: TAF	२	ск: VC	DW:	SFS	ск: TAR		
CTxDOT January 2023	CONT	SECT	JOB		н	GHWAY		
REVISIONS	0901	29	092, E	tc.	CR,	Etc.		
	DIST	COUNTY				SHEET NO.		
	PAR	l	.amar,	E†c	·.	48		

TABLES OF ESTIMATED QUANTITIES WITH 2:1 HEADER SLOPE

																					_
	TYPE	Tx28	Girder	'S			TYPE	Tx34	Girders	5			TYPE	Tx40	Girder	s				TYPE	Т
Bar	No.	Size	Lei	ngth	Weight	Bar	No.	Size	Len	gth	Weight	Bar	No.	Size	Ler	igth	Weight		Bar	No.	
A	10	#11	35	'-0"	1,860	А	10	#11	35'-	0"	1,860	А	10	#11	35'	-0"	1,860	IΓ	А	10	Γ
D (7)	2	#9	1'	-8"	11	D(7)	2	#9	1'-8	В"	11	D(7	2	#9	1'-	8"	11	ΙΓ	D (7)	2	Γ
н	8	#6	35	'-8"	429	н	8	#6	35'-	8"	429	н	10	#6	35'	-8"	536	ΙΓ	н	10	Γ
L	18	#6	4'	-0"	108	L	18	#6	4'-(D"	108	L	18	#6	4'-	0"	108	ΙE	L	18	
S	33	#5	11	'-6"	396	S	33	#5	11'-	6"	396	S	33	#5	11'	-6"	396		S	33	
U	4	#6	8'	-1"	49	U	4	#6	8'-1	1"	49	U	4	#6	8'-	1"	49		U	4	
V	35	#5	11	'-4"	414	V	35	#5	12'-	4"	450	V	35	#5	13'	-4"	487		V	35	
wH1	14	#6	9'	-5"	198	wH1	14	#6	10'-	5"	219	wH1	14	#6	11'	-5"	240		wH1	14	
wH2	20	#6	7'	-8"	230	wH2	20	#6	8'-8	В"	260	wH2	24	#6	9'-	8"	348		wH2	24	
wS	18	#4	7'-	10"	94	wS	20	#4	7'-1	0"	105	wS	22	#4	7'-'	10"	115		wS	24	
wV	18	#5	11	'-4"	213	wV	20	#5	12'-	4"	257	wV	22	#5	13'	-4"	306		wV	24	
Reinfor	cing Steel			Lb	4,002	Reinfor	cing Steel			Lb	4,144	Reinfor	cing Steel			Lb	4,456	ΙΓ	Reinforc	cing Steel	
Class "(C" Concret	e		CY	19.6	Class "(C" Concret	e		CY	21.2	Class "	C" Concre	te		CY	22.9	ΙΓ	Class "C	C" Concret	íe
																		/ L		_	

TABLES OF ESTIMATED QUANTITIES WITH 3:1 HEADER SLOPE

TYPE Tx28 Girders									
Bar	No.	Size	Len		Weight				
				<u> </u>					
A	10	#11	35'	-0"	1,860				
D(7)	2	#9	1'-8"		11				
н	8	#6	35'-8"		429				
L	18	#6	4'-	0"	108				
S	33	#5	11'	-6"	396				
U	4	#6	8'-	8'-1"					
V	35	#5	11'	11'-4"					
wH1	14	#6	13'	-5"	282				
wH2	20	#6	11'	-8"	350				
wS	26	#4	7'-1	0"	136				
wV	26	#5	11'	-4"	307				
Reinfor	cing Steel			Lb	4,342				
Class "	C" Concret	e		CY	22.1				

	TYPE	Tx34	Girder	5		
Bar	No.	Size	Len	gth	Weight	
А	10	#11	35'-	-0"	1,860	
D (7)	2	#9	1'-	8"	11	
Н	8	#6	35'-	-8"	429	
Г	18	#6	4'-	0"	108	
s	33	#5	11'-	-6"	396	
U	4	#6	8'-	1"	49	
V	35	#5	12'-	-4"	450	
wH1	14	#6	14'-	·5"	303	
wH2	20	#6	12'-	·8"	381	
wS	28	#4	7'-1	0"	147	
wV	28	#5	12'-	-4"	360	
Reinford	cing Steel			Lb	4,494	
Class "0	C" Concret	e		CY	23.9	

	TYPE	Tx40	Girder	S		
Bar	No.	Size	Len	gth	Weight	Bar
А	10	#11	35'	-0"	1,860	А
D(7)	2	#9	1'-	8"	11	D
н	10	#6	35'	-8"	536	н
L	18	#6	4'-	0"	108	L
S	33	#5	11'	-6"	396	S
U	4	#6	8'-	8'-1"		U
V	35	#5	13'	13'-4"		V
wH1	14	#6	16'	-5"	345	wH1
wH2	24	#6	14'	-8"	529	wH2
wS	32	#4	7'-1	0"	167	wS
wV	32	#5	13'	-4"	445	wV
Reinfor	cing Steel		•	Lb	4,933	Rein
Class "(C" Concret	e		CY	26.4	Class

 Bar
 No.

 A
 10

 D
 7

 2
 1

 H
 10

 L
 18

 S
 33

 U
 4

 V
 35

 wH1
 14

 wH2
 24

 wS
 34

 wV
 34

 wV
 34

 a
 1

 wV
 34

 c
 2

 wV
 34

 a
 2

 wV
 34

 a
 34

 wV
 34

#6

#5

#6

#6

#4

#5

8'-1"

14'-4"

17'-5"

15'-8"

7'-10"

14'-4"

Lb

CY

49

523

366

565

178

508

5,100

28.4

U

V

wH1

wH2

wS

wV

4

35

14

28

38

38

Reinforcing Steel

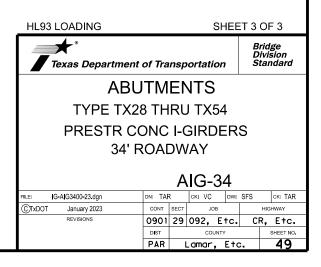
Class "C" Concrete

(7) Omit Dowels D at end of multi-span unit. Adjust reinforcing steel total accordingly.

(13) Quantities shown are for one abutment only (with approach slab). With no approach slab, add 1.4 CY Class "C" concrete and 214 lbs reinforcing steel for 4 additional Bars H.

DATE

		(12)								
Tx46 (Girder	s			TYPE	Tx54	Girder	s		
Size	Len	gth	Weight	Bar	No.	Size	Len	Weight		
#11	35'-0" 1,860			A	10	#11	35'	35'-0"		
#9	1'-	8"	11	D (7)	2	#9	1'-8"		11	
#6	35'-	-8"	536	н	12	#6	35'	-8"	643	
#6	4'-	0"	108	L	18	#6	4'-	0"	108	
#5	11'-	-6"	396	S	33	#5	11'	-6"	396	
#6	8'-	1"	49	U	4	#6	8'-	1"	49	
#5	14'-	-4"	523	V	35	#5	15'	-8"	572	
#6	12'-5"		261	wH1	14	#6	13'-5"		282	
#6	10'-8"		385	wH2	28	#6	11'	-8"	491	
#4	7'-10"		126	wS	26	#4	7'-1	0"	136	
#5	14'-	-4"	359	wV	26	#5	15'-	-8"	425	
		Lb	4,614	Reinford	cing Steel			Lb	4,973	
;		CY	24.6	Class "(C" Concret	е		CY	26.8	
		(12)		-						
Tx46 (Girder	s			TYPE	Tx54 (Girder	s		
Size	Len	gth	Weight	Bar	No.	Size	Len	gth	Weight	
#11	35'-	-0"	1,860	А	10	#11	35'	-0"	1,860	
#9	1'-	8"	11	D (7)	2	#9	1'-	8"	11	
#6	35'-	-8"	536	н	12	#6	35'	-8"	643	
#6	4'-	0"	108	L	18 #6 4'-0'			0"	108	
#5	11'-	-6"	396	S	33	#5	11'	396		
			1							



#6

#5

#6

#6

#4

#5

8'-1"

15'-8"

19'-5"

17'-8"

7'-10"

15'**-**8"

Lb

CY

49

572

408

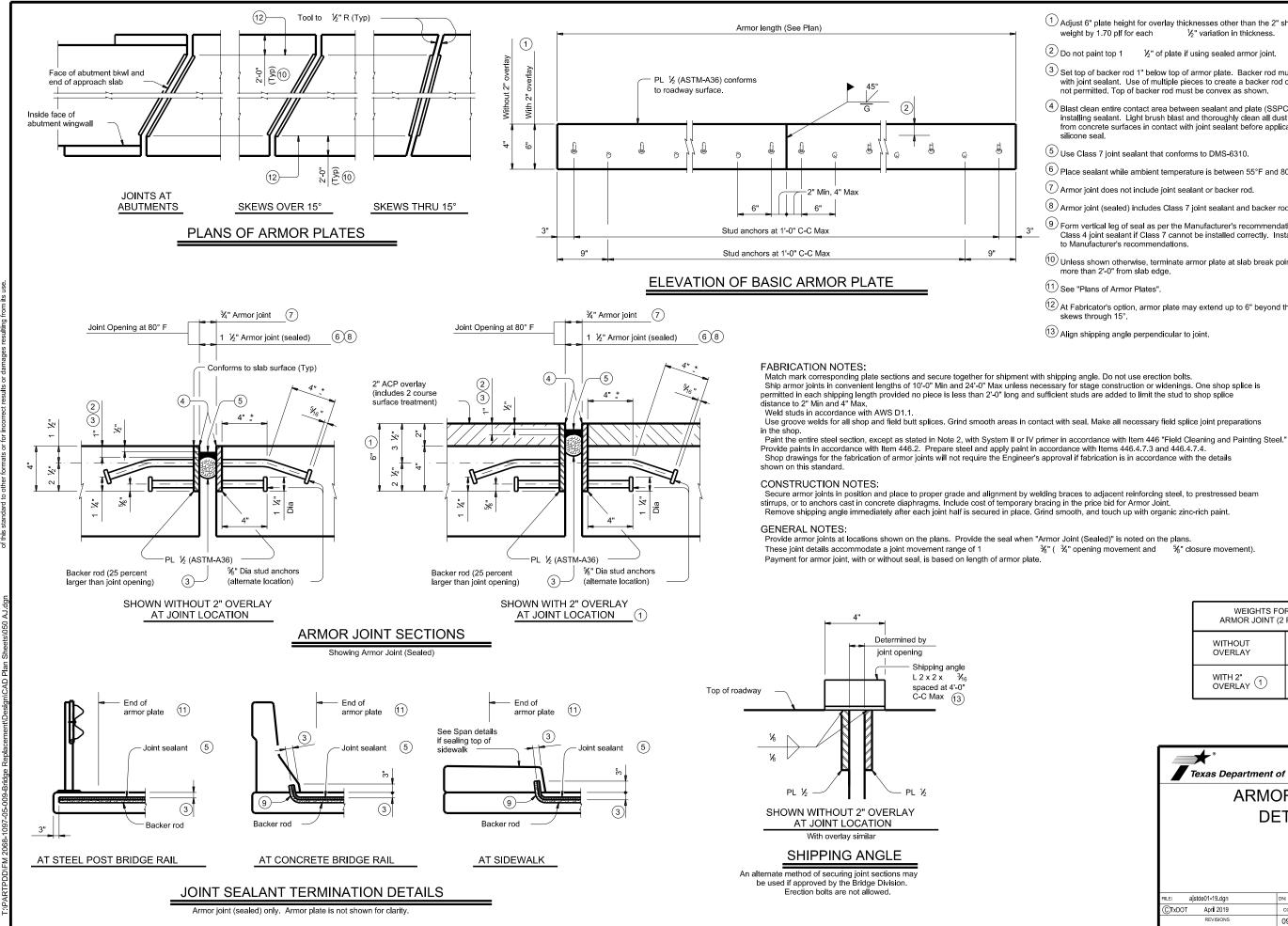
743

199

621

5,610

31.6



warranty of JISCLAIMER: The use of this standard is governe ind is made by TxDOT for any purpo:

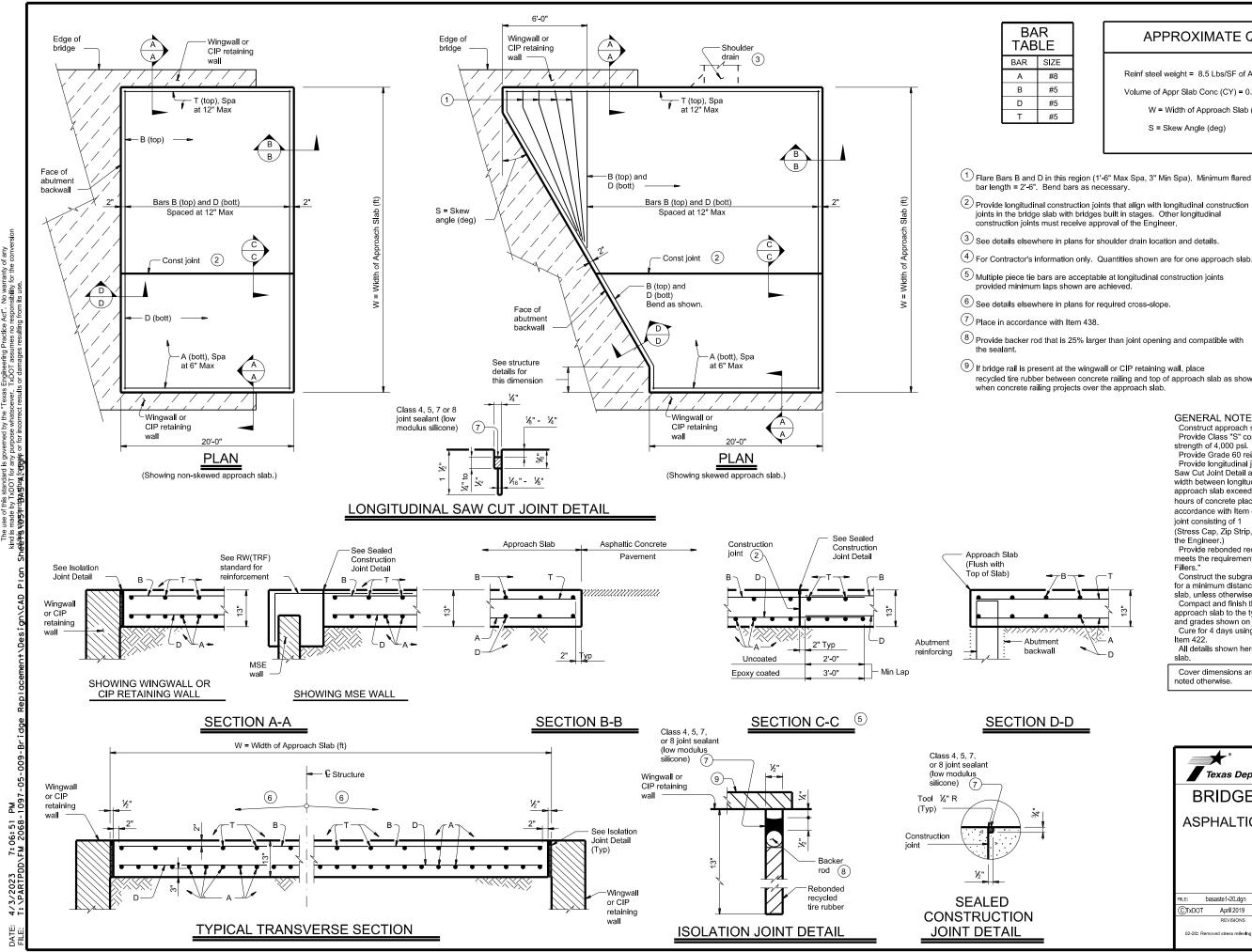
> Σ 7.06.49

- (1) Adjust 6" plate height for overlay thicknesses other than the 2" shown. Adjust weight by 1.70 plf for each 1/2" variation in thickness.
- 2 Do not paint top 1 ½" of plate if using sealed armor joint.
- 3 Set top of backer rod 1" below top of armor plate. Backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown
- (4) Blast clean entire contact area between sealant and plate (SSPC-SP10) before installing sealant. Light brush blast and thoroughly clean all dust and debris from concrete surfaces in contact with joint sealant before application of silicone seal.
- (5) Use Class 7 joint sealant that conforms to DMS-6310.
- $^{(6)}$ Place sealant while ambient temperature is between 55°F and 80°F and is rising.
- (7) Armor joint does not include joint sealant or backer rod.
- 8 Armor joint (sealed) includes Class 7 joint sealant and backer rod.
- (9) Form vertical leg of seal as per the Manufacturer's recommendations. Use Class 4 joint sealant if Class 7 cannot be installed correctly. Install according to Manufacturer's recommendations.
- (10) Unless shown otherwise, terminate armor plate at slab break point if break is more than 2'-0" from slab edge.
- (1) See "Plans of Armor Plates".
- 12 At Fabricator's option, armor plate may extend up to 6" beyond this point for skews through 15°.
- (13) Align shipping angle perpendicular to joint.

 $\frac{3}{8}$ " ($\frac{3}{4}$ " opening movement and $\frac{5}{8}$ " closure movement).

WEIGHTS FO ARMOR JOINT (2	
WITHOUT OVERLAY	16.10 plf
WITH 2" OVERLAY	22.90 plf

				Div	dge ision				
Texas Departr		Standard							
ARMOR JOINT									
	DETAILS								
		A	۲ſ						
n⊾e: ajstde01-19.dgn	DN: TxDOT	ск: ТхДОТ		XDOT	ск: ТхDOT				
ਸ⊾e: ajstde01-19.dgn ⓒTxDOT April 2019	DN: TXDOT	ск: ТхDOT			CK: TXDOT				
, ,		ск: ТхDOT т јов	ow: 1	н					
CTxDOT April 2019	CONT SEC	ск: ТхDOT т јов	DW: 1	н	GHWAY				



PN PN 7:06:51 .FM 2068-4/3/2023 T. . DARTPI

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APPROXIMATE QUANTITIES

4

Reinf steel weight = 8.5 Lbs/SF of Approach Slab

Volume of Appr Slab Conc (CY) = 0.802W + 0.02W² Tan S

W = Width of Approach Slab (ft)

S = Skew Angle (deg)

recycled tire rubber between concrete railing and top of approach slab as shown

GENERAL NOTES:

Construct approach slab in accordance with Item 422. Provide Class "S" concrete with a minimum compressive strength of 4,000 psi. Provide Grade 60 reinforcing steel.

Provide longitudinal joints as shown on the Longitudinal Saw Cut Joint Detail at lane lines and shoulders when width between longitudinal construction joints or edges of approach slab exceeds 16 feet. Saw cut joints within 24 hours of concrete placement to a depth of 1 %" and seal in accordance with Item 438. Alternately, provide a controlled joint consisting of 1 $\frac{1}{2}$ " vinyl or plastic joint former (Stress Cap, Zip Strip, Stress Lock, or equal as approved by the Engineer.)

1/3" rebonded

Provide rebonded recycled tire rubber joint filler that meets the requirements of DMS-6310. "Joint Sealants and Fillers "

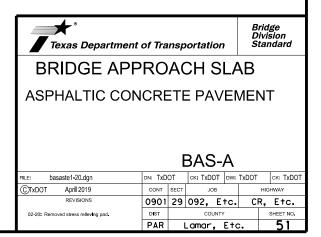
Construct the subgrade or subbase away from the bridge for a minimum distance of 100 feet prior to the approach slab, unless otherwise indicated on the plans.

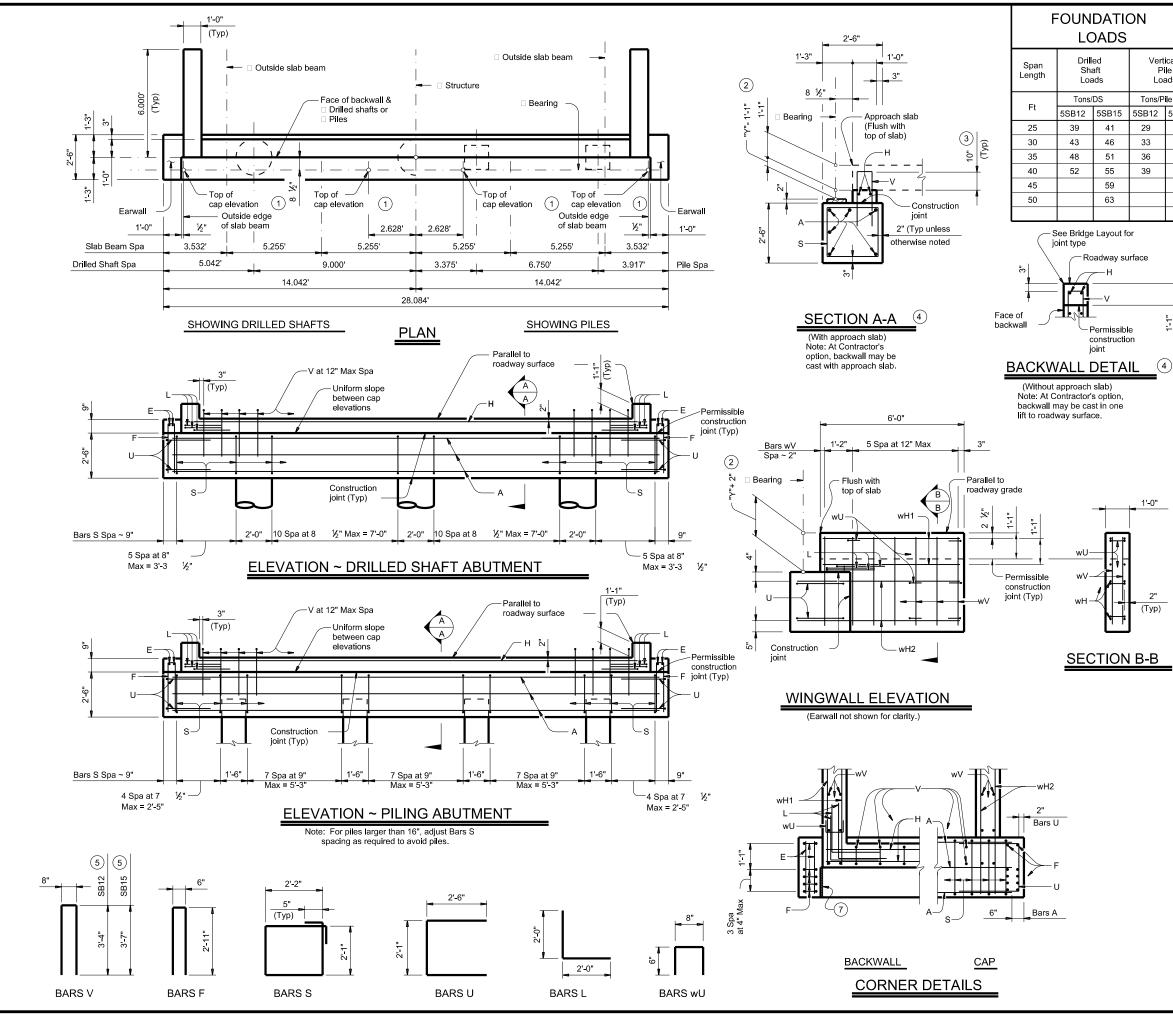
Compact and finish the subgrade or foundation for the approach slab to the typical cross-section and to the lines and grades shown on the plans.

Cure for 4 days using water or membrane curing per Item 422.

All details shown herein are subsidiary to bridge approach slab

Cover dimensions are clear dimensions, unless noted otherwise





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	•									
3	Vertical Pile Loads									
6	Tons/P	ile								
SB15	5SB12	5SB15								
41	29	31								
46	33	34								
51	36	38								
55	39	41								
59		44								
63		47								

	TABLE OF ESTIMATED(6)										
		C	QUANTI	TIES							
Bar	No.	Size	Length	5	Weight	(5)					
Dai	NU.	Size	5SB12	5SB15	5SB12	5SB15					
А	6	#11	27'-1"	27'-1"	863	863					
Е	4	#4	2'-2"	2'-2"	6	6					
F	10	#4	6'-4"	6'-4"	43	43					
н	2	#5	25'-8"	25'-8"	54	54					
L	. 6 #6 4'-0" 4'-0" 36 36										
S	34	#4	9'-4"	9'-4"	212	212					

7'-1"

7'-4"

5'-8"

6'-11"

1'-8"

3'-10"

7'-1"

7'-10"

6'-11"

5'-8"

1'-8"

4'-1"

Lb

CY

43

191

68

83

14

112

1,725

8.8

43

68

83

14

119

1,745

9.2

204

(1) Top of cap elevations are based on section depths shown on Span Details.

#6

#5

#6

#6

#4

#5

4

25

8

8

12

28

Reinforcing Steel

CI "C" Conc (Abut)

(2) See Span Details for "Y".

S

U

V

wH1

wH2

wU

wV

(3) Increase as required to maintain 3" from finished grade.

- (4) See Bridge Layout to determine if approach slab is present.
- 5 See Bridge Layout for beam type used in the superstructure.
- 6 Quantities shown are for one abutment only (with approach slab). Without approach slab, add 1.0 CY Class "C" concrete and 54 Lb reinforcing steel for 2 additional Bars H.
- (7) ½" preformed bituminous fiber material between slab beam and earwall. Bond to earwall with an approved adhesive. Cast inside face of earwall perpendicular to cap. (Typ)

GENERAL NOTES:

- Designed according to AASHTO LRFD Bridge Design Specifications. Designed for a normal embankment header slope
- of 3:1 and a maximum span length of 50 feet. See Bridge Layout for header slope and foundation type, size, and length.
- See Common Foundation Details (FD) standard sheet for all foundation details and notes.
- See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment
- details, if applicable. See applicable rail details for rail anchorage in wingwalls
- These abutment details may be used with standard SPSB-24 only.

Cover dimensions are clear dimensions, unless noted otherwise.

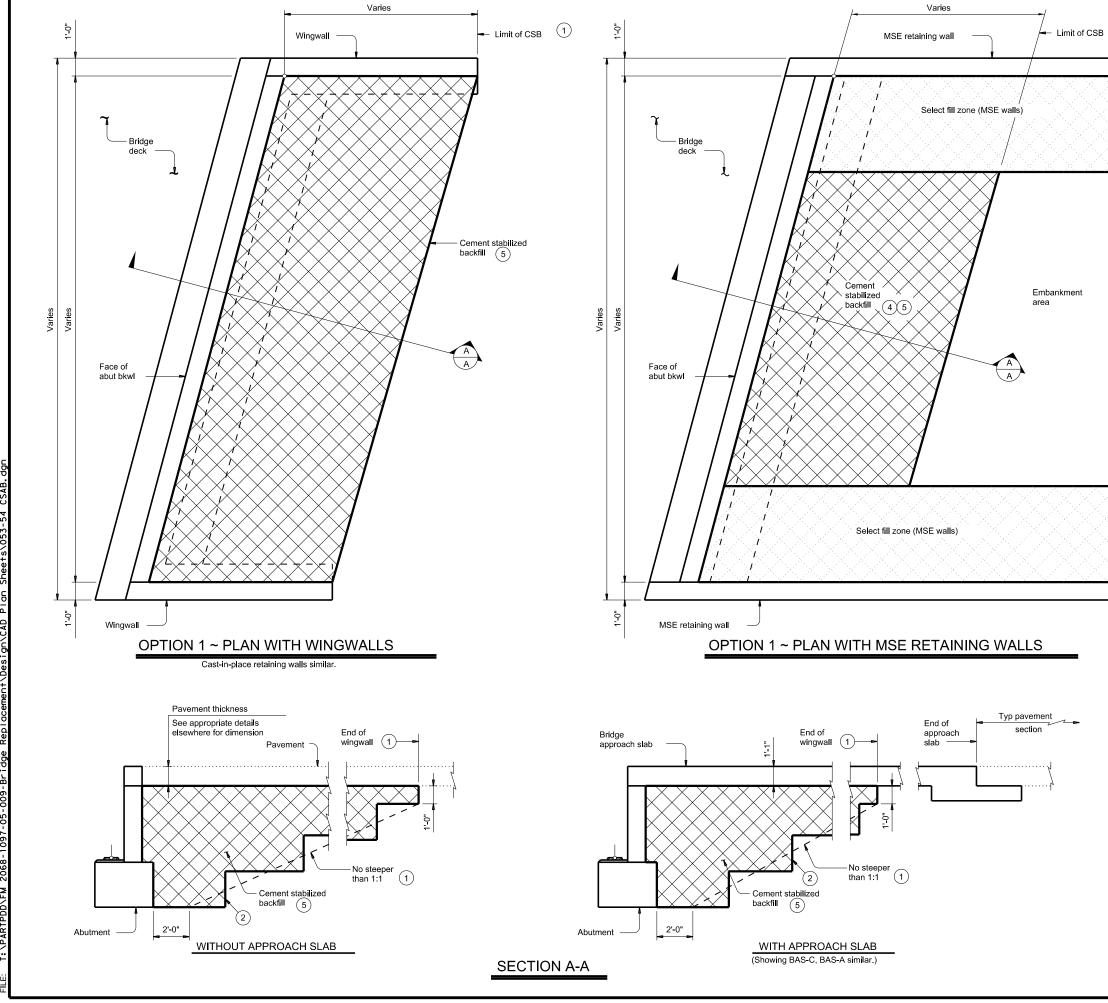
Reinforcing bar dimensions shown are out-to-out of bar.

MATERIAL NOTES:

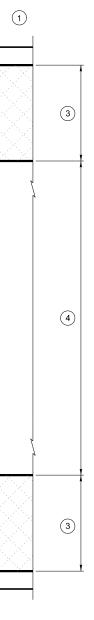
- Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere in
- the plans. Provide Grade 60 reinforcing steel.

HL93 LOADING





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- (1) Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.
- 2 Bench backfill as shown with 12" (approximate) bench depths.
- Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.
- When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.
- (5) If shown in the plans, flowable backfill can be used as a substitute for cement stabilized backfill with the following constraints:

a). If flowable backfill is to be placed over MSE backfill, then a filter fabric will be placed over the MSE backfill prior to placement of the flowable fill, and b). Place flowable fill in lifts not exceeding 2 feet in height. Place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowablifty).

GENERAL NOTES:

See the Bridge Layout for selected Option. Option 1 is intended for construction only requiring plasticity index (PI) controlled embankment fill or excavation in competent soils/rocks in order to construct the abutment. Option 2 is intended for new construction requiring high plasticity embankment fill with a PI greater than 30 or pavement built in poor native soil. Poor soils are defined as high plasticity clays or expansive clays.

Construct abutment backfill in accordance with Item 400, "Excavation and Backfill for Structures".

Provide Cement Stabilized Backfill (CSB) meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments.

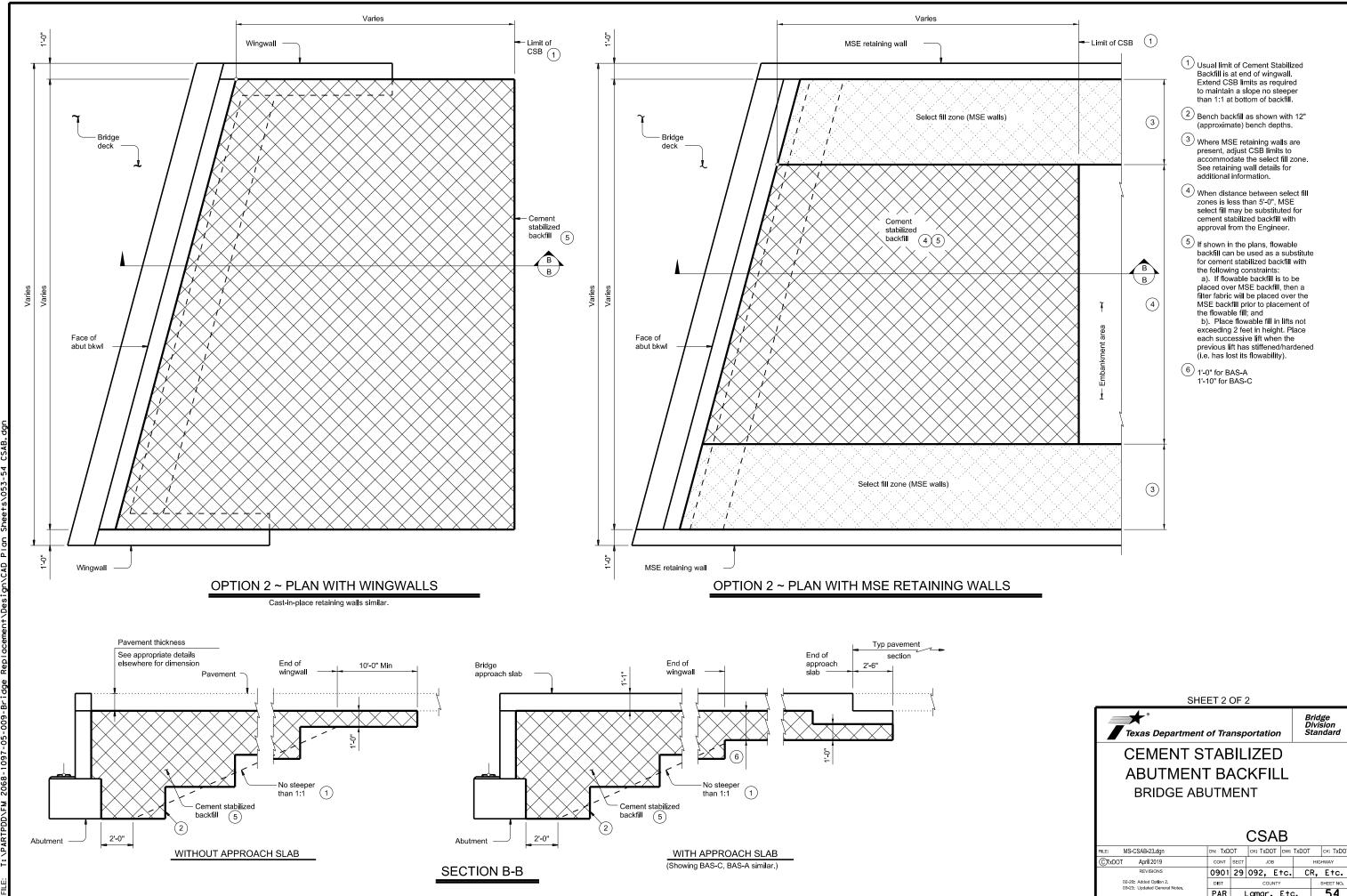
If required elsewhere in the plans, provide Flowable Backfill meeting the requirements of Item 401, "Flowable Backfill", to the limits shown at bridge abutments.

Details are drawn showing left forward skew. See Bridge Layout for actual skew direction. These details do not apply when Concrete Block

These details do not apply when Concrete Block retaining walls are used in lieu of wingwalls.

