FINAL PLANS

NAME OF CONTRACTOR:
DATE OF LETTING:
DATE WORK BEGAN:
DATE WORK COMPLETED:
DATE WORK ACCEPTED:
SUMMARY OF CHANGE ORDERS:

STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

0 ____

PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT

FEDERAL AID PROJECT

BR 2022 (518), ETC

CCSJ: 0918-11-100, ETC.

CR 110, ETC.

KAUFMAN COUNTY

LIMITS: CSJ 0918-11-100: CR 110 AT JONES CREEK CSJ 0918-11-101: CR 279 AT BACHELOR CREEK RELIEF CSJ 0918-11-102: CR 312 AT MUDDY CEDAR CREEK TRIBUTARY

CSJ: 0918-11-100 ROADWAY = 322.00 FT. = 0.061 MI.

RETUGE = 45.00 FT. = 0.009 MI.

CSJ TOTAL = 367.00 FT. = 0.070 MI.

CSJ: 0918-11-101 ROADWAY = 408.00 FT. = 0.077 MI.

RETUGE = 50.00 FT. = 0.010 MI.

CSJ: 0918-11-102 ROADWAY = 458.00 FT. = 0.087 MI.

CSJ: 0918-11-102 ROADWAY = 200.00 FT. = 0.038 MI.

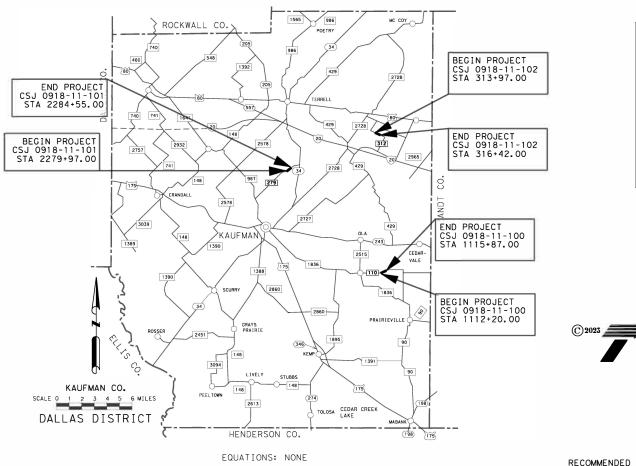
BRIDGE = 45.00 FT. = 0.038 MI.

BRIDGE = 45.00 FT. = 0.009 MI.

TYPE OF WORK: FOR THE CONSTRUCTION OF BRIDGE REPLACEMENT CONSISTING OF: REPLACE BRIDGE AND APPROACHES

CSJ TOTAL = 245.00 FT. = 0.047 MI.

PROJECT TOTAL = 1070.00 FT. = 0.204 MI.



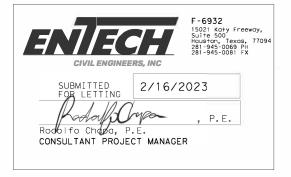
DIV. NO.	FEDER	AL AID PROJECT NO.	NO.
6	BR 20)22 (518),ETC	CR 110,ETC
STATE	DISTRICT	COUNTY	SHEET NO.
TEXAS	DALLAS	KAUFMAN	_
CONTROL	SECTION	JOB	1 1
0918	1 1	100, ETC.	•
	6 STATE TEXAS CONTROL	6 BR 20 STATE DISTRICT TEXAS DALLAS CONTROL SECTION	6 BR 2022 (518), ETC STATE DISTRICT COUNTY TEXAS DALLAS KAUFMAN CONTROL SECTION JOB

FUNCTIONAL CLASS = RURAL LOCAL ROAD
DESIGN SPEEDS = MEETS OR EXCEEDS EXISTING

EXISTING ADT: 30 (2023) - CR 110 25 (2023) - CR 279 30 (2023) - CR 312 FUTURE ADT: 40 (2043) - CR 110 35 (2043) - CR 279 40 (2043) - CR 312

NOTE:

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF TRANSPORTATION, NOVEMBER 1, 2014, AND THE CONTRACT PROVISIONS LISTED AND DATED AS FOLLOWS SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION CONTRACTS (FORM FHWA 1273, JULY 5, 2022)





P.E.

2/23/2023

Lane Selman

-- 29F92BAFCARN⊒A8.ENGINEER

RECOMMENDED 2/24/2023
FORUS JAMES P. Langell , P. E.
DIRECTOR OF TRANSPORTATION
9867 PLANNETHOS & DEVELOPMENT

APPROVED 2/24/2023

CESSON CLEMENS , P. E.
A879EODIDEDBAGAI ENGINEER

WORK WAS COMPLETED ACCORDING TO THE PLANS AND CONTRACT.

signature of Registrant & Date

© 2023 by Texas Department of Transportation; all rights reserved

EXCEPTIONS: NONE
RAILROAD CROSSINGS: NONE

SHEET NO. **DESCRIPTION** I. GENERAL TITLE SHEET 2 INDEX OF SHEETS PROJECT LAYOUT 3 - 5 6 - 8 TYPICAL SECTIONS 9 , 9A-9D GENERAL NOTES ESTIMATE AND QUANTITIES 10 , 10A SUMMARY OF QUANTITIES 11 - 12 13 EARTHWORK QUANTITY SUMMARY 14 SUMMARY OF SMALL SIGNS II. TRAFFIC CONTROL PLAN 15 TRAFFIC CONTROL NARRATIVE 16 TRAFFIC CONTROL PLAN DETOUR PLAN- CR 110 17 TRAFFIC CONTROL PLAN DETOUR PLAN- CR 279 18 TRAFFIC CONTROL PLAN DETOUR PLAN- CR 312 19 - 30 *BC(1)-21 THRU BC(12)-21 *WZ(RCD)-13 31 III. ROADWAY DETAILS 32 - 34 SURVEY CONTROL INDEX 35 - 37 HORIZONTAL AND VERTICAL CONTROL 38 - 40 ALIGNMENT DATA 41 - 44 ROADWAY PLAN & PROFILE 45 - 47 RIPRAP DETAILS 48 *BED-14 49 *GF (31)-19 50 *GF (31)MS-19 51 - 52 *GF (31) TR TL3-20 53 *SGT(10S)31-16 54 *SGT (11S) 31-18 55 *SGT (12S) 31-18 56 *SGT (15) -31-20 57 *TE (HMAC) - 11 58 *WF (1) -10 59 *WF(2)-10 IV. RETAINING WALL DETAILS NONE V. DRAINAGE DETAILS 60 CR 110 DRAINAGE AREA MAP 61 - 64 CR 110 HYDRAULIC DATA 65 CR 110 SCOUR ANALYSIS 66 CR 279 DRAINAGE AREA MAP 67 - 70 CR 279 HYDRAULIC DATA CR 279 SCOUR ANALYSIS 71 72 CR 312 DRAINAGE AREA MAP 73 - 76 CR 312 HYDRAULIC DATA 77 CR 312 SCOUR ANALYSIS VI. UTILITIES NONE VII. BRIDGE DETAILS 78 CR 110 JONES CREEK ESTIMATED QUANTITIES 79 CR 110 JONES CREEK BRIDGE LAYOUT 80 CR 110 JONES CREEK BRIDGE TYPICAL SECTIONS 81 CR 110 JONES CREEK CAP ELEVATIONS 82 - 83 CR 110 JONES CREEK BORING LOGS 84 CR 279 BACHELOR CREEK RELIEF ESTIMATED QUANTITIES 85 CR 279 BACHELOR CREEK RELIEF BRIDGE LAYOUT 86 CR 279 BACHELOR CREEK RELIEF BRIDGE TYPICAL SECTIONS 87 CR 279 BACHELOR CREEK RELIEF CAP ELEVATIONS CR 279 BACHELOR CREEK RELIEF BORING LOGS 88 - 89

CR 312 MUDDY CEDAR CREEK TRIBUTARY ESTIMATED QUANTITIES

91 CR 312 MUDDY CEDAR CREEK TRIBUTARY BRIDGE LAYOUT 92 CR 312 MUDDY CEDAR CREEK TRIBUTARY BRIDGE TYPICAL SECTIONS 93 CR 312 MUDDY CEDAR CREEK TRIBUTARY CAP ELEVATIONS 94 - 95 CR 312 MUDDY CEDAR CREEK TRIBUTARY BORING LOGS 96 **BAS-A 97 98 **APSB-24 99 **APSB-24-15 100 **SPSB-24 101 **SPSB-24-15 102 **PSB-5SB15 **PSBEB 103 104 **PSBRA 105 **PSBSD 106 - 107 **FD 108 - 109 **TYPE T221 110 - 111 *SRR VIII. TRAFFIC ITEMS 112 - 114 SIGNING & PAVEMENT MARKINGS 115 - 117 *D&OM(1)-20 THRU D&OM(3)-20 118 *D&OM(5)-20 119 *SMD (GEN) -08 120 *SMD(SLIP-1)(DAL) 121 - 122 *SMD(SLIP-2)-08 THRU SMD(SLIP-3)-08 123 - 124 *TSR (3)-13 THRU TSR(4)-13 IX. ENVIRONMENTAL ISSUES ENVIORMENTAL PERMITS, ISSUES AND COMMITMENTS (EPIC) (DAL) 125 - 127 128 - 133 STORM WATER POLLUTION PREVENTION PLAN (SWP3) 134 - 136

SW3P LAYOUT *EC(1)-16 *EC(2)-16

139 - 141 *EC(9)-16 *VEGETATION ESTABLISHMENT SHEET (DAL)

X. MISCELLANEOUS ITEMS

NONE

137

138

142

THE STANDARD SHEETS SPECIFICALLY IDENTIFIED WITH "*" HAVE BEEN SELECTED BY ME UNDER OR MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.

RODOLFO CHAPA (No. 105922)

DATE: 3/1/2023

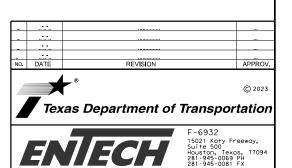


THE STANDARD SHEETS SPECIFICALLY
IDENTIFIED WITH "**" HAVE BEEN SELECTED
BY ME UNDER OR MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.

LLOYD WOLF (No. 73649)

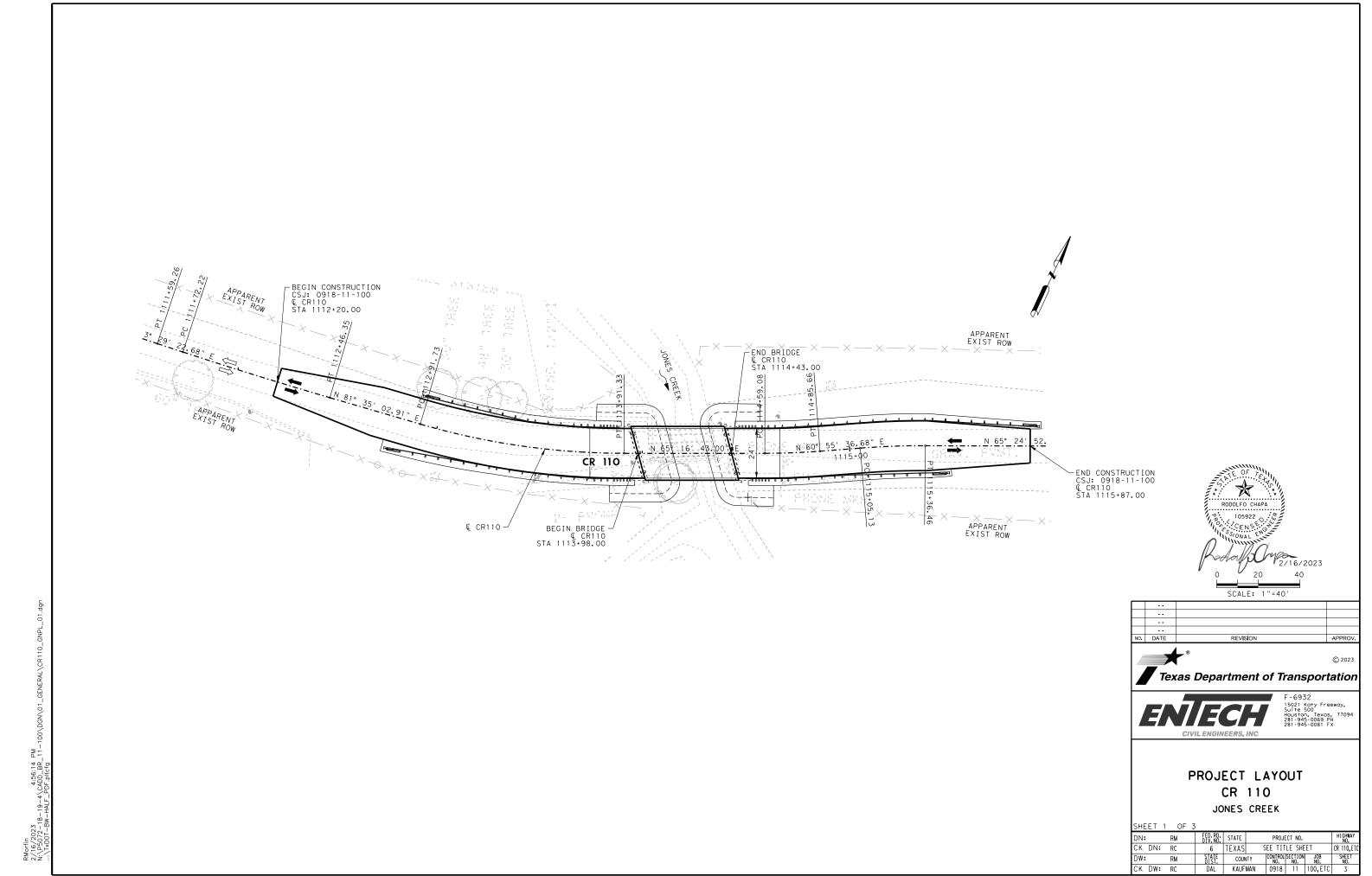
DATE: 3/1/2023



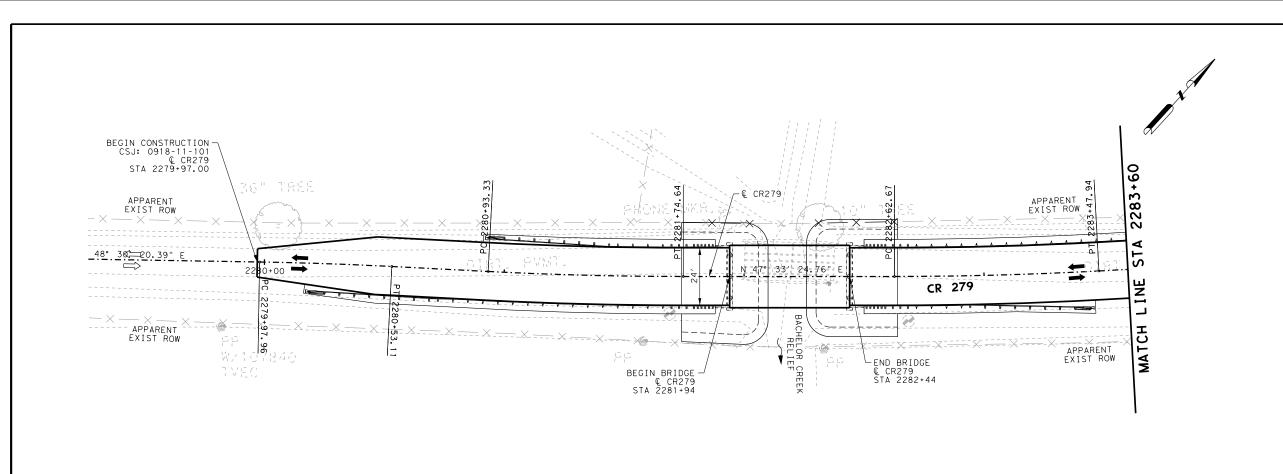


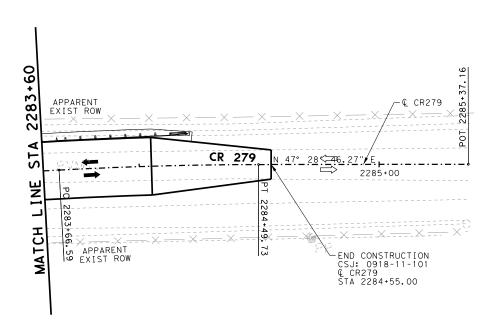
INDEX OF SHEETS

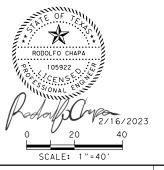
11661 1	OI I							
N:	RM	FED.RD. DIV.NO.	STATE		HIGHWAY NO.			
K DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
W:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
K DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	2



CR110_GNPL_01.dgr







SCALE: 1"=40"

...
...
...
NO. DATE REVISION APPROV



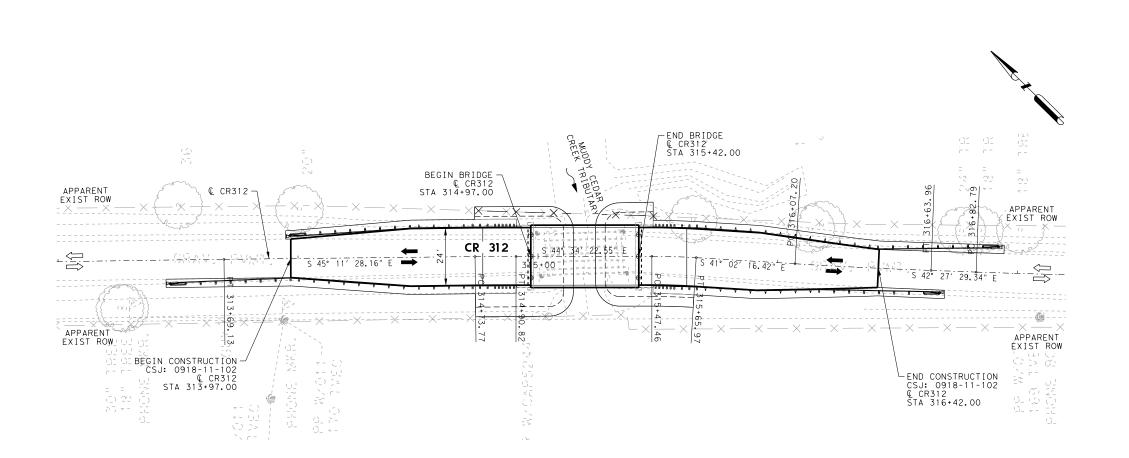


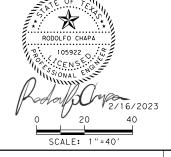
F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

PROJECT LAYOUT
CR 279
BACHELOR CREEK RELIEF

CHEET 2 OF 3

SHEET	2 OF 3	1						
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
OW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100, ETC	4





NO.	DATE	REVISION	APPROV



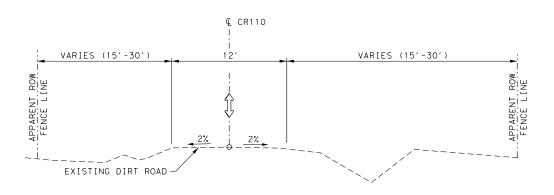


PROJECT LAYOUT CR 312

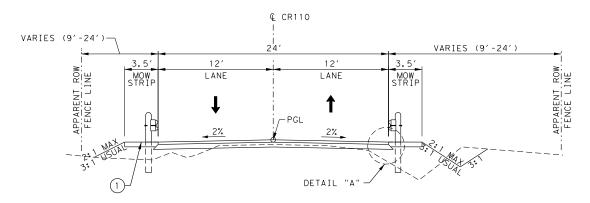
MUDDY CEDAR CREEK TRIBUTARY

SHEET 3 OF

SHEEL 3	OF 3	1					
DN:	RM	FED.RD. DIV.NO.	STATE	PROJ	ECT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	SEE TIT	LE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUN	ITY CONTROL	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN 0918	11	100,ETC	5



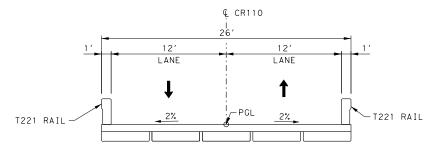
EXISTING TYPICAL SECTION STA 1112+20 TO STA 1113+98 STA 1114+43 TO STA 1115+87



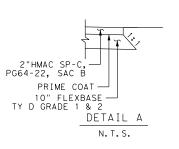
PROPOSED TYPICAL SECTION

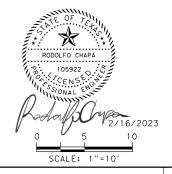
STA 1112+20 TO STA 1113+98
STA 1114+43 TO STA 1115+87

SEE ROADWAY PLAN & PROFILE SHEETS FOR GUARD FENCE AND MOW STRIP LIMITS.



PROPOSED BRIDGE SECTION
STA 1113+98 TO STA 1114+43





NO.	DATE	REVISION	APPROV.					
4		•	© 2023					
4	Texas Department of Transportation							



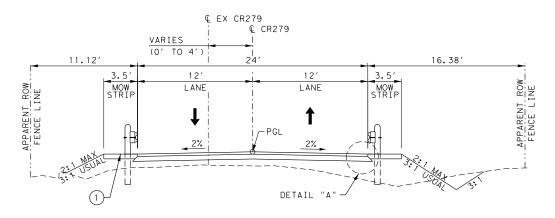
F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

TYPICAL SECTIONS
CR 110

JIILL I	01 3	,						
N:	RM	FED. RD. DIV. NO.	STATE	STATE PROJECT NO.				
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE		CR 110,ETC
OW:	RM	STATE DIST.	COUN	NTY CONTROL SECTION JO		JOB NO.	SHEET NO.	
CK DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	6

EXISTING TYPICAL SECTION

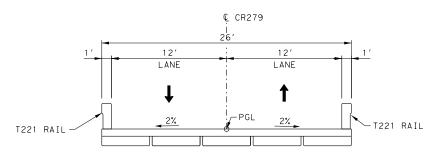
STA 2279+97 TO STA 2281+94
STA 2282+44 TO STA 2284+55



PROPOSED TYPICAL SECTION

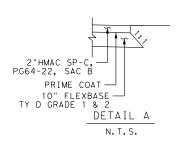
STA 2279+97 TO STA 2281+94
STA 2282+44 TO STA 2284+55

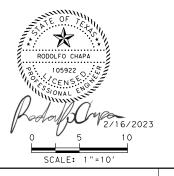
1) SEE ROADWAY PLAN & PROFILE SHEETS FOR GUARD FENCE AND MOW STRIP LIMITS.



PROPOSED BRIDGE SECTION
STA 2281+94 TO STA 2282+44

RMorfin 2/16/2023 N:\P5672-18-19-4\CADD_BR_11-100\DGN\01_GE \`."nor1-BW-HALF_PDF.pitefg





L								
Ν	10.	DATE	REVISION	APPROV.				
	4		*	© 2023				
l.	Texas Department of Transportation							



F-6932 1502! Koty Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 FX

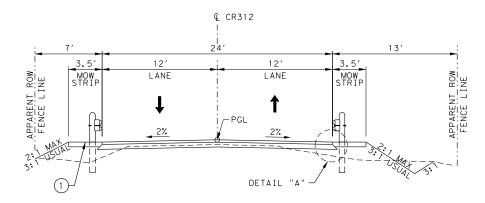
TYPICAL SECTIONS
CR 279

SHEET 2 OF 3

	. 01 3							
N:	RM	FED.RD. DIV.NO.	STATE		HIGHWAY NO.			
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	EΤ	CR 110,ETC
)W:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100, ETC	7

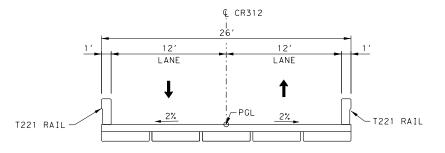
EXISTING TYPICAL SECTION

STA 313+97 TO STA 314+97
STA 315+42 TO STA 316+42

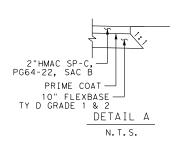


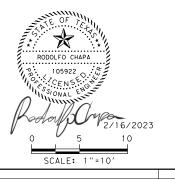
PROPOSED TYPICAL SECTION
STA 313+97 TO STA 314+97
STA 315+42 TO STA 316+42

SEE ROADWAY PLAN & PROFILE SHEETS FOR GUARD FENCE AND MOW STRIP LIMITS.



PROPOSED BRIDGE SECTION
STA 314+97 TO STA 315+42





NO.	DATE	REVISION	APPROV.
	*	*	© 2023
	Тех	as Department of Transpor	tation



F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

TYPICAL SECTIONS
CR 312

SHEET 3 OF 3

JIILLI J	01 3							
N:	RM	FED.RD. DIV.NO.	STATE	STATE PROJECT NO.				
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	EΤ	CR 110,ETC
OW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100, ETC	8

County: Kaufman County

Highway: CR 110, etc.

SPECIFICATION DATA

Table 1: Soil Constants Requirements						
Itom	Description	Plasti	city Index	Note		
Item	Description	Max	Min	Note		
132	Embankment (Final) (DC) (Ty C1)	40	8	1		
132	Embankment (Final) (DC) (Ty C2)	25	8	2		

Note 1: Material excavated from the project must meet the PI requirements when used in the top 10 feet of embankment that supports the pavement structure or other locations shown in the plans. Do not use shale and obtain approval to incorporate shaley clay produced by the construction project.

Note 2: Use as a non-select embankment backfill as defined under Item 423.2.4.1. Use as an embankment to backfill behind abutments to the extent of the approach slab or to backfill areas enclosed by an abutment and / or retaining walls or other locations as shown in the plans.

	Table 2: Basis of Estimate for Permanent Construction							
Item Description Thickness Rate Quantity								
162	Block Sod	N/A	See S	pecifications	2849 SY			
166 *	Fertilizer (12-6-6)	N/A	500	Lbs./Ac	0.15 Ton			
168	Vegetative Watering (Warm)**	N/A	12	MG/Ac/Day	424 MG			
310	Prime Coat	N/A	0.20	Gal/SY	413 Gal			
3077	SP MIXES	See Plans	110	Lbs./SY/In	228 Ton			

^{*}For Contractor's information only.

Note:

- (1) Base material weight based on 1.50 Ton/CY (dry-compacted)
- (2) Asphalt weight based on 110 Lbs./SY/In
- (3) Subgrade weight based on 1.48 Ton/CY (dry-compacted)

Table 3: Basis of Estimate for Temporary Erosion Control Items						
Item	Item Description Rate Quantity					
164	Drill Seeding (Temp) (Warm or Cool)	See Specifications		2849 SY		
166 *	Fertilizer (12-6-6)	500	Lb/Ac	0.16 Ton		
168	Vegetative Watering (Warm)**	12	MG/Ac/Day	425 MG		

^{*}For Contractor's information only.

CSJ: 0918-11-100, etc. Sheet 9

County: Kaufman County

Highway: CR 110, etc.

GENERAL

The construction, operation and maintenance of the proposed project will be consistent with the state implementation plan as prepared by the Texas Commission on Environmental Quality.

The disturbed area for this project, as shown on the plans is 0.51 acres for CR 110, 0.49 acres for CR 279 and 0.30 acres for CR 312. However, the Total Disturbed Area (TDA) will establish the required authorization for storm water discharges. The TDA of this project will be determined by the sum of the disturbed area in all project locations in the contract, and all disturbed area on all Project-Specific Locations (PSL) located in the project limits and/or within 1 mile of the project limits. The department will obtain an authorization to discharge storm water from the Texas Commission on Environmental Quality (TCEQ) for the construction site as shown on the plans, according to the TDA of the project. The Contractor will obtain any required authorization from the TCEQ for the discharge of storm water from any PSL for construction support activities on or off the project row according to the TDA of the project. When the TDA for the project exceeds 1 acre, provide a copy of the appropriate application of permit (NOI, or Construction Site Notice) to the Engineer, for any PSL located in the project limits or within 1 mile of the project limits. Follow the directives and adhere to all requirements set forth in the TCEQ, Texas Pollution Discharge Elimination System, Construction General Permit (TPDES, CGP).

This project required consultation and permitting with environmental resource agencies as outlined in the plan set Environmental Permits Issues and Commitments (EPIC) Sheet. There is a high probability that an environmentally sensitive area could be encountered on the Contractor designated Project-Specific Locations (PSL) for this project (haul roads, equipment staging areas, borrow pits, disposal sites, field offices, storage areas, parking areas, etc.). Item 7.6 "Project-Specific Locations" provides a listing of regulatory agencies that may need to be contacted regarding this project.

Leave all right of way areas undisturbed until actual construction is to be performed in said areas.

Questions may be submitted via the Letting Pre-Bid Q&A web page. This webpage can be accessed from the Notice to Contractors dashboard located at the following Address: https://tableau.txdot.gov/views/ProjectInformationDashboard/NoticetoContractors or Contractor questions on this project are to be addressed to the following individual(s):

Lane Selman, P.E. <u>Lane.Selman@txdot.gov</u>
Nicholas Wadlington, P.E. <u>Nicholas.Wadlington@txdot.gov</u>

Contractor questions will be accepted through email, phone, and in person by the above individuals.

All Contractor questions will be reviewed by the Engineer. All questions and any corresponding responses that are generated will be posted through the same Letting Pre-Bid Q&A web page.

General Notes Sheet A General Notes Sheet B

^{**}Use summer rate for calculation, adjust for actual field conditions/temperatures as necessary.
See Vegetation Establishment Plan Sheet for estimated daily rates.

^{**}Use summer rate for calculation, adjust for actual field conditions/temperatures as necessary. See Vegetation Establishment Sheet for estimated daily rates.

County: Kaufman County

Highway: CR 110, etc.

The Letting Pre-Bid Q&A web page for each project can be accessed by using the dashboard to navigate to the project you are interested in by scrolling or filtering the dashboard using the controls on the left. Hover over the blue hyperlink for the project you want to view the Q&A for and click on the link in the window that pops up.

Paper copies of cross-sections may be produced by using the provided .PDF file located on the above FTP Website at the bidders' expense and at copying companies. This data is for non-construction purposes only and it is the responsibility of the prospective bidder to validate the enclosed data with appropriate plans, specifications and estimate for the project(s).

Item 5:

Underground utilities owned by the Texas Department of Transportation may be present within the Right-Of-Way on this project. For signal, illumination, surveillance, and communications & control maintained by TxDOT, call the TxDOT Traffic Signal Office (214-320-6682) for locates a minimum of 48 hours in advance of excavation. For irrigation systems, call TxDOT Landscape Office (214-320-6205) for locates a minimum of 48 hours in advance of excavation. If city or town owned irrigation facilities are present, call the appropriate department of the local city or town a minimum of 48 hours in advance of excavation. The Contractor is liable for all damages when utilities are damaged due to Contractor's negligence including, but not limited to, repair or replacement at the Contractor's expense.

For the project to be deemed complete, permanently stabilize all unpaved disturbed areas of the project with a vegetative cover at a minimum of 70% density for the control of erosion.

Place construction stakes/station markings at intervals of no more than 100 feet or as directed by the Engineer. Place stakes and markings so as not to interfere with normal construction operations.

Submit all shop drawings, working drawings, or other documents which require review sufficiently in advance of scheduled construction to allow no less than thirty (30) calendar days for review and response.

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/inside-txdot/forms-publications/consultants-contractors/publications/bridge.html#design. Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

<u>Item 6:</u>

This project has a structure along CR 110 over Jones Creek with surface coatings which contains a hazardous constituent which is silver/black lead-based paint (LBP) on the steel beams at 700 ppm. The Contractor is responsible for the health and safety of his employees and compliance with all OSHA standards and regulations.

Paint containing hazardous materials will be removed by a third party, 10.1.1

CSJ: 0918-11-100, etc. Sheet 9A

County: Kaufman County

Highway: CR 110, etc.

To comply with the latest provisions of Build America, Buy America Act (BABA Act) of the Bipartisan Infrastructure Law, the Contractor must submit a notarized original of the TxDOT Construction Material Buy America Certification Form for all items classified as construction materials. This form is not required for materials classified as a manufactured product.

Refer to the Buy America Material Classification Sheet for clarification on material categorization.

The Buy America Material Classification Sheet is found online at https://www.txdot.gov/business/resources/materials/buy-america-material-classification-sheet.html for clarification on material categorization.

<u>Item 7:</u>

Repair or replace any structures and utilities that might have been damaged by negligence or a failure to have utility locates performed.

Holiday restrictions – The Engineer may decide that no lane closures or construction operations shall be allowed during the restricted periods listed in the following holiday schedule. TxDOT has the right to lengthen, shorten, or otherwise modify these restricted periods as actual, or expected, traffic conditions may warrant. Working days will not be charged for these restricted periods. No additional compensation will be allowed for these closures (i.e., overhead, delays, stand-by, barricades or any other associated cost impacts).

- New Year's Eve and Day (5 am on December 31 thru 10:00 pm January 1)
- Easter Holiday weekend (5 am on Friday thru 10:00 pm Sunday)
- Memorial Day weekend (5 am on Friday thru 10:00pm Monday)
- Independence Day (5 am on July 3 thru 10:00 pm on July 5)
- Labor Day weekend (5 am on Friday thru 10:00 pm Monday)
- Thanksgiving Holiday (5 am on Wednesday thru 10:00 pm Sunday)
- Christmas Holiday (5 am on December 23 thru 10:00 pm December 26)

No significant traffic generator events identified.

Item 8

This Project will be a Standard Workweek.

Nighttime work is allowed in accordance with Article 8.3.3.

Meet weekly with the Engineer to notify him or her of planned work for the upcoming week.

Provide the Engineer with a daily work schedule of planned work.

Critical Path Method (CPM) schedule in P6 format will be required for this project. Submit baseline schedule and obtain approval prior to beginning construction. The Estimate will be held if monthly schedule update is not submitted.

<u>Item 100:</u>

Remove the existing roadway small signs, delineators and object markers as shown on the

County: Kaufman County

Highway: CR 110, etc.

plans, or as directed, during construction within the right of way. Small sign, delineator and object marker removals are subsidiary to this Item.

The limits of preparing right of way will be measured at CR 110 at Jones Creek from Sta. 1112+20.00 to Sta. 1115+87.00 along the centerline of construction, at CR 279 at Bachelor Creek Relief from Sta. 2279+97.00 to Sta. 2284+55.00 along the centerline of construction, and at CR 312 at Muddy Cedar Creek Tributary from Sta. 313+97.00 to Sta. 316+42.00 along the centerline of construction.

Item 110:

Excavated shale is not an acceptable material for embankment.

<u>Items 110 and 132:</u>

Scarify and loosen the excavated areas, unpaved surface areas, except rock, to a depth of at least 8 inches and compact in accordance with the specifications.

Item 132:

Excavated material from the project site has not been determined to be suitable for embankment. The bidder assumes all risk for the use of excavated materials for embankment and is expected to meet all material requirements for embankment regardless of the source.

Perform Tex-106-E (Plasticity Index) by an approved laboratory on excavated soils from sources outside right of way when used in roadway embankment. Provide the test results at no expense to the department. The Engineer will sample, and test soils produced by the construction project for specification requirements or material sources specified in the plans.

Earth embankment Type C1 and C2, are mainly composed of material other than shale. Furnish material that is free from vegetation or other objectionable material and that conforms to the requirements of Table 1 (Sheet A). If necessary, treat material with lime slurry in accordance with Item 260, "Lime Treatment (Road-Mixed)" in order to meet these requirements. Use Tex-121-E, figure 1, page 4 to calculate the amount of lime required. When lime treated subgrade is specified, 3000 PPM is the maximum allowed sulfate content in the top 3 feet when material comes from borrow source. Follow recommendations of 260.4.4 for mixing and mellowing. The Engineer will test material placed or excavated to a depth of one foot below and laterally to one foot outside the proposed treatment limit. Lime treatment of this material will not be paid for directly but will be considered subsidiary to this item.

Do not use shaley clays in embankment unless approved in writing

Use embankment material Type C2 described in Table 1 "Soil Constants Requirements" for embankments behind bridge abutments to the extent of the bridge approach slabs, and other embankments enclosed by an abutment and / or retaining walls.

Item 160

Sequence construction operations to salvage topsoil from one location and spread on areas ready to receive topsoil. Keep stockpiling of topsoil to a minimum.

CSJ: 0918-11-100, etc. Sheet 9B

County: Kaufman County

Highway: CR 110, etc.

Use fertile clay or loam from the project site not more than six inches below natural grade as topsoil.

Item 161:

Provide tickets representing quantity of compost delivered to site.

Item 247

Construct uniform layer thickness of 12 inches, or less with the required density and moisture content. Minimum PI is equal to three (3) for all grades.

tem 301:

Provide liquid antistripping agents unless otherwise directed. Add the minimum dosage determined by the manufacturer or higher dosage determined by design requirement and try subsequent trials at 0.25% increments.

Item 320:

Use a self-propelled wheel mounted MTV capable of receiving mix from the haul trucks, separate from the paver. It shall have a minimum storage capacity of approximately 25 tons. It shall be equipped with a pivoting discharge conveyor and shall completely and thoroughly remix the material prior to placement. The effectiveness of the MTV's remixing ability is subject to the approval of the Engineer. In addition, the paver shall have a surge storage insert with a minimum capacity of 20 tons.

The use of windrow pick-up equipment is allowed except on the first course of roadway material placed over the subgrade.

Item 400:

Structural Excavation is not paid for directly but is considered subsidiary to pertinent Items.

Item 416:

Provide a minimum of one core per bent, regardless of placement method.

<u>Item 420:</u>

Apply an ordinary surface finish to all concrete surfaces within 30 days after form removal.

Form columns to a point a minimum of one foot below the proposed future or existing bottom of channel elevation indicated on the bridge layouts by an acceptable method. This form work is not paid for directly but is considered subsidiary to this item.

BENT NUMBERING:

For bridges with four or more spans, number every third bent (counting the abutments) on the up-station and down-station faces of the outside column(s) at approximately the mid height of the column. For structures with three columns or less per bent, place numbers on column A. Where there are four or more columns per bent, place numbers on both outside columns. Bent numbers shall be as shown on the bridge layout.

Sheet F

County: Kaufman County

Highway: CR 110, etc.

All materials, labor and incidentals associated with placing bent numbers are subsidiary to the various bid items.

For bridges with aesthetic treatments, the numbering will be incorporated into the aesthetics package.

NATIONAL BRIDGE INVENTORY NUMBERS:

Provide National Bridge Inventory (NBI) numbers on all bridge structures and bridge class culverts.

Where beam types allow access to the face of abutment backwall, place NBI numbers on the face of each abutment backwall using 3" block numbers. Locate NBI numbers between the outside beams at opposite corners of the bridge.

Where beam types do not allow access to the face of abutment backwall, place NBI numbers on the face of each abutment cap using 3" block numbers. Locate NBI numbers below the outside beams at opposite corners of the bridge.

Where a bridge begins, ends, or contains a bent common to multiple structures, place NBI numbers on both faces near both ends of the common bent cap. The number placed at each of the four locations will correspond to the NBI number assigned to the bridge immediately above the number. Locate NBI numbers below the outside beam. Place using 3" Block Numbers.

For Bent Numbering and NBI Numbering, furnish materials that conform to the pertinent requirements of the following items:

- Stencil ink, black 11 oz., spray can (lead, CFC, and CFHC free). Black spray will be waterproof, weather resistance and dry instantly on all surfaces, without smearing, smudging, or rippling and
- Die cut stencils or
- Brass stencil, 3 in., numbers and letters, adjustable interlocking stencil, set content 92piece numbers and letters, legend height 3 in., symbol height 3 in. Stencils must be industrial grade and interlocking.

All materials, labor and incidentals associated with placing NBI numbers are subsidiary to the various bid items.

Item 421:

Furnish mix designs to the Engineer in a format compatible to the latest version of the Department's Construction Management System (Site Manager). Mix Design templates will be provided by the Engineer.

Provide High Performance Concrete (HPC) of the class specified for the following bridge components: approach slabs, abutments, bents, columns, slabs, sidewalks, and medians.

CSJ: 0918-11-100, etc. Sheet 9C

County: Kaufman County

Highway: CR 110, etc.

Provide High Performance Concrete (HPC) of the class specified for all railing and permanent concrete traffic barrier placed on bridges or approach slabs. HPC concrete is not required for portions of rail or concrete traffic barrier not located on a bridge.

Provide sulfate resistant concrete for box culverts and all drilled shafts.

Strength evaluation using maturity testing, Tex-426-A, may be used for all concrete elements except drilled shafts and mass concrete pours.

Provide a digital hydraulic compression testing Machine and accessories. The machine shall have a minimum testing range of 2500 pounds force to 250,000 pounds force with a hydraulic switching valve to allow for rapid advancing, hold, controlled advancing and rapid retracting. The machine shall have a load cell to measure compressive forces within the testing range and shall be calibrated and verified in accordance with ASTM latest version. The Machine can meet or exceed the following when approved by the Engineer:

ELE International ACCU-TEK250 Digital Compression Tester including accessories or Forney F-250EX Standard Compression Machine including accessories or TxDOT approved equal.

Supply the Engineer with a list of certified personnel and copies of their current ACI certificates before beginning production and when personnel changes are made. Supply hard copies of calibration reports for testing equipment when required by the Engineer.

Item 425:

Repair "Safety Harness Pole Holes" in beams in accordance with Item 429 prior to placement of the Bridge Slab. This work is considered subsidiary to the various bid items.

Item 440:

Provide reinforcing steel with epoxy coating meeting the requirements of item 440 for the following bridge components: approach slab, slab, sidewalk, median, concrete traffic barrier, and rail.

Epoxy coated reinforcing is not required for portions of rail or concrete traffic barrier not located on a bridge.

Reinforcing for abutments, bents and columns are not required to be epoxy coated.

R-bars (I-beams, U-beams, X-Beams and TX Girders), Z-bars (boxes), and H-bars (Slab beams) are not required to be epoxy coated.

All ties, chairs and other appurtenances used with epoxy coated reinforcing shall be epoxy coated or non-metallic.

Fiber Reinforced Concrete (FRC) can be used as a substitute for Non-Structural Class Reinforced Concrete in Mow-Strip and Rip Rap Items as approved. FRC may also be used for other Non-Structural Class Reinforced Concrete Items as approved.

County: Kaufman County

Highway: CR 110, etc.

Item 442:

Use temperature Zone 1 for CVN testing.

Item 500

Material On Hand (MOH) will not be used in calculating partial payments for Mobilization.

Item 502:

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, which could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

Access will be provided to all business and residences at all times. Where turning radii are limited during phased construction at intersections, provide all weather surfaces such as RAP or base in turning movements to accommodate and to protect the traffic from edge drop-offs. Materials, labor, maintenance and removal for these temporary accesses and radii will not be paid for directly but will be considered subsidiary to the various bid items.

Place barricades and signs in locations that do not obstruct the sight distance of drivers entering the highway from driveways or side streets.

Do not operate or park any equipment/machinery closer than 30 feet from the traveled roadway after sunset unless authorized by the Engineer.

When moving unlicensed equipment on or across any pavement or public highways, protect the pavement from all damage using an acceptable method.

Provide written proposed lane closure information by 1:00 PM on the business day prior to the proposed lane closures. Do not close lanes when this requirement is not met.

Item 506:

Take all practicable precautions to prevent debris from being discharged into the Waters of Texas or a designated wetland. Install Best Management Practices before demolition begins and maintain them during the demolition. Remove any debris or construction material that escapes containment devices and are discharged into the restricted areas before the next rain event or within 24 hours of the discharge.

If temporary construction stream crossings are allowed under a Nationwide Permit, submit in writing for approval the type and location of each temporary stream crossing. Use temporary bridges, timber mats, or other structurally sound and non-eroding material for temporary stream crossings. A temporary culvert crossing will consist of storm sewer pipes and 4- to 8-inch nominal size rock. Temporary stream crossings must not cause more than minimal changes to the hydraulic flow characteristics of the stream, increase flooding, or cause more than minimal degradation of water quality. Remove the temporary stream crossings in their entirety and

CSJ: 0918-11-100, etc. Sheet 9D

County: Kaufman County

Highway: CR 110, etc.

return the affected areas to their pre-existing elevation. All work and materials use for temporary construction stream crossings will not be paid for directly but are subsidiary to pertinent Items.

Concrete Washouts are required per the CGP. The Concrete Washout Area(s) structural controls must consist of temporary berms, temporary shallow pits, and/or temporary storage tanks to prevent contaminated runoff and must be lined as to prevent contamination of underlying soil. Ensure pits properly maintained including removal of concrete as not to allow overflow. The location(s) of washout area will be approved by the Engineer. When washout pits are no longer needed, they will be removed, and area will be restored to original condition. This work, materials and labor will not be measured or paid for directly but will be subsidiary to Item 506, "Temporary Erosion, Sedimentation, and Environmental Controls."

Item 540:

Furnish one type of post throughout the project except as specifically noted in the plans.

Item 585:

Use Surface Test Type A.

Items 644:

Provide two (2) sets of shop drawings for signs. The shop drawings shall conform to the details shown on the plans. The shop drawings shall show the details of the panels, wind beams, stiffeners, joint backing plates, splices, fasteners, brackets, and sign support connections. The shop drawings shall show letter types and sizes, interline spacing and message arrangements. Affix a sign identification decal to the back of all signs and mark out the installation date in accordance with Item 643.

Prior to taking elevations to determine lengths for fabrication of signposts and/or sign support towers, obtain verification of all proposed locations.

All sign mounts shall have a clamp base system for all small roadside sign assemblies.

Item 672:

Black adhesive will be used on asphalt pavements and white adhesive will be used on concrete pavements.

Item 730:

At the discretion of the Engineer, mow non-paved areas within the project prior to placement of permanent vegetation. Mow up to three (3) cycles per growing season.

Item 3077

Use aggregate that meets the Surface Aggregate Classification (SAC) requirement of Class B.

Provide PG binder 64-22 in Type SP-C mixture.



Estimate & Quantity Sheet

CONTROLLING PROJECT ID 0918-11-100

DISTRICT Dallas

COUNTY Kaufman

Report Created On: Feb 27, 2023 2:50:26 PM

HIGHWAY CR 110, CR 279, CR 312

		CONTROL SECTION	ои јов	0918-11-1	.00	0918-11-101 0918-11-102					
	PROJECT		ECT ID	A0006483	34	A0006	4835	A0006	4836		
		С	OUNTY	Kaufmar	n	Kaufr	man	Kaufr	man	TOTAL EST. TOTAL FINAL	
		ніс	HWAY	CR 110		CR 2	279	CR 3	312		IIIVAL
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL	EST.	FINAL	EST.	FINAL		
	100-6002	PREPARING ROW	STA	3.670		4.580		2.450		10.700	
	110-6001	EXCAVATION (ROADWAY)	CY	147.000		168.000		107.000		422.000	
	132-6025	EMBANKMENT (FINAL) (DENS CONT) (TY C1)	CY	248.000		506.000		253.000		1,007.000	
	132-6026	EMBANKMENT (FINAL) (DENS CONT) (TY C2)	CY	66.000		62.000		48.000		176.000	
	161-6017	COMPOST MANUF TOPSOIL (4")	SY	1,308.000		984.000		557.000		2,849.000	
	162-6002	BLOCK SODDING	SY	1,308.000		984.000		557.000		2,849.000	
	164-6051	DRILL SEED (TEMP)(WARM OR COOL)	SY	1,308.000		984.000		557.000		2,849.000	
	168-6001	VEGETATIVE WATERING	MG	390.000		293.000		166.000		849.000	
	247-6304	FL BS (CMP IN PLACE) (TY D GR 1-2)(10")	SY	711.000		947.000		398.000		2,056.000	
	310-6027	PRIME COAT(MC-30 OR AE-P)	GAL	143.000		190.000		80.000		413.000	
	416-6002	DRILL SHAFT (24 IN)	LF	114.000		174.000		132.000		420.000	
	420-6014	CL C CONC (ABUT)(HPC)	CY	19.000		18.400		18.400		55.800	
	422-6002	REINF CONC SLAB (HPC)	SF	1,170.000		1,300.000		1,170.000		3,640.000	
	422-6016	APPROACH SLAB (HPC)	CY	45.000		39.000		39.000		123.000	
	425-6012	PRESTR CONC SLAB BEAM (5SB15)	LF	221.410		247.500		222.500		691.410	
	432-6031	RIPRAP (STONE PROTECTION)(12 IN)	CY	191.000		237.000		209.000		637.000	
	432-6045	RIPRAP (MOW STRIP)(4 IN)	CY	22.000		25.000		21.000		68.000	
	450-6005	RAIL (TY T221)(HPC)	LF	114.000		124.000		114.000		352.000	
	454-6003	ARMOR JOINT	LF	46.000		44.000		44.000		134.000	
	496-6009	REMOV STR (BRIDGE 0 - 99 FT LENGTH)	EA	1.000		1.000		1.000		3.000	
	500-6001	MOBILIZATION	LS	0.330		0.370		0.300		1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	МО	18.000						18.000	
	506-6003	ROCK FILTER DAMS (INSTALL) (TY 3)	LF	16.000		16.000		10.000		42.000	
	506-6011	ROCK FILTER DAMS (REMOVE)	LF	16.000		16.000		10.000		42.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	474.000		842.000		564.000		1,880.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	474.000		842.000		564.000		1,880.000	
	506-6041	BIODEG EROSN CONT LOGS (INSTL) (12")	LF	192.000		158.000		90.000		440.000	
	506-6043	BIODEG EROSN CONT LOGS (REMOVE)	LF	192.000		158.000		90.000		440.000	
	540-6001	MTL W-BEAM GD FEN (TIM POST)	LF	175.000		250.000		225.000		650.000	
	540-6006	MTL BEAM GD FEN TRANS (THRIE-BEAM)	EA	4.000		4.000		4.000		12.000	
	544-6001	GUARDRAIL END TREATMENT (INSTALL)	EA	4.000		4.000		4.000		12.000	
	552-6003	WIRE FENCE (TY C)	LF			114.000		105.000		219.000	
	552-6004	WIRE FENCE (TY D)	LF	12.000						12.000	
	644-6001	IN SM RD SN SUP&AM TY10BWG(1)SA(P)	EA	2.000		2.000		2.000		6.000	
	658-6014	INSTL DEL ASSM (D-SW)SZ (BRF)CTB (BI)	EA	2.000		4.000		2.000		8.000	
	658-6016	INSTL DEL ASSM (D-SW)SZ (BRF)GF1 (BI)	EA	9.000		10.000		12.000		31.000	
	730-6107	FULL - WIDTH MOWING	CYC	1.000	<u></u>					1.000	



DISTRICT	COUNTY	CCSJ	SHEET
Dallas	Kaufman	0918-11-100, etc.	10



Estimate & Quantity Sheet

CONTROLLING PROJECT ID 0918-11-100

DISTRICT Dallas

COUNTY Kaufman

Report Created On: Feb 27, 2023 2:50:26 PM

IIGHWAY	CR 110,	CR 279,	CR 312

	CONTROL SECTION JOB		0918-11-	100	0918-11	1-101	0918-1	1-102			
	PROJECT ID		A000648	834	A00064	4835	A0006	4836			
COUNTY		Kaufma	an	Kaufn	nan	Kaufr	man	TOTAL EST.	TOTAL FINAL		
		ніс	YAWH	CR 11	0	CR 2	79	CR 312			
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL	EST.	FINAL	EST.	FINAL		
	734-6002	LITTER REMOVAL	CYC	1.000						1.000	
	3077-6013	SP MIXESSP-CSAC-B PG64-22	TON	79.000		105.000		44.000		228.000	
	18	SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING)	LS	1.000						1.000	
		EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART)	LS	1.000					·	1.000	



DISTRICT	COUNTY	CCSJ	SHEET
Dallas	Kaufman	0918-11-100, etc.	10A

SUMMARY OF TCP QUA	NTITIES
ITEM	502
DESC. CODE	6001
LOCATION	BARRICADES, SIGNS AND TRAFFIC HANDLING
	МО
CCSJ: 0918-11-100	18
PROJECT TOTALS	18

SUMMARY	OF REMOVAL	QUANTITIES			
ITEM	496	* 644	* 658		
DESC. CODE	6009	6076	6060		
LOCATION	REMOV STR (BRIDGE O - 99 FT LENGTH)	REMOVE SM RD SN SUP&AM	REMOVE DELIN & OBJECT MARKER ASSMS		
	EA	EA	EA		
CSJ: 0918-11-100	1	1 2			
CSJ: 0918-11-101	1	2	4		
CSJ: 0918-11-102	1	2	4		
PROJECT TOTALS	3	6	12		

^{*} FOR CONTRACTOR'S INFORMATION ONLY (ITEM SUBSIDIARY TO ITEM 100)

SUMMARY OF SIGNING QUA	NTITITES
ITEM	644
DESC. CODE	6001
LOCATION	IN SM RD SN SUP&AM TY10BWG (1)SA(P)
	EA
CSJ: 0918-11-100	2
CSJ: 0918-11-101	2
CSJ: 0918-11-102	2
PROJECT TOTALS	6

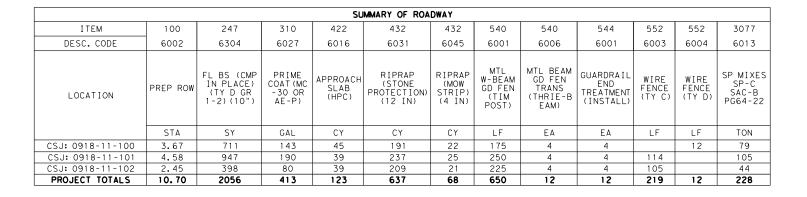
RChapo 2/24/2023 N:\P5072-18-19-4\CADD_BR_11-100\DGN\01_GEI \`T.nonT-RW-HALF_PDF.pitcfg

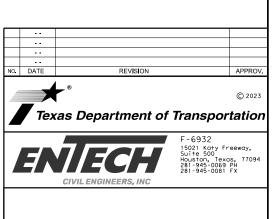
SUMMARY OF PAVEMENT M	ARKING QUAI	NTITIES	
ITEM	658	658	
DESC. CODE	6014	6016	
LOCATION	INSTL DEL ASSM (D-SW)SZ (BRF)CTB (BI)	INSTL DEL ASSM (D-SW) SZ (BRF) GF1 (BI)	
	EA	EA	
CSJ: 0918-11-100	2	9	
CSJ: 0918-11-101	4	10	
CSJ: 0918-11-102	2	12	
PROJECT TOTALS	8	31	

				5	SUMMARY OF S	SW3P QUANTIT	IES						
ITEM	161	162	164	**166	168	506	506	506	506	506	506	730	734
DESC. CODE 6017 6002 6051 6002		6001	6003	6011	6038	6039	6041	6043	6107	6002			
LOCATION	COMPOST MANUF TOPSOIL (4")	BLOCK SODDING	DRILL SEED (TEMP) (WARM OR COOL)	FERTILIZER	VEGETATIVE WATERING	ROCK FILTER DAMS (INSTALL) (TY 3)	ROCK FILTER DAMS (REMOVE)	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDMT CONT FENCE (REMOVE)	BIODEG EROSN CONT LOGS (INSTL) (12")	BIODEG EROSN CONT LOGS (REMOVE)	FULL - WIDTH MOWING	LITTER REMOVAL
	SY	SY	SY	TON	MG	LF	LF	LF	LF	LF	LF	CYC	CYC
CSJ: 0918-11-100	1308	1308	1308	0.14	390	16	16	451	451	183	183	1	1
CSJ: 0918-11-101	984	984	984	0.11	293	16	16	802	802	150	150		
CSJ: 0918-11-102	557	557	557	0.06	166	10	10	537	537	85	85		
SUBTOTAL	2849	2849	2849	0.31	849	42	42	1790	1790	418	418	1	1
5% ADDITIONAL CSJ: 0918-11-100								23	23	9	9		
5% ADDITIONAL CSJ: 0918-11-101								40	40	8	8		
5% ADDITIONAL CSJ: 0918-11-102								27	27	5	5		
PROJECT TOTALS	2849	2849	2849	0.31	849	42	42	1880	1880	440	440	1	1

NOTE: SW3P QUANTITIES ARE BEING INCREASED BY 5% TO ACCOUNT FOR DIFFERING SITE CONDITIONS AND REPLACEMENTS DUE TO NORMAL WEAR DURING CONSTRUCTION.