SHEET 1 OF 2

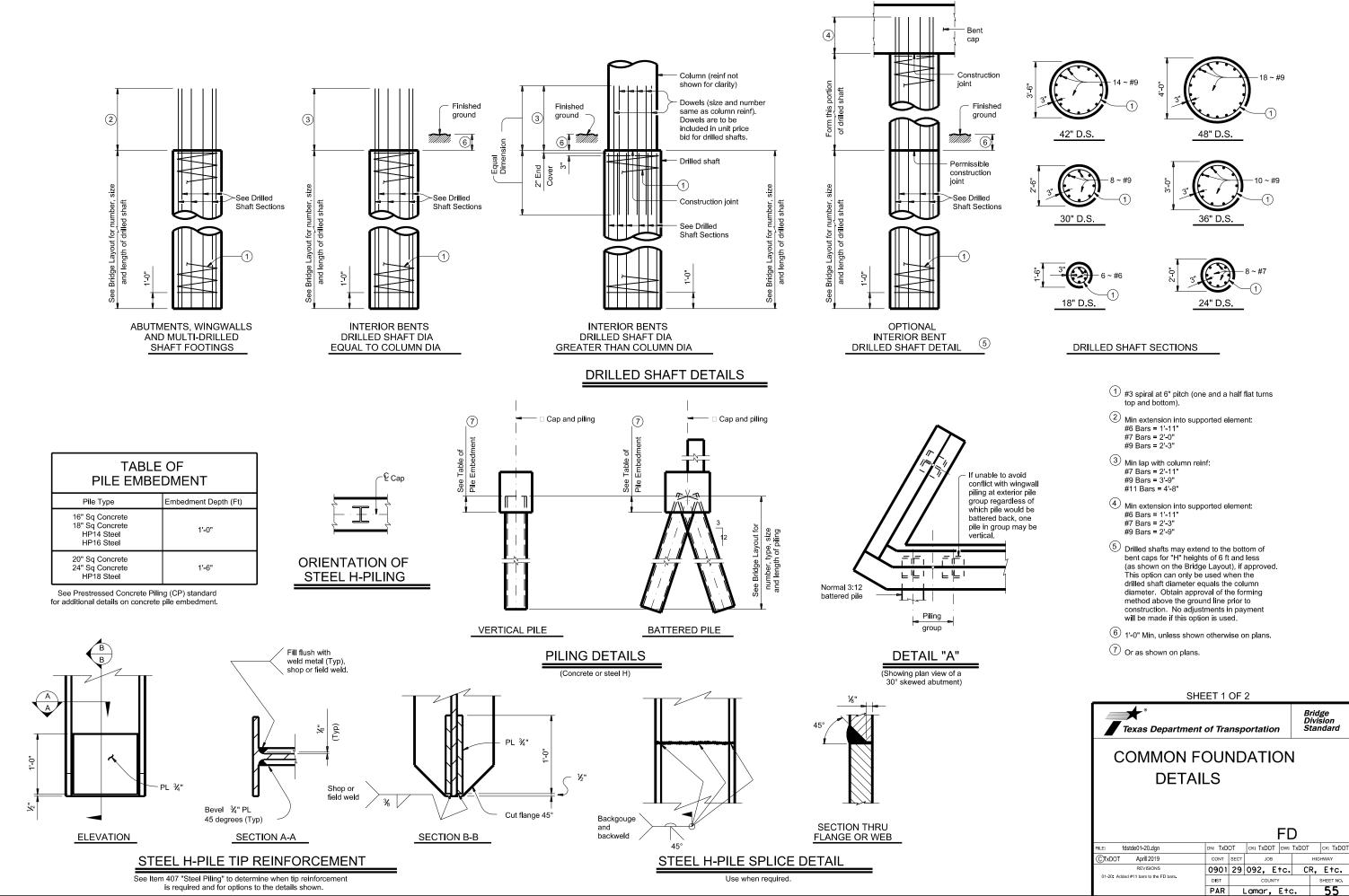
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CEMENT STABILIZED ABUTMENT BACKFILL BRIDGE ABUTMENT									
			CSA	В					
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CTxDOT April 2019	CONT	SECT	JOB			HIGHWAY			
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02-20: Added Option 2. 03-23: Updated General Notes.	DIST	COUNTY			SHEET NO.				
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CTxDOT April 2019	CONT	SECT	JOB		HIGHWAY		
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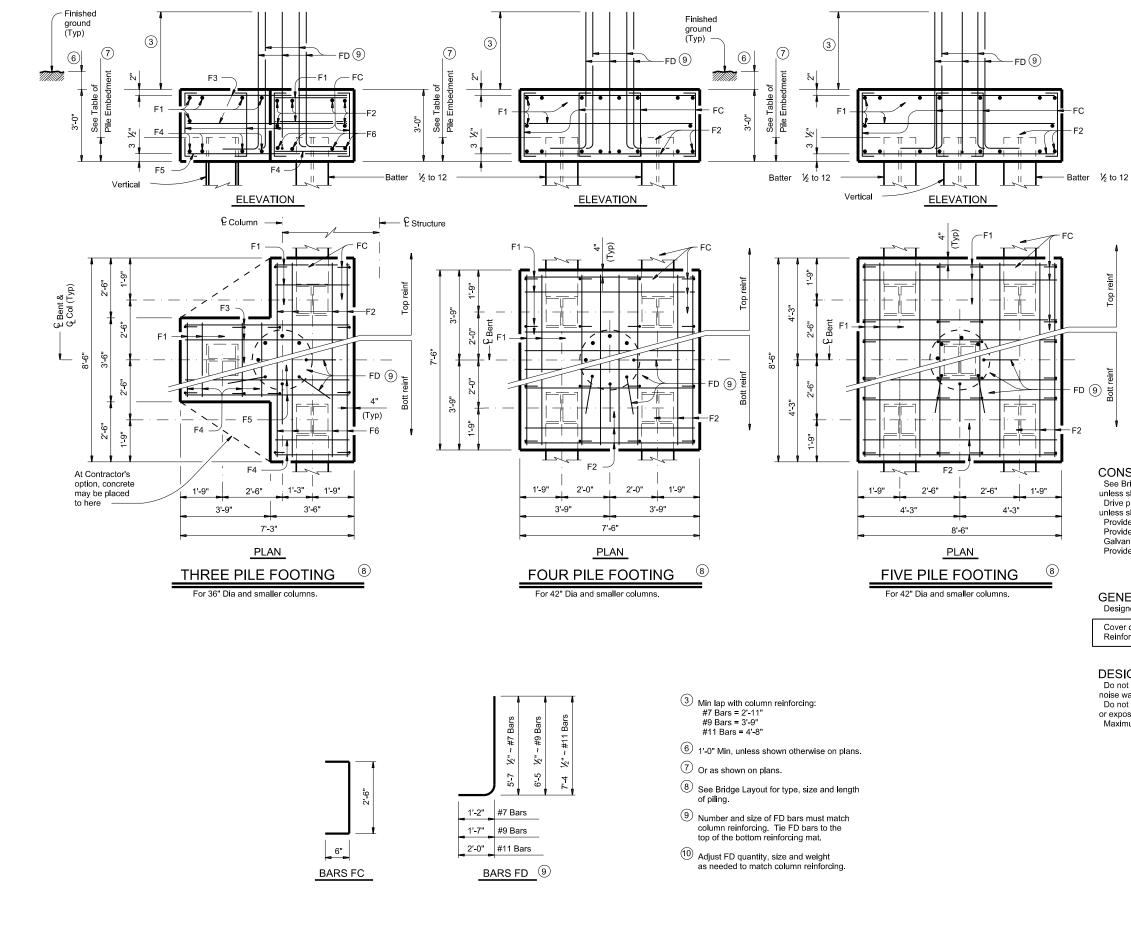
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	bent caps for \square neights of 6 it and less
	(as shown on the Bridge Layout), if approved.
	This option can only be used when the
	drilled shaft diameter equals the column
	diameter. Obtain approval of the forming
	method above the ground line prior to
	construction. No adjustments in payment
	will be made if this option is used.
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SHEET 1 OF 2								
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COMMON FOUNDATION DETAILS FD								
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TABLE OF FOOTING
QUANTITIES FOR
30" COLUMNS

ONE 3 PILE FOOTING											
Bar	No.	Size	Length	Weight							
F1	11	#4	3'- 2"	23							
F2	6	#4	8'- 2"	33							
F3	6	#4	6'- 11"		28						
F4	8	#9	3'- 2"		86						
F5	4	#9	6'- 11"		94						
F6	4	#9	8'- 2"		111						
FC	12	#4	3'- 6"		28						
FD 1	8	#9	8'- 1"		220						
Reinfo	rcing St	eel		Lb	623						
Class	"C" Con	crete		CY	4.8						
		ONE 4 P	LE FOOTING	G							
Bar	No.	Size	Length		Weight						
F1	20	#4	7'- 2"		96						
F2	16	#8	7'- 2"		306						
FC	16	#4	3'- 6"		37						
FD 1	8	#9	8'- 1"		220						
Reinfo	rcing St	eel		Lb	659						
Class	"C" Con	crete		CY	6.3						
		ONE 5 P	LE FOOTING	G							
Bar	No.	Size	Length		Weight						
F1	20	#4	8'- 2"		109						
F2	16	#9	8'- 2"		444						
FC	24	#4	3'- 6"		56						
FD 1	8	#9	8'- 1"		220						
Reinforcing Steel Lb 829											
Class	"C" Con	crete		CY	8.0						

CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise. Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile

Drive pling under adutitient wingwalls to a minimum resistance of 10 10 unless shown otherwise. Provide Class C Concrete (fc = 3,600 psi), unless shown otherwise. Provide Grade 60 reinforcing steel. Galvanize reinforcing if shown elsewhere in the plans. Provide bar laps for drilled shaft reinforcing, where required, as follows:

Uncoated or galvanized (#6) ~ 2'-6" Uncoated or galvanized (#7) ~ 2'-11"

Uncoated or galvanized (#9) ~ 3'-9"

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

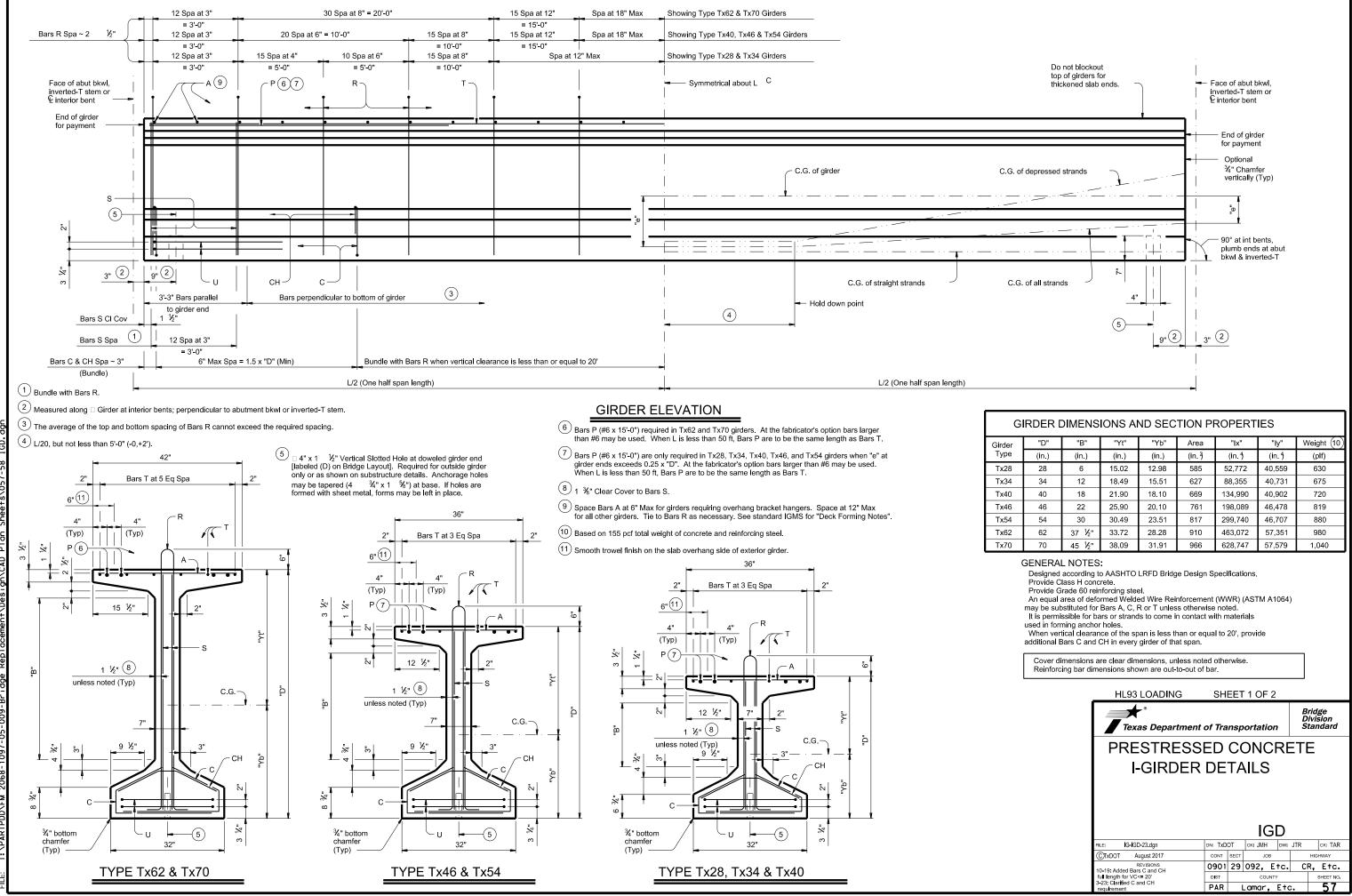
DESIGNER NOTES:

Do not use the drilled shaft details shown on this standard for retaining wall, noise wall, barrier, or sign foundations without structural evaluation. Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.

Maximum allowable pile loads for the footings shown are: 72 Tons/Pile with 24" Dia Columns 80 Tons/Pile with 30" Dia Columns

- - 100 Tons/Pile with 36" Dia Columns 120 Tons/Pile with 42" Dia Columns

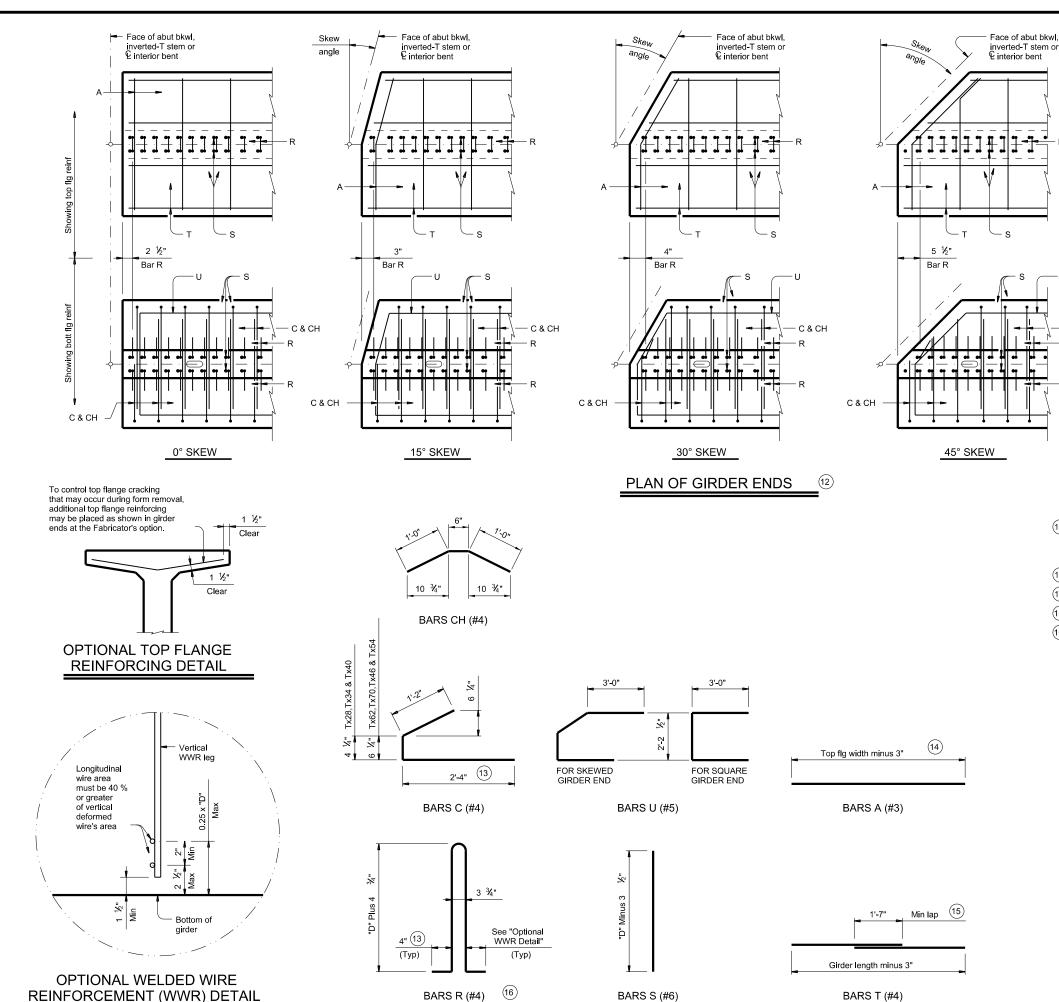
SHEET 2 OF 2								
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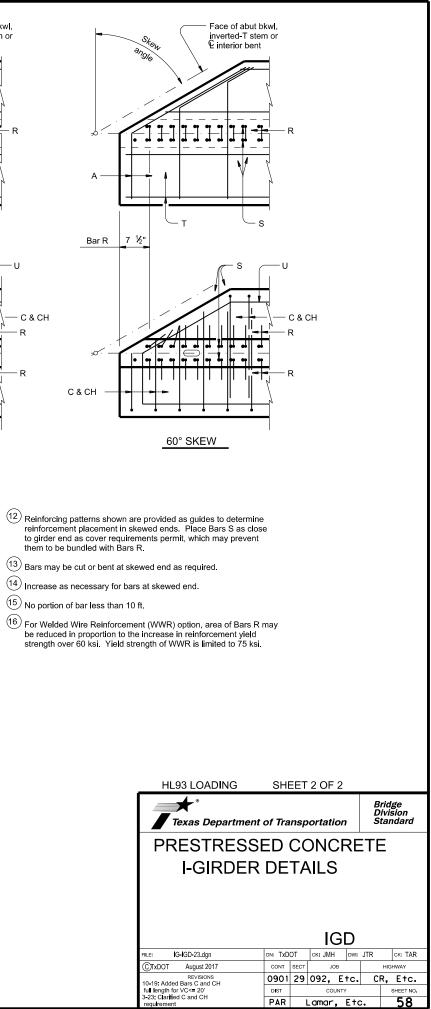
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		GIRDER DIMENSIONS AND SECTION PROPERTIES										
"D"	"B"	"Yt"	"Yb"	Area	"lx"	"ly"	Weight (10)					
(in.)	(in.)	(in.)	(in.)	(in. ³)	(in. 1)	(in. 1)	(plf)					
28	6	15.02	12.98	585	52,772	40,559	630					
34	12	18.49	15.51	627	88,355	40,731	675					
40	18	21.90	18.10	669	134,990	40,902	720					
46	22	25.90	20.10	761	198,089	46,478	819					
54	30	30.49	23.51	817	299,740	46,707	880					
62	37 ½"	33.72	28.28	910	463,072	57,351	980					
70	45 ½"	38.09	31.91	966	628,747	57,579	1,040					
	(in.) 28 34 40 46 54 62	(in.) (in.) 28 6 34 12 40 18 46 22 54 30 62 37 ½"	(in.) (in.) (in.) 28 6 15.02 34 12 18.49 40 18 21.90 46 22 25.90 54 30 30.49 62 37 ½" 33.72	(in.) (in.) (in.) (in.) 28 6 15.02 12.98 34 12 18.49 15.51 40 18 21.90 18.10 46 22 25.90 20.10 54 30 30.49 23.51 62 37 ½" 33.72 28.28	(in.) (in.) (in.) (in.) (in.) 28 6 15.02 12.98 585 34 12 18.49 15.51 627 40 18 21.90 18.10 669 46 22 25.90 20.10 761 54 30 30.49 23.51 817 62 37 ½" 33.72 28.28 910	(in.) (in.) (in.) (in.) (in.) (in.) 28 6 15.02 12.98 585 52.772 34 12 18.49 15.51 627 88,355 40 18 21.90 18.10 669 134,990 46 22 25.90 20.10 761 198,089 54 30 30.49 23.51 817 299,740 62 37 ½" 33.72 28.28 910 463,072	(in.) (in.) <th< td=""></th<>					

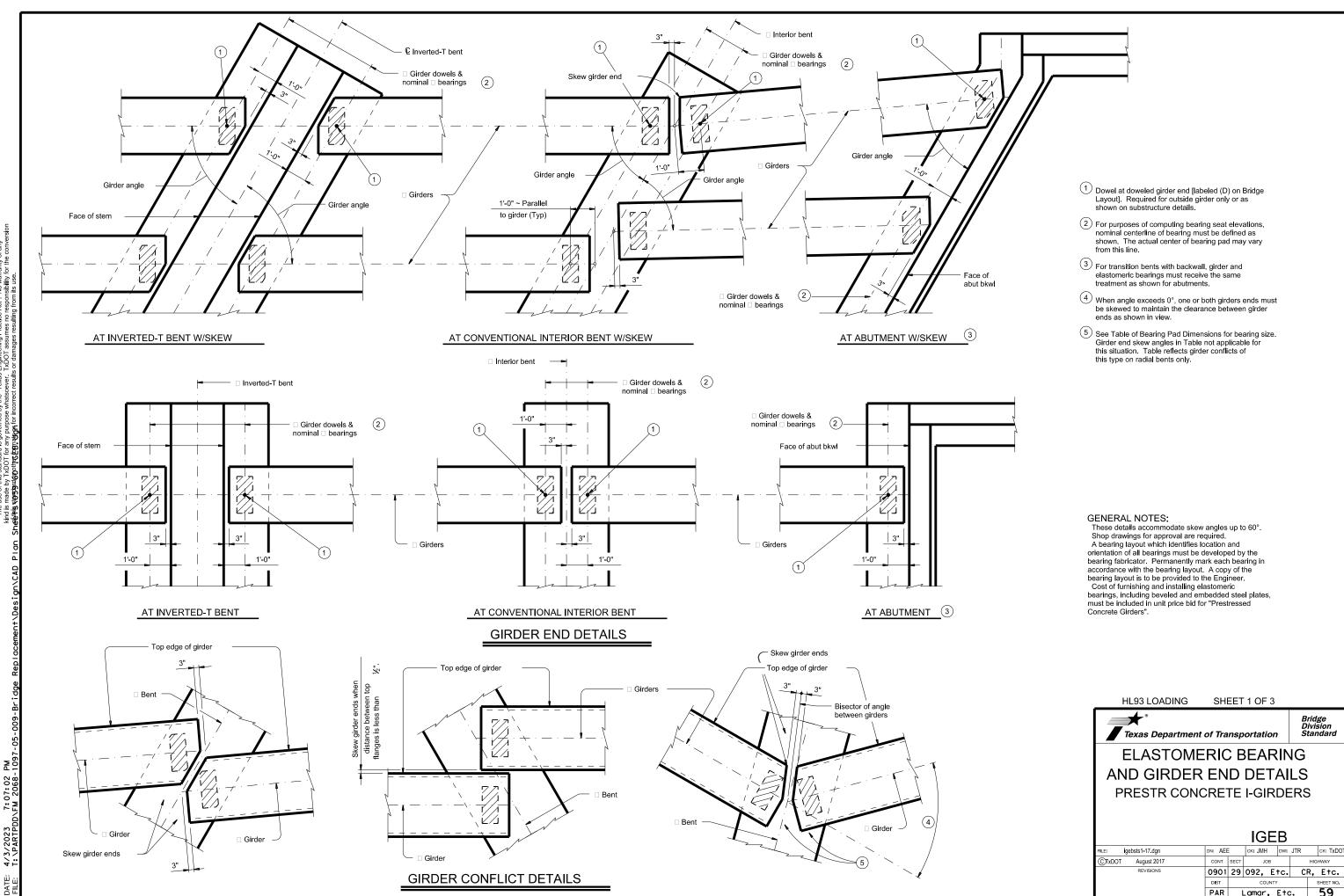


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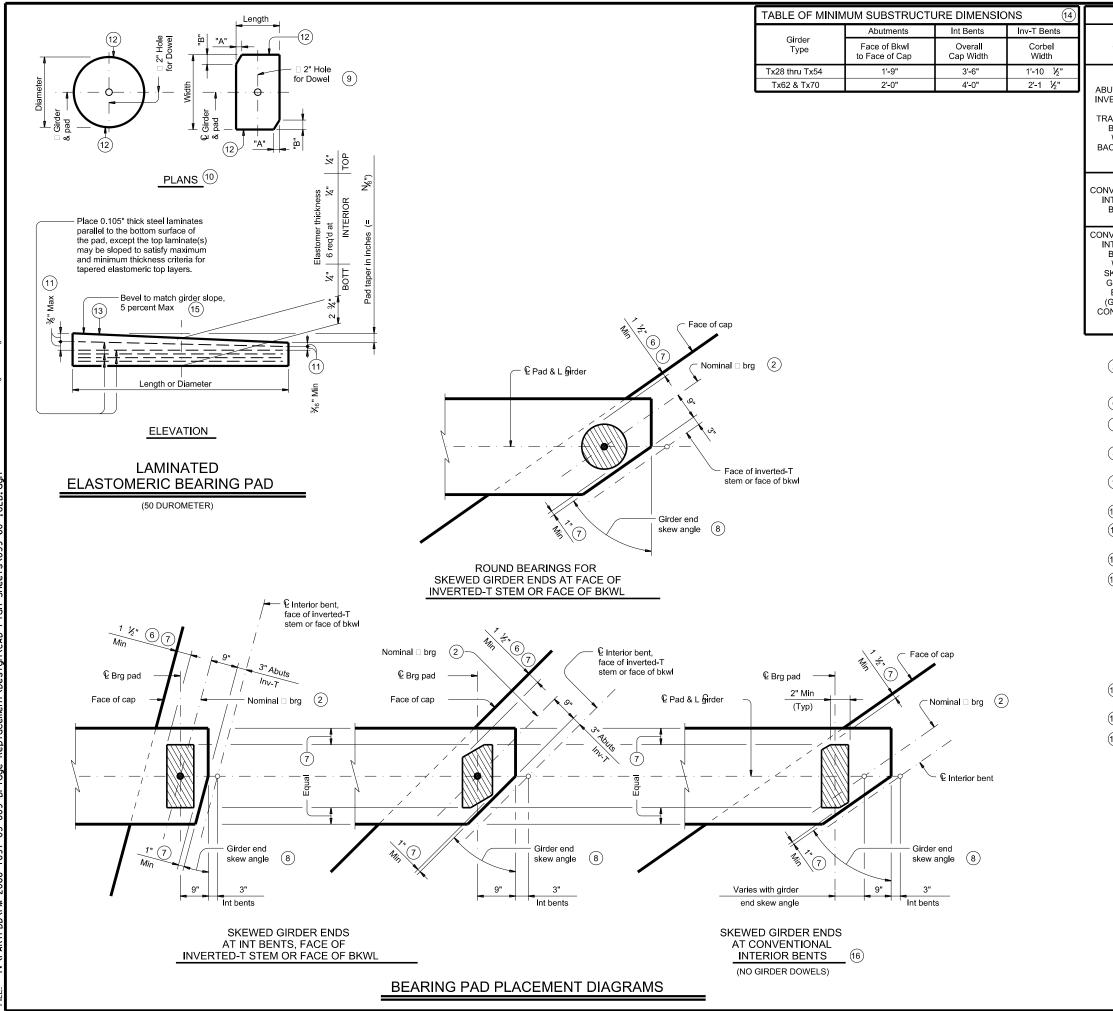
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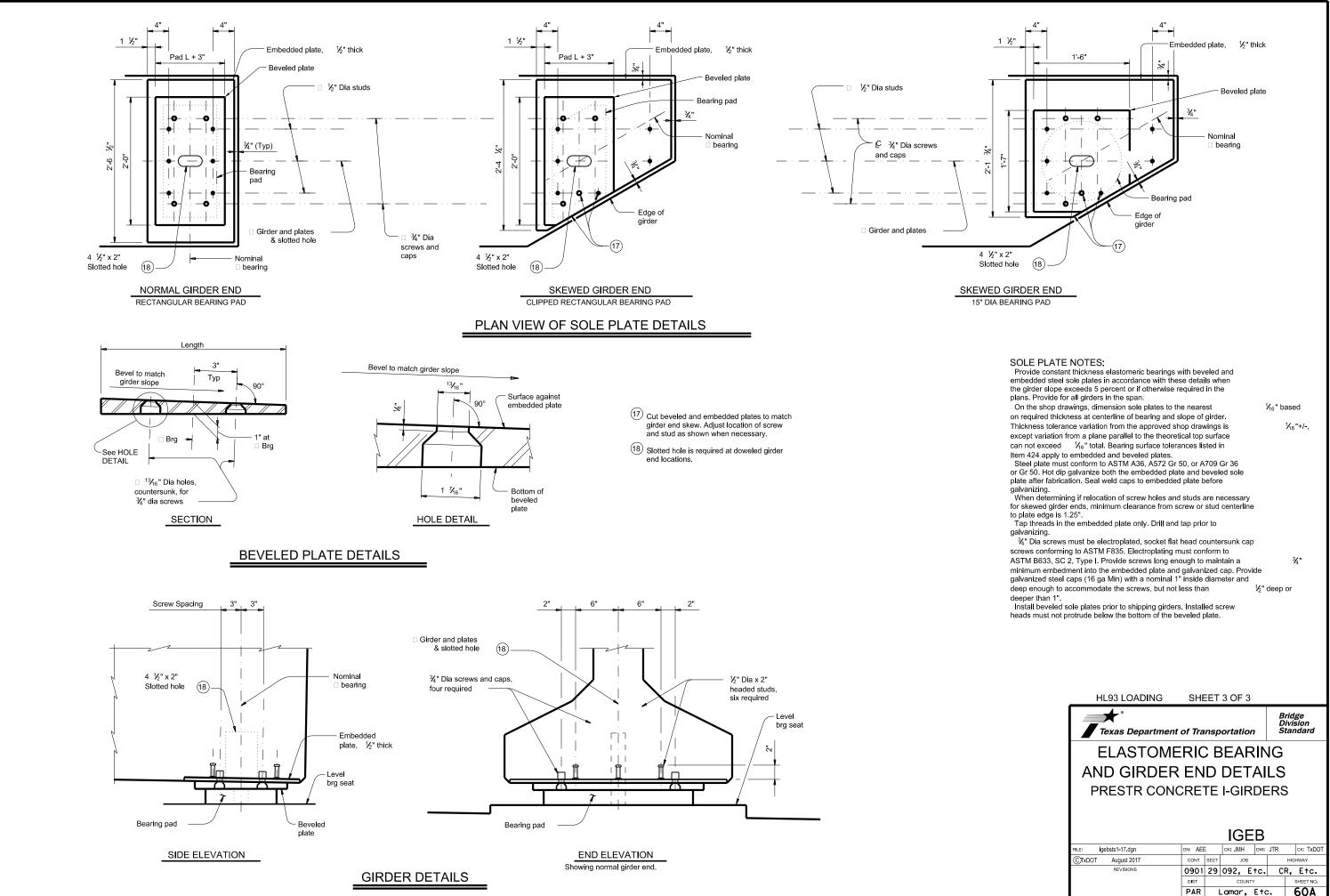
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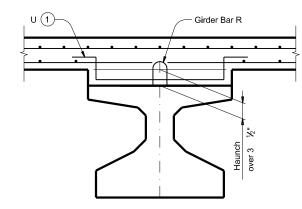
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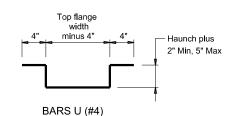
TABLE OF BEARING PAD DIMENSIONS											
Bent	Girder	Bearing	Girder End								
Bent Type	Type	Type (13)	Skew Angle Range	Lgth x Wdth	Dimensions						
		(13) G-1-"N"	0° thru 21°	8" x 21"	"A"	"B"					
THEFT	Tx28,Tx34,	G-1- N G-2-"N"	21°+ thru 30°	8 x 21 8" x 21"	"A" 	2 1/2"					
UTMENTS, 'ERTED-T	Tx40,Tx46 & Tx54	G-3-"N"	30°+ thru 45°	9" x 21"	-	4 ½"					
AND ANSITION	G 1/10 1	G-4-"N"	45°+ thru 60°	15" Dia							
BENTS	T	G-5-"N"	0° thru 21°	9" x 21"							
WITH CKWALLS	Tx62 &	G-6-"N"	21°+ thru 30°	9" x 21"		2 1/2"					
	Tx70	G-7-"N" G-8-"N"	30°+ thru 45° 45°+ thru 60°	10" x 21" 10" x 21"	-	4 ½" 4 ¼"					
	Ty20 Ty24										
VENTIONAL	Tx28,Tx34, Tx40,Tx46										
ITERIOR BENTS	& Tx54	G-1-"N"	0° thru 60°	8" x 21"							
	Tx62 & Tx70	G-5-"N"	0° thru 60°	9" x 21"							
VENTIONAL	Tx28,Tx34,	G-1-"N"	0° thru 18°	8" x 21"							
BENTS	Tx40,Tx46	G-2-"N" G-9-"N"	18°+ thru 30° 30°+ thru 45°	8" x 21" 8" x 21"	-	2 ½" 3"					
WITH KEWED	& Tx54	G-10-"N"	45°+ thru 60°	9" x 21"		3 1/3"					
GIRDER		G-10- N G-5-"N"	0° thru 18°	9" x 21"							
ENDS GIRDER	Tx62	G-5-"N"	18°+ thru 30°	9" x 21"							
NFLICTS)	& Tx70	G-11-"N"	30°+ thru 45°	9" x 21"	1 ½"	1 1⁄2"					
(16)		G-12-"N"	45°+ thru 60°	9" x 21"	3"	1 ¾"					
 6 3" for inverted-T. 7 Place centerline pad as near nominal centerline bearing as possible between limits shown. 8 Girder end skew angle is equal to 90" minus the girder angle except at some conflicting girders. 9 Provide 2" dia hole only at locations required. See Substructure details for location. (0) See Table of Bearing Pad Dimensions for dimensions. (1) Maximum and minimum layer thicknesses shown are for elastomer only, on tapered layers. (2) Locate Permanent Mark here. (3) Indicate BEARING TYPE on all pads. For tapered pads, locate BEARING TYPE on the high side. The Fabricator must include the value of "N" (amount of taper in ½" increments) in this mark. Examples: N=0, (for 0" taper) N=2, (for ½" taper) (etc.) Fabricated pad top surface slope must not vary from plan girder slope by more than (<u>colWMN</u>.) (4) Substructure dimensions must satisfy the minimums provided to accommodate the elastomeric bearing shown on this standard. (5) See sheet 3 of 3 for beveled plate use when slopes exceed 5 percent. (6) If girder end is skewed for a girder conflict at an interior bent and a beveled sole plate is required, use bearing type for abutments at this case. 											
	HL93 LOADING SHEET 2 OF 3										
	ŀ	-	-	f Transportation							
		ELA	ASTOME	RIC BEARIN	١G						
			GIRDER	END DETA	IS						
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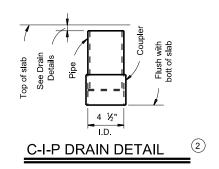


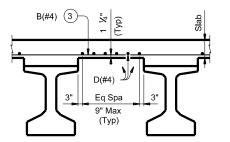
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HAUNCH REINFORCING DETAIL



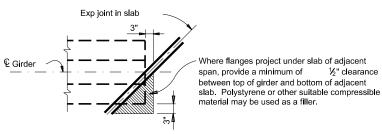




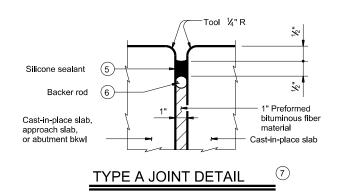
TYPICAL PART TRANSVERSE SLAB SECTION WITHOUT PCP

(4)

Top reinforcing steel not shown for clarity.