** FOR CONTRACTOR'S INFORMATION ONLY (ITEM SUBSIDIARY TO ITEMS 162 & 164)





SUMMARY OF QUANTITIES

	01 2								
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		H[GHWAY NO.	
K DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET.	CR 110,ET	٠,
W:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.	
K DW:	RC	DAL	KAUF	MAN	0918	11	100, ETC	11	1

416

6002

ITEM

DESC. CODE

RMorfin 2/16/2023 N:\P5072-18-19-4\CADD_BR_11-100\DGN\01_GENERAL\K/ \TxNOT-BW-HALF_PDF.pitcig

SUMMARY OF BRIDGE QUANTITIES

6002

6012

LF

221.41

247.50

222.50

691.41

6014

450

6005

LF

114

124

114

352

454

6003

LF

46

44

44

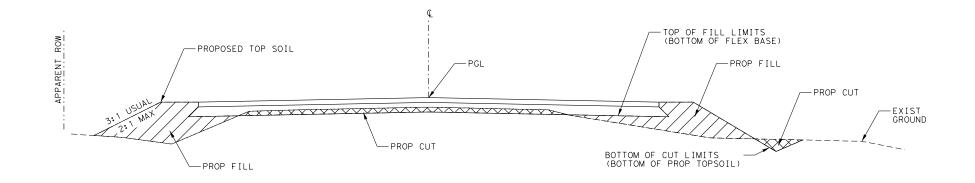
134

	•••		·	
	••			
NO.	DATE	REVISION		APPROV.
4	Тех	as Department of	Transpor	© 2023 tation
	E A	IECH CIVIL ENGINEERS, INC	F-6932 15021 Katy Fr Suite 500 Houston, Texo 281-945-0069 281-945-0081	s, 77094 PH

SUMMARY OF QUANTITIES

SHEET 2 OF 2

SHEET Z	UF Z							
DN:	RM	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ET
DW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	12

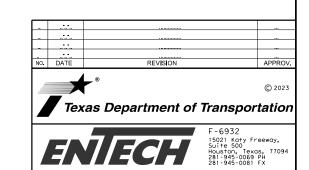


	SUMMARY OF EARTHWORK										
ITEM	110	132	132								
DESC. CODE	6001	6025	6026								
STATION	EXCAVATION (ROADWAY)	EMBANKMENT (FINAL) (DENS CONT) (TY C1)	EMBANKMENT (FINAL) (DENS CONT) (TY C2)								
	CY	CY	CY								
	CR 110										
1112+20	0	0									
1113+00	50	21									
1113+98	36	141	35								
JONES CREEK BRIDGE											
1115+00	14	46	31								
1115+87	47	40									
CSJ 0918-11-100 TOTAL	147	248	66								

N.T.S

	CR 279									
2279+97										
2280+00	2	0								
2281+00	67	30								
2281+94	31	181	31							
BACHELOR CREEK RELIEF 6	BRIDGE									
2283+00	0	155	31							
2284+00	35	124								
2284+55	33	16								
CSJ 0918-11-101 TOTAL	168	506	62							

	CR 312		
312+97			
313+00	1	0	
314+00	37	22	
314+97	22	134	22
MUDDY CEDAR CREEK TRIB	JTARY BRIDGE		
316+00	19	81	26
316+42	28	16	
CSJ 0918-11-102 TOTAL	107	253	48
TOTAL	422	1007	176



EARTHWORK QUANTITY SUMMARY

DN:	RM	FED. RD. DIV. NO.	STATE	PRO	HIGHWAY NO.		
CK DN:	RC	6	TEXAS	SEE TI	TLE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUNT	Y CONTRO	L SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUFM	IAN 0918	11	100, ETC	13

				SUMMARY	OF SI	V A	ALL SIG	N S			
of from its use. NOO NOO NOO NOO NOO NOO NOO NOO NOO NO	ET S	SIGN NO. N	SIGN NOMENCLATURE	SIGN FOR CSJ:0918-11-100	DIMENSIONS	(TYPE	POST TYPE WON FRP = Fiberglass TWT = Thin-Wall 10BWG = 10 BWG	POSTS	ANCHOR TYPE MOUNTING DESIGNATION UA=Universal Conc PREFABRICATED 1EXT or 2EXT = # of Ext UB=Universal Bolt SA=Slipbase-Conc P = "Plain" WC = 1.12 #/ft Wing Channel WS=Wedge Steel U = "U" EXAL= Extruded Alum Sign WP=Wedge Plastic	TY = TYPE	
ssumes no responsible or damages resulting	,	1,2	W8-13aT	BRIDGE MAY TOE IN COLD WEATHER	36"X36"	X	1 OBWG	1	SA P		ALUMINUM SIGN BLANKS THICKNESS Square Feet Minimum Thickness Less than 7.5 0.080" 7.5 to 15 0.100"
by TxDOT for any purpose whatsoever. TxDOT assumes no responsibility dard to other formats or for incorrect results or damages resulting from		1,2	W8-13aT	FOR CSJ: 0918-11-101 BRIDGE MAY ICE IN COLD WEATHER	36"X36"	X	1 OBWG	1	SA P		The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website. http://www.txdot.gov/
Kind is made by TxDOI for any pu KeluilBl&. Helmard to other format		1,2	W8-13aT	FOR CSJ: 0918-11-102 BRIDGE MAYCE IN COLD WEATHER	36"X36"	X	1 OBWG	1	SA P		NOTE: 1. Sign supports shall be located as shown on the plans, except that the Engineer may shift the sign supports, within design guidelines, where necessary to secure a more desirable location or to avoid conflict with utilities. Unless otherwise shown on the plans, the Contractor shall stake and the Engineer will verify all sign support locations.
K ND_PVMT_MRK\S+dDe+dils											For installation of bridge mount clearar signs, see Bridge Mounted Clearance Sign Assembly (BMCS)Standard Sheet. For Sign Support Descriptive Codes, see Sign Mounting Details Small Roadside Signs General Notes & Details SMD(GEN).
11-100\DGN\08_TRAFFIC											Traffice Operation Standard St
4:56:21 PM 18-19-4\CADD_BR											SUMMARY OF SMALL SIGNS
DATE: 2/16/2023 FILE: N:\P5072-1											SOSS

SEQUENCE OF CONSTRUCTION

PHASE 1

- 1. IMPLEMENT SW3P.
- 2. PLACE DETOUR SIGNS AND BARRICADES. REFER TO BARRICADE AND CONSTRUCTION STANDARD DETAILS BC(1)-21 THRU BC (12)-21. PLACE TYPE III BARRICADES IN ADVANCE OF ROADWAY CLOSURE WHILE ALLOWING LOCAL TRAFFIC TO MAINTAIN ACCESS ALONG CR 110 DURING CONSTRUCTION. REFER TO WORK ZONE ROAD CLOSURE STANDARD DETAILS. CLOSE EXISTING BRIDGE AND ROADWAY APPROACHES.
- 3. REMOVE EXISTING BRIDGE AND ROADWAY APPROACHES.
- 4. CONSTRUCT PROPOSED BRIDGE AND ROADWAY APPROACHES. GRADE ROADSIDE DITCHES. INSTALL STONE RIPRAP. INSTALL PERMANENT EROSION CONTROL. INSTALL PERMANENT PAVEMENT MARKINGS.
- 5. REMOVE TEMPORARY SW3P DEVICES ONCE DISTURBED SOILS HAVE REACHED FINAL STABILIZATION IN THEIR CONTROL AREAS, AS DIRECTED OR AUTHORIZED BY ENGINEER.
- 6. REMOVE DETOUR SIGNS AND BARRICADES. OPEN PROPOSED BRIDGE AND ROADWAY APPROACHES.

CSJ: 0918-11-101: CR 279 @ BACHELOR CREEK RELIEF

PHASE 2

- 1. IMPLEMENT SW3P.
- 2. PLACE DETOUR SIGNS AND BARRICADES. REFER TO BARRICADE AND CONSTRUCTION
 STANDARD DETAILS BC(1)-21 THRU BC (12)-21. PLACE TYPE III BARRICADES IN ADVANCE OF ROADWAY
 CLOSURE WHILE ALLOWING LOCAL TRAFFIC TO MAINTAIN ACCESS ALONG CR 279 DURING
 CONSTRUCTION. REFER TO WORK ZONE ROAD CLOSURE STANDARD DETAILS. CLOSE EXISTING
 BRIDGE AND ROADWAY APPROACHES.
- 3. REMOVE EXISTING BRIDGE AND ROADWAY APPROACHES.
- 4. CONSTRUCT PROPOSED BRIDGE AND ROADWAY APPROACHES. GRADE ROADSIDE DITCHES. INSTALL STONE RIPRAP. INSTALL PERMANENT EROSION CONTROL. INSTALL PERMANENT PAVEMENT MARKINGS.
- 5. REMOVE TEMPORARY SW3P DEVICES ONCE DISTURBED SOILS HAVE REACHED FINAL STABILIZATION IN THEIR CONTROL AREAS, AS DIRECTED OR AUTHORIZED BY ENGINEER.
- 6. REMOVE DETOUR SIGNS AND BARRICADES. OPEN PROPOSED BRIDGE AND ROADWAY APPROACHES.

CSJ: 0918-11-102: CR 312 @ MUDDY CEDAR CREEK TRIBUTARY

PHASE 3

- 1. IMPLEMENT SW3P.
- 2. PLACE DETOUR SIGNS AND BARRICADES. REFER TO BARRICADE AND CONSTRUCTION STANDARD DETAILS BC(1)-21 THRU BC (12)-21. PLACE TYPE III BARRICADES IN ADVANCE OF ROADWAY CLOSURE WHILE ALLOWING LOCAL TRAFFIC TO MAINTAIN ACCESS ALONG CR 312 DURING CONSTRUCTION. REFER TO WORK ZONE ROAD CLOSURE STANDARD DETAILS. CLOSE EXISTING BRIDGE AND ROADWAY APPROACHES.
- 3. REMOVE EXISTING BRIDGE AND ROADWAY APPROACHES.
- 4. CONSTRUCT PROPOSED BRIDGE AND ROADWAY APPROACHES. GRADE ROADSIDE DITCHES. INSTALL STONE RIPRAP. INSTALL PERMANENT EROSION CONTROL. INSTALL PERMANENT PAVEMENT MARKINGS.
- 5. REMOVE TEMPORARY SW3P DEVICES ONCE DISTURBED SOILS HAVE REACHED FINAL STABILIZATION IN THEIR CONTROL AREAS, AS DIRECTED OR AUTHORIZED BY ENGINEER.
- 6. REMOVE DETOUR SIGNS AND BARRICADES. OPEN PROPOSED BRIDGE AND ROADWAY APPROACHES.

GENERAL NOTES

1. CONSTRUCT ONE BRIDGE AT A TIME.



SCALE: N.T.S.

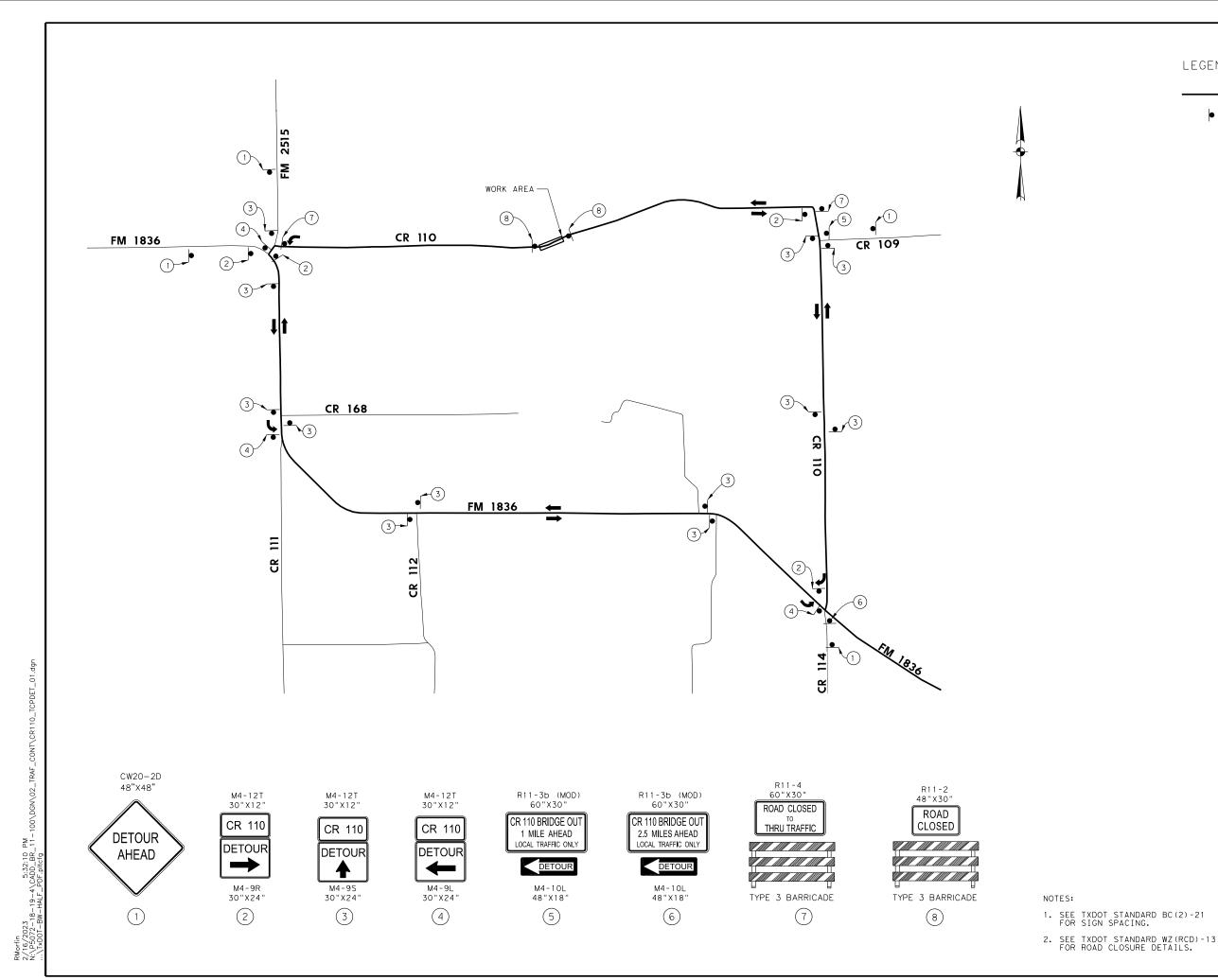


15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0089 PH 281-945-0081 FX

F-6932

TRAFFIC CONTROL NARRRATIVE

SUFF!	OF I							
DN:	RM	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	-11	100, ETC	15



LEGEND:

- DETOUR ROUTE

DETOUR SIGN



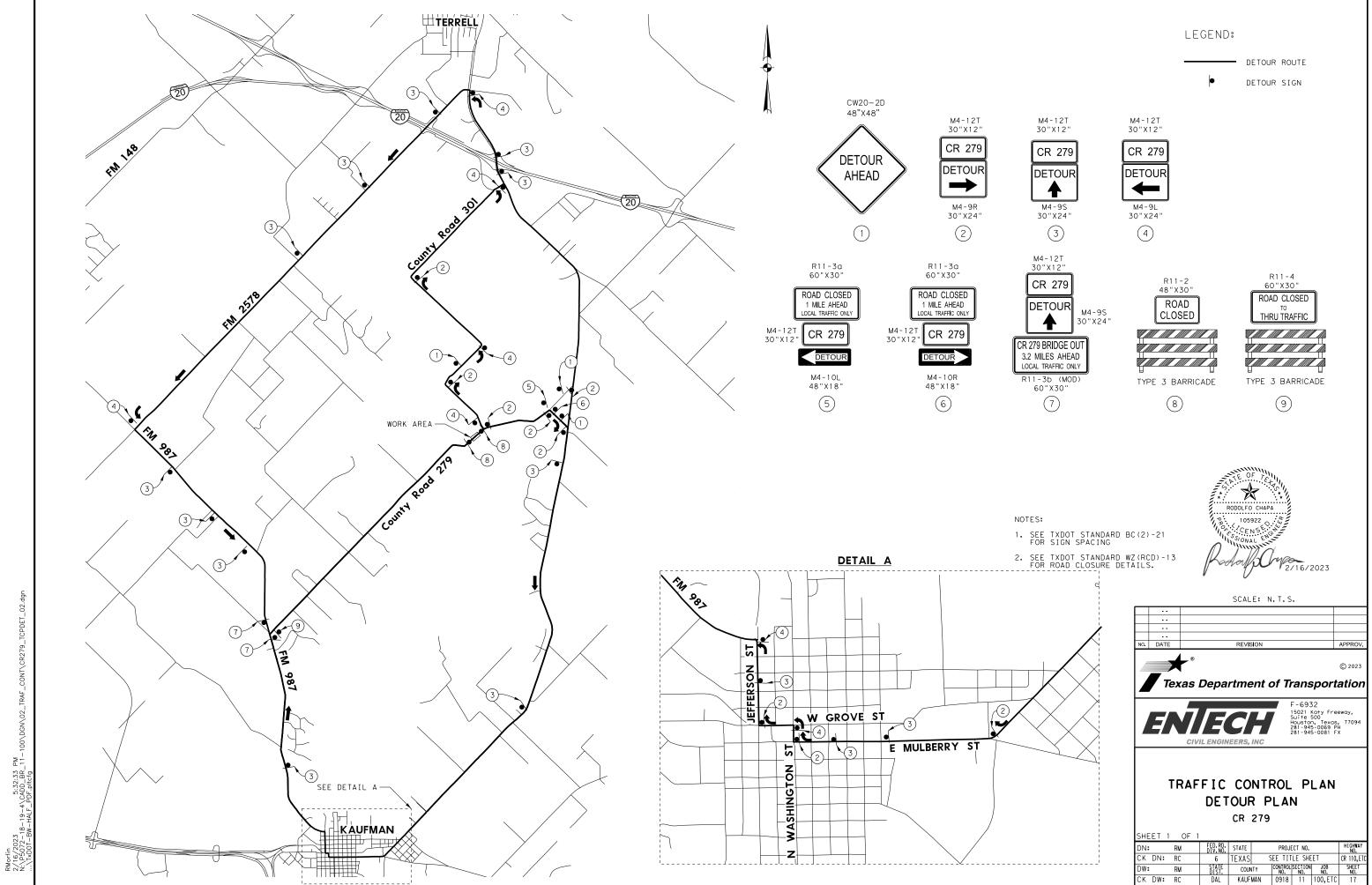
		SCALE: N. I. S.	
NO.	DATE	REVISION	APPROV.
		L ®	





TRAFFIC CONTROL PLAN DETOUR PLAN CR 110

JIILL I	OI I							
DN:	RM	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	16



CP279 TCPDET 02 dan

LEGEND:

DETOUR ROUTE

DETOUR SIGN



SCALE: N.T.S.

		SCALE: N. I. S.						
NO.	DATE	REVISION	APPROV.					





F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

TRAFFIC CONTROL PLAN DETOUR PLAN CR 312

SHEET 1 OF 1

NOTES:

1. SEE TXDOT STANDARD BC(2)-21 FOR SIGN SPACING

2. SEE TXDOT STANDARD WZ(RCD)-13 FOR ROAD CLOSURE DETAILS.

JIILL I	01 1							
DN:	RM	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100, ETC	18

2/16/2023 4:56:46 PM N:\P5072-18-19-4\CADD BR 11-100\DGN\Q2 TRAF CONT\S+dDe+q:18\DG

- 1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. The development and design of the Traffic Control Plan (TCP) is the responsibility of the Engineer.
- 3. The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop, sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- 6. When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- 7. The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- 9. The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES, CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:

- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel," or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

- Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT

http://www.txdot.gov

COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD)

DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS)

MATERIAL PRODUCER LIST (MPL)

ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)"

STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD)

TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD)

TRAFFIC ENGINEERING STANDARD SHEETS

SHEET 1 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION GENERAL NOTES AND REQUIREMENTS

BC(1)-21

FILE:	bc-21.dgn	DN: T	(DOT	ck: TxDOT	DW:	TxDO	T	ck: TxD0
© TxD0T	November 2002	CONT	SECT	JOB			HIG	HWAY
4-03	REVISIONS 7-13	0918	11	100,ET	c.	CR	11	O,ETC.
9-07	8-14	DIST		COUNTY			S	HEET NO.
5-10	5-21	DAL		KAUFMA	٩N			19

ROAD

₽ 6

4:56:46

2

CLOSED R11-2

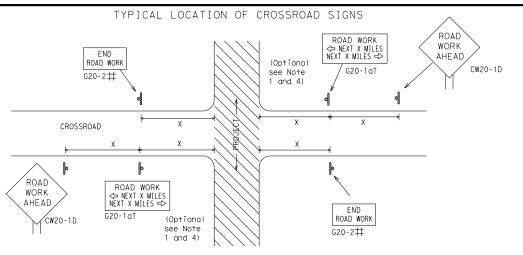
Type 3

devices

B

Barricade or

channelizing



- \sharp May be mounted on back of "ROAD WORK AHEAD" (CW20-1D) sign with approval of Engineer.
- The typical minimum signing on a crossroad approach should be a "ROAD WORK AHEAD" (CW20-1D) sign and a (G20-2) "END ROAD WORK" sign, unless noted otherwise in plans.
- 2. The Engineer may use the reduced size 36" x 36" ROAD WORK AHEAD (CW20-1D) sign mounted back to back with the reduced size 36" x 18" "END ROAD WORK" (G20-2) sign on low volume crossroads (see Note 4 under "Typical Construction Warning Sign Size and Spacing"). See the "Standard Highway Sign Designs for Texas" manual for sign details. The Engineer may omit the advance warning signs on low volume crossroads. The Engineer will determine whether a road is low volume as per TMUTCD Part 5. This information shall be shown in the plans.
- Based on existing field conditions, the Engineer/Inspector may require additional signs such as FLAGGER AHEAD, LOOSE GRAVEL, or other appropriate signs. When additional signs are required, these signs will be considered part of the minimum requirements. The Engineer/Inspector will determine the proper location and spacing of any sign not shown on the BC sheets, Traffic Control Plan sheets or the Work Zone Standard Sheets.
- 4. The "ROAD WORK NEXT X MILES" (G20-1aT) sign shall be required at high volume crossroads to advise motorists of the length of construction in either direction from the intersection. The Engineer will determine whether a roadway is considered high volume.

WORK AREAS IN MULTIPLE LOCATIONS WITHIN CSJ LIMITS

CW1-4

CW13-1P

Channelizing

- 5. Additional traffic control devices may be shown elsewhere in the plans for higher volume crossroads.
- 6. When work occurs in the intersection area, appropriate traffic control devices, as shown elsewhere in the plans or as determined by the Engineer/Inspector, shall be in place.

ROAD

WORK

⅓ MIL

CW20-1E

 $\times \times G20-6T$

END ROAD WORK

G20-2 * *

ROAD

WORK

AHEAD

CW20-1D

BEGIN T-INTERSECTION $\times \times$ G20-9TP ZONE ★ R20-5T FINES DOLIBL ★ R20-5aTP WHEN WORKERS ARE PRESENT ROAD WORK <⇒ NEXT X MILES * X G20-25T WORK ZONE G20-1bTI INTERSECTED 1000' -1500' 1 Block - City - Hwy 1000'-1500' - Hwy 1 Block - City ROADWAY \Rightarrow ROAD WORK G20-16TR NEXT X MILES ⇒ 80' WORK ZONE G20-2bT * * BEGIN WORK \times \times G20-9TP ZONE TRAFFI G20-6T $+ \times R20-5T$ FINES DOUBLE \times \times R20-5aTP ROAD WORK G20-2

CSJ LIMITS AT T-INTERSECTION

- 1. The Engineer will determine the types and location of any additional traffic control devices, such as a flagger and accompanying signs, or other signs, that should be used when work is being performed at or near an intersection.
- 2. If construction closes the road at a T-intersection, the Contractor shall place the "CONTRACTOR NAME"(G20-6T) sign behind the Type 3 Barricades for the road closure (see BC(10) also). The "ROAD WORK NEXT X MILES" left arrow(G20-1bTL) and "ROAD WORK NEXT X MILES" right arrow (G20-1bTR)" signs shall be replaced by the detour signing called for in the plans.

SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING AT THE CSJ LIMITS

SIGNS

STATE LAW

 \triangleleft

 \Rightarrow

R20-3

TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING $^{\text{I,5,6}}$

SIZE

onventional

Road

48" x 48

36" × 36

48" x 48"

Expressway/ Freeway	Posted Speed	S
	MPH	(Δ
48" × 48"	30	
70 / 70	35	
	40	
	45	
48" × 48"	50	
	55	
	60	
	65	
48" × 48"	70	
	75	
	80	1
	*	

Sign∆ bacing " X " Feet Apprx. 120 160 240 320 400 500^{2} 600² 700 2 800 ² 900 ² 000 ² *

SPACING

* For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.

 \triangle Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

Sign

Number

or Series

 $CW20^{4}$

CW21

CW22

CW23

CW25

CW14

CW1, CW2,

CW7, CW8,

CW9, CW11

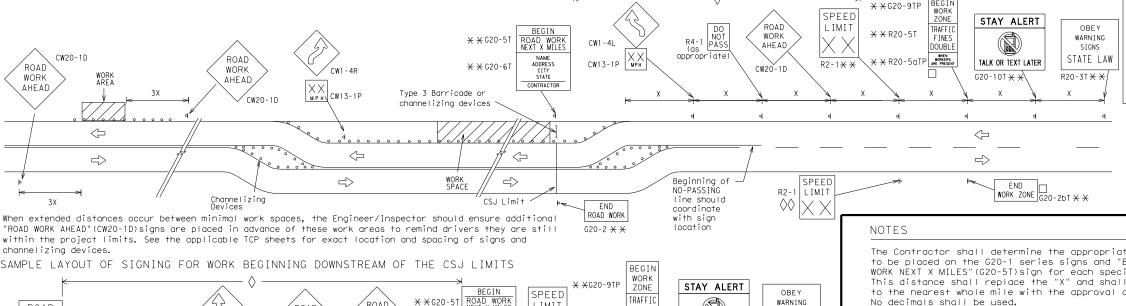
CW3, CW4,

CW5, CW6,

CW10, CW12

CW8-3,

- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- or more advance warning.
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D) signs may be used on low volume crossroads at the discretion of the Engineer as per TMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.
- 6. See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design



LIMIT

-CSJ Limi

R2-1

CONTRACTOR

¥ ¥R20-5T

 \times \times R20-5aTP

FINES

DOUBLE

SPEED R2-1

LIMIT

TALK OR TEXT LATER

END

WORK ZONE G20-25T *

The Contractor shall determine the appropriate distance to be placed on the G20-1 series signs and "BEGIN ROAD WORK NEXT X MILES"(G20-5T) sign for each specific project. This distance shall replace the "X" and shall be rounded to the nearest whole mile with the approval of the Engineer

The "BEGIN WORK ZONE" (G20-9TP) and "END WORK ZONE" (G20-2b) shall be used as shown on the sample layout when advance signs are required outside the CSJ Limits. They inform the motorist of entering or leaving a part of the work zone lying outside the CSJ Limits where traffic fines may double if workers are present.

	LEGEND						
\square	Type 3 Barricade						
000 Channelizing Devices							
•	Sign						
X	See Typical Construction Warning Sign Size and Spacing chart or the TMUTCD for sign spacing requirements.						

SHEET 2 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION PROJECT LIMIT

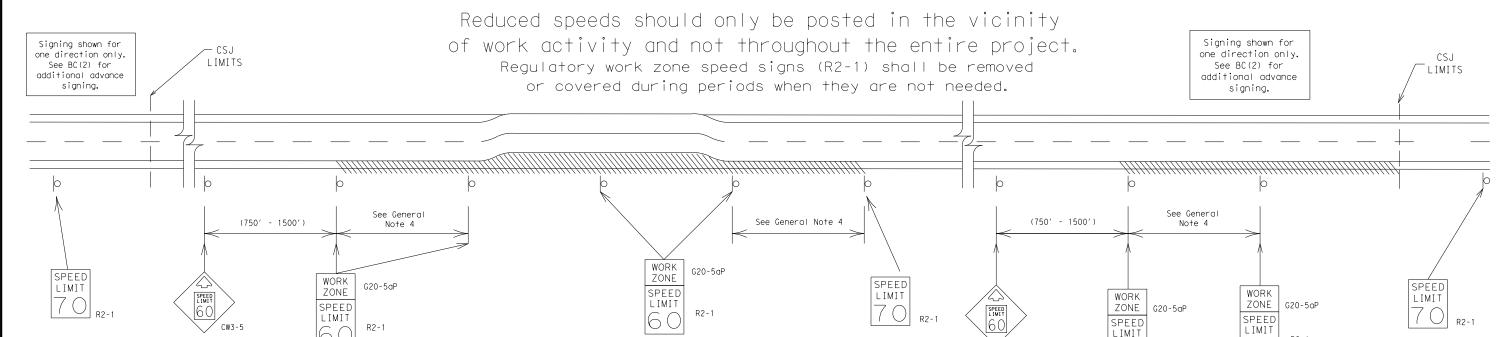
BC(2)-21

ILE:	bc-21.dgn	DN: To	<dot< td=""><td>ck: TxDOT</td><td>DW:</td><td>TxDC</td><td>)T</td><td>ck: TxDOT</td></dot<>	ck: TxDOT	DW:	TxDC)T	ck: TxDOT
D TxDOT	November 2002	CONT	SECT	JOB			HIG	HWAY
	REVISIONS	0918	11	100,ET	С.	CR	11	O,ETC.
9-07	8-14	DIST		COUNTY			S	HEET NO.
7-13	5-21	DAL		KAUFMA	٩N			20
0.0								

imes CSJ limit signing is required for highway construction and maintenance work, with the exception of mobile operations. Area for placement of "ROAD WORK AHEAD" (CW20-1D) sign and other signs or devices as called for on the Traffic Contractor will install a regulatory speed limit sign at the end of the work zone.

TYPICAL APPLICATION OF WORK ZONE SPEED LIMIT SIGNS

Work zone speed limits shall be regulatory, established in accordance with the "Procedures for Establishing Speed Zones," and approved by the Texas Transportation Commission, or by City Ordinance when within Incorporated City Limits.



GUIDANCE FOR USE:

LONG/INTERMEDIATE TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit should be included on the design of the traffic control plans when restricted geometrics with a lower design speed are present in the work zone and modification of the geometrics to a higher design speed is not feasible.

Long/Intermediate Term Work Zone Speed Limit signs, when approved as described above, should be posted and visible to the motorist when work activity is present. Work activity may also be defined as a change in the roadway that requires a reduced speed for motorists to safely negotiate the work area, including:

- a) rough road or damaged pavement surface
- b) substantial alteration of roadway geometrics (diversions)
- c) construction detours
- d) grade
- e) width
- f) other conditions readily apparent to the driver

As long as any of these conditions exist, the work zone speed limit signs should remain in place.

SHORT TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit may be included on the design of the traffic control plans when workers or equipment are not behind concrete barrier, when work activity is within 10 feet of the traveled way or actually in the traveled way.

Short Term Work Zone Speed Limit signs should be posted and visible to the motorists only when work activity is present. When work activity is not present, signs shall be removed or covered. (See Removing or Covering on BC(4)).

GENERAL NOTES

- 1. Regulatory work zone speed limits should be used only for sections of construction projects where speed control is of major importance.
- 2. Regulatory work zone speed limit signs shall be placed on supports at a 7 foot minimum mounting height.
- 3. Speed zone signs are illustrated for one direction of travel and are normally posted for each direction of travel.
- 4. Frequency of work zone speed limit signs should be:

40 mph and greater 0.2 to 2 miles

35 mph and less

0.2 to 1 mile

- 5. Regulatory speed limit signs shall have black legend and border on a white reflective background (See "Reflective Sheeting" on BC(4)).
- 6. Fabrication, erection and maintenance of the "ADVANCE SPEED LIMIT" (CW3-5) sign, "WORK ZONE" (G20-5aP) plaque and the "SPEED LIMIT" (R2-1) signs shall not be paid for directly, but shall be considered subsidiary to Item 502.
- 7. Turning signs from view, laying signs over or down will not be allowed, unless as otherwise noted under "REMOVING OR COVERING" on BC(4).
- Techniques that may help reduce traffic speeds include but are not limited to:
 Law enforcement.
 - B. Flagger stationed next to sign.
 - C. Portable changeable message sign (PCMS).
 - D. Low-power (drone) radar transmitter.
 - E. Speed monitor trailers or signs.
- 9. Speeds shown on details above are for illustration only. Work Zone Speed Limits should only be posted as approved for each project.
- 10. For more specific guidance concerning the type of work, work zone conditions and factors impacting allowable regulatory construction speed zone reduction see TxDOT form #1204 in the TxDOT e-form system.

SHEET 3 OF 12



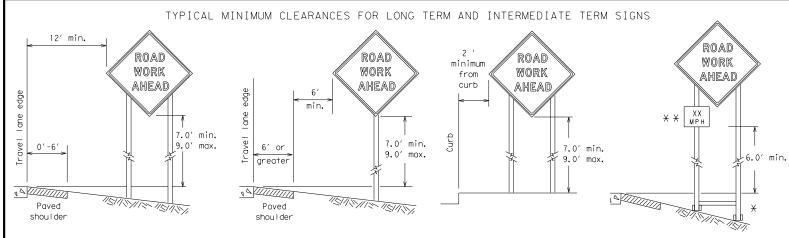
Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION WORK ZONE SPEED LIMIT

BC(3)-21

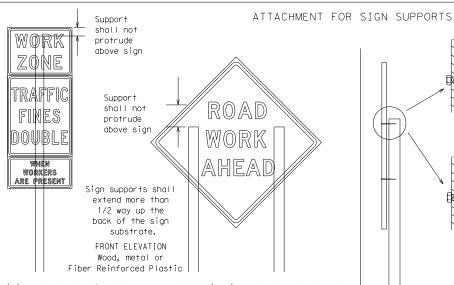
		_		_				
Ξ:	bc-21.dgn	DN: Tx[OT	ck: TxDOT	DW:	TxDOT	ck: TxDOT	
TxDOT	November 2002	CONT	SECT	JOB		н	HIGHWAY	
	REVISIONS		11	100,ET	c.	CR 1	10,ETC.	
9-07 7-13	8-14 5-21	DIST		COUNTY			SHEET NO.	
1-13	5-21	DAL		KAUFMA	٩N		21	

R2-1



* When placing skid supports on unlevel ground, the leg post lengths must be adjusted so the sign appears straight and plumb. Objects shall NOT be placed under skids as a means of leveling.

X When plaques are placed on dual-leg supports, they should be attached to the upright nearest the travel lane. Supplemental plaques (advisory or distance) should not cover the surface of the parent sign.



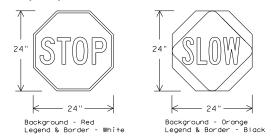
Splicing embedded perforated square metal tubing in order to extend post height will only be allowed when the splice is made using four bolts, two above and two below the spice point. Splice must be located entirely behind the sign substrate, not near the base of the support. Splice insert lengths should be at least 5 times nominal post size, centered on the splice and of at least the same gauge material.

Attachment to wooden supports will be by bolts and nuts or screws. Use TxDOT's or manufacturer's recommended procedures for attachina sign substrates to other types of sign supports

Nails shall NOT be allowed. Each sign shall be attached directly to the sign support. Multiple signs shall not be joined or spliced by any means. Wood supports shall not be extended or repaired by splicing or other means.

STOP/SLOW PADDLES

- 1. STOP/SLOW paddles are the primary method to control traffic by flaggers. The STOP/SLOW paddle size should be 24" x 24". STOP/SLOW paddles shall be retroreflectorized when used at night.
- 3. STOP/SLOW paddles may be attached to a staff with a minimum length of 6' to the bottom of the sign.
- 4. Any lights incorporated into the STOP or SLOW paddle faces shall only be as specifically described in Section 6E.03 Hand Signaling Devices in the TMUTCD.



SHEETING RE	QUIREMENT	rs (when used at night)
USAGE	COLOR	SIGN FACE MATERIAL
BACKGROUND	RED	TYPE B OR C SHEETING
BACKGROUND	ORANGE	TYPE B _{fl} OR C _{fl} SHEETING
LEGEND & BORDER	WHITE	TYPE B OR C SHEETING
LEGEND & BORDER	BLACK	ACRYLIC NON-REFLECTIVE FILM

CONTRACTOR REQUIREMENTS FOR MAINTAINING PERMANENT SIGNS WITHIN THE PROJECT LIMITS

SIDE ELEVATION

Wood

- 1. Permanent signs are used to give notice of traffic laws or regulations, call attention to conditions that are potentially hazardous to traffic operations, show route designations, destinations, directions, distances, services, points of interest, and other geographical, recreational, specific service (LOGO), or cultural information. Drivers proceeding through a work zone need the same, if not better route guidance as normally installed on a roadway without construction
- 2. When permanent regulatory or warning signs conflict with work zone conditions, remove or cover the permanent signs until the permanent sign message matches the roadway condition. For details for covering large guide signs see the TS-CD standard.
- When existing permanent signs are moved and relocated due to construction purposes, they shall be visible to motorists at all times.
- 4. If existing signs are to be relocated on their original supports, they shall be installed on crashworthy bases as shown on the SMD Standard sheets. The signs shall meet the required mounting heights shown on the BC Sheets or the SMD Standards. This work should be paid for under the appropriate pay item for relocating existing signs.
- If permanent signs are to be removed and relocated using temporary supports the Contractor shall use crashworthy supports as shown on the BC standard sheets, TLRS standard sheets or the CWZTCD list. The signs shall meet the required mounting heights shown on the BC, or the SMD standard sheets during construction. This work should be paid for under the appropriate pay item for relocating existing signs.
- Any sign or traffic control device that is struck or damaged by the Contractor or his/her construction equipment shall be replaced as soon as possible by the Contractor to ensure proper guidance for the motorists. This will be subsidiary to Item 502

GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports
- All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and guide the traveling public safely through the work zone.
- 5. The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the IMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so the Engineer can verify the correct procedures are being followed.
- The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used for identification shall be 1 inch.
- 9. The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)

- The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in regard to crashworthiness and duration of work requirements.
 - a. Long-term stationary work that occupies a location more than 3 days.
 - Intermediate-term stationary work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting more than one hour.
 - Short-term stationary daytime work that occupies a location for more than 1 hour in a single daylight period. Short, duration work that occupies a location up to 1 hour.
- e. Mobile work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except as shown for supplemental plaques mounted below other signs.
- The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above
- the ground. Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- 4. Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to appropriate Long-term/Intermediate sign height.
- Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

SIZE OF SIGNS

1. The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

SIGN SUBSTRATES

- The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports.
- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave.
- 3. All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6" centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- 1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300
- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).
- 2. White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL} , shall be used for rigid signs with orange backgrounds.

SIGN LETTERS

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- 1. When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- 2. Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any intersections where the sign may be seen from approaching traffic.
- 3. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely covered when not required.
- 4. When signs are covered, the material used shall be opaque, such as heavy mill black plastic, or other materials which will cover the entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.
- Burlap shall NOT be used to cover signs. Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a
- constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted
- for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used.
- Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured
- with rubber bases may be used when shown on the CWZTCD list. Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face. SHEET 4 OF 12



BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

Traffic Safety Division Standard

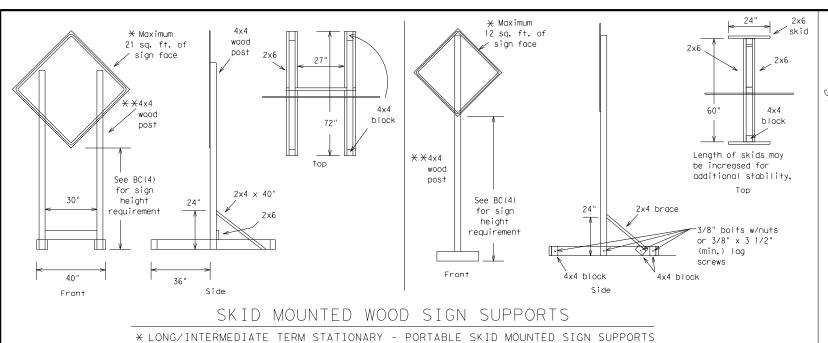
BC(4)-21

ILE:	bc-21.dgn	DN: T	<dot< th=""><th>ck: TxDOT</th><th>DW:</th><th>TxDO</th><th>T </th><th>ck: TxDOT</th></dot<>	ck: TxDOT	DW:	TxDO	T	ck: TxDOT
C TxDOT	November 2002	CONT	SECT	JOB		HIGHWAY		HWAY
		0918	11	100,ET	С.	CR	110	O,ETC.
9-07	8-14	DIST		COUNTY			SI	HEET NO.
7-13	5-21	DAL		KAUFMA	lΝ			22

4:56:48

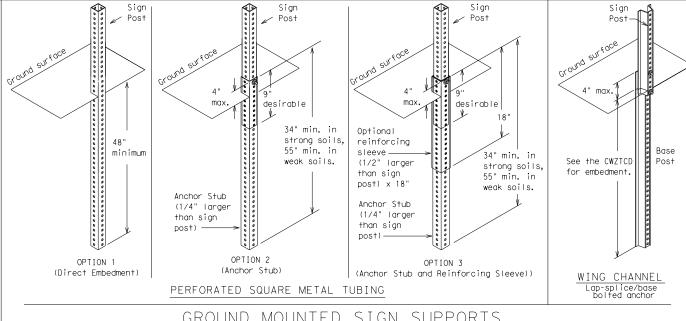
2/16/2023 N: \P5072-1

- weld starts here



upright

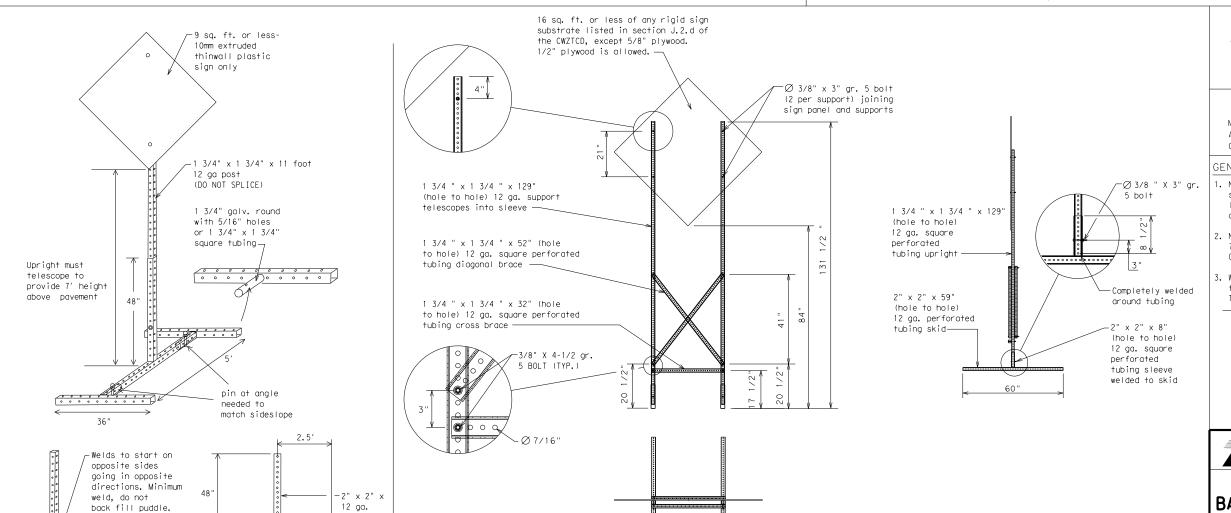
SINGLE LEG BASE



GROUND MOUNTED SIGN SUPPORTS

Refer to the CWZTCD and the manufacturer's installation procedure for each type sign support. The maximum sign square footage shall adhere to the manufacturer's recommendation.

Two post installations can be used for larger signs.



32′

WEDGE ANCHORS

Both steel and plastic Wedge Anchor Systems as shown on the SMD Standard Sheets may be used as temporary sign supports for signs up to 10 square feet of sign face. They may be set in concrete or in sturdy soils if approved by the Engineer. (See web address for "Traffic Engineering Standard Sheets" on BC(1)).

OTHER DESIGNS

MORE DETAILS OF APPROVED LONG/INTERMEDIATE AND SHORT TERM SUPPORTS CAN BE FOUND ON THE CWZTCD LIST. SEE BC(1) FOR WEBSITE LOCATION.

- . Nails may be used in the assembly of wooden sign supports, but 3/8" bolts with nuts or 3/8" x 3 1/2" lag screws must be used on every joint for final
- 2. No more than 2 sign posts shall be placed within a 7 ft. circle, except for specific materials noted on the CW7TCD List.
- When project is completed, all sign supports and foundations shall be removed from the project site. This will be considered subsidiary to Item 502.
 - ★ See BC(4) for definition of "Work Duration."
- ** Wood sign posts MUST be one piece. Splicing will NOT be allowed. Posts shall be painted white.
- ☐ See the CWZTCD for the type of sign substrate that can be used for each approved sign support.

SHEET 5 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION TYPICAL SIGN SUPPORT

BC(5) - 21

FILE: bc-21.dgn	DN: To	<dot< th=""><th>ck: TxDOT</th><th>DW:</th><th>TxDOT</th><th>ck: TxDOT</th></dot<>	ck: TxDOT	DW:	TxDOT	ck: TxDOT
©TxDOT November 2002	CONT	SECT	JOB		ні	GHWAY
REVISIONS	0918	11	100,ET	c.	CR 1	IO,ETC.
9-07 8-14	DIST		COUNTY			SHEET NO.
7-13 5-21	DAL		KAUFMA	۸N		23

SKID	MOUNTED	PERFORATE	ED SQUA	RE S	TEEL T	UBING	SIGN	SUPPORTS
	* LONG/INT	ERMEDIATE TERM	STATIONAR	r - PORT	TABLE SKID	MOUNTED	SIGN SUP	PORTS

WHEN NOT IN USE, REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

PORTABLE CHANGEABLE MESSAGE SIGNS

- The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO," "FOR." "AT." etc.
- Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- 7. The message term "WEEKEND" should be used only if the work is to start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- 8. The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- 9. Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- Do not use the word "Danger" in message.
 Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT"
- on a PCMS. Drivers do not understand the message. 13. Do not display messages that scroll horizontally or vertically across
- the face of the sign.

 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be
- abbreviated, unless shown in the TMUTCD.

 15. PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE	ABBREVIATION
Access Road	ACCS RD	Major	MAJ
Alternate	ALT	Miles	MI
Avenue	AVE	Miles Per Hour	MPH
Best Route	BEST RTE	Minor	MNR
Boulevard	BLVD	Monday	MON
Bridge	BRDG	Normal	NORM
Canno+	CANT	North	N
Center	CTR	Northbound	(route) N
Construction Ahead	CONST AHD	Parking	PKING
CROSSING	XING	Road	RD
Detour Route	DETOUR RTE	Right Lane	RT LN
Do Not	DONT	Saturday	SAT
East	E	Service Road	SERV RD
Eastbound	(route) E	Shoulder	SHLDR
Emergency	EMER	Slippery	SLIP
Emergency Vehicle	EMER VEH	South	S
Entrance, Enter	ENT	Southbound	(route) S SPD
Express Lane	EXP LN	Speed	SPU
Expressway	EXPWY	Street	
XXXX Feet	XXXX FT	Sunday	SUN
Fog Ahead	FOG AHD	Telephone	PHONE
Freeway	FRWY, FWY	Temporary	TEMP
Freeway Blocked	FWY BLKD	Thursday	THURS
Friday	FRI	To Downtown	TO DWNTN
Hazardous Driving		Traffic	TRAF
Hazardous Material		Travelers	TRVLRS
High-Occupancy	HOV	Tuesday	TUES
Vehicle		Time Minutes	TIME MIN
Highway	HWY	Upper Level	UPR LEVEL
Hour(s)	HR. HRS	Vehicles (s)	VEH, VEHS
Information	INFO	Warning	WARN
It Is	ITS	Wednesday	WED
Junction	JCT	Weight Limit	WT LIMIT
Left	LFT	West	W
Left Lane	LFT IN	Westbound	(route) W
Lane Closed	LN CLOSED	Wet Pavement	WET PVMT
Lower Level	LWR LEVEL	Will Not	WONT
Maintenance	MAINT		
Mu:ntenance	MAINI		

Roadway

designation # IH-number, US-number, SH-number, FM-number

RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES (The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

Road/Lane/Rar	mp	Closure List	Other Co	ndi	tion List	
FREEWAY CLOSED X MILE		FRONTAGE ROAD CLOSED	ROADWORK XXX FT		ROAD REPAIRS XXXX FT	
ROAD CLOSED AT SH XXX		SHOULDER CLOSED XXX FT	FLAGGER XXXX FT		LANE NARROWS XXXX FT	
ROAD CLSD AT FM XXXX		RIGHT LN CLOSED XXX FT	RIGHT LN NARROWS XXXX FT		TWO-WAY TRAFFIC XX MILE	
RIGHT X LANES CLOSED		RIGHT X LANES OPEN	MERGING TRAFFIC XXXX FT		CONST TRAFFIC XXX FT	
CENTER LANE CLOSED		DAYTIME LANE CLOSURES	LOOSE GRAVEL XXXX FT		UNEVEN LANES XXXX FT	
NIGHT LANE CLOSURES		I-XX SOUTH EXIT CLOSED	DETOUR X MILE		ROUGH ROAD XXXX FT	
VARIOUS LANES CLOSED		EXIT XXX CLOSED X MILE	ROADWORK PAST SH XXXX		ROADWORK NEXT FRI-SUN	
EXIT CLOSED		RIGHT LN TO BE CLOSED	BUMP XXXX FT		US XXX EXIT X MILES	
MALL DRIVEWAY CLOSED		X LANES CLOSED TUE - FRI	TRAFFIC SIGNAL XXXX FT		LANES SHIFT]*

Phase 2: Possible Component Lists

А		e/E Lis	ffect on Trave st	è	Location List		Warning List		* * Advance Notice List
	MERGE RIGHT		FORM X LINES RIGHT		AT FM XXXX		SPEED LIMIT XX MPH		TUE-FRI XX AM- X PM
	DETOUR NEXT X EXITS		USE XXXXX RD EXIT		BEFORE RAILROAD CROSSING		MAXIMUM SPEED XX MPH		APR XX- XX X PM-X AM
	USE EXIT XXX		USE EXIT I-XX NORTH		NEXT X MILES		MINIMUM SPEED XX MPH		BEGINS MONDAY
	STAY ON US XXX SOUTH		USE I-XX E TO I-XX N		PAST US XXX EXIT		ADVISORY SPEED XX MPH		BEGINS MAY XX
	TRUCKS USE US XXX N		WATCH FOR TRUCKS		XXXXXXX TO XXXXXXX		RIGHT LANE EXIT		MAY X-X XX PM - XX AM
	WATCH FOR TRUCKS		EXPECT DELAYS		US XXX TO FM XXXX		USE CAUTION		NEXT FRI-SUN
	EXPECT DELAYS		PREPARE TO STOP				DRIVE SAFELY		XX AM TO XX PM
	REDUCE SPEED XXX FT		END SHOULDER USE				DRIVE WITH CARE		NEXT TUE AUG XX
×	USE OTHER ROUTES		WATCH FOR WORKERS						TONIGHT XX PM- XX AM
n Phase 2.	STAY IN LANE) *			*	* See Ar	oplication Guide	lines 1	Note 6.

APPLICATION GUIDELINES

- 1. Only 1 or 2 phases are to be used on a PCMS.
- The 1st phase (or both) should be selected from the "Road/Lane/Ramp Closure List" and the "Other Condition List".

* LANES SHIFT in Phase 1 must be used with STAY IN LANE in

- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- 2. Roadway designations IH, US, SH, FM and LP can be interchanged as appropriate.
- 3. EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- 5. ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary.
- 7. FT and MI, MILE and MILES interchanged as appropriate.
- Distances or AHEAD can be eliminated from the message if a location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR
CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4)
PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE
UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION
OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC, THE FOUR DRUMS
SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

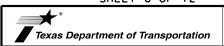
FULL MATRIX PCMS SIGNS

XXXXXXXX BLVD

CLOSED

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under "PORTABLE CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol"(CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the Engineer, it shall maintain the legibility/visibility requirement listed above.
- 3. When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and shall not substitute for, or replace that sign.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(7), for the same size arrow.

SHEET 6 OF 12



Traffic Safety Division Standard

PORTABLE CHANGEABLE MESSAGE SIGN (PCMS)

BC(6)-21

ILE:	bc-21.dgn	DN: T>	(DOT	ck: TxDOT	DW:	TxDO	T CK: TxDOT
C TxDOT	November 2002	CONT	SECT	JOB			HIGHWAY
	REVISIONS	0918	11	100,ET	С.	CR	110,ETC.
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	DAL		KAUFMA	lΝ		24

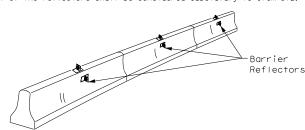
100

₽ 0

4:56:49 8-19-4\CA

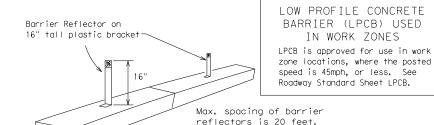
2/1

- 1. Barrier Reflectors shall be pre-auglified, and conform to the color and reflectivity requirements of DMS-8600. A list of pregualified Barrier Reflectors can be found at the Material Producer List web address shown on BC(1).
- 2. Color of Barrier Reflectors shall be as specified in the TMUTCD. The cost of the reflectors shall be considered subsidiary to Item 512.