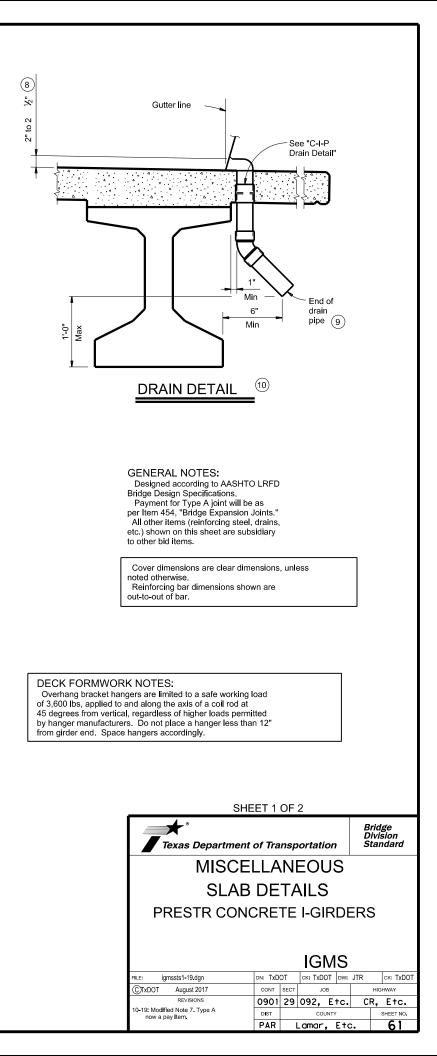
TREATMENT AT GIRDER END FOR SKEWED SPANS

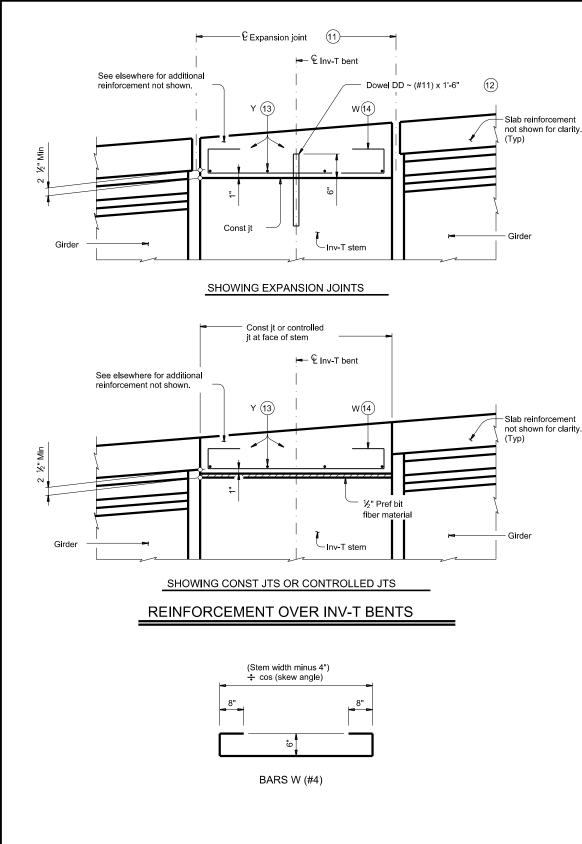


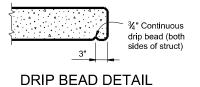
- (1) Space Bars U with girder Bars R in all areas where measured haunch exceeds 3
- 2 Roughen outside of PVC with coarse rasp or equal to ensure bond with cast-in-place concrete.

1⁄2".

- (3) Bars B(#4) spaced at 9" Max with 2" end cover. Overhang option, Contractor's may end alternating bars B(#4) at centerline outside girder.
- (4) Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated ~ #4 = 1'-7" Epoxy coated ~ #4 = 2'-5"
- (5) Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- 6 1 ¼" backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- The maximum distance between Type A expansion joints is 100'. See Bridge Layout for location of joints.
- 8 Drain entrance formed in rail or sidewalk.
- (9) Water may not be discharged onto girders.
- (10) All drain pipe and fittings to be 4" diameter (Sch 40) PVC. See Item 481 "Pipe for Drains" for pipe, connections and solvent welding. Bend reinforcing steel to clear PVC 1". Drain length and location is as directed by the Engineer. Drains are not permitted over roadways or railways, or within 10-0" of bent caps. Degrease outside of exposed PVC, apply acrylic water base primer, then coat with same surface finishing material as used for outside girder face. Variations of the above designs, as required for the type of rail used and its location on the structure, may be installed with the approval and direction of the Engineer.



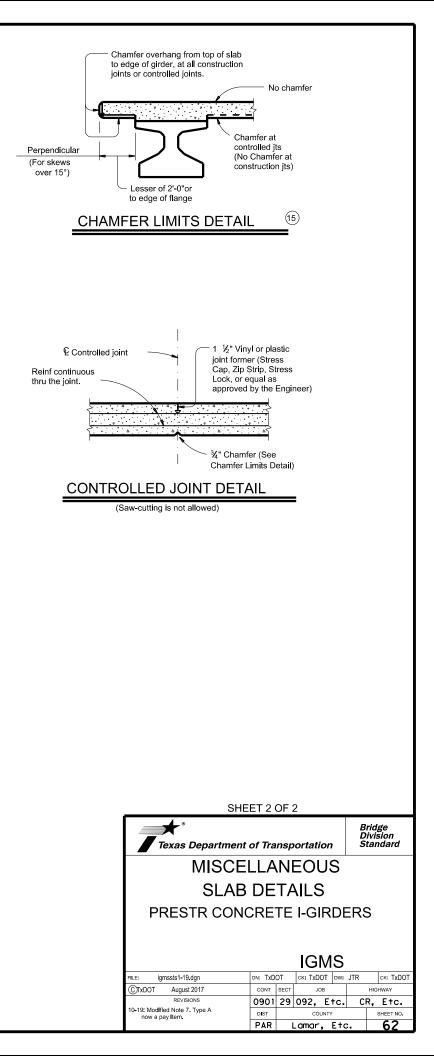




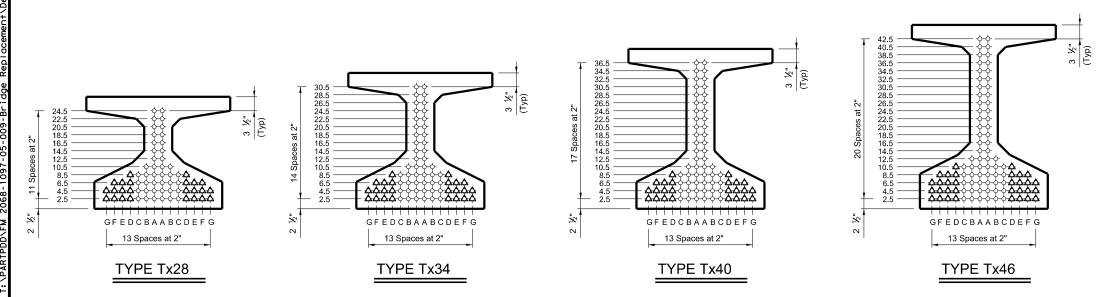


- (2) Dowels DD (#11) spaced at 5 Ft Max. See Inv-T bents for quantity and location.
- 13 Space Bars Y (#4) at 12" Max. Use 2" end cover. Number of Bars Y must satisfy spacing limit. Place parallel to bent.
- (14) Space Bars W at 12" Max (3" from end of cap). Tilt if necessary to maintain cover requirements. Place parallel to longitudinal slab reinforcement.
- (15) See Span details for type of joint and joint locations.

DATE



DESIGNED GIRDERS		DEPR	ESSED	CONCRETE		OPTIONAL DESIGN					LOAD RATING										
					PR	ESTRESS	SING STR/	ANDS		STR	AND			DESIGN	DESIGN	REQUIRED	LIVE			FACT	ORS
STRUCTURE	SPAN NO.	GIRDER NO.	GIRDER TYPE	NON- STD STRAND PATTERN	TOTAL NO.	SIZE	STRGTH	"e"	"e" END	PAT NO.		RELEASE STRGTH	MINIMUM 28 DAY COMP STRGTH f [°] C	LOAD COMP STRESS (TOP □) (SERVICE I)	LOAD TENSILE STRESS (BOTT □) (SERVICE III)	MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I)		TOR	STREM	NGTH I	SERVICE III
						(in)	(ksi)	(in)	(in)		(in)	(ksl)	(ksl)	fct(ksl)	fcb(ksl)	(klp-ft)	Moment	Shear	Inv	Opr	Inv
	40	ALL	Tx28		12	0.6	270	10.48	10.48			4.700	5.000	1.095	-1.501	1584	0.730	0.920	1.78	2.31	2.23
	45	ALL	Tx28		12	0.6	270	10.48	10.48			4.500	5.000	1.382	-1.829	1527	0.700	0.920	1.46	1.89	1.65
Type Tx28 Girders	50	ALL	Tx28		14	0.6	270	10.48	10.19	2	4.5	5.300	5.300	1.693	-2.204	1753	0.680	0.930	1.42	1.84	1.49
34' Roadway	55 60	ALL ALL	Tx28 Tx28		16 20	0.6 0.6	270 270	10.23 9.88	9.23 6.28	4	8.5 22.5	4.000 4.000	5.300 6.100	2.049 2.418	-2.615 -3.048	2038 2343	0.660 0.650	0.930 0.940	1.38 1.48	1.78 1.99	1.18 1.16
8.5" Slab	65	ALL	Tx28		20 24	0.6	270	9.88 9.65	6.28 6.31	4	22.5 24.5	4.000	6.200	2.418	-3.048	2343	0.630	0.940	1.48	1.99	1.16
	70	ALL	Tx28		24	0.6	270	9.56	7.10	4	24.5	5.400	6.700	3.254	-3.985	2033	0.620	0.940	1.44	1.84	1.02
	75	ALL	Tx28		32	0.6	270	9.11	5.73	6	24.5	5.900	7.000	3.734	-4.513	3306	0.610	0.950	1.27	1.71	1.05
	40	ALL	Tx34		10	0.6	270	13.01	13.01		21.0	4.000	5.000	0.863	-1.147	1714	0.750	0.900	1.70	2.20	2.38
	45	ALL	Tx34		12	0.6	270	13.01	13.01			4.000	5.000	1.088	-1.403	1917	0.730	0.910	1.73	2.24	2.18
	50	ALL	Tx34		14	0.6	270	13.01	13.01			5.100	5.100	1.342	-1.699	2123	0.710	0.910	1.68	2.18	2.01
	55	ALL	Tx34		14	0.6	270	13.01	13.01			4.900	5.000	1.607	-2.002	2116	0.690	0.910	1.40	1.81	1.52
Type Tx34 Girders	60	ALL	Tx34		14	0.6	270	13.01	12.44	2	6.5	4.000	5.000	1.907	-2.333	2420	0.670	0.920	1.16	1.50	1.06
34' Roadway 8.5" Slab	65	ALL	Tx34		18	0.6	270	12.57	11.23	4	10.5	4.000	5.000	2.216	-2.680	2747	0.660	0.920	1.31	1.75	1.12
	70	ALL	Tx34		22	0.6	270	12.28	7.92	4	28.5	4.000	5.600	2.565	-3.062	3093	0.650	0.930	1.36	1.93	1.13
	75	ALL	Tx34		24	0.6	270	12.18	9.51	4	20.5	4.700	5.700	2.921	-3.436	3414	0.630	0.930	1.23	1.89	1.04
	80	ALL	Tx34		28	0.6	270	12.01	8.30	4	30.5	5.100	6.000	3.303	-3.843	3772	0.620	0.930	1.30	1.96	1.04
	85	ALL	Tx34		32	0.6	270	11.64	7.89	6	26.5	5.700	6.500	3.721	-4.282	4149	0.610	0.930	1.35	1.85	1.01
	40	ALL	Tx40		10	0.6	270	15.60	15.60			4.000	5.000	0.719	-0.935	1780	0.780	0.890	1.93	2.50	2.89
	45	ALL	Tx40		12	0.6	270	15.60	15.60			4.000	5.000	0.894	-1.130	2096	0.750	0.890	2.00	2.60	2.74
	50	ALL	Tx40		12	0.6	270	15.60	15.60			4.000	5.000	1.102	-1.367	2286	0.730	0.900	1.61	2.08	2.09
	55	ALL	Tx40		14	0.6	270	15.60	15.60			4.300	5.000	1.329	-1.619	2488	0.710	0.900	1.63	2.12	1.95
Type Tx40 Girders	60	ALL	Tx40		14	0.6	270	15.60	15.60			4.200	5.000	1.565	-1.886	2511	0.700	0.900	1.35	1.75	1.50
34' Roadway	65 70	ALL ALL	Tx40 Tx40		16 18	0.6	270 270	15.35 15.16	15.35 14.27	4	8.5	5.000 4.000	5.000 5.000	1.828 2.115	-2.166 -2.473	2835 3194	0.680 0.670	0.910 0.910	1.37	1.77	1.42 1.20
8.5" Slab	70	ALL	Tx40 Tx40		20	0.6 0.6	270	15.00	14.27	4	0.5 12.5	4.000	5.000	2.115	-2.473	3194	0.650	0.910	1.35 1.31	1.76 1.76	1.20
	80	ALL	Tx40 Tx40		20	0.6	270	13.00	9.43	4	36.5	4.000	5.100	2.390	-3.103	3899	0.640	0.910	1.37	1.96	1.09
	85	ALL	Tx40		26	0.6	270	14.68	9.76	4	36.5	4.400	5.300	3.034	-3.435	4273	0.630	0.920	1.29	1.92	1.01
	90	ALL	Tx40		30	0.6	270	14.40	9.20	6	32.5	4.900	5.600	3.407	-3.814	4683	0.620	0.920	1.41	1.98	1.01
	95	ALL	Tx40		34	0.6	270	14.07	9.13	6	34.5	5.500	6.300	3.770	-4.184	5085	0.610	0.920	1.41	1.90	1.04
	40	ALL	Tx46		10	0.6	270	17.60	17.60			4.000	5.000	0.632	-0.746	1857	0.810	0.880	2.14	2.78	3.45
5 0	45	ALL	Tx46		10	0.6	270	17.60	17.60			4.000	5.000	0.791	-0.908	2196	0.780	0.880	1.77	2.29	2.76
1	50	ALL	Tx46		12	0.6	270	17.60	17.60			4.000	5.000	0.966	-1.093	2608	0.760	0.880	1.79	2.32	2.54
	55	ALL	Tx46		12	0.6	270	17.60	17.60			4.000	5.000	1.163	-1.296	2737	0.740	0.890	1.48	1.92	2.00
3	60	ALL	Tx46		12	0.6	270	17.60	17.60			4.000	5.000	1.367	-1.502	2690	0.720	0.890	1.23	1.60	1.56
	65	ALL	Tx46		12	0.6	270	17.60	17.60			4.000	5.000	1.598	-1.735	2973	0.710	0.890	1.01	1.31	1.16
Type Tx46 Girders	70	ALL	Tx46		14 16	0.6	270 270	17.60 17.35	17.60	4	6.5	4.000	5.000	1.843	-1.972 -2.225	3322	0.690	0.900	1.08	1.40	1.11 1.02
8.5" Slab	75 80	ALL ALL	Tx46 Tx46		20	0.6 0.6	270	17.35	16.85 15.40	4	6.5 12.5	4.000 4.000	5.000 5.000	2.100 2.372	-2.225	3704 4098	0.680 0.670	0.900 0.900	1.11 1.32	1.44 1.72	1.02
	85	ALL	Tx46 Tx46		20	0.6	270	16.88	15.40	4	12.5	4.000	5.000	2.668	-2.489	4098	0.660	0.900	1.32	1.72	1.13
	90	ALL	Tx46		26	0.6	270	16.68	12.07	4	34.5	4.000	5.000	2.964	-3.046	4885	0.640	0.900	1.47	1.93	1.05
	95	ALL	Tx46		30	0.6	270	16.40	9.20	6	42.5	4.100	5.000	3.298	-3.369	5363	0.640	0.910	1.50	2.05	1.02
	100	ALL	Tx46		34	0.6	270	16.07	9.72	6	42.5	4.700	5.400	3.628	-3.680	5800	0.630	0.910	1.48	1.99	1.07
	105	ALL	Tx46		38	0.6	270	15.81	10.13	6	42.5	5.300	6.100	3.988	-4.013	6260	0.620	0.910	1.44	1.94	1.10
	110	ALL	Tx46		40	0.6	270	15.70	11.50	6	34.5	5.900	6.900	4.364	-4.359	6732	0.610	0.910	1.35	1.90	1.02



DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TXDOT for any purpose wh TXDOT assumes no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use. U/C

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NON-STANDARD STRAND PATTERNS

STRAND ARRANGEMENT AT COF GIRDER

1 Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

PATTERN

Tension = 0.24 f'ci 🗸

Optional designs must likewise conform.

(2) Portion of full HL93.

DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to AASHTO Manual for Bridge Evaluation.

Optional designs for girders 120 feet or longer must have a calculated residual camber equal to or greater than that of the designed girder.

Prestress losses for the designed girders have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform.

FABRICATION NOTES:

Provide Class H concrete. Provide Grade 60 reinforcing steel bars. Use low relaxation strands, each pretensioned to 75 percent of

fpu. Strand debonding must comply with Item 424.4.2.2.2.4. Full-length debonded strands are only permitted in positions marked . Double Δ wrap full-length debonded strands in outer most position of each

row. When shown on this sheet, the Fabricator has the option of furnishing either the designed girder or an approved optional design. All optional design submittals must be signed, sealed and

dated by a Professional Engineer registered in the State of Texas. Seal cracks in girder ends exceeding 0.005" in width as directed by the Engineer. The fabricator is permitted to decrease the spacing of Bars R and S by providing additional bars to help limit crack width provided the decreased spacing results in no less than 1" clear between bars. The fabricator must take an approved corrective action if cracks greater than 0.005" form on a repetitive basis

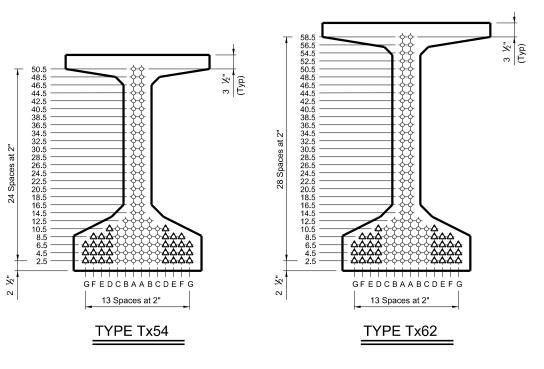
DEPRESSED STRAND DESIGNS:

Locate strands for the designed girder as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc., beginning each row in the "A" position and working outward until the required number of strands is reached. All strands in the "A" position must be depressed, maintaining the 2" spacing so that, at the girder ends, the upper two strands are in the position shown in the table.

HL93 LOADING	SH	EET	1 OF	2							
Texas Department	Bridge Division Standard										
I-GIRDER DE	PRESTRESSED CONCRETE I-GIRDER STANDARD DESIGNS 34' ROADWAY										
FILE: IG-IGSD34-23.dan	DN: VC	G	SD-C	04	SES	CK: TAR					
CTxDOT January 2023	CONT	SECT	OC TAIL			HIGHWAY					
REVISIONS	0901	29	092,	Etc.	C	R, Etc.					
	DIST		COL	NTY	_	SHEET NO.					
	PAR	L	.amar,	Etc		63					

Г				D	ESIGNE	d girde	RS				DEPR	ESSED	CONC	RETE		OPTION	IAL DESIGN			LC	DAD RA	
	STRUCTURE	SPAN NO.	GIRDER NO.	GIRDER TYPE	NON- STD STRAND PATTERN	PRE TOTAL NO.	SIZE	SING STR STRGTH	"e"	"e" END	STR PAT		RELEASE STRGTH	MINIMUM 28 DAY COMP STRGTH fc	DESIGN LOAD COMP STRESS (TOP □) (SERVICE I)	DESIGN LOAD TENSILE STRESS (BOTT □) (SERVICE III)	REQUIRED MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I)	DISTRII FAC	2	STREM		SERVICE III
							(in)	(ksi)	(in)	(in)		(in)	(ksl)	(ksl)	fct(ksl)	fcb(ksl)	(klp-ft)	Moment	Shear	Inv	Opr	Inv
		40	ALL	Tx54		12	0.6	270	21.01	21.01			4.000	5.000	0.525	-0.609	1923	0.840	0.870	3.01	3.90	4.78
		45	ALL	Tx54		12	0.6	270	21.01	21.01			4.000	5.000	0.655	-0.740	2276	0.810	0.870	2.53	3.28	3.94
		50	ALL	Tx54		12	0.6	270	21.01	21.01			4.000	5.000	0.801	-0.889	2686	0.780	0.870	2.08	2.69	3.15
		55	ALL	Tx54		12	0.6	270	21.01	21.01			4.000	5.000	0.963	-1.053	3130	0.760	0.880	1.74	2.25	2.55
		60	ALL	Tx54		14	0.6	270	21.01	21.01			4.000	5.000	1.137	-1.230	3608	0.750	0.880	1.77	2.30	2.40
		65	ALL	Tx54		14	0.6	270	21.01	21.01			4.000	5.000	1.326	-1.413	3561	0.730	0.880	1.51	1.96	1.94
		70	ALL	Tx54		16	0.6	270	20.76	20.76			4.200	5.000	1.517	-1.597	3822	0.710	0.880	1.56	2.03	1.87
т	vpe Tx54 Girders	75	ALL	Tx54		16	0.6	270	20.76	20.76			4.100	5.000	1.733	-1.807	3824	0.700	0.890	1.33	1.73	1.48
1	34' Roadway	80	ALL	Tx54		18	0.6	270	20.56	20.56			5.000	5.000	1.956	-2.021	4232	0.690	0.890	1.36	1.76	1.45
	8.5" Slab	85	ALL	Tx54		18	0.6	270	20.56	19.67	4	8.5	4.000	5.000	2.199	-2.251	4659	0.680	0.890	1.17	1.52	1.07
		90	ALL	Tx54		22	0.6	270	20.28	18.46	4	14.5	4.000	5.000	2.447	-2.485	5091	0.670	0.890	1.39	1.80	1.19
		95	ALL	Tx54		24	0.6	270	20.17	17.84	4	18.5	4.000	5.000	2.716	-2.735	5544	0.660	0.890	1.39	1.80	1.08
		100	ALL	Tx54		28	0.6	270	20.01	14.29	4	44.5	4.000	5.000	2.987	-2.987	5999	0.650	0.900	1.52	2.01	1.12
		105	ALL	Tx54		30	0.6	270	19.81	13.01	6	40.5	4.100	5.000	3.281	-3.257	6476	0.640	0.900	1.37	1.98	1.00
		110	ALL	Tx54		34	0.6	270	19.48	11.71	6	50.5	4.400	5.200	3.575	-3.526	6952	0.630	0.900	1.41	2.10	1.04
		115	ALL	Tx54		38	0.6	270	19.22	12.27	6	50.5	5.000	5.800	3.895	-3.816	7452	0.620	0.900	1.50	2.07	1.09
		120	ALL	Tx54		40	0.6	270	19.11	13.11	6	46.5	5.400	6.300	4.219	-4.120	8011	0.620	0.900	1.38	2.02	1.01
		125	ALL	Tx54		44	0.6	270	18.83	12.64	8	42.5	5.800	6.900	4.564	-4.430	8537	0.610	0.900	1.47	1.98	1.02
		60	ALL	Tx62		14	0.6	270	25.78	25.78			4.000	5.000	0.898	-1.031	3755	0.770	0.870	2.01	2.60	2.89
		65	ALL	Tx62		14	0.6	270	25.78	25.78			4.000	5.000	1.045	-1.184	4235	0.750	0.870	1.72	2.23	2.40
		70	ALL	Tx62		14	0.6	270	25.78	25.78			4.000	5.000	1.197	-1.342	4194	0.730	0.870	1.48	1.92	1.98
		75	ALL	Tx62		16	0.6	270	25.53	25.53			4.000	5.000	1.367	-1.517	4489	0.720	0.880	1.53	1.98	1.89
		80	ALL	Tx62		16	0.6	270	25.53	25.53			4.000	5.000	1.541	-1.697	4446	0.710	0.880	1.31	1.70	1.53
		85	ALL	Tx62		18	0.6	270	25.33	25.33			4.000	5.000	1.730	-1.890	4869	0.700	0.880	1.35	1.75	1.46
		90	ALL	Tx62		18	0.6	270	25.33	25.33			4.000	5.000	1.923	-2.085	5322	0.690	0.880	1.17	1.52	1.16
T	ype Tx62 Girders 34' Roadway	95	ALL	Tx62		20	0.6	270	25.18	24.78	4	6.5	4.000	5.000	2.132	-2.295	5799	0.680	0.880	1.20	1.56	1.10
	8.5" Slab	100	ALL	Tx62		22	0.6	270	25.05	23.96	4	10.5	4.000	5.000	2.342	-2.506	6277	0.670	0.880	1.23	1.60	1.03
		105	ALL	Tx62		26	0.6	270	24.85	22.70	4	18.5	4.000	5.000	2.571	-2.732	6781	0.660	0.890	1.42	1.85	1.12
		110	ALL	Tx62		28	0.6	270	24.78	20.21	4	36.5	4.000	5.000	2.799	-2.957	7281	0.650	0.890	1.34	1.86	1.03
		115	ALL	Tx62		32	0.6	270	24.40	15.40	6	54.5	4.000	5.000	3.047	-3.199	7810	0.640	0.890	1.40	2.04	1.05
		120	ALL	Tx62		34	0.6	270	24.25	16.84	6	48.5	4.400	5.200	3.292	-3.438	8332	0.630	0.890	1.32	2.02	1.00
<u>c</u>		125	ALL	Tx62		38	0.6	270	23.99	16.09	6	56.5	4.800	5.700	3.566	-3.712	8951	0.630	0.890	1.40	2.13	1.04
Бþ		130	ALL	Tx62		40	0.6	270	23.88	17.88	6	46.5	5.300	6.300	3.827	-3.966	9497	0.620	0.890	1.38	2.09	1.01
-24.		135	ALL	Tx62		44	0.6	270	23.60	15.96	8	50.5	5.500	6.500	4.114	-4.240	10077	0.610	0.890	1.46	2.07	1.01





NON-STANDARD STRAND PATTERNS

PATTERN	STRAND ARRANGEMENT AT &OF GIRDER

(1) Based on the following allowable stresses (ksi):

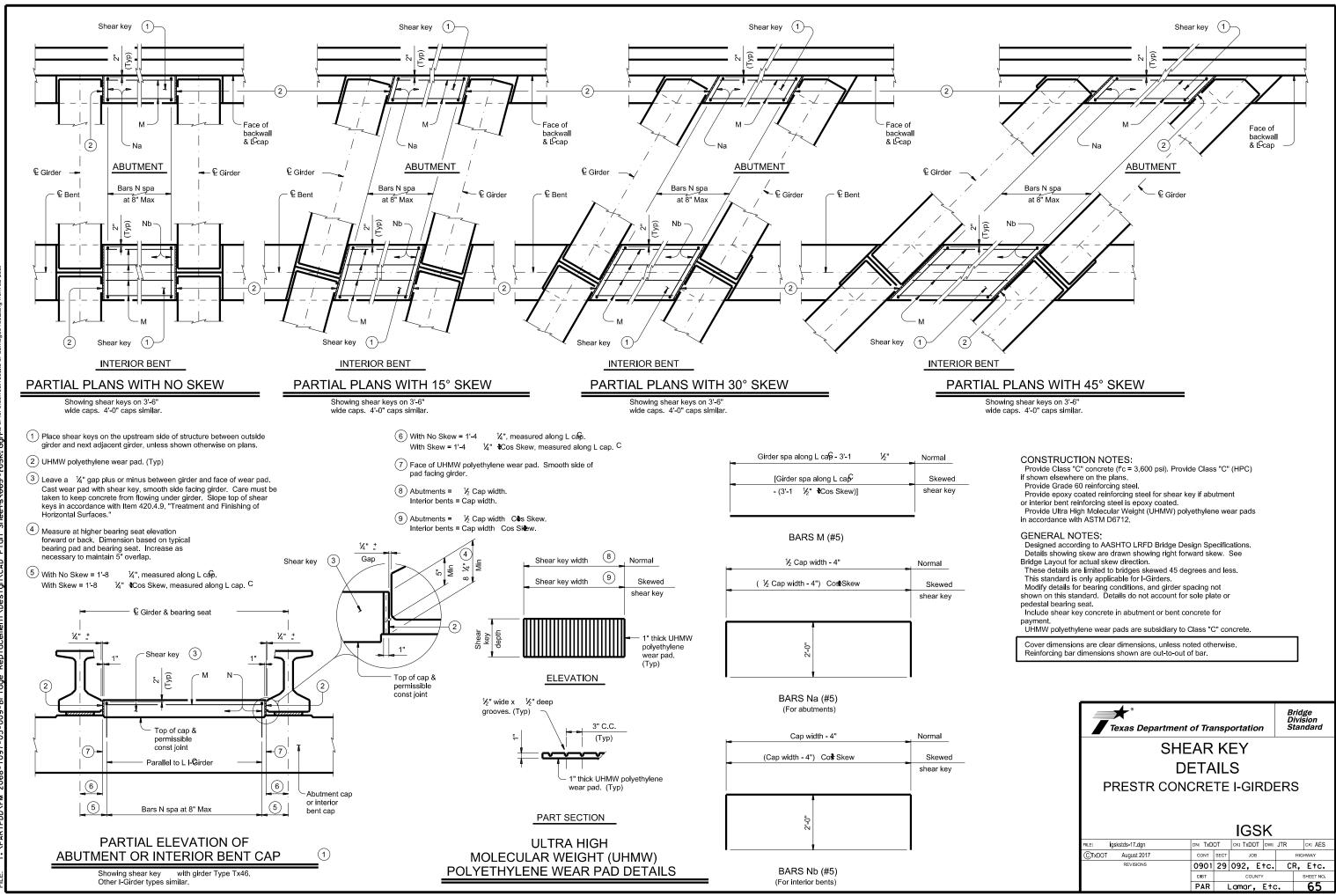
Compression = 0.65 fci

Tension = 0.24 fci 🗸

Optional designs must likewise conform.

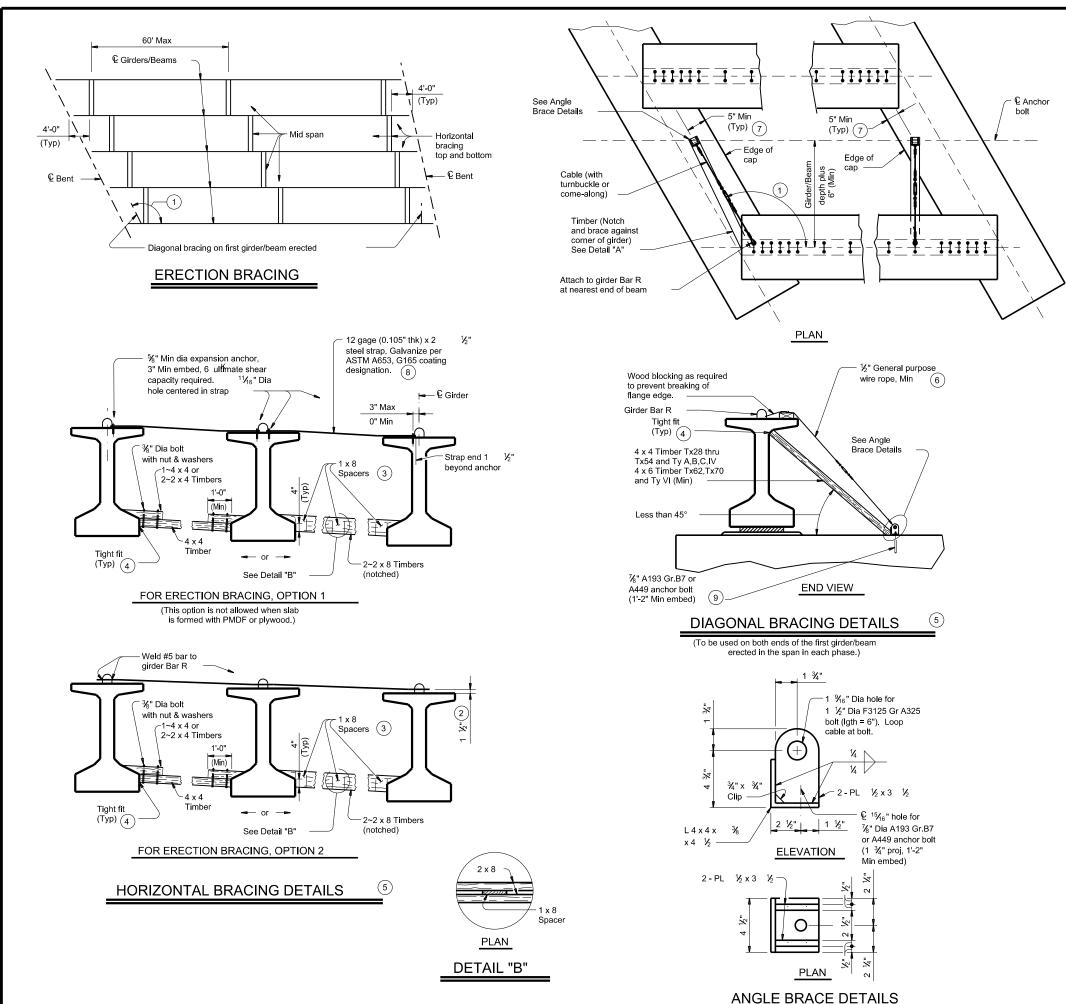
(2) Portion of full HL93.

HL93 LOADING	SHE	EET	2 OF 2							
Texas Department of Transportation										
PRESTRESS I-GIRDER DE 34' R	ST SIC	AN SN	NDAF S			E				
	I	G	SD-34	4						
FILE:	dn: VC		ск: TAR	DW:	SFS	ск: TAR				
©TxDOT	CONT	SECT	JOB			HIGHWAY				
REVISIONS	0901	29	092, E	tc.	CF	2, Etc.				
	DIST		COUNT	(SHEET NO.				
	PAR		Lamar,	E†c	.	64				



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HAULING & ERECTION:

The Contractor's attention is directed to the possible lateral instability of prestressed concrete girders and beams over 130' long, especially during hauling and erection. The use of the following methods to improve stability is encouraged: Locate lifting devices at the maximum practical distance from girder ends; use external lateral stiffening devices during hauling and erection; lift with vertical lines using two machines; and take care in handling to minimize inertial and impact forces.

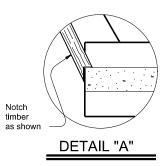
ERECTION BRACING:

Erection bracing details shown are considered the minimum for fulfilling the bracing requirements of Item 425.