CONCRETE TRAFFIC BARRIER (CTB)

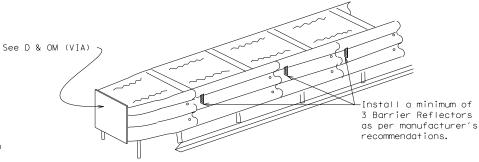
- 3. Where traffic is on one side of the CTB, two (2) Barrier Reflectors shall be mounted in approximately the midsection of each section of CTB. An alternate mounting location is uniformly spaced at one end of each CTB. This will allow for attachment of a barrier grapple without damaging the reflector. The Barrier Reflector mounted on the side of the CTB shall be located directly below the reflector mounted on top of the barrier, as shown in the detail above.
- 4. Where CTB separates two-way traffic, three barrier reflectors shall be mounted on each section of CTB. The reflector unit on top shall have two yellow reflective faces (Bi-Directional) while the reflectors on each side of the barrier shall have one yellow reflective face, as shown in the detail above.
- 5. When CTB separates traffic traveling in the same direction, no barrier reflectors will be required on top of the CTB.
- 6. Barrier Reflector units shall be yellow or white in color to match the edgeline being supplemented.
- 7. Maximum spacing of Barrier Reflectors is forty (40) feet.
- 8. Pavement markers or temporary flexible-reflective roadway marker tabs shall NOT be used as CTB delineation.
- 9. Attachment of Barrier Reflectors to CTB shall be per manufacturer's
- 10.Missing or damaged Barrier Reflectors shall be replaced as directed by the Engineer.
- 11. Single slope barriers shall be delineated as shown on the above detail.



LOW PROFILE CONCRETE BARRIER (LPCB)

Attach the delineators as per manufacturer's recommendations.

IN WORK ZONES



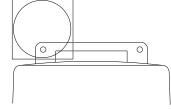
DELINEATION OF END TREATMENTS

END TREATMENTS FOR CTB'S USED IN WORK ZONES

End treatments used on CTB's in work zones shall meet the apppropriate crashworthy standards as defined in the Manual for Assessing Safety Hardware (MASH). Refer to the CWZTCD List for approved end treatments and manufacturers.

BARRIER REFLECTORS FOR CONCRETE TRAFFIC BARRIER AND ATTENUATORS

Type C Warning Light or approved substitute mounted on a drum adjacent to the travel way.



Warning reflector may be round or square. Must have a yellow reflective surface area of at least 30 square inches

WARNING LIGHTS

- 1. Warning lights shall meet the requirements of the TMUTCD.
- 2. Warning lights shall NOT be installed on barricades.
- 3. Type A-Low Intensity Flashing Warning Lights are commonly used with drums. They are intended to warn of or mark a potentially hazardous area. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "FL". The Type A Warning Lights shall not be used with signs manufactured with Type B_{FL} or C_{FL} Sheeting meeting the requirements of Departmental Material Specification DMS-8300.
- 4. Type-C and Type D 360 degree Steady Burn Lights are intended to be used in a series for delineation to supplement other traffic control devices. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "SB".
- 5. The Engineer/Inspector or the plans shall specify the location and type of warning lights to be installed on the traffic control devices.
- 6. When required by the Engineer, the Contractor shall furnish a copy of the warning lights certification. The warning light manufacturer will certify the warning lights meet the requirements of the latest ITE Purchase Specifications for Flashing and Steady-Burn Warning Lights. 7. When used to delineate curves, Type-C and Type D Steady Burn Lights should only be placed on the outside of the curve, not the inside.
- 8. The location of warning lights and warning reflectors on drums shall be as shown elsewhere in the plans.

WARNING LIGHTS MOUNTED ON PLASTIC DRUMS

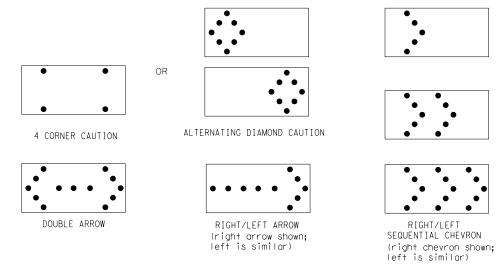
- 1. Type A flashing warning lights are intended to warn drivers that they are approaching or are in a potentially hazardous area.
- 2. Type A random flashing warning lights are not intended for delineation and shall not be used in a series.
- 3. A series of sequential flashing warning lights placed on channelizing devices to form a merging taper may be used for delineation. If used, the successive flashing of the sequential warning lights should occur from the beginning of the taper to the end of the merging taper in order to identify the desired vehicle path. The rate of flashing for each light shall be 65 flashes per minute, plus or minus 10 flashes.
- 4. Type C and D steady-burn warning lights are intended to be used in a series to delineate the edge of the travel lane on detours, on lane changes, on lane closures, and on other similar conditions.
- 5. Type A, Type C and Type D warning lights shall be installed at locations as detailed on other sheets in the plans.
- 6. Warning lights shall not be installed on a drum that has a sign, chevron or vertical panel.
- 7. The maximum spacing for warning lights on drums should be identical to the channelizing device spacing.

WARNING REFLECTORS MOUNTED ON PLASTIC DRUMS AS A SUBSTITUTE FOR TYPE C (STEADY BURN) WARNING LIGHTS

- 1. A warning reflector or approved substitute may be mounted on a plastic drum as a substitute for a Type C, steady burn warning light at the discretion of the Contractor unless otherwise noted in the plans.
- 2. The warning reflector shall be yellow in color and shall be manufactured using a sign substrate approved for use with plastic drums listed
- 3. The warning reflector shall have a minimum retroreflective surface area (one-side) of 30 square inches.
- 4. Round reflectors shall be fully reflectorized, including the area where attached to the drum.
- 5. Square substrates must have a minimum of 30 square inches of reflectorized sheeting. They do not have to be reflectorized where it attaches to the drum.
- 6. The side of the warning reflector facing approaching traffic shall have sheeting meeting the color and retroreflectivity requirements for DMS 8300-Type B or Type C.
- 7. When used near two-way traffic, both sides of the warning reflector shall be reflectorized.
- 8. The warning reflector should be mounted on the side of the handle nearest approaching traffic.
- 9. The maximum spacing for warning reflectors should be identical to the channelizing device spacing requirements.

Arrow Boards may be located behind channelizing devices in place for a shoulder taper or merging taper, otherwise they shall be delineated with four (4) channelizing devices placed perpendicular to traffic on the upstream side of traffic.

- 1. The Flashing Arrow Board should be used for all lane closures on multi-lane roadways, or slow moving maintenance or construction activities on the travel lanes.
- 2. Flashing Arrow Boards should not be used on two-lane, two-way roadways, detours, diversions or work on shoulders unless the "CAUTION" display (see detail below) is used.
- The Engineer/Inspector shall choose all appropriate signs, barricades and/or other traffic control devices that should be used in conjunction with the Flashing Arrow Board.
- 4. The Flashing Arrow Board should be able to display the following symbols:



- 5. The "CAUTION" display consists of four corner lamps flashing simultaneously, or the Alternating Diamond Caution mode as shown.
- The straight line caution display is NOT ALLOWED.
- The Flashing Arrow Board shall be capable of minimum 50 percent dimming from rated lamp voltage. The flashing rate of the lamps shall not be less than 25 nor more than 40 flashes per minute.
- Minimum lamp "on time" shall be approximately 50 percent for the flashing arrow and equal intervals of 25 percent for each sequential phase of the flashing chevron.
- 9. The sequential arrow display is NOT ALLOWED.
 10. The flashing arrow display is the TxDOT standard; however, the sequential chevron display may be used during daylight operations.

- 11. The Flashing Arrow Board shall be mounted on a vehicle, trailer or other suitable support.12. A Flashing Arrow Board SHALL NOT BE USED to laterally shift traffic.13. A full matrix PCMS may be used to simulate a Flashing Arrow Board provided it meets visibility,
- flash rate and dimming requirements on this sheet for the same size arrow. 14. Minimum mounting height of trailer mounted Arrow Boards should be 7 feet from roadway
- to bottom of panel.

	R	EQUIREMENTS	
TYPE	MINIMUM SIZE	MINIMUM NUMBER OF PANEL LAMPS	MINIMUM VISIBILITY DISTANCE
В	30 × 60	13	3/4 mile
С	48 × 96	15	1 mile

ATTENTION Flashing Arrow Boards shall be equipped with automatic dimmina devices.

WHEN NOT IN USE, REMOVE THE ARROW BOARD FROM THE RIGHT-OF-WAY OR PLACE THE ARROW BOARD BEHIND CONCRETE TRAFFIC BARRIER OR GUARDRAIL.

FLASHING ARROW BOARDS

SHEET 7 OF 12

TRUCK-MOUNTED ATTENUATORS

- 1. Truck-mounted attenuators (TMA) used on TxDOT facilities must meet the requirements outlined in the Manual for Assessing Safety Hardware (MASH).
- Refer to the CWZTCD for the requirements of Level 2 or Level 3 TMAs.
- 3. Refer to the CWZTCD for a list of approved TMAs.
- 4. TMAs are required on freeways unless otherwise noted n the plans
- 5. A TMA should be used anytime that it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the work performance.
- 6. The only reason a TMA should not be required is when a work area is spread down the roadway and the work crew is an extended distance from the TMA.



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION ARROW PANEL, REFLECTORS, WARNING LIGHTS & ATTENUATOR

BC(7) - 21

ILE:	bc-21.dgn	DN: T	<dot< td=""><td>ck: TxDOT</td><td>DW:</td><td>TxDOT</td><td>ck: TxDOT</td></dot<>	ck: TxDOT	DW:	TxDOT	ck: TxDOT
C) TxDOT	November 2002	CONT	SECT	JOB		F	HIGHWAY
		0918	11	100,ET	С.	CR 1	10,ETC.
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	DAI		KAUFMA	١N		25

101

1. For long term stationary work zones on freeways, drums shall be used as

the primary channelizing device.

2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only

if personnel are present on the project at all times to maintain the

- cones in proper position and location.

 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWTTCD).
- Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

GENERAL NOTES

Pre-qualified plastic drums shall meet the following requirements:

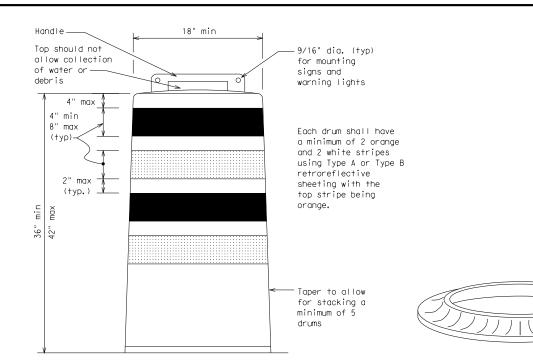
- Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

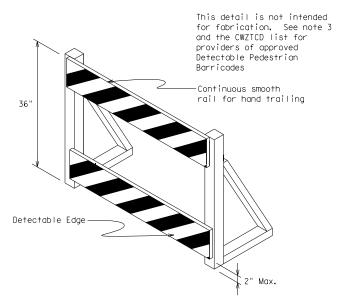
RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

BALLAST

- 1. Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- S. Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.





DETECTABLE PEDESTRIAN BARRICADES

- 1. When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
- Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- 3. Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian path.
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian
- Warning lights shall not be attached to detectable pedestrian barricades.
- 6. Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.



18" x 24" Sign (Maximum Sign Dimension) Chevron CW1-8, Opposing Traffic Lane Divider, Driveway sign D70a, Keep Right R4 series or other signs as approved by Engineer

See Ballast



12" x 24"
Vertical Panel
mount with diagonals
sloping down towards
travel way

Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums

SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS

- Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD.
- 2. Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL} Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
- 3. Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
- 4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
- Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection
- Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts.
- 7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans.
- R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.

SHEET 8 OF 12



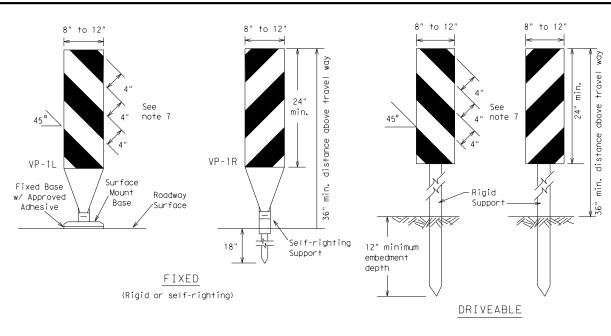
Traffic Safety Division Standard

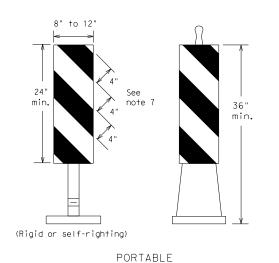
BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC(8)-21

LE: bc-21.dgn	DN: T	OOT	ck: TxDOT	DW:	T×DOT	ck: TxDOT
TxDOT November 2002	CONT	SECT	JOB		HI	GHWAY
	0918	11	100,ET	С.	CR 1	O,ETC.
1-03 8-14 1-07 5-21	DIST		COUNTY			SHEET NO.
'-13	DAL		KAUFMA	٩N		26

4:56:50

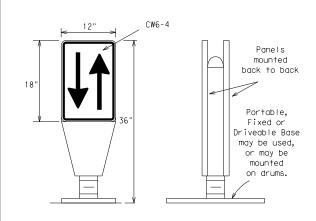




- 1. Vertical Panels (VP's) are normally used to channelize traffic or divide opposing lanes of traffic.
- 2. VP's may be used in daytime or nighttime situations. They may be used at the edge of shoulder drop-offs and other areas such as lane transitions where positive daytime and nighttime delineation is required. The Engineer/Inspector shall refer to the Roadway Design Manual for additional requirements on the use VP's for drop-offs.
- 3. VP's should be mounted back to back if used at the edge of cuts adjacent to two-way two lane roadways. Stripes are to be reflective orange and reflective white and should always slope downward toward the travel lane.
- 4. VP's used on expressways and freeways or other high speed roadways, may have more than 270 square inches of retroreflective area facing traffic.

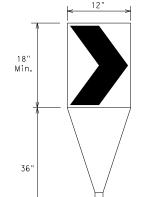
 5. Self-righting supports are available with portable base.
- See "Compliant Work Zone Traffic Control Devices List"
- 6. Sheeting for the VP's shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300, unless noted otherwise.
- 7. Where the height of reflective material on the vertical panel is 36 inches or greater, a panel stripe of 6 inches shall be used.

VERTICAL PANELS (VPs)



- 1. Opposing Traffic Lane Dividers (OTLD) are delineation devices designed to convert a normal one-way roadway section to two-way operation. OTLD's are used on temporary centerlines. The upward and downward arrows on the sign's face indicate the direction of traffic on either side of the divider. The base is secured to the pavement with an adhesive or rubber weight to minimize movement caused by a vehicle impact or wind gust.
- 2. The OTLD may be used in combination with 42"
- 3. Spacing between the OTLD shall not exceed 500 feet. 42" cones or VPs placed between the OTLD's should not exceed 100 foot spacing.
- 4. The OTLD shall be orange with a black nonreflective legend. Sheeting for the OTLD shall be retroreflective Type $B_{\text{FL}}\,\text{or}$ Type $C_{\text{FL}}\,\text{conforming}$ to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.

OPPOSING TRAFFIC LANE DIVIDERS (OTLD)



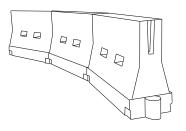
Fixed Base w/ Approved Adhesive (Driveable Base, or Flexible Support can be used)

- 1. The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the out side of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflec-tive legend. Sheeting for the chevron shall be retroreflective Type BFL or Type CFL conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- 6. For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS

GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- 6. Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final payement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.



LONGITUDINAL CHANNELIZING DEVICES (LCD)

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- 2. LCDs may be used instead of a line of cones or drums.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- 1. Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- 2. Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings.
- 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- 5. When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

Posted Speed	Formula	Minimum Desirable Taper Lengths X X			Suggested Maximum Spacing of Channelizing Devices			
		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent		
30	2	150′	165′	180′	30′	60′		
35	$L = \frac{WS^2}{60}$	205′	225′	245′	35′	70′		
40	60	265′	295′	320′	40'	80′		
45		450′	4951	540′	45′	90′		
50		500′	550′	600′	50′	100′		
55	L=WS	550′	605′	660′	55′	110′		
60	L 113	600′	660′	720′	60′	120′		
65		650′	715′	780′	65′	130′		
70		700′	770′	840′	70′	140′		
75		750′	825′	900′	75′	150′		
80		800′	880′	960′	80′	160′		

 $X \times Y$ Taper lengths have been rounded off. L=Length of Taper (FT.) W=Width of Offset (FT.) S=Posted Speed (MPH)

SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

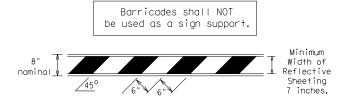
BC(9) - 21

ILE:	bc-21.dgn	DN: To	<dot< td=""><td>ck: TxDOT</td><td>DW:</td><td>TxD01</td><td>CK: TXDOT</td></dot<>	ck: TxDOT	DW:	TxD01	CK: TXDOT
C) TxDOT	November 2002	CONT	SECT	JOB			HIGHWAY
		0918	11	100,ET	c.	CR	110,ETC.
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	DAL		KAUFMA	٩N		27

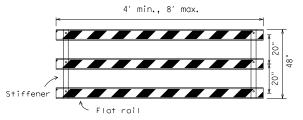
₽ 6

TYPE 3 BARRICADES

- 1. Refer to the Compliant Work Zone Traffic Control Devices List (CWZTCD) for details of the Type 3 Barricades and a list of all materials used in the construction of Type 3 Barricades.
- Type 3 Barricades shall be used at each end of construction projects closed to all traffic.
- 3. Barricades extending across a roadway should have stripes that slope downward in the direction toward which traffic must turn in detouring. When both right and left turns are provided, the chevron striping may slope downward in both directions from the center of the barricade. Where no turns are provided at a closed road, striping should slope downward in both directions toward the center of roadway.
- 4. Striping of rails, for the right side of the roadway, should slope downward to the left. For the left side of the roadway, striping should slope downward to the right.
- Identification markings may be shown only on the back of the barricade rails. The maximum height of letters and/or company logos used for identification shall be 1"
- Barricades shall not be placed parallel to traffic unless an adequate clear zone is provided.
- Warning lights shall NOT be installed on barricades.
- 8. Where barricades require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand is recommended. The sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight. Sand bags shall not be stacked in a manner that covers any portion of a barricade rails reflective sheeting. Rock, concrete, iron, steel or other solid objects will NOT be permitted. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs. Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall not be used for sandbags. Sandbags shall only be placed along or upon the base supports of the device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners.
- Sheeting for barricades shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300 unless otherwise noted.

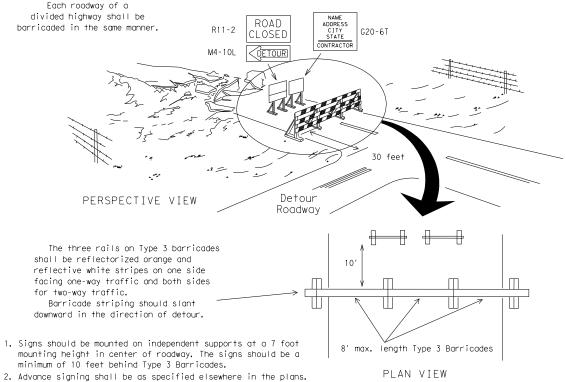


TYPICAL STRIPING DETAIL FOR BARRICADE RAIL

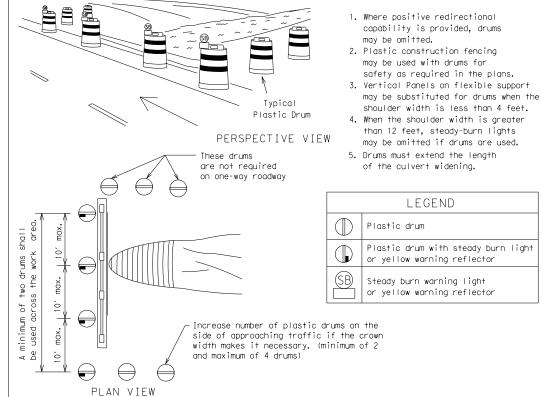


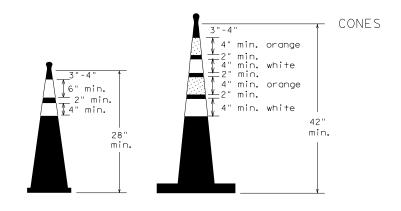
Stiffener may be inside or outside of support, but no more than 2 stiffeners shall be allowed on one barricade.

TYPICAL PANEL DETAIL FOR SKID OR POST TYPE BARRICADES

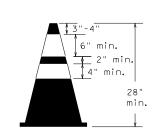


TYPE 3 BARRICADE (POST AND SKID) TYPICAL APPLICATION

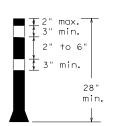




Two-Piece cones

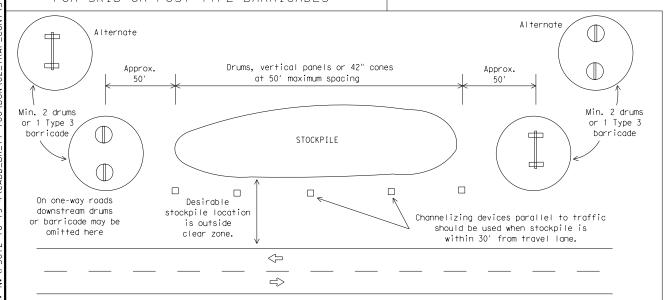


One-Piece cones



CULVERT WIDENING OR OTHER ISOLATED WORK WITHIN THE PROJECT LIMITS

Tubular Marker



TRAFFIC CONTROL FOR MATERIAL STOCKPILES

28" Cones shall have a minimum weight of 9 1/2 lbs.

42" 2-piece cones shall have a minimum weight of 30 lbs. including base.

- 1. Traffic cones and tubular markers shall be predominantly orange, and meet the height and weight requirements shown above.
- 2. One-piece cones have the body and base of the cone molded in one consolidated unit. Two-piece cones have a cone shaped body and a separate rubber base. or ballast, that is added to keep the device upright and in place.
- 3. Two-piece cones may have a handle or loop extending up to 8" above the minimum height shown, in order to aid in retrieving the device.
- 4. Cones or tubular markers shall have white or white and orange reflective bands as shown above. The reflective bands shall have a smooth, sealed outer surface and meet the requirements of Departmental Material Specification DMS-8300 Type A or Type B.
- 5. 28" cones and tubular markers are generally suitable for short duration and short-term stationary work as defined on BC(4). These should not be used for intermediate-term or long-term stationary work unless personnel is on-site to maintain them in their proper upright position.
- 6. 42" two-piece cones, vertical panels or drums are suitable for all work zone durations.
- 7. Cones or tubular markers used on each project should be of the same size and shape.

SHEET 10 OF 12



Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC(10)-21

:	bc-21.dgn	DN: T>	(DOT	ck: TxDOT	DW:	T×D0	T	ck: TxDOT
TxDOT	November 2002	CONT	SECT	JOB			HIG	HWAY
	REVISIONS	0918	11	100,ET	c.	CR	11	O,ETC.
9-07	8-14	DIST		COUNTY			5	SHEET NO.
7-13	5-21	DAL		KAUFMA	٩N			28

2/16/2023 4:56:50 PM N:\P5072-18-19-4\Cadd BR 11-100\DGN\02 TRAF CONT\S+dDetgils\bs-2

WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing povement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Additional supplemental pavement marking details may be found in the plans or specifications.
- 4. Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- 7. All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

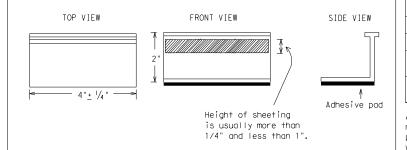
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- Removal of raised pavement markers shall be as directed by the Fnaineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10.Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECURE TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARKER TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guidemarks shall meet the requirements of DMS-8242.
- 2. Tabs detailed on this sheet are to be inspected and accepted by the Engineer or designated representative. Sampling and testing is not normally required, however at the option of the Engineer, either "A" or "B" below may be imposed to assure quality before placement on the
 - A. Select five (5) or more tabs at random from each lot or shipment and submit to the Construction Division, Materials and Pavement Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix five (5) tabs at 24 inch intervals on an asphaltic pavement in a straight line. Using a medium size passenger vehicle or pickup, run over the markers with the front and rear tires at a speed of 35 to 40 miles per hour, four (4) times in each direction. No more than one (1) out of the five (5) reflective surfaces shall be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new povements. See Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARKS

- Raised pavement markers used as guidemarks shall be from the approved product list, and meet the requirements of DMS-4200.
- All temporary construction raised pavement markers provided on a project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applied or butyl rubber pad for all surfaces, or thermoplastic for concrete surfaces.
- Guidemarks shall be designated as: YELLOW - (two amber reflective surfaces with yellow body). WHITE - (one silver reflective surface with white body).

DEPARTMENTAL MATERIAL SPECIFICATIO	NS
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
TRAFFIC BUTTONS	DMS-4300
EPOXY AND ADHESIVES	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS	DMS-8241
TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242

A list of prequalified reflective raised pavement markers, non-reflective traffic buttons, roadway marker tabs and other pavement markings can be found at the Material Producer List web address shown on BC(1).

SHEET 11 OF 12

Traffic Safety Division Standard

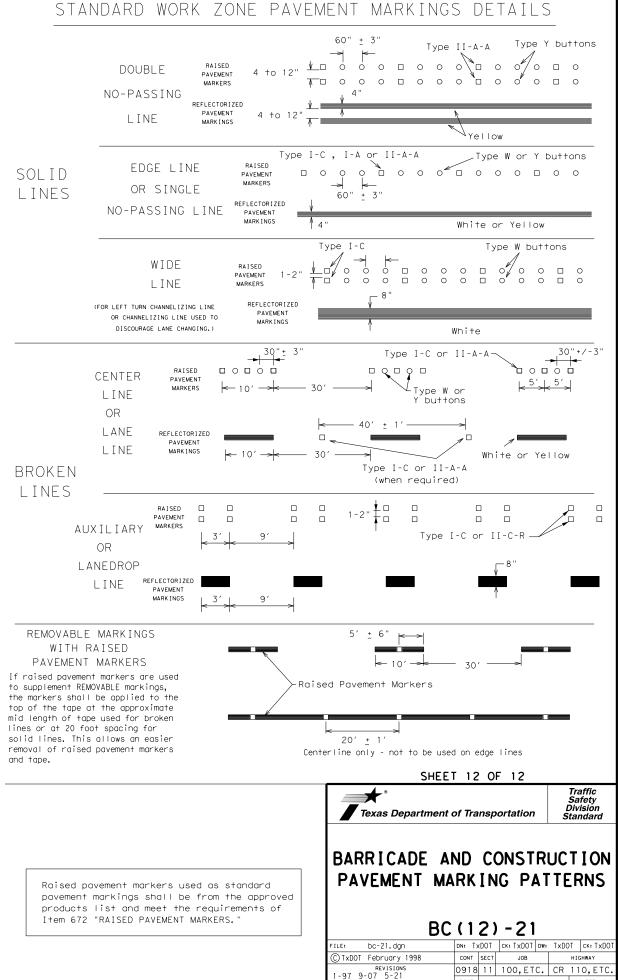


BARRICADE AND CONSTRUCTION PAVEMENT MARKINGS

BC(11)-21

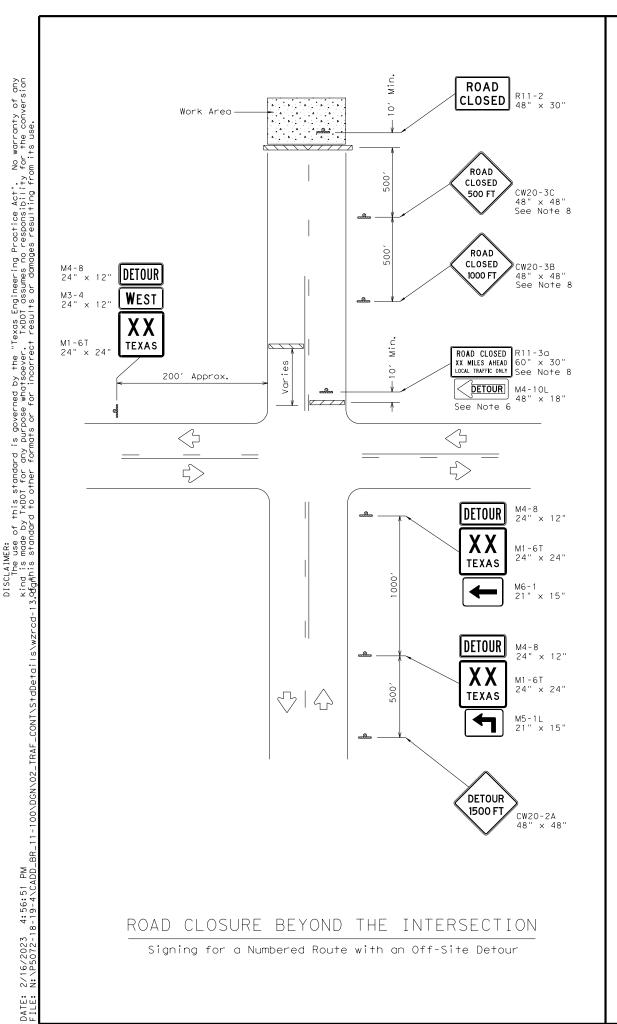
FILE: bc-21.dgn	DN: T	(DOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
© TxDOT February 1998	CONT	SECT	JOB		н	IGHWAY
REVISIONS 2-98 9-07 5-21	0918	11	100,ET	c.	CR 1	10,ETC.
1-02 7-13	DIST		COUNTY			SHEET NO.
11-02 8-14	DAL		KAUFMA	٩N		29

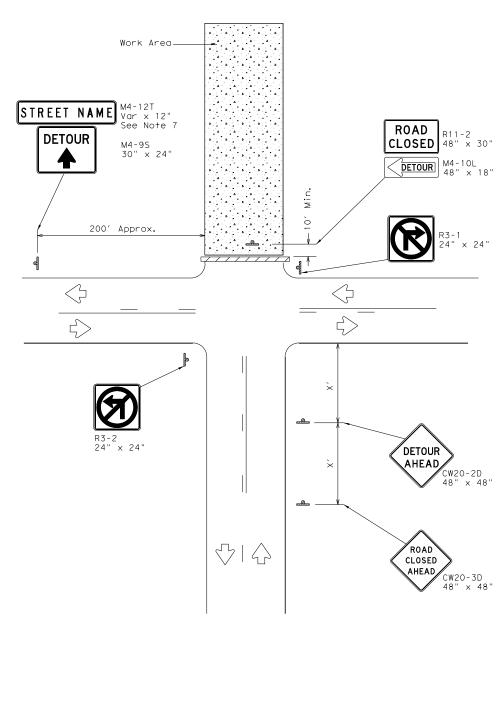
105



2-98 7-13 11-02 8-14

KAUFMAN





ROAD CLOSURE AT THE INTERSECTION Signing for an Un-numbered Route with an Off-Site Detour

LEGEND						
	Type 3 Barricade					
•	Sign					

Posted Speed *	Minimum Sign Spacing "X" Distance
30	120′
35	160′
40	240′
45	320′
50	400′
55	500′
60	600′
65	700′
70	800′
75	900′

* Conventional Roads Only

GENERAL NOTES

- 1. This sheet is intended to provide details for temporary work zone road closures. For permanent road closure details see the
- 2. Barricades used shall meet the requirements shown on Barricade and Construction Standard BC(10) and listed on the Compliant Work Zone Traffic Control Devices list (CWZTCD).
- 3. Stockpiled materials shall not be placed on the traffic side of barricades.
- 4. Barricades at the road closure should extend from pavement edge to pavement edge.
- 5. Detour signing shown is intended to illustrate the type of signing that is appropriate for numbered routes or un-numbered routes as labeled. It does not indicate the full extent of detour signing required. Detour routes should be signed as shown elsewhere in
- 6. If the road is open for a significant distance beyond the intersection or there are significant origin/destination points beyond the intersection, the signs and barricades at this location should be located at the edge of the traveled way.
- 7. The Street Name (M4-12T) sign is to be placed above the DETOUR (M4-9S) sign.
- 8. For urban areas where there is a shorter distance between the intersection and the actual closure location, the ROAD CLOSED XX MILES AHEAD (R11-3a) sign may be replaced with a ROAD CLOSED TO THRU TRAFFIC (R11-4) sign. If adequate space does not exist between the intersection and the closure a single ROAD CLOSED AHEAD (CW20-3D) sign spaced as per the table above may replace the ROAD CLOSED 1000 FT (CW20-3B) and ROAD CLOSED 500 FT (CW20-3C) signs.
- 9. Signs and barricades shown shall be subsidiary to Item 502. Locations where these details will be required shall be as shown elsewhere in the plans.



Traffic Operations Division Standard

WORK ZONE ROAD CLOSURE DETAILS

WZ (RCD) - 13

				_		
ile: wzrcd-13.d	gn DN: T	×DOT	ck: TxDOT	DW:	T×DOT	ck: TxDOT
TxDOT August 199	5 CONT	SECT	JOB HIGHWAY		SHWAY	
REVISIONS	0918	3 11	100,ET	c.	CR 11	O,ETC.
-97 4-98 7-13	DIST	COUNTY SHEET NO.				
-98 3-03	DAL		KAUFMA	٩N		31

	LINE DATA							
LINE		BE	LENGTH					
L1	N	83°	29′	23"	Е	12.96′		
L2	N	81°	35′	03"	Е	45.38′		
L3	N	65°	16′	43"	Ε	67.75′		
L4	N	60°	55′	37"	Ε	19.46′		
L5	N	64°	53′	39"	Ε	76.50′		
L6	N	68°	06′	05"	Ε	66.39′		

CONTROL MONUMENT INVERSE							
FROM	TO	BEARING	DISTANCE				
F1300272	F1300282	N 78°14′00"E	177.31′				
F1300282	F1300292	N 46°11′36"E	127.10′				
F1300292	F1300302	N 76° 43′ 40"E	216.32′				

ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202). NORTH AMERICAN DATUM OF 1983 (NAD 83). 2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED

NOTES

ADJUSTMENT FACTOR OF 1.00012. 2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY

3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXNA. VERTICAL SURVEY METHOD: DIGITAL LEVELING

5. UNIT OF MEASURE: U.S. SURVEY FEET

6. FIELD SURVEYS WERE PERFORMED BETWEEN APRIL, 2022 AND NOVEMBER, 2022.

F1300292

JONES CHEEK

END PROPOSED BASELINE CSJ: 0918-11-100 STA. 1118+27.21 N = 6,888,019.34E = 2,690,617.29LAT. = 32°32′29.61244" N LONG. = $96^{\circ}09'23.16131"$ W

END PROJECT

PI STATION = 1115+20.81

PC STATION = 1115+05.13 PT STATION = 1115+36.46

TANGENT = 15.68

LENGTH = 31.33

RADIUS = 350.00

DELTA = 5° 07' 45.40" (RT)

DEGREE OF CURVE = 16° 22′ 12.80"

STA. 1115+87.00

N = 6,887,922.47

E = 2,690,397.56

THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E.

O. DATE REVISION Texas Department of Transportation

LANDTECH

2525 North Loop West, Suite 300. Houston Texas 77008 T: 713-861-7068 F: 713-861-4131
TBPE Registration No. F-1364; TBPLS Registration No. 10019100

CR 110 OVER JONES CREEK SURVEY CONTROL INDEX

SIILLI I OI	J						
DN:	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	6	TEXAS		SEE TIT	LE SHE		CR 110,ETC
DW:	STATE DIST.	COUN	ITY	CONTROL S	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	DΔI	KΔIJF	MΔN	0918	11	100. FTC	32

TANGENT = 24.30 LENGTH = 48.59RADIUS = 868.00 PC STATION = 1117+12.23 PT STATION = 1117+60.82 PI STATION = 1114+72.38 $DELTA = 4^{\circ} 21' 06.32" (LT)$ DEGREE OF CURVE = 16° 22′ 12.80" TANGENT = 13.30 LENGTH = 26.58 RADIUS = 350.00 PC STATION = 1114+59.08 PT STATION = 1114+85.66 F1300282 C3 BEGIN PROJECT

F1300272

PI STATION = 1113+41.87 DELTA = 16° 18′ 19.91" (LT) DEGREE OF CURVE = 16° 22′ 12.80" TANGENT = 50.14 LENGTH = 99.60RADIUS = 350.00 PC STATION = 1112+91.73

PT STATION = 1113+91.33

BEGIN PROPOSED BASELINE CSJ: 0918-11-100 STA. 1110+00.00 N = 6,887,775.96E = 2,689,836.48 LAT. = 32°32′27.37691" N LONG. = 96°09′32.34424" W

PI STATION = 1111+38.48 $DELTA = 1^{\circ} 04' 05.21" (LT)$

PC STATION = 1111+17.71

TANGENT = 20.78LENGTH = 41.55RADIUS = 2,229.00

DEGREE OF CURVE = 2° 34′ 13.69"

PI STATION = 1112+09.29 DELTA = 1° 54′ 19.77" (LT)DEGREE OF CURVE = 2° 34′ 13.69" TANGENT = 37.07 LENGTH = 74.13 RADIUS = 2,229.00 PC STATION = 1111+72.22

STA. 1112+20.00

N € 6,887,798.84

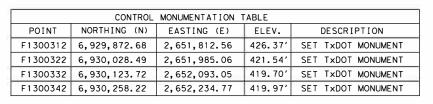
PT STATION = 1111+59.26

|E = 2,690,055.27

PI STATION = 1117+36.53 DELTA = 3° 12′ 26.27" (RT) DEGREE OF CURVE = 6° 36′ 03.23"

PT STATION = 1112+46.35

SCALE: 1"=50



LINE	BEARING	LENGTH
L1	N 48° 30′ 21" E	97.96′
L2	N 50° 30′ 19" E	40.22'
L3	N 47° 33′ 25" E	88.03′
L4	N 44° 27′ 53" E	18.65′
L5	N 47° 28′ 46" E	87.42′

CONTROL MONUMENT INVERSE								
FROM	TO	BEARING	DISTANCE					
F1300312	F1300322	N 47°54′42"E	232.44′					
F1300322	F1300332	N 48° 35′ 27"E	143.99′					
F1300332	F1300342	N 46°29′53"E	195.38'					

CSJ: 0918-11-101 STA. 2285+37.16 N = 6,930,306.03E = 2,652,259.09_AT. = 32°39′36.21018" N LONG. = 96°16′40.73278" W C4 PI STATION = 2284+08.17 $DELTA = 3^{\circ} 00' 53.66'' (RT)$ F1300342 TANGENT =41.58 LENGTH = 83.14 RADIUS = 1.580.00

END PROJECT STA. 2284+55.00 N = 6,930,250.50= 2,652,198.54

> THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

1. ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202), NORTH AMERICAN DATUM OF 1983 (NAD 83),

2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED

2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT

RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY

3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE

NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL

REFERENCE POINT TXNA. VERTICAL SURVEY METHOD:

6. FIELD SURVEYS WERE PERFORMED BETWEEN APRIL,

5. UNIT OF MEASURE: U.S. SURVEY FEET

ADJUSTMENT FACTOR OF 1.00012.

DIGITAL LEVELING

2022 AND NOVEMBER, 2022.

JACOB J. LUPHER

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED

IO. DATE REVISION Texas Department of Transportation

LANDTECH

2525 North Loop West, Suite 300. Houston Texas 77008 T: 713-861-7068 F: 713-861-4131
TBPE Registration No. F-1364; TBPLS Registration No. 10019100

CR 279 OVER BACHELOR CREEK RELIEF SURVEY CONTROL INDEX

EET Z OF					
:	FED. RD. DIV. NO.	STATE	PROJECT NO.		HIGHWAY NO.
DN:	6	TEXAS	SEE TITLE SHE		CR 110,ETC
:	STATE DIST.	COUNTY	CONTROL SECTION	JOB NO.	SHEET NO.
DW:	DAL	KAUFMAN	0918 11	100. FTC	33

PI STATION = 2281+33.99 DELTA = 2° 56′ 54.52" (LT)DEGREE OF CURVE = 3° 37′ 34.73" TANGENT = 40.66 LENGTH = 81.31 RADIUS = 1,580.00PC STATION = 2280+93.33 PT STATION = 2281+74.64

> PI STATION = 2280+25.54 DELTA = 1° 59′ 58.89" (RT) DEGREE OF CURVE = 3° 37′ 34.73" TANGENT = 27.57LENGTH = 55.14 RADIUS = 1,580.00PC STATION = 2279+97.96 PT STATION = 2280+53.11

BEGIN PROJECT STA. 2279+97.00

N = 6,929,941.89

E = 2,651,860.44

PROPOSED BASELINE CSJ: 0918-11-101 STA. 2279+00.00 N = 6,929,877.63E = 2,651,787.78LAT. = 32°39′32.07052" N LONG. = 96°16′46.35091"

F1300312

NOTES

AND INCORPORATED INTO THIS PS&E.

SHEET 2 OF 3

DN: CK DW: СК

PC STATION = 2283+66.59PT STATION = 2284+49.73

DEGREE OF CURVE = 3° 37′ 34.73"

F1300332

END

PROPOSED BASELINE

С3 PI STATION = 2283+05.32 $DELTA = 3^{\circ} 05' 32.14" (LT)$ DEGREE OF CURVE = 3° 37′ 34.73" TANGENT = 42.65LENGTH = 85.27

RADIUS = 1,580.00 PC STATION = 2282+62.67 PT STATION = 2283+47.94

F1300322

BACHELOR CREEK SUBST

SCALE: 1"=50

	LINE DATA								
LINE	BEARING	LENGTH							
L1	S 45° 59′ 59.40" E	27.99′							
L2	S 45° 11′ 28.16" E	104.64′							
L3	S 44° 34′ 22.55" E	56.64'							
L4	S 41° 02′ 16.42" E	41.23′							
L5	S 42° 27′ 29.34" E	18.83′							
L6	S 45° 08′ 56.23" E	47.50′							

CONTROL MONUMENT INVERSE							
FROM	FROM TO BEARING						
F1300362	F1300352	N 45° 49′41"W	345.77′				
F1300372	F1300362	N 43°51′39"W	148.01′				
F1300382	F1300372	N 44° 22′ 22"W	347.94′				

ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202), NORTH AMERICAN DATUM OF 1983 (NAD 83), 2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES
AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED ADJUSTMENT FACTOR OF 1.00012.

2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY METHOD: TXDOT RTN

3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXNA. VERTICAL SURVEY METHOD: DIGITAL LEVELING

5. UNIT OF MEASURE: U.S. SURVEY FEET

6. FIELD SURVEYS WERE PERFORMED BETWEEN APRIL, 2022 AND NOVEMBER, 2022.

CSJ: 0918-11-102 STA. 312+00.00 N = 6,944,435.10E = 2,688,646.44 LAT. = 32°41′48.17183" N LONG. = $96^{\circ} 09'31.49297"$ W PI STATION = 312+98.56 DELTA = 0° 48' 31.24" (RT)DEGREE OF CURVE = 0° 34′ 22.65" TANGENT = 70.57LENGTH = 141.14 RADIUS = 10,000.00 PC STATION = 312+27.99 PT STATION = 313+69.13 F1300362 BEGIN PROJECT STA. 313+97.00 N = 6,944,297.25E = 2,688,787.18PI STATION = 314+82.29 DELTA = 0° 37′ 05.61" (RT) DEGREE OF CURVE = 3° 37′ 34.73" TANGENT = 8.52 LENGTH = 17.05 RADIUS = 1,580.00PC STATION = 314+73.77 PT STATION = 314+90.82 C3 PI STATION = 315+56.72 $DELTA = 3^{\circ} 32' 06.12'' (RT)$ DEGREE OF CURVE = 19° 05′ 54.94" TANGENT = 9.26LENGTH = 18.51 RADIUS = 300.00 PC STATION = 315+47.46 PT STATION = 315+65.97

F1300352

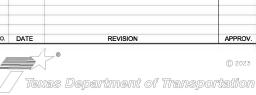
EXISTING BASELINE

THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E.



LANDTECH 2525 North Loop West, Suite 300.

Houston Texas 77008 T: 713-861-7068 F: 713-861-4131
TBPE Registration No. F-1364; TBPLS Registration No. 10019100

CR 312 OVER MUDDY CEDAR CREEK TRIB SURVEY CONTROL INDEX

SHEET 3 OF 3

N:	FED. RD. DIV. NO.	STATE	STATE PROJECT NO.		HIGHWAY NO.	
K DN:	6	TEXAS	SEE TIT	LE SHE		CR 110,ETC
W:	STATE DIST.	COUNTY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
K DW:	DAL	KAUFMAN	0918	11	100, ETC	34

PI STATION = 316+35.58 $DELTA = 1^{\circ} 25' 12.92" (LT)$ DEGREE OF CURVE = 2° 30′ 07.20" TANGENT = 28.38LENGTH = 56.76

RADIUS = 2,290.00PC STATION = 316+07.20 PT STATION = 316+63.96

EXISTING BASELINE CSJ: 0918-11-102 STA. 318+37.84 N = 6,943,978.70E = 2,689,091.69LAT. = 32°41′43.55841" N

LONG. = $96^{\circ} 09'26.40266''$ W

F1300382

PI STATION = 317+36.57DELTA = 2° 41′ 26.89" (LT) DEGREE OF CURVE = 2° 30′ 07.20" TANGENT = 53.78

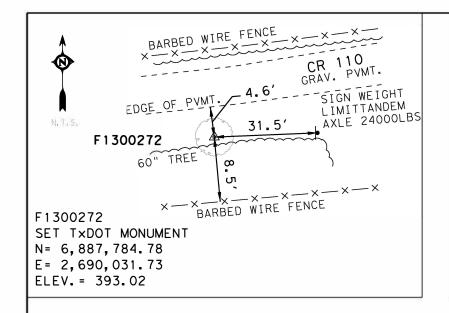
F1300372

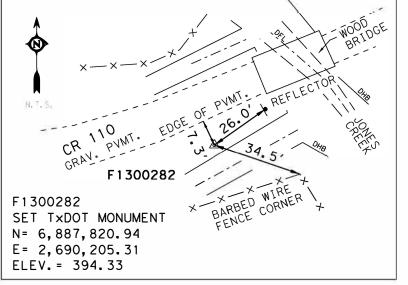
LENGTH = 107.55 RADIUS = 2,290.00PC STATION = 316+82.79

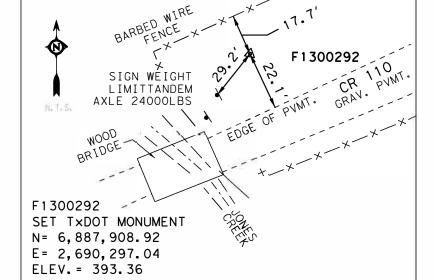
PT STATION = 317+90.34

END PROJECT STA. 316+42.00

N = 6,944,119.97E = 2,688,956.12







- 1. ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202), NORTH AMERICAN DATUM OF 1983 (NAD 83), 2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED ADJUSTMENT FACTOR OF 1.00012.
- 2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT RTM (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY METHOD. TXDOT PTM.
- 3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).
- 4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) OPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXNA. VERTICAL SURVEY METHOD: DIGITAL LEVELING
- 5. UNIT OF MEASURE: U.S. SURVEY FEET
- FIELD SURVEYS WERE PERFORMED BETWEEN APRIL, 2022 AND NOVEMBER, 2022.



THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

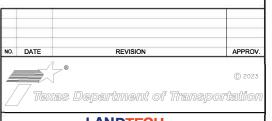


JACOB J. LUPHER

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E.



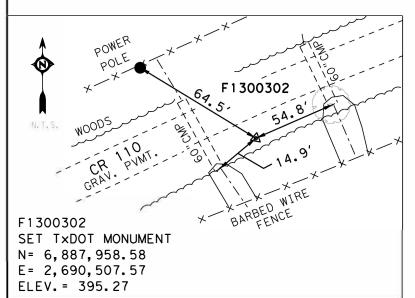
LANDTECH 2525 North Loop West, Suite 300, Houston, Texas 77008

Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131 TBPE Registration No. F-1364; TBPLS Registration No. 10019100

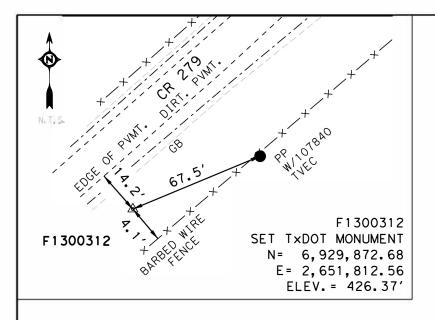
CR 110
OVER JONES CREEK
HORIZONTAL AND VERTICAL
CONTROL

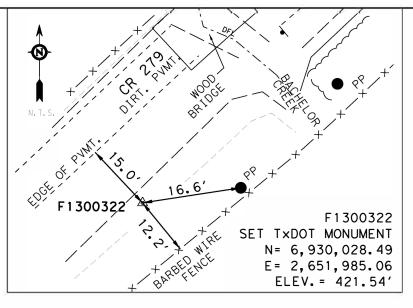
SHEET 1 OF 3

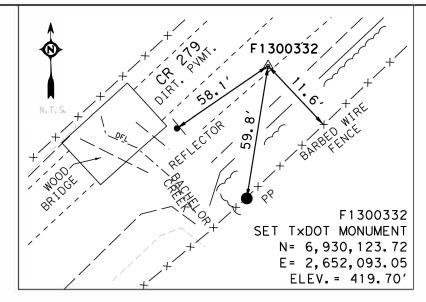
DN:	FED. RD. DIV. NO.	STATE	PROJE	CT NO.		HIGHWAY NO.
CK DN:	6	TEXAS	SEE TITI	LE SHE	ET	CR 110,ETC
DW:	STATE DIST.	COUNTY	CONTROL S	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	DAL	KAUFMAN	0918	11	100, ETC	35



FMAN CO BRIDGES\CADD\01_CR-110_Jones\MRF01_CR-110_TxDOT_PS&E_H&V.dgr







NOTE

- 1. ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202), NORTH AMERICAN DATUM OF 1983 (NAD 83), 2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED ADJUSTMENT FACTOR OF 1.00012.
- 2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY MFTHOD: TXDOT RTN
- 3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).
- 4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXNA. VERTICAL SURVEY METHOD: DIGITAL LEVELING
- 5. UNIT OF MEASURE: U.S. SURVEY FEET
- 6. FIELD SURVEYS WERE PERFORMED BETWEEN APRIL, 2022 AND NOVEMBER, 2022.



THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

Jacob Jopha

JACOB J. LUPH

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E.



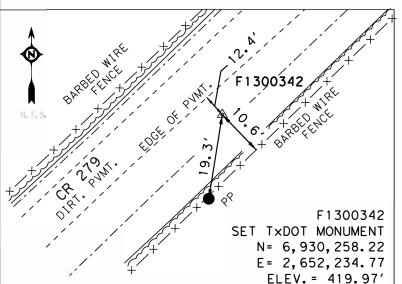
T: 713-861-7708 F: 713-861-4131 TBPE Registration No. F-1364; TBPLS Registration No. 10019100

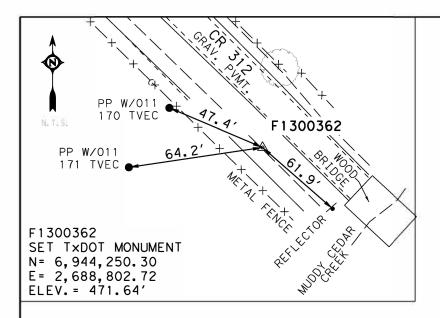
2525 North Loop West, Suite 300, Houston, Texas 77008

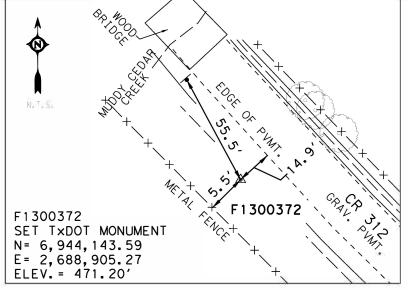
CR 279
OVER BACHELOR CREEK RELLEF
HORIZONTAL AND VERTICAL
CONTROL

SHEET 2 OF 3

DN:	FED.RD. DIV.NO.	STATE	PROJECT NO).	HIGHWAY NO.
CK DN:	6	TEXAS	SEE TITLE S		CR 110,ETC
DW:	STATE DIST.	COUNTY	CONTROL SECTION	ON JOB NO.	SHEET NO.
CK DW:	DAL	KAUFMAÑ	0918 11	100, ETC	36







- 1. ALL BEARINGS AND COORDINATES ARE BASED ON THE TEXAS COORDINATE SYSTEM, NORTH CENTRAL ZONE, (4202), NORTH AMERICAN DATUM OF 1983 (NAD 83), 2011 ADJUSTMENT, EPOCH 2010.00. ALL DISTANCES AND COORDINATES SHOWN ARE SURFACE VALUES AND MAY BE CONVERTED TO GRID BY DIVIDING BY THE COMBINED ADJUSTMENT FACTOR OF 1.00012.
- 2. HORIZONTAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXKU. HORIZONTAL SURVEY METHOD: TXDOT RTN
- 3. ALL ELEVATIONS HEREON ARE REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).
- 4. VERTICAL CONTROL WAS DERIVED FROM TXDOT RTN (VRS) GPS OBSERVATIONS BASED ON TXDOT REGIONAL REFERENCE POINT TXNA. VERTICAL SURVEY METHOD: DIGITAL LEVELING
- 5. UNIT OF MEASURE: U.S. SURVEY FEET
- 6. FIELD SURVEYS WERE PERFORMED BETWEEN APRIL, 2022 AND NOVEMBER, 2022.



THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.

Aged John

JACOB J. LUPHER

REGISTERED PROFESSIONAL LAND SURVEYOR TEXAS REGISTRATION NO. 6606

1/30/2023

THIS SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E.



2525 North Loop West, Suite 300, Houston, Texas 77008 T: 713-861-7068 F: 713-861-4131

T: 713-861-7068 F: 713-861-4131
TBPE Registration No. F-1364; TBPLS Registration No. 10019100

CR 312
OVER MUDDY CEDAR CREEK TRIB
HORIZONTAL AND VERTICAL
CONTROL

ı			
SHEET	マ	OF	3

DN:	FED. RD. DIV. NO.	STATE	PROJE	HIGHWAY NO.		
CK DN:	6	TEXAS	SEE TIT	CR 110,ETC		
D W:	STATE DIST.	COUNTY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	DAL	KAUFMAN	0918	11	100,ETC	37

S:\2022\2220056-KAUFMAN CO BRIDGES\CADD\11_CR-312_Muddy\MRF11_CR-312_TXDOT_PS&E_H&V.dgn \$PLOTTER\$

Curve Data

	*	*		
1111+38.48	N	6,887,789.0936	E	2,689,974.3388
1° 04′ 05.21"	(LT)			
2° 34′ 13.69"				
20.7772				
41.5533				
2,229.0000				
41.5527				
0.0968				
1111+17.71	N		E	2,689,953.6553
1111+59.26	N		E	2,689,994.9821
	N	6,890,006.0753	E	2,689,742.2515
84° 01′ 25.29" E				
	1111+38.48 1° 04' 05.21" 2° 34' 13.69" 20.7772 41.5533 2,229.0000 0.0968 41.5527 0.0968 1111+17.71 1111+59.26	1111+38.48 N 1° 04′ 05.21" (LT) 2° 34′ 13.69" 20.7772 41.5533 2,229.0000 0.0968 41.5527 0.0968 1111+17.71 N 1111+59.26 N N 84° 33′ 27.90" E 83° 29′ 22.68" E	1111+38.48 N 6,887,789.0936 1° 04' 05.21" (LT) 2° 34' 13.69" 20.7772 41.5533 2,229.0000 0.0968 41.5527 0.0968 1111+17.71 N 6,887,787.1230 1111+59.26 N 6,887,791.4493 N 6,890,006.0753	1111+38.48 N 6,887,789.0936 E 1° 04' 05.21" (LT) 2° 34' 13.69" 20.7772 41.5533 2,229.0000 0.0968 41.5527 0.0968 1111+17.71 N 6,887,787.1230 E 1111+59.26 N 6,887,791.4493 E N 6,890,006.0753 E 84° 33' 27.90" E 83° 29' 22.68" E

Course from PT EX*CR110*1 to PC EX*CR110*2 N 83° 29' 22.68" E Dist 12.9622

Curve Data

				*	*		
Curve EX*CR	110*2						
P.I. Statio	on		1112+09.29	N	6,887,797.1220) E	2,690,044.6901
Delta	=	1°	54′ 19.77"	(LT)			
Degree	=	2°	34′ 13.69"				
Tangent	=		37.0684				
Length	=		74.1301				
Radius	=		2,229.0000				
External	=		0.3082				
Long Chord	=		74.1266				
Mid. Ord.	=		0.3082				
P.C. Statio	on		1111+72.22	N	6,887,792.9190	ЭЕ	2,690,007.8607
P.T. Statio	on		1112+46.35	N	6,887,802.5472	2 E	2,690,081.3594
C.C.				N	6,890,007.5450	ЭЕ	2,689,755.1301
Back	= N 8	3° 29	9′ 22.68" E				
Ahead	= N 8	1° 35	5′ 02.91" E				
Chord Bear	= N 82	2° 32	2′ 12.80" E				

Course from PT EX*CR110*2 to PC P*CR110*1 N 81° 35′ 02.91" E Dist 45.3778

Curve Data

Curve P*CR11	0 * 1							
P.I. Statio	n	1.1	13+41.87	N	6,887,8	16.5270	E	2,690,175.8499
Delta	=	16° 18	19.91"	(LT)				
Degree	=	16° 22	12.80"					
Tangent	=		50.1413					
Length	=		99.6048					
Radius	=		350.0000					
External	=		3.5734					
Long Chord	=		99.2690					
Mid. Ord.	=		3.5373					
P.C. Statio	n	1.1	12+91.73	N	6,887,8	09.1885	E	2,690,126.2485
P.T. Statio	n	1.1	13+91.33	N	6,887,8	37.4964	E	2,690,221.3958
C.C.				N	6,888,1	55.4197	E	2,690,075.0237
Back	= N 81	° 35′	02.92" E					
Ahead	= N 65		43.00" E					
Chord Bear	= N 73	° 25′	52.96" E					

Course from PT P*CR110*1 to PC P*CR110*2 N 65° 16′ 43.00" E Dist 67.7468

Curve Data

		*	*		
Curve P*CR110*2					
P.I. Station	1114+72.38	N	6,887,871.3899	E	2,690,295.0131
Delta =	4° 21′ 06.32"	(LT)			
Degree =	16° 22′ 12.80"				
Tangent =	13.2981				
Length =	26.5834				
Radius =	350.0000				
External =	0.2525				
Long Chord =	26.5770				
Mid. Ord. =	0.2524			_	
P.C. Station	1114+59.08	N		E	2,690,282.9338
P.T. Station	1114+85.66	N	6,887,877.8518	E	2,690,306.6356
C.C.		N	6,888,183.7518	E	2,690,136.5616
Back = N	65° 16′ 43.00" E				
Ahead = N	60° 55′ 36.68" E				
Chord Bear = N	63° 06′ 09.84" E				

Course from PT P*CR110*2 to PC P*CR110*3 N 60° 55′ 36.68" E Dist 19.4643

CR 110 @ JONES CREEK (Q CR110) CONT'D

Curve Data

Curve P*CR1	10×3					
P.I. Statio	on	1115+20	D. 81 N	6,887,895.0044	1 E	2,690,337.3062
Delta	=	5° 07′ 45.	40" (RT)			
Degree	=	16° 22′ 12.	. 80"			
Tangent	=	15.6	5770			
Length	=	31.	3330			
Radius	=	350.0	0000			
External	=	0.	3509			
Long Chord	=	31.3	3225			
Mid. Ord.	=	0.	3506			
P.C. Statio	on	1115+05	5.13 N	6,887,887.3100) E	2,690,323.6474
P.T. Statio	on	1115+36	5.46 N	6,887,901.4469) E	2,690,351.5982
C.C.			N	6,887,582.3668	3 E	2,690,495.4312
Back	= N	60° 36′ 21.96	5" E			
Ahead	= N	65° 44′ 07.35				
Chord Bear	= N	63° 10′ 14.66	5" E			

Course from PT P*CR110*3 to 11008 N 65° 24′ 52.63" E Dist 99.2630

Point 11008 N 6,887,942.7451 E 2,690,441.8622 Sta 1116+35.72

Course from 11008 to PC EX*CR110*5 N 64° 53′ 38.74" E Dist 76.5015

Curve Data

Curve EX*CR	110*5						
P.I. Statio	on	1	117+36.53	N	6,887,985.5148	E	2,690,533.1412
Delta	=	3° 1:	2′ 26.27"	(RT)			
Degree	=	6° 3	6′ 03.23"				
Tangent	=		24.3008				
Length	=		48.5888				
Radius	-		868.0000				
External	=		0.3401				
Long Chord	=		48.5825				
Mid. Ord.	=		0.3400				
P.C. Statio			117+12.23	N	6,887,975.2041	E	2,690,511.1363
P.T. Statio	on	1	117+60.82	N	6,887,994.5781	E	2,690,555.6886
C. C.				N	6,887,189.2084	E	2,690,879.4224
Back		64° 53′	38.74" E				
Ahead			05.02" E				
Chord Bear	= N 6	66° 29′	51.88" E				

Course from PT EX*CR110*5 to 11010 N 68° 06′ 05.02" E Dist 66.3939

N 6,888,019.3407 E 2,690,617.2918 Sta 1118+27.21

Ending chain P*CR110 description



SCALE: N.T.S.

NO.	DATE	REVISION	APPROV.
4	Tex	xas Department of Transpor	© 2023 tation



ALIGNMENT DATA CR 110

SHEET 1 OF 3

51111	01 3							
DN:	RM	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
DW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	38

Beginning chain P*CR279 description

Point 27901 N 6,929,877.6278 E 2,651,787.7816 Sta 2279+00.00

Course from 27901 to PC P*CR279*10 N 48° 30′ 20.39" E Dist 97.9632

Curve Data

		*	· *		
Curve P*CR279*10					
P.I. Station	2280+25.54	N	6,929,960.8025	Ε	2,651,881.8122
Delta =	1° 59′ 58.89"	(RT)			, ,
Degree =	3° 37′ 34.73"				
Tangent =	27.5748				
Length =	55.1439				
Radius =	1,580.0000				
External =	0.2406				
Long Chord =	55.1411				
Mid. Ord. =	0.2406				
P.C. Station	2279+97.96	N	-,,	Ε	2,651,861.1582
P.T. Station	2280+53.11	N		Ε	2,651,903.0912
C.C.		N	6,928,759.0795	Ε	2,652,907.9809
Back = N	48° 30′ 20.39" E				
Ahead = N	50° 30′ 19.28" E				
Chord Bear = N	49° 30′ 19.84" E				

Course from PT P*CR279*10 to PC P*CR279*11 N 50° 30′ 19.28" E Dist 40.2229

Curve Data

				*			
Curve P*CR2	79×11						
P.I. Statio	on		2281+33.99	N	6,930,029,7841	Ε	2,651,965,5095
Delta	=	2°	56′ 54.52"	(LT)	•		• •
Degree	=	3°	37' 34.73"				
Tangent	=		40.6629				
Length	=		81.3079				
Radius	=		1,580.0000				
External	=		0.5232				
Long Chord	=		81.2989				
Mid. Ord.	=		0.5230				
P.C. Statio	on		2280+93.33	N	6,930,003.9223	Ε	2,651,934.1306
P.T. Statio	on		2281+74.64	N	6,930,057.2258	Ε	2,651,995.5166
C.C.				N	6,931,223.1830	Ε	2,650,929.2410
Back	= N	50° 30	′ 19.28" E				
Ahead	= N	47° 33	′ 24.76" E				
Chord Bear	= N	49° 01	′ 52.02" E				

Course from PT P*CR279*11 to PC P*CR279*12 N 47° 33′ 24.76" E Dist 88.0329

Curve Data

279*12						
ion		2283+05.32	N	6,930,145.4160	Ε	2,652,091.9514
=	3°	05′ 32.14"	(LT)	• •		
=	3°	37′ 34.73"				
=		42.6468				
=		85.2728				
=		1,580.0000				
=		0.5754				
=		85.2625				
=		0.5752				
ion		2282+62.67	N	6,930,116.6355	E	2,652,060.4803
ion		2283+47.94	N	6,930,175.8523	Ε	2,652,121.8241
			N	6,931,282.5927	E	2,650,994.2047
= N	47° 3	3′ 24.76" E				
= N	44° 2	7′ 52.62" E				
= N	46° 0	0′ 38.69" E				
	= = = = = = = = = = = = = = = = = = =	ion = 3° = 3° = 3° = = = = = = = = = = = = = = = = = = =	2283+05.32 = 3° 05' 32.14" = 3° 37' 34.73" = 42.6468 = 85.2728 = 0.5752 = 0.5754 = 85.2625 0.5752 ion 2282+62.67 ion 2283+47.94 = N 47° 33' 24.76" E = N 44° 27' 52.62" E	283+05.32 N = 3° 05′ 32.14" (LT) = 3° 37′ 34.73" = 42.6468 = 85.2728 = 1,580.0000 = 0.5754 = 85.2625 = 0.5752 ion 2282+62.67 N ion 2283+47.94 N = N 47° 33′ 24.76" E = N 44° 27′ 52.62" E	Tion 2283+05.32 N 6,930,145.4160 = 3° 05′ 32.14" (LT) = 3° 37′ 34.73" = 42.6468 = 85.2728 = 1,580.0000 = 0.5754 = 85.2625 = 0.5752 ion 2282+62.67 N 6,930,116.6355 ion 2283+47.94 N 6,930,175.8523 = N 47° 33′ 24.76" E = N 44° 27′ 52.62" E	Tion 2283+05.32 N 6,930,145.4160 E = 3° 05′ 32.14″ (LT) = 3° 37′ 34.73″ = 42.6468 = 85.2728 = 1,580.0000 = 0.5754 = 85.2625 ion 2282+62.67 N 6,930,116.6355 E ion 2283+47.94 N 6,930,175.8523 E = N 47° 33′ 24.76″ E = N 44° 27′ 52.62″ E

Course from PT P*CR279*12 to PC P*CR279*13 N 44° 27′ 52.62" E Dist 18.6508

Curve Data

		*	*		
Curve P*CR279*13					
P.I. Station	2284+08.17	N	6,930,218.8375	E	2,652,164.0134
Delta =	3° 00′ 53.66"	(RT)			
Degree =	3° 37′ 34.73"				
Tangent =	41.5794				
Length =	83.1396				
Radius =	1,580.0000				
External =	0.5470				
Long Chord =	83.1300				
Mid. Ord. =	0.5468			_	
P.C. Station	2283+66.59	N	6,930,189.1630	E	2,652,134.8884
P.T. Station	2284+49.73	N	6,930,246.9391	E	2,652,194.6589
C. C.		N	6,929,082.4225	E	2,653,262.5078
Back = N	44° 27′ 52.62" E				
Ahead = N	47° 28′ 46.27" E				
Chord Bear = N	45° 58′ 19.45" E				

Course from PT P*CR279*13 to 27908 N 47° 28′ 46.27" E Dist 87.4240

N 6,930,306.0250 E 2,652,259.0935 Sta 2285+37.16

Ending chain P*CR279 description

SCALE: N.T.S.

NO.	DATE	REVISION	APPROV.
4		•	© 2023
4	Tex	ras Denartment of Transnor	tation



ALIGNMENT DATA CR 279

SHEET 2 OF 3

SHEET Z	. Or J	1						
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
OW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	39

Point 31201 N 6,944,435.0984 E 2,688,646.4390 Sta 312+00.00

Course from 31201 to PC EX*CR312*01 S 45° 59′ 59.40" E Dist 27.9916

Curve Data

				*	*		
Curve EX*CR3	312×01						
P.I. Statio	on		312+98.56	N	6,944,366.6304	E	2,688,717.3393
Delta	=	0°	48′ 31.24"	(RT)			
Degree	=	0°	34' 22.65"				
Tangent	=		70.5717				
Length	=		141.1410				
Radius	=	1	0,000.0000				
External	=		0.2490				
	=		141.1399				
Mid. Ord.	=		0.2490				
P.C. Statio			312+27.99	N	6,944,415.6538	E	2,688,666.5744
P.T. Statio	on		313+69.13	N	6,944,316.8955		2,688,767.4072
C.C.				N	6,937,222.2760	E	2,681,719.9698
Back			0′ 59.40" E				
Ahead		45° 11					
Chord Bear	= S 4	45° 35	6′ 43.78" E				

Course from PT EX*CR312*01 to PC EX*CR312*02 S 45° 11′ 28.16" E Dist 104.6375

Curve Data

			*			
Curve EX*CR31	2*02					
P.I. Station		314+82.29	N	6,944,237,1454	E	2,688,847,6912
Delta =	0°	37′ 05.61"	(RT)			
Degree =	3°	37′ 34.73"				
Tangent =		8.5242				
Length =		17.0483				
Radius =		1,580.0000				
External =		0.0230				
Long Chord =		17.0482				
Mid. Ord. =		0.0230				
P.C. Station		314+73.77	N	6,944,243.1528	E	2,688,841.6436
P.T. Station		314+90.82	N	6,944,231.0731	E	2,688,853.6736
C.C.			N	6,943,122.2029	E	2,687,728.1485
		1′ 28.16" E				
		34′ 22.55" E				
Chord Bear =	S 44° 5	52′ 55.35" E				

Course from PT EX*CR312*02 to PC EX*CR312*03 S 44° 34′ 22.55" E Dist 56.6444

Curve Data

312*03					
n	315+56.72	N	6,944,184.1273	E	2,688,899.9248
= 3°	32′ 06.12"	(RT)			•
= 19°	05′ 54.94"				
=	9.2576				
=	18.5094				
Ξ.	300.0000				
=	0.1428				
=	18.5065				
=	0.1427				
n	315+47.46	N	6,944,190.7221	E	2,688,893.4276
n	315+65.97	N		E	2,688,906.0029
		N	6,943,980.1771	E	2,688,679.7203
= S 44° 3	34′ 22.55" E				
= S 41° C	02′ 16.42" E				
= S 42° 4	18′ 19.48" E				
	= 3° = 19° = = = = = = = = = = = = = = = = = = =	315+56.72 = 3° 32′ 06.12" = 19° 05′ 54.94" = 9.2576 = 18.5094 = 300.0000 = 0.1428 = 18.5065 = 0.1427 on 315+47.46 on 315+65.97 = \$ 44° 34′ 22.55" E = \$ 41° 02′ 16.42" E	315+56.72 N 315+65.97 N	315+56.72 N 6,944,184.1273 = 3° 32′ 06.12" (RT) = 19° 05′ 54.94" = 9.2576 = 18.5094 = 300.0000 = 0.1428 = 18.5065 = 0.1427 on 315+47.46 N 6,944,190.7221 on 315+65.97 N 6,944,177.1445 N 6,943,980.1771 = \$ 44° 34′ 22.55" E = \$ 41° 02′ 16.42" E	315+56.72 N 6,944,184.1273 E 3° 32′ 06.12" (RT) 19° 05′ 54.94" 9.2576 18.5094 300.0000 0.1428 18.5065 0.1427 10. 315+47.46 N 6,944,190.7221 E 315+65.97 N 6,944,177.1445 E 8 44° 34′ 22.55" E 8 41° 02′ 16.42" E

Course from PT EX*CR312*03 to PC EX*CR312*04 S 41° 02′ 16.42" E Dist 41.2260

Curve Data

		*	*		
Curve EX*CR312*	04				
P.I. Station	316+35.58	N	6,944,124.6395	E	2,688,951.7058
Delta =	1° 25′ 12.92"	(LT)			
Degree =	2° 30′ 07.20"				
Tangent =	28.3839				
Length =	56.7648				
Radius =	2,290.0000				
External =	0.1759				
Long Chord =	56. 7633				
Mid. Ord. =	0.1759			_	
P.C. Station	316+07.20	N		E	2,688,933.0702
P.T. Station	316+63.96	N	6,944,103.6987	Ε	2,688,970.8664
C.C.		N	6,945,649.5667	E	2,690,660.3611
Back = S					
Ahead = S					
Chord Bear = S	41° 44′ 52.88" E				

Course from PT EX*CR312*04 to PC EX*CR312*05 S 42° 27' 29.34" E Dist 18.8290

Curve EX*CR312*05 317+36.57 N 2° 41′ 26.89" (LT) 2° 30′ 07.20" 53.7829 107.5461 2,290.0000 0.6315 107.5362 0.6313 316+82.79 N P.I. Station Delta = 6,944,050.1278 E 2,689,019.8832 Degree Tangent Length Radius External Long Chord = Mid. Ord. = P.C. Station P.T. Station 6,944,089.8072 E 6,944,012.1965 E 6,945,635.6752 E 2,688,983.5770 317+90.34 N 2,689,058.0122 2,690,673.0716 Back = S 42° 27′ 29.34" E Ahead = S 45° 08′ 56.23" E Chord Bear = S 43° 48′ 12.78" E

CR 312 @ MUDDY CEDAR CREEK TRIBUTARY (C CR312) CONT'D

Course from PT EX*CR312*05 to 31207 S 45° 08′ 56.23" E Dist 47.5007

Point 31207 N 6,943,978.6959 E 2,689,091.6875 Sta 318+37.84

------Ending chain EX*CR312 description



SCALE: N.T.S.

NO.	DATE	REVISION	APPROV.					
4		├ ®	© 2023					
1	Texas Department of Transportation							

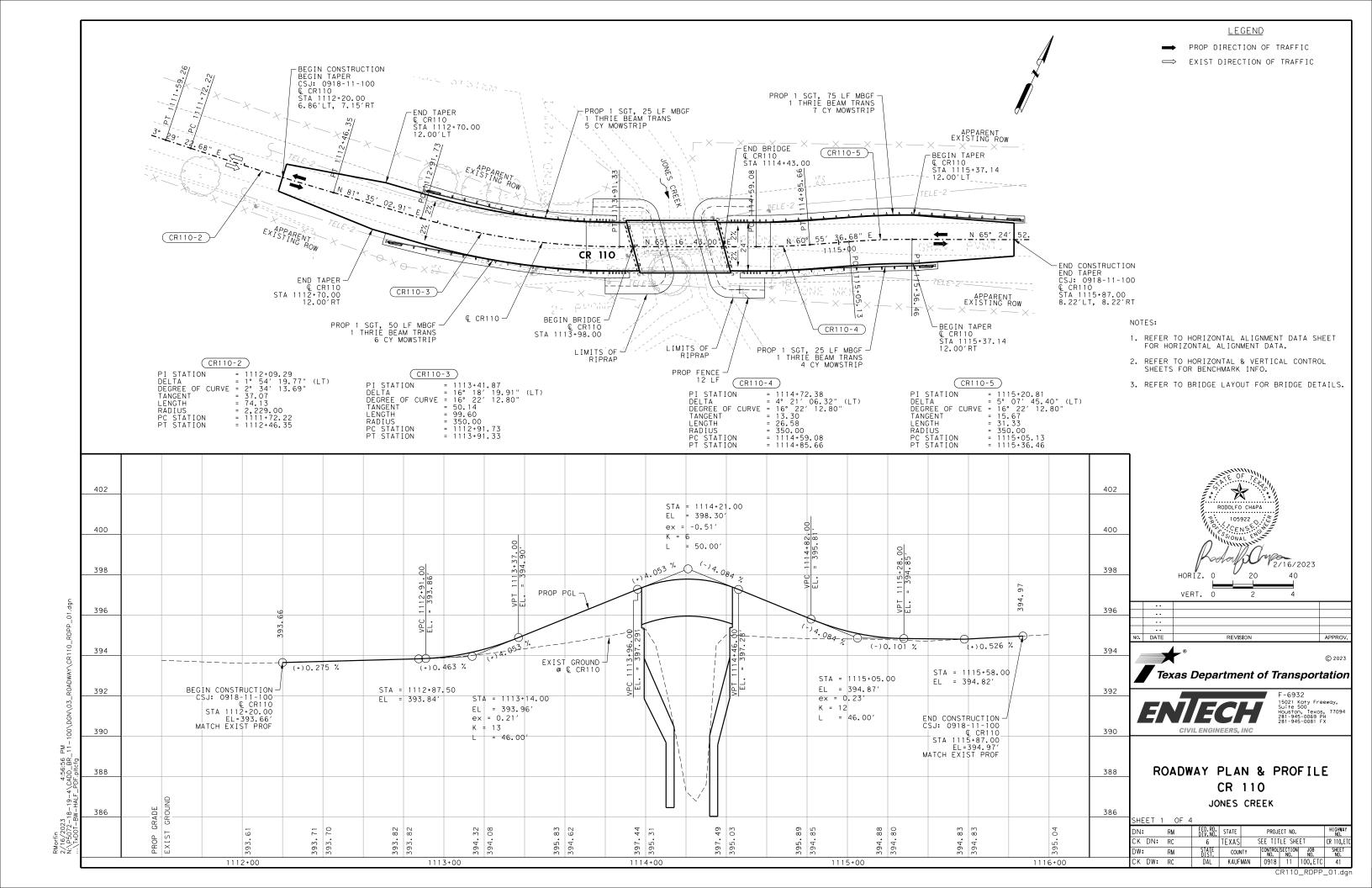


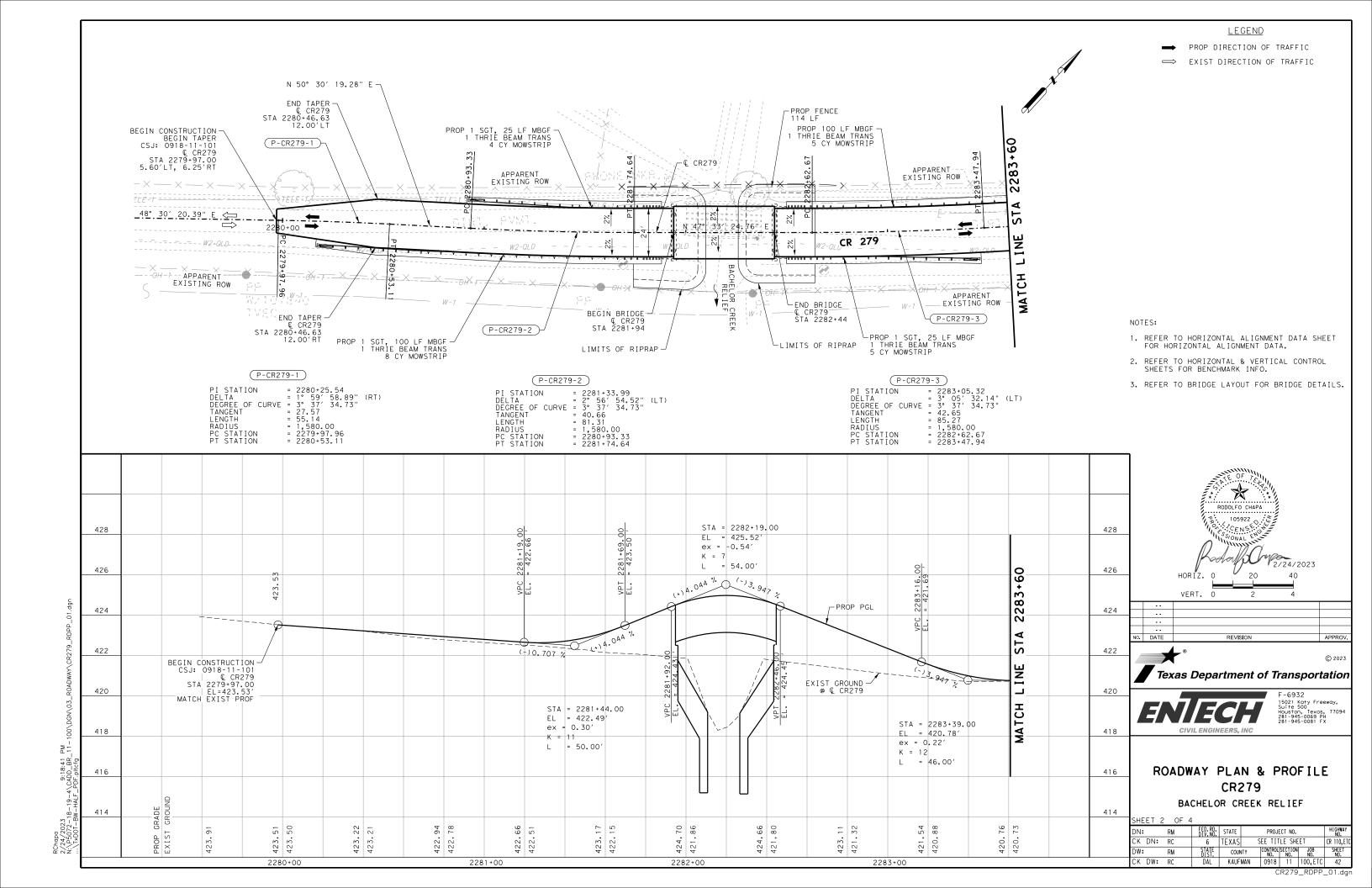
F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

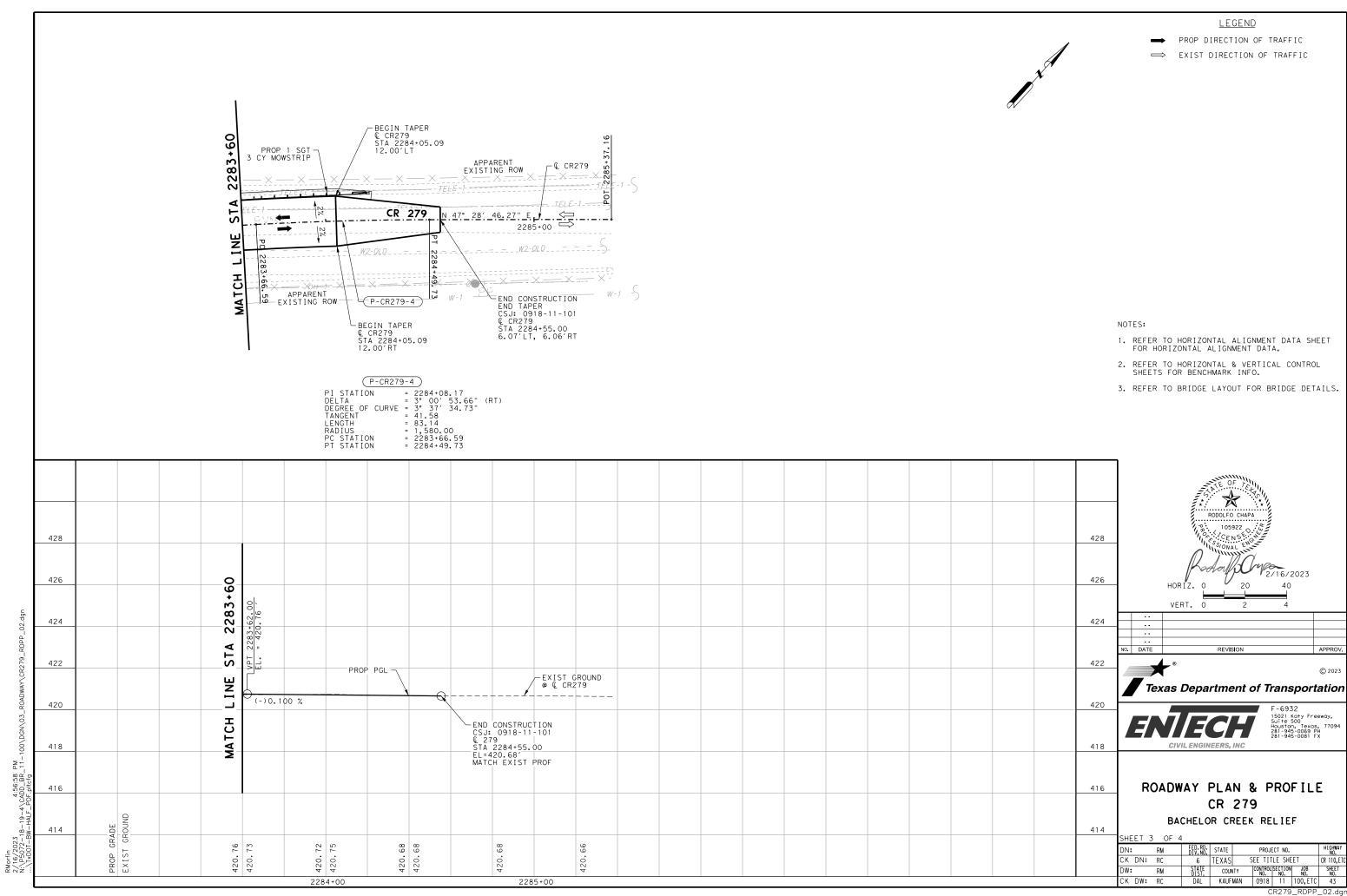
ALIGNMENT DATA CR 312

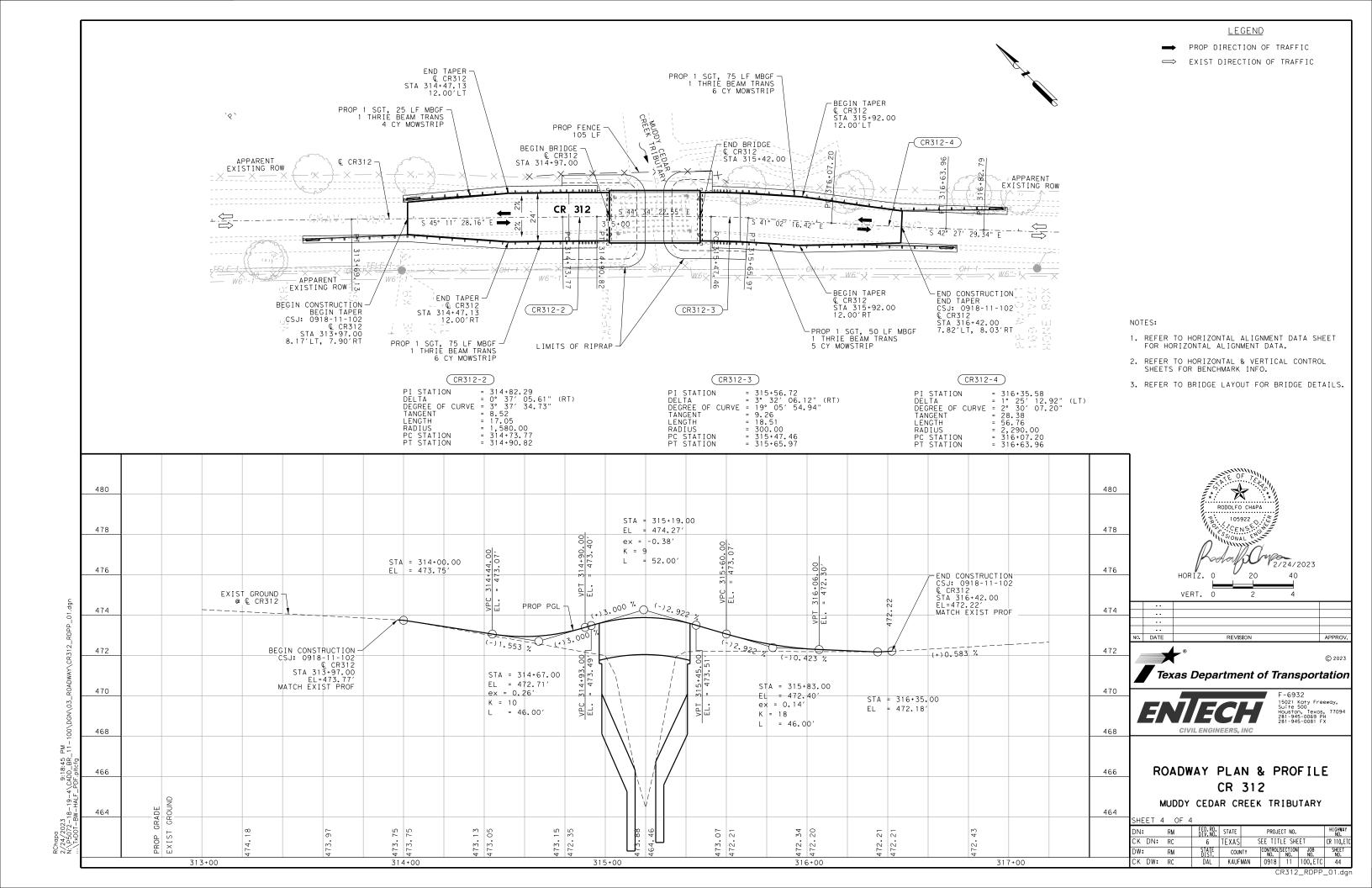
SHEET 3 OF 3

JIILL 1 3	, 01 3							
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S	EE TIT	LE SHE		CR 110,ETC
OW:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUF	MAN	0918	11	100.ETC	40



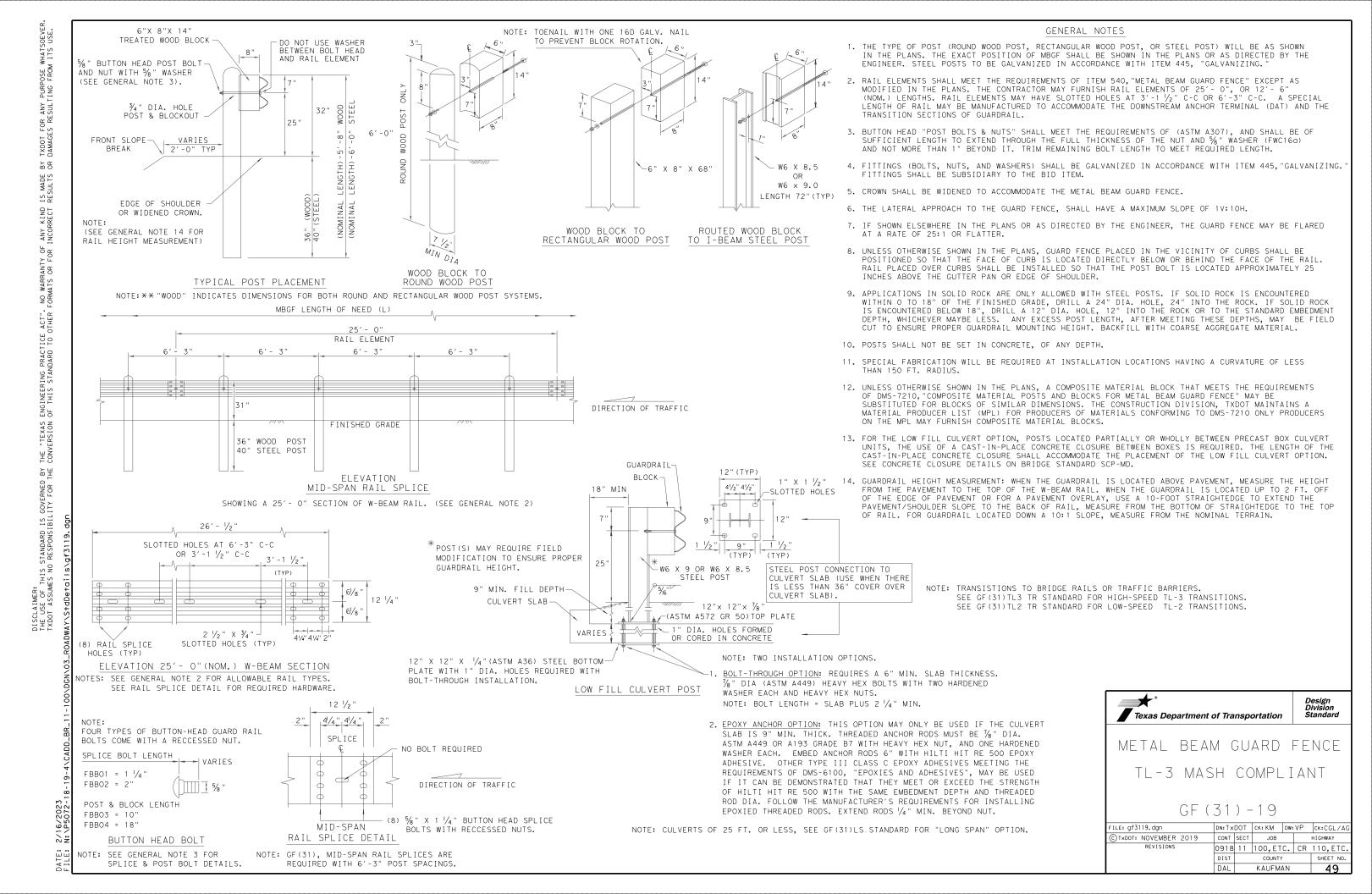






RM

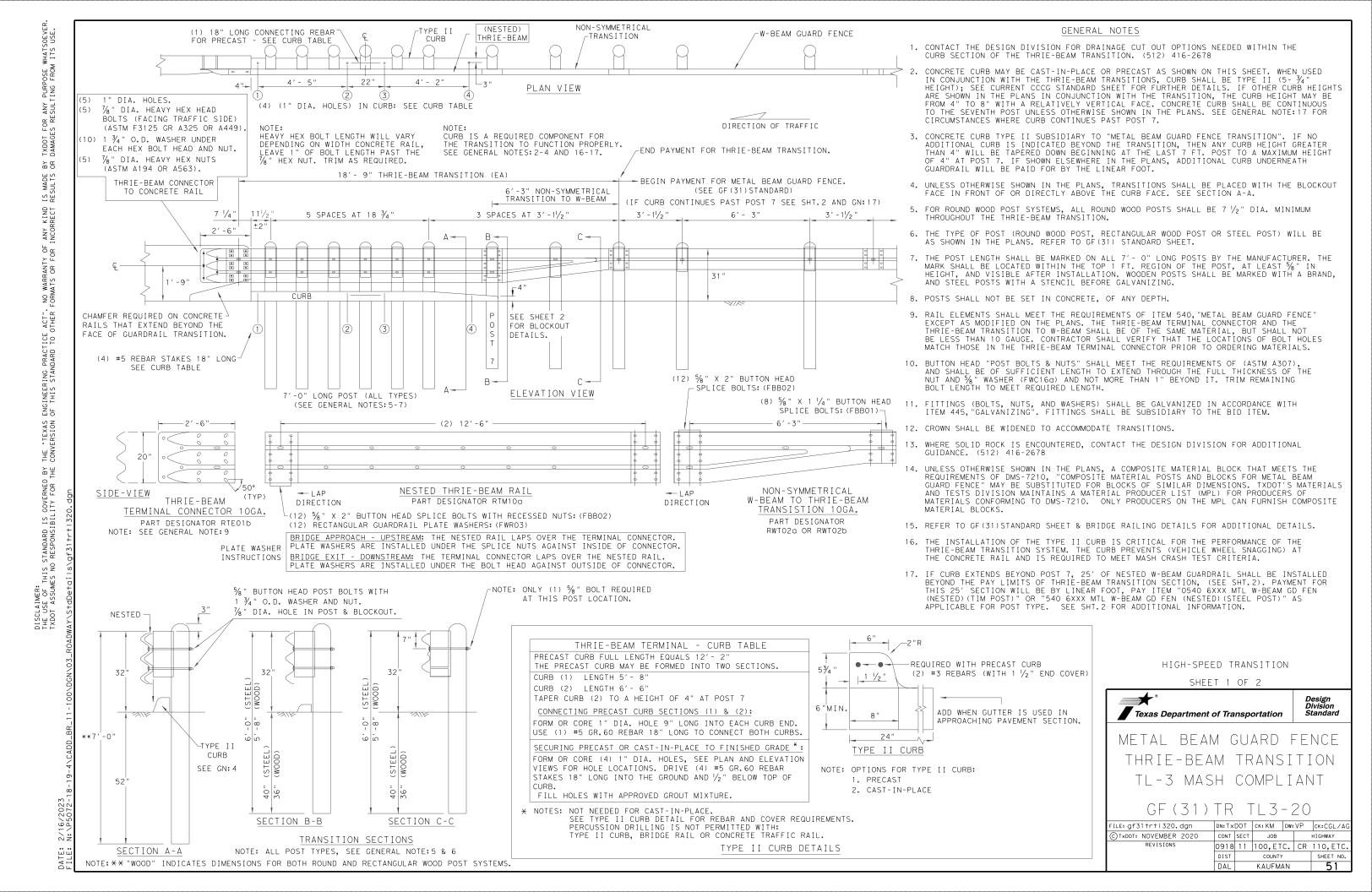
Fnd of



KAUFMAN

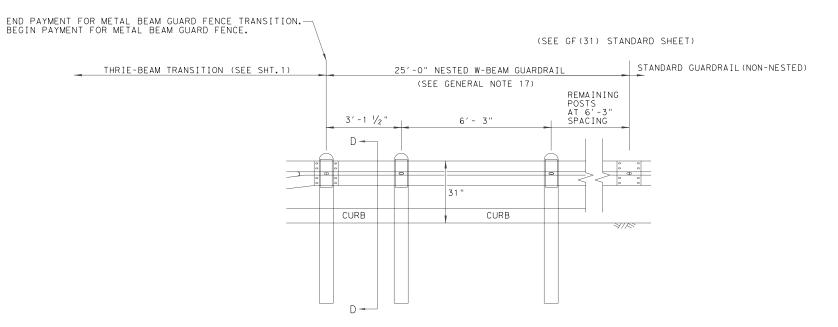
Curb shown on top of mow strip

embedment throughout the system.

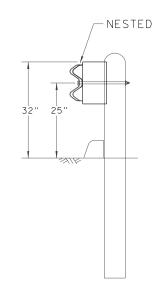


DATE: 2/16/2023 FILE: N: \P5072-

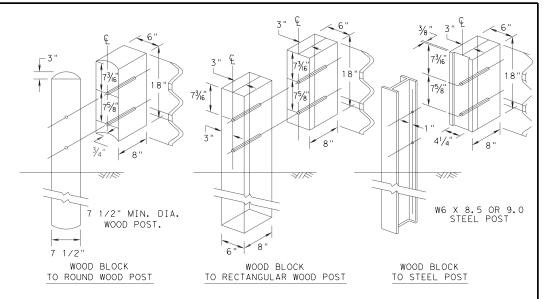
REQUIRED ALTERNATIVE FOR CONTINUOUS CURB EXTENDING PAST POST 7 (SEE SHT. 1 GENERAL NOTE 17)



ELEVATION VIEW



SECTION D-D



THRIE BEAM TRANSITION BLOCKOUT DETAILS

HIGH-SPEED TRANSITION

SHEET 2 OF 2



METAL BEAM GUARD FENCE THRIE-BEAM TRANSITION TL-3 MASH COMPLIANT

GF(31)TR TL3-20

ILE: gf31trtl320.dgn	DN: Tx	DOT	ck: KM	DW:	KM	ck:CGL/AG
TxDOT: NOVEMBER 2020	CONT	SECT	JOB			HIGHWAY
REVISIONS	0918	11	100,ET	С.	CR	110,ETC.
	DIST		COUNTY			SHEET NO.
	DAI		ΚΔΠΕΜΔ	N		52

NOTE: STEEL I-BEAM POST W6 X 8.5 (6'-0") PN:533G STANDARD WOOD BLOCKOUTS (6"X8"X14") PN:4076 5/8" X 10" HGR BOLT PN: 3500G LINE AT THE BACK OF POST #2 THRU #8 FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: TRINITY HIGHWAY AT 1 (888) 323-6374. 2525 N. STEMMONS FREEWAY, DALLAS, TX 75207 FROM THE CENTERLINE OF POST(1) & POST(0) HGR NUT PN: 3340G AT (POSTS 2 THRU 8) ANCHOR PADDLE ANGLE STRUT-PN: 15204A-2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE; SOf+Stop END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL. PN:620237B PN: 15202G 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD. POST (8) POST (7: POST (6) POST(5) POST (3) ANCHOR RAIL TO - POST (2) DETAIL 1 POST(0) PLAN VIEW BEGIN LENGTH OF NEED MASH TEST LEVEL 3 (TL-3) LENGTH OF SoftStop TERMINAL (50'-9 1/2") TRAFFIC FLOW 4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD. 50'-9 1/2" STANDARD INSTALLATION LENGTH (MASH TL-3 SoftStop) 5. HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. END PAYMENT FOR SGT BEGIN STANDARD 6. A COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS. ANCHOR RAIL WITH SLOTS - (THREADED THRU HEAD) SEE SoftStop MANUAL FOR COMPLETE DETAILS MBGF δγ MIDDLE SLOT CUTOUT OUTSIDE SLOTS CUTOUT— (1) 1 3/4" X 6'-10 1/4" (2)1/2" X 6'-9 5/8" 7. IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL AND REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE. made sults - SoftStop FACE SEE GN(3) MBGF LAPPED IN DIRECTION OF TRAFFIC FLOW 8. POSTS SHALL NOT BE SET IN CONCRETE. 25'-0" DOWNSTREAM W-BEAM GUARDRAIL PN:61G SoftStop ANCHOR RAIL (12GA) PN: 15215G & NOTE:B IT IS ACCEPTABLE TO INSTALL THE SOFTSTOP IMPACT HEAD PARALLEL TO THE GRADE LINE OR WITH AN UPWARD TILT. kind rect 3'-1 1/2"(+/-) ANCHOR PADDLE 10. DO NOT ATTACH THE SoftStop SYSTEM DIRECTLY TO A RIGID BARRIER. -PN: 15204A SEE NOTE: C END OF 11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE SOftStop SYSTEM BE CURVED. ANCHOR RAIL
PN: 15215G POST 32 12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR DO NOT BOLT ANCHOR RAIL TO RAIL 25'-0" _RAIL 25'-0" SEE A HEIGHT SEE DETAIL 2 PN: 15215G ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER. POST(2) RAIL HEIGHT RAIL HEIGHT NOTE: A THE INSTALLATION HEIGHT OF FULLY ASSEMBLED ANCHOR POST WILL 13/16" DIA.-- ¹³/₁₆" DIA. (8) 5/8" x 1 - 1/4" HGR BOLTS VARY FROM 3-3/4" MIN. TO 4" MAX. ABOVE FINISHED GRADE. ∠(8) 5/8"× 1- 1/4" GR BOLTS YIELDING YIELDING HOLES HOLES PN: 3360G NOTE: B PART PN: 5852B RIGHT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING) PN: 3360G DEPTH HEX NUTS PART PN:5851B LEFT-SIDE (HIGH INTENSITY REFLECTIVE SHEETING) %" HEX NUTS PN: 3340G (TYP 1-8) SEE SEE DETAIL 3 PN: 3340G 6'-1% NOTE: C W-BEAM SPLICE LOCATED BETWEEN LINE POST (4) AND LINE POST (5) GUARDRAIL PANEL 25'-0" PN: 61G POST (8 POST(5) POST(4) POST(3) POST(2) POST(1) 6'-0" (SYTP) ANCHOR RAIL 25'-0" PN: 15215G HARDWARE FOR POST(2) THRU POST(8) ELEVATION VIEW PN: 15000G PN: 15203G LAP GUARDRAIL IN DIRECTION OF TRAFFIC FLOW. (1) %"× 10" HGR BOLT PN: 3500G (1) \(\frac{1}{8} \)" HGR HEX NUT PN: 3340G ANGLE STRUT MAIN SYSTEM COMPONENTS (1) $\frac{5}{8}$ " × 1 $\frac{3}{4}$ ". -PN: 15202G NOTE: DO NOT BOLT ANCHOR RAIL PANEL TO POST (2) POST(0) PRODUCT DESCRIPTION ASSEMBLY MANUAL (LATEST REV.) SoftStop HEAD (SEE MANUAL FOR RIGHT-LEFT APPROACH) PN 3391G ALTERNATE BLOCKOUT PN: 15205A 15215G 1 SoftStop ANCHOR RAIL (12GA) WITH CUTOUT SLOTS SEE GENERAL NOTE: 6 (2) 5/8" WASHERS SoftStop DOWNSTREAM W-BEAM RAIL (12GA) (25'- 0") 6" X 8" X 14 (1) 1/6 " HEX NUT ANCHOR PLATE WASHER 61G | 1 4" X 7 ½" X 14" BLOCKOUT COMPOSITE PN 4372G -BLOCKOUT -(1) ⁵/8 ' "Texas ersion '/2" THICK PN:15206G POST #0 - ANCHOR POST (6'- 5 1/8") HGR HEX NUT ANCHOR KEEPER WOOD -PN: 105286 15203G 1 POST #1 - (SYTP) (4'- 9 1/2") 1" ROUND WASHER F463 PN: 4902G -PN: 4076B PN 3340G PLATE (24 GA)-(2) % PN: 6777B NOTE:

DO NOT BOLT

ANCHOR RAIL TO 15000G POST #2 - (SYTP) (6'- 0") ROUND WASHERS PN: 15207G POST #3 THRU #8 - I-BEAM (W6 x 8.5) (6'- 0") DETAIL 1 PN: 3240G (2) \%6" x 2 \1/2" HEX HD BOLT GR-5 ΔΙ ΤΕΡΝΔΤΕ BLOCKOUT - WOOD (ROUTED) (6" x 8" x 14") 4076B SHOWN AT POST(1) - POST (2) BLOCKOUT BLOCKOUT WOOD -6" X 8" X 14" -BLOCKOUT WOOD BLOCKOUT - COMPOSITE (4" \times 7 $\frac{1}{2}$ " \times 14") W-BEAM RAIL NEAR GROUND PN: 105285G W-BEAM RAIL DETAIL 2 GENERAL NOTE: 6 15204A 1 ANCHOR PADDLE rned for † ANCHOR KEEPER PLATE (24 GA) 5/8" HGR NUT HGR POST BOLT SHOWN AT POST(1) PN: 3340G PN: 3500G 15206G 1 ANCHOR PLATE WASHER (1/2 " THICK) (2) %6 " ROUND WASHER -HGR POST BOLT PN: 3500G HGR POST BOLT ANCHOR POST ANGLE (10" LONG) (WIDE) PN: 3240G-PN: 3500G ·% " HGR NUT 5% " HGR NUT PN: 3340G HARDWARE POST 32' ANCHOR PADDLE--1" NUT PN:3908G SHALL BE SECURELY TIGHTENED HEIGHT HEIGHT 31" RAIL 31" RAIL (2) % " HEX N A563 GR.DH " HEX NUT-4902G 1" ROUND WASHER F436 13/6"DIAMETER YIELDING HOLES HEIGHT AFTER FINAL ASSEMBLY LOCATED IN FLANGES BUT NOT DEFORMING THE 3908G 1" HEAVY HEX NUT A563 GR.DH W-BEAM FLATTENED KEEPER PLATE. ¾" × 2 ½" HEX BOLT A325 (4 PLIES) this (3701G 4 3/4" ROUND WASHER F436 POST 17" - 1/2 — SEE NOTE: A (HOLES APROXIMATELY CENTERED AT FINISHED GRADE) HEIGHT 3704G 2 34" HEAVY HEX NUT A563 GR. DH FINISHED FINISHED PN: 15202G FINISHED 3360G 16 $\frac{5}{8}$ " × 1 $\frac{1}{4}$ " W-BEAM RAIL SPLICE BOLTS HGR GRADE GRADE GRADE 3340G 25 58" W-BEAM RAIL SPLICE NUTS HGR 13/16" DIA. 3500G " x 10" HGR POST BOLT A307 (2) 3/4" × 2 1/2" HEX BOLT (TYP) PN: 3717G YIELDING 3391G 1 5/8" × 1 3/4" HEX HD BOLT A325 9 1/2" 5/8" × 9" HEX HD BOLT A325 5/8" WASHER F436 LINE POST POST(2) 4489G (3, 4, 5, 6, 7 & 8) (4) 3/4" FLAT WASHER 4372G 4 (TYP) PN: 3701G $\frac{1}{6}$ " \times 2 $\frac{1}{2}$ " HEX HD BOLT GR-5 105285G $\frac{1}{6}$ " \times 1 $\frac{1}{2}$ " HEX HD BOLT GR-5 (2) ¾" HEX NUT (TYP) PN: 3704G POST(1) 6'- 1 % " POST DEPTH 3240G 6 5/6" ROUND WASHER (WIDE) % " HEX NUT A563 GR. DH 5852B 1 HIGH INTENSITY REFLECTIVE SHEETING - SEE NOTE: B ISOMETRIC VIEW SECTION VIEW B-B SECTION VIEW A-A (2) ANCHOR POST ANGLE POST(1 & 2) 6'-0" (W6 X 8.5) I-BEAM POST PN: 533G 6'-0" (W6 X 8.5) PN: 15201G (SYTP) I-BEAM POST PN: 15000G W6 X 8.5 I-BEAM POST SHOWING FRONT VIEW POST(1) STANDARD WOOD BLOCKOUT NOTE: DO NOT BOLT ANCHOR RAIL PANEL TO POST(2) Texas Department of Transportation 4'-9 ½" (W6 X 8.5) (SYTP) I-BEAM POST PN: 15203G NOTE: NO BLOCKOUT INSTALLED AT POST(1) NOTE: NO BLOCKOUT INSTALLED AT POST (1) DETAIL 3 TRINITY HIGHWAY AT POST(0) 50' APPROACH GRADING APPROX 5'-10"-SOFTSTOP END TERMINAL 6'-5 3/8" (W6 X 15) STANDARD I-BEAM POST PN: 15205A MBGF MASH - TL-3 TRAFFIC FLOW APPROACH GRADING SGT (10S) 31-16 1V: 10H OR FLATTER EDGE OF PAVEMENT SEE PRODUCT ASSEMBLY MANUAL NOTE: ADJUST WIDTH ACCORDINGLY WHEN OFFSET IS USED. (OFFSET "OPTION" SHOWN) RAIL OFFSET ILE: sg+10s3116 DN: TxDOT CK: KM DW: VP FOR ADDITIONAL GUIDANCE, CONT SECT JOB 2 C) TxDOT: JULY 2016 HIGHWAY THIS STANDARD IS A BASIC REPRESENTATION OF THE SOf+S+OP END TERMINAL, IT IS NOT INTENDED TO REPLACE THE PRODUCT DESCRIPTION ASSEMBLY MANUAL. 0918 11 100, ETC. CR 110, ETC APPROACH GRADING AT GUARDRAIL END TREATMENTS KAUFMAN

GENERAL NOTES

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: LINDSAY TRANSPORTATION SOLUTIONS (LTS) BARRIER SYSTEMS, INC. AT (707) 374-6800
- FOR INSTALLATION, REPAIR, & MAINTENANCE REFER TO THE; MAX-TENSION INSTALLATION INSTRUCTION MANUAL. P/N MANMAX REV D (ECN 3516).
- 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURE'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- ALL STEEL COMPONENTS ARE GALVANIZED PER ASTM A123 OR EQUIVALENT UNLESS OTHERWISE STATED.
- 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POST WITH COMPOSITE BLOCKOUTS.
- 7. COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST(MPL)FOR CERTIFIED PRODUCERS.
- 8. REFER TO INSTALLATION MANUAL FOR SPECIFIC PANEL LAPPING GUIDANCE.
- 9. IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL FOR INSTALLATION GUIDANCE.
- 10. POSTS SHALL NOT BE SET IN CONCRETE.
- 11. A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POST TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST
- 12. MAX-TENSION SYSTEM SHALL NEVER BE INSTALLED WITHIN A CURVED SECTION
- 13. IF A DELINEATION MARKER IS REQUIRED, MARKER SHALL BE IN ACCORDANCE
- 14. THE SYSTEM IS SHOWN WITH 12'-6" MBGF PANELS, 25'-0" MBGF PANELS
- 15. A MINIMUM OF 12'-6" OF 12GA. MBGF IS REQUIRED IMMEDIATELY DOWNSTREAM OF THE MAX-TENSION SYSTEM.