Required erection bracing must be placed immediately after erection of each girder and remain in place until additional bracing as required for slab placement is in place. This standard is needed in all cases to meet requirements for Slab Placement Bracing.

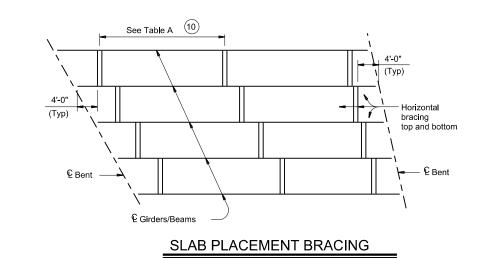
PHASED CONSTRUCTION:

Place erection and slab placement bracing for all girders in a phase as shown in these details. For phases after first, also place erection and slab placement bracing between outer girder of completed phase and adjacent girder of current phase. When the phase construction joint is between girders, top bracing can be omitted.

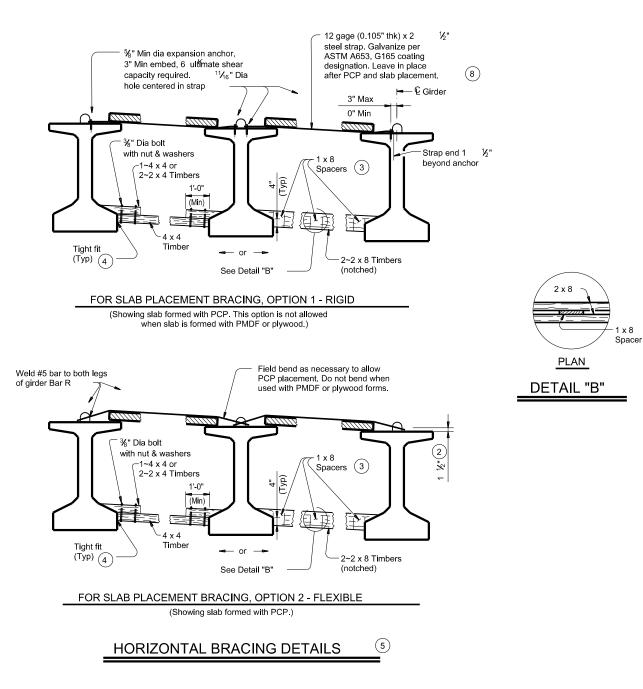


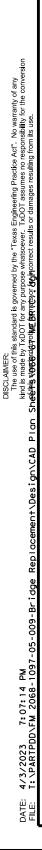
- (1) If angle shown exceeds 120 degrees, move diagonal brace to other side of girder/beam and place square to girder/beam. This may prevent exterior girder from being erected first.
- (2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R (See Sheet 2 of 2).
- (3) Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- 6 All hardware used with cable must be able to develop a minimum 25 kips breaking strength. Use thimbles at all loops in cable. Install cable clamps with saddles bearing against the live end and U-bolts bearing aginst the dead end.
- (7) It is acceptable to tie anchor bolts to cap reinforcement.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- Anchor bolt may be drilled and epoxied in place. Provide 25k minimum pullout. Core drill hole.

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		TABI	LE A							
OPTION 1-RIG	ID BRACING (STEEL	STRAP)	OPTION 2-FLEXIBLE BRACING (NO. 5 OVER PCP)							
	Maximum Braci	ng Spacing		Maximum Bracing Spacing						
Girder or Beam Type	Slab Overhang less than 4'-0"	Slab Overhang 4'-0" and greater (11)	Girder or Beam Type	Slab Overhang less than 4'-0" (11)	Slab Overhang 4'-0" and greater (11)					
Tx28	¼ points	¼ points	Tx28	¼ points	¼ points					
Tx34	¼ points	1⁄4 points	Tx34	¼ points	¼ points					
Tx40	¼ points	¼ points	Tx40	¼ points	¼ points					
Tx46	¼ points	1∕8 points	Tx46	¼ points	1∕% points					
Tx54	¼ points	1∕8 points	Tx54	¼ points	1∕% points					
Tx62	¼ points	⅓ points	Tx62	¼ points	¼ points					
Tx70	¼ points	⅓ points	Tx70	¼ points	⅓ points					
A	$\frac{1}{8}$ points	⅓ points	A	2.0 ft	1.5 ft					
В	⅓ points	⅓ points	В	3.0 ft	2.0 ft					
С	$\frac{1}{8}$ points	⅓ points	С	4.5 ft	2.0 ft					
IV	¼ points	¼ points	IV	¼ points	4.0 ft					
VI	¼ points	⅓ points	VI	¼ points	4.0 ft					





(2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R.

- (3) Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- (4) Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- 10 Bracing spacing ($$ ¼ and $$ ¼ points) measured between first and last typical brace location.
- (1) Measure slab overhang from centerline of girder or beam. When overhang varies in span, determine bracing spacing based on largest overhang.

SLAB PLACEMENT BRACING:

The details for slab placement bracing are considered minimum for fulfilling the requirements of Specification Items 422 and 425. Required slab placement bracing must remain in place until slab concrete has attained a compressive strength of 3000 psi.

GENERAL NOTES:

Bracing details for spans longer than 150' are not provided. The Contractor must submit proposed bracing details for such conditions to the Engineer for approval prior to erection.

Systems equal to or better than those shown may be used provided details of such systems are submitted to and approved by the Engineer prior to erection.

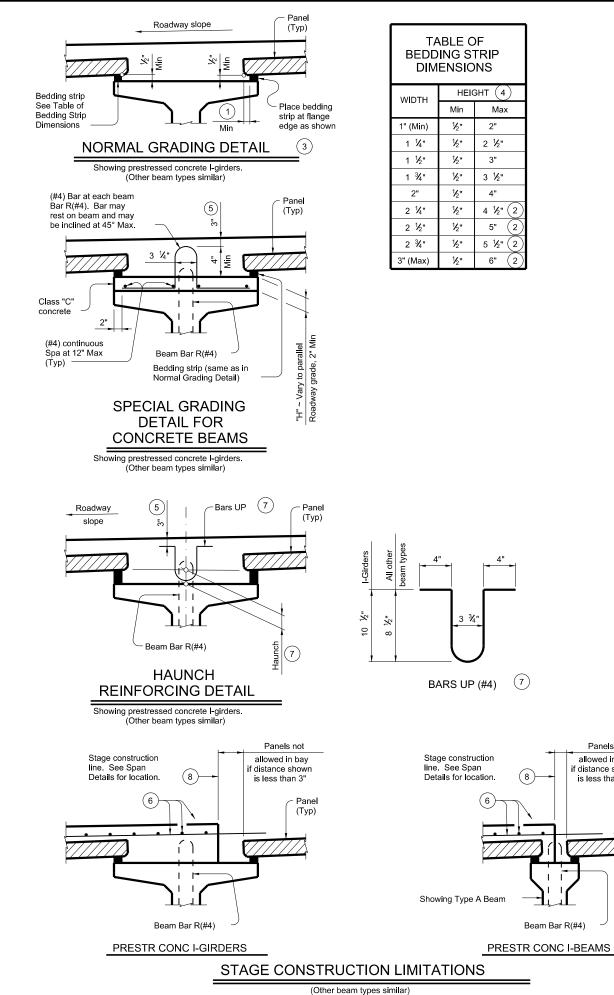
Use of these systems or details does not relieve the Contractor of the responsibility for the adequacy of the bracing and the safety of the structure.

Removal of bracing for short periods of time to align girders and beams is permissible.

All turn-buckles, come-alongs, anchors and other connections must be capable of developing the full strength of the cable shown.

Furnish anchor bolts and nuts in accordance with Item 449, "Anchor Bolts".

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1 2" Min for I-girders, 1 lash 2" Min for all other beam types. (2) Allowed for prestressed concrete I-girders, not allowed on other beam types.

(3) To reduce the quantity of cast-in-place concrete, bedding strip thickness may be increased in ¹/₄" increments. Bedding strips must be comprised of one layer. Bond bedding strips to the beams with an adhesive compatible with bedding strips. Bedding strips over 2.5" high may need to be bonded to panels. The same thickness strip must be used under any one panel edge and the maximum change in thickness between adjacent panels is Alternatively, bedding strips may be cut to grade. Panels may be supported by an alternate method, using a commercial product, if approved by the Engineer of Bridge Design, Bridge Division. If bedding strips exceed 6" high for I-Girders, 4" high for all other beam types, use Special Grading Detail for Concrete Beams or submit an alternate method to the Bridge Division for approval.

(4) Height must not exceed twice the width.

(5) Provide clear cover as indicated unless otherwise shown on Span Details.

- $^{(6)}$ See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- $\binom{7}{}$ Space Bars UP(#4) with Beam Bars R(#4) in all areas where measured haunch exceeds 3 $\frac{1}{2}$ " with I-girders, and 3" for all other beam types. Epoxy coating for Bars UP is not required.

 $\binom{8}{10}$ Do not locate construction joints on top of a panel.

Seal joint between panels when

Panels not

allowed in bay

if distance shown

is less than 3"

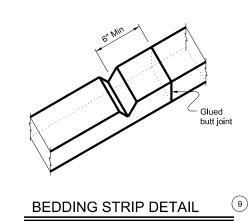
Pane (Typ) (9) Butt adjacent bedding strips together with adhesive. Cut v-notches, approx deep, in the top of the bedding strips at 8' o.c..



1/1"

gap exceeds ¼" with polyurethane sealant or expanding foam sealer. 0" - 1" Max Make seal flush with top of panel. Allowable Gap Pane PANEL JOINTS

> (Panel reinforcing not shown for clarity. The gap cannot be considered as a panel fabrication tolerance. Adjust panel placement to minimize joint openings.



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CONSTRUCTION NOTES:

Erected panels must bear uniformly on bedding strips of extruded polystyrene placed along top flange edges. Placing panels to minimize joint openings is recommended.

If additional blocking is needed, special grading details for supporting the panels and extra reinforcing between beam and slab will be considered subsidiary to deck construction.

Bars U, shown on PCP-FAB, may be bent over or cut off if necessary. Care must be taken to ensure proper cleaning of

construction debris and consolidation of concrete material under the edges of the panels. Bedding strips must be placed at beam flange edges so that adequate space is provided for the mortar to flow a minimum of 1 the panels as the slab concrete is placed.

½" under

To allow the proper amount of mortar to flow between beam and panel, the minimum vertical opening must be at least χ ". Roadway cross-slope reduces the opening available for entry of the mortar. Bedding strips varying in thickness across the beam are therefore required.

For clear span between U-beams less than or equal to 18", see Permissible Slab Forming Detail on Miscellaneous Slab Detail sheets, UBMS.

MATERIAL NOTES: Provide Grade 60 reinforcing steel in the cast-in-place slab. See Table of Reinforcing Steel for size and spacing of reinforcement.

If the top and bottom layer of reinforcing steel is shown on the Span Details to be epoxy coated, then the D, E, P, & Z bars must be epoxy coated.

Provide bar Laps, where required, as follows: Uncoated ~ #4 = 1'-7"

Epoxy Coated ~ #4 = 2'-5"

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.

Panel placement may follow either Option 1 or Option 2 except Option 1 must be used if the skew exceeds 45 degrees

Use of Prestressed Concrete Panels is not permitted for horizontally curved steel plate or tub girders. See Span Details for other possible restrictions on their use.

These details are to be used in conjunction with the Span Details, PCP-FAB and other applicable standard drawings. When panel support (bedding strips) deviates from what

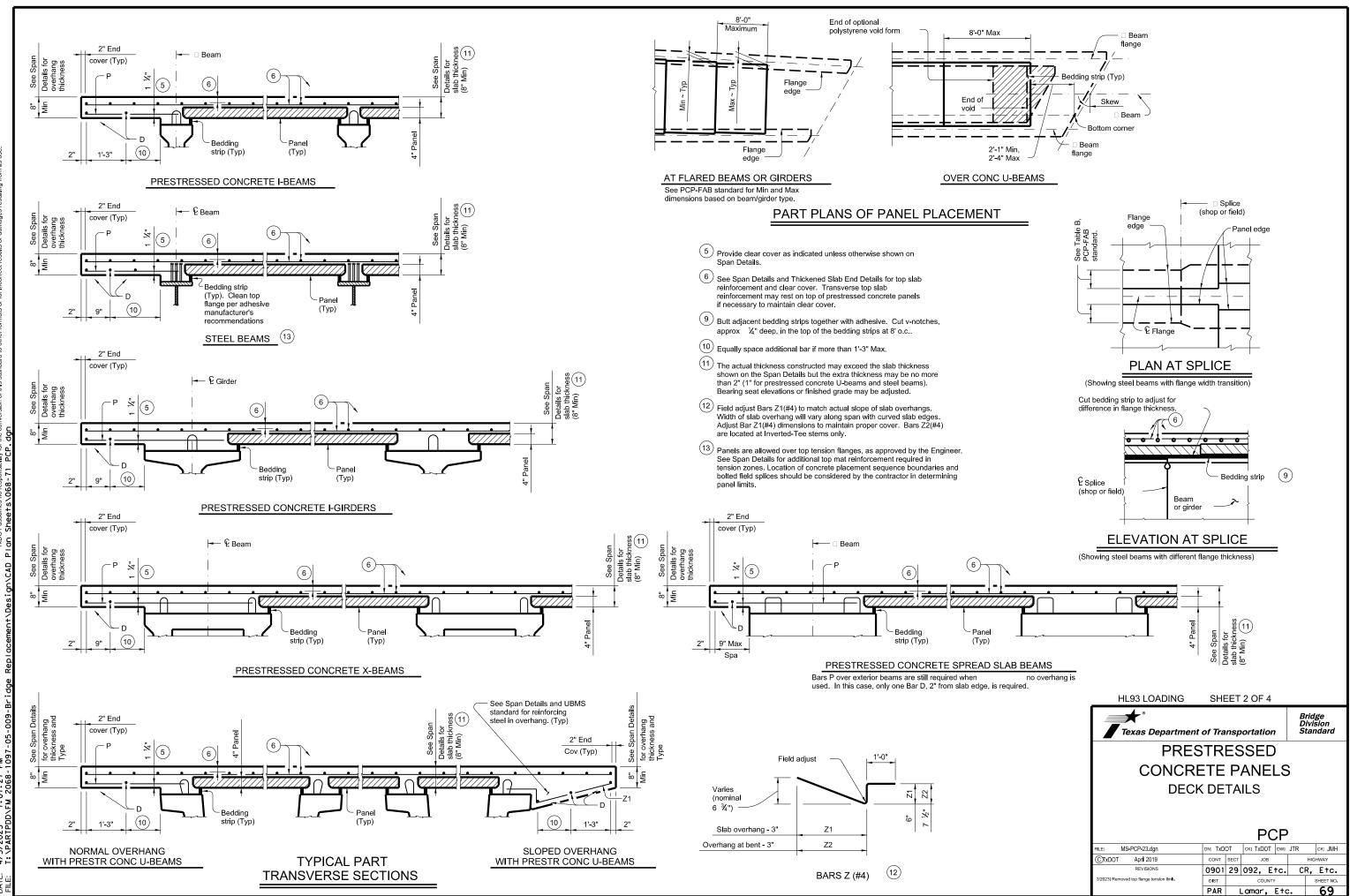
is shown herein, provide details signed and sealed by a professional Engineer. Any additional reinforcement or concrete required on

this standard is considered subsidiary to the bid Item "Reinforced Concrete Slab".

Cover dimensions are clear dimensions, unless noted otherwise

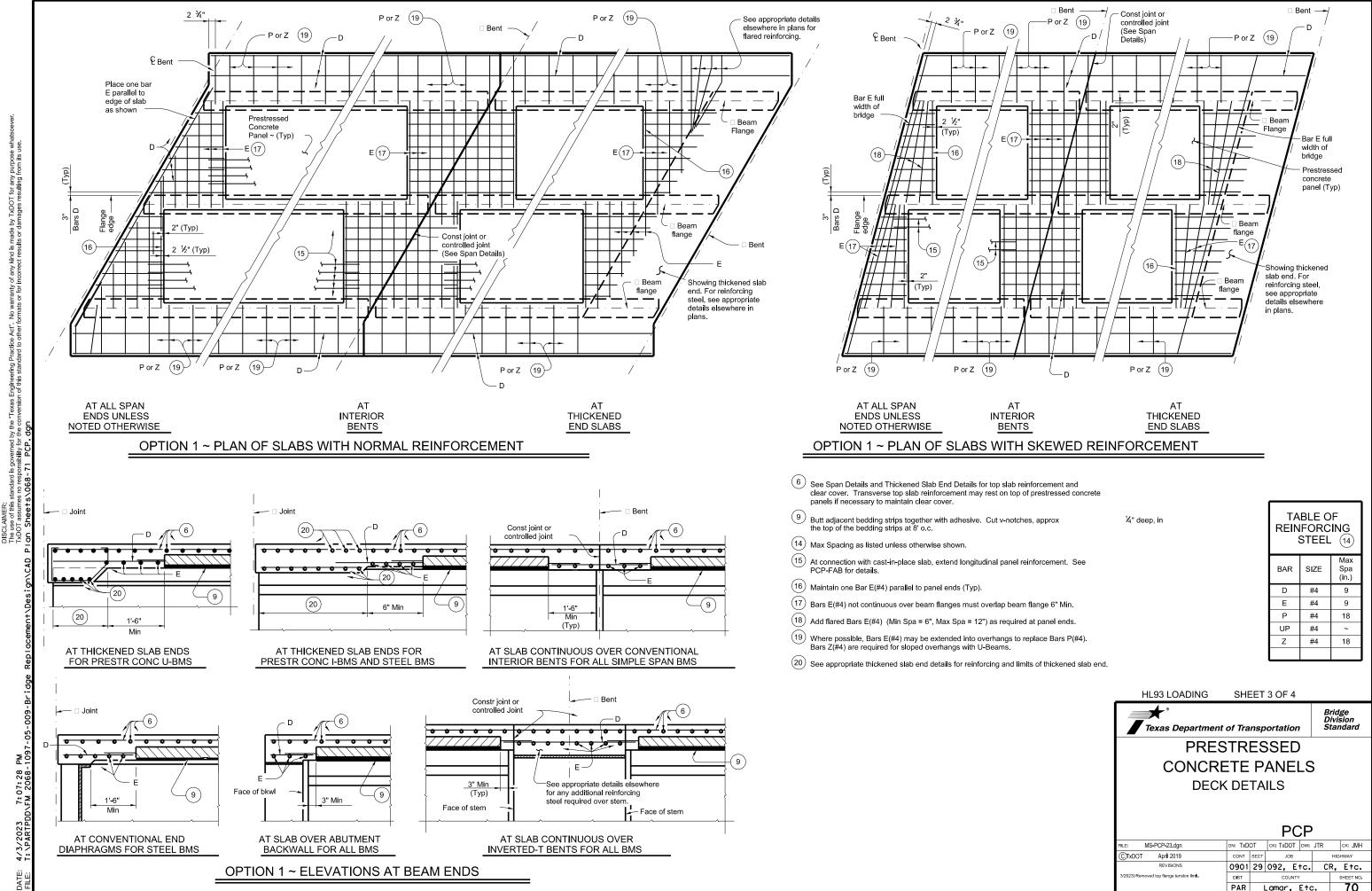
Reinforcing bar dimensions shown are out-to-out of bar.

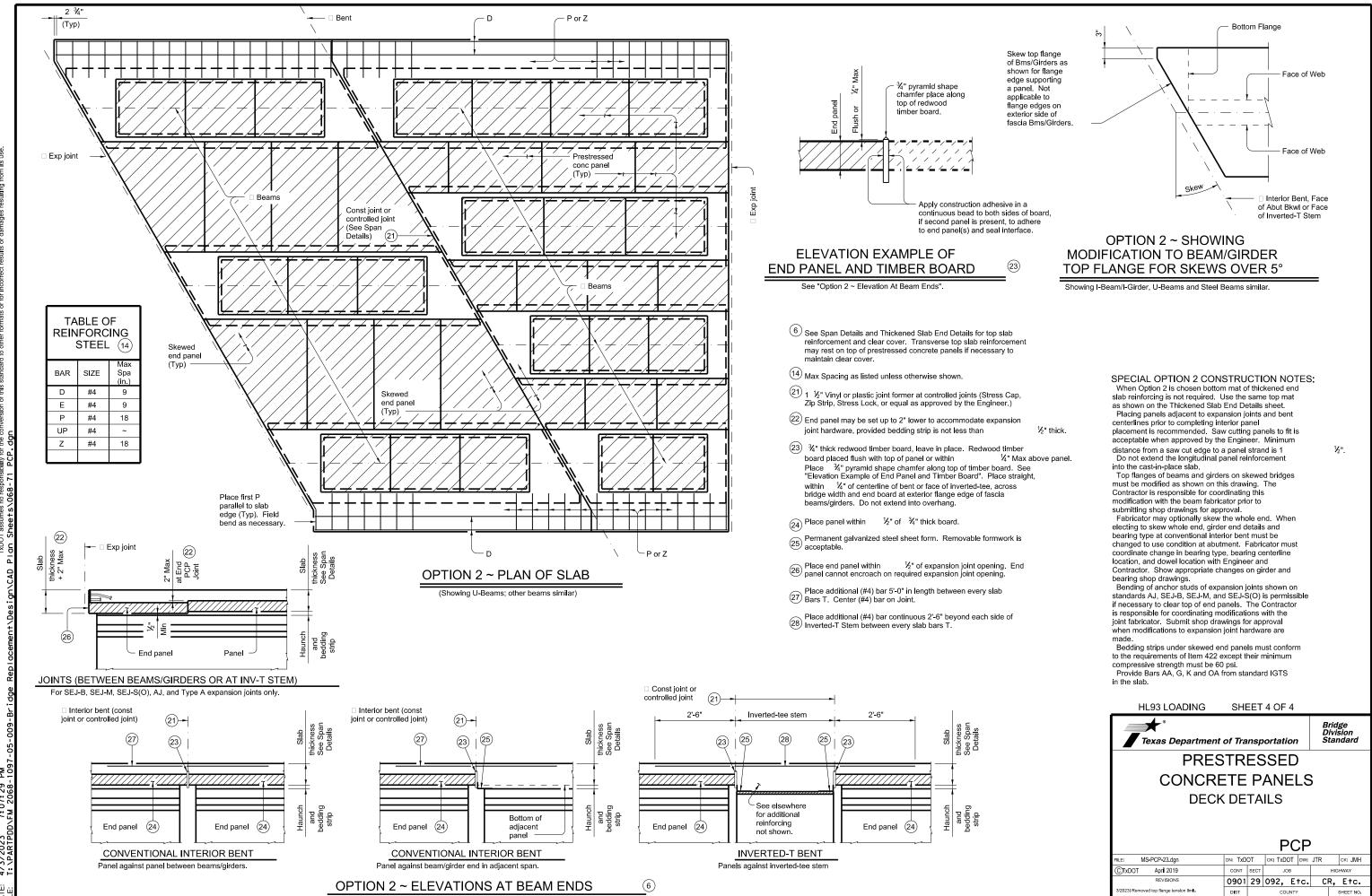
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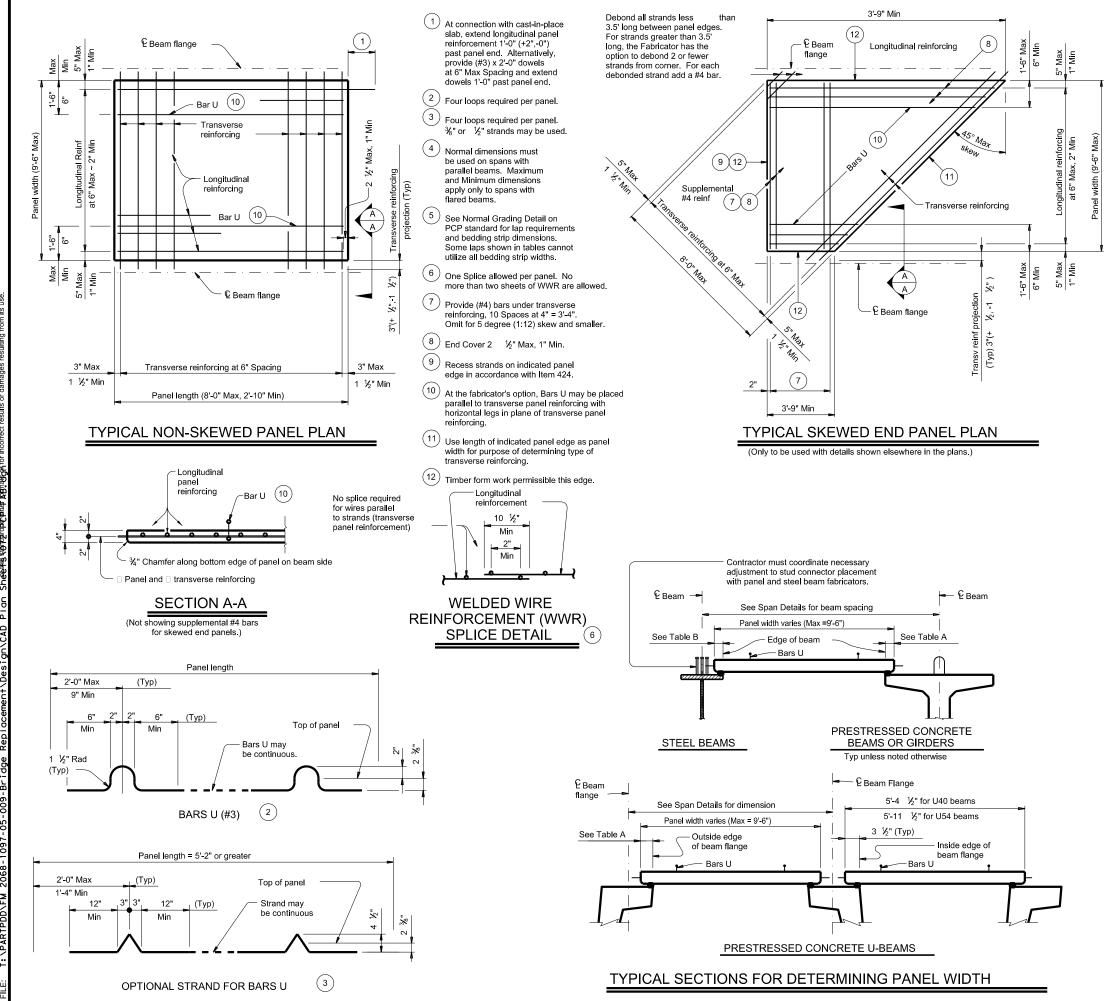


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	TABLE	A (1)(5)	TA	BLE B	4	5)
Beam Type	Normal (In.)	Min (In.)	Max (In.)	Top Flange Width	Normal (In.)	Min (In.)	Max (In.)
А	3	2 1/2	3 1/2	11" to 12"	2 3⁄4	2 ½	2 3/4
В	3	2 1/2	3 1/2	Over 12" to 15"	3 1/4	33	1⁄4
С	4	84	1/2	Over 15" to 18"	4 3	4	3⁄4
IV	6	4 7	1/2	Over 18"	5 3	1/2	6 ¼
VI	6 1/2	4 ½"	8 ½				
U40 - 54	5 1/2	5 ½	7				
Tx28-70	6	57	1/2				
XB20 - 40	4	84	1/2				
XSB12 - 15	4	84	1/2				

GENERAL NOTES:

Provide Class H concrete for panels. Release strength fci=3,500 psi. Minimum 28 day strength fc=5,000 psi.

Finish top of panel to a roughness between a No. 6 and No. 9 concrete surface profile, inclusive, as specified by the International Concrete Repair Institute (ICRI).

Shop drawings for the fabrication of panels will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.

A panel layout which identifies location of each panel must be developed by the Fabricator. Permanently mark each panel in accordance with the panel layout. A copy of the layout is to be provided to the Engineer.

TRANSVERSE PANEL REINFORCEMENT:

For panel widths over 5', use % or % Dia (270k) prestressing strands with a tension of 14.4 kips per strand.

For panel widths over 3'-6" up to and including 5', use %" or %" Dia (270k) prestressing strands with a tension of 14.4 kip per strand. Optionally,

(#4) Grade 60 reinforcing bars may be used in lieu of prestressed strands. For panel widths up to 3'-6", use (#4) Grade 60 reinforcing bars (prestressed strands alone are not allowed).

Place transverse panel reinforcement at panel centroid and space at 6" Max.

LONGITUDINAL PANEL REINFORCEMENT:

Any of the following options may be used for longitudinal panel reinforcement:

1. (#3) Grade 60 reinforcing steel at 6" Max Spacing. No splices allowed. 2. ¾" Dia prestressing strands at 4 ½" Max Spacing

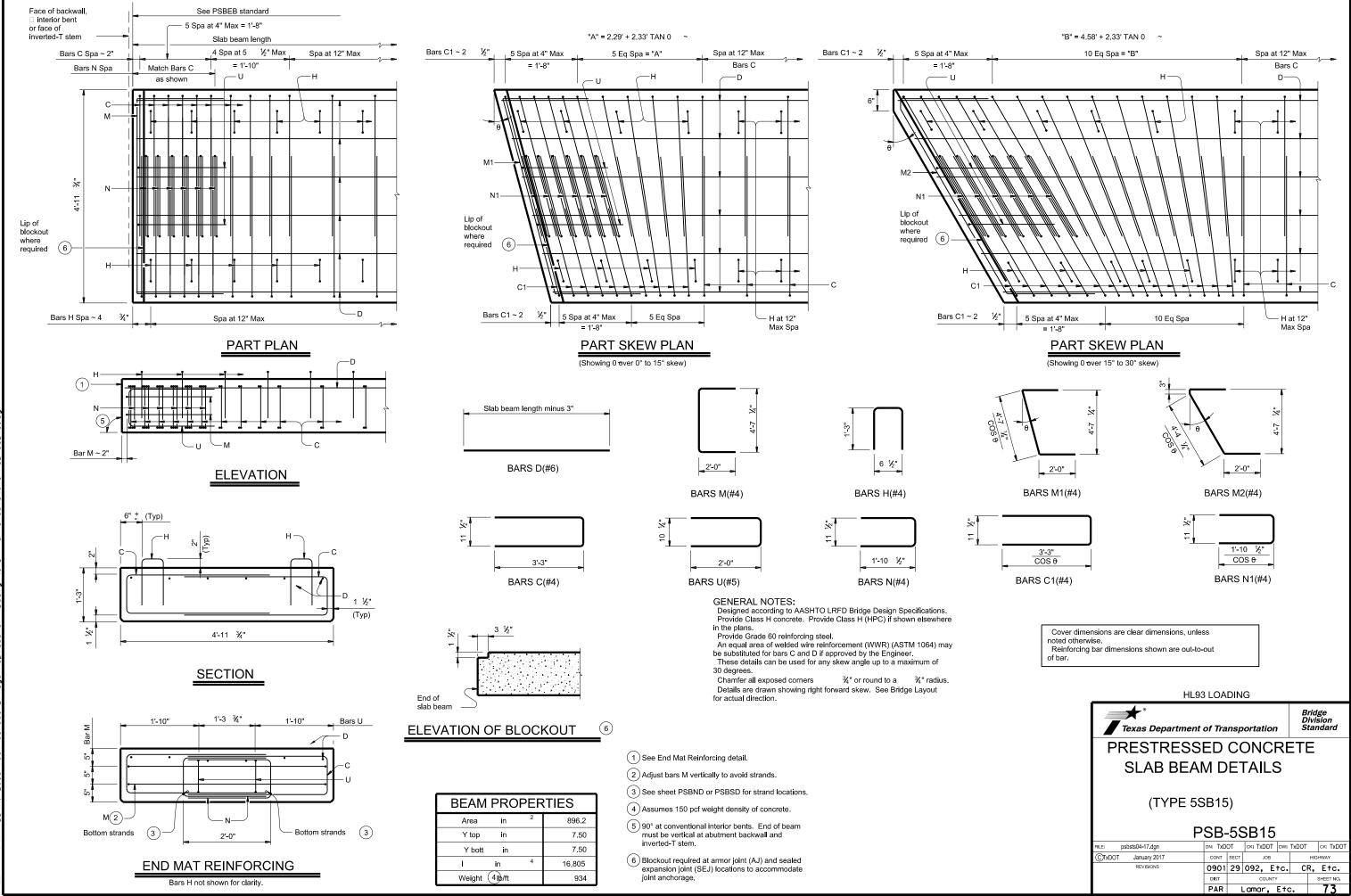
(unstressed). No splices allowed.

3. ½" Dia prestressing strands at 6" Max Spacing (unstressed). No splices allowed.

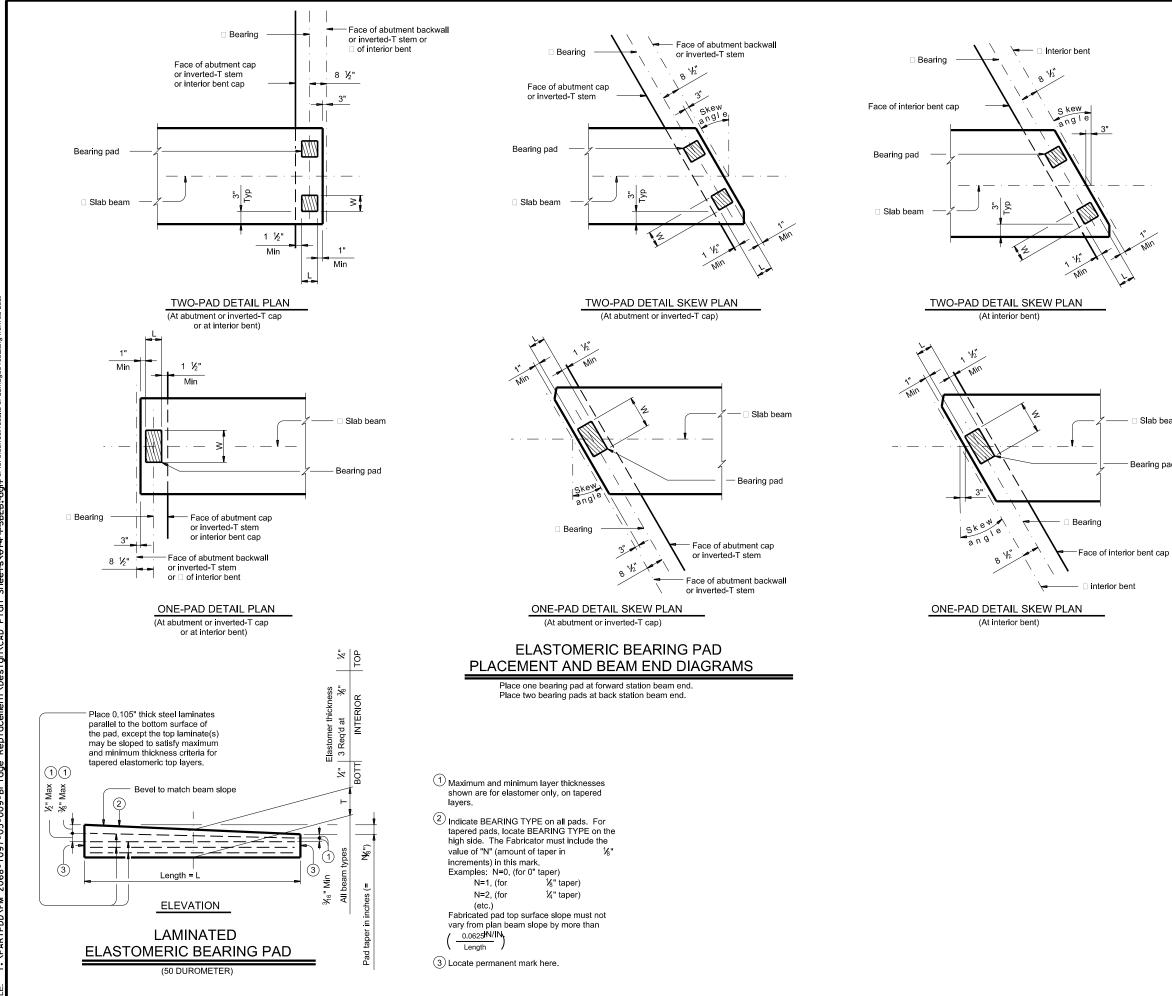
No splices allowed. 4. Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) providing 0.22 sq in per foot of panel width. Wires larger than D11 not permitted. Provide transverse wires to ensure proper handling of reinforcing. One splice per panel is allowed. See WWR Splice Detail.