ITEM#	PART NUMBER	DESCRIPTION	QT'
1	BSI-1610060-00	SOIL ANCHOR - GALVANIZED	1
2	BSI-1610061-00	GROUND STRUT - GALVANIZED	1
3	BSI-1610062-00	MAX-TENSION IMPACT HEAD	1
4	BSI-1610063-00	W6×9 I-BEAM POST 6FTGALVANIZED	1
5	BSI-1610064-00	TSS PANEL - TRAFFIC SIDE SLIDER	1
6	BSI-1610065-00	ISS PANEL - INNER SIDE SLIDER	1
7	BSI-1610066-00	TOOTH - GEOMET	1
8	BSI-1610067-00	RSS PLATE - REAR SIDE SLIDER	1
9	B061058	CABLE FRICTION PLATE - HEAD UNIT	1
10	BSI-1610069-00	CABLE ASSEMBLY - MASH X-TENSION	2
1.1	BSI-1012078-00	X-LITE LINE POST-GALVANIZED	8
12	B090534	8" W-BEAM COMPOSITE-BLOCKOUT XT110	8
13	BSI-4004386	12'-6" W-BEAM GUARD FENCE PANELS 12GA.	4
14	BSI-1102027-00	X-LITE SQUARE WASHER	1
15	BSI-2001886	5/8" X 7" THREAD BOLT HH (GR.5)GEOMET	1
16	BSI-2001885	¾" X 3" ALL-THREAD BOLT HH (GR.5)GEOMET	4
1 7	4001115	5/8" X 1 1/4" GUARD FENCE BOLTS (GR.2)MGAL	48
18	2001840	5/8" X 10" GUARD FENCE BOLTS MGAL	8
19	2001636	5/8" WASHER F436 STRUCTURAL MGAL	2
20	4001116	5/8" RECESSED GUARD FENCE NUT (GR.2)MGAL	59
21	BSI-2001888	5/8" X 2" ALL THREAD BOLT (GR.5)GEOMET	1
22	BSI-1701063-00	DELINEATION MOUNTING (BRACKET)	1
23	BSI-2001887	1/4" X 3/4" SCREW SD HH 410SS	7
24	4002051	GUARDRAIL WASHER RECT AASHTO FWR03	1
25	SEE NOTE BELOW	HIGH INTENSITY REFLECTIVE SHEETING	1
26	4002337	8" W-BEAM TIMBER-BLOCKOUT, PDB01B	8
27	BSI-4004431	25' W-BEAM GUARDRAIL PANEL, 8-SPACE, 12GA.	2
28	MANMAX Rev-(D)	MAX-TENSION INSTALLATION INSTRUCTIONS	1

Texas Department of Transportation

MAX-TENSION END TERMINAL MASH - TL-3

SGT (11S) 31-18

.E: sgt11s3118.dgn	DN: T×0	то	ск: КМ	DW:	DW: T×DOT		CK: CL
T×DOT: FEBRUARY 2018	CONT	SECT	JOB		HIC		HWAY
REVISIONS	0918	11	100,ET	c.	CR	11	O,ETC.
	DIST		COUNTY			S	HEET NO.
	DAL		KAUFMA	٩N			54

NUMBERS

MS3000

MTPHP1A

MTPHP1B

UHP2A

HP2B

E750

S760

F770

MS785

CBSP-14

G12025

G1203A

P675

G1209

W0516

B580122

B580904A

W050

N050 B340854A

N030

N100

W100

N012A

CT - 100S

B581002

Design Division Standard

HIGHWAY

55

DIST

E3151

B5160104A

P621

SF1303

FOR ANY PURPOSE RESULTING FROM

MADE BY TXDOT TS OR DAMAGES

OF ANY KIND IS INCORRECT RESUL

. NO WARRANTY FORMATS OR FOR

THE "TEXAS ENGINEERING PRACTICE ACT" CONVERSIONOF THIS STANDARD TO OTHER

GOVERNED BY ITY FOR THE

DISCLAIMER: THE USE OF THIS STANDARD IS TXDOT ASSUMES NO RESPONSIBIL

TXDOT FOR ANY PURPOSE WHATSOEVE DAMAGES RESULTING FROM ITS USE. * NOTE: GUARDRAIL PANELS 2 & 3 (ITEM C) MAY BE SUBSTITUTED NOTE: THERE ARE NO SUBSTITUTE GUARDRAIL WITH ONE 25'-0" GUARDRAIL PANEL (ITEM D). PANELS FOR (MODIFIED PANEL 4) END OF LENGTH OF NEED PANEL 1 MODIFIED PANEL 2 PANEL 3 9'-4 1/2' 12'-6" 12'-6" -3′ 1½" -|-3′ 1½ " FIELDSIDE FACE GR PANEL –(B2) GR PANEL C GR PANEL POSŤ 3 PLAN VIEW $_{\rm OR}^{\rm BY}$ LENGTH OF NEED COMPOSITE BLOCKOUTS (ITEM F) MAY BE SUBSTITUTED WITH (ITEM G) WOOD BLOCKOUTS. MADE SUL TS NOTE: CONFIRM ALL POST OFFSET'S AS SHOWN ON THE PRODUCT DESCRIPTION ASSEMBLY MANUAL. END PAYMENT FOR SGT IS RES ANY KIND INCORRECT H BEGIN STANDARD 31 MBGF TRAFFIC FLOW RAIL SPLICE HARDWARE LAP GUARDRAIL SPLICES IN DIRECTION OF TRAFFIC FLOW GRABBER TEETH LOCKED ONTO FRONT (8) 5/8" X 1 1/4" GR BOLTS MANTY OF OF THE MODIFIED GUARDRAIL PANEL YIELDING POST HARDWARE WITH 5/8" GR HEX NUTS (1) $\frac{5}{8}$ "× 10" GR BOLT NO BOLTS IN WITH 5/8" GR HEX NUT REAR TWO HOLES -(c, f) (c, f) NO WARR. FORMATS (b, f) (b, f) -(b, f) -(b, f) ENGINEERING PRACTICE ACT". OF THIS STANDARD TO OTHER YIELDING E-POST HEIGHT RAIL HEIGHT VFINISHED GRADE 1/2 " YIELDING (g, (2i), j, k)POST HOLES AT 41 DEPTH (TYP 8-2) STRUT HARDWARE "TEXAS POST POST 8 POST 7 POST 6 POST 5 POST 4 POST 3 ELEVATION VIEW ITEM (E) (YIELDING POST 8 THRU 2) ARE MODIFIED W6X8.5 STEEL THE POST WITH FOUR $\frac{1}{2}$ " YIELDING HOLES, TWO HOLES PER FLANGE. DISCLAIMER: THE USE OF THIS STANDARD IS GOVERNED BY TXDOT ASSUMES NO RESPONSIBILITY FOR THE 5 1/2" X 7 1/2" X 50" WOOD BREAKAWAY POST WOOD STRIKE BLOCK (K)-FIELD SIDE 6" X 8" X 14" W6X8.5 I-BEAM POST TRAFFIC WITH YEILDING HOLES COMPOSITE BLOCKOUT STRIKE PLATE (L) _SIDE_ 17" GUARDRAIL N-ITEM (F) E I TEM REFLECTIVE SHEETING PROVIDED BY COMPANY SGET (A)-IMPACT HEAD SEE (GENERAL NOTE 3) (1) 5/8" X 10" GR BOL BEARING (O) (1) 5/8" GR NUT BEARING O PLATE PLATE −(H)STRU1 $(2) \frac{1}{2}$ MAXIMUM TUBE HEIGHT YEILDING HOLE (b,(2d),e,f) 5/8" × 10" GR BOLT 5/8" FLAT WASHER PÖST LENGTH ABOVE GROUND (2) \FINISHED (1) 5/8" LOCK WASHER GRADE 70" TUBE GR NUT TÜBE EMBED DEPTH —(I) FOUNDATION TUBE 6" X 8" X 72" (I) THICKNESS SIDE VIEW POST 1 FRONT END VIEW POST 1 FIELD SIDE VIEW POST 8 - POST 3 (TYP) 50' APPROACH GRADING APPROX 5'-10" SGET MAXIMUM (OFFSET), HORIZONTAL FLARE STANDARD OVER THE FIRST 50 FEET = 1 FOOT. MBG EDGE OF PAVEMENT--2'-0" MAX. RAIL OFFSET 25 NOTE: ADJUST WIDTH ACCORDINGLY WHEN OFFSET IS USED. (OFFSET "OPTION" SHOWN)

APPROACH GRADING AT GUARDRAIL END TREATMENTS

GENERAL NOTES

- 1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202
- 2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL.
- 3. MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.
- 5. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS.
- 7. POSTS SHALL NOT BE SET IN CONCRETE.

MODIFIED

POST 2

OFFSET DISTANCE 3 TO POST 2 = 8 3 TO POST 1 = 6

TRAFFIC-SIDE VIEW

4 111111

POST 2

STRUT POST

-(Q)BCT CABLE

PPIPE SLEEVE

STRUT (H)-

3" X 3" X 80"

1/4 " THICKNESS

CĂBLE @

└(H)STRUT

(b, (2d), e, f)

(a, d, f)

-(Q)

POŚT

GRABBER HARDWARE

(h,(2i),e,f

BREAKAWAY

POST ()

-(H)STRUT

12′-6"

BGR PANEL

(b, (2d), e, f)

DO NOT BOLT MODIFIED (PANEL 4) TO WOOD POST

 (I,m)

BEARING

PLATE

MODIFIED B-

(h, (2i), J, K

(6k)

RAIL

POST 1

TRAFFIC SIDE VIEW

NO BOLTS IN REAR TWO HOLES

YEILDING

POST

Œ

POST 2 STRUT POST

APPROACH GRADING

(1V: 10H OR FLATTER)

MPACT HEAD

-(1,m)³/₈" X 3" GR5 LAG SCREWS

SEE PLAN VIEW

REINFORCEMENT

N GUARDRAIL GRABBER

MPLATE

(6h) $\frac{1}{2}$ " X 1 $\frac{1}{4}$ " BOLTS

(12i) $\frac{1}{2}$ " FLAT WASHER (6j) $\frac{1}{2}$ " LOCK WASHER

5∕8" HEX NUT

NOTE: TWO FLAT WASHERS

SIDE VIEW

REINFORCEMENT PLATE

WITH GUARDRAIL GRABBER

PER BOLT, ONE EACH SIDE OF PANEL.

- RF I D

- 8. IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.
- HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
- A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.





SPIG INDUSTRY, LLC SINGLE GUARDRAIL TERMINAL SGET - TL-3 - MASH

SGT (15) 31-20

00111	· /	\sim		- `	_			
: sgt153120. dgn	DN:TxDOT CK:KM DW:V		VP		CK: VP			
xDOT: APRIL 2020	CONT	SECT	JOB			HIGHWAY		
REVISIONS	0918	11	100,ETC. C		CR	11	O,ETC.	
	DIST		COUNTY			SHEET NO.		
	DAL		KAUFMAN				56	

THIS STANDARD IS A BASIC REPRESENTATION OF THE SGET TERMINAL SYSTEM AND IS NOT INTENDED TO REPLACE THE MANUFACTURER'S ASSEMBLY MANUAL

GENERAL NOTES

- 1. UNLESS OTHERWISE SHOWN IN THE PLANS, A VERTICAL EDGE IS PERMISSIBLE FOR HMAC PLACED GREATER THAN 5" BELOW THE EDGE OF PAVEMENT AND FOR THICKNESS OF HMAC LESS THAN 2.5".
- 2. FOR FURTHER INFORMATION REGARDING THE ROADSIDE AND PAVEMENT DETAILS, SEE TYPICAL SECTIONS.
- 3. PAYMENT FOR TAPERED EDGE WILL BE IN ACCORDANCE WITH APPLICABLE ITEMS IN THE CONTRACT.
- 4. THE SLOPE OF THE TAPERED EDGE SHALL BE 1.75H:1V OR FLATTER.
- 5. THE TAPERED EDGE SHALL BE PRODUCED BY USE OF A SCREED ATTACHMENT CAPABLE OF PRODUCING A SMOOTH COMPACTED SURFACE. ADDITIONAL COMPACTING EFFORT BEHIND THE SCREED IS NOT REQUIRED.

Texas Department of Transportation

Design Division Standard

TAPERED EDGE DETAILS
HMAC PAVEMENT

TE (HMAC) - 11

| Trib |

(NOT TO SCALE)

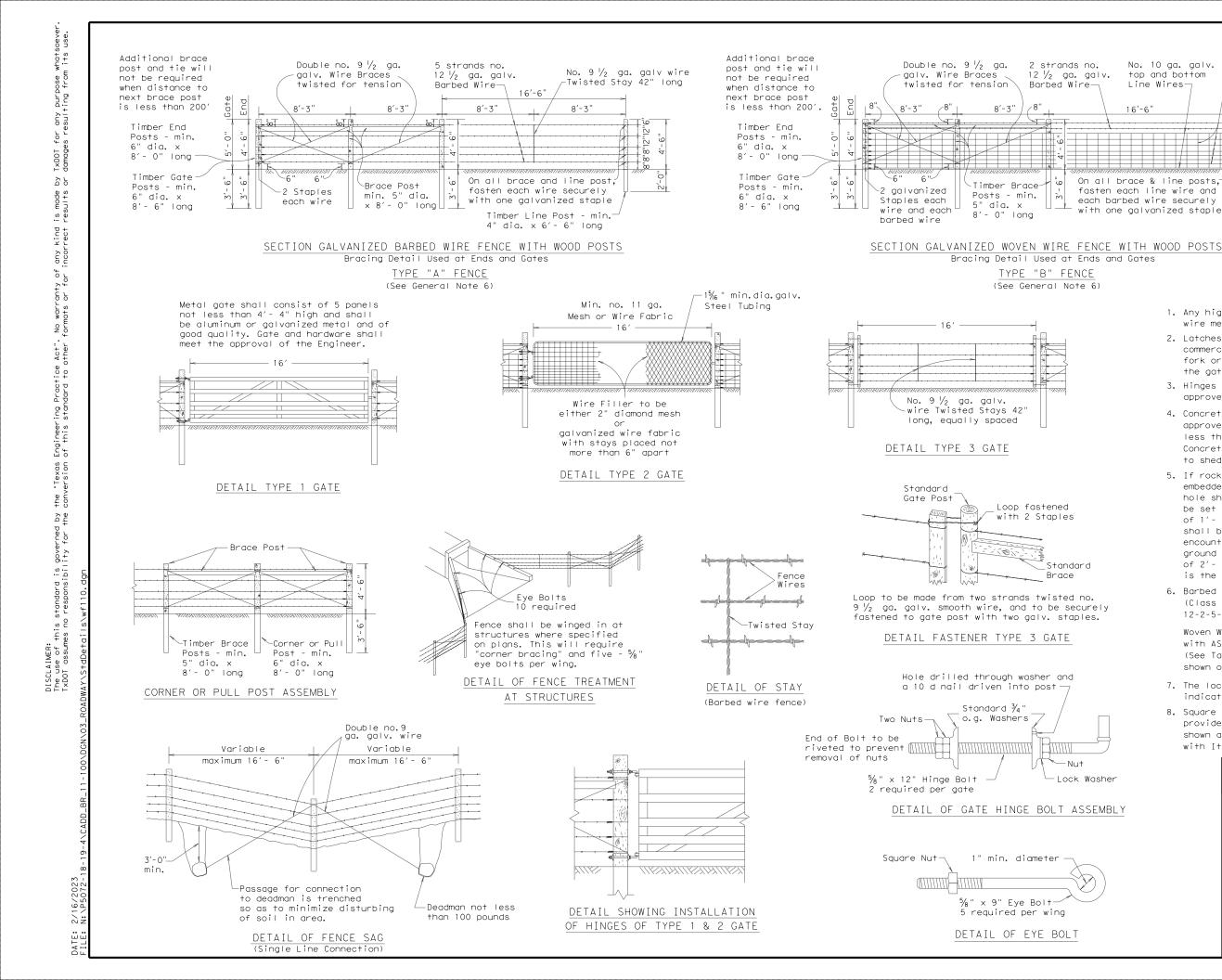


TABLE OF EQUIVALENT SIZES

FOR OPTIONAL SHAPE

FOR OFITONAL SHAFE											
Minimum Diameter of Round Post (Inches)	Minimum Equivalent Dimension for Each Side of Square Post (Inches)										
4	3 1/2										
5	4 1/2										
6	5 1/4										

GENERAL NOTES

1. Any high point which interferes with the placing of wire mesh shall be excavated to provide 2" clearance.

No. 12 $\frac{1}{2}$ ga. galv. -Line Wires and

Timber Line Post - min.

4" dia. x 6′- 6" long

Vertical Stays

No. 10 ga. galv.

top and bottom

Line Wires-

16'-6"

On all brace & line posts,

fasten each line wire and

each barbed wire securely

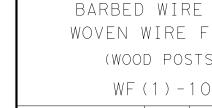
-Lock Washer

with one galvanized staple.

- 2. Latches for Type 1 and Type 2 gates shall be good commercial quality and design latches of the spring, fork or chain type. All latches shall be suitable for the gate and shall be approved by the Engineer.
- 3. Hinges for Type 2 gates shall be commercial design approved by the Engineer suitable for post and gate.
- 4. Concrete shall be of the design and consistency approved by the Engineer and shall contain not less than 4 sacks of cement per cubic yard. Concrete footings are to be crowned at the top
- 5. If rock is encountered at a depth less than the embedded depth required, a 15" or larger diameter hole shall be drilled for the post and the post shall be set in concrete. If rock is encountered at a depth of 1'- 6" or more below the ground surface, the hole shall be drilled to the required depth. If rock is encountered at a depth less than 1'- 6" below the ground surface, the holes shall be drilled a minimum of 2'- 0" into the rock or to the depth whichever is the lesser depth.
- 6. Barbed Wire shall be in accordance with ASTM A 121 (Class 1) Design designation 12-2-4-1 4R or 12-2-5-1 4R, or as approved by the Engineer.

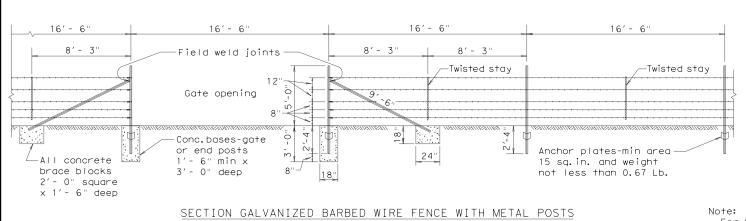
Woven Wire Fence (Type B) shall be in accordance with ASTM A 116 (Class 1) No. 12-1/2 Grade 60 (See Table 1 ASTM A 116) to the height and design shown on the plans, or as approved by the Engineer.

- 7. The location of gates and corner posts will be as indicated elsewhere on these plans
- 8. Square wood posts may be used in lieu of round posts provided minimum equivalent size requirements, as shown are met. All wood posts shall be in accordance with Item 552, "Wire Fence.



FILE: wf110.dgn	DN: TxDOT CK: AM DW:		DW: '	VP		CK:	
© TxDOT 1994	CONT	SECT	JOB		ні		HWAY
REVISIONS	0918	11	100,ET	CR	11	O,ETC.	
	DIST		COUNTY			SHEET NO.	
	DAL	KAUFMAN					58

Texas Department of Transportation BARBED WIRE AND WOVEN WIRE FENCE (WOOD POSTS)



Field weld joints No.10 ga. galv. top & bottom line wires Gate opening No.12 ½ ga. Conc. bases-aate galv. Tine wires or end posts -All concrete 1'- 6" min x & vertical stays Anchor plates-min area brace blocks 3'- 0" deep 2'- 0" square 15 sq.in. and weight not less than 0.67 Lb. x 1'- 6" deep

16' - 6"

BRACING DETAIL USED AT ENDS AND GATES

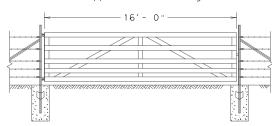
TYPE "C" FENCE (See General Note 8) For Steel pipe and T-Post requirements. (See General Notes 6 & 7)

SECTION GALVANIZED WOVEN WIRE FENCE WITH METAL POSTS

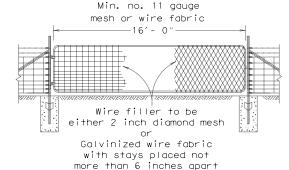
BRACING DETAIL USED AT ENDS AND GATES

TYPE "D" FENCE (See General Note 8)

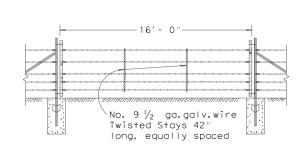
Metal gate shall consist of 5 panels not less than 4'- 4" high and shall be aluminum or galvanized metal and of good quality. Gate and hardware shall meet the approval of the engineer.



DETAIL TYPE 1 GATE

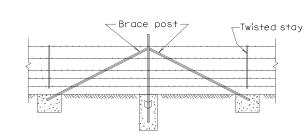


DETAIL TYPE 2 GATE

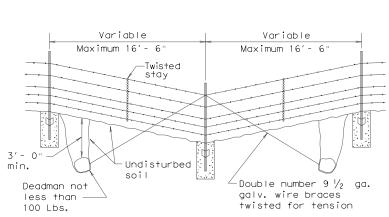


16' - 6"

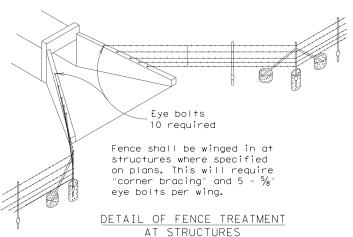
DETAIL TYPE 3 GATE

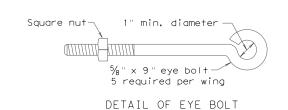


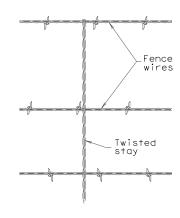
CORNER OR PULL POST ASSEMBLY



DETAIL OF FENCE SAG







DETAIL OF STAY (Barbed Wire Fence)

GENERAL NOTES

1. Any high point which interferes with the placing of wire mesh shall be excavated to provide a 2 inch clearance.

16' - 6"

- 2. Latches for Type 1 and Type 2 gates shall be good commercial quality and design latch of the spring, fork or chain type. All latches shall be suitable to the gate and shall be approved by the Engineer.
- 3. Hinges for Type 2 gates shall be a commercial design approved by the Engineer suitable for post and gate.
- 4. Concrete shall be of the design and consistency approved by the Engineer and shall contain not less than 4 sacks of cement per cubic yard. Concrete footings are to be crowned at the top to shed water.
- 5. Steel anchor plates shall be of a design and thickness sufficient to prevent turning of the post in firm soil.
- 6. Steel pipe end posts, corner and pull posts shall be a minimum of 2" Std. pipe (2.375" O.D., 0.154" wall thickness) with a $1\frac{1}{4}$ " Std. pipe brace (1.660" O.D. 0.140" wall thickness), with a 2"x2"x1/4" angle, or other as approved by the Engineer. Fasteners for securing barbed wire or woven wire fence to metal posts shall be a minimum of 11 gauge galvanized steel wire. Tubular posts shall be fitted with water malleable iron caps.
- 7. If Steel pipe is used for posts and braces, use standard pipe in accordance with ASTM A 53, Class B or A 501. For T-Posts use steel that meets ASTM A 702. Metal line posts shall be not less than 6'-6" in length and shall weigh not less than (1.33 lbs./lin.ft.). These Items shall be in accordance with Item 552, "Wire Fence.
- 8. Barbed Wire shall be in accordance with ASTM A 121, Class 1 Design designation 12-2-4-1 4R or 12-2-5-1 4R, or as approved by the Engineer.

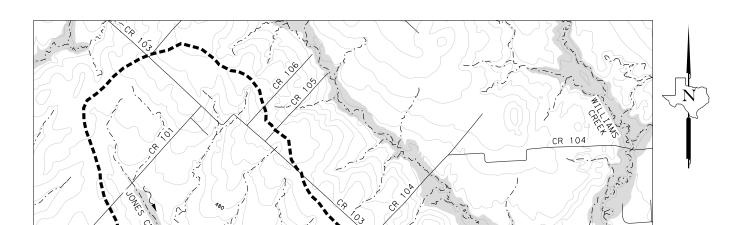
Woven Wire Fence (Type D) shall be in accordance with ASTM A 116, Class 1 No. 12-1/2 Grade 60 (See Table 1 ASTM A 116) to the height and design shown on the plans, or as approved by the Engineer.

9. The location of gates and corner posts will be as indicated elsewhere in these plans.



WF (2) -10

ILE: wf210.dgn	DN: Tx	TOC	CK: AM	DW:			CK:
TxDOT 1996	CONT	SECT	JOB			HIGHWAY CR 110, ETC.	
REVISIONS	0918	11	100,ET	С.	C. CR		O,ETC.
	DIST		COUNTY			SHEET NO.	
	DAL		KAUFMAN				59



BEGIN PROJECT STA 1112+20.00



--- DRAINAGE AREA BOUNDARY

SURFACE RUNOFF DIRECTION



EXISTING CONTOURS



FEMA ZONE A AREA



WATERSHED NAME AREA (SQ. MI)

DA ID	STREAM NAME	ROADWAY	AREA (SQ. MI)	CURVE NUMBER	TIME OF CONCENTRATION (MIN)	LAG TIME (MIN)	
DA 1	JONES CREEK	CR-110	10.31	70	395.6	238	

NRCS UN	IT HYDROGRAP	H METHOD IN	HEC-HMS					
PEAK DISCHARGE (CFS)								
2-YR	10-YR	50-YR	100-YR					
1,244.5	2,700.4	4,726.0	5,737.4					

NOTES:

-END PROJECT STA 1115+87.00

-BRIDGE AT JONES CREEK

- 1. RUNOFF COMPUTATIONS PERFORMED WITH HEC-HMS 4.3 AND VERIFIED BY OMEGA EM REGRESSION EQUATION ANALYSIS.
- RAINFALL DEPTHS WERE OBTAINED FROM NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION (NOAA) ATLAS 14, VOLUME 11.
- 3. STORMS WERE MODELED AS 24-HOUR DURATION EVENTS USING TEMPORAL DISTRIBUTION.
- 4. RUNOFF VOLUME WAS COMPUTED USING THE SCS CURVE NUMBER LOSS MODEL.
- 5. SOURCE OF TOPOGRAPHY DATA: 2013 USGS 10FT DEM AND PROJECT TOPOGRAPHIC SURVEY.
- 6. TIME OF CONCENTRATION (Tc) WAS COMPUTED USING NRCS METHOD.



0 2,000 4,000 SCALE: 1"=4,000'

NO.	DATE	REVISION	APPROV.							
	1									





281-945-0069 PH 281-945-0081 FX

CR 110
DRAINAGE AREA MAP
JONES CREEK

SHEET 1 OF 1

SHEET I OF I										
DN:	DN	FED. RD. DIV. NO.	STATE		HIGHWAY NO.					
CK DN:	WZ	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC		
DW:	DW	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.		
CK DW:	WZ	DAL	KAUF	MAN	0918	11	100,ETC	60		

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.00205 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0350D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA: USDA/NRCS-NGCE 2016 LIDAR AND PROJECT TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.



NO.	DATE	REVISION	APPROV.							
4	×	├ ®	© 2023							
4	Texas Department of Transportation									



OR AAA

CR 110
HYDRAULIC DATA
JONES CREEK

SHEET 1 OF 4

,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	01 7								
N:	DN	FED.RD. DIV.NO.	STATE		PROJECT NO.				
CK DN:	WZ	6	TEXAS	S	EE TIT	LE SHE		CR 110,ETC	
)W:	DW	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.	
CK DW:	W7	DAL	KAUF	MAN	0918	11	100.ETC	61	

HYDRAULIC MODEL RESULTS

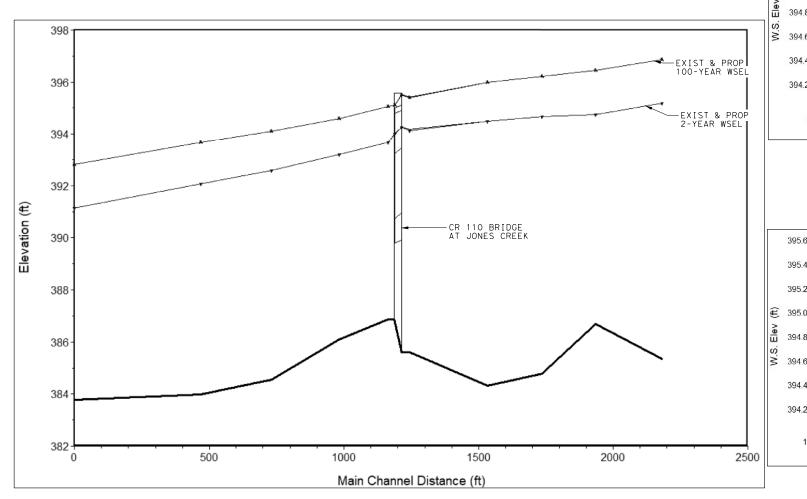
	HEC-RAS RESULTS FOR CR-110 BRIDGE (MULTIPLE OPENING) AT JONES CREEK													
HEC-RAS STATION	2-YEAR FLOW (CFS)		2	2-YEAR WSEL (FT)		2-YEAR VELOCITY (FT/S)		100-YEAR FLOW (CFS)		100-YEAR WSEL (FT)			100-YEAR VELOCITY (FT/S)	
314.10.1	EXISTING	PROPOSED	EXISTING	PROPOSED	DIFF.	EXISTING	PROPOSED	EXISTING	PROPOSED	EXISTING	PROPOSED	DIFF.	EXISTING	PROPOSED
2464	1245	1245	395.20	395.19	-0.01	3.36	3.36	5737	5737	396.86	396.86	0.00	4.81	4.81
2218	1245	1245	394.76	394.75	-0.01	4.59	4.64	5737	5737	396.45	396.45	0.00	4.32	4.31
2019	1245	1245	394.67	394.66	-0.01	2.24	2.26	5737	5737	396.21	396.22	0.01	3.53	3.52
1814	1245	1245	394.51	394.49	-0.02	2.91	2.94	5737	5737	395.98	395.99	0.01	3.89	3.87
1525 (U/S ROW)	1245	1245	394.19	394.15	-0.04	3.25	3.39	5737	5737	395.39	395.42	0.03	5.28	5.18
1500					CR-11	O BRIDGE	(MULTIPLE	OPENING)	AT JONES	CREEK				
1446 (U/S ROW)	1245	1245	393.67	393.67	0.00	4.73	4.73	5737	5737	395.06	395.06	0.00	4.81	4.81
1266	1245	1245	393.20	393.20	0.00	4.09	4.09	5737	5737	394.61	394.60	-0.01	4.64	4.64
1013	1245	1245	392.59	392.59	0.00	3.93	3.93	5737	5737	394.11	394.11	0.00	3.73	3.74
750	1245	1245	392.07	392.07	0.00	3.58	3.58	5737	5737	393.67	393.67	0.00	3.93	3.94
282	1245	1245	391.16	391.16	0.00	3.82	3.82	5737	5737	392.81	392.81	0.00	4.00	4.00

NOTES:

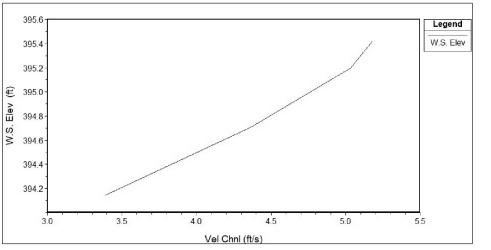
- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.00205 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257CO350D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA:
 USDA/NRCS-NGCE 2016 LIDAR AND PROJECT
 TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.

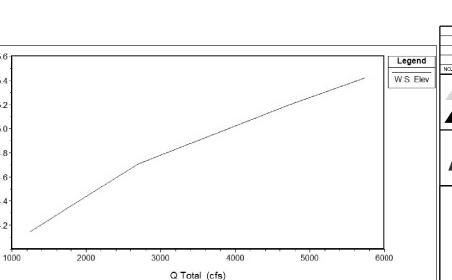
HYDRAULIC DATA JONES CREEK BRIDGE

Q2 = 812 CFS V2 = 3.39 FPS Q100 = 1125 CFS V100 = 5.18 FPS HW2 = 394.15' HW100 = 395.42'



RMorfin 2/16/2023 N:\P5072-18-19-4\CADD_BR_11-\`*.nn1-RW-HALF_PDF.pitcfg





395.4

395.2

394.8

394.4

394.2



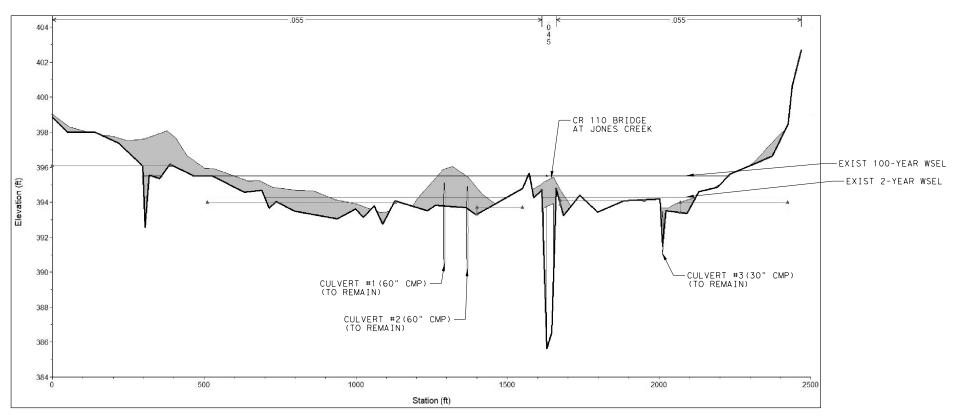
APPROV REVISION © 2023 Texas Department of Transportation

F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

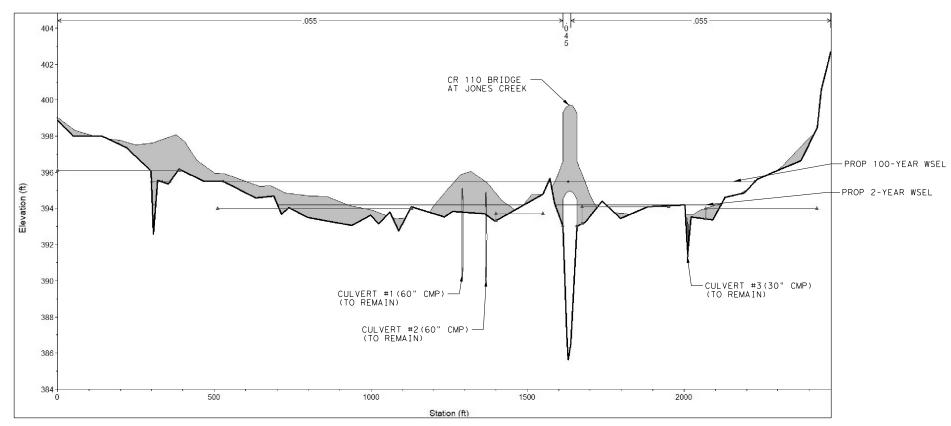
CR 110 HYDRAULIC DATA JONES CREEK

SHEET 2 OF 4

JIILLI Z	_ 01 7							
DN:	DN	FED. RD. DIV. NO.	STATE		HIGHWAY NO.			
CK DN:	WZ	6 TEXAS SEE TITLE SHEET (CR 110,ET	
DW:	DW	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	WZ	DAL	KAUF	MAN	0918	11	100,ETC	62



EXISTING CR 110 BRIDGE AT JONES CREEK



PROPOSED CR 110 BRIDGE AT JONES CREEK

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.00205 FT/FT.
- 4. MULTIPLE OPENING ANALYSIS WAS CONDUCTED FOR THE HYDRAULICS.
- 5. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0350D EFFECTIVE JULY 3, 2012.
- 6. SOURCE OF TOPOGRAPHY DATA:
 USDA/NRCS-NGCE 2016 LIDAR AND PROJECT
 TOPOGRAPHIC SURVEY.
- 7. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.

HYDRAULIC DATA JONES CREEK BRIDGE



Texas Department of Transportation



CR 110
HYDRAULIC DATA
JONES CREEK

SHEET 3 OF 4

SHEET S	UF 4							
DN:	DN	FED. RD. DIV. NO.	STATE		PROJE	CT NO.		H[GHWAY NO.
CK DN:	WZ	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ET
DW:	DW	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	WZ	DAL	KAUF	MAN	0918	11	100,ETC	63

								CR-110								
Plan: PR Open#1:	River 1 Culvert #1	Reach 1 R Profile: 2	S: 1500 -year	Plan: PR Open#1:	River 1 Culvert #2		S: 1500 -year	Plan: PR	River 1	Reach 1 RS: Profile: 2-yea		#2: Bridge	Plan: PR Open#3:	River 1 Culvert #3	Reach 1 R Profile: 2	S: 1500 -year
Q Culv Group (cfs)	61.07	Culv Full Len (ft)		Q Culv Group (cfs)	64.15	Culv Full Len (ft)		E.G. US. (f+)	394.26	Element	Inside BR US	Inside BR DS	Q Culv Group (cfs)	13.54	Culv Full Len (ft)	24.00
# Barrels	1.00	Culv Vel US (f†/s)	3.79	# Barrels	1.00	Culv Vel US (ft/s)	3.80	W.S. US. (f†)	394.10	E.G. Elev	394.16	394.03	# Barrels	1.00	Culv Vel US (ft/s)	2.76
Q Barrel (cfs)	61.07	Culv Vel DS (ft/s)	3.75	Q Barrel (cfs)	64.15	Culv Vel DS (ft/s)	3.74	Q Total (cfs)	812.24	W.S. Elev	393.84	393.62	Q Barrel (cfs)	13.54	Culv Vel DS (ft/s)	2.76
E.G. US. (ft)	394.27	Culv Inv El Up (ft)	390.11	E.G. US. (ft)	394.27	Culv Inv El Up (ft)	389.92	Q Bridge (cfs)	812.24	Crit W.S.	391.74	392.05	E.G. US. (ft)	394.15	Culv Inv El Up (ft)	390.97
W.S. US. (ft)	394.26	Culv Inv El Dn (ft)	390.02	W.S. US. (f+)	394.26	Culv Inv El Dn (ft)	389.80	Q Weir (cfs))	Max Chl Dpth (ft)	8.23	6.75	W.S. US. (ft)	394.14	Culv Inv El Dn (ft)	390.73
E.G. DS (ft)	393.89	Culv Frctn Ls (ft)	0.05	E.G. DS (f+)	393.89	Culv Frctn Ls (ft)	0.05	Weir Sta Lft (ft)		Vel Total (ft/s)	4.57	5.15	E.G. DS (f+)	393.90	Culv Frctn Ls (ft)	0.09
W.S. DS (ft)	393.88	Culv Exit Loss (ft)	0.21	W.S. DS (f+)	393.88	Culv Exit Loss (ft)	0.21	Weir Sta Rgt (ft)		Flow Area (sq ft)	177.68	157.76	W.S. DS (f+)	393.88	Culv Exit Loss (ft)	0.10
Delta EG (ft)	0.37	Culv Entr Loss (ft)	0.11	Delta EG (ft)	0.37	Culv Entr Loss (ft)	0.11	Weir Submerg		Froude # Chl	0.28	0.35	Delta EG (ft)	0.25	Culv Entr Loss (ft)	0.06
Delta WS (ft)	0.37	Q Weir (cfs)	228.87	Delta WS (ft)	0.37	Q Weir (cfs)	228.87	Weir Max Depth (ft)		Specif Force (cu	560.30	475.90	Delta WS (ft)	0.26	Q Weir (cfs)	64.62
E.G. IC (ft)	393.23	Weir Sta Lft (ft)	917.83	E.G. IC (f+)	393.13	Weir Sta Lft (ft)	917.83	Min El Weir Flow (ft)	394.11	Hydr Depth (ft)	3.95	3.51	E.G. IC (f+)	394.14	Weir Sta Lft (ft)	1845.00
E.G. OC (ft)	394.27	Weir Sta Rgt (ft)	1490.36	E.G. OC (ft)	394.26	Weir Sta Rgt (ft)	1490.36	Min El Prs (ft)	395.59	W.P. Total	49.44	47.89	E.G. OC (ft)	394.15	Weir Sta Rgt (ft)	2101.58
Culvert Control	Outlet	Weir Submerg	0.28	Culvert Control	Outlet	Weir Submerg	0.28	Delta EG (ft)	0.38	Conv. Total	13765.70	11534.10	Culvert Control	Outlet	Weir Submerg	0.21
Culv WS Inlet (ft)	393.93	Weir Max Depth (ft)	0.85	Culv WS Inlet (ft)	393.93	Weir Max Depth (ft)	0.85	Delta WS (ft)	0.70	Top Width (ft)	45.00	45.00	Culv WS Inlet (ft)	393.47	Weir Max Depth (ft)	0.52
Culv WS Outlet (ft)	393.88	Weir Avg Depth (ft)	0.37	Culv WS Outlet (ft)	393.88	Weir Avg Depth (ft)	0.37	BR Open Area (sq ft)	239.28	Frctn Loss (ft)	0.11	0.12	Culv WS Outlet (ft)	393.23	Weir Avg Depth (ft)	0.18
Culv Nml Depth (ft)	3.10	Weir Flow Area (sq ft)	128.64	Culv Nml Depth (ft)	2.92	Weir Flow Area (sq ft)	128.64	BR Open Vel	5.15	C & E Loss	0.03	0.02	Culv Nml Depth (ft)		Weir Flow Area (sq ft)	46.14
Culv Crt Depth (ft)	2.20	Min El Weir Flow	393.78	Culv Crt Depth (ft)	2.26	Min El Weir Flow	393.78	BR Sluice Coef		Shear Total	0.78	1.02	Culv Crt Depth (ft)	1.24	Min El Weir Flow	393.68
								BR Sel Method	Energy onl	y Power Total	3.57	5.25				

	CR-110															
Plan: PR Open#1:	River 1 Culvert #1	Reach 1 Profile: 1	RS: 1500 00-year	Plan: PR Open#1: (River 1 Culvert #2	Reach 1 R Profile: 10	RS: 1500 00-year	Plan: PR		Reach 1 RS: ofile: 100-yea	1500 Open or	#2: Bridge	Plan: PR Open#3: C	River 1 ulvert #3		S: 1500 00-year
Q Culv Group (cfs)	76.22	Culv Full Len (ft)	24.00	Q Culv Group (cfs)	78.64	Culv Full Len (ft)	24.00	E.G. US. (ft)	395.55	Element	Inside BR US	Inside BR DS	Q Culv Group (cfs)	18.70	Culv Full Len (ft)	24.00
# Barrels	1.00	Culv Vel US (ft/s)	3.88	# Barrels	1.00	Culv Vel US (ft/s)	4.01	W.S. US. (ft)	395.42	E.G. Elev	395.35	395.17	# Barrels	1.00	Culv Vel US (ft/s)	3.81
Q Barrel (cfs)	76.22	Culv Vel DS (ft/s)	3.88	Q Barrel (cfs)	78.64	Culv Vel DS (ft/s)	4.01	Q Total (cfs)	1413.58	W.S. Elev	395.17	394.91	Q Barrel (cfs)	18.70	Culv Vel DS (ft/s)	3.81
E.G. US. (ft)	395.56	Culv Inv El Up (ft)	390.11	E.G. US. (ft)	395.56	Culv Inv El Up (ft)	389.92	Q Bridge (cfs)	1125.15	Crit W.S.	393.02	393.24	E.G. US. (ft)	395.58	Culv Inv El Up (ft)	390.97
W.S. US. (f+)	395.51	Culv Inv El Dn (ft)	390.02	W.S. US. (ft)	395.51	Culv Inv El Dn (ft)	389.80	Q Weir (cfs)		Max Chl Dpth (ft)	9.56	8.04	W.S. US. (f+)	395.45	Culv Inv El Dn (ft)	390.73
E.G. DS (ft)	395.17	Culv Frctn Ls (ft)	0.08	E.G. DS (f+)	395.17	Culv Frctn Ls (ft)	0.06	Weir Sta Lft (ft)		Vel Total (ft/s)	2.60	3.14	E.G. DS (f+)	395.17	Culv Frctn Ls (ft)	0.17
W.S. DS (ft)	395.12	Culv Exit Loss (ft)	0.19	W.S. DS (f+)	395.12	Culv Exit Loss (ft)	0.21	Weir Sta Rgt (ft)		Flow Area (sq ft)	433.54	358.30	W.S. DS (ft)	395.07	Culv Exit Loss (ft)	0.13
Delta EG (ft)	0.39	Culv Entr Loss (ft)	0.12	Delta EG (ft)	0.39	Culv Entr Loss (ft)	0.12	Weir Submerg		Froude # Chl	0.19	0.26	Delta EG (ft)	0.42	Culv Entr Loss (ft)	0.11
Delta WS (ft)	0.39	Q Weir (cfs)	2650.15	Delta WS (ft)	0.39	Q Weir (cfs)	2650.15	Weir Max Depth (ft)		Specif Force (cu	949.04	790.10	Delta WS (ft)	0.37	Q Weir (cfs)	1500.11
E.G. IC (ft)	393.68	Weir Sta Lft (ft)	592.61	E.G. IC (f+)	393.56	Weir Sta Lft (ft)	592.61	Min El Weir Flow (ft)	394.11	Hydr Depth (ft)	1.71	1.38	E.G. IC (f+)	395.53	Weir Sta Lft (ft)	1845.00
E.G. OC (ft)	395.55	Weir Sta Rgt (ft)	1505.00	E.G. OC (ft)	395.57	Weir Sta Rgt (ft)	1505.00	Min El Prs (ft)	395.59	W.P. Total	266.71	265.05	E.G. OC (ft)	395.58	Weir Sta Rgt (ft)	2229.38
Culvert Control	Outlet	Weir Submerg	0.67	Culvert Control	Outlet	Weir Submerg	0.67	Delta EG (ft)	0.38	Conv. Total (cfs)	26433.30	21971.20	Culvert Control	Outlet	Weir Submerg	0.66
Culv WS Inlet (ft)	395.11	Weir Max Depth (ft)	2.13	Culv WS Inlet (ft)	394.92	Weir Max Depth (ft)	2.13	Delta WS (ft)	0.50	Top Width (ft)	253.37	258.92	Culv WS Inlet (ft)	393.47	Weir Max Depth (ft)	1.87
Culv WS Outlet (ft)	395.02	Weir Avg Depth (ft)	1.09	Culv WS Outlet (ft)	394.80	Weir Avg Depth (ft)	1.09	BR Open Area (sq ft)	239, 28	Frctn Loss (ft)	0.09	0.08	Culv WS Outlet (ft)	393.23	Weir Avg Depth (ft)	1.27
Culv Nml Depth (ft)		Weir Flow Area (sq ft)	890.04	Culv Nml Depth (ft)		Weir Flow Area (sq ft)	890.04	BR Open Vel	5.21	C & E Loss	0.04	0.09	Culv Nml Depth (ft)		Weir Flow Area (sq ft)	488.93
Culv Crt Depth (ft)	2.47	Min El Weir Flow	393.78	Culv Crt Depth (ft)	2.51	Min El Weir Flow	393.78	BR Sluice Coef		Shear Total	0.29	0.35	Culv Crt Depth (ft)	1.47	Min El Weir Flow	393.68
								BR Sel Method	Energy only	Power Total	0.75	1.10				

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.00205 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0350D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA:
 USDA/NRCS-NGCE 2016 LIDAR AND PROJECT
 TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 01/20/2021.



NO.	DATE	REVISION	APPROV.				
© 2023 Texas Department of Transportation							
		F C072					



F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

CR 110
HYDRAULIC DATA
JONES CREEK

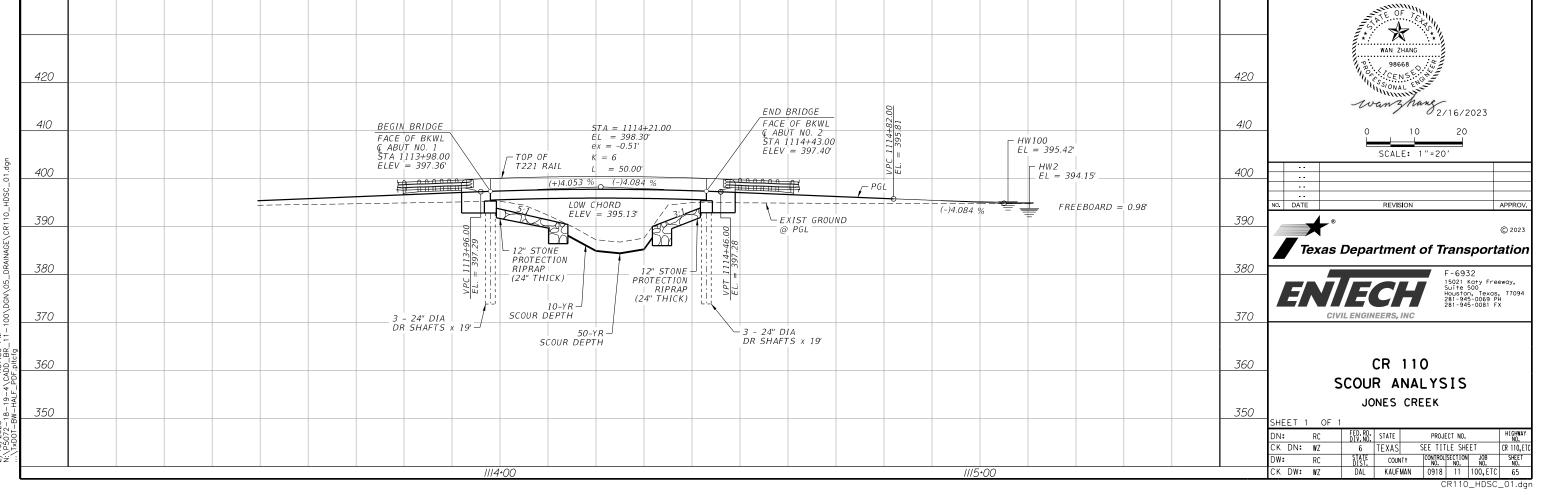
SHEET 4 OF 4

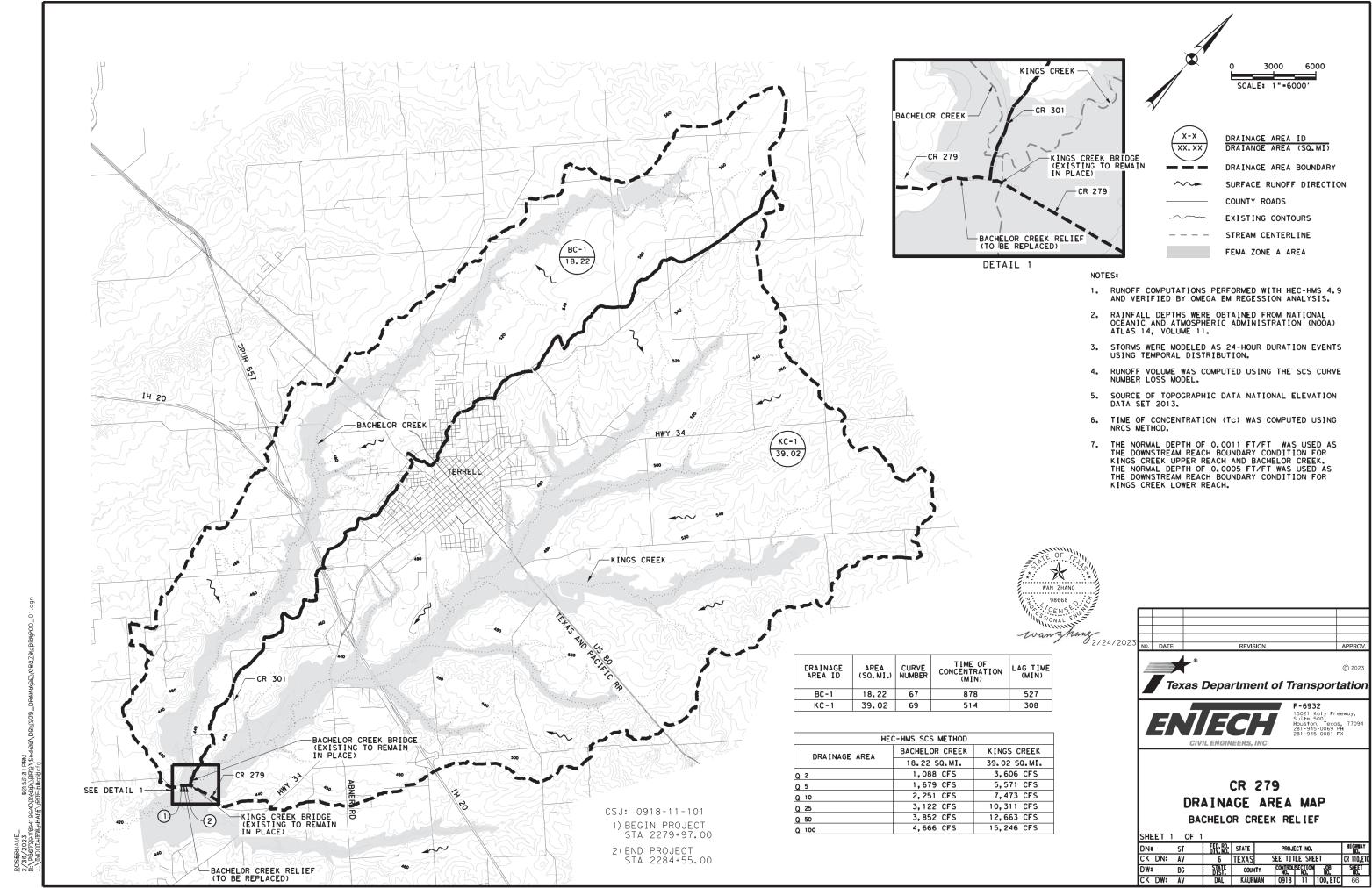
HEEL .	1 01 7							
N:	DN	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
K DN:	WZ	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
W:	DW	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
K DW:	WZ	DAL	KAUF	MAN	0918	11	100, ETC	64

Hydraulic Design Data QC (10-yr)								
Contraction Scour								
	Input Data							
	Channe I							
Average Depth (ft):	6.34							
Approach Velocity (ft/s):	3.27							
BR Average Depth (ft):	6.34							
BR Opening Flow (cfs):	921							
BR Top WD (ft):	45							
Grain Size D50 (mm):	0.2							
Approach Flow (cfs):	921							
Approach Top WD (ft):	70							
K1 Coefficient:	0.69							
Results								
Scour Depth Ys (ft):	2.26							
Critical Velocity (ft/s):	1.32							
Equation:	Live							

Hydraulic Design Data QS (50-yr)							
Contraction Scour							
	Input Data						
	Channe I						
Average Depth (ft):	6.77						
Approach Velocity (ft/s):	3.72						
BR Average Depth (ft):	6.77						
BR Opening Flow (cfs):	1142						
BR Top WD (ft):	45						
Grain Size D50 (mm):	0.2						
Approach Flow (cfs):	1142						
Approach Top WD (ft):	70						
K1 Coefficient:	0.69						
Results							
Scour Depth Ys (ft):	2.41						
Critical Velocity (ft/s):	1.33						
Equation:	Live						

- 1. SCOUR ANALYSIS IS BASED ON TXDOT GEOTECHNICAL MANUAL (GM).
 FHWA H.E.C.-18, "EVALUATING SCOUR AT BRIDGES", 5TH EDITION, AND
 TXDOT "SCOUR EVALUATION GUIDE REVISED AUGUST 2020", CHAPTER 8.
 ABUTMENT SCOUR EQUATIONS IN HEC-18 TEND TO OVER ESTIMATE ABUTMENT
 SCOUR DEPTHS. BRIDGE ABUTMENTS WILL BE ARMORED WITH STONE
 PROTECTION RIPRAP. NO APPRECIABLE ABUTMENT SCOUR IS ANTICIPATED.
- 2. THE D50 SOIL PARTICLE SIZE FOR THIS PROJECT IS THE MINIMUM SIZE ALLOWED BY THE GM. THE SOIL DATA IS FROM "THE GEOTECHNICAL INVESTIGATION FOR KAUFMAN COUNTY ON & OFF-SYSTEM BRIDGE REPLACEMENT PROJECT" REPORT.
- 3. THE PROPOSED BRIDGE IS A SINGLE SPAN STRUCTURE. PIER SCOUR IS NOT APPLICABLE FOR THIS BRIDGE.
- 4. THE TOTAL MAXIMUM CALCULATED SCOUR DEPTH IS TAKEN AS THE MAXIMUM OF HORIZONTAL CONTRACTION AND VERTICAL PRESSURE SCOUR. THE MAXIMUM SCOUR DEPTH OCCURED DURING THE 50-YR STORM AND IS 2.41'.
- 5. SEE "HYDROLOGY/HYDRAULIC REPORT FOR JONES CREEK" DATED NOVEMBER 4TH, 2022 FOR ADDITIONAL INFORMATION.