No combination of longitudinal reinforcement options in a panel is allowed. Place longitudinal panel reinforcement above or below transverse panel reinforcement. Must be placed above transverse panel reinforcement for skewed end panels with supplemental (#4) reinforcement.

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			E OF DIMENSI SLAB BM)						
One-Pad (Ty SB1-"N") 2 Two-Pad (Ty SB2-"N")											
W	L	Т	W	L	Т						
14" 7" 2" 7" 7" 2"											
followi (1) Al wh no spa	ng condition Il one, two ai lere the mini t less than 2 an is not mo	nd three spa mum span le 5' and the m	n units ength is aximum								

Slab beam

Bearing pad

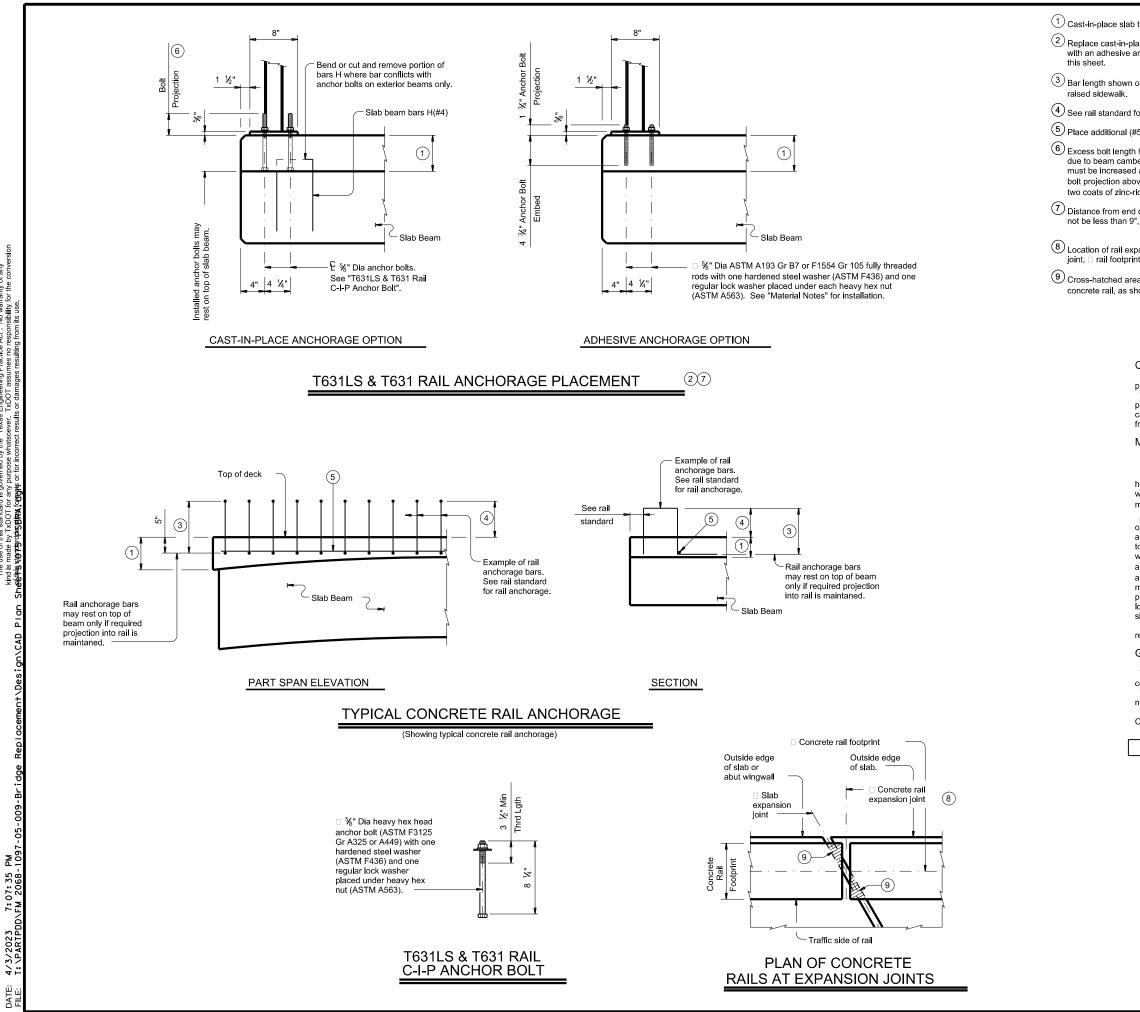
These details accommodate skew angles up to 30°. Shop drawings for approval are required. A bearing layout which identifies location and orientation of all bearings must be developed by the bearing fabricator. Permanently mark each bearing in

GENERAL NOTES:

accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer.

Cost of furnishing and installing elastomeric bearings must be included in unit price bid for "Prestressed Concrete Slab Beams".

HL93 LOADING * Bridge Division Standard Texas Department of Transportation ELASTOMERIC BEARING AND BEAM END DETAILS PRESTR CONCRETE SLAB BEAM **PSBEB** DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDOT ILE: psbste06-17.dgn CTxDOT January 2017 CONT SECT JOB HIGHWAY 0901 29 092, Etc. CR, Etc. REVISIONS DIST PAR Lamar, Etc. 74



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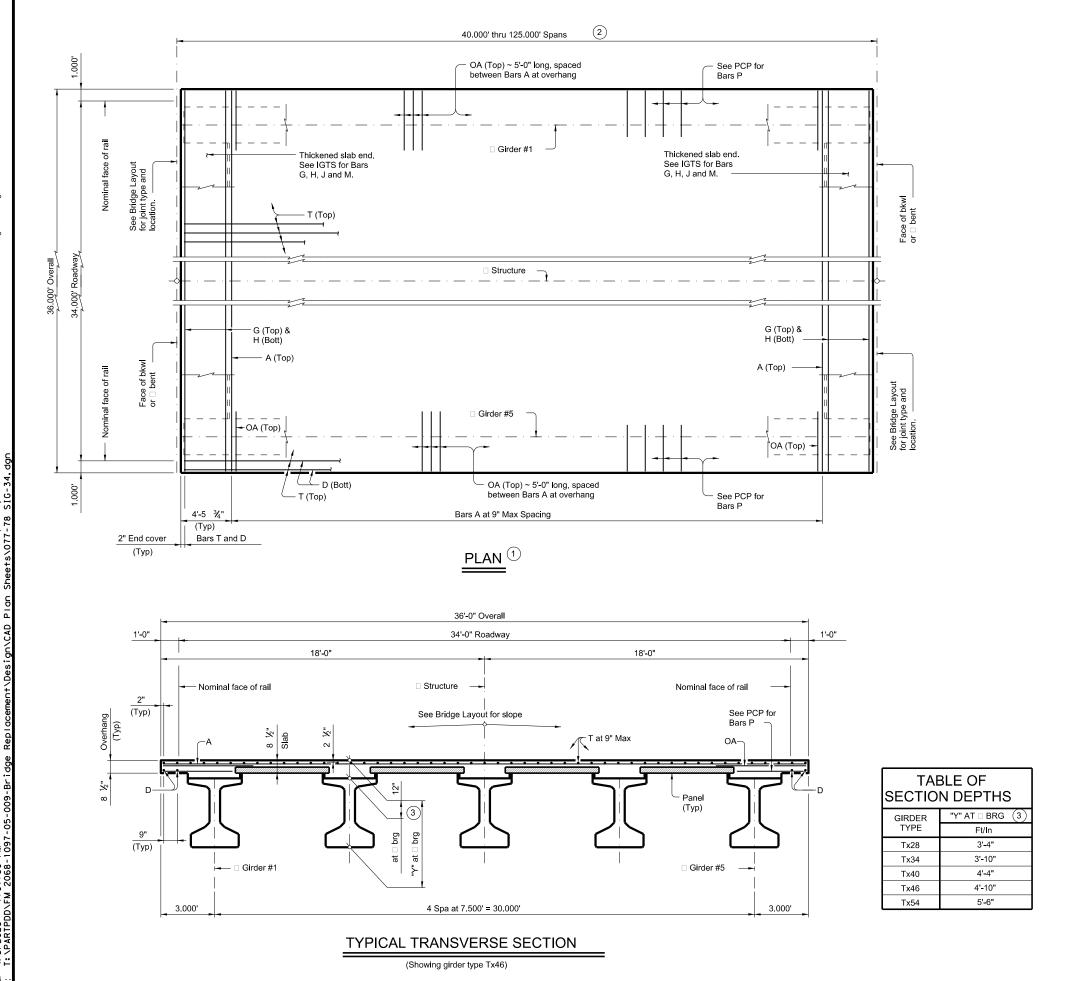
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ab thickness varies due to bean	n camber (5" minimum).		
-place anchor bolts shown on T e anchor system or cast-in-plac		1	
n on rail standard, minus 1	$ m \sc ll$ ". Adjust bar length	for a	
d for projection from finished gr	ade or top of sidewalk.		
(#5) longitudinal bar.			
th has been provided to accom mber. If slab thickness on spar ed accordingly. After posts hav bove nuts of more than c-rich paint conforming to the Ita	details exceed 7", bolt length ve been set and bolts tightened χ " must be cut off and pair	1,	
nd of top outside edge of slab to 9", except: 15° Skew: 1'-0" (ac 30° Skew: 1'-3" (acute corr	cute corner only)		
expansion joint must be at the ir print and perpendicular to slab c		n	
area must have ½" prefo shown.	rmed bitumuminous fiber mate	erial under	
provide minimum cover show Test adhesive anchors in ac per 100 anchors installed. Pr capacity if any of the tests do from testing as directed. MATERIAL NOTES: Galvanize all steel compone Provide Grade 60 reinforcin Cast-in-place anchorage sy hex head anchor bolts (ASTM washer (ASTM F436) and on must conform to ASTM A563 Adhesive anchors for T6311 or F1554 Gr 105 fully threade and one regular lock washer to ASTM A563 requirements. wingwall using a Type III, Cla anchor embedment depth is - a nominal bond strength in te must be accounted for). Sub published literature showing f load to the Engineer for appr size, drilling, and clean out, n Epoxy coat or galvanize reiir reinforcement is epoxy coate GENERAL NOTES: Designed in accordance wit This standard is for use with concrete slab. This standard may require r not apply to median barriers. This standard does not prov C412, PR11, PR22 and PR3 See rail standards for appro	e field bent as required to clear n on standard rail detail sheet cordance with Item 450.3.3, " erform corrective measures to n on meet the required test loa ents of steel rail system. g steel. stem for T631LS and T631 Ra <i>A</i> F3125 Gr 325 or A449) with e regular lock washer placed u requirements. Embed anchor <i>S</i> and T631 Rail must be ad rods with one hardened stee placed under each heavy hex. Embed fully threaded rod in the ster of a single anchor, Na, of mit signed and sealed calculat the proposed anchor adhesive oval prior to use. Anchor insta- nust be in accordance with Iter- forcing steel shown on this sta d or galvanized. h AASHTO LRFD Bridge Desi- n structures with a 5" minimum modification for interior rails. T ride details for Type T221P, T2	s. Tests". Test 3 anchors provide adequate d. Repair damage il must be %" one hardened steel under heavy hex nut. Nuts r bolts 4 ½" min %" Dia ASTM R433 (el washer (ASTM F436) nut. Nuts must conform o slab and/or abutment ive. Minimum adhesive chosen must be able to achier of 8 kips (edge distance ichosen must be able to achier ichosen must be able to a	Gr B7
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s	TRUCTURE	SPAN LENGTH	BEAM NO.	BEAM TYPE	NON- STD	TOTAL		STRGTH		"e"	TOT NO.	DIST FROM	NO	OF	NUME D	BER OF S EBONDE	D TO	RELI STF	EASE M RGTH	/INIMUM 28 DAY	LOAD COMP STRESS	LOAD TENSILE STRESS	MINIMUM ULTIMATE MOMENT	DISTRIBU FACT	UT I ON FOR				
					STRAND PATTERN	NO.		fpu	¢	END	DEB	BOTTOM	TOTAL	DE- BONDED	3 6			5 f	d	COMP STRGTH fc	(TOP C) (SERVICE I)	(BOTT Q) (SERVICE III)	CAPACITY (STRENGTH I)	(2		STRE	ENGTH	SERVICE III	
-		(ft) 25	ALL	5SB12		8	(in) 0.6	(ksi) 270	(in) 3.50	(in) 3.50	0	(in) 2.5	8	0 0	0 0	0	0 0	(k) 4.0	si)	(ksi) 5.000	fct (ksi) 0.914	fcb (ksi) -1.217	(kip-ft) 448	Moment 0.450	Shear 0.450	Inv 1.40	Opr 1.82	Inv 1.71	
	24' ROADWAY SB12 BEAM	30	ALL	5SB12		10	0.6	270	3.50	3.50	0	2.5	10	0	0 0	0	0 0) 4.0	000	5.000	1.292	-1.685	530	0.450	0.450	1.25	1.62	1.29	
	SB12 BEAW	35 40	ALL ALL	5SB12 5SB12		14 18	0.6 0.6	270 270	3.50 3.50	3.50 3.50	0	2.5 2.5	14 18	0	0 0					5.000 5.000	1.730 2.218	-2.219 -2.796	675 820	0.450 0.440	0.450 0.440	1.33 1.34	1.73 1.74	1.23 1.12	
		25	ALL	5SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0 0		0 0) 4.0		5.000	0.725	-0.897	551	0.450	0.450	1.77	2.29	2.41	
	24' ROADWAY	30 35	ALL ALL	5SB15 5SB15		8 10	0.6 0.6	270 270	5.00 5.00	5.00 5.00	0	2.5 2.5	8 10	0 0	0 0	-				5.000 5.000	1.020 1.361	-1.244 -1.640	574 708	0.450 0.450	0.450 0.450	1.23 1.15	1.59 1.49	1.45 1.14	 Based on the following allowable stresses (ksi): Compression = 0.65 f'ci
	SB15 BEAM	40	ALL	5SB15		14	0.6	270	5.00	5.00	0	2.5	14	0 2	0 0	1				5.000 5.000	1.739	-2.068	864	0.440	0.440	1.32	1.71	1.19 1.08	Tension = 0.24 f'ci $$
		45 50	ALL ALL	5SB15 5SB15		18 24	0.6 0.6	270 270	5.00 5.00	5.00 5.00	2 8	2.5 2.5	18 24	2 8	4 4					5.000 5.000	2.179 2.680	-2.574 -3.153	1054 1276	0.440 0.440	0.440 0.440	1.34 1.33	1.73 1.72	1.08	Optional designs must likewise conform.
	8' ROADWAY	25	ALL	5SB12		8	0.6	270	3.50	3.50	0	2.5	8	0	0 0	0	0 0) 4.0		5.000	0.903	-1.184	444	0.430	0.430	1.47	1.91	1.80	2 Portion of full HL93.
	SB12 BEAM	30 35	ALL ALL	5SB12 5SB12		10 12	0.6 0.6	270 270	3.50 3.50	3.50 3.50	0	2.5 2.5	10 12	0 0	0 0	1				5.000 5.000	1.276 1.708	-1.639 -2.159	508 647	0.430 0.430	0.430 0.430	1.32 1.18	1.71 1.53	1.37 1.02	
		40	ALL	5SB12		18	0.6	270	3.50	3.50	0	2.5	18	0	0 0	0	0 0			5.000	2.200	-2.744	799	0.430	0.430	1.37	1.78	1.17	
		25 30	ALL ALL	5SB15 5SB15		8 8	0.6 0.6	270 270	5.00 5.00	5.00 5.00	0	2.5 2.5	8 8	0 0	0 0	-) 4.0) 4.0		5.000 5.000	0.716 1.007	-0.874 -1.212	529 570	0.430 0.430	0.430 0.430	1.85 1.29	2.40 1.67	2.53 1.53	DESIGN NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.
	28' ROADWAY SB15 BEAM	35	ALL	5SB15		10	0.6	270	5.00	5.00	0	2.5	10	0	0 0	0	0 0) 4.0	000	5.000	1.343	-1.598	680	0.430	0.430	1.21	1.57	1.22	Load rated using Load and Resistance Factor Rating according to AASHTO Manual for Bridge Evaluation. Prestress losses for the designed beams have been calculated for a
		40 45	ALL ALL	5SB15 5SB15		14 18	0.6 0.6	270 270	5.00 5.00	5.00 5.00	0	2.5 2.5	14 18	0 2	0 0 2 0	-				5.000 5.000	1.725 2.149	-2.032 -2.508	842 1013	0.430 0.420	0.430 0.420	1.36 1.41	1.76 1.82	1.24 1.16	relative humidity of 60 percent. Optional designs must likewise conform.
		50	ALL	5SB15		22	0.6	270	5.00	5.00	6	2.5	22	6	4 2	0	0 0			5.000	2.643	-3.073	1227	0.420	0.420	1.33	1.72	1.01	FABRICATION NOTES: Provide Class H concrete.
	30' ROADWAY	25 30	ALL ALL	4SB12 4SB12		6 8	0.6 0.6	270 270	3.50 3.50	3.50 3.50	0	2.5 2.5	6 8	0 0	0 0	1				5.000 5.000	0.904 1.277	-1.187 -1.646	341 407	0.340 0.340	0.340 0.340	1.38 1.32	1.79 1.71	1.67 1.37	Provide Grade 60 reinforcing steel. Use low relaxation strands, each pretensioned to 75 percent of fpu. Full-length debonded strands are not permitted in positions "A" and "B".
	SB12 BEAM	35	ALL	4SB12		10	0.6	270	3.50	3.50	0	2.5	10	0	0 0	-	0 0	4.0	000	5.000	1.711	-2.169	518	0.340	0.340	1.24	1.60	1.08	Strand debonding must comply with Item 424.4.2.2.2.4. When shown on this sheet, the Fabricator has the option of furnishing
		40 25	ALL	4SB12 4SB15		14 6	0.6	270	3.50 5.00	3.50 5.00	0	2.5 2.5	14 6	0	0 0	-				5.000 5.000	2.205 0.723	-2.758	640 431	0.340 0.350	0.340	1.34	1.73 2.19	1.11 2.32	either the designed beam or an approved optional beam design. All optional design submittals and shop drawings must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.
do u		30	ALL	4SB15		6	0.6	270	5.00	5.00	0	2.5	6	0	0 0	0	0 0) 4.0	000	5.000	1.017	-1.231	438	0.350	0.350	1.16	1.50	1.37	Locate strands for the designed beam as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5",
	80' ROADWAY SB15 BEAM	35 40	ALL ALL	4SB15 4SB15		8 12	0.6 0.6	270 270	5.00 5.00	5.00 5.00	0	2.5 2.5	8 12	0	0 0					5.000 5.000	1.346 1.729	-1.605 -2.043	545 675	0.340 0.340	0.340 0.340	1.21 1.47	1.57 1.91	1.21 1.38	then row "4.5". Place strands within a row as follows: 1) Locate a strand in each "A" position.
PSB		45 50	ALL ALL	4SB15 4SB15		14 18	0.6 0.6	270 270	5.00 5.00	5.00 5.00	2	2.5 2.5	14 18	2 4	2 0			, I		5.000 5.000	2.166 2.665	-2.542 -3.115	823 998	0.340 0.340	0.340 0.340	1.33 1.32	1.73 1.71	1.06 1.02	 Place strand symmetrically about vertical centerline of beam. Space strands as equally as possible across the entire width. Do not debond strands in position "A". Distribute debonded strands
2016 1016			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	10010		10	0.0	2.10	0.00	0.00		2.0	10	·	2 2					0.000	2.000	0.110		0.010	0.010	1.02		1.02	symmetrically about the vertical centerline. Increase debonded lengths working outward, with debonding staggered in each row.
2 2 2 2																													
PIO																													
n\CAI															•	4	٦						— г						
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Ú+C		****** ******					4.5 — 2.5 —		>	>	>	◆	>	~~~~ ~~~~ + + + +	ļ						********* *********** + + + + + + + + +		4.5		>	>	~~~~ ~~~~~ + + + +	>	
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-Bric			1" _ =	1"							1"	1"								1"	1"					1"	1 "		HL93 LOADING
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- 05				/				-	,			_ 20								/			=						PRESTRESSED CONCRETE
-109																													SLAB BEAM STD DESIGNS
2068																													(TY SB12 OR SB15)
Ρ																													24', 28' & 30' ROADWAY
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standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TxDOT for any purpose wh se no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use. \$\created{symbol 10.1} **8 SIG-34.4gn** DISCLAIMER: The use of this s TxDOT assume: 7:07:39 PM D\FM 2068-109 4/3/2023 T:\PARTPD DATE: EII E

BAR TABLE

	NDLL
BAR	SIZE
А	#4
D	#4
G	#4
Н	#4
J	#4
М	#4
OA	#5
Р	#4
Т	#4

(1) If multi-span units (with slab continuous over interior bents) are indicated on the Bridge Layout, see standard IGCS for adjustment to slab reinforcement and quantities.

(2) Span lengths for prestressed concrete I-Girder type: Type Tx28 for spans lengths 40.000' thru 75.000'. Type Tx34 for spans lengths 40.000' thru 85.000'. Type Tx40 for spans lengths 40.000' thru 95.000'.

Type Tx46 for spans lengths 40.000' thru 110.000'.

Type Tx54 for spans lengths 40.000' thru 125.000'.

(3) "Y" value shown is based on theoretical girder camber, dead load deflection from an 8 ½" concrete slab, a constant roadway grade, and using precast panels (PCP). The Contractor will adjust this value as necessary for any roadway vertical curve.

HL93 LOADING			5	SHEE	ET 1 (OF 2				
Texas Department	of Tra	nsp	ortatio	on	Di	idge vision andard				
PRESTRESSED CONCRETE I-GIRDER SPANS (TYPE Tx28 THRU Tx54) 34' ROADWAY										
		S	SIG-:	34						
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©TxDOT January 2023	CONT	SECT	JOI	в		HIGHWAY				
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	COU	INTY		SHEET NO.						
	PAR		Lamar,	, Etc		77				

TABLE OF DEAD LOAD DEFLECTIONS

TYPE T	x28 GIF	RDERS	TYPE 1	rx34 GIF	RDERS	TYPE 1	rx40 GIF	RDERS	TYPE -	Fx46 GIF	RDERS	TYPE 1	Tx54 GIF	RDERS
SPAN LENGTH	"A"	"B"	SPAN LENGTH	"A"	"B"	SPAN LENGTH	"A"	"B"	SPAN LENGTH	"A"	"B"	SPAN LENGTH	"A"	"B"
Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft	Ft
40	0.008	0.012	40	0.005	0.007	40	0.003	0.005	40	0.002	0.003	40	0.001	0.002
45	0.014	0.019	45	0.008	0.012	45	0.005	0.008	45	0.004	0.005	45	0.002	0.003
50	0.021	0.030	50	0.013	0.018	50	0.008	0.012	50	0.006	0.008	50	0.004	0.005
55	0.031	0.045	55	0.019	0.027	55	0.012	0.017	55	0.008	0.012	55	0.006	0.008
60	0.045	0.064	60	0.027	0.038	60	0.018	0.025	60	0.012	0.017	60	0.008	0.011
65	0.063	0.089	65	0.037	0.053	65	0.025	0.035	65	0.017	0.024	65	0.011	0.016
70	0.085	0.121	70	0.051	0.073	70	0.033	0.047	70	0.023	0.032	70	0.015	0.021
75	0.113	0.161	75	0.068	0.096	75	0.044	0.063	75	0.030	0.043	75	0.020	0.028
			80	0.088	0.125	80	0.058	0.082	80	0.039	0.056	80	0.026	0.037
			85	0.113	0.161	85	0.074	0.105	85	0.050	0.072	85	0.033	0.047
			-			90	0.093	0.133	90	0.064	0.090	90	0.042	0.060
						95	0.117	0.166	95	0.079	0.113	95	0.052	0.075
									100	0.098	0.139	100	0.065	0.092
									105	0.119	0.170	105	0.079	0.112

110

0.144

0.205

1⁄4 Pt 🗕 Sym abt 🗆 span 🛛 🗕 🗕 🗆 🗕 🗕 'n

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0.161

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120

125

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0.163

0.193

0.228

DEAD LOAD **DEFLECTION DIAGRAM**

Calculated deflections shown are due to the concrete slab on interior girders only (Ec = 5000 ksi). Adjust values as required for exterior girders and if optional slab forming is used. These values may require field verification.

TABLE OF ESTIMATED QUANTITIES											
	Prestressed Concrete Girders										
SPAN LENGTH	REINF CONCRETE SLAB	ABUT TO INT BT	INT BT TO INT BT	ABUT TO ABUT	TOTAL REINF STEEL						
Ft	SF	LF	LF	LF	Lb						
40	1,440	197.50	197.50	197.50	3,312						
45	1,620	222.50	222.50	222.50	3,726						
50	1,800	247.50	247.50	247.50	4,140						
55	1,980	272.50	272.50	272.50	4,554						
60	2,160	297.50	297.50	297.50	4,968						
65	2,340	322.50	322.50	322.50	5,382						
70	2,520	347.50	347.50	347.50	5,796						
75	2,700	372.50	372.50	372.50	6,210						
80	2,880	397.50	397.50	397.50	6,624						
85	3,060	422.50	422.50	422.50	7,038						
90	3,240	447.50	447.50	447.50	7,452						
95	3,420	472.50	472.50	472.50	7,866						
100	3,600	497.50	497.50	497.50	8,280						
105	3,780	522.50	522.50	522.50	8,694						
110	3,960	547.50	547.50	547.50	9,108						
115	4,140	572.50	572.50	572.50	9,522						
120	4,320	597.50	597.50	597.50	9,936						
125	4,500	622.50	622.50	622.50	10,350						

(4) Fabricator will adjust lengths for girder slopes as required.

⁽⁵⁾ Reinforcing steel weight is calculated using an approximate factor of 2.3 lbs/SF.

MATERIAL NOTES: Provide Class S concrete (f'c = 4,000 psi).

Provide Class S (HPC) concrete if shown elsewhere in Provide Grade 60 reinforcing steel. Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated ~ #4 = 1'.7"

Epoxy coated \sim #4 = 2'-5" Deformed welded wire reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A, D, OA, P or T unless noted otherwise.

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.

Multi-span units, with slab continuous over interior bents, may be formed with the details shown on this sheet and the I-Girder Continuous Slab Detail (IGCS) standard.

See I-Girder Thickened Slab End Details (IGTS) standard for details and quantity adjustments. See Prestressed Concrete Panels (PCP) standard and

Prestressed Concrete Panel Fabrication Details (PCP-FAB) standard for panel details not shown.

See I-Girder Miscellaneous Slab Details (IGMS) standard for miscellaneous details. See applicable rail details for rail anchorage in slab.

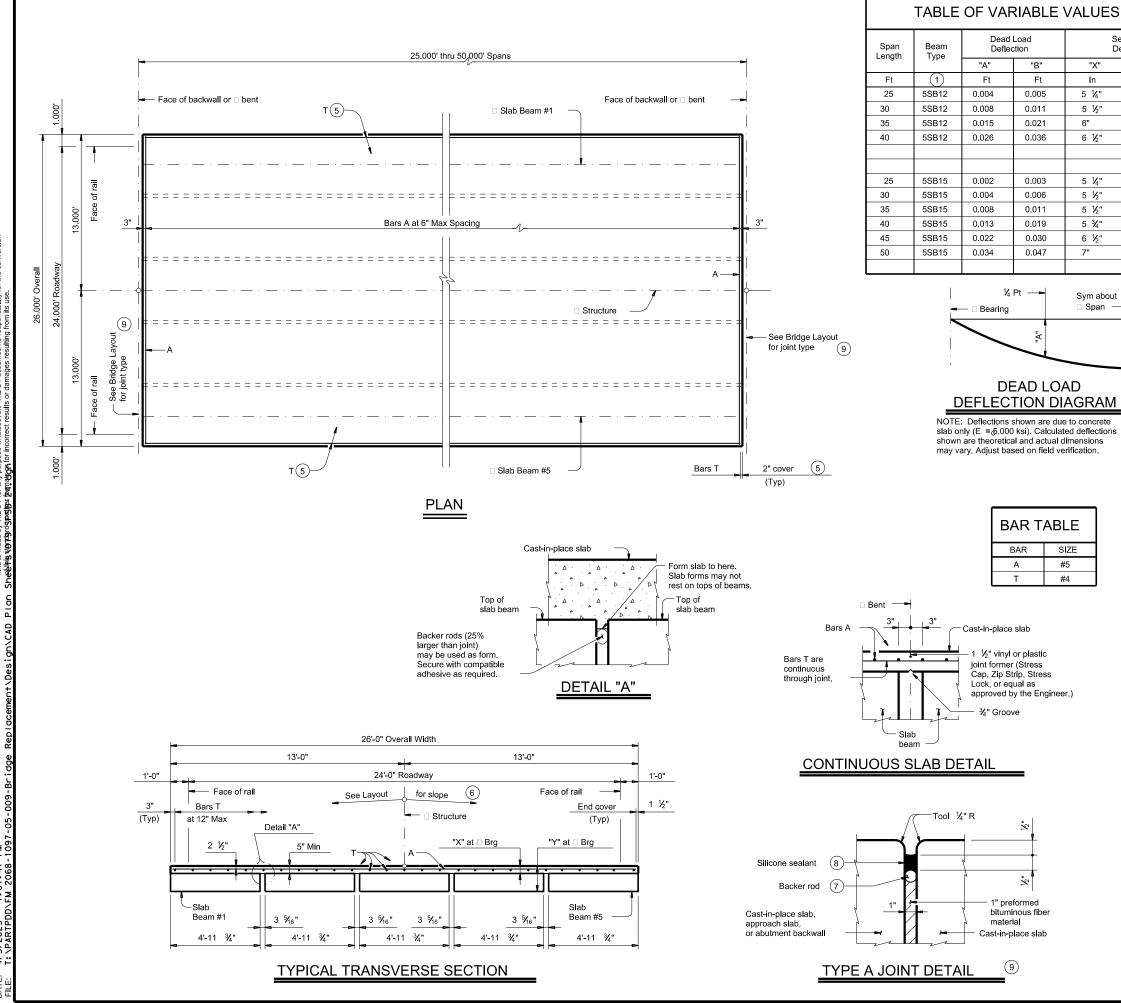
See Permanent Metal Deck Forms (PMDF) standard for

details and quantity adjustments if this option is used.

This standard does not support the use of transition bents.

Cover dimensions are clear dimensions, unless noted otherwise.

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I-GIRD (TYPE Tx28	PRESTRESSED CONCRETE I-GIRDER SPANS (TYPE Tx28 THRU Tx54) 34' ROADWAY									
		S	G-3	34						
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		Y"	
	Ft/	n	
	1'-5	1⁄4"	
	1'-5	1⁄2"	
	1'-6"		
	1'-6	1∕2"	
	1'-8	1⁄4"	
	1'-8	1⁄2"	
	1'-8	1∕2"	
	1'-8	¾"	
	1'-9	1⁄2"	
	1'-10	"	

TABLE OF ESTIMATED QUANTITIES

SPAN	REINF CONCRETE SLAB		PRESTR CONC SLAB BEAM (5SB12 OR 5SB15)						
LENGTH	(SLAB BEAM)	ABUT TO INT BT	INT BT TO INT BT	ABUT TO ABUT	REINF STEEL				
Ft	SF	LF (4)	LF (4)	LF (4)	Lb				
25	650	122.50	122.50	122.50	1,820				
30	780	147.50	147.50	147.50	2,180				
35	910	172.50	172.50	172.50	2,550				
40	1,040	197.50	197.50	197.50	2,910				
45	1,170	222.50	222.50	222.50	3,280				
50	1,300	247.50	247.50	247.50	3,640				

(1) See Bridge Layout for beam type used in the superstructure. These standards do not provide for the use of both SB12 and SB15 beams within the same structure.

- (2) Reinforcing steel weight is calculated using an approximate factor of 2.8 Lbs/SF.
- (3) Based on theoretical beam camber, dead load deflections of 5" cast-in-place concrete slab and a constant grade. The Contractor will adjust these values for any vertical curve.
- (4) Fabricator will adjust beam lengths for beam slopes as required.
- 5 Where slab is continuous over Interior Bents, Bars T are continuous through Joint. See "Continuous Slab Detail".
- (6) This standard does not provide for changes in roadway cross-slopes within the structure.
- (7) 1 $\frac{1}{4}$ " backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- (8) Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- (9) See Bridge Layout for expansion joint locations. If using Type A expansion joints, the maximum distance between joints is 100 feet. Type A joints are subsidiary to Item 422, "Concrete Superstructures".

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Two- or three-span units, with slab continuous over interior bents. may be formed with the details shown on this sheet. See applicable rail details for rail anchorage in slab. This standard does not support the use of transition bents.

Cover dimensions are clear dimensions, unless noted otherwise.

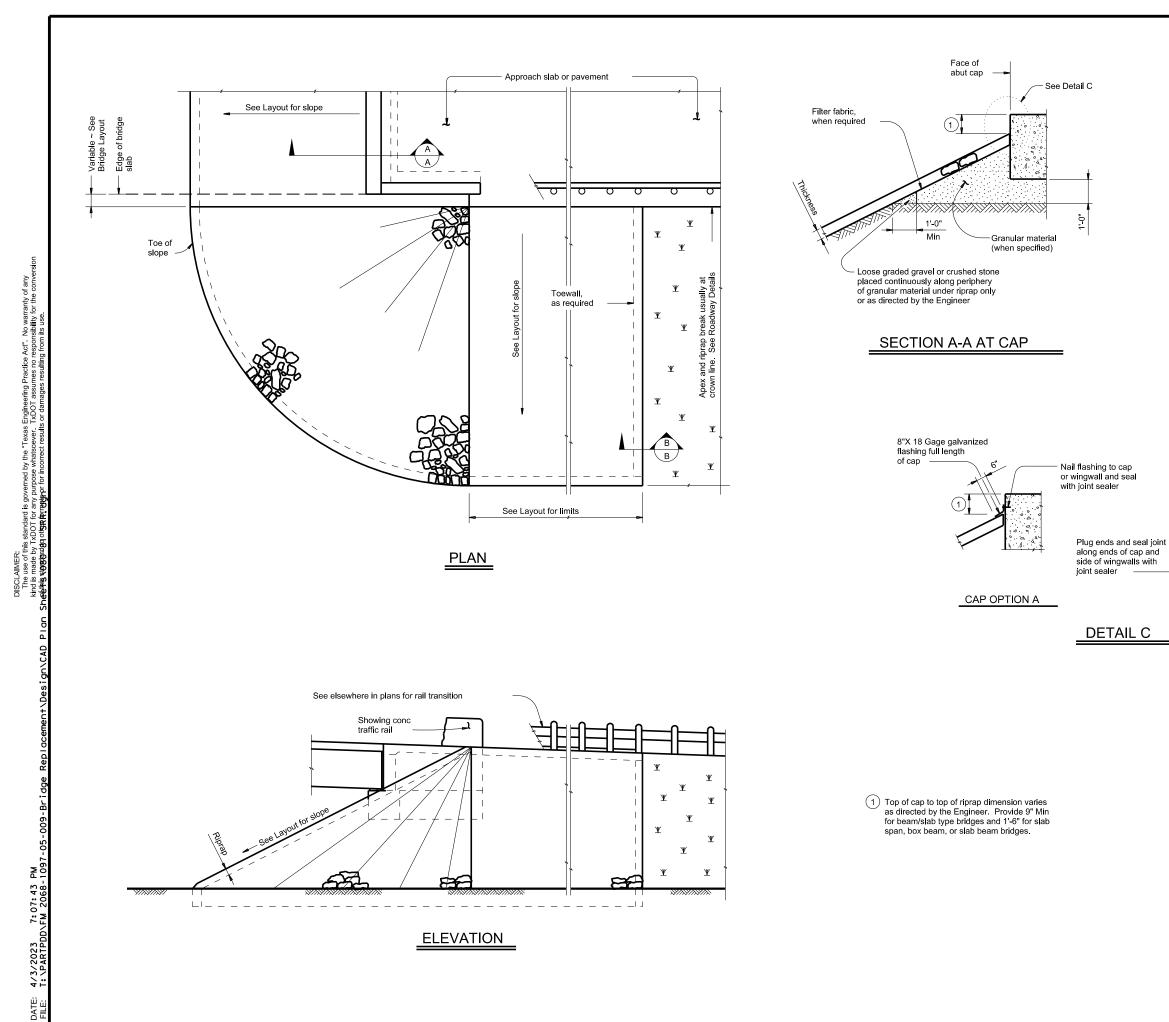
MATERIAL NOTES: Provide Class S concrete (f'c = 4,000 psi). Provide Class S (HPC) concrete if shown elsewhere in the plans. Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated \sim #4 = 1'-7"

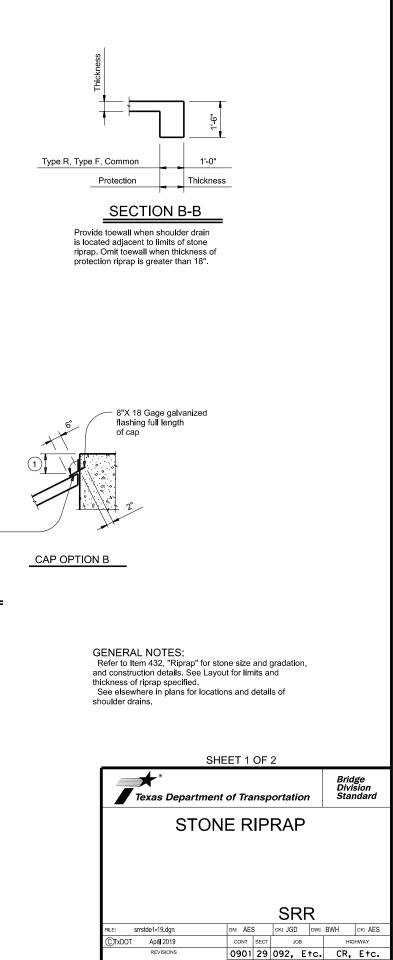
~ #5 = 2'-0"

Epoxy coated	~ #4 = 2'-5'
	~ #5 = 3'-0'

Deformed welded wire reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A or T unless noted otherwise.

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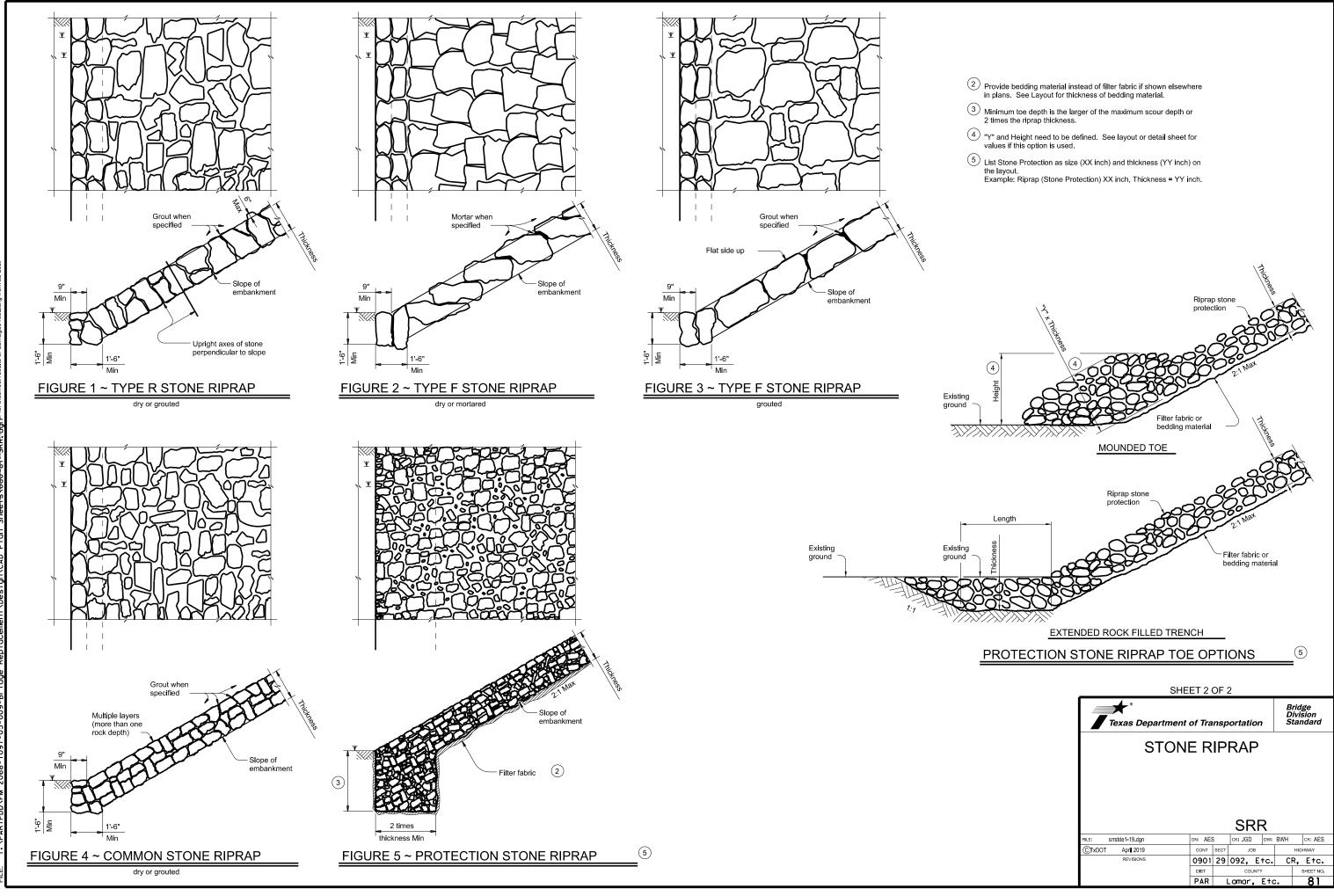




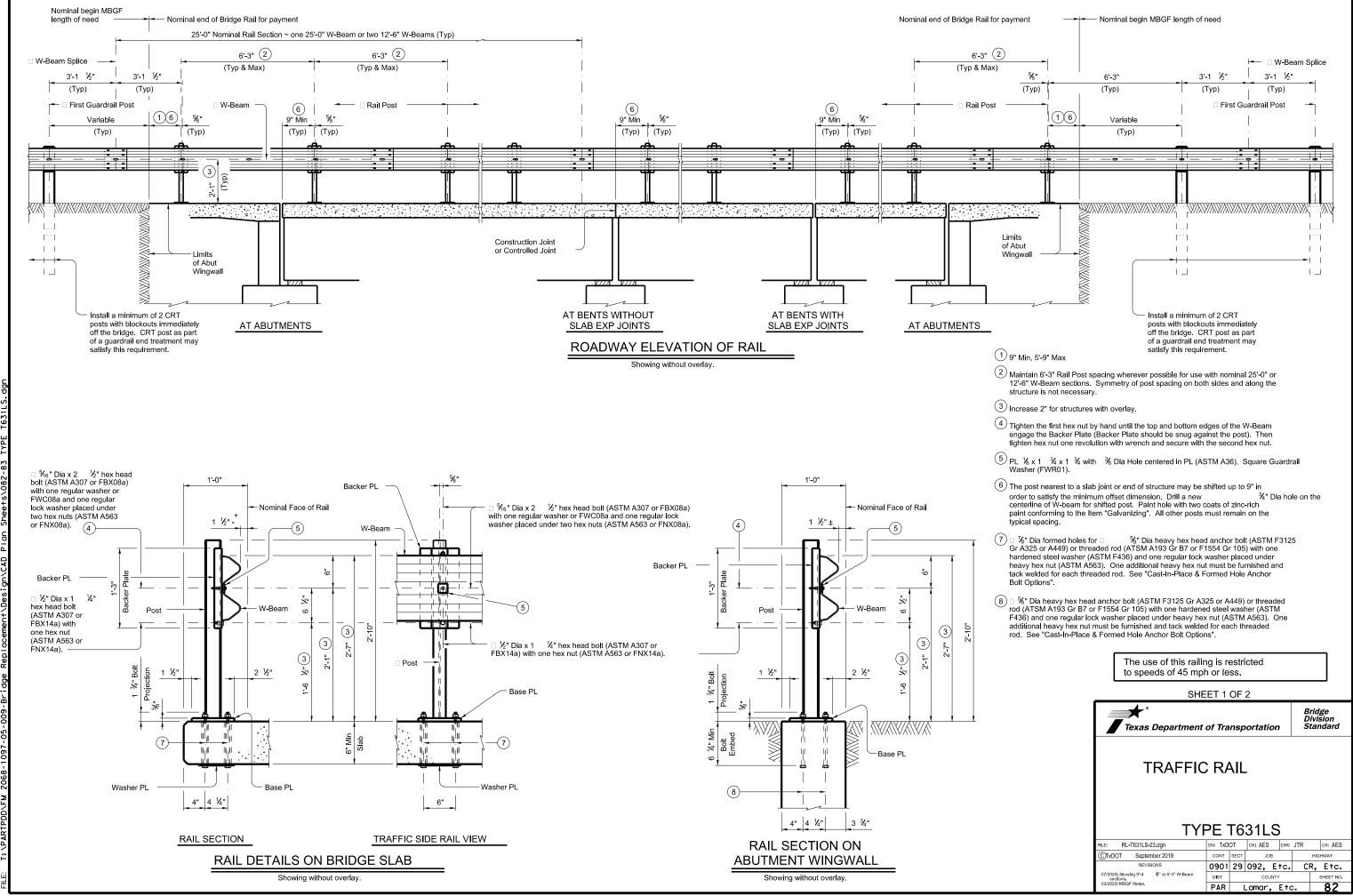
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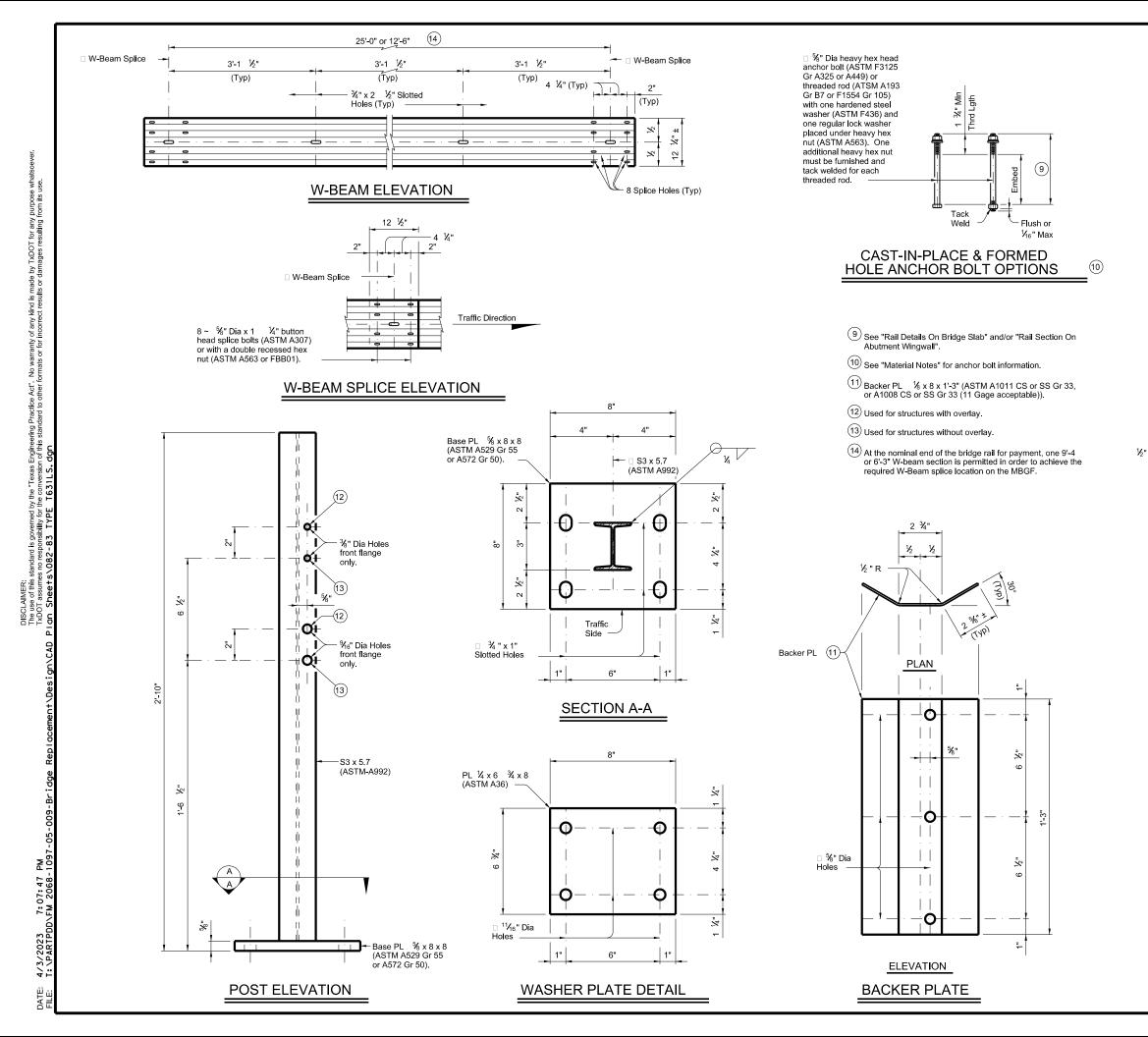
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MBGF AND END TREATMENT NOTES:

This traffic railing must be anchored by metal beam guard fence (MBGF) and/or guard fence end treatments. Determine MBGF length of need in accordance with the Roadway Design Manual, unless otherwise specified. The minimum MBGF length of need required for anchoring the railing is: SGT; or DAT plus 12.5' of MBGF, as applicable. Provide CRT posts as shown in "Roadway Elevation of Rail." The SGT and DAT plus 12.5' MBGF must be installed tangent to primary roadway.

CONSTRUCTION NOTES:

Face of rail post must be plumb unless otherwise approved by the Engineer. Post must be perpendicular to adjacent roadway grade. Use epoxy mortar under post base plates if gaps larger than 1/16" exist

Fully anchored guardrail must be attached to each end of rail. A metal beam guard fence transition is not used with this rail. At the Contractor's option anchor bolts may be an adhesive anchor system. See "Material Notes".

Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

It is recommended to show a Rail Layout with rail posts and W-beam splices. Fabricator must submit erection drawings to the Engineer for approval.

Round or chamfer exposed edges of rail post and backer plate to approximately \mathcal{Y}_{16} " by grinding. Shop drawings are not required for this rail. to approximately

MATERIAL NOTES: Galvanize all steel components.

Anchor bolts for base plate must be %" Dia ASTM F3125 Gr A325 or A449 bolts (or ASTM A193 Gr B7 or F1554 Gr 105 threaded rods with one tack welded heavy hex nut each) with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements.

Optional adhesive anchorage system must be $$\%^{\prime\prime}$$ Gr B7 or F1554 Gr 105 fully threaded rods with one hardened 5/8" Dia ASTM A193 steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 ¾". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing."

W-beam must meet the requirements of Item 540, "Metal Beam Guard Fence" except as modified in the plans. The Contractor may furnish rail elements of 25'-0" or 12'-6" (Nominal) lengths and a single rail element of 9'-4 1/2" or 6'-3" (Nominal) length. W-Beam must have slotted holes at 3'-1

Some part numbers from the "Task Force 13" Guide to Standardized Highway Barrier Hardware have been furnished for quick reference.

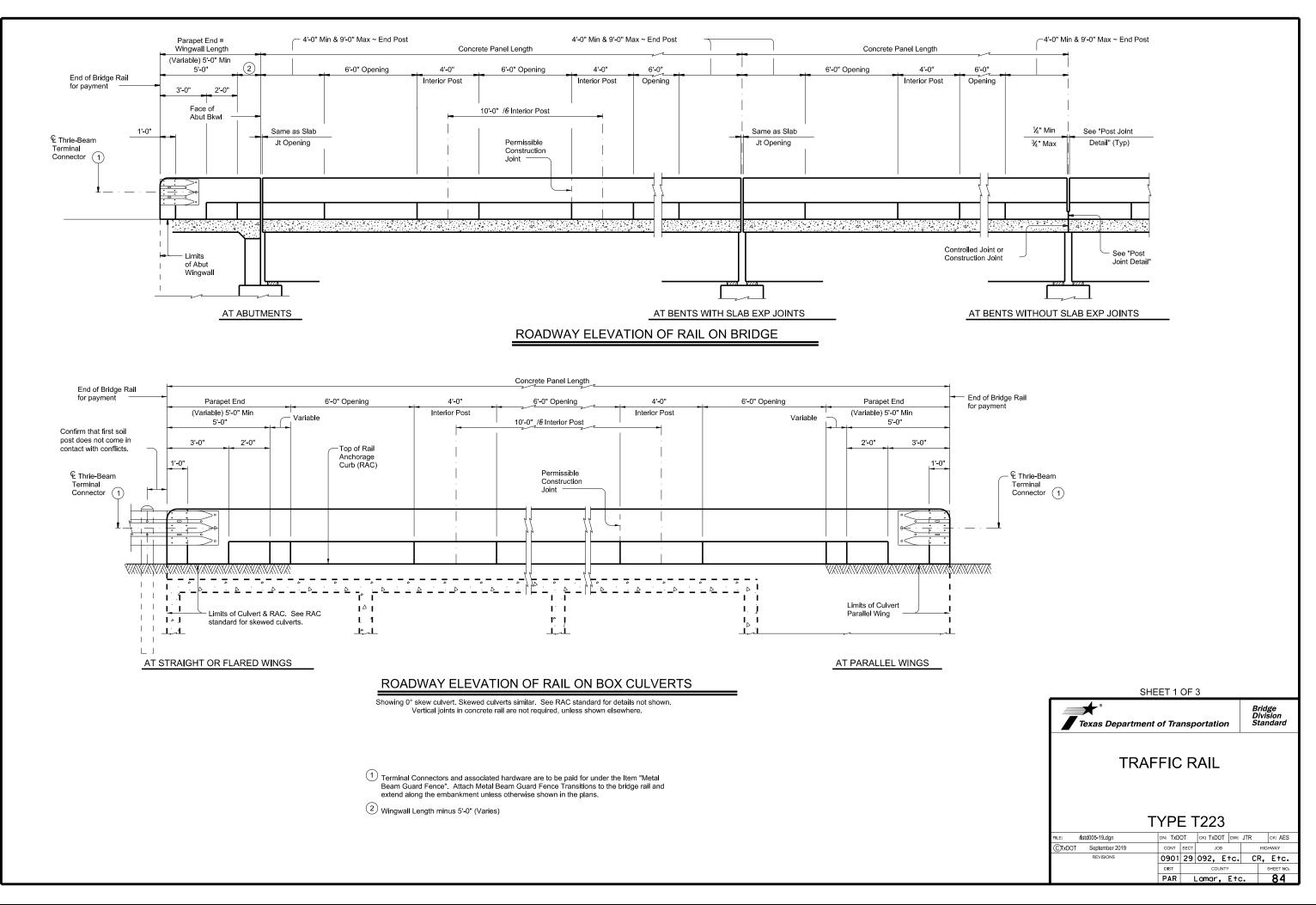
GENERALANOTE Successfully evaluated by full-scale crash test to meet MASH TL-2 criteria. This railing can be used for speeds of 45 mph and less.

This rail is designed to deflect approximately 2' to 2'-6" as it contains and redirects the errant vehicle. This rail may not be installed on top of or behind curbs that project above finished grade, on bridges with expansion joints providing more than 5" movement, on retaining walls, or on grade separations and interchanges. Repairs to impact-damaged post and base plate unit are not

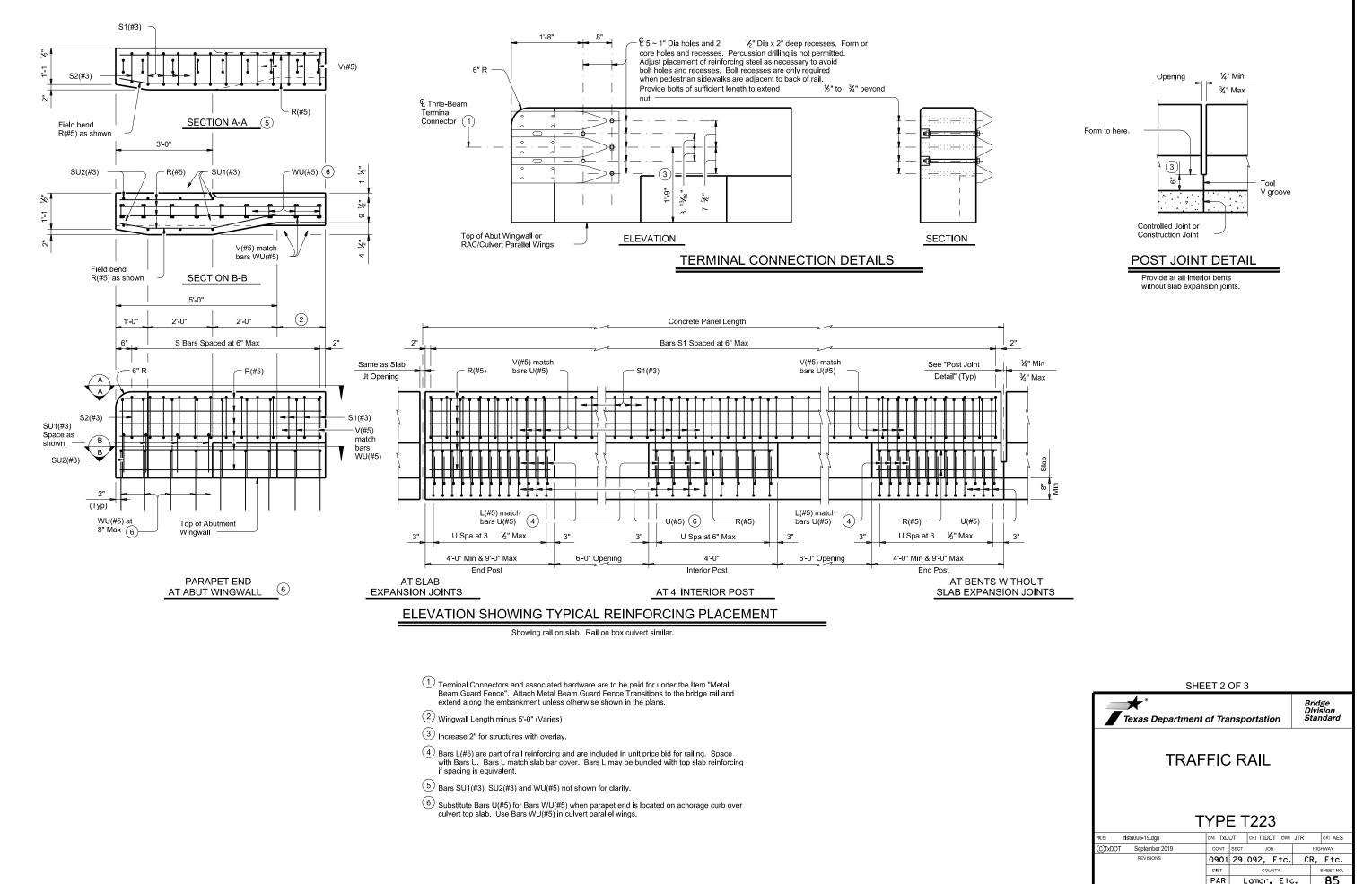
permitted. Replace all impact-damaged posts with a new post and base plate unit.

Average weight of railing with no overlay: 13 plf total.

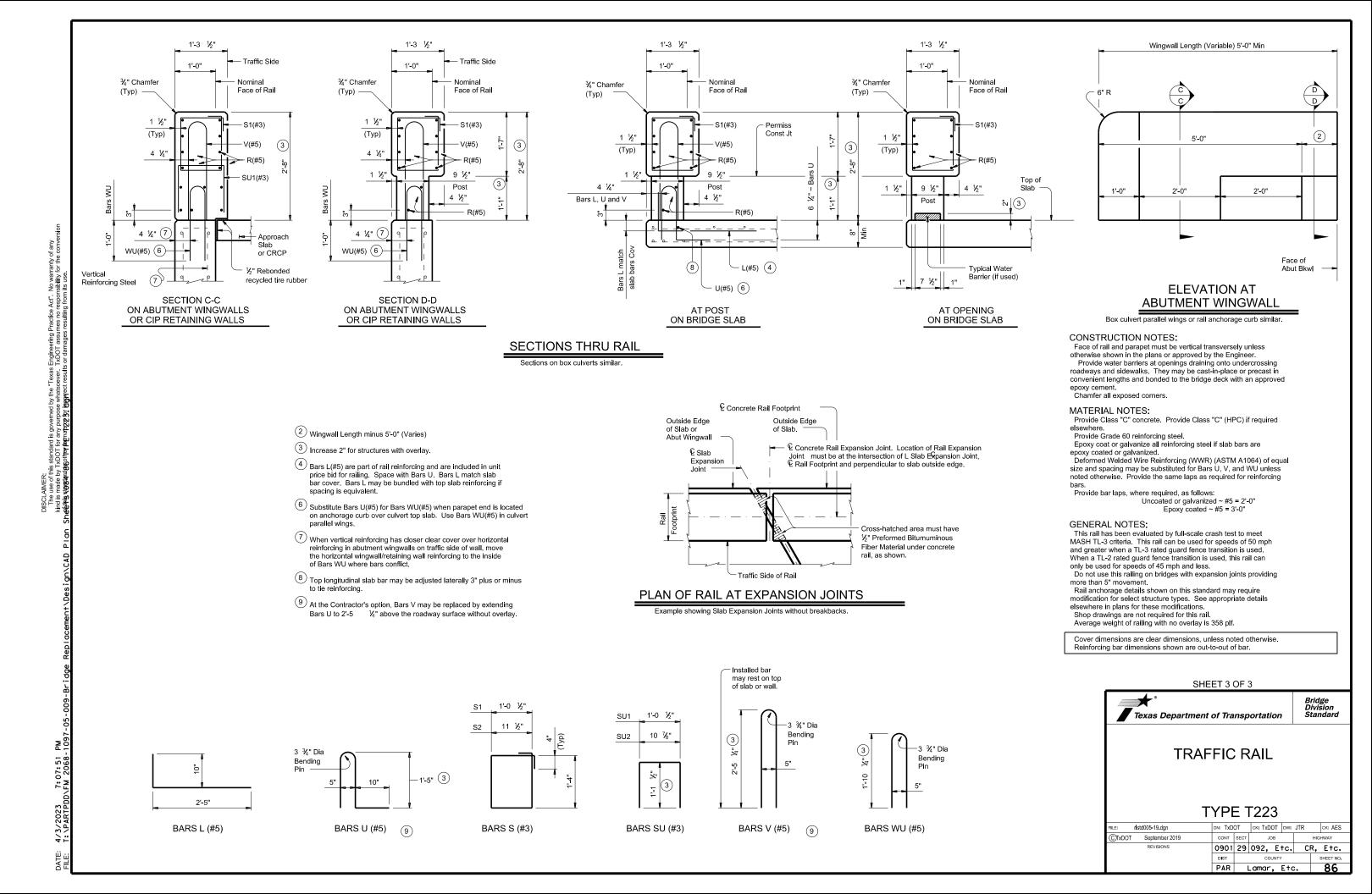
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REVISIONS	0901	29	092,	Etc.	CR,	Etc.
07/2020: Allowing 9'-4 ½" or 6'-3" W-Beam sections.	DIST		COUN	ITY		SHEET NO.
03/2023: MBGF Notes.	PAR		_amar,	E†c	.	83

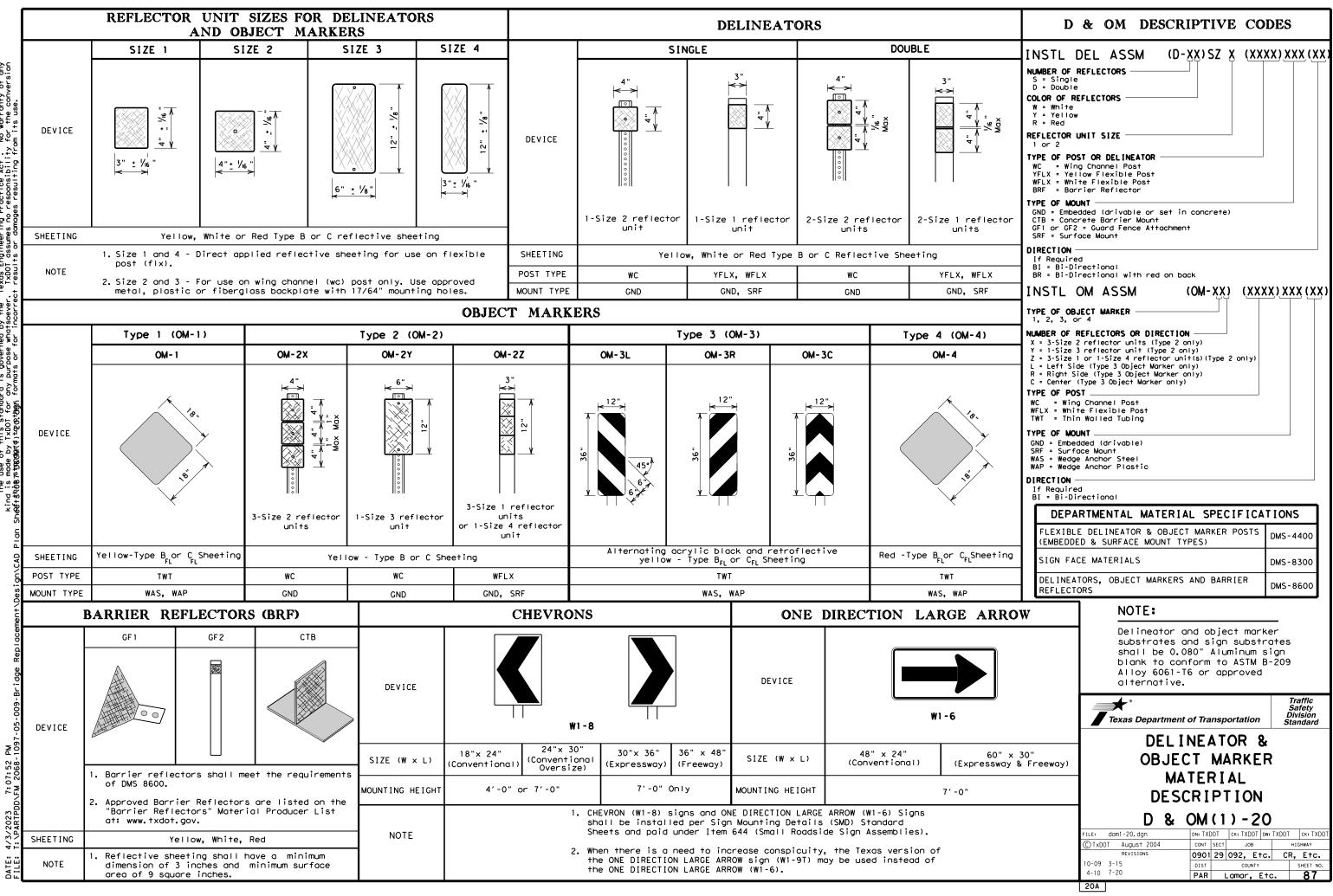


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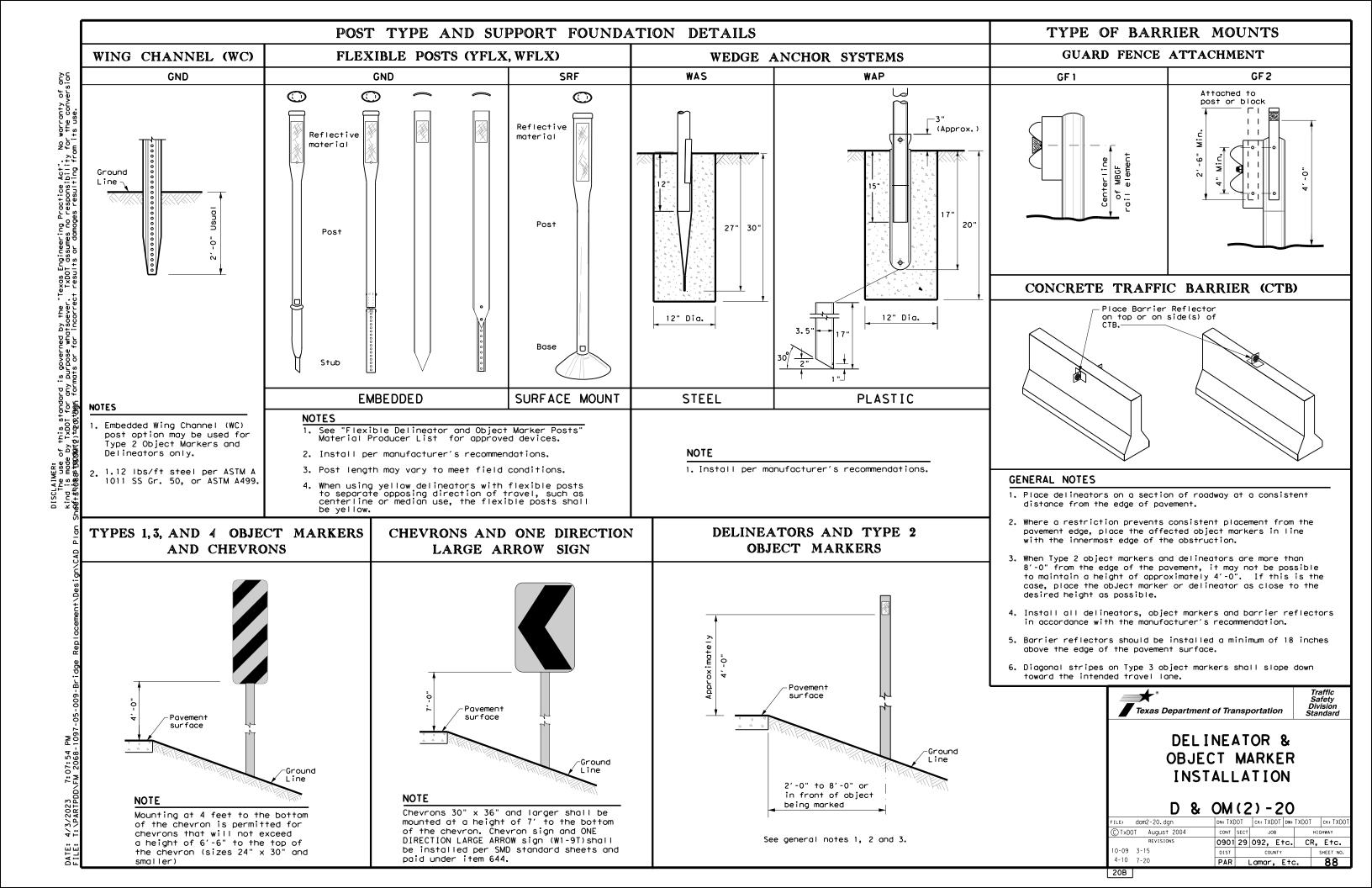