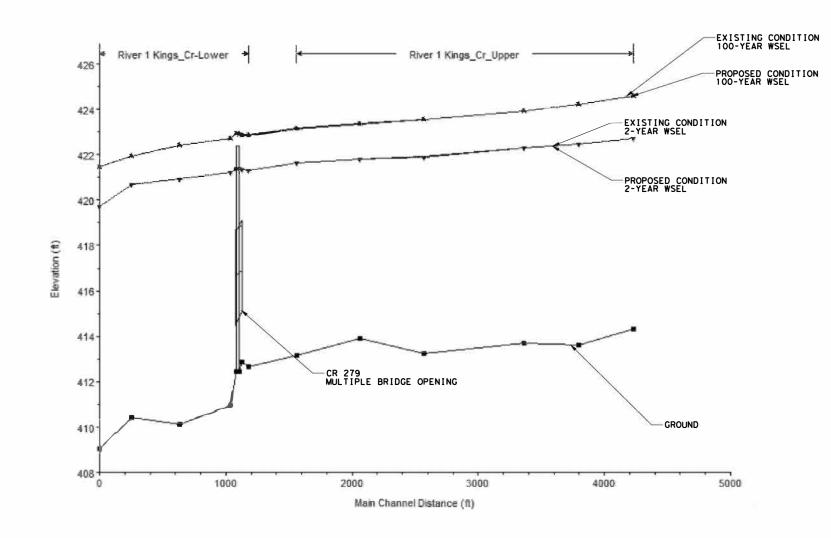
HIGHNAY NO. CR 110,ETC SHEET NO. 67

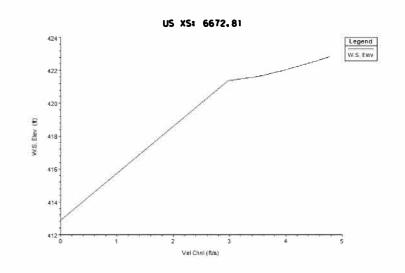
HYDRAULIC MODEL RESULTS

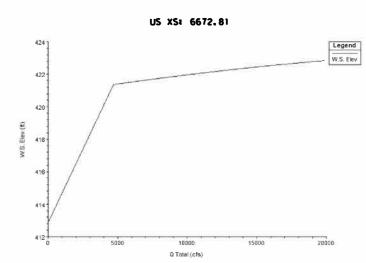
HEC-RAS	REACH	2-YEAR FLOW	2	YEAR WSEL (F	T)	2-YEAR VELO	CITY (FT/S)	100-YEAR	100	O-YEAR WSEL	FT)	100-YEAR VEL	OCITY (FT/S)
STATION	REACH	(CFS)	EXISTING	PROPOSED	DIFFERENCE	EXISTING	PROPOSED	FLOW (CFS)	EXISTING	PROPOSED	DIFFERENCE	EXISTING	PROPOSED
9887.00	BACHELOR	1088	423. 90	423. 90	0.00	5. 88	5. 88	4666	425. 67	425. 67	0.00	6. 44	6. 44
9210.18	BACHELOR	1088	423. 51	423.51	0.00	4. 36	4. 36	4666	424. 72	424. 72	0.00	8. 33	8. 34
8443.41	BACHELOR	1088	422. 69	422. 69	0.00	5. 43	5. 43	4666	423. 97	423. 98	0.01	7. 40	7. 38
7899.98	BACHELOR	1088	422. 34	422. 34	0.00	4. 46	4. 47	4666	423.51	423. 51	0.00	5. 87	5. 88
7113.35	BACHELOR	1088	421.15	421.14	-0.01	6. 34	6. 35	4666	423. 04	423. 03	-0.01	4. 42	4. 48
9772.00	KINGS_UPPER	3606	422. 70	422. 70	0.00	4. 28	4. 28	15246	424.57	424. 57	0.00	5. 96	5. 96
9342.74	KINGS_UPPER	3606	422. 47	422. 47	0.00	3. 83	3. 83	15246	424. 20	424. 20	0.00	5.88	5. 89
8906.70	KINGS_UPPER	3606	422. 28	422. 28	0.00	3. 63	3. 63	15246	423. 94	423. 93	-0.01	5. 37	5. 37
8110.07	KINGS_UPPER	3606	421.91	421.91	0.00	4. 18	4. 18	15246	423.54	423. 54	0.00	4. 73	4. 74
7607.04	KINGS_UPPER	3606	421.81	421.81	0.00	3. 01	3. 02	15246	423.36	423. 36	0.00	4.88	4.88
7103.88	KINGS_UPPER	3606	421.64	421.64	0.00	3. 13	3. 14	15246	423. 16	423. 15	-0.01	3. 74	3. 75
6721.00	KINGS_LOWER	4694	421.30	421.29	-0.01	5. 48	5.51	19912	422. 87	422.87	0.00	6. 08	6. 10
6672.81	KINGS_LOWER	4694	421.36	421.35	-0.01	2. 95	2. 96	19912	422.87	422. 87	0.00	4. 77	4. 79
6637.00	KINGS_LOWER		1				BRIDGE	3.0					
6583.93	KINGS_LOWER	4694	421.20	421.20	0.00	5. 00	5. 00	19912	422. 70	422. 70	0.00	6. 78	6. 78
6177.62	KINGS_LOWER	4694	420. 94	420. 94	0.00	4. 07	4. 06	19912	422. 42	422. 42	0.00	4.84	4. 84
5797.99	KINGS_LOWER	4694	420. 69	420. 69	0.00	5. 13	5. 09	19912	421.94	421.94	0.00	8. 63	8. 61
5542.79	KINGS_LOWER	4694	419. 73	419. 74	0. 01	8. 58	8. 58	19912	421.47	421.47	0.00	9, 65	9, 65

NOTES:

- 1. HEC-RAS VERSION 6.3.1 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION
 WAS BASED ON NORMAL DEPTH CALCULATION
 AND BED SLOPE OF 0.0011 FT/FT FOR
 BACHELOR AND KINGS CREEK UPPER REACH
 AND 0.0005 FT/FT OR KINGS CREEK
 LOWER REACH.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257CO200D EFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHIC DATA NATIONAL ELEVATION DATA SET 2013.
- COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 01/20/2021.









NO. DATE REVISION APPROV.

© 2023

Texas Department of Transportation

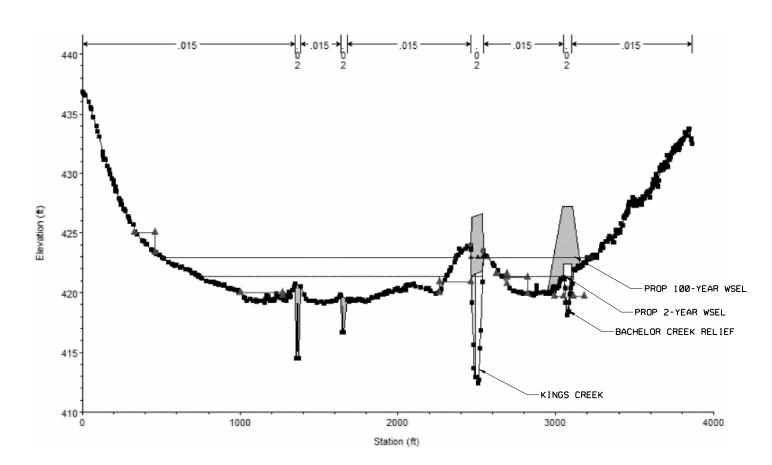
F-6932



CR 279
HYDRAULIC DATA
BACHELOR CREEK RELIEF

SHEET 2	OF 4				
DN:	ST	FED. RD. DIV. NO.	STATE	PROJECT NO.	H I GHRAY
CK DN:	AV	6	TEXAS	SEE TITLE SHEET	CR 110,ET
DW:	BG	STATE DIST.	COUNTY	CONTROLISECTION	JOB SHEET
CK DW:	AV	DAL	KAUFMAN	0918 11 10	0,ETC 68

EXISTING CR 279 BRIDGE AT BACHELOR CREEK

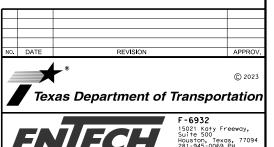


PROPOSED CR 279 BRIDGE AT BACHELOR CREEK

NOTES:

- HEC-RAS VERSION 6.3.1 USED FOR THE ANALYSIS.
- ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION
 WAS BASED ON NORMAL DEPTH CALCULATION
 AND BED SLOPE OF 0.0011 FT/FT FOR
 BACHELOR AND KINGS CREEK UPPER REACH
 AND 0.0005 FT/FT OR KINGS CREEK
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0200D EFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHIC DATA NATIONAL ELEVATION DATA SET 2013.
- COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 01/20/2021.





CR 279 HYDRAULIC DATA BACHELOR CREEK RELIEF

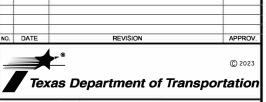
SHEET 3 OF 4

SHEE	. 1 3	Ur	4						
DN:		ST	FED.RD. DIV.NO.	STATE	ı	PROJE	CT NO.		HIGHWAY NO.
CK [ON:	A۷	6	TEXAS	SEE	TIT	LE SHE	-	CR 110,ETC
DW:		BG	STATE DIST.	COU		TROL S	SECTION NO.	JOB NO.	SHEET NO.
CK D)W:	A۷	DAL	KAUF	MAN 0	918	11	100,ETC	69

PLAN: PROP BACHELOR CREEK RELIEF RS: 6637 PROFILE: 2 YEAR					PLAN: PROP BACHELOR CREEK RELIEF RS: 6637 PROFILE: 100 YEAR					
E.G. US. (FT)	421.31	ELEMENT	INSIDE BR US	INSIDE BR DS	E.G. US. (FT)	422. 92	ELEMENT	INSIDE BR US	INSIDE BR DS	
W. S. US. (FT)	421.31	E.G. ELEV (FT)	421.30	421.30	W.S. US. (FT)	422. 92	E.G. ELEV (FT)	422.89	422. 86	
Q TOTAL (CFS)	88. 81	W.S. ELEV (FT)	421.28	421.27	Q TOTAL (CFS)	314. 16	W.S. ELEV (FT)	422. 83	422.86	
Q BRIDGE (CFS)	88. 81	CRIT W.S.	419.55	419.56	Q BRIDGE (CFS)	287. 31	CRIT W.S.	420.61	420.60	
Q WEIR (CFS)	9	MAX CHL DPTH	3. 17	3. 16	Q WEIR (CFS)		MAX CHL DPTH (FT)	4. 72	4. 75	
WEIR STA LFT (FT)	**	VEL TOTAL (FT/S)	1.18	1.18	WEIR STA LFT (FT)		VEL TOTAL (FT/S)	1.91	0. 55	
WEIR STA RGT (FT)		FLOW AREA (SQ FT)	75. 24	75. 09	WEIR STA RGT (FT)		FLOW AREA (SQ FT)	150. 29	522. 35	
WE IR SUBMERG		FROUDE # CHL	0.00	0.00	WEIR SUBMERG		FROUDE # CHL	0.16	0. 04	
WEIR MAX DEPTH (FT)	1	SPECIF FORCE	86. 49	86. 24	WEIR MAX DEPTH (FT)		SPECIF FORCE	275. 70	774.87	
MIN EL WEIR FLOW (FT)	422. 23	HYDR DEPTH	1.68	1.68	MIN EL WEIR FLOW (FT)	422. 23	HYDR DEPTH	2. 52	2. 46	
MIN EL PRS	422. 40	W.P. TOTAL	45. 19	45. 14	MIN EL PRS	422.40	W.P. TOTAL	161.75	317. 06	
DELTA EG	0.02	CONV. TOTAL	7853. 7	7832. 4	DELTA EG	0.06	CONV. TOTAL	12521.0	79222. 1	
DELTA WS	0. 03	TOP WIDTH	44. 71	44. 67	DELTA WS (FT)	0.06	TOP WIDTH	59. 72	211.93	
BR OPEN AREA (SQ FT)	130. 27	FRCTN LOSS	0.00	0.00	BR OPEN AREA (SQ FT)	130. 27	FRCTN LOSS (FT)	0.00	0.00	
BR OPEN VEL	1.18	C & E LOSS (FT)	0.00	0.01	BR OPEN VEL	2. 21	C & E LOSS (FT)	0. 03	0.00	
BR SLUICE COEF		SHEAR TOTAL (LB/SQ FT)	0.01	0.01	BR SLUICE COEF		SHEAR TOTAL (LB/SQ FT)	0. 04	0.00	
BR SEL METHOD	ENERGY	POWER TOTAL (LB/FT S)	0. 02	0.02	BR SEL METHOD	ENERGY	POWER TOTAL (LB/FT S)	0. 07	0.00	
	PLAN: PRO				PLAN: PROP KINGS CREEK RS: 6637					
E.G. US.		PROFILE: 2 YEA			PROFILE: 100 YEAR					
(FT)	421.35	E.G. ELEV		INSIDE BR DS	(FT) W. S. US.	422. 94	ELEMENT E.G. ELEV		INSIDE BR DS	
Q TOTAL	421.28	(FT) W. S. ELEV	421.26	421.26	(FT)	422.81	(FT)	422. 76	422. 68	
(CFS)	1984.27	(FT)	421.22	421.24	(CFS)	9044.84	(FT)	422. 76	422. 68	
Q BRIDGE (CFS)	1247. 76	CRIT W.S.	417.60	417.59	Q BRIDGE (CFS)	1186.82	CRIT W.S.	421.58	421.55	
Q WEIR (CFS)		MAX CHL DPTH (FT)	8. 79	8. 80	Q WEIR (CFS)		MAX CHL DPTH (FT)	10.32	10. 24	
WEIR STA LFT (FT)		VEL TOTAL (FT/S)	1. 39	1.16	WEIR STA LFT (FT)		VEL TOTAL (FT/S)	0. 43	0.49	
WEIR STA RGT (FT)		FLOW AREA (SQ FT)	900. 22	1074.55	WEIR STA RGT (FT)		FLOW AREA (SQ FT)	2785. 56	2429. 18	
WEIR SUBMERG		FROUDE # CHL	0.14	0.12	WEIR SUBMERG		FROUDE # CHL	0. 02	0. 03	
WEIR MAX DEPTH (FT)		SPECIF FORCE (CU FT)	1747. 18	1933. 71	WEIR MAX DEPTH (FT)		SPECIF FORCE (CU FT)	5151.67	4598. 48	
MIN EL WEIR FLOW (FT)	419.82	HYDR DEPTH (FT)	1.50	1.65	MIN EL WEIR FLOW (FT)	419. 22	HYDR DEPTH (FT)	2. 74	2. 75	
MIN EL PRS	421.81	W.P. TOTAL (FT)	620.77	674.51	MIN EL PRS	421.81	W.P. TOTAL (FT)	1177. 99	1046.30	
DELTA EG (FT)	0. 0 6	CONV. TOTAL	130109.4	155516.0	DELTA EG (FT)	Ø . Ø8	CONV. TOTAL	471328.0	404344.3	
DELTA WS	0. 05	TOP WIDTH (FT)	749. 59	652. 90	DELTA WS	0. 15	TOP WIDTH (FT)	1018. 45	882.84	
BR OPEN AREA (SQ FT)	425. 93	FRCTN LOSS (FT)	0. 01	0. 01	BR OPEN AREA (SQ FT)	425. 93	FRCTN LOSS (FT)	0. 01	0. 03	
BR OPEN VEL	3. 14	C & E LOSS (FT)	0. 02	0. 00	BR OPEN VEL	2. 79	C & E LOSS (FT)	0.02	0. 01	
BR SLUICE COEF		SHEAR TOTAL (LB/SQ FT)	0. 02	0. 02	BR SLUICE COEF		SHEAR TOTAL (LB/SQ FT)	0. 05	0. 07	
BR SEL		POWER TOTAL			BR SEL		POWER TOTAL			

- HEC-RAS VERSION 6.3.1 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND BED SLOPE OF 0.0011 FT/FT FOR BACHELOR AND KINGS CREEK UPPER REACH AND 0.0005 FT/FT OR KINGS CREEK LOWER REACH.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0200D EFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHIC DATA NATIONAL ELEVATION DATA SET 2013.
- COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 01/20/2021.







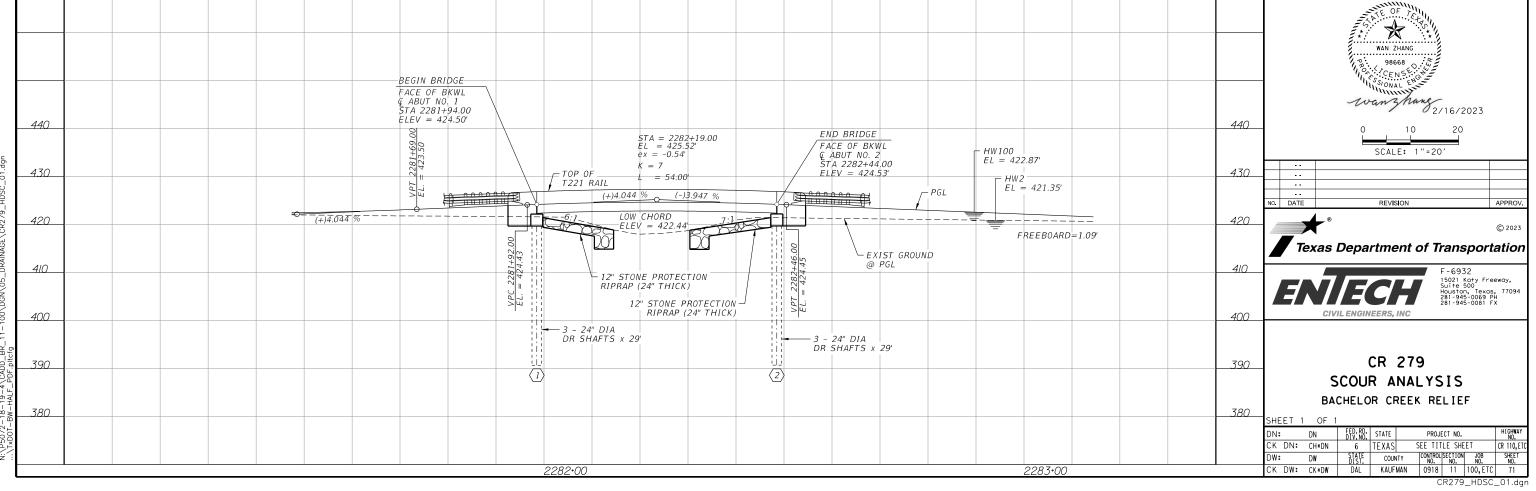
CR 279
HYDRAULIC DATA
BACHELOR CREEK RELIEF

SHEET 4	OF 4				
DN:	ST	FED. RD. DIV. NO.	STATE	PROJECT NO.	H I CHWAY
CK DN:	AV	6	TEXAS	SEE TITLE SHEET	CR 110,ETC
DW:	BG	STÁTE DIST.	COUNTY	CONTROLISECTION JOB NO. NO. NO.	SHEET NO.
CK DW:	AV	DAL	KAUFMAN	0918 11 100, ETC	70

Hydraulic Design Data QC (10-yr)									
Contraction Scour									
	Input Data								
	Channe I								
Average Depth (ft):	1.81								
Approach Velocity (ft/s):	1.99								
BR Average Depth (ft):	1.69								
BR Opening Flow (cfs):	88.81								
BR Top WD (ft):	44.67								
Grain Size D50 (mm) :	0.2								
Approach Flow (cfs):	82.67								
Approach Top WD (ft):	23.05								
K1 Coefficient:	0.69								
Results									
Scour Depth Ys (ft):	-0.46								
Critical Velocity (ft/s) :	1.07								
Equation:	Live								

Hydraulic Design Data QS	(50-yr)
Contraction Scour	-
	Input Data
	Channe I
Average Depth (ft):	3.27
Approach Velocity (ft/s):	2.74
BR Average Depth (ft):	2.52
BR Opening Flow (cfs):	287.31
BR Top WD (ft):	50
Grain Size D50 (mm) :	0.2
Approach Flow (cfs):	214.14
Approach Top WD (ft):	23.9
K1 Coefficient:	0.69
Results	
Scour Depth Ys (ft):	0.01
Critical Velocity (ft/s):	1.18
Equation :	Live

- 1. SCOUR ANALYSIS IS BASED ON TXDOT GEOTECHNICAL MANUAL (GM).
 FHWA H.E.C.-18, "EVALUATING SCOUR AT BRIDGES", 5TH EDITION, AND
 TXDOT "SCOUR EVALUATION GUIDE REVISED AUGUST 2020", CHAPTER 8.
 ABUTMENT SCOUR EQUATIONS IN HEC-18 TEND TO OVER ESTIMATE ABUTMENT
 ESTIMATE ABUTMENT SCOUR DEPTHS. BRIDGE ABUTMENTS WILL BE ARMORED
 WITH STONE PROTECTION RIPRAP. NO APPRECIABLE ABUTMENT SCOUR
 IS ANTICIPATED.
- 2. THE D50 SOIL PARTICLE SIZE FOR THIS PROJECT IS THE MINIMUM SIZE ALLOWED BY THE GM. THE SOIL DATA IS FROM "THE GEOTECHNICAL INVESTIGATION FOR KAUFMAN COUNTY ON & OFF-SYSTEM BRIDGE REPLACEMENT PROJECT" REPORT.
- 3. THE PROPOSED BRIDGE IS A SINGLE SPAN STRUCTURE. PIER SCOUR IS NOT APPLICABLE FOR THIS BRIDGE.
- 4. THE TOTAL MAXIMUM CALCULATED SCOUR DEPTH IS TAKEN AS THE MAXIMUM OF HORIZONTAL CONTRACTION AND VERTICAL PRESSURE SCOUR. THE MAXIMUM SCOUR DEPTH OCCURED DURING THE 50-YR STORM AND IS 2.64'.
- 5. SEE "HYDROLOGY/HYDRAULIC REPORT FOR MUDDY CEDAR TRIBUTARY" DATED NOVEMBER 4TH, 2022 FOR ADDITIONAL INFORMATION.

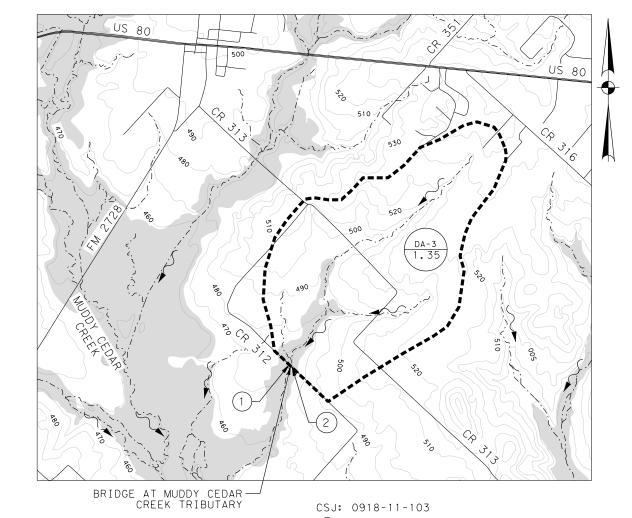


SURFACE RUNOFF DIRECTION

EXISTING CONTOURS FEMA ZONE A AREA

WATERSHED NAME AREA (SQ. MI)

X-X
XX. XX



① BEGIN PROJECT STA 313+97.00

② END PROJECT STA 316+42.00

DA ID	STREAM NAME	ROADWAY	AREA (SQ. MI)	CURVE NUMBER	TIME OF CONCENTRATION (MIN)	LAG TIME (MIN)
DA 3	MUDDY CEDAR CREEK TRIBUTARY	CR-312	1.35	66	102.8	62

NRCS UNIT HYDROGRAPH METHOD IN HEC-HMS									
PEAK DISCHARGE (CFS)									
2-YR	10-YR	50-YR	100-YR						
343.9	794.8	1,390.6	1,672.0						

NOTES:

- 1. RUNOFF COMPUTATIONS PERFORMED WITH HEC-HMS 4.3 AND VERIFIED BY OMEGA EM REGRESSION EQUATION ANALYSIS.
- RAINFALL DEPTHS WERE OBTAINED FROM NATIONAL OCEANIC AND ATMOSPHERIC ADMINISTRATION (NOAA) ATLAS 14, VOLUME 11.
- 3. STORMS WERE MODELED AS 24-HOUR DURATION EVENTS USING TEMPORAL DISTRIBUTION.
- 4. RUNOFF VOLUME WAS COMPUTED USING THE SCS CURVE NUMBER LOSS MODEL.
- 5. SOURCE OF TOPOGRAPHY DATA: 2013 USGS 10FT DEM AND PROJECT TOPOGRAPHIC SURVEY.
- 6. TIME OF CONCENTRATION (Tc) WAS COMPUTED USING NRCS METHOD.



1,500 3,000 SCALE: 1"=3,000'

NO.	DATE	REVISION	APPROV.
		1 @	





CR 312 DRAINAGE AREA MAP MUDDY CEDAR CREEK TRIBUTARY

SHEET	1	OF	1		
N:		RC		FED.RD. DIV.NO.	STATI

HIGHWAY NO. CR 110, ET PROJECT NO. CK DN: WZ SEE TITLE SHEET 6 TEXAS STATE COUNTY
DIST. KAUFMAN

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.0066 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0225D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA: USDA/NRCS-NGCE 2016 LIDAR AND PROJECT TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.



SCALE; NTS

DATE	REVISION	APPROV.
Tex	ras Department of Transpor	© 2023 tation
	F-6932	



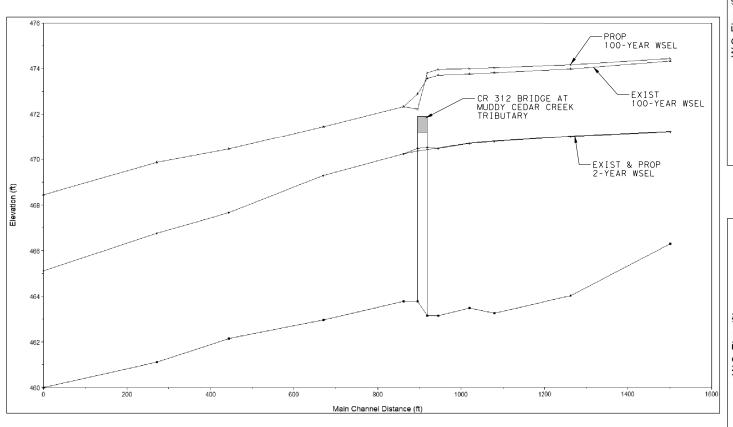
CR 312 HYDRAULIC DATA

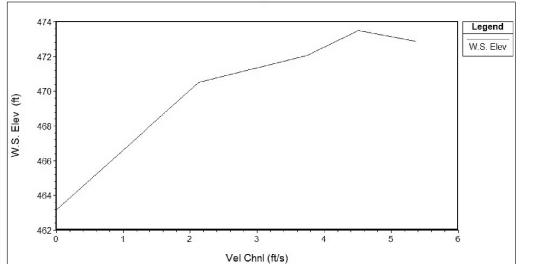
MUDDY CEDAR CREEK TRIBUTARY

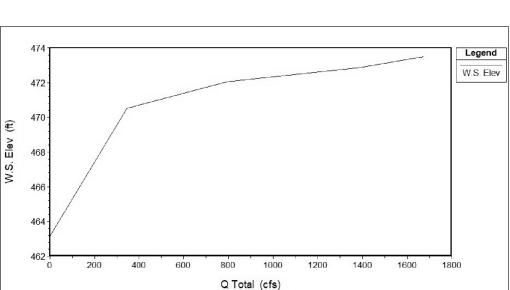
				HEC-RA	S RESULTS	S FOR CR-3	312 AT MUD	DY CEDAR	CREEK TRI	BUTARY					
HEC-RAS STATION	2-YEAR FLOW (CFS)		2	2-YEAR WSEL 2			2-YEAR VELOCITY		100-YEAR FLOW (CFS)		100-YEAR WSEL			100-YEAR VELOCITY	
JIAITON	EXISTING	PROPOSED	EXISTING	PROPOSED	DIFF.	EXISTING	PROPOSED	EXISTING	PROPOSED	EXISTING	PROPOSED	DIFF.	EXISTING	PROPOSED	
1848	344	344	471.25	471.23	-0.02	5.91	5.95	1672	1672	474.34	474.45	0.11	3.82	3.54	
1610	344	344	471.03	471.01	-0.02	2.54	2.55	1672	1672	473.98	474.16	0.18	3.54	3.21	
1428	344	344	470.83	470.80	-0.03	2.72	2.73	1672	1672	473.82	474.04	0.22	3.02	2.70	
1367	344	344	470.74	470.71	-0.03	2.87	2.90	1672	1672	473.78	474.01	0.23	2.85	2.51	
1293 (U/S ROW)	344	344	470.51	470.48	-0.03	3,55	3.58	1672	1672	473.72	473.97	0.25	2.82	2.47	
1270					CR-	312 BRIDGE	AT MUDDY	CEDAR CR	EEK TRIBU	TARY				•	
1210 (D/S ROW)	344	344	470.25	470.25	0.00	3.72	3.72	1672	1672	472.34	472.34	0.00	5.98	5.98	
1018	344	344	469.30	469.30	0.00	4.57	4.57	1672	1672	471.44	471.44	0.00	5.25	5.25	
792	344	344	467.68	467.68	0.00	5.47	5.47	1672	1672	470.47	470.47	0.00	5.52	5.52	
619	344	344	466.76	466.76	0.00	4.82	4.82	1672	1672	469.87	469.87	0.00	5.28	5.28	
347	344	344	465.13	465.13	0.00	5.30	5.30	1672	1672	468.45	468.45	0.00	6.95	6.95	

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.0066 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0225D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA: USDA/NRCS-NGCE 2016 LIDAR AND PROJECT TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.

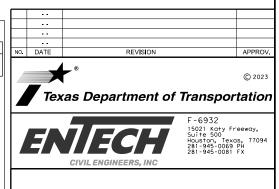
HYDRAULIC DATA MUDDY CEDAR CREEK TRIBUTARY BRIDGE







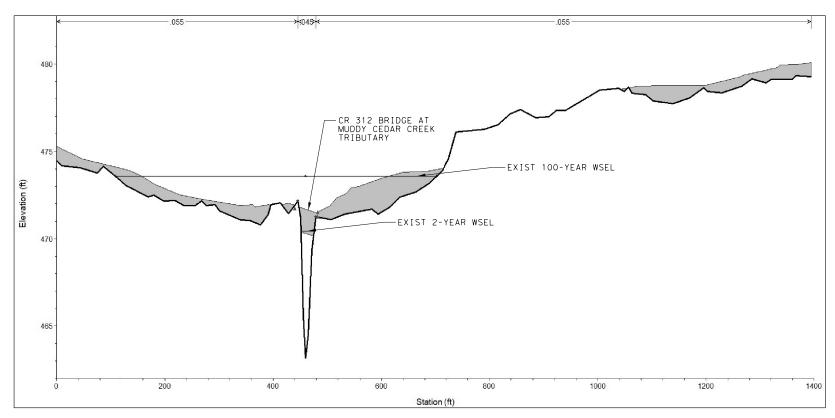




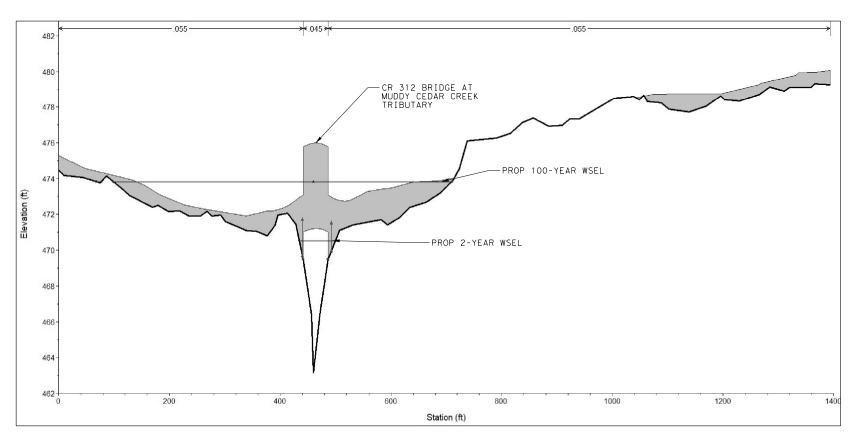
CR 312
HYDRAULIC DATA
MUDDY CEDAR CREEK TRIBUTARY

SHEET 2 OF 4

,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,								
N:	RC	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	WZ	6	TEXAS	S	EE TIT	LL 511L		CR 110,ETC
OW:	RC	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	W7	DΔI	KΔHF	MΔN	0918	11	100. FTC	74



EXISTING CR 312 BRIDGE AT MUDDY CEDAR CREEK TRIBUTARY



PROPOSED CR 312 BRIDGE AT MUDDY CEDAR CREEK TRIBUTARY

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.0066 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0350D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA:
 USDA/NRCS-NGCE 2016 LIDAR AND PROJECT
 TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.

HYDRAULIC DATA MUDDY CEDAR CREEK TRIBUTARY BRIDGE



Texas Department of Transportation

F-6932
1501 katy Freeway,
Suite 500
Houston, Texas, 77094
281-945-0081 FX

CR 312
HYDRAULIC DATA
MUDDY CEDAR CREEK TRIBUTARY

SHEET 3 OF 4

011221 0	, 0, ,							
ON:	RC	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	WZ	6	TEXAS	,	EE TIT	LL JIIL		CR 110,ETC
OW:	RC	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	W7	DAI	KAUF	MAN	0918	11	100, FTC	75

		CR-312		
Plan: PR	River 1	Reach 1 RS: 1270	Profile	e: 2-year
E.G. US. (f+)	470.68	Element	Inside BR US	Inside BR DS
W.S. US. (f+)	470.48	E.G. Elev (ft)	470.58	470.56
Q Total (cfs)	343.90	W.S. Elev (ft)	470.51	470.48
Q Bridge (cfs)	343.90	Crit W.S. (ft)	467.46	467.52
Q Weir (cfs)		Max Chi Dpth (ft)	7.35	6.70
Weir Sta Lft (ft)		Vel Total (ft/s)	2.13	2.16
Weir Sta Rgt (ft)		Flow Area (sq ft)	161.44	159.00
Weir Submerg		Froude # Chl	0.14	0.15
Weir Max Depth (ft)		Specif Force (cu ft)	382.28	370.35
Min El Weir Flow (ft)	471.92	Hydr Depth (ft)	3.59	3.53
Min El Prs (ft)	471.72	W.P. Total (ft)	49.54	48.78
Delta EG (ft)	0.21	Conv. Total (cfs)	11716.40	11863.50
Delta WS (ft)	0.23	Top Width (ft)	45.00	45.00
BR Open Area (sq ft)	211.16	Frctn Loss (ft)	0.02	0.05
BR Open Vel	2.16	C & E Loss (ft)	0.00	0.04
BR Sluice Coef		Shear Total (Ib/sq ft)	0.18	0.17
BR Sel Method	Energy only	Power Total (lb/ft s)	0.37	0.37

		CR-312		
Plan: PR	River 1 F	Reach 1 RS: 1270	Profile:	100-year
E.G. US. (f+)	473.80	Element	Inside BR US	Inside BR DS
W.S. US. (f+)	473.74	E.G. Elev (ft)	473.70	473.22
Q Total (cfs)	1672.00	W.S. Elev (ft)	473.49	472.30
Q Bridge (cfs)	958.47	Crit W.S. (ft)	470.42	470.46
Q Weir (cfs)		Max Chl Dpth (ft)	10.33	8.52
Weir Sta Lft (ft)		Vel Total (ft/s)	3.05	7.03
Weir Sta Rgt (ft)		Flow Area (sq ft)	548.28	237.90
Weir Submerg		Froude # Chl	0.20	0.46
Weir Max Depth (ft)		Specif Force (cu ft)	1335.78	1101.68
Min El Weir Flow (ft)	471.92	Hydr Depth (ft)	1.41	1.71
Min El Prs (ft)	471.72	W.P. Total (ft)	486.40	234.98
Delta EG (ft)	1.12	Conv. Total (cfs)	20708.30	12388.50
Delta WS (ft)	1.41	Top Width (ft)	389.79	139.16
BR Open Area (sq ft)	211.16	Frctn Loss (ft)	0.27	0.26
BR Open Vel (ft/s)	4.54	C & E Loss (ft)	0.21	0.29
BR Sluice Coef		Shear Total (Ib/sq ft)	0.46	1.15
BR Sel Method	Energy only	Power Total (lb/ft s)	1.40	8.09

- 1. HEC-RAS 6.2 USED FOR THE ANALYSIS.
- 2. ALL ELEVATIONS BASED ON THE NAV88 VERTICAL DATUM.
- 3. THE STARTING WATER SURFACE ELEVATION WAS BASED ON NORMAL DEPTH CALCULATION AND A BED SLOPE OF 0.0066 FT/FT.
- 4. SITE IS DESIGNATED AS FEMA ZONE A AS ON PANEL 48257C0225D EFFECTIVE JULY 3, 2012.
- 5. SOURCE OF TOPOGRAPHY DATA:
 USDA/NRCS-NGCE 2016 LIDAR AND PROJECT
 TOPOGRAPHIC SURVEY.
- 6. COORDINATION WITH THE LOCAL FLOODPLAIN ADMINISTRATOR WAS PERFORMED ON 07/30/2022.



ı				
ſ				
ı				
ı				
I	NO.	DATE	REVISION	APPROV.
ſ			1 -	





F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

CR 312
HYDRAULIC DATA
MUDDY CEDAR CREEK TRIBUTARY

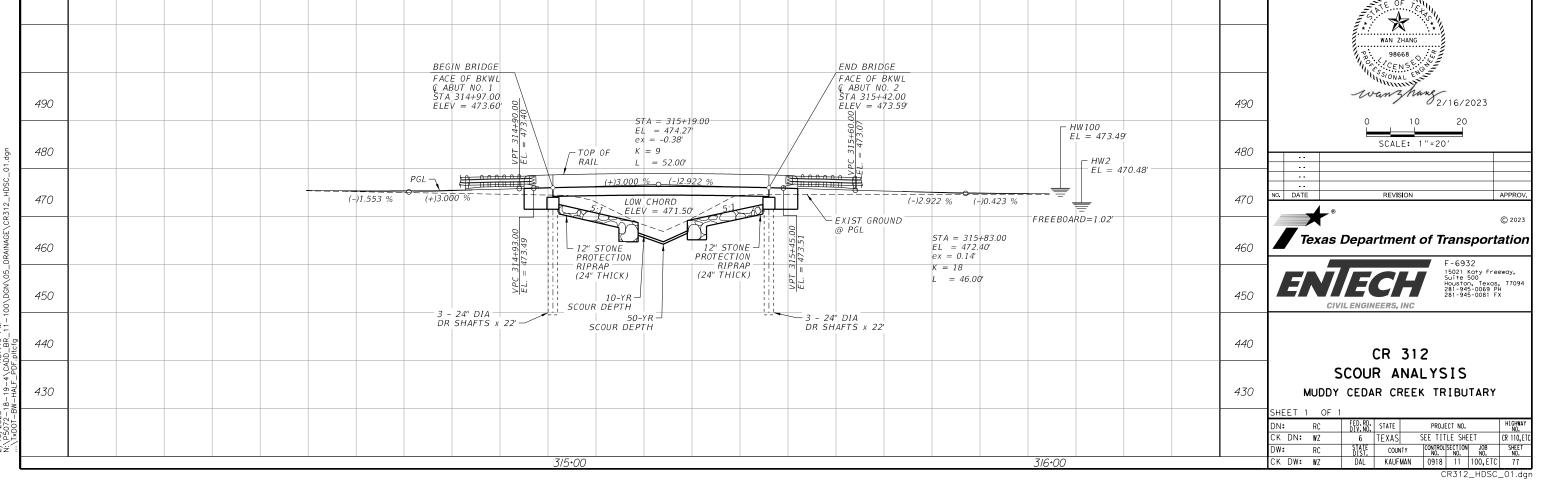
SHEET 4 OF 4

0.1221	. 0							
ON:	RC	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
CK DN:	WZ	6	TEXAS	,	EE TIT	LL JIIL		CR 110,ETC
OW:	RC	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	W7	DAI	KAUF	MAN	0918	11	100, FTC	76

Hydraulic Design Data QC	(10-yr)
Contraction Scour	_
	Input Data
	Channe I
Average Depth (ft):	4.40
Approach Velocity (ft/s) :	3.28
BR Average Depth (ft):	4.40
BR Opening Flow (cfs):	575
BR Top WD (ft):	45
Grain Size D50 (mm) :	0.2
Approach Flow (cfs):	575
Approach Top WD (ft):	80
K1 Coefficient:	0.69
Results	
Scour Depth Ys (ft):	2.14
Critical Velocity (ft/s):	1.24
Equation:	Live

Hydraulic Design Data QS	(50-yr)
Contraction Scour	-
	Input Data
	Channe I
Average Depth (ft):	5.41
Approach Velocity (ft/s):	2.95
BR Average Depth (ft):	5.41
BR Opening Flow (cfs):	637
BR Top WD (ft):	45
Grain Size D50 (mm) :	0.2
Approach Flow (cfs):	637
Approach Top WD (ft):	80
K1 Coefficient:	0.69
Results	
Scour Depth Ys (ft):	2.64
Critical Velocity (ft/s):	1.29
Equation :	Live

- 1. SCOUR ANALYSIS IS BASED ON TXDOT GEOTECHNICAL MANUAL (GM).
 FHWA H.E.C.-18, "EVALUATING SCOUR AT BRIDGES", 5TH EDITION, AND
 TXDOT "SCOUR EVALUATION GUIDE REVISED AUGUST 2020", CHAPTER 8.
 ABUTMENT SCOUR EQUATIONS IN HEC-18 TEND TO OVER ESTIMATE ABUTMENT
 ESTIMATE ABUTMENT SCOUR DEPTHS. BRIDGE ABUTMENTS WILL BE ARMORED
 WITH STONE PROTECTION RIPRAP. NO APPRECIABLE ABUTMENT SCOUR
 IS ANTICIPATED.
- 2. THE D50 SOIL PARTICLE SIZE FOR THIS PROJECT IS THE MINIMUM SIZE ALLOWED BY THE GM. THE SOIL DATA IS FROM "THE GEOTECHNICAL INVESTIGATION FOR KAUFMAN COUNTY ON & OFF-SYSTEM BRIDGE REPLACEMENT PROJECT" REPORT.
- 3. THE PROPOSED BRIDGE IS A SINGLE SPAN STRUCTURE. PIER SCOUR IS NOT APPLICABLE FOR THIS BRIDGE.
- 4. THE TOTAL MAXIMUM CALCULATED SCOUR DEPTH IS TAKEN AS THE MAXIMUM OF HORIZONTAL CONTRACTION AND VERTICAL PRESSURE SCOUR. THE MAXIMUM SCOUR DEPTH OCCURED DURING THE 50-YR STORM AND IS 2.64'.
- 5. SEE "HYDROLOGY/HYDRAULIC REPORT FOR MUDDY CEDAR TRIBUTARY" DATED NOVEMBER 4TH, 2022 FOR ADDITIONAL INFORMATION.



SUMMARY OF ESTIMATED QUANTITIES ①									
	0416 6002	0420 6014	0422 6002	0425 6012	0450 6005	0454 6003			
ITEM	DRILL SHAFT (24 IN)			PRESTR CONC SLAB BEAM (5SB15)	RAIL (TY T221) (HPC)	ARMOR JOINT			
DESCRIPTION									
	LF	CY	SF	LF	LF	LF			
ABUTMENTS (1 AND 2)	114	19				46			
45.00' PRESTR CONC SLAB BEAM UNIT TYPE 5SB15			1170	221.41	114				
TOTAL	114	19	1170	221.41	114	46			

1) PROP NBI NUMBER: 18-130-0-AA03-47-003





F -6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0089 PH 281-945-0081 FX

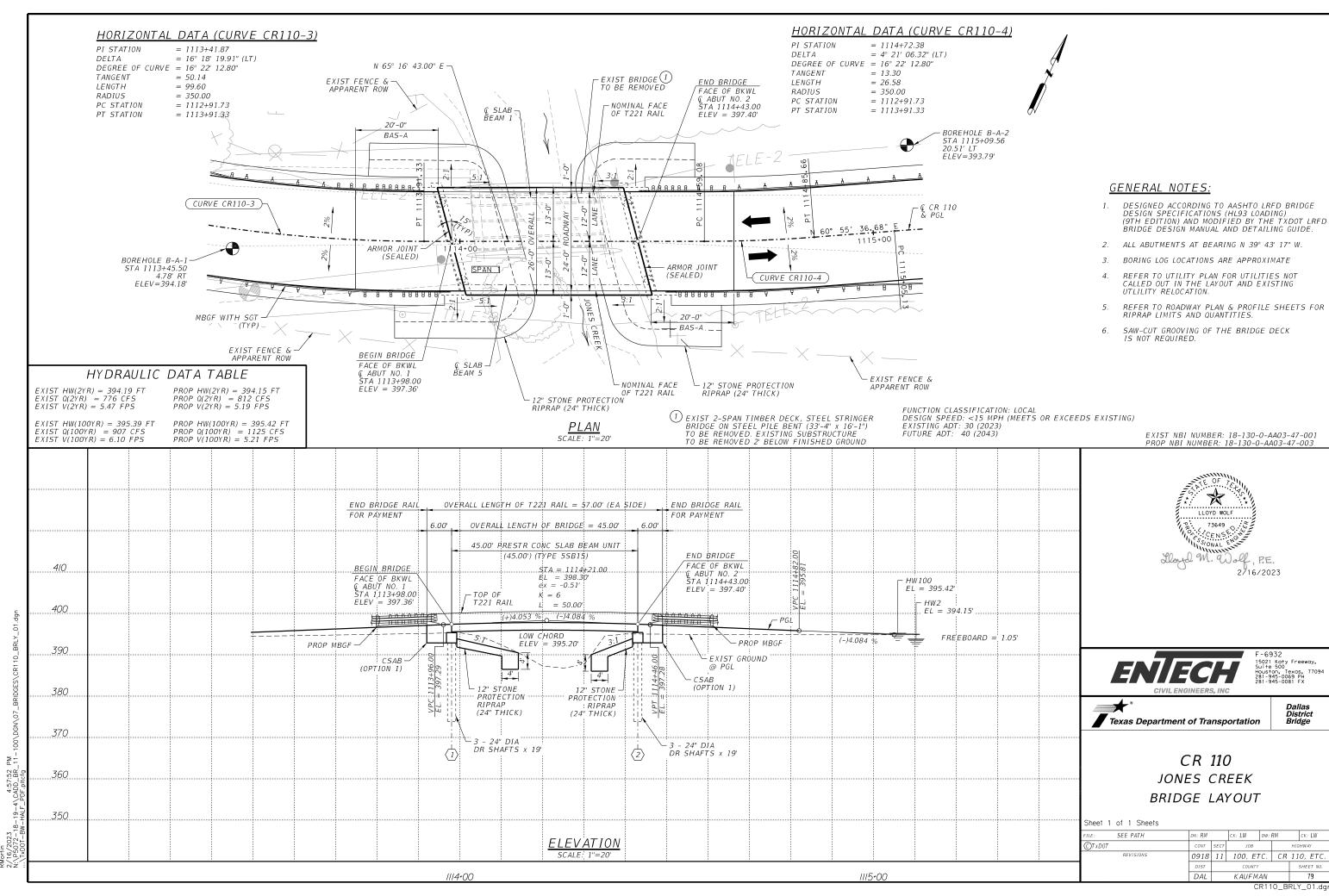


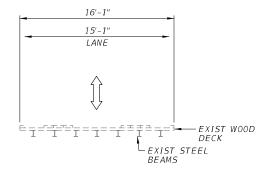
Dallas District Bridge

CR 110 JONES CREEK ESTIMATED QUANTITIES

Sheet 1 of 1 Sheets

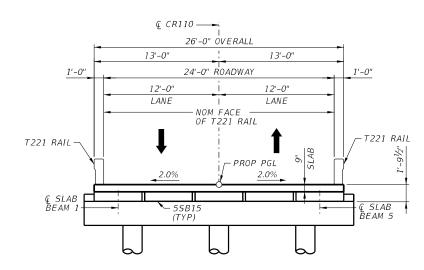
SEE PATH	DN: RM		ck: LW	ow: RM			CK: LW
OT	CONT	SECT	JOB		HIGHWAY		HWAY
	0918	11	100, ETC. CR COUNTY KAUFMAN		CR	CR 110, ETC.	
	DIST					SHEET NO.	
	DAL				78		





EXISTING BRIDGE SECTION

TO BE REMOVED



PROPOSED BRIDGE TYPICAL SECTION

SCALE 1"=10'







Dallas District Bridge

CR 110 JONES CREEK TYPICAL SECTIONS

Sheet 1 of 1 Sheets					
FILE: SEE PATH	DN: RM		ck: LW	ow: RM	CK: LW
©T×D0T	CONT	SECT	JOB		HIGHWAY
	0918	11	100, ET	C. CR	110, ETC.
	DIST		COUNTY		SHEET NO.
	DAL		KAUFM.	AN	80







Dallas District Bridge

CR 110 JONES CREEK CAP ELEVATIONS

Sheet 1 of 1 Sheets						
FILE: SEE PATH	DN: RM		CK: LW	DW:	RM	CK: LW
©T×D0T	CONT	SECT	JOB			HIGHWAY
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4 <i>N</i>		81

DRILLING LOG

1 of 2

WinCore Version 3.3 County Kaufman Highway CR 110 CSJ 0918-11-100

B-A-01 Bridge Structure Station

1113+45.50 4.78' RT

Dallas District Date 7-12-22 Grnd. Elev. 394.18 ft GW Elev. 380.18 ft

Organization: B2Z Engineering

	L	Texas Cone		Triaxi	al Test		Prop	ertie	s	
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	ΡI	Wet Den. (pcf)	Additional Remarks
			CLAY, Sandy Lean Clay, Dk. Brown to Brown, Med. Stiff to Stiff, Dry to Moist (CL)		. ,					
5		8 (6) 9 (6)				15.1				-200 = 57.6%
10		11 (6) 16 (6)				18.3	32	14		-200 = 57.8%
15		6 (6) 7 (6)				21.7				-200 = 60.7%
4.2 20		50 (1) 50 (0.5)	SHALE, Shale, Lt. Gray to Dk. Gray to Gray, Hard, Wet (Comprised Primarily of Fat Clay) (CH)			16.5	48	33		
25		50 (1) 50 (0.5)	Filmaniy or Fat Clay) (CTI)			62.3				-200 = 97.6%
30		50 (1) 50 (0.5)				54.9	53	34		
35		50 (1.5) 50 (1.5)				38.8				-200 = 96.7%
40		50 (1) 50 (0.5)				43.0	51	34		
40										
45		50 (1) 50 (0)				44.5				-200 = 97.6%
50		50 (0) 50 (0)				51.0	49	32		
9.2 55		50 (0) 50 (0)	SAND, Clayey Sand, Lt. Gray to Gray, Very Dense, Wet (SC)			39.8				-200 = 44.1%
60		50 (2) 50 (1)				34.2	32	15		
Remark	s: X	Y Coord 2690180	0.0464, 6887816.5394 (Provided By Clien	t).						
			rmation provided on this boring log is reprected. The actual groundwater elevation m							

Logger: JR

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8, Wincore\A - CR 110\B-A-01 & B-A-02.CLG

DRILLING LOG

WinCore Version 3.3 County Kaufman Highway CR 110 CSJ 0918-11-100

Structure Bridge Station Offset

1113+45.50 4.78' RT

B-A-01

Dallas District Date 7-12-22 Grnd. Elev. 394.18 ft GW Elev. 380.18 ft

2 of 2

	L	Texas Cone		Triaxial Test		Prop	ertie		
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Deviator Press. Stress (psi) (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
			SAND, Clayey Sand, Lt. Gray to Gray, Very Dense, Wet (SC)	(FSS)					
65 -		50 (1.5) 50 (1)			28.2				-200 = 30.3%
		50 (2) 50 (1)			24.6	20	12		
70 - - -		33 (2) 33 (1)			24.0	23	14		
75 -		50 (1.5) 50 (0.5)			48.0				-200 = 34.8%
- - 14.2 80 -		50 (1.5) 50 (0.5)			37.8	40	23		
- - -									
85 -									
90 -									
-									
95 -									
100-									
- -									
105									
110									
-									
115									
120									