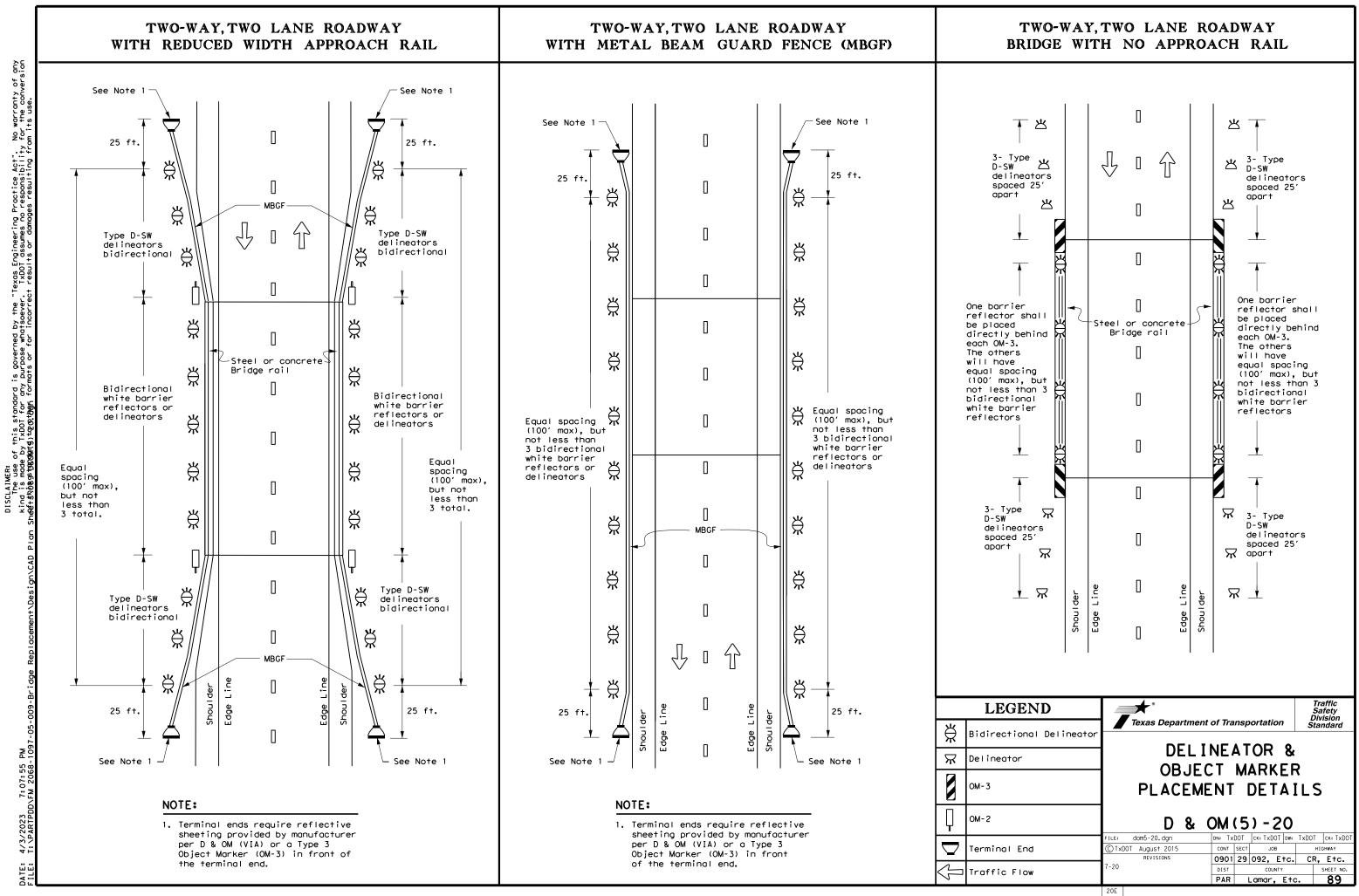
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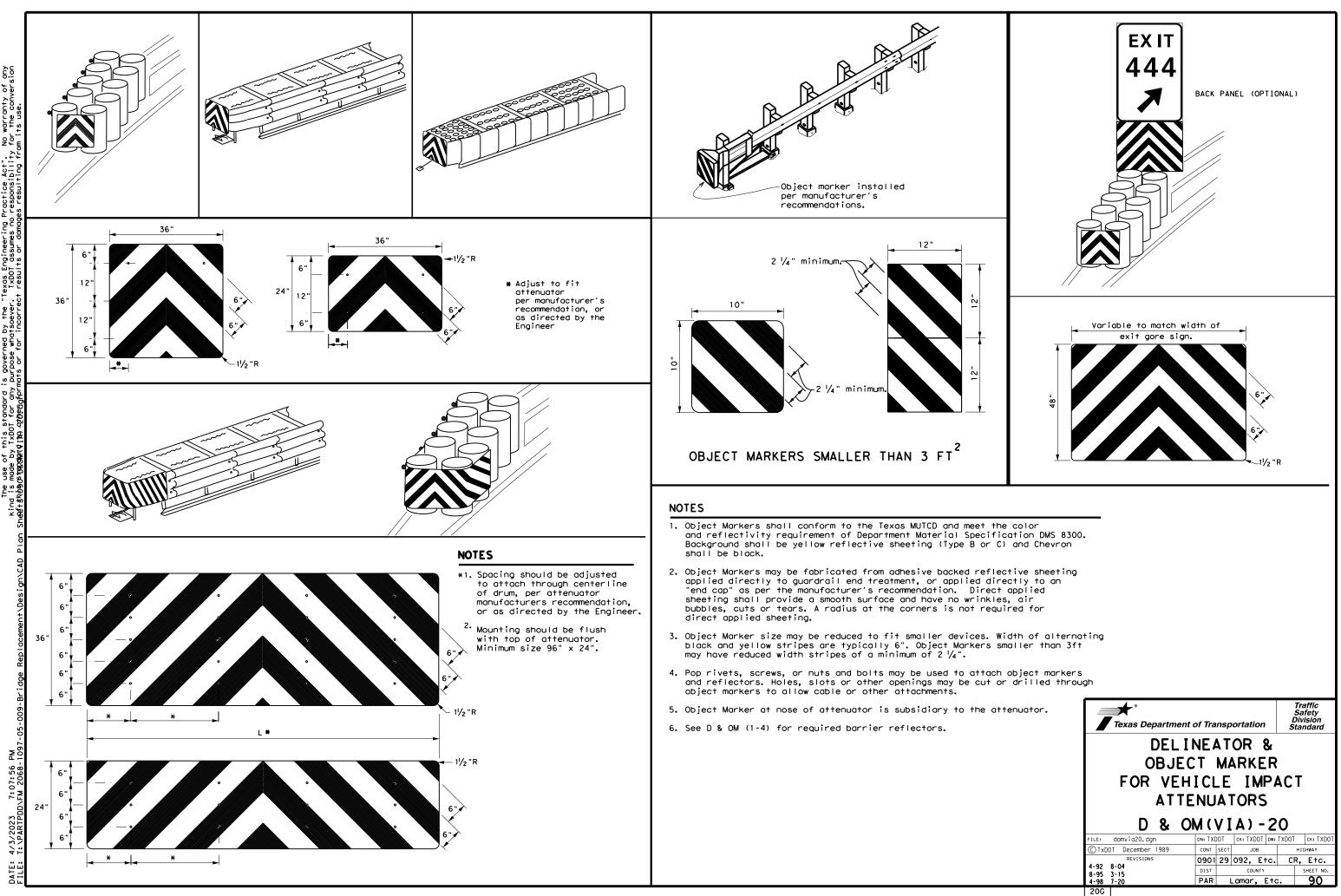


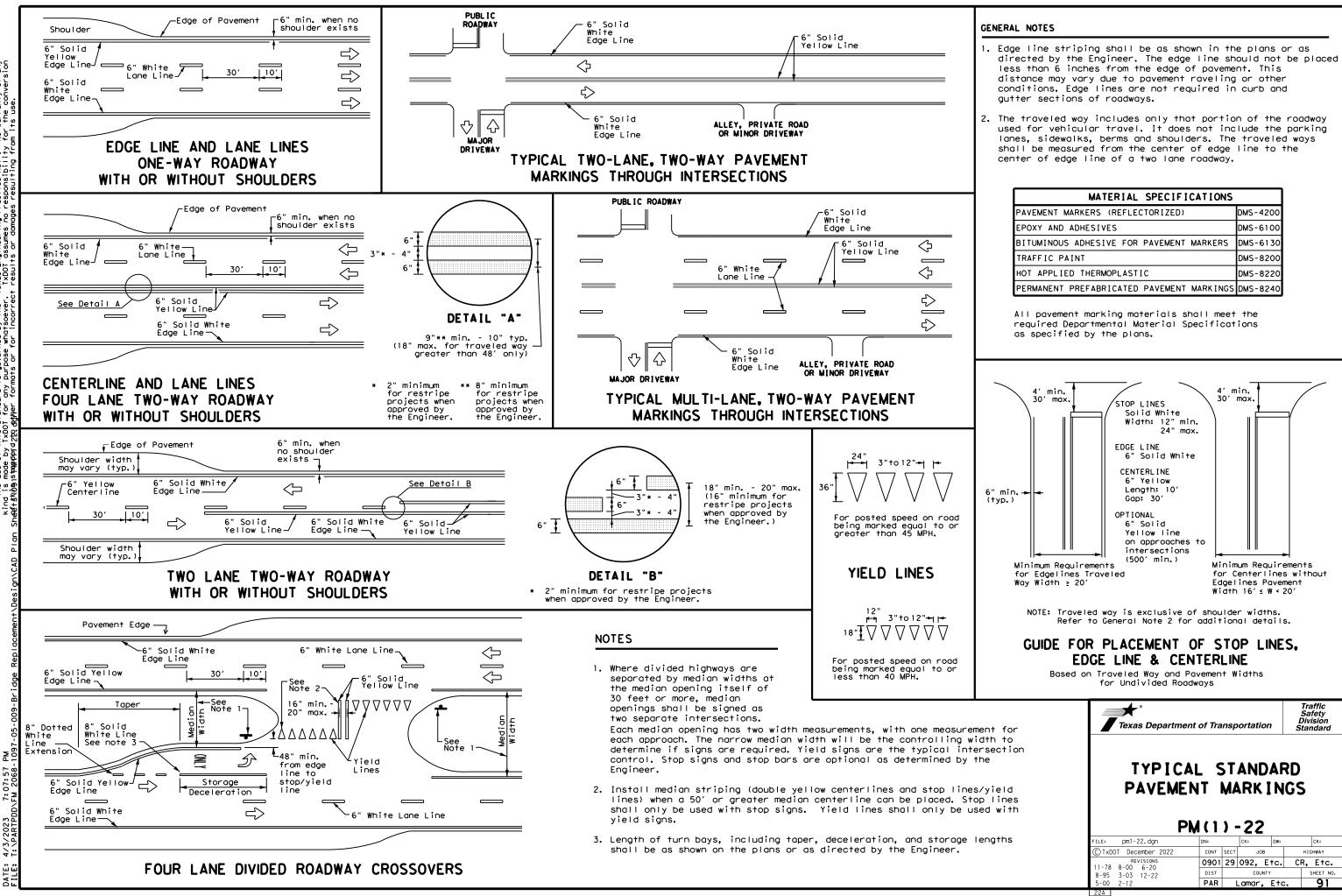


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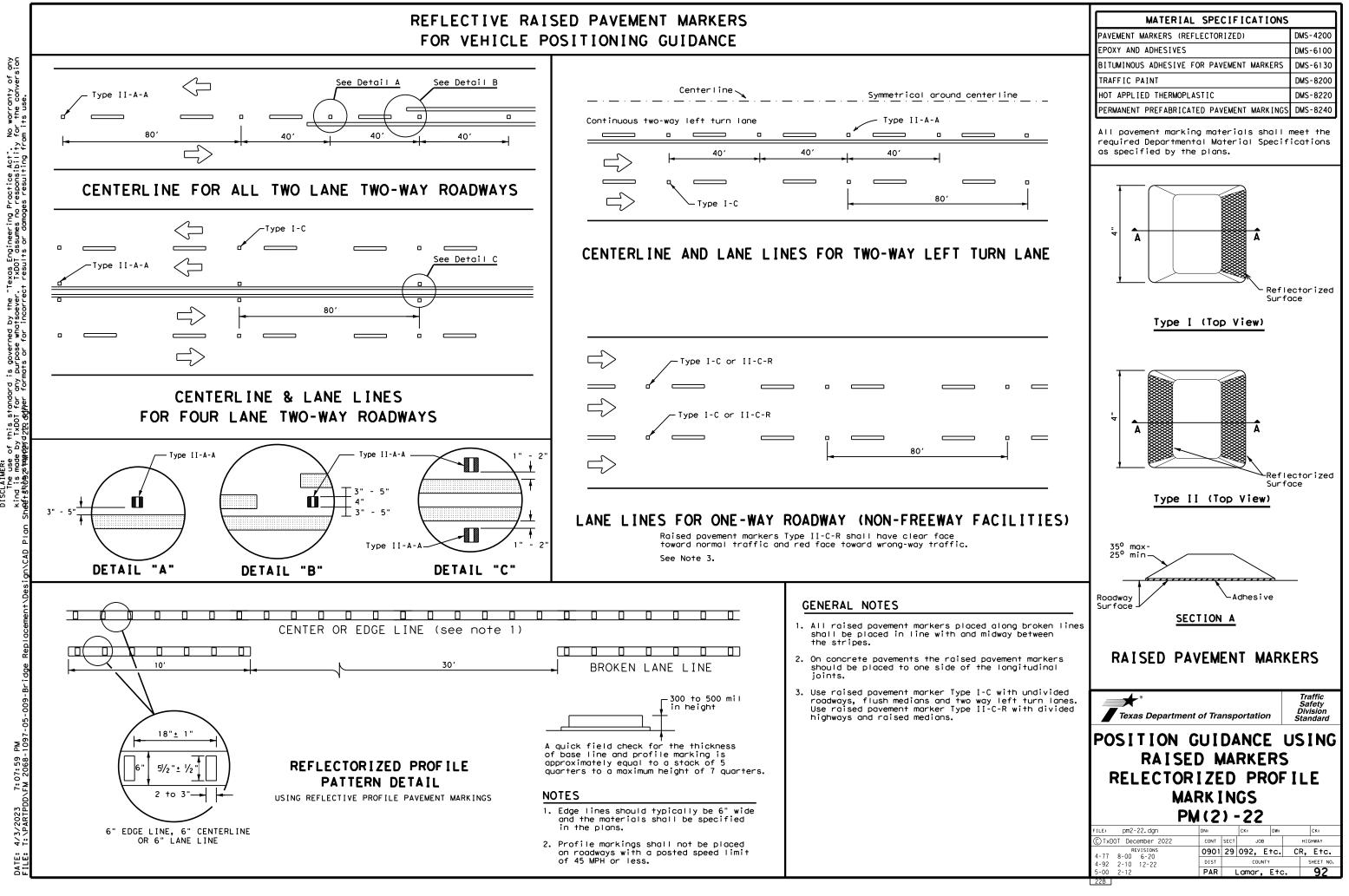
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MATERIAL SPECIFICATIONS	
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
EPOXY AND ADHESIVES	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
TRAFFIC PAINT	DMS-8200
HOT APPLIED THERMOPLASTIC	DMS-8220
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240

FOR VEHICLE POSITIONING GUIDANCE

DISCL



STORMWATER POLLUTION PRVENTION PLAN (SWP3):

This SWP3 has been developed in accordance with TxDOT policy for projects disturbing less than 1 acre of soil, and not part of a larger common plan of development.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept at the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans, and the project's environmental permits, issues, and commitments (EPICs).

1.0 SITE/PROJECT DESCRIPTION BRIDGE REPLACEMENT

1.1 PROJECT CONTROL SECTION JOB (CSJ): 0901 - 29 - 092

1.2 PROJECT LIMITS:

From: SOUTHWEST OF THE CITY OF ROXTON ON COUNTY ROAD (CR26320) AT MORRISON CREEK

1.3 PROJECT COORDINATES:

BEGIN: (Lat) 33°31'0.37"N ,(Long) 95°46'42.89"W

END: (Lat) 33°31'0.15"N ,(Long) 95°46'38.68"W

1.4 TOTAL PROJECT AREA (Acres): .32

1.5 TOTAL AREA TO BE DISTURBED (Acres): .01 (31%)

1.6 NATURE OF CONSTRUCTION ACTIVITY:

INCLUDES PREP ROW, EMBANKMENT FOR FILL, ROAD GRADING, DITCH GRADING, EROSION AND SEDIMENTARY CONTROLS, EXCAVATION, AND TOPSOIL WORK FOR FINAL SEEDING.

1.7 MAJOR SOIL TYPES:

Soil Type	Description	X Grading operations, excavation, and embankment
NAHATCHE	CONSISTING OF CLAY LOAM	 X Excavate and prepare subgrade for proposed paver widening Remove existing culverts, safety end treatments (SE X Remove existing metal beam guard fence (MBGF), X Install proposed pavement per plans Install culverts, culvert extensions, SETS X Install mow strip, MBGF, bridge rail X Place flex base X Rework slopes, grade ditches X Blade windrowed material back across slopes X Revegetation of unpaved areas X Achieve site stabilization and remove sediment and erosion control measures Other: Other:
		1

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

- PSLs determined during preconstruction meeting
- □ PSLs determined during construction
- No PSLs planned for construction

Туре	Sheet #s
All off POW PSLs required by th	o Contractor are the Contractor's

All off-ROW PSLs required by the Contractor are the Contractor's | responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the
Construction Activity Schedule and Ceasing Record in
Attachment 2.3.)
X Mobilization
Install sediment and erosion controls
X Blade existing topsoil into windrows, prep ROW, clear and gru
x Remove existing pavement
X Grading operations, excavation, and embankment
X Excavate and prepare subgrade for proposed pavement
widening
Remove existing culverts, safety end treatments (SETs)
X Remove existing metal beam guard fence (MBGF), bridge rail
X Install proposed pavement per plans
Install culverts, culvert extensions, SETs
🗴 Install mow strip, MBGF, bridge rail
X Place flex base

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- X Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- X Solvents, paints, adhesives, etc. from various construction activities
- X Transported soils from offsite vehicle tracking
- X Construction debris and waste from various construction activities
- X Contaminated water from excavation or dewatering pump-out water

- □ Sanitary waste from onsite restroom facilities
- X Trash from various construction activities/receptacles
- □ Long-term stockpiles of material and waste
- Other: ______
- □ Other:_____
- Other: ______

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters

cerving watere.	
Tributaries	Classified Waterbody
MORRISON CREEK	NORTH SULPUR RIVER 030
Add (*) for impaired waterbodies	s with pollutant in ().

1.12 ROLES AND RESPONSIBILITIES: TXDOT

X Development of plans and specifications

X Perform SWP3 inspections

X Maintain SWP3 records and update to reflect daily operations

Other: _____

Other: _____

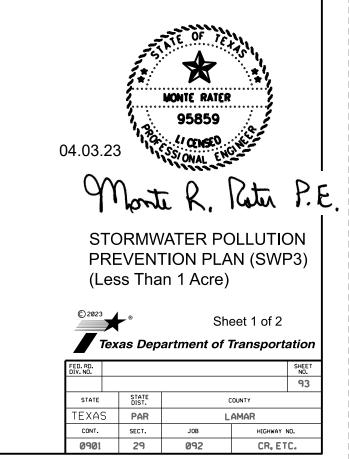
1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

- X Maintain schedule of major construction activities
- X Install, maintain and modify BMPs

Other:

□ Other:_____



2.0 BEST MANAGEMENT PRACTICES (BMPs)	2.3 PERMANENT CONTRO	DLS:						
AND CONTROLS, INSPECTION, AND	(Coordinate post-construction BMPs with appropriate TxDOT							
MAINTENANCE	maintenance sections.)			2.5 POLLUTION PREVENT	ION MEASURES:			
	BMPs To Be Left In Place Po	Ps To Be Left In Place Post Construction:			X Chemical Management			
The Contractor shall be the responsible party for implementing	Туре		oning	X Concrete and Materials Wa	aste Management			
the BMPs described herein and for complying with the SWP3	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	From	То	🛛 🗴 Debris and Trash Manager	nent			
for control of erosion and sedimentation during day-to-day				🗴 Dust Control				
operations. The Contractor shall implement changes to this				🛛 🕱 Sanitary Facilities				
SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.				X Other:				
SWP3 of the CGP.								
2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:								
T/P				□ Other:				
 Protection of Existing Vegetation 								
 Vegetated Buffer Zones 				│ □ Other:				
Soil Retention Blankets								
Mulching/ Hydromulching								
Soil Surface Treatments								
 Temporary Seeding X Permanent Planting, Sodding or Seeding 	Refer to the Environmental L	avout Sheets/ SWP3	l avout Sheets					
 Biodegradable Erosion Control Logs 	located in Attachment 1.2 of							
Rock Filter Dams/ Rock Check Dams				2.6 VEGETATED BUFFER ZONES:				
Vertical Tracking				Natural vegetated buffers sha				
□ □ Interceptor Swale				protect adjacent surface wate	-			
				I manage and had bla due to				
🗆 🕱 Riprap				zones are not feasible due to				
Diversion Dike				additional sediment control m				
 Diversion Dike Temporary Pipe Slope Drain 			I S		easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control 	2.4 OFFSITE VEHICLE TF		ILS:	additional sediment control m into this SWP3.	easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes 	X Excess dirt/mud on road r	emoved daily	ILS:	additional sediment control m	easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	X Excess dirt/mud on road r X Haul roads dampened for	emoved daily dust control		additional sediment control m into this SWP3.	easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes 	X Excess dirt/mud on road r	emoved daily dust control covered with tarpauli		additional sediment control m into this SWP3.	easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be a X Stabilized construction ex 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	easures have been ir	ncorporated		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be only 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	easures have been ir	ncorporated		
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 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be of X Stabilized construction ex Other:	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	easures have been ir	ncorporated		
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 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be of X Stabilized construction ex Other: Other: Other: 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	easures have been ir	ncorporated		
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 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be of X Stabilized construction ex Other: Other: Other: 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	ayout Sheets/ SWP3 I	oning To		
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 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be of X Stabilized construction ex Other: Other: Other: 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	ayout Sheets/ SWP3 I	oning To		
 Diversion Dike Temporary Pipe Slope Drain Embankment for Erosion Control Paved Flumes Other:	 X Excess dirt/mud on road r X Haul roads dampened for X Loaded haul trucks to be of X Stabilized construction ex Other: Other: Other: 	emoved daily dust control covered with tarpauli t	n	additional sediment control m into this SWP3.	ayout Sheets/ SWP3 I	oning To		

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- ${\tt X}$ Potable water sources
- 🕱 Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3 .

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.



STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 2 of 2

Texas Department of Transportation

FED. RD. DIV. NO.	PROJECT NO.			SHEET NO.		
STATE		STATE DIST.	COUNTY			
TEXAS	S	PAR	LAMAR			
CONT.		SECT.	JOB HIGHWAY NO.			
0901		29	092 CR, ETC.			

STORMWATER POLLUTION PREVENTION PLAN (SWP3):

This SWP3 has been developed in accordance with the TPDES Construction General Permit TXR150000 (CGP). The Texas Department of Transportation (TxDOT) ensures that project specifications include adequate best management practices (BMPs) for this project.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept in the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans and the projects environmental permits, issues, and commitments (EPICs). A copy of the CGP is included in Attachment 2.12 of the SWP3 binder.

1.0 SITE/PROJECT DESCRIPTION

1.1 PROJECT CONTROL SECTION JOB (CSJ): 1097-05-009

1.2 PROJECT LIMITS:

From: At E FORK JERNIGAN CREEK

1.3 PROJECT COORDINATES:

BEGIN: (Lat) 33.3468575°, (Long) -95.8076573°

END: (Lat) <u>33.3471289°</u>, (Long) <u>-95.8115013°</u>

1.4 TOTAL PROJECT AREA (Acres): 3.24

1.5 TOTAL AREA TO BE DISTURBED (Acres): 0.91 (28.1%)

1.6 NATURE OF CONSTRUCTION ACTIVITY:

BRIDGE REPLACEMENT

1.7 MAJOR SOIL TYPES:

Soil Type	Description
TRINITY CLAY, 0-1% SLOPES	CLAY, MODERATELY WELL DRAINED, HIGH RATE OF RUNOFF, SLIGHT EROSION POTENTIAL
WILSON SILT LOAM, 0-2% SLOPES	SILT LOAM, MODERATELY WELL DRAINED, VERY HIGH RATE OF RUNOFF, MODERATE EROSION POTENTIAL
DEPORT CLAY, 1-3% SLOPES	CLAY, SOMEWHAT POORLY DRAINED, VERY HIGH RATE OF RUNOFF, MODERATE EROSION POTENTIAL

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

- □ PSLs determined during preconstruction meeting
- ☑ PSLs determined during construction
- No PSLs planned for construction

Туре	Sheet #s	
responsibility. The Contractor sh		
by local, state, federal laws for o shall provide diagrams, areas of	disturbance, acreage, and	
BMPs for all off-ROW PSLs with	in one mile of the project.	
1.9 CONSTRUCTION ACTIVI		
(Use the following list as a starti Construction Activity Schedule a		
Attachment 2.5.)		

Mobilization

 $\ensuremath{\mathbb{X}}$ Install sediment and erosion controls

- $\ensuremath{\mathbb{X}}$ Blade existing topsoil into windrows, prep ROW, clear and gru
- ☑ Remove existing pavement
- $\ensuremath{\mathbb{X}}$ Grading operations, excavation, and embankment
- ✗ Excavate and prepare subgrade for proposed pavement widening
- I Remove existing culverts, safety end treatments (SETs)
- X Remove existing metal beam guard fence (MBGF), bridge rail
- Install proposed pavement per plans

☐ Install culverts, culvert extensions, SETs

- X Install mow strip, MBGF, bridge rail
- In Place flex base
- ☐ Rework slopes, grade ditches
- Blade windrowed material back across slopes
- X Achieve site stabilization and remove sediment and erosion control measures

] Other: _____

Other:

] Other: _____

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- X Sediment laden stormwater from stormwater conveyance over disturbed area
- ✗ Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- Ճ Solvents, paints, adhesives, etc. from various construction activities
- X Transported soils from offsite vehicle tracking
- X Construction debris and waste from various construction activities
- X Contaminated water from excavation or dewatering pump-out water
- X Sanitary waste from onsite restroom facilities
- X Trash from various construction activities/receptacles
- $\ensuremath{\mathbb{X}}$ Long-term stockpiles of material and waste
- ||
 □ Other: _____

||
□ Other: _____

Other:

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

	receiving waters.	
	Tributaries	Classified Waterbody
	EAST FORK JERNIGAN CREEK, 0307D	JIM CHAPMAN LAKE, 0307
b		
	* Add (*) for impaired waterbodies	s with pollutant in ().
	1.12 ROLES AND RESPONSIE	BILITIES: TXDOT
	X Development of plans and spe	cifications
	X Submit Notice of Intent (NOI) to	o TCEQ (≥5 acres)
	X Post Construction Site Notice	
	X Submit NOI/CSN to local MS4	
	X Perform SWP3 inspections X Maintain SWP3 records and up	data to reflect daily operations
	X Complete and submit Notice of	
	X Maintain SWP3 records for 3 ye	ears
	□ Other:	
	Other:	

1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

X Submit Notice of Intent (NOI) to TCEQ (≥5 acres)

X Post Construction Site Notice

X Submit NOI/CSN to local MS4

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

X Complete and submit Notice of Termination to TCEQ

X Maintain SWP3 records for 3 years

□ Other: _____

Other:

Other:

1.14 LOCAL MUNICIPAL SEPARATE STORM SEWER SYSTEM (MS4) OPERATOR COORDINATION:

MS4 Entity



04/03/2023

(alle O. Bayal, P.E.

STORMWATER POLLUTION PREVENTION PLAN (SWP3)



Texas Department of Transportation

FED. RD. DIV. NO.		PROJECT NO.				SHEET NO.	
		0901-29-092, Etc.					95
STATE		STATE DIST. COUNTY					
TEXA	5	PAR Lamar, Etc.			•		
CONT.		SECT.	JOB HIGHWAY NO.			Y NO.	
090:	l	29	Ø92,	Etc.		CR,	Etc.

STORMWATER POLLUTION PREVENTION PLAN (SWP3):

2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:

T / P

- \Box \Box Protection of Existing Vegetation
- □ □ Vegetated Buffer Zones
- □ □ Soil Retention Blankets
- Geotextiles
- Image: Mulching / Hydromulching
- □ □ Soil Surface Treatments
- 🛛 🗆 Temporary Seeding
- □ X Permanent Planting, Sodding or Seeding
- □ □ Biodegradable Erosion Control Logs
- 🛛 🗆 Rock Filter Dams/ Rock Check Dams
- □ □ Vertical Tracking
- □ □ Interceptor Swale
- 🗆 🛛 Riprap
- Diversion Dike
- Temporary Pipe Slope Drain
- □ □ Embankment for Erosion Control
- Paved Flumes
- □ □ Other:_____
- Other:_____
- □ □ Other:_____
- □ □ Other:_____

2.2 SEDIMENT CONTROL BMPs:

Т/Р

- □ □ Biodegradable Erosion Control Logs
- □ □ Dewatering Controls
- □ □ Inlet Protection
- 🛛 🗆 Rock Filter Dams/ Rock Check Dams
- Sandbag Berms
- 🛛 🗆 Sediment Control Fence
- □ □ Stabilized Construction Exit
- □ □ Floating Turbidity Barrier
- Vegetated Buffer Zones
- □ □ Vegetated Filter Strips
- □ □ Other:_____
- Other: ______
- □ □ Other:_____
- Other: ______

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

Sediment control BMPs requiring design capacity calculations (See SWP3 Attachment 1.3.):

T / P

- Sediment Trap
 - Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
 - □ 3,600 cubic feet of storage per acre drained
- □ □ Sedimentation Basin
 - □ Not required (<10 acres disturbed)
 - □ Required (>10 acres) and implemented.
 - □ Calculated volume runoff from 2-year, 24-hour storm for each acre of disturbed area
 - □ 3,600 cubic feet of storage per acre drained

Other:

- $\hfill\square$ Required (>10 acres), but not feasible due to:
- Available area/Site geometry
- □ Site slope/Drainage patterns
- □ Site soils/Geotechnical factors
- Public safety

2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Tuno	Tuno Stationing				
Туре	From	То	protect a		
			zones ar		
			additiona		
			into this S		
Pofer to the Environmental Love	out Shooto/ SM/D	2 Lovout Shooto			
Refer to the Environmental Layo		S Layour Sheers			
located in Attachment 1.2 of this	SVVP3				

2.4 OFFSITE VEHICLE TRACKING CONTROLS:

- I Excess dirt/mud on road removed daily
- ${\tt X}$ Haul roads dampened for dust control
- $\ensuremath{\mathbb{X}}$ Loaded haul trucks to be covered with tarpaulin
- X Stabilized construction exit
- _ • • •
- □ Other:_____
- □ Other:_____
- □ Other:

2.5 POLLUTION PREVENTION MEASURES:

- X Chemical Management
- X Concrete and Materials Waste Management

Other:_____

- $\ensuremath{\mathbb{X}}$ Debris and Trash Management
- 🛛 Dust Control
- X Sanitary Facilities
- Other:_____

Other: _____

□ Other:

2.6 VEGETATED BUFFER ZONES:

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

	Tuno	Static	oning
	Туре	From	То
Sheets			
Refe	r to the Environmental La	vout Sheets/ SWP3 L	avout Sheets
	ed in Attachment 1.2 of th		

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.5 of this SWP3.