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8. Wincore\A - CR 110\B-A-01 & B-A-02.CLG







Dallas District Bridge

CR 110 JONES CREEK BORING LOGS

Sheet 1 of 2 Sheets						
FILE: SEE PATH	DN: RM		CK: LW	DW:	RM	ck: LW
©TxD0T	CONT	SECT	JOB			HIGHWAY
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	AN		82

DRILLING LOG

1 of 2

WinCore Version 3.3

County Kaufman Highway CR 110 CSJ 0918-11-100

B-A-02 Bridge Structure Station

1115+09.56 20.51' LT

District Date

Dallas 7-13-22 Grnd. Elev. 393.79 ft GW Elev. 374.79 ft

	L	Texas Cone			al Test		LIOP	ertie		·
Elev. (ft)	O G	Penetrometer	Strata Description	Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
			CLAY, Lean Clay w/ Sand to Sandy							
	\forall		Lean Clay, Dk. Grayish Brown to Brown, Med. Stiff to Stiff, Dry							
		5 (6) 5 (6)	to Moist (CL)							
5 -		3 (6) 3 (6)				8.4				-200 = 78.8%
10		7 (6) 9 (6)				19.2	39	23		-200 = 63.9%
		7 (6) 9 (6)				20.2				-200 = 47.9%
78.8 15			SAND, Clayey Sand, Grayish Brown,							200 47.070
	-		Loose, Moist (SC)							
		50 (4 5) 50 (0 5)								
73.8 20		50 (1.5) 50 (0.5)	SHALE, Shale, Lt. Gray to Gray,			18.8	55	37		
			Hard, Moist to Wet (Comprised							
	+=		of Fat Clay) (CH)							
25		50 (1) 50 (0.5)				55.8				-200 = 94.3%
	+=									
	- 🗐	50 (1) 50 (0.5)				52.5	56	35		
30		00 (1) 00 (0.0)				32.3	30	33		
	-									
35	-	50 (1.5) 50 (0.5)				51.9				-200 = 97.0%
	+=									
40		50 (1) 50 (0)				49.6	57	37		
	-									
	튈									
	-	50 (1) 50 (0)				51.1				-200 = 95.5%
45	Ħ	00 (1) 00 (0)				31.1				-200 - 95.5%
	-									
	重									
50	-	50 (1.5) 50 (1)				52.3	58	32		
	囯									
	-									
38.8 55	臣	50 (1.5) 50 (0.5)				59.8				-200 = 59.0%
	-		SHALE, Shale, Lt. Gray, Hard,							
			Wet (Comprised of Sandy Lean Clay) (CL)							
	ŧ	50 (1.5) 50 (0)	` ′			44.7	40	25		
60	-	() (-)		\vdash		44.7	40	20		1

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location

where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8, Wincore\A - CR 110\B-A-01 & B-A-02.CLG

WinCore

Version 3.3

DRILLING LOG

County Kaufman Highway CR 110 0918-11-100 CSJ

B-A-02 Structure Bridge Station 1115+09.56 Offset 20.51' LT

Dallas Date 7-13-22 Grnd. Elev. 393.79 ft GW Elev. 374.79 ft

District

2 of 2

	L	Texas Cone			al Test		Prop	ertie		
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SHALE, Shale, Lt. Gray, Hard, Wet (Comprised of Sandy Lean Clay) (CL)		. ,				. ,	
28.8 65 - - -		50 (0.5) 50 (0)	SAND, Clayey Sand, Lt. Gray to Gray, Very Dense to Dense, Wet			34.3				-200 = 37.2%
70 -		50 (0) 50 (0)	(SC)			26.1	30	15		
- - 75 -		50 (3) 50 (3)				42.8				-200 = 33.1%
- - -		30 (6) 50 (5)				41.6	30	21		
13.8 80 - - - -		(-)				41.0	00	-1		
85 - -										
90 -										
95 -										
- - 100-										
-										
105- - -										
110										
115										
120-										

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8. Wincore\A - CR 110\B-A-01 & B-A-02.CLG







Dallas District Bridge

CR 110 JONES CREEK BORING LOGS

Sheet 2 of 2 Sheets

FILE: SEE PATH	DN: RM		ck: LW		ck: LW					
©TxD0T	CONT	SECT JOB				HIGHWAY				
	0918	11	100, ET	С.	CR	11	0, ETC.			
	DIST		COUNTY			5	HEET NO.			
	DAL		KAUFM.	4N			83			
CD110 DDIC 02 1-										

SUMMA	RY OF ES	STIMATED	QUANTIT	IES ①		
	0416 6002	0420 6014	0422 6002	0425 6012	0450 6005	0454 6003
ITEM	DRILL SHAFT (24 IN)	CL C CONC (ABUT) (HPC)	REINF CONC SLAB (HPC)	PRESTR CONC SLAB BEAM (5SB15)	RAIL (TY T221) (HPC)	ARMOR JOINT
DESCRIPTION	I F	СҮ	SF	LF	LF	LF
	LI	CI	31	LI	LI	LI
ABUTMENTS (1 AND 2)	174	18.4				44
50.00' PRESTR CONC SLAB BEAM UNIT TYPE 5SB15			1300	247.50	124	
TOTAL	174	18.4	1300	247.50	124	44

1) PROP NBI NUMBER: 18-130-0-AA02-62-005



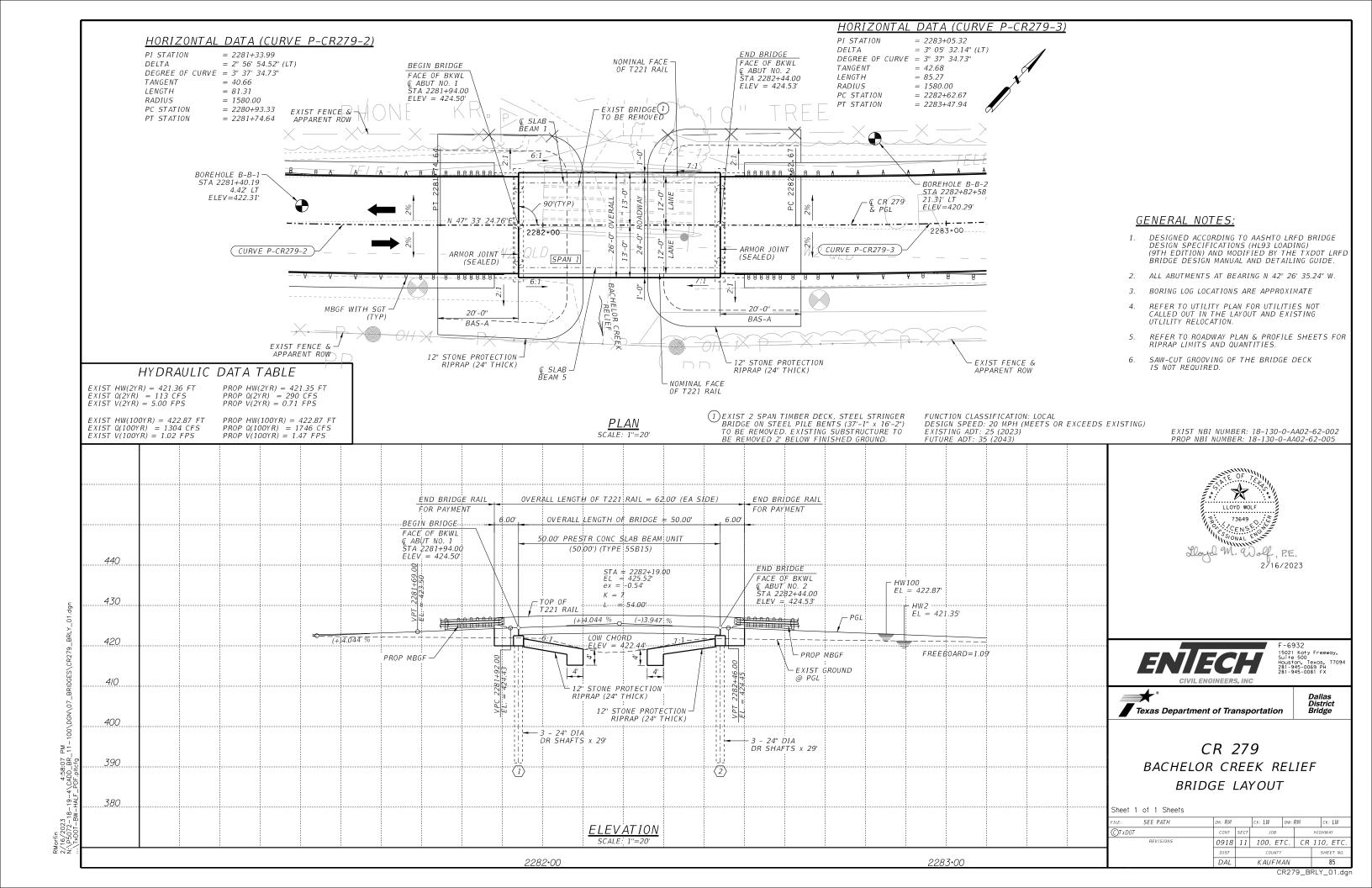


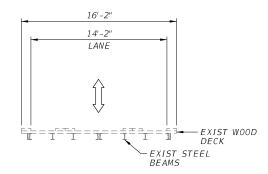


Dallas District Bridge

CR 279 BACHELOR CREEK RELIEF ESTIMATED QUANTITIES

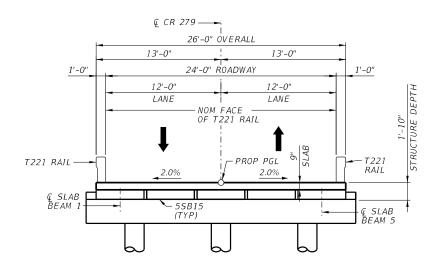
Sheet 1 of 1 Sheets							
FILE: SEE PATH	DN: RM		CK: LW	DW:	RM		ck: LW
©TxD0T	CONT	SECT	JOB			HIGH	łWAY
	0918	11	100, ET	С.	CR	110), ETC.
	DIST		COUNTY			5	HEET NO.
	DAL		KAUFM.	AN			84





EXISTING BRIDGE SECTION

TO BE REMOVED



PROPOSED BRIDGE TYPICAL SECTION

SCALE 1"=10'







Dallas District Bridge

CR 279 BACHELOR CREEK RELIEF TYPICAL SECTIONS

Sheet 1 of 1 Sheets						
FILE: SEE PATH	DN: RM		CK: LW	ow: RM		CK: LW
©T x D O T	CONT	SECT	JOB		HI	SHWAY
	0918	11	100, ET	C. C.	R 11	10, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4N		86







Dallas District Bridge

CR 279 BACHELOR CREEK RELIEF CAP ELEVATIONS

Sheet 1 of 1 Sheets						
FILE: SEE PATH	DN: RM		CK: LW	DW: F	RM	ck: LW
©T×D0T	CONT	SECT	JOB			HIGHWAY
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4 <i>N</i>		87

DRILLING LOG

1 of 2

County Kaufman WinCore Highway CR 279 Version 3.3 CSJ 0918-11-101

B-B-01 Bridge Structure 2281+40.19 Station 4.42' LT

		B-B-01					District		Dallas
	ure	Bridge					Date		7-14-22
ı	n	2281+40	.19				Grnd. Ele	ev.	422.31 ft
		4.42' LT					GW Elev.		398.31 ft
	Triaxia	al Test		Prop	ertie	es			
	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den.		Add	litional Re

Organization: B2Z Engineering

Elev. (ft)	0	Texas Cone	Strata Description							Additional Dan	
` '	G	Penetrometer	Strata Description	Lateral Deviat Press. Stres (psi) (psi)	s MC	L	L	Ы	Wet Den. (pcf)	Additional Remarks	
_			SAND, Clayey Sand, Lt. Brown,	(F=-)					(1)		
			Med. Dense, Dry (SC)								
		11 (6) 12 (6)			4.6	,	4			-200 = 34.2%	
5 -		(-) (-)			4.0		_	•		-200 - 34.2 /6	
-											
		10 (6) 12 (6)									
12.3 10 -		10 (0) 12 (0)	CLAY, Sandy Lean Clay, Lt. Grayish		14.8	<u> </u>				-200 = 51.4%	
_			Brown to Grayish Brown, Stiff								
		42 (6) 40 (6)	to Very Stiff, Dry to Moist (CL)								
15 —		13 (6) 19 (6)			10.3	3	7_	26			
_											
20 -		11 (6) 13 (6)			21.6	<u> </u>				-200 = 58.2%	
-											
25 -		12 (6) 23 (6)			19.1	_ 2	8	17			
_											
_											
92.3 30 [—]		47 (6) 49 (6)			27.6	3				-200 = 89.0%	
-			CLAY, Fat Clay, Grayish Brown to Gray, Hard, Moist to Wet (CH)								
			to Gray, Flara, Moist to Wet (City								
35 —		47 (6) 50 (5)			41.7	' 6	5	45			
-											
40 -		50 (6) 47 (6)			31.8	3				-200 = 91.5%	
-											
- 77.3 45 -		50 (3.5) 50 (1)			55.3	3 6	7	46			
11.3 45			SHALE, Shale, Gray, Hard, Wet			_					
_			(Comprised of Fat Clay and Fat Clay w/ Sand) (CH)								
	Ē	50 (0.5) 50 (0.5)	, (,		61.8	2				-200 = 91.2%	
50 -		, , , ,								200 01.270	
_											
	F	50 (0.5) 50 (0.5)			46.0	, 5	'n	36			
55 — —		()			40.0			50		1	
-											
		50 (1) 50 (0.5)								200 - 20 00/	
60 -	=		0 0000 000007 5040 (Burnell 15 5"	4)	60.9		_			-200 = 82.0%	
Remarks	5. XY	Coora 2651960	6.9360, 6930037.5840 (Provided By Clie	nı).							

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8, Wincore\B - CR 279\B-8-01 & B-8-02.CLG

WinCore

Version 3.3

DRILLING LOG

B-B-01

County Kaufman Highway CR 279 CSJ 0918-11-101

Structure Bridge Station 2281+40.19 Offset 4.42' LT

District Date Grnd. Elev. 422.31 ft GW Elev. 398.31 ft

Dallas

7-14-22

	L	Texas Cone		Triaxial Test		Pro	perti		
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Deviator Press. Stress (psi) (psi)	мс	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SHALE, Shale, Gray, Hard, Wet (Comprised of Fat Clay and Fat	u , u ,					
-	臣		Clay w/ Sand) (CH)						
65 -	E	50 (1.5) 50 (0.5)			56.1	70	47		
-	ŧ								
-									
70 -	Ē	50 (1) 50 (0.5)			51.5				-200 = 89.7%
70 -									
-	Ē								
-	Ē	50 (1) 50 (0.5)			53.1	75	E2		
75 -	Ē	00 (1) 00 (0.0)			33.1	73	33		
-	ŧ								
-		50 (0.5) 50 (0.5)							
42.3 80 -	F	50 (0.5) 50 (0.5)			41.8				-200 = 49.3%
-									
-	+								
85 -	1								
-	+								
-									
-	+								
90 -]								
-	+								
-	7								
95 -	1								
	-								
-	1								
100-	+								
-	1								
-	+								
105	1								
-	+								
	1								
110-	+								
110-]								
-	1								
	4								
115	1								
-	4								
-	1								
120									

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Lab\8. Wincore\8 - CR 279\B-B-01 & B-B-02.CLG







Dallas District Bridge

CR 279 BACHELOR CREEK RELIEF BORING LOGS

FILE: SEE PATH						
FILE: SEE PAIN	DN: RM		ck: LW	DW:	RM	CK: LW
©T×D0T	CONT SECT JOB			HIGHWAY		
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4N		88

DRILLING LOG

1 of 2

WinCore Version 3.3

County Kaufman Highway CR 279 CSJ 0918-11-101

B-B-02 Bridge Structure Station 21.31' LT

2282+82.58

Dallas District Date 7-15-22 Grnd. Elev. 420.29 ft GW Elev. 399.29 ft

Organization: B2Z Engineering

	L	Texas Cone		Triaxial 1	Test		Properti	es	
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral De Press. St (psi) (p	viator tress osi)	МС	LL PI	Wet Den. (pcf)	Additional Remarks
-			CLAY, Lean Clay w/ Sand, Dk. Brown to Brown, Very Stiff, Dry (CL)					``	
5 -		11 (6) 22 (6)				13.6	35 23		-200 = 76.0%
-									
i10.3 10 -		10 (6) 12 (6)	SAND, Clayey Sand, Brown to Lt.			19.3			-200 = 34.0%
-			Brown, Med. Dense, Dry to Moist (SC)						
15 -		11 (6) 14 (6)				13.9	36 22		
-		6 (6) 7 (6)				40.5			200 - 54 704
.00.3 20 - - -		0 (0) 7 (0)	CLAY, Sandy Lean Clay, Lt. Brown to Lt. Grayish Brown, Med. Stiff			16.5			200 = 51.7%
-			to Very Stiff, Moist to Wet (CL)						
25 -		13 (6) 19 (6)				21	42 27		_
-									
90.3 30 -		28 (6) 50 (5.5)	CLAY, Fat Clay, Gray, Hard, Wet			24.8			-200 = 94.4%
-			(CH)						
85.3 35 -		50 (3) 50 (2.5)				47.5	55 40		
- - -			SHALE, Shale, Gray to Dk. Gray, Hard, Wet (Comprised Primarily of Fat Clay w/ Sand) (CH)						
40 -		50 (2.5) 50 (2.5)				51.2			-200 = 76.3%
-	Ē								
45 -	Ē	50 (2) 50 (0.5)				52.0	56 37		
-	Ē								
50 -	Ē	50 (1) 50 (0.5)				67.9			-200 = 88.7%
-	Ē								
55 -		50 (0.5) 50 (0.5)				47.9	45 30		-
-									
60 -	F	50 (1) 50 (0)				65.0			-200 = 79.5%
	s: X\	Coord 2652060	0.5143, 6930145.7070 (Provided By Client	t).					
			rmation provided on this boring log is repre- cted. The actual groundwater elevation ma						

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\Info from Lab\8, Wincore\B - CR 279\B-8-01 & B-8-02.CLG

WinCore

Version 3.3

DRILLING LOG

B-B-02

County Kaufman Highway CR 279 CSJ 0918-11-101

Structure Bridge Station 2282+82.58 Offset 21.31' LT

Dallas District Date 7-15-22 Grnd. Elev. 420.29 ft GW Elev. 399.29 ft 2 of 2

	L	Texas Cone			al Test					
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
			SHALE, Shale, Gray to Dk. Gray, Hard, Wet (Comprised Primarily of Fat Clay w/ Sand) (CH)		. ,				,	
	ŧ	50 (1) 50 (0.5)	or at only in ourily (ori)			56.5	62	44		
65		00 (1) 00 (0.0)				36.3	02	44		
	E									
	Ē	50 (1) 50 (0.5)								
70		30 (1) 30 (0.3)				56.5				-200 = 84.1%
	Ē									
		50 (3) 50 (2)								
75	Ē	30 (3) 30 (2)				60.3	67	48		
	▐									
		EO (4) EO (0 E)								
0.3 80	F	50 (1) 50 (0.5)				48.3				-200 = 51.7%
	-									
85	+									
	11									
90	+									
	+									
95	-									
	1									
	+ 1									
100	7									
	+									
105	1									
	+									
110	1									
	1									
	+									
115	1									
	+									
	-									
120	1									
		Coord DEFONE	0.5143, 6930145.7070 (Provided By Cli	ant)						

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Lab\8. Wincore\8 - CR 279\B-B-01 & B-B-02.CLG







Dallas District Bridge

CR 279 BACHELOR CREEK RELIEF BORING LOGS

Sheet 2 of 2 Sheets

FILE:	SEE PATH DN: RM CK: LW DW: RM				RM		CK: LW		
©TxD0T		CONT	SECT	JOB			HIG	HWAY	
		0918	11	100, ET	С.	CR	11	O, ETC.	
		DIST		COUNTY	SHEET NO		SHEET NO.		
		DAL		KAUFM.	ΑN			89	

SUMMA	ARY OF ES	TIMATED	QUANTIT	IES ①		
	0416 6002	0420 6014	0422 6002	0425 6012	0450 6005	0454 6003
DESCRIPTION	DRILL SHAFT (24 IN)	CL C CONC (ABUT) (HPC)	REINF CONC SLAB (HPC)	PRESTR CONC SLAB BEAM (5SB15)	RAIL (TY T221) (HPC)	ARMOR JOINT
	LF	CY	SF	LF	LF	LF
ABUTMENTS (1 AND 2)	132	18.4				44
45.00' PRESTR CONC SLAB BEAM UNIT TYPE 5SB15			1170	222.50	114	
TOTAL	132	18.4	1170	222.50	114	44

1) PROP NBI NUMBER: 18-130-0-AA03-12-003



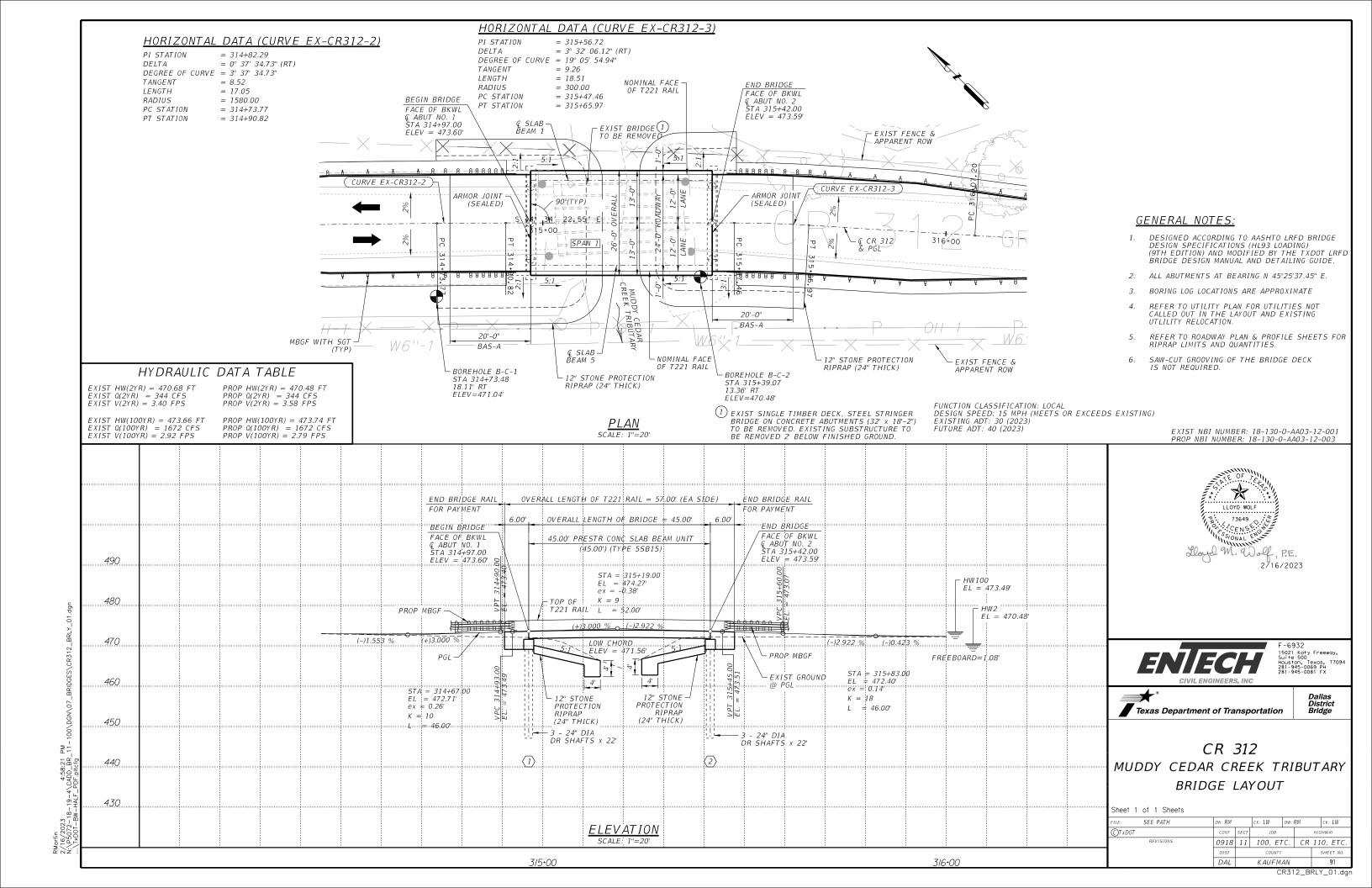




Dallas District Bridge

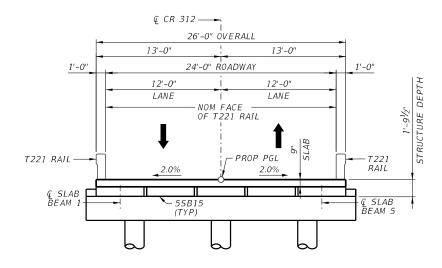
CR 312 MUDDY CEDAR CREEK TRIBUTARY ESTIMATED QUANTITIES

Sheet	1 of 1 Sheets							
FILE:	SEE PATH	DN: RM		CK: LW	DW: F	RM	С	K: LW
©T x D0T		CONT	SECT	JOB			HIGH	WAY
		0918	11	100, ET	С.	CR	110	, ETC.
		DIST		COUNTY			5F	HEET NO.
		DAL		KAUFM.	4N			90



EXISTING BRIDGE SECTION

TO BE REMOVED



PROPOSED BRIDGE TYPICAL SECTION

SCALE 1"=10'







Dallas District Bridge

CR 312 MUDDY CEDAR CREEK TRIBUTARY TYPICAL SECTIONS

Sheet 1 of 1 Sheets						
FILE: SEE PATH	DN: RM		ck: LW	ow: RM		CK: LW
©T×D0T	CONT	SECT	JOB		HIG	HWAY
	0918	11	100, ET	C. CF	11	O, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4N		92







Dallas District Bridge

CR 312 MUDDY CEDAR CREEK TRIBUTARY CAP ELEVATIONS

Sheet 1 of 1 Sheets						
FILE: SEE PATH	DN: RM		ck: LW	ow: RI	1	ck: LW
©T x D0T	CONT	SECT	JOB			HIGHWAY
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	4N		93

DRILLING LOG

1 of 2

WinCore Version 3.3 County Kaufman Highway CR 312 CSJ 0918-11-102

B-C-01 Bridge Structure Station

314+73.48 18.11' RT

Dallas District Date 8-16-22 Grnd. Elev. 471.04 ft GW Elev. 439.04 ft

Organization: B2Z Engineering

Ele (fi	ev.	0	Texas Cone Penetrometer	Strata Description	Lateral Press.	Deviator Stress	мс	LL	ы	Wet Den.	Additional Remarks
(,,	,	G		SAND, Clayey Sand, Lt. Brown to	(psi)	(psi)				(pcf)	
	_			Lt. Grayish Brown, Dense, Dry							
	-	-		(SC)							
	5 -		33 (6) 37 (6)				3.7				-200 = 44.3%
	-										
	-	-									
1.	10 -		15 (6) 14 (6)				5.8	38	22		-200 = 60.0%
	-			CLAY, Sandy Lean Clay, Lt. Gray to Grayish Brown, Very Stiff,							
	_			Dry to Moist (CL)							
	- 15 -		16 (6) 19 (6)				28.3				-200 = 59.1%
	-										
	_										
	-		25 (6) 31 (6)				27.6	64	38		
1.	20 -		., .,	CLAY, Fat Clay w/ Sand, Gray to			27.0		-		
	-			Dk. Brown, Hard, Moist (CH)							
	-		50 (3.5) 50 (3.5)				22.4				200 - 74 50/
	25 -		00 (0.0) 00 (0.0)				22.4				-200 = 74.5%
	-										
	_		E0 (4) E0 (0 E)								
11.	30 -		50 (1) 50 (0.5)	SHALE, Shale, Dk. Gray to Gray,			19.2	55	34		
	_			Hard, Moist to Wet (Comprised							
	-	Ē		Primarily of Fat Clay w/ Sand) (CH)							
	35 -	Ē	50 (1) 50 (0.5)	(6.1)			98.7				-200 = 70.9%
	_	Ē									
	-	E									
	40 -	E	50 (1) 50 (0.5)				85.8	71	46		
	-	Ē									
	-	▐									
	45 -		50 (1.5) 50 (0)				89.8				-200 = 82.0%
	-	Ē									
	-	Ē									
	50 -	Ē	50 (1) 50 (0.5)				99.7	76	49		
	-	Ē									
	-	Ē									
	55 -	Ē	50 (1) 50 (0)				85.5				-200 = 43.9%
	-	Ē									
	-	臣									
	-	E	50 (1) 50 (0)				85.9	71	44		
	60 -	_		8.6750, 6944230.5032 (Provided By Clic			-5.5				1

where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Labi\0. Wincore\C - CR 312\B-C-01 & B-C-02.CLG

DRILLING LOG

2 of 2

WinCore Version 3.3 County Kaufman Highway CR 312 CSJ 0918-11-102

B-C-01 Bridge Structure 314+73.48 Station Offset 18.11' RT

Dallas District Date 8-16-22 Grnd. Elev. 471.04 ft GW Elev. 439.04 ft

	L	T C		Triaxi	al Test	t Properties		s		
Elev. (ft)	O G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SHALE, Shale, Dk. Gray to Gray, Hard, Moist to Wet (Comprised Primarily of Fat Clay w/ Sand)						.,	
65 -		50 (1.5) 50 (0.5)	(CH)			82.1				-200 = 83.0%
70 -		50 (1.5) 50 (0.5)				80.5	72	43		
- - 75 -		50 (1) 50 (0)								
-		50 (4.5) 50 (0.5)								
1. 80 - -		50 (1.5) 50 (0.5)								
85 -										
90 -										
- - 95 -										
-										
100-	-									
105										
110										
115										
-										
120			3.6750, 6944230.5032 (Provided By Clic							

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Lab\8. Wincore\C - CR 312\B-C-01 & B-C-02.CLG







Dallas District Bridge

CR 312 MUDDY CEDAR CREEK TRIBUTARY BORING LOGS

Sheet 1 of 2 Sheets

FILE: SEE PATH	DN: RM		CK: LW	DW: RM		CK: LW
©T x DOT	CONT	SECT	JOB		HIG	HWAY
	0918	11	11 100, ETC. CR		CR 11	O, ETC.
	DIST					SHEET NO.
	DAL	KAUFMAN 9		94		

DRILLING LOG

1 of 2

WinCore Version 3.3

County Kaufman Highway CR 312 CSJ 0918-11-102

B-C-02 Bridge Structure Station

315+39.07 13.36' RT

Dallas District 8-17-22 Date Grnd. Elev. 470.48 ft GW Elev. 422.48 ft

Organization: B2Z Engineering

lev. (ft)	O	Penetrometer	Strata Description	Lateral I	Stress	MC	LL	ы	Wet Den.	Additional Remarks
			CLAY, Sandy Lean Clay, Lt. Gray,	(psi)	(psi)				(pcf)	
-			Hard to Stiff, Dry (CL)							
-										
5 -		35 (6) 40 (6)				8.8	40	24		
-										
-										
10 -		9 (6) 9 (6)				10.3				-200 = 53.5%
-										
-										
- 5 15 -		15 (6) 16 (6)				34.8	86	57		-200 = 95.4%
- 10			CLAY, Fat Clay to Fat Clay w/							
_			Sand, Lt. Gray to Brown, Very Stiff to Hard, Dry to Moist (CH)							
-		35 (6) 27 (6)	,			30.2				-200 = 70.4%
20 -		(-) (-)				30.2				-200 - 70.470
-	/									
-		50 (5) 50 (2.5)				00.2	e E	40		
25 -		00 (0) 00 (2.0)				22.3	65	40		
-										
-		50 (0.5) 50 (0.5)								
5 30 -		50 (0.5) 50 (0.5)	SHALE, Shale, Gray to Dk. Gray,			19.5				-200 = 62.9%
-			Hard, Moist to Wet (Comprised							
-	Ē		Primarily of Fat Clay w/ Sand) (CH)							
35 -		50 (1) 50 (0.5)	(City			20.0	59	39		
-	Ē									
-	Ē									
40 -	E	50 (1.5) 50 (1)				20.6				-200 = 68.0%
-	謈									
-										
45 -		50 (2) 50 (1)				22	64	44		
-										
-										
50 -	蛗	22 (6) 50 (5.5)				19.5				-200 = 80.8%
-	臣									
	Ē									
	Ē	50 (1.5) 50 (0.5)				46.5	67	50		
55 - -							ν.			
-										
-		50 (1) 50 (0)				66.5				200 - 80 59/
60 -				4>		00.5				-200 = 80.5%
ernarks	s: XY	Coord 26888/8	3.0207, 6944187.3241 (Provided By Cli	ent).						

where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:\UOBS\Entech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Labi\0. Wincore\C - CR 312\B-C-01 & B-C-02.CLG

Logger: JR



Version 3.3

DRILLING LOG

County Kaufman Highway CR 312 CSJ 0918-11-102

B-C-02 Bridge Structure 315+39.07 Station Offset 13.36' RT

Dallas Date 8-17-22 Grnd. Elev. 470.48 ft GW Elev. 422.48 ft

District

2 of 2

_ L Texas		Texas Cone	exas Cone		Triaxial Test Properties					
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral E Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SHALE, Shale, Gray to Dk. Gray, Hard, Moist to Wet (Comprised Primarily of Fat Clay w/ Sand)						``	
65 -		50 (1.5) 50 (0.5)	(СН)			73.3	64	44		
70 -		50 (1.5) 50 (0.5)				69.2				-200 = 77.1%
75 -		50 (1) 50 (0.5)				73.5	68	47		
-		50 (1) 50 (0.5)								
90.5 80 - - -		30 (1) 30 (0.3)				75.5				200 = 81.6%
85 -										
90 -										
95 -										
100										
105										
110-										
-										
115- - -										
120	1									

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

B:UOBSIEntech\TxDOT 36-8IDP5072 - On & Off-System Bridge PS&E - Statewide & Houston\WA#2 (Entech WA#4) - Kaufman County\TechProd\Borings\info from Lab\8. Wincore\C - CR 312\B-C-01 & B-C-02.CLG







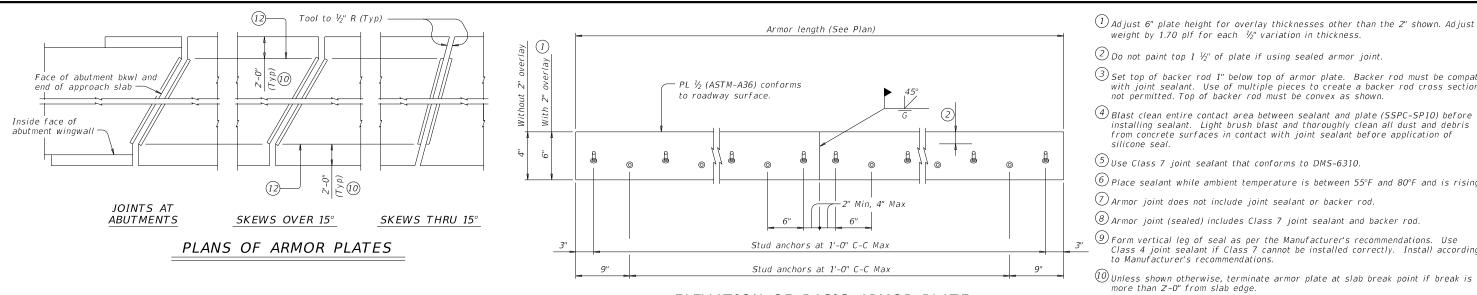
Dallas District Bridge

CR 312 MUDDY CEDAR CREEK TRIBUTARY BORING LOGS

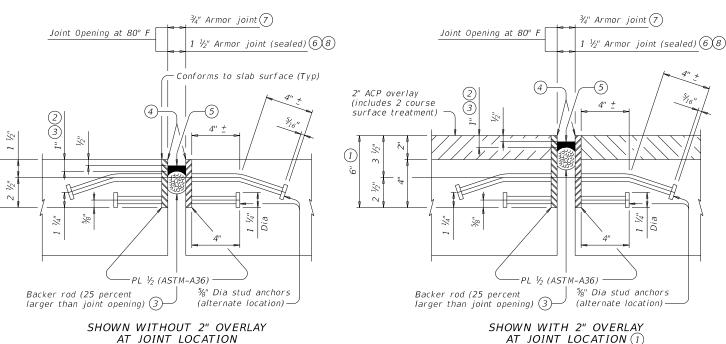
Sheet 2 of 2 Sheets

FILE: SEE PATH	DN: RM		CK: LW	DW:	RM	CK: LW
©TxD0T	CONT	SECT	JOB			HIGHWAY
	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL	KAUFMAN				95



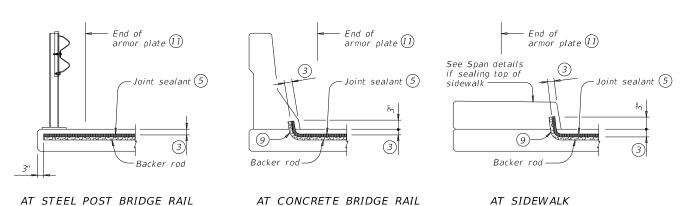


ELEVATION OF BASIC ARMOR PLATE



ARMOR JOINT SECTIONS

Showing Armor Joint (Sealed)



JOINT SEALANT TERMINATION DETAILS

Armor joint (sealed) only. Armor plate is not shown for clarity.

3 Set top of backer rod 1" below top of armor plate. Backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown. 4 Blast clean entire contact area between sealant and plate (SSPC-SP10) before installing sealant. Light brush blast and thoroughly clean all dust and debris from concrete surfaces in contact with joint sealant before application of

(5) Use Class 7 joint sealant that conforms to DMS-6310.

weight by 1.70 plf for each $\frac{1}{2}$ " variation in thickness.

 $\stackrel{ullet}{ ext{ }}$ Place sealant while ambient temperature is between 55°F and 80°F and is rising.

(7) Armor joint does not include joint sealant or backer rod.

8 Armor joint (sealed) includes Class 7 joint sealant and backer rod.

(9) Form vertical leg of seal as per the Manufacturer's recommendations. Use Class 4 joint sealant if Class 7 cannot be installed correctly. Install according to Manufacturer's recommendations.

10 Unless shown otherwise, terminate armor plate at slab break point if break is more than 2'-0" from slab edge.

(1) See "Plans of Armor Plates".

② At Fabricator's option, armor plate may extend up to 6" beyond this point for skews through 15°.

 $\widehat{ ext{(1)}}$ Align shipping angle perpendicular to joint.

FABRICATION NOTES:

Match mark corresponding plate sections and secure together for shipment with shipping angle. Do not use erection bolts.

Ship armor joints in convenient lengths of 10'-0" Min and 24-0" Max unless necessary for stage construction or widenings. One shop splice is permitted in each shipping length provided no piece is less than 2'-0" long and sufficient studs are added to limit the stud to shop splice distance to 2" Min and 4" Max. Weld studs in accordance with AWS D1.1.

Use groove welds for all shop and field butt splices. Grind smooth areas in contact with seal. Make all necessary field splice joint preparations

Paint the entire steel section, except as stated in Note 2, with System II or IV primer in accordance with Item 446 "Field Cleaning and Painting Steel." Provide paints in accordance with Item 446.2. Prepare steel and apply paint in accordance with Items 446.4.7.3 and 446.4.7.4.

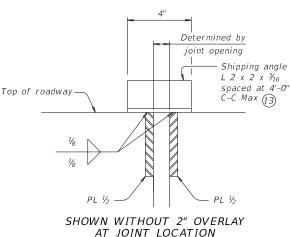
Shop drawings for the fabrication of armor joints will not require the Engineer's approval if fabrication is in accordance with the details

CONSTRUCTION NOTES:

Secure armor joints in position and place to proper grade and alignment by welding braces to adjacent reinforcing steel, to prestressed beam stirrups, or to anchors cast in concrete diaphragms. Include cost of temporary bracing in the price bid for Armor Joint. Remove shipping angle immediately after each joint half is secured in place. Grind smooth, and touch up with organic zinc-rich paint.

Provide armor joints at locations shown on the plans. Provide the seal when "Armor Joint (Sealed)" is noted on the plans.

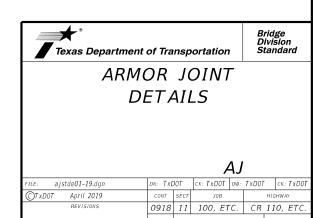
These joint details accommodate a joint movement range of 1 \(\frac{3}{4}'' \) opening movement and \(\frac{7}{6}'' \) closure movement). Payment for armor joint, with or without seal, is based on length of armor plate.



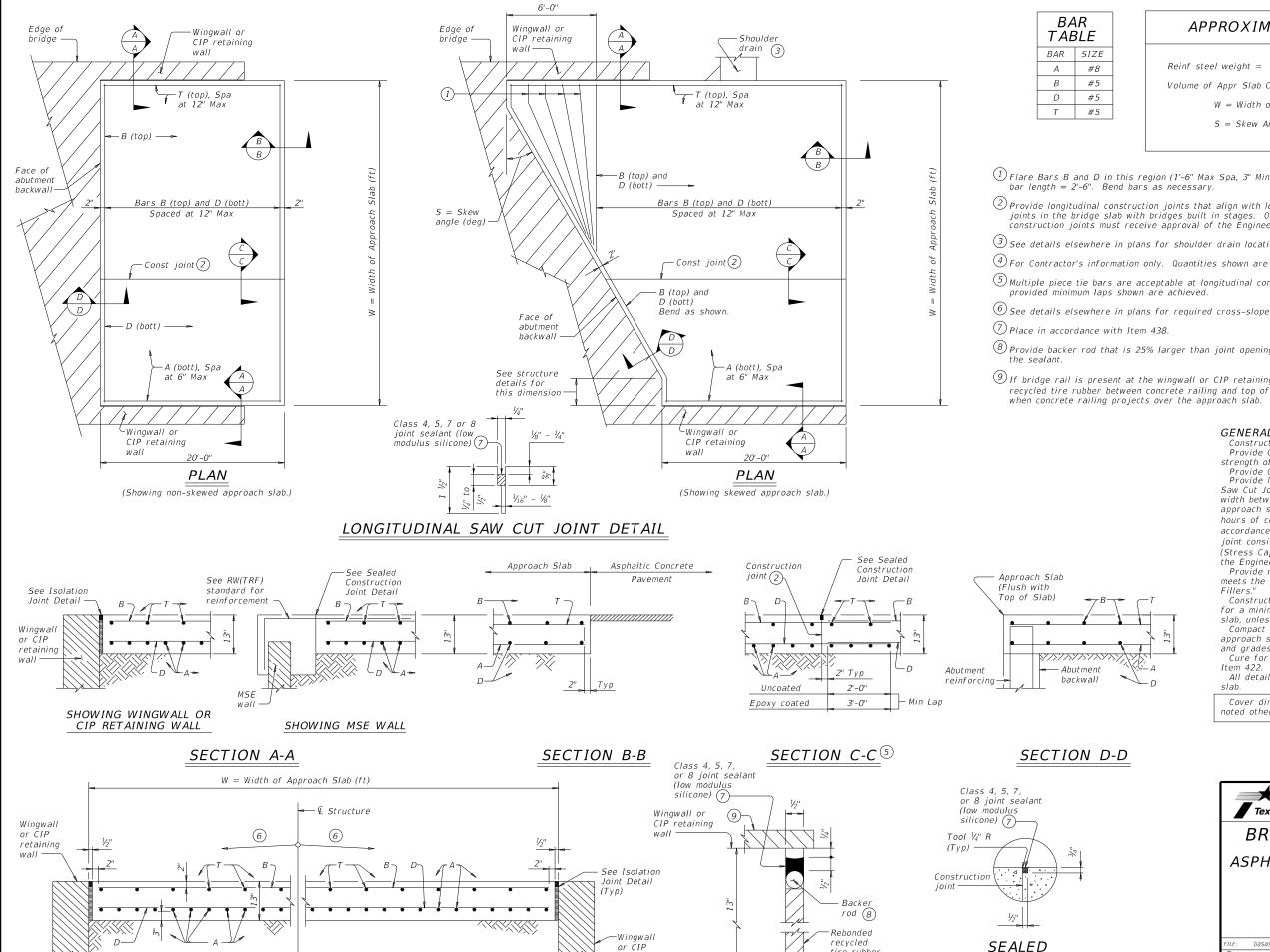
With overlay similar SHIPPING ANGLE

An alternate method of securing joint sections may be used if approved by the Bridge Division. Erection bolts are not allowed.

	WEIGHTS FOR ONE ARMOR JOINT (2 PLATES)								
WITHOUT OVERLAY	-	16.10 plf							
WITH 2" OVERLAY	1)	22.90 plf							



KAUFMAN



retaining

ISOLATION JOINT DETAIL

wall

TYPICAL TRANSVERSE SECTION

∑ 0

4:58:34

2/16/2023

APPROXIMATE QUANTITIES 4

Reinf steel weight = 8.5 Lbs/SF of Approach Slab

Volume of Appr Slab Conc (CY) = $0.802W + 0.02W^2$ Tan S

W = Width of Approach Slab (ft)

S = Skew Angle (deg)

- 1) Flare Bars B and D in this region (1'-6" Max Spa, 3" Min Spa). Minimum flared bar length = 2'-6". Bend bars as necessary.
- 2) Provide longitudinal construction joints that align with longitudinal construction joints in the bridge slab with bridges built in stages. Other longitudinal construction joints must receive approval of the Engineer.
- (3) See details elsewhere in plans for shoulder drain location and details.
- 4 For Contractor's information only. Quantities shown are for one approach slab.
- (5) Multiple piece tie bars are acceptable at longitudinal construction joints provided minimum laps shown are achieved.
- 8 Provide backer rod that is 25% larger than joint opening and compatible with
- (9) If bridge rail is present at the wingwall or CIP retaining wall, place ½" rebonded recycled tire rubber between concrete railing and top of approach slab as shown when concrete railing projects over the approach slab.

GENERAL NOTES:

Construct approach slab in accordance with Item 422. Provide Class "S" concrete with a minimum compressive strength of 4,000 psi.

Provide Grade 60 reinforcing steel.

Provide longitudinal joints as shown on the Longitudinal Saw Cut Joint Detail at lane lines and shoulders when width between longitudinal construction joints or edges of approach slab exceeds 16 feet. Saw cut joints within 24 hours of concrete placement to a depth of 1 $\frac{1}{2}$ " and seal in accordance with Item 438. Alternately, provide a controlled joint consisting of 1 $\frac{1}{2}$ " vinyl or plastic joint former (Stress Cap, Zip Strip, Stress Lock, or equal as approved by the Engineer.)

Provide rebonded recycled tire rubber joint filler that meets the requirements of DMS-6310. "Joint Sealants and Fillers.

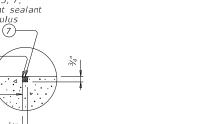
Construct the subgrade or subbase away from the bridge for a minimum distance of 100 feet prior to the approach slab, unless otherwise indicated on the plans. Compact and finish the subgrade or foundation for the approach slab to the typical cross-section and to the lines

and grades shown on the plans.

Cure for 4 days using water or membrane curing per Item 422.

All details shown herein are subsidiary to bridge approach

Cover dimensions are clear dimensions, unless



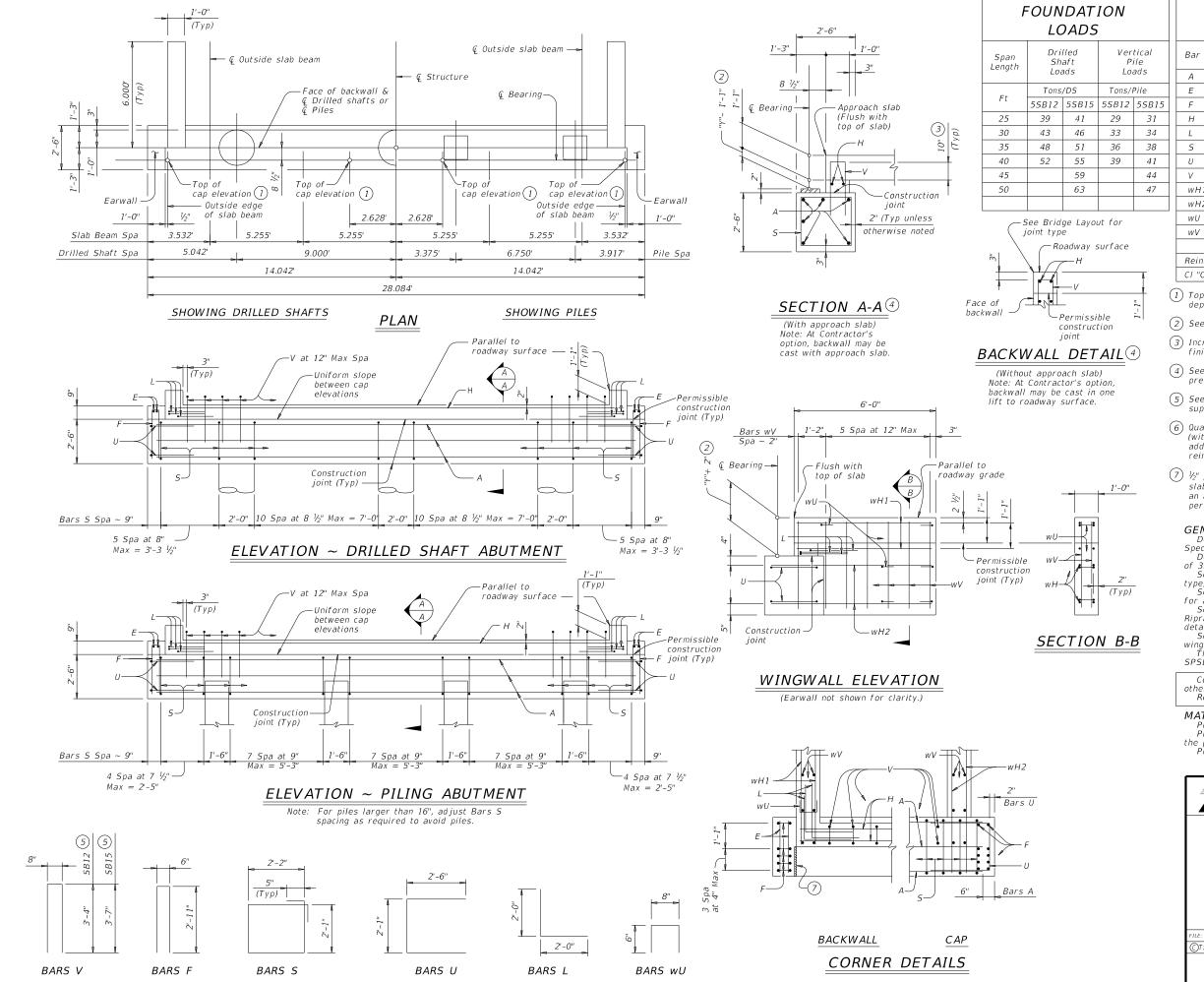
SEALED CONSTRUCTION JOINT DETAIL



BRIDGE APPROACH SLAB ASPHALTIC CONCRETE PAVEMENT

BAS-A

LE: basaste1-20.dgn	DN: TXE	OT	ck: TxD0T	DW:	TxD0T	ck: TxD0T
TXDOT April 2019	CONT	SECT	JOB		HI	GHWAY
REVISIONS	0918	11	100, ET	С.	CR 1:	10, ETC.
02-20: Removed stress relieving pad.	DIST			SHEET NO.		
	DAL		KAUFM.	ΑN		97



₽ 0

4:58:35 3-19-4\[A]

2/16/2023

TABLE OF ESTIMATED 6 **QUANTITIES**

Par	Bar No. Size					Weight	5
Баі	NO.	3126	5SB12	551	315	5SB12	5SB15
Α	6	#11	27'-1"	2	7'-1"	863	863
Е	4	#4	2'-2"		2'-2"	6	6
F	10	#4	6'-4"		6'-4"	43	43
Н	2	#5	25'-8"	2.	5'-8"	54	54
L	6	#6	4'-0"		4'-0"	36	36
5	34	#4	9'-4"		9'-4"	212	212
U	4	#6	7'-1"	7'-1"		43	43
V	25	#5	7'-4"	7'	-10"	191	204
wH1	8	#6	5'-8"		5'-8"	68	68
wH2	8	#6	6'-11"	6	-11"	83	83
wU	12	#4	1'-8"		1'-8"	14	14
wV	28	#5	3'-10"		4'-1"	112	119
Reinfo	rcing S	teel			Lb	1,725	1,745
CI "C"	Conc (A	but)			CY	8.8	9.2

- (1) Top of cap elevations are based on section depths shown on Span Details.
- (2) See Span Details for "Y".
- ③ Increase as required to maintain 3" from finished grade.
- (4) See Bridge Layout to determine if approach slab is present.
- 5 See Bridge Layout for beam type used in the superstructure.
- 6) Quantities shown are for one abutment only (with approach slab). Without approach slab, add 1.0 CY Class "C" concrete and 54 Lb reinforcing steel for 2 additional Bars H.
- 7) ½" preformed bituminous fiber material between slab beam and earwall. Bond to earwall with an approved adhesive. Cast inside face of earwall perpendicular to cap. (Typ)

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

Designed for a normal embankment header slope

Designed for a normal embankment header slope of 3:1 and a maximum span length of 50 feet. See Bridge Layout for header slope and foundation type, size, and length. See Common Foundation Details (FD) standard sheet for all foundation details and notes. See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment

details, if applicable. See applicable rail details for rail anchorage in

wingwalls.
These abutment details may be used with standard SPSB-24 only.

Cover dimensions are clear dimensions, unless noted

Reinforcing bar dimensions shown are out-to-out of bar

MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere in the plans.
Provide Grade 60 reinforcing steel.