STORMWATER POLLUTION PREVENTION PLAN (SWP3)



Sheet 2 of 2

Texas Department of Transportation

FED. RD. DIV. NO.	PROJECT NO.							ET).
		0901-29-092, Etc.						
STATE		STATE DIST.	COUNTY					
TEXAS	5	PAR	Lamar, Etc.					
CONT.		SECT.	JOE	3	HIGHWAY NO.			
0901	L	29	Ø92,	Etc.		CR,	Etc	

I. STORMWATER POLLUTION PR	REVENTION-CLEAN WATER AG	CT SECTION 402	II. CULTURAL RESOURCES		VI. HAZARDOUS MA
required for projects with 1 or m disturbed soilmust protect for er Item 506.	Discharge Permit or Construction C nore acres disturbed soil. Projects osion and sedimentation in accorda eceive discharges from this proje	with any ance with	Refer to TxDOT Standard Specifical archeological artifacts are found dur archeological artifacts (bones, burnt work in the immediate area and cor	ring construction. Upon discovery of rock, flint, pottery, etc.) cease	General (applies Comply with the Hazar hazardous materials by making workers aware provided with personal
They may need to be notified pr	rior to construction activities.		No Action Required	Required Action	Obtain and keep on-sil used on the project, w
1,			Action No.		Points, ocids, solvents,
2.			ACTION NO.		compounds or additives products which may be
No Action Required	Required Action		1.		Maintain an adequate s In the event of a spill,
Action No.			2.		in accordance with saf
1. Prevent stormwater pollution by accordance with TPDES Perm	y controlling erosion and sedimenta nit TXR 150000	ation in	3.		immediately. The Contr of all product spills.
2 Comply with the SW3P and re	evise when necessary to controlpo	lution or	4.		Contact the Engineer in • Dead or distress
required by the Engineer.			IV. VEGETATION RESOURCES		 Trosh piles, drun Undesirable sme
	(CSN) with SW3P information on or ublic and TCEQ, EPA or other inspe		Preserve native vegetation to the e	-	 Evidence of lead Does the project
4. When Controctor project speci	ific locations (PSL's) increase distu Jomit NOI to TCEQ and the Engineer	rbed soil	164, 192, 193, 506, 730, 751, 752 in	ction Specification Requirements Specs 162, order to comply with requirements for 1g, and tree/brush removalcommitments.	replocements (brid
II. WORK IN OR NEAR STREAM		ANDS CLEAN WATER	🛛 No Action Required	Required Action	If "No", then no fr If "Yes", then TxD(
ACT SECTIONS 401 AND	ng, dredging, excovating or other wa	al is sou	Action No.		Are the results of Yes
woter bodies, rivers, creeks, str			1.		If "Yes", then TxD
The Contractor must adhere to the following permit(s):	o all of the terms and conditions as	ssociated with			the notification, de octivities as neces
the ronowing permittar.			2.		15 working doys p
No Permit Required			3.		If "No", then TxDO
	not Required (less than 1/10th ac	re waters or	4.		scheduled demolitic In either cose, the octivities and/or d
Notionwide Permit 14 - PCN	Required (1/10 to <1/2 ocre, 1/3	in tidal waters)			asbestos consultar
Individual 404 Permit Require	ed			THREATENED, ENDANGERED SPECIES,	Any other evidence
Other Nationwide Permit Req	quired: NWP•		CRITICAL HABITAT, STATE LIS AND MIGRATORY BIRDS.	TED SPECIES, CANDIDATE SPECIES	on site. Hazardous
•	the US permit applies to, location actices planned to controlerosion, s		🛛 No Action Required	Required Action	Action No. 1. LEAD INSP THE STEE
1. CR 1137 at Mitcham Branch	(CSJ 0901-28-103) in Southern Hop	okins County	Action No.		THE STEE LOCATION AN EMPLO SHALL BE
2.			1.		REQUIRED PAINT IS FACILITAT
3.			2.		LCP INSPE FOR ADDI
4.			3.		AT 903-7
		and the second se			VII. OTHER ENVIRO
	igh water marks of any areos requi of the US requiring the use of a r dge Layouts.	-	4.		(includes region
Best Management Practices:			If any of the listed species are observe		No Action
Erosion	Sedimentation	Post-Construction TSS	do not disturb species or habitat and co work may not remove active nests from		Action No.
Temporary Vegetation	Sedimentotion	Vegetotive Filter Strips	nesting season of the birds associated are discovered, cease work in the imme		1.
Bionkets/Matting	Rock Berm	Retention/Irrigation Systems	Engineer immediately.		2.
Mulch	Triangular Filter Dike	Exlended Detention Bosin			3.
Sodding	Sond Bog Berm	Constructed Wetlands			1
Interceptor Swale	Strow Bole Dike	Wet Bosin		ABBRE VIATIONS	
Diversion Dike	Brush Berms	Erosion Control Compost	BMP: Best Management Proctice CCP: Construction General Permit	SPCC: Spill Prevention Control and Counterneosure SWBP: Storm Water Pollution Prevention Plan	
Erosion Control Compost	Erosion Control Compost	Mulch Filler Berm and Socks	DSHS: Texos Deportment of State Health Ser FHWA: Federal Highway Administration	vices PON: Pre-Construction Notification PSL: Project Specific Location	
Mulch Filter Berm and Socks	Mulch Filter Berm and Socks	Compost Filter Berm and Socks	MOA: Memor and um of Agreement	TCEC: Texos Commission on Environmental Quality	
Compost Filter Berm and Socks	Compost Filter Berm and Socks	Vegetation Lined Ditches	MOU: Memorandum of Understanding MS4: Municipal Separate Stormwater Sewer		"
	Stone Outlet Sediment Traps	Sond Filter Systems	MBTA; Migrotory Bird Treaty Act NOT: Notice of Termination	TxDOT: Texas Department of Transportation T&E: Threatened and Endangered Species	
	Sediment Bosins	Grossy Swales	NWP: Nationwide Permit NCI: Notice of Intent	USACE: U.S. Army Corps of Engineers USFWS: U.S. Fish and Wildlife Service	

DATE: FILE:

TERIALS OR CONTAMINATION ISSUES to all projects): d Communication Act (the Act) for personnel who will be working with conducting safety meetings prior to beginning construction and of potential hazards in the workplace. Ensure that all workers are protective equipment appropriate for any hazardous materials used. te Material Safety Data Sheets (MSDS) for all hazardous products hich may include, but are not limited to the following categories: osphalt products, chemical additives, fuels and concrete curing s. Provide protected storage, off bare ground and covered, for hozordous. Maintain product labelling as required by the Act. supply of on-site spill response materials, as indicated in the MSDS. take actions to mitigate the spill as indicated in the MSDS, fe work practices, and contact the District Spill Coordinator actor shall be responsible for the proper containment and cleanup any of the following are detected: sed vegetation (not identified as normal) ns, canister, barrels, etc. ells or odors hing or seepage of substances involve any bridge class structure rehabilitation or dge class structures not including box culverts)? No No urther action is required. OT is responsible for completing asbestas assessment/inspection.

the osbestos inspection positive (is osbestos present)?

DOT must retain a DSHS licensed asbestos consultant to assist with evelop abatement/miligation procedures, and perform management asary. The notification form to DSHS must be postmarked at least prior to scheduled demolition.

)T is still required to notify DSHS 15 working days prior to any on.

Contractor is responsible for providing the date(s) for abotement lemolition with careful coordination between the Engineer and nt in order to minimize construction delays and subsequent claims.

indicating possible hazardous materials or contamination discovered Materials or Contamination Issues Specific to this Project:

Required

Required Action

SPECTION REPORTS FOR THE MORRISON CREEK BRIDGE INDICATION THAT PAINT ON EL STRUCTURES CONTAINS LEAD. ANY COATINGS, PAINT, OR OTHER ITEMS AT THIS N SHALL BE TREATED AS LEAD CONTAINING PAINT (LCP). FOR TASKS THAT EXPOSE OYEE TO LEAD ABOVE THE PERMISSIBLE EXPOSURE LIMIT (PEL), THE CONTRACTOR E RESPONSIBLE FOR PROVIDING EXPOSURE ASSESSMENT AND WORKER PROTECTION AS D UNDER OSHA 1926.62 (LEAD IN CONSTRUCTION). WHEN STRIPPING BACK OF LEAD PERFORMED AS A PROTECTIVE MEASURE, STRIP BACK SUFFICIENT LCP TO TE THE PROJECT WORK. PECTION REPORTS ARE AVAILABLE FOR REVIEW AT THE PARIS DISTRICT OFFICE. DITIONAL INFORMATION CONTACT TXDOT'S DISTRICT ENVIRONMENTAL COORDINATOR 737-9300.

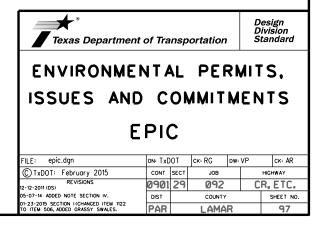
ONMENTAL ISSUES

nalissues such as Edwards Aquifer District, etc.)

Required

Required Action

0901-29-092 CR 26320 AT MORRISON CREEK



	. STORMWATER POLLUTION F			111.	CULTURAL RESOURCES			VI. HAZARDOU
		r Discharge Permit or Constr			Pofor to Typot Stordard Correl	floations '	n the event historical issues or	General (a
	· · ·	1 or more acres disturbed so	• •		-		construction. Upon discovery of	Comply with the
	Item 506.	for erosion and sedimentati	fon in accordance with		-	-	ck, flint, pottery, etc.) cease	hazardous mater making workers (
		nay receive discharges from	this project.		work in the immediate area and			provided with pe
		ed prior to construction act			🗙 No Action Required	🗌 Re	quired Action	Obtain and keep used on the pro
	1.				Action No.			Paints, acids, s compounds or add
	2.							products which r
	No Action Required	Required Action			1.			Maintain an adea In the event of
	Action No.				2.			in accordance wi immediately, The
	 Prevent stormwater pollu accordance with TPDES Pe 	ution by controlling erosion ermit TXR 150000	and sedimentation in		3.			of all product s
6 00000	· •	d revise when necessary to co	ontrol pollution or		4.			Contact the Engi * Dead or di
	required by the Engineer			Ιν.	VEGETATION RESOURCES			* Trash pile * Undesirabl
		Notice (CSN) with SW3P inform the public and TCEQ. EPA or			Preserve native vegetation to			* Evidence o Does the pro
	A When Contractor project	specific locations (PSL's)	increase disturbed soil				pecification Requirements Specs 162, ler to comply with requirements for	replacements
		submit NOI to TCEQ and the					, and tree/brush removal commitments.	If "No". th
	I. WORK IN OR NEAR STREA		ETLANDS CLEAN WATER		No Action Required	C Re	quired Action	If "Yes", the Are the resu
		filling, dredging, excavati	ng or other work in any		Action No.			
		eks, streams, wetlands or we			1.			If "Yes", th the notifica
	the following permit(s):	e to all of the terms and co	nattions associated with		2.			activities a
					3.			15 working d
	No Permit Required							If "No", the scheduled der
	wetlands affected)	PCN not Required (less than	1/10th acre waters or		4.			In either ca activities a
	Nationwide Permit 14 -	PCN Required (1/10 to <1/2 (acre, 1/3 in tidal waters)					asbestos con:
	🔲 Individual 404 Permit R	Required					NED, ENDANGERED SPECIES,	Any other evi on site. Haz
	Other Nationwide Permit	Required: NWP#			CRITICAL HABITAT, STATE AND MIGRATORY BIRDS.	LISTED S	PECIES, CANDIDATE SPECIES	
·	Required Actions: List wate	ers of the US permit applies	to, location in project					
	-	Practices planned to control			🗙 No Action Required	Red	quired Action	Action No. 1. LEAD INSPE THAT PAINI
5	1. FM 2068 AT EAST FORK JE	RNIGAN CREEK (CSJ 1097-05-00	09) IN DELTA COUNTY		Action No.			ITEMS AT 1 THAT EXPOS CONTRACTOR
	2.				1.			PROTECTION BACK OF LE
	3.				2.			TO FACILIT LCP INSPEC FOR ADDITI
	4.				3.			AT 903-737 VII. OTHER EN
		ary high water marks of any	areas requiring work					(includes
		ers of the US requiring the			4.			🗙 No Act
	Best Management Practic	ces:			-		cease work in the immediate area, ct the Engineer immediately. The	Action No.
	Erosion	Sedimentation	Post-Construction TSS	wor	k may not remove active nests	from bridg	es and other structures during	1.
	🗙 Temporary Vegetation	🗙 Silt Fence	Vegetative Filter Strips		e discovered, cease work in the		the nests. If caves or sinkholes area, and contact the	2.
	Blankets/Matting	🗙 Rock Berm	Retention/Irrigation Systems	Eng	ineer immediately.			3.
8	Mulch	🔲 Triangular Filter Dike	Extended Detention Basin					
	Sodding	☐ Sand Bag Berm	Constructed Wetlands		1 ICT OF	ABBREVIATI	ONS	1
	Interceptor Swale		Wet Basin					
	 Diversion Dike	— Brush Berms	 Erosion Control Compost	CGP: C	Best Management Practice Construction General Permit	SW3P:	Spill Prevention Control and Countermeasure Storm Water Pollution Prevention Plan	
	Erosion Control Compost	Erosion Control Compost	Mulch Filter Berm and Socks		exas Department of State Health Serv ederal Highway Administration	ices PCN: PSL:	Pre-Construction Notification Project Specific Location	
	Mulch Filter Berm and Socks	 ☐ Mulch Filter Berm and Socks	Compost Filter Berm and Socks	MOA: M	kenorandum of Agreement kenorandum of Understanding	TCEQ:	Texas Commission on Environmental Quality Texas Pollutant Discharge Elimination System	
	Compost Filter Berm and Socks	s 🗌 Compost Filter Berm and Sock:	s 🗙 Vegetation Lined Ditches	MS4: M	Aunicipal Separate Stormwater Sewer S	system TPWD:	Texas Parks and Wildlife Department	
			Sand Filter Systems	NOT: N	ligratory Bird Treaty Act lotice of Termination	T&E:	Texas Department of Transportation Threatened and Endangered Species	
		Sediment Basins	🗌 Grassy Swales	INMP: N	lationwide Permit		U.S. Army Corps of Engineers	1

US MATERIALS OR CONTAMINATION ISSUES

(applies to all projects):

The Hazard Communication Act (the Act) for personnel who will be working with erials by conducting safety meetings prior to beginning construction and a aware of potential hazards in the workplace. Ensure that all workers are personal protective equipment appropriate for any hazardous materials used. ep on-site Material Safety Data Sheets (MSDS) for all hazardous products roject, which may include, but are not limited to the following categories: , solvents, asphalt products, chemical additives, fuels and concrete curing additives. Provide protected storage, off bare ground and covered, for h may be hazardous. Maintain product labelling as required by the Act.

dequate supply of on-site spill response materials, as indicated in the MSDS. of a spill, take actions to mitigate the spill as indicated in the MSDS, with safe work practices, and contact the District Spill Coordinator The Contractor shall be responsible for the proper containment and cleanup of spills.

ngineer if any of the following are detected: distressed vegetation (not identified as normal) iles, drums, canister, barrels, etc. able smells or odors

of leaching or seepage of substances

roject involve any bridge class structure rehabilitation or

ts (bridge class structures not including box culverts)?

No No

then no further action is required. then TxDOT is responsible for completing asbestos assessment/inspection.

sults of the asbestos inspection positive (is asbestos present)?

🗙 No

then TxDOT must retain a DSHS licensed asbestos consultant to assist with cation, develop abatement/mitigation procedures, and perform management as necessary. The notification form to DSHS must be postmarked at least days prior to scheduled demolition.

then TxDOT is still required to notify DSHS 15 working days prior to any demolition.

case, the Contractor is responsible for providing the date(s) for abatement and/or demolition with careful coordination between the Engineer and posultant in order to minimize construction delays and subsequent claims.

evidence indicating possible hazardous materials or contamination discovered Nazardous Materials or Contamination Issues Specific to this Project:

Action Required 🛛 🛛 🔀 Required Action

SPECTION REPORTS FOR THE FM 2068 AT EAST FORK JERNIGAN CREEK INDICATE INT ON THE STEEL STRUCTURES CONTAINS LEAD. ANY COATINGS, PAINT, OR OTHER T THIS LOCATION SHALL BE TREATED AS LEAD CONTAINING PAINT (LCP). FOR TASKS POSE AN EMPLOYEE TO LEAD ABOVE THE PERMISSIBLE EXPOSURE LIMIT (PEL), THE TOR SHALL BE RESPONSIBLE FOR PROVIDING EXPOSURE ASSESSMENT AND WORKER ION AS REQUIRED UNDER OSHA 1926.62 (LEAD IN CONSTRUCTION). WHEN STRIPPING LEAD PAINT IS PERFORMED AS A PROTECTIVE MEASURE, STRIP BACK SUFFICIENT LCP LITATE THE PROJECT WORK. PECTION REPORTS ARE AVAILABLE FOR REVIEW AT THE PARIS DISTRICT OFFICE. ITIONAL INFORMATION CONTACT TXDOT'S DISTRICT ENVIRONMENTAL COORDINATOR 737-9300.

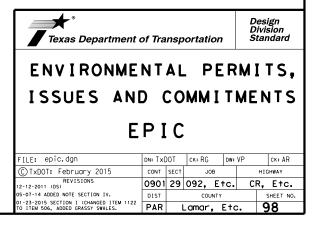
ENVIRONMENTAL ISSUES

es regional issues such as Edwards Aquifer District, etc.)

Action Required

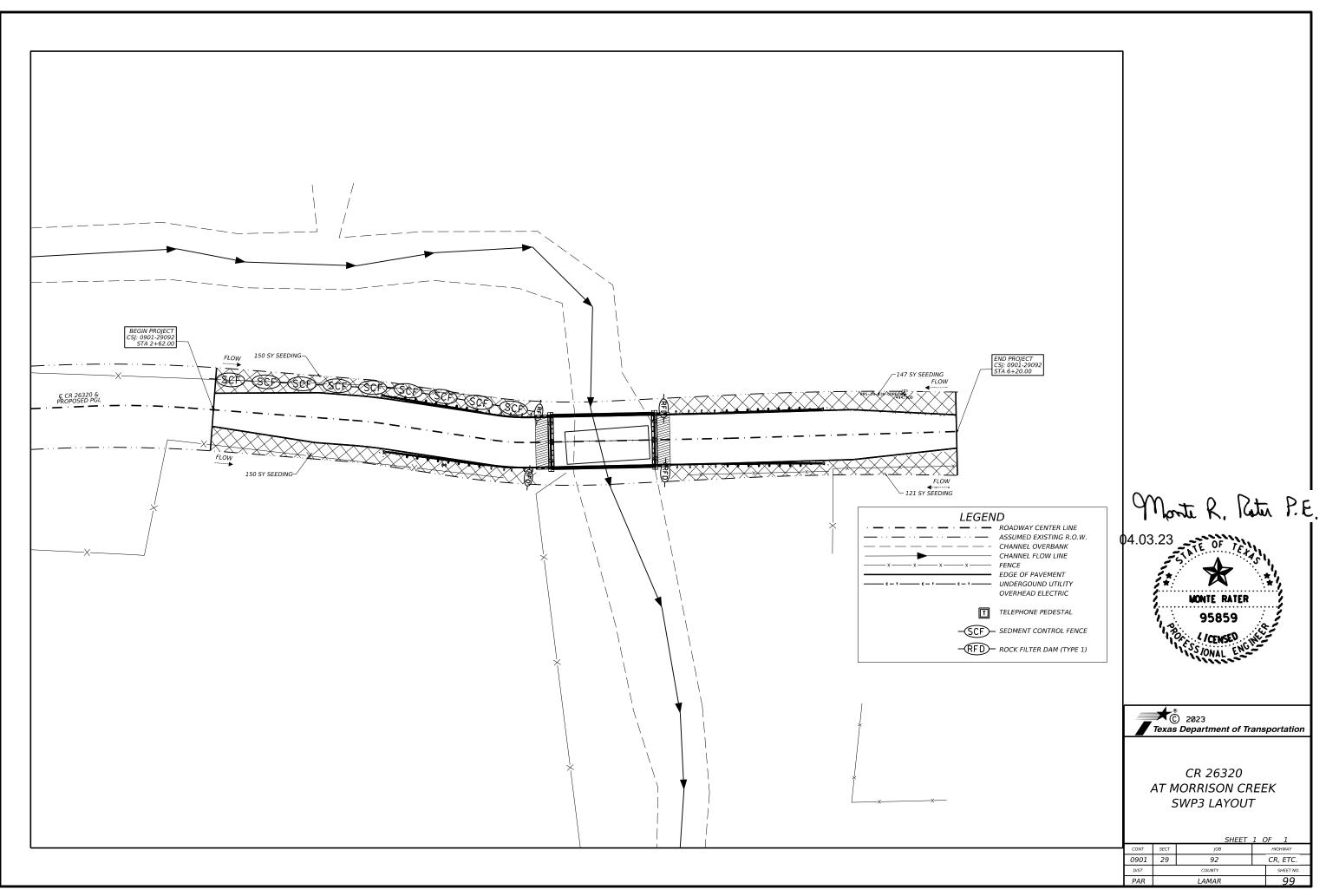
Required Action

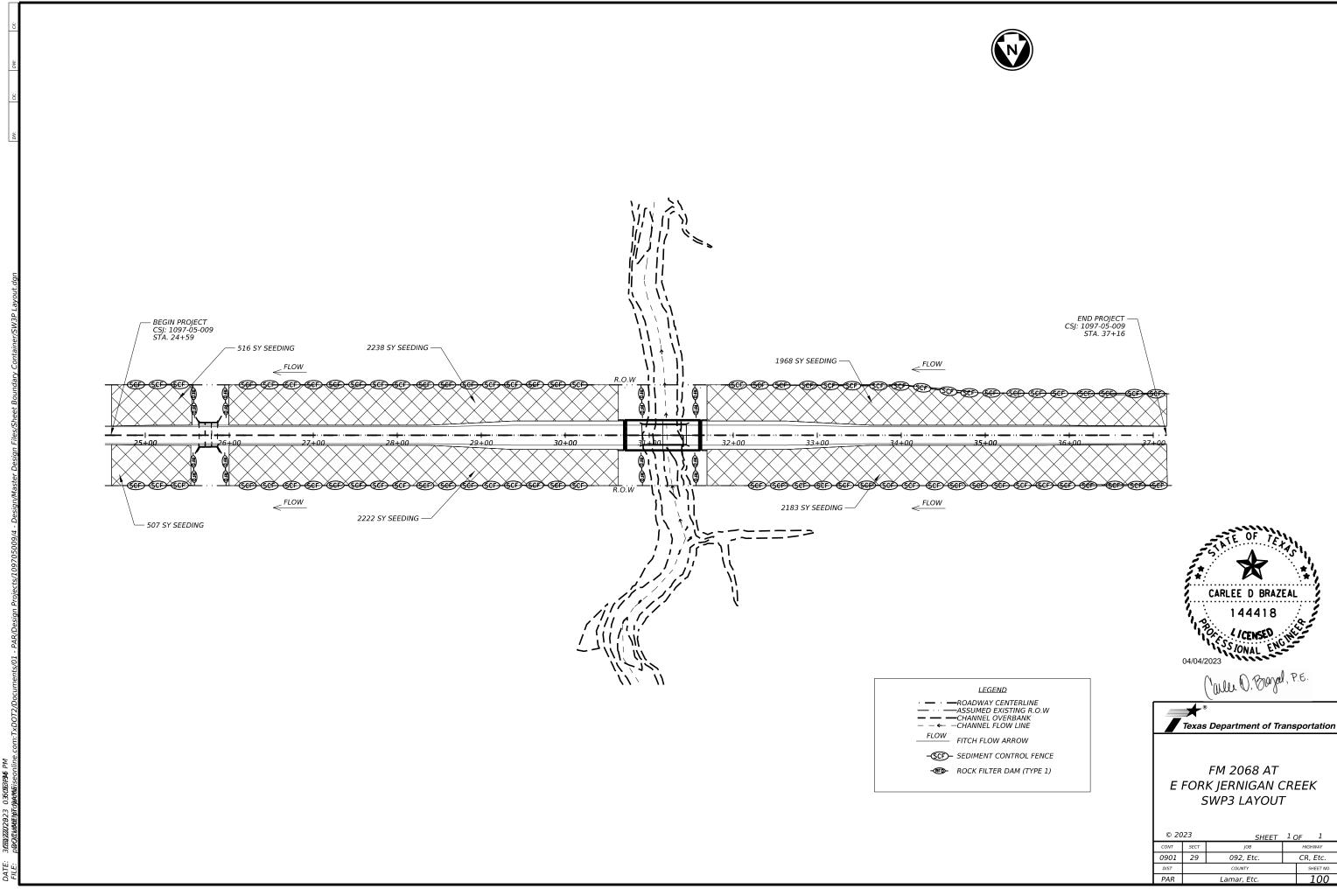
1097-05-009 FM 2068 AT E FORK JERNIGAN CREEK



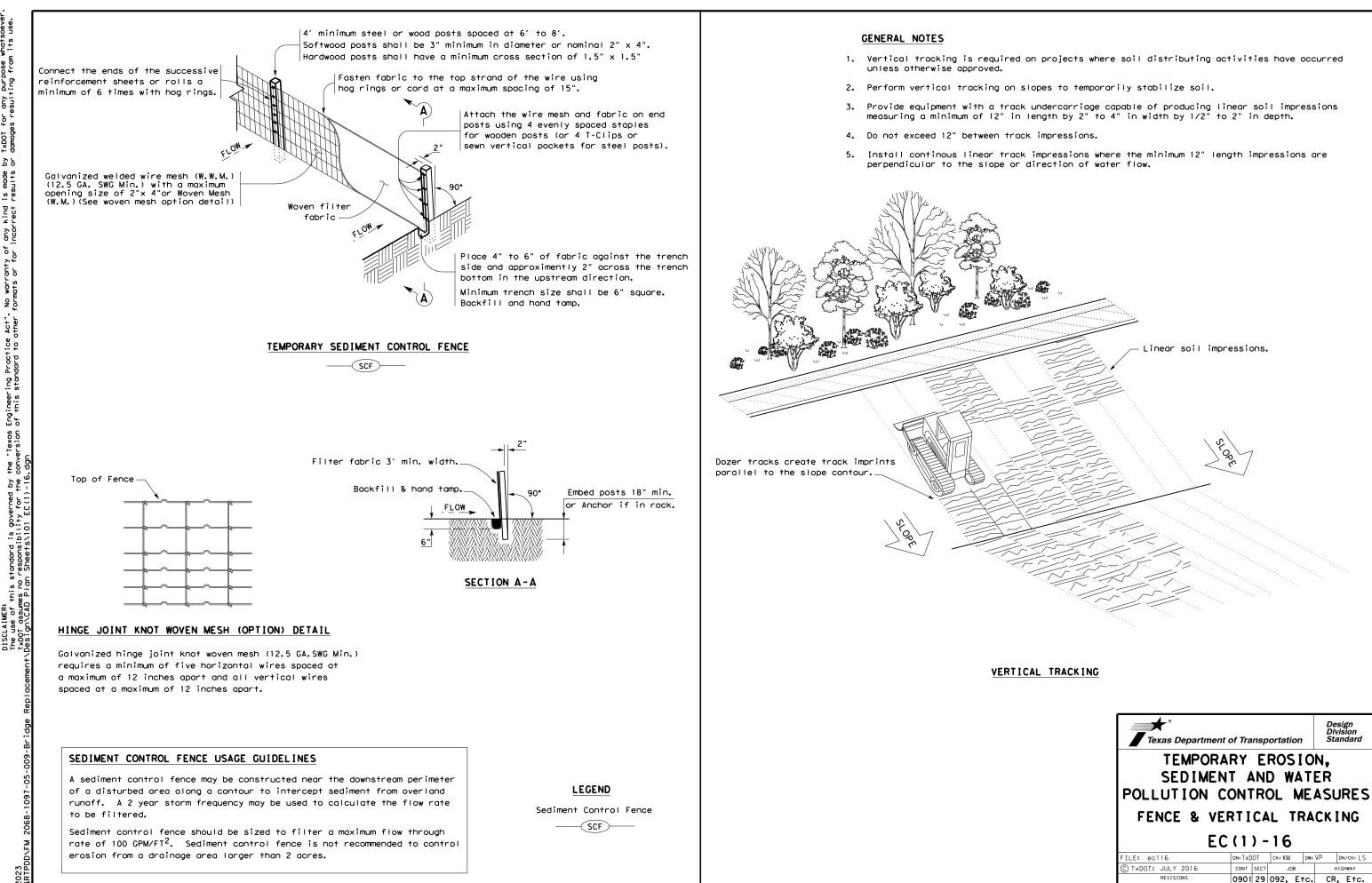




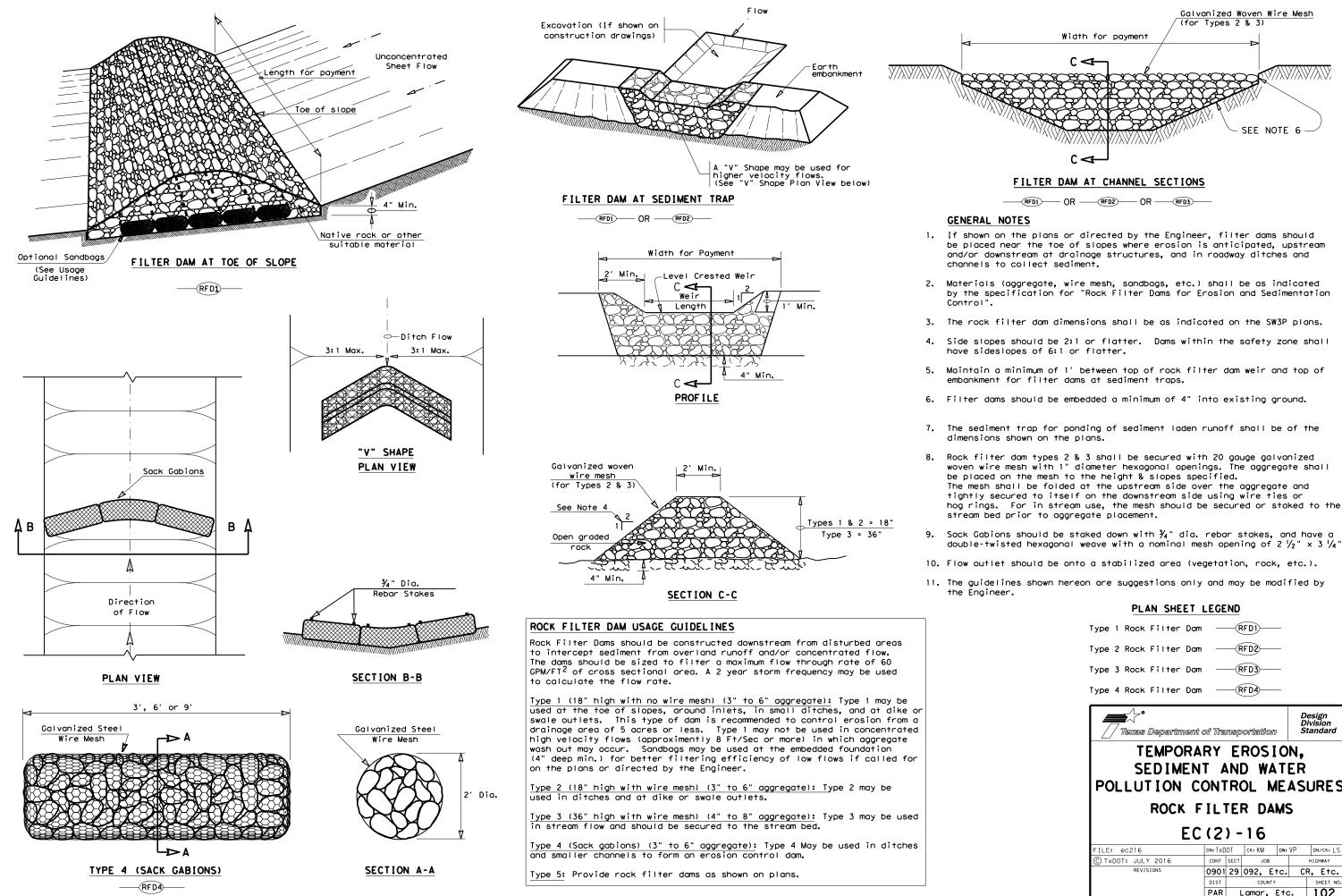




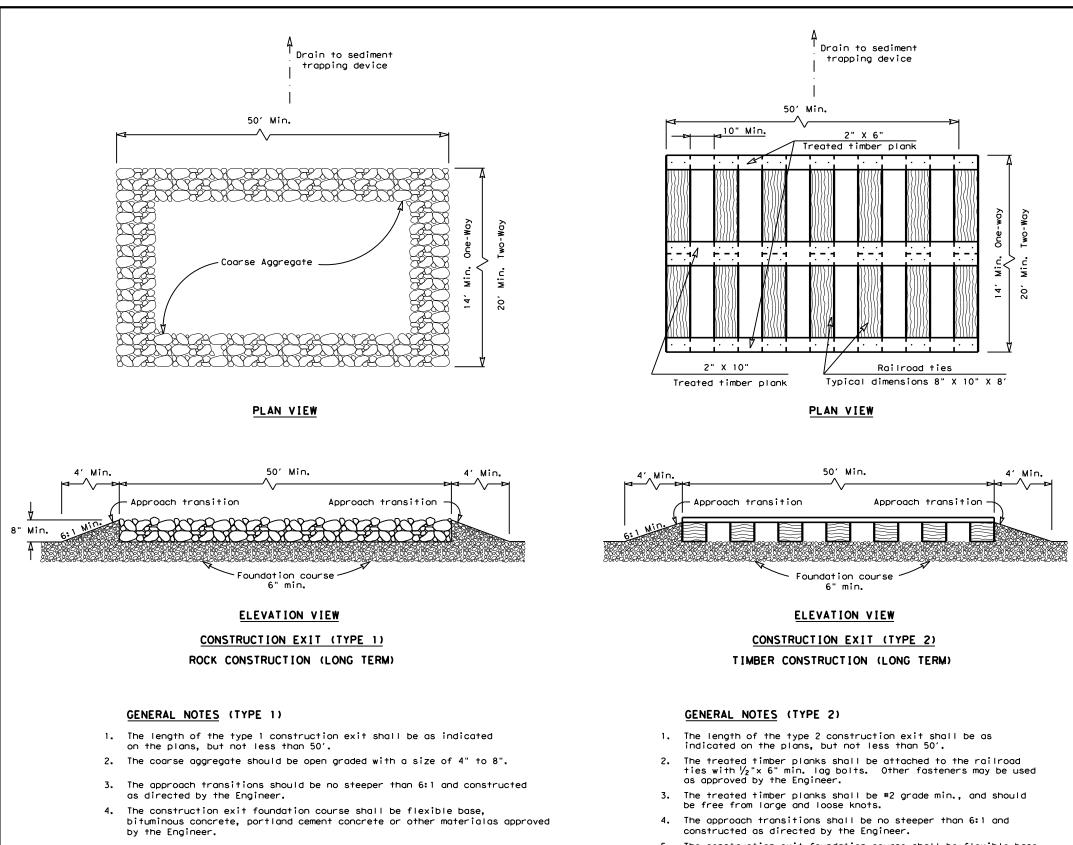
© 20	23	SHEET	SHEET ¹ OF			
CONT	SECT	JOB		YWAY		
0901	29	092, Etc.		Etc.		
DIST		COUNTY	COUNTY SHEET N			
PAR	Lamar, Etc.				100	



*						sign ision	
Texas Departme	ent of Tra	nsp	ortatio	n	Sta	ndard	
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES FENCE & VERTICAL TRACKING							
E E	C (1	/ -	01				
FILE: ec116	dn: Tx[OT	ск:КМ	DW:	VP	DN/CK: LS	
C TxDOT: JULY 2016	CONT	SECT	JOB		н	GHWAY	
REVISIONS	0901	29	092, 1	Etc.	CR,	.	
121151010						Etc.	
NET DIONS	DIST		COUN	TΥ		SHEET NO.	



Type 1 Rock Filter Dam		-RFD1-					
Type 2 Rock Filter Dam		RFD2					
Type 3 Rock Filter Dam		RFD3					
Type 4 Rock Filter Dam		-RFD4-					
 Texas Department of	of Trai	nsportati	ion	Di	esign vision andard		
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES ROCK FILTER DAMS							
EC(2)-16							
FILE: ec216	dn:TxD(DT CK:KM	DW:	VP	DN/CK: LS		
C TxDOT: JULY 2016	CONT	SECT JO	рв	,	HIGHWAY		
REVISIONS	0901	29 092,	Etc.	CR	, E†c.		
	DIST		JNTY		SHEET NO.		
	PAR		. Etc				



- 5. The construction exit shall be graded to allow drainage to a sediment trapping device.
- 6. The guidelines shown hereon are suggestions only and may be modified by the Engineer.
- 7. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.

- 5. The construction exit foundation course shall be flexible base, bituminous concrete, portland cement concrete or other material as approved by the Engineer.
- 6. The construction exit should be graded to allow drainage to a sediment trapping device.
- 7. The guidelines shown hereon are suggestions only and may be modified by the Engineer.
- 8. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.