HL93 LOADING



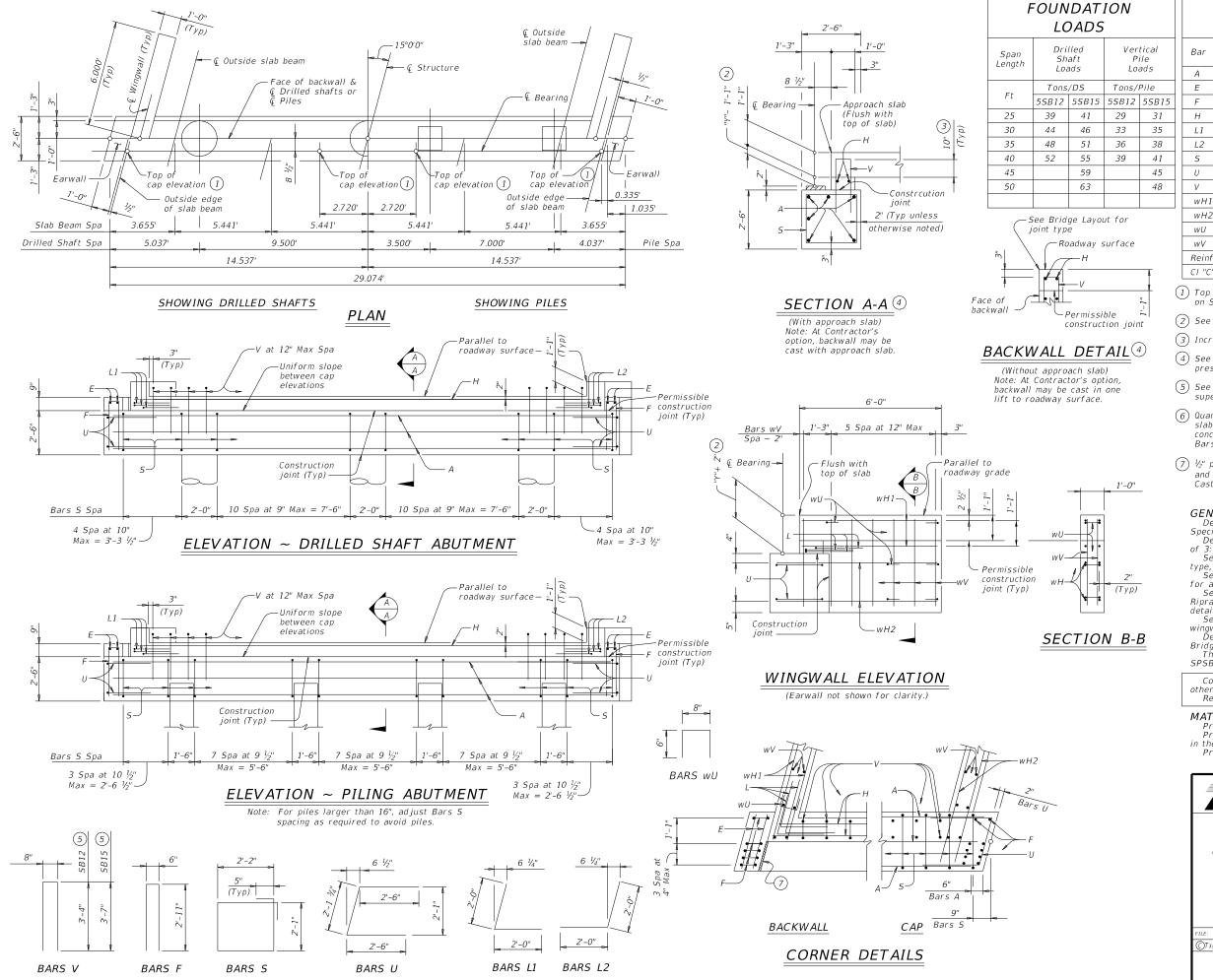
Bridge Division Standard

ABUTMENTS PRESTR CONCRETE SLAB BEAM

24' ROADWAY

APSB-24

.E: psbste09-17.dgn	DN: Tx	D0T	ck: TxDOT	DW:	TxD0T	ck: TxD0T	
TxDOT January 2017	CONT	SECT JOB		HI	HIGHWAY		
REVISIONS	0918	11	100, ET	100, ETC. CR			
	DIST	COUNTY				SHEET NO.	
	DAL			ΚΔΙΙΕΜΔΝ			



4:58:36

2/16/2023

TABLE OF ESTIMATED 6 **QUANTITIES**

		_													
Bar	No.	Size	Length	(5		Weight	(5)								
Баі	NO.	3126	5SB12	551	315	5SB12	5SB15								
Α	6	#11	28'-1"	2	8'-1"	895	895								
Ε	4	#4	2'-3"		2'-3"	6	6								
F	10	#4	6'-4"		6'-4"	43	43								
Н	2	#5	26'-7"	2	6'-7"	56	56								
L1	3	#6	4'-0"		4'-0"	18	18								
L2	3	#6	4'-0"	4'-0"		18	18								
S	32	#4	9'-4"	9'-4"		200	200								
U	4	#6	7'-2"		7'-2"	43	43								
V	26	#5	7'-4"	7'	-10"	199	212								
wH1	8	#6	5'-8"		5'-8"	68	68								
wH2	8	#6	6'-11"	6	-11"	83	83								
wU	12	#4	1'-8"		1'-8"	14	14								
wV	28	#5	3'-10"		4'-1"	112	119								
Reinfo	rcing S	teel			Lb	1,755	1,775								
CI "C"	Conc (A	but)			CY	9.1	9.5								

- (1) Top of cap elevations are based on section depths shown on Span Details.
- (2) See Span Details for "Y".
- (3) Increase as required to maintain 3" from finished grade.
- 4) See Bridge Layout to determine if approach slab is
- (5) See Bridge Layout for beam type used in the
- (6) Quantities shown are for one abutment only (with approach slab). Without approach slab, add 1.0 CY Class "C" concrete and 56 Lb reinforcing steel for 2 additional
- 7 $\frac{1}{2}$ " preformed bituminous fiber material between slab beam and earwall. Bond to earwall with an approved adhesive. Cast inside face of earwall perpendicular to cap. (Typ)

GENERAL NOTES:

- Designed according to AASHTO LRFD Bridge Design
- Specifications.

 Designed for a normal embankment header slope of 3:1 and a maximum span length of 50 feet. See Bridge Layout for header slope and foundation
- type, size, and length.
 See Common Foundation Details (FD) standard sheet for all foundation details and notes.
- See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment details, if applicable. See applicable rail details for rail anchorage in
- wingwalls.
 Details are drawn showing right forward skew. See
- Bridge Layout for actual skew direction.

 These abutment details may be used with standard SPSB-24-15 only.

Cover dimensions are clear dimensions, unless noted

Reinforcing bar dimensions shown are out-to-out of bar.

MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere

Provide Grade 60 reinforcing steel. HL93 LOADING



ABUTMENTS

PRESTR CONCRETE SLAB BEAM

24' ROADWAY

15° SKEW

Bridge Division Standard

APSB-24-15

LE: psbste10-17.dgn	DN: Tx	D0T	ck: TxD0T	DW:	TxD0T	ck: TxD0T	
TxDOT January 2017	CONT	SECT	ECT JOB HIGHWAY		GHWAY		
REVISIONS	0918	11	100, ETC.		CR 1	CR 110, ETC.	
	DIST	COUNTY		SHEET NO.			
	DAL	KAUFMAN				99	

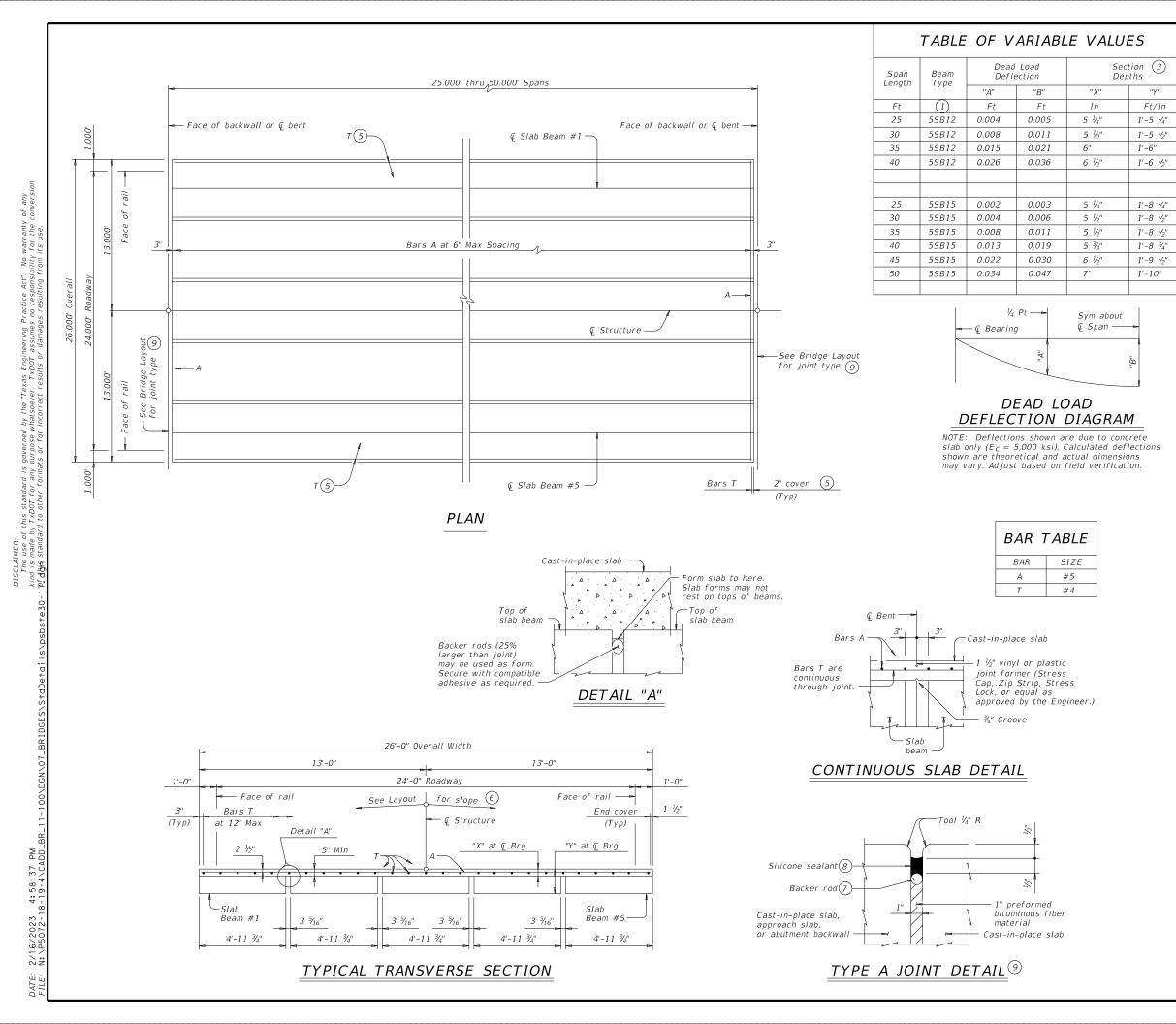


TABLE OF ESTIMATED QUANTITIES

SPAN	REINF CONCRETE SLAB	(5S	TOTAL (2) REINF		
LENGTH	(SLAB BEAM)	ABUT TO INT BT	INT BT TO INT BT	ABUT TO ABUT	STEEL
Ft	SF	LF 4	LF 4	LF 4	Lb
25	650	122.50	122.50	122.50	1,820
30	780	147.50	147.50	147.50	2,180
35	910	172.50	172.50	172.50	2,550
40	1,040	197.50	197.50	197.50	2,910
45	1,170	222.50	222.50	222.50	3,280
50	1,300	247.50	247.50	247.50	3,640

- $\widehat{1}$ See Bridge Layout for beam type used in the superstructure. These standards do not provide for the use of both SB12 and SB15 beams within the same structure.
- (2) Reinforcing steel weight is calculated using an approximate factor of 2.8 Lbs/SF.
- (3) Based on theoretical beam camber, dead load deflections of 5" cast-in-place concrete slab and a constant grade. The Contractor will adjust these values for any vertical curve.
- 4 Fabricator will adjust beam lengths for beam slopes as required
- (5) Where slab is continuous over Interior Bents, Bars T are continuous through Joint. See "Continuous Slab Detail".
- (6)This standard does not provide for changes in roadway cross-slopes within the structure.
- (7) 1 $\frac{1}{4}$ " backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- (8) Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- Superstructures".

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Two- or three-span units, with slab continuous over interior bents. may be formed with the details shown on this sheet.

See applicable rail details for rail anchorage in slab.

This standard does not support the use of transition bents.

Cover dimensions are clear dimensions, unless noted otherwise.

MATERIAL NOTES:

Provide Class S concrete (f'c = 4,000 psi).

Provide Class S (HPC) concrete if shown elsewhere in the plans. Provide Grade 60 reinforcing steel.

Provide bar laps, where required, as follows: Uncoated ~ #4 = 1'-7"

~ #5 = 2'-0"

Epoxy coated ~ #4 = 2'-5" ~ #5 = 3'-0'

Deformed welded wire reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A or T unless noted otherwise.

HL93 LOADING



Bridge Division Standard

PRESTRESSED CONCRETE SLAB BEAM SPANS (TY SB12 OR SB15)

24' ROADWAY

SPSB-24

E: psbste30-17.dgn	DN: Tx	D0T	ck: TxDOT	DW:	TxD0T	ck:TxD0T
TxDOT January 2017	CONT	SECT	JOB		H	IGHWAY
REVISIONS	0918	11	100, ET	С.	CR 1	10, ETC.
	DIST	COUNTY				SHEET NO.
	DAL		KAUFM	AN		100

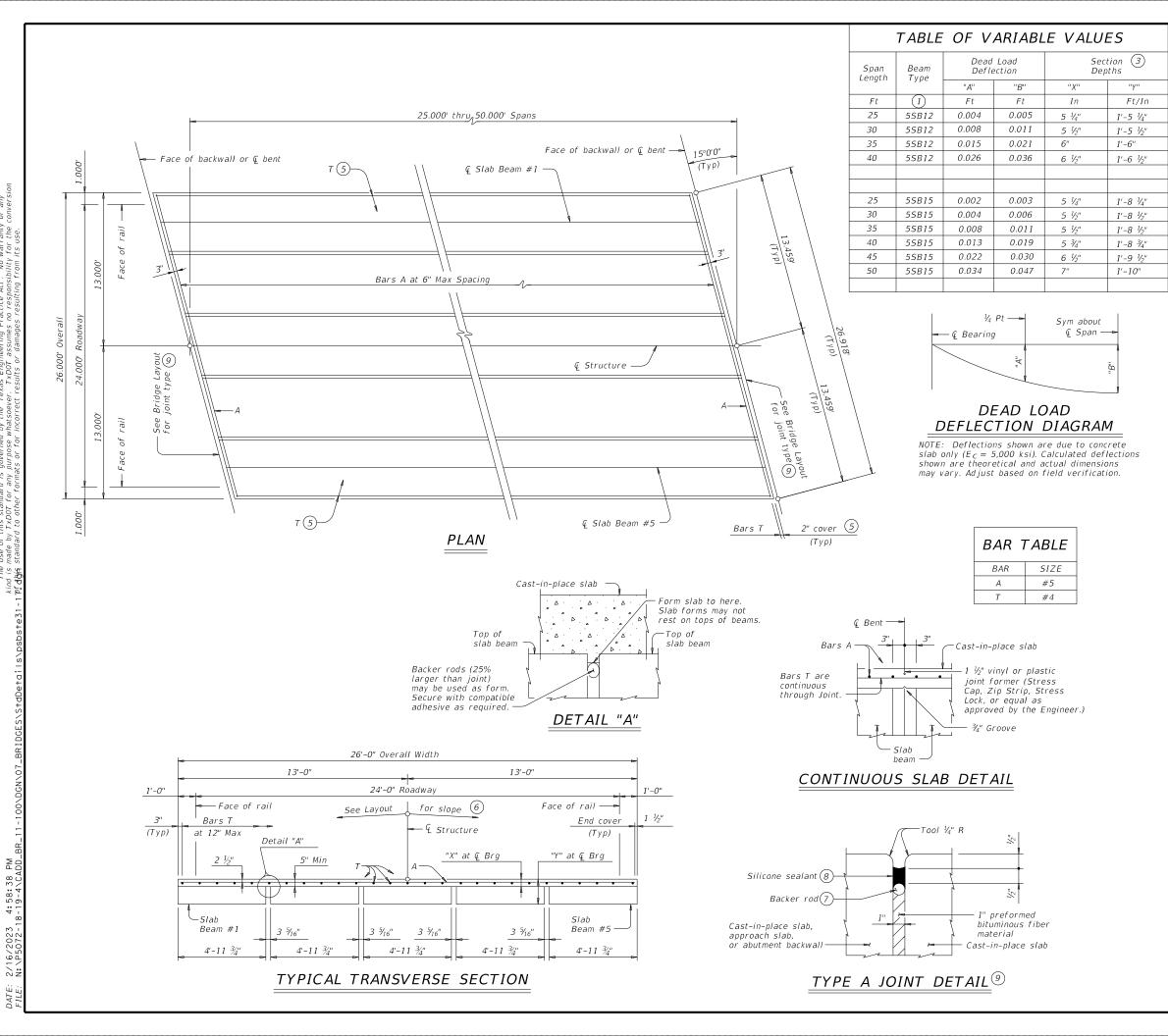


TABLE OF ESTIMATED QUANTITIES

SPAN CONCRETE SLAB (SLAB BEAM)	CONCRETE	(5S	TOTAL 2		
	(SLAB	ABUT TO INT BT	INT BT TO INT BT	ABUT TO ABUT	STEEL
Ft	SF	LF 4	LF 4	LF (4)	Lb
25	650	122.46	122.50	122.41	1,820
30	780	147.46	147.50	147.41	2,180
35	910	172.46	172.50	172.41	2,550
40	1,040	197.46	197.50	197.41	2,910
45	1,170	222.46	222.50	222.41	3,280
50	1,300	247.46	247.50	247.41	3,640

- 1) See Bridge Layout for beam type used in the superstructure. These standards do not provide for the use of both SB12 and SB15 beams within the same structure.
- (2) Reinforcing steel weight is calculated using an approximate factor of 2.8 Lbs/SF.
- 3 Based on theoretical beam camber, dead load deflections of 5" cast-in-place concrete slab and a constant grade.
- $\stackrel{\textstyle igoplus}{4}$ Fabricator will adjust beam lengths for beam slopes as required
- (5) Where slab is continuous over Interior Bents, Bars T are continuous through Joint. See "Continuous Slab Detail".
- 6 This standard does not provide for changes in roadway cross-slopes within the structure.
- multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- 8 Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- See Bridge Layout for expansion joint locations. If using Type
 A expansion joints, the maximum distance between joints is 100
 feet. Type A joints are subsidiary to Item 422, "Concrete"

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. This standard does not provide for vertical curves in roadway grade within the structure.

Two- or three-span units, with slab continuous over interior bents, may be formed with the details shown on this sheet.

See applicable rail details for rail anchorage in slab. Details are drawn showing right forward skew. See Bridge Layout for actual skew direction.

This standard does not support the use of transition bents.

Cover dimensions are clear dimensions, unless noted otherwise.

MATERIAL NOTES:

Provide Class S (HPC) concrete if shown elsewhere in the plans.

Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows:

Uncoated ~ #4 = 1'-7" ~ #5 = 2'-0"

Epoxy coated $\sim #4 = 2'-5''$

~ #5 = 3'-0"

Deformed welded wire reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A or T unless noted HL93 LOADING



Bridge Division Standard

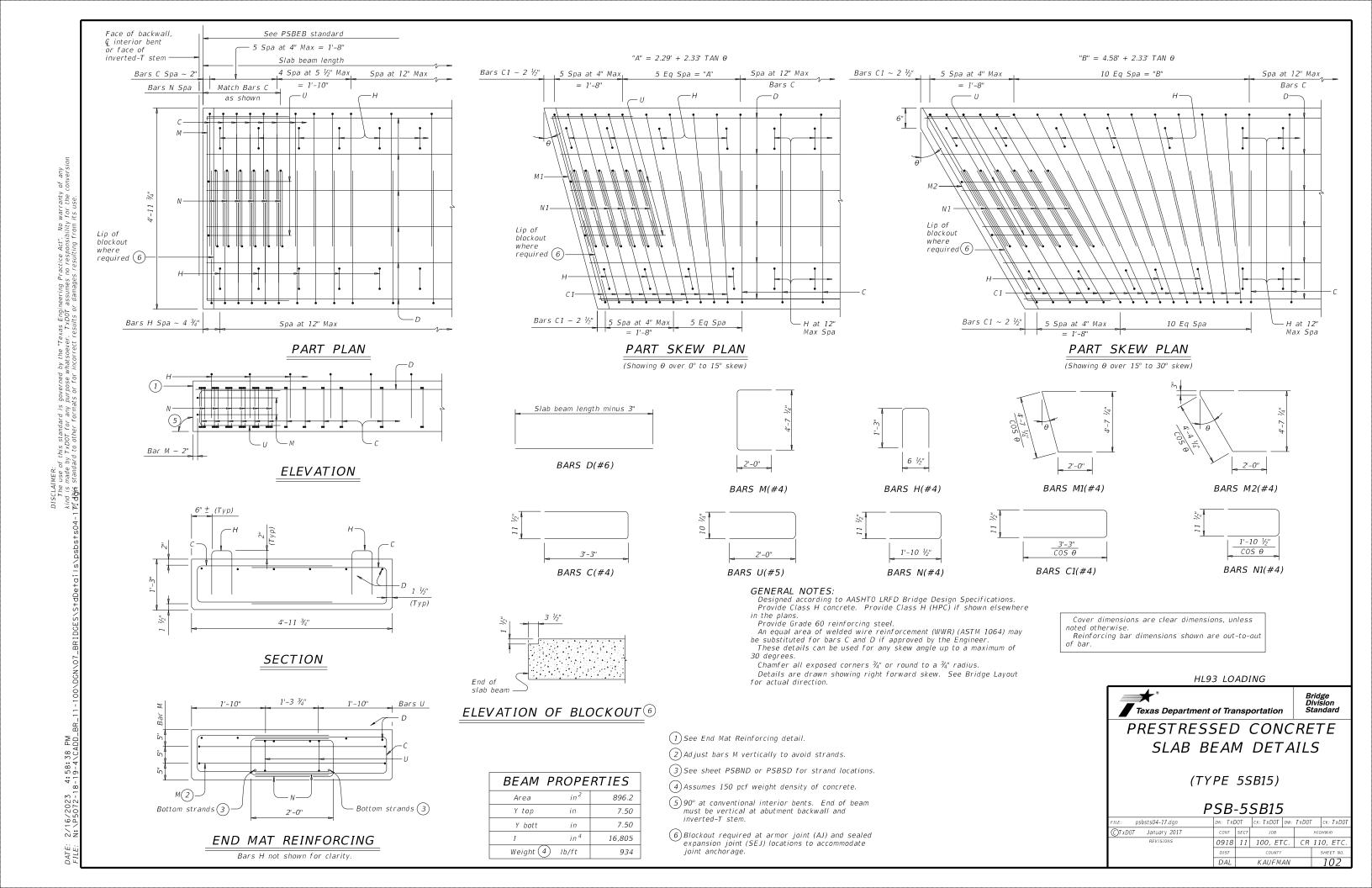
PRESTRESSED CONCRETE SLAB BEAM SPANS (TY SB12 OR SB15)

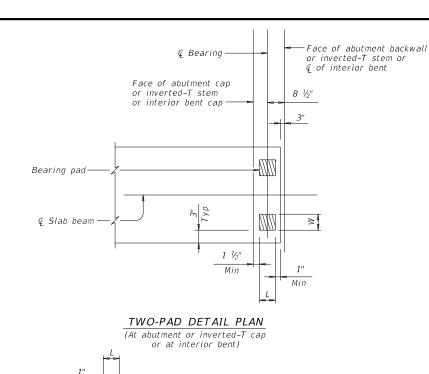
24' ROADWAY

15° SKEW

SPSB-24-15

LE: psbste31-17.dgn	DN: TX	D0T	ck: TxDOT	DW:	TxD0T	ck: TxD0T
TxDOT January 2017	CONT	SECT	JOB		HI	SHWAY
REVISIONS	0918	11	100, ETC. CR		CR 1:	10, ETC.
	DIST	COUNTY KALLEMAN			SHEET NO.	
	DAL					101





Min

- Face of abutment cap or inverted-T stem or interior bent cap

Face of abutment backwall

ONE-PAD DETAIL PLAN

or inverted-T stem

or & of interior bent

© Slab beam

-Bearing pad

Min

Q Bearing−

4:58:39 PM

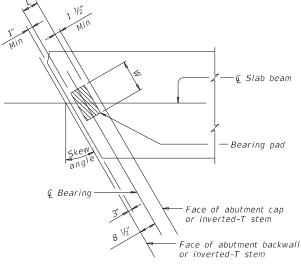
2/16/2023 N: \P5072-1

Face of abutment backwall **Q** Bearing ⋅ or inverted-T stem Face of abutment cap or inverted-T stem Bearing pad-€ Slab beam

← Interior bent Face of interior bent cap Bearing pad -€ Slab beam-

(At abutment or inverted-T cap)

TWO-PAD DETAIL SKEW PLAN



(At abutment or inverted-T cap)

ELASTOMERIC BEARING PAD PLACEMENT AND BEAM END DIAGRAMS

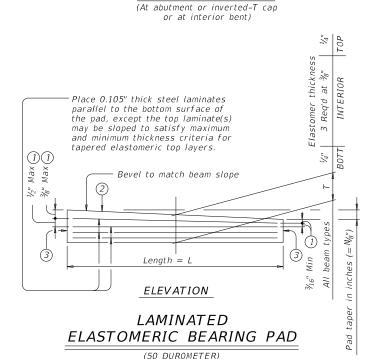
Place one bearing pad at forward station beam end. Place two bearing pads at back station beam end.

ONE-PAD DETAIL SKEW PLAN

GENERAL NOTES: These details accommodate skew angles up to 30°.

Shop drawings for approval are required. A bearing layout which identifies location and orientation of all bearings must be developed by the bearing fabricator. Permanently mark each bearing in accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer.

Cost of furnishing and installing elastomeric bearings must be included in unit price bid for "Prestressed Concrete Slab Beams".



- shown are for elastomer only, on tapered
- value of "N" (amount of taper in 1/8" increments) in this mark. Examples: N=0, (for 0" taper) N=1, (for $\frac{1}{8}$ " taper)

vary from plan beam slope by more than 0.0625" \ IN/IN.

(3) Locate permanent mark here.

1 Maximum and minimum layer thicknesses

2 Indicate BEARING TYPE on all pads. For tapered pads, locate BEARING TYPE on the high side. The Fabricator must include the N=2, (for ½" taper) Fabricated pad top surface slope must not

HL93 LOADING

TABLE OF BEARING PAD DIMENSIONS

(ALL PRESTR CONC SLAB BM TYPES)

One-Pad (Ty SB1-"N") (2) Two-Pad (Ty SB2-"N") (

Pad sizes shown are applicable for the

(1) All one, two and three span units where the minimum span length is not less than 25' and the maximum

span is not more than 50'.

(2) Skews less than or equal to 30°.

following conditions:

14"

W



Texas Department of Transportation

ELASTOMERIC BEARING AND BEAM END DETAILS

PRESTR CONCRETE SLAB BEAM

DCDED

	PODED					
LE: psbste06-17.dgn	DN: Tx	D0T	ck: TxDOT	DW:	TxD0T	ck: TxD0T
TxDOT January 2017	CONT	SECT	JOB			HIGHWAY
REVISIONS	0918	11	100,ET	С.	CR	110,ETC.
	DIST	COUNTY				SHEET NO.
	DAI	DAI KALIEMAN			103	

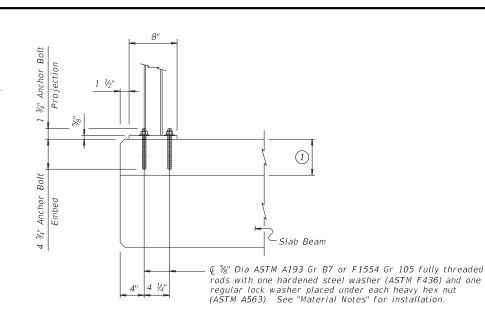
TWO-PAD DETAIL SKEW PLAN (At interior bent)

€ Slab beam -Bearing pad -Face of interior bent cap © interior bent

> ONE-PAD DETAIL SKEW PLAN (At interior bent)

2/16/2023

PART SPAN ELEVATION



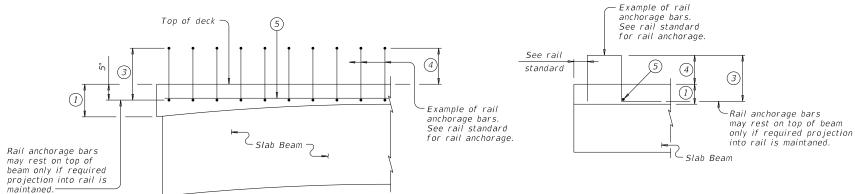
ADHESIVE ANCHORAGE OPTION

SECTION

T631LS & T631 RAIL ANCHORAGE PLACEMENT 20

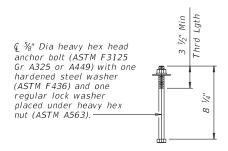
1

Slab Beam

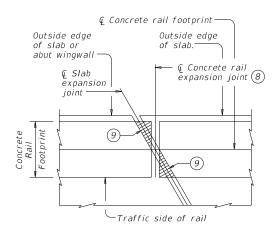


TYPICAL CONCRETE RAIL ANCHORAGE

(Showing typical concrete rail anchorage)



T631LS & T631 RAIL C-I-P ANCHOR BOLT



PLAN OF CONCRETE RAILS AT EXPANSION JOINTS

- (1) Cast-in-place slab thickness varies due to beam camber (5" minimum).
- 2 Replace cast-in-place anchor bolts shown on T631LS and T631 Rail standard with an adhesive anchor system or cast-in-place anchor bolts shown on
- $\begin{tabular}{ll} \hline \end{tabular}$ Bar length shown on rail standard, minus 1 $\end{tabular}$. Adjust bar length for a
- 4) See rail standard for projection from finished grade or top of sidewalk.
- 5 Place additional (#5) longitudinal bar.
- 6 Excess bolt length has been provided to accommodate a variable slab thickness due to beam camber. If slab thickness on span details exceed 7", bolt length must be increased accordingly. After posts have been set and bolts tightened, bolt projection above nuts of more than 1/2" must be cut off and painted with two coats of zinc-rich paint conforming to the Item 445 "Galvanizing".
- Distance from end of top outside edge of slab to center of first bolt group can not be less than 9", except: 15° Skew: 1'-0" (acute corner only) 30° Skew: 1'-3" (acute corner only)
- (a) Location of rail expansion joint must be at the intersection of (a) slab expansion joint, (a) rail footprint and perpendicular to slab outside edge.
- 9 Cross-hatched area must have $1\!\!/_2$ " preformed bitumuminous fiber material under concrete rail, as shown.

CONSTRUCTION NOTES:

Rail anchorage bars may be field bent as required to clear rail reinforcing or provide minimum cover shown on standard rail detail sheets.

Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

MATERIAL NOTES:

Galvanize all steel components of steel rail system.

Provide Grade 60 reinforcing steel.

Cast-in-place anchorage system for T631LS and T631 Rail must be 5%" Dia heavy hex head anchor bolts (ASTM F3125 Gr 325 or A449) with one hardened steel washer (ASTM F436) and one regular lock washer placed under heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed anchor bolts 4 1/2" minimum.

Adhesive anchors for T631LS and T631 Rail must be 5%" Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 3/4". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole

size, drilling, and clean out, must be in accordance with Item 450, "Railing." Epoxy coat or galvanize reinforcing steel shown on this standard if rail reinforcement is epoxy coated or galvanized.

GENERAL NOTES:

Designed in accordance with AASHTO LRFD Bridge Design Specifications. This standard is for use with structures with a 5" minimum cast-in-place concrete slab.

This standard may require modification for interior rails. This standard does not apply to median barriers.

This standard does not provide details for Type T221P, T224, T80HT, T80SS, C412, PR11, PR22 and PR3 rails on slab beam bridges.

See rail standards for approved speed restrictions, notes and details not shown.

Cover dimensions are clear dimensions, unless noted otherwise.



Bridge Division Standard

RAIL ANCHORAGE **DETAILS**

PRESTR CONCRETE SLAB BEAMS

PSBRA

FILE: psbste07-18.dgn	DN: TXL	DOT TOO	ck: TxD0T	DW:	JTR	ск: ЈМН
©TxD0T January 2017	CONT	SECT	JOB			HIGHWAY
REVISIONS	0918	11	100,ET	С.	CR	110,ETC.
03-18: Updated adhesive anchor notes.	DIST		COUNTY			SHEET NO.
	DAL		KAUFM	ΔN		104

	1									-	
×	no responsibility for	the conversion of	f this standard	to other	formats or i	or in	correct ri	esults or	damages resultir	ng from its use.	
Is\psbsts08-21.dgn											

2/16/2023 N:\P5072-1

					E	DESIG	NED E	BEAMS	STRAIG	HT S	STRAND.	5)										OPTION	AL DESIGI	V			AD RA		
					Р	RESTRE	ESSING .	STRANDS				DEBC	NDED ST						CONC		DESIGN	DESIGN	REQUIRED	LIVE			FACTC	DRS	
STRUCTURE	SPAN LENGTH	BEAM NO.	BEAM TYPE	NON- STD 7 STRAND PATTERN	TOTAL NO.	SIZE	STRGTH	"e" (L	"e" END	TOT NO. DEB	DIST FROM BOTTOM	STR	. OF ANDS DE-		(ft	R OF S BONDE from	D TO end)		RELEASE STRGTH	MINIMUM 28 DAY COMP STRGTH	LOAD COMP STRESS (TOP @) (SERVICE I)	LOAD TENSILE STRESS (BOTT Q) (SERVICE III)	MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I)	DISTRI FAC	TOR	STRE	ENGTH I	SERVICE III	
	(ft)			.,		(in)	f pu (ksi)	(in)	(in)		(in)	TOTAL	BONDED	3	6	9	12	15	f'ci (ksi)	f'c (ksi)	fct (ksi)	fcb (ksi)	(kip-ft)	Moment	Shear	Inv	0pr	Inv	+
	25	ALL	55B12		8	0.6	270	3.50	3.50	0	2.5	8	0	0	0	0	0	0	4.000	5.000	0.914	-1.217	448	0.450	0.450	1.40	1.82	1.71	1
24' ROADWAY	30	ALL	5SB12		10	0.6	270	3.50	3.50	0	2.5	10	0	0	0	0	0	0	4.000	5.000	1.292	-1.685	530	0.450	0.450	1.25	1.62	1.29	
SB12 BEAM	35	ALL	5SB12		14	0.6	270	3.50	3.50	0	2.5	14	0	0	0	0	0	0	4.000	5.000	1.730	-2.219	675	0.450	0.450	1.33	1.73	1.23	
	40	ALL	5SB12		18	0.6	270	3.50	3.50	0	2.5	18	0	0	0	0	0	0	4.000	5.000	2.218	-2.796	820	0.440	0.440	1.34	1.74	1.12	
	25	ALL	5SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0	0	0	0	0	4.000	5.000	0.725	-0.897	551	0.450	0.450	1.77	2.29	2.41	1
	30	ALL	5SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0	0	0	0	0	4.000	5.000	1.020	-1.244	574	0.450	0.450	1.23	1.59	1.45	
24' ROADWAY	35	ALL	5SB15		10	0.6	270	5.00	5.00	0	2.5	10	0	0	0	0	0	0	4.000	5.000	1.361	-1.640	708	0.450	0.450	1.15	1.49	1.14	
SB15 BEAM	40	ALL	5SB15		14	0.6	270	5.00	5.00	0	2.5	14	0	0	0	0	0	0	4.000	5.000	1.739	-2.068	864	0.440	0.440	1.32	1.71	1.19	
	45	ALL	5SB15		18	0.6	270	5.00	5.00	2	2.5	18	2	2	0	0	0	0	4.000	5.000	2.179	-2.574	1054	0.440	0.440	1.34	1.73	1.08	
	50	ALL	5SB15		24	0.6	270	5.00	5.00	8	2.5	24	8	4	4	0	0	0	4.000	5.000	2.680	-3.153	1276	0.440	0.440	1.33	1.72	1.11	
28' ROADWAY	25	ALL	5SB12		8	0.6	270	3.50	3.50	0	2.5	8	0	0	0	0	0	0	4.000	5.000	0.903	-1.184	444	0.430	0.430	1.47	1.91	1.80	
SB12 BEAM	30	ALL	5SB12		10	0.6	270	3.50	3.50	0	2.5	10	0	0	0	0	0	0	4.000	5.000	1.276	-1.639	508	0.430	0.430	1.32	1.71	1.37	
	35	ALL	55B12		12	0.6	270	3.50	3.50	0	2.5	12	0	0	0	0	0	0	4.000	5.000	1.708	-2.159	647	0.430	0.430	1.18	1.53	1.02	
	40	ALL	5SB12		18	0.6	270	3.50	3.50	0	2.5	18	0	0	0	0	0	0	4.000	5.000	2.200	-2.744	799	0.430	0.430	1.37	1.78	1.17	
	25	ALL	5SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0	0	0	0	0	4.000	5.000	0.716	-0.874	529	0.430	0.430	1.85	2.40	2.53	
	30	ALL	5SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0	0	0	0	0	4.000	5.000	1.007	-1.212	570	0.430	0.430	1.29	1.67	1.53	
28' ROADWAY SB15 BEAM	35	ALL	5SB15		10	0.6	270	5.00	5.00	0	2.5	10	0	0	0	0	0	0	4.000	5.000	1.343	-1.598	680	0.430	0.430	1.21	1.57	1.22	
	40	ALL	5SB15		14	0.6	270	5.00	5.00	0	2.5	14	0	0	0	0	0	0	4.000	5.000	1.725	-2.032	842	0.430	0.430	1.36	1.76	1.24	
	45	ALL	5SB15		18	0.6	270	5.00	5.00	2	2.5	18	2	2	0	0	0	0	4.000	5.000	2.149	-2.508	1013	0.420	0.420	1.41	1.82	1.16	
	50	ALL	5SB15		22	0.6	270	5.00	5.00	6	2.5	22	6	4	2	0	0	0	4.000	5.000	2.643	-3.073	1227	0.420	0.420	1.33	1.72	1.01	
	25	ALL	4SB12		6	0.6	270	3.50	3.50	0	2.5	6	0	0	0	0	0	0	4.000	5.000	0.904	-1.187	341	0.340	0.340	1.38	1.79	1.67	
30' ROADWAY	30	ALL	4SB12		8	0.6	270	3.50	3.50	0	2.5	8	0	0	0	0	0	0	4.000	5.000	1.277	-1.646	407	0.340	0.340	1.32	1.71	1.37	
SB12 BEAM	35	ALL	4SB12		10	0.6	270	3.50	3.50	0	2.5	10	0	0	0	0	0	0	4.000	5.000	1.711	-2.169	518	0.340	0.340	1.24	1.60	1.08	
	40	ALL	4SB12		14	0.6	270	3.50	3.50	0	2.5	14	0	0	0	0	0	0	4.000	5.000	2.205	-2.758	640	0.340	0.340	1.34	1.73	1.11	
	25	ALL	4SB15		6	0.6	270	5.00	5.00	0	2.5	6	0	0	0	0	0	0	4.000	5.000	0.723	-0.888	431	0.350	0.350	1.69	2.19	2.32	
	30	ALL	4SB15		6	0.6	270	5.00	5.00	0	2.5	6	0	0	0	0	0	0	4.000	5.000	1.017	-1.231	438	0.350	0.350	1.16	1.50	1.37	
30' ROADWAY	35	ALL	4SB15		8	0.6	270	5.00	5.00	0	2.5	8	0	0	0	0	0	0	4.000	5.000	1.346	-1.605	545	0.340	0.340	1.21	1.57	1.21	
SB15 BEAM	40	ALL	4SB15		12	0.6	270	5.00	5.00	0	2.5	12	0	0	0	0	0	0	4.000	5.000	1.729	-2.043	675	0.340	0.340	1.47	1.91	1.38	
	45	ALL	4SB15		14	0.6	270	5.00	5.00	2	2.5	14	2	2	0	0	0	0	4.000	5.000	2.166	-2.542	823	0.340	0.340	1.33	1.73	1.06	
	50	ALL	4SB15		18	0.6	270	5.00	5.00	4	2.5	18	4	2	2	0	0	0	4.000	5.000	2.665	-3.115	998	0.340	0.340	1.32	1.71	1.02	

1) Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

Tension = $0.24\sqrt{f'ci}$

Optional designs must likewise conform.

2 Portion of full HL93.

DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to AASHTO Manual for Bridge Evaluation.

Prestress losses for the designed beams have been calculated for a

relative humidity of 60 percent. Optional designs must likewise conform.

FABRICATION NOTES: Provide Class H concrete.

Provide Grade 60 reinforcing steel.

Use low relaxation strands, each pretensioned to 75 percent of fpu. Full-length debonded strands are not permitted in positions "A" and "B". Strand debonding must comply with Item 424.4.2.2.2.4.

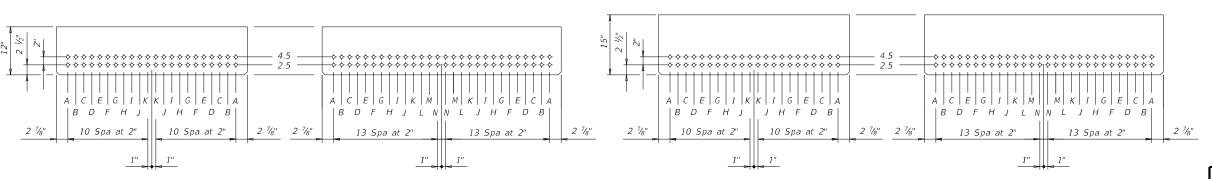
When shown on this sheet, the Fabricator has the option of furnishing either the designed beam or an approved optional beam design. All optional design submittals and shop drawings must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.

Locate strands for the designed beam as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5". Place strands within a row as follows:

1) Locate a strand in each "A" position.

2) Place strand symmetrically about vertical centerline of beam.

3) Space strands as equally as possible across the entire width. Do not debond strands in position "A". Distribute debonded strands symmetrically about the vertical centerline. Increase debonded lengths working outward, with debonding staggered in each row.



TXDOT 4SB12 SLAB BEAM

TXDOT 5SB12 SLAB BEAM

TXDOT 4SB15 SLAB BEAM

TXDOT 5SB15 SLAB BEAM

HL93 LOADING

PRESTRESSED CONCRETE SLAB BEAM STD DESIGNS (TY SB12 OR SB15)

24', 28' & 30' ROADWAY

PSBSD

FILE: psbsts08-21.dgn	DN: SF	RW.	CK: BMP	DW:	SF5	ck: 5DB
©TxD0T January 2017	CONT	SECT	JOB			HIGHWAY
REVISIONS 1-21: Added load rating.	0918	11	100, ET	С.	CR	110, ETC.
1 211 Madea load Fating.	DIST		COUNTY			SHEET NO.
	DAL		KAUFM.	ΑN		105

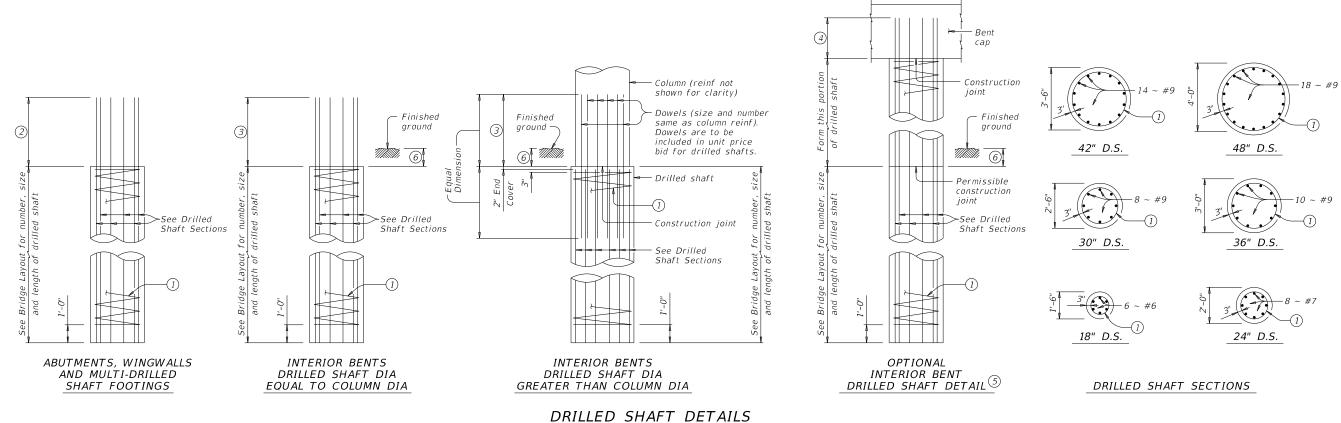
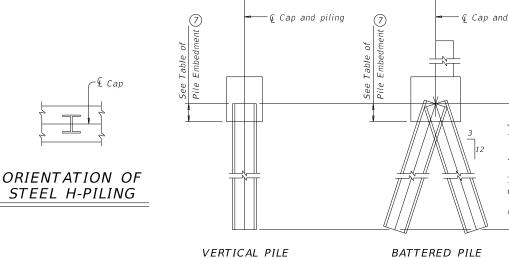
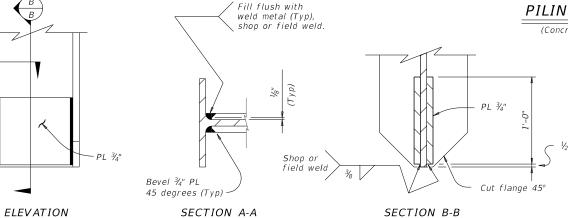


TABLE OF PILE EMBEDMENT Pile Type Embedment Depth (Ft) 16" Sq Concrete 18" Sq Concrete HP14 Steel 1'-0" HP16 Steel 20" Sq Concrete 24" Sq Concrete 1'-6" HPİ8 Steel

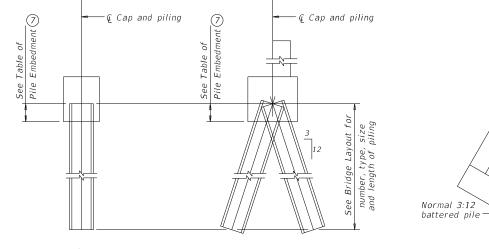
See Prestressed Concrete Piling (CP) standard for additional details on concrete pile embedment.





STEEL H-PILE TIP REINFORCEMENT

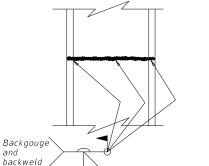
See Item 407 "Steel Piling" to determine when tip reinforcement is required and for options to the details shown.



BATTERED PILE

PILING DETAILS

(Concrete or steel H)



SECTION THRU FLANGE OR WEB

STEEL H-PILE SPLICE DETAIL

Use when required.

1) #3 spiral at 6" pitch (one and a half flat turns top and bottom).

Min extension into supported element:

#6 Bars = 1'-11" #7 Bars = 2'-0" #9 Bars = 2'-3"

3 Min lap with column reinf. #7 Bars = 2'-11" #9 Bars = 3'-9" #11 Bars = 4'-8"

4 Min extension into supported element: #6 Bars = 1'-11"

 $\#7 \; Bars = 2'-3''$ $#9 \ Bars = 2'-9"$

If unable to avoid

conflict with wingwall

group regardless of

which pile would be battered back, one

pile in group may be

vertical.

Piling

group

DETAIL "A"

(Showing plan view of a 30° skewed abutment)

piling at exterior pile

⑤ Drilled shafts may extend to the bottom of bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the drilled shaft diameter equals the column diameter. Obtain approval of the forming method above the ground line prior to construction. No adjustments in payment will be made if this option is used.

6 1'-0" Min, unless shown otherwise on plans.

7 Or as shown on plans.

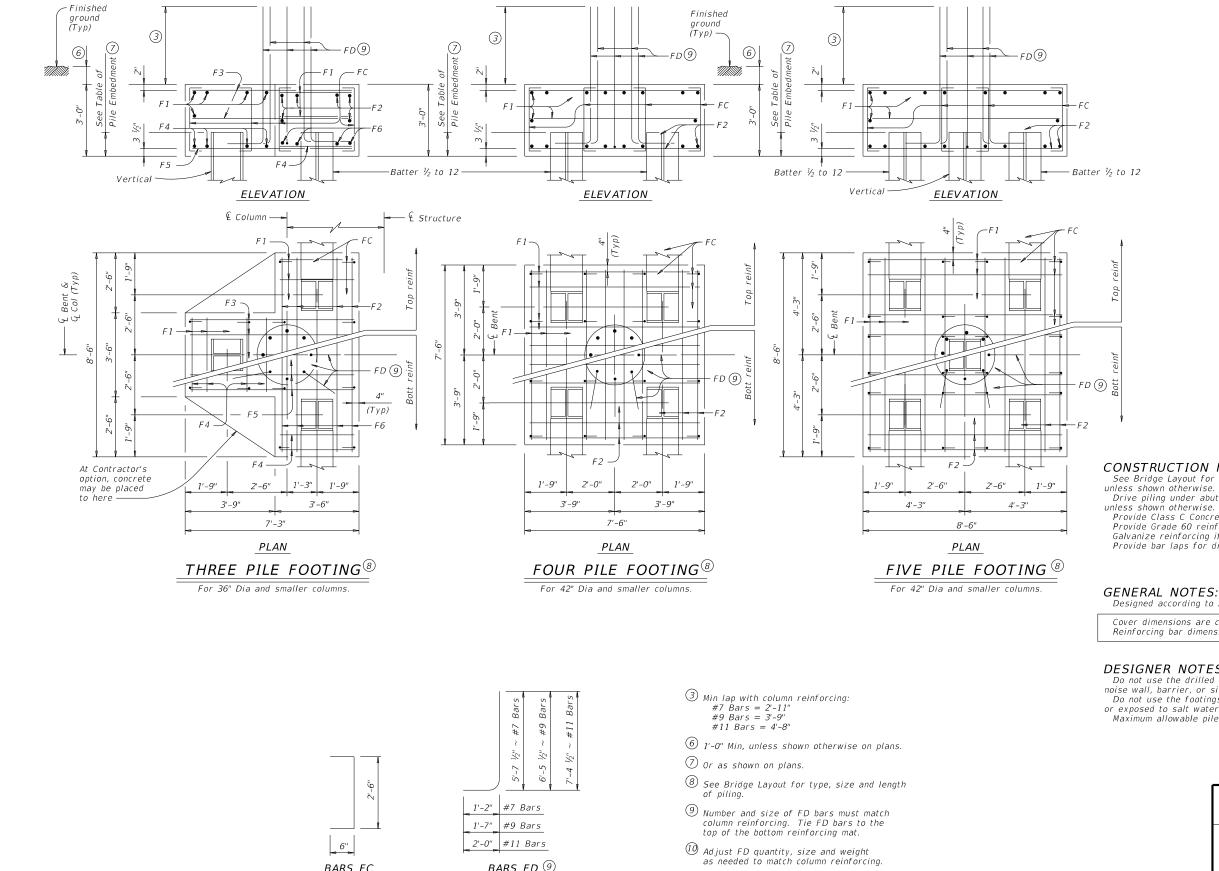
SHEET 1 OF 2



COMMON FOUNDATION **DETAILS**

FD

FILE: fdstde01-20.dgn	DN: TXL	70T	ck: TxD0T	DW:	TxD0T	CK: TXDOT
©TxDOT April 2019	CONT	SECT	JOB		HI	GHWAY
REVISIONS	0918	11	100,ET	C.	CR 1	10,ETC.
01-20: Added #11 bars to the FD bars.	DIST		COUNTY			SHEET NO.
	DAL		KAUFM	ΔN		106



BARS FD 9

BARS FC

2/16/2023 N:\P5072-1

TABLE OF FOOTING QUANTITIES FOR 30" COLUMNS

		ONE 3	PILE FOOT	rING	
Bar	No.	Size	Lengt	h	Weight
F 1	11	#4	3'- 2	"	23
F2	6	#4	8'- 2	"	33
F3	6	#4	6'- 1	l"	28
F4	8	#9	3'- 2	"	86
F5	4	#9	6'- 1	l"	94
F6	4	#9	8'- 2	"	111
FC	12	#4	3'- 6	"	28
FD (10)	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	623
Class	"C" Cc	ncrete		CY	4.8
		ONE 4	PILE FOOT	ING	
Bar	No.	Size	Lengt	h	Weight
F 1	20	#4	7'- 2	"	96
F2	16	#8	7'- 2	"	306
FC	16	#4	3'- 6	"	<i>37</i>
FD 10	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	659
Class	"C" Cc	ncrete		CY	6.3
		ONE 5	PILE FOOT	ING	
Bar	No.	Size	Lengt	h	Weight
F 1	20	#4	8'- 2	"	109
F2	16	#9	8'- 2	"	444
FC	24	#4	3'- 6	"	56
FD (10)	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	829
Class	"C" Cc	ncrete		CY	8.0
_					_

CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise.

Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile unless shown otherwise.

Provide Class C Concrete (f'c = 3,600 psi), unless shown otherwise.
Provide Grade 60 reinforcing steel.
Galvanize reinforcing if shown elsewhere in the plans.

Provide bar laps for drilled shaft reinforcing, where required, as follows: Uncoated or galvanized (#6) ~ 2'-6" Uncoated or galvanized (#7) ~ 2'-11" Uncoated or galvanized (#9) ~ 3'-9"

Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

DESIGNER NOTES:

Do not use the drilled shaft details shown on this standard for retaining wall,

noise wall, barrier, or sign foundations without structural evaluation.

Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.

Maximum allowable pile loads for the footings shown are:
72 Tons/Pile with 24" Dia Columns
80 Tons/Pile with 30" Dia Columns
100 Tons/Pile with 36" Dia Columns 120 Tons/Pile with 42" Dia Columns

SHEET 2 OF 2



COMMON FOUNDATION **DETAILS**

FD

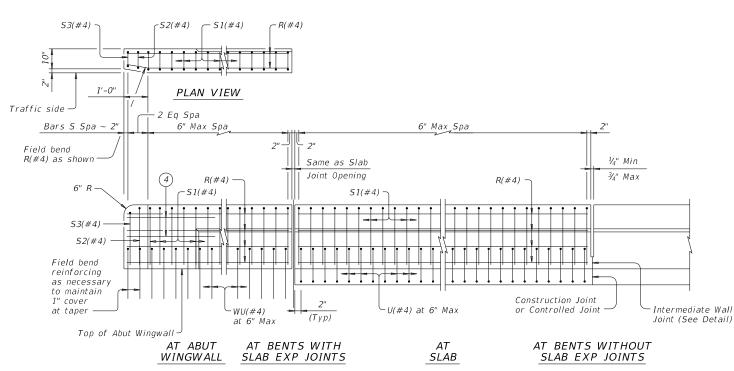
Bridge Division Standard

: fdstde01-20.dgn	DN: TXE	DOT.	ck: TxD0T	DW:	TxD0T		ck: TxD0T
TxDOT April 2019	CONT	SECT	JOB			HIG	HWAY
REVISIONS	0918	11	100,ET	С.	CR	11	O,ETC.
1-20: Added #11 bars to the FD bars.	DIST		COUNTY				SHEET NO.
	DAL		KAUFM	ΔN			107

€ Thrie-Beam Terminal

Connector (1)

Wingwall Length Parapet Panel Length Parapet Panel Length (Variable) 5'-0" Min End of Bridge Rail -Face of Abut Bkwl for payment € Intermediate Wall Joint (See Detail) — 3'-0" (2) - End of Offset in back of rail € Thrie-Beam Terminal ½" Min Same as Slab Same as Slab Connector (1) Jt Opening Jt Opening ¾" Max Intermediate Wall Construction Joint or Controlled Joint Joint (See Detail) limits of Abut Wingwall AT ABUTMENTS AT BENTS WITH SLAB EXP JOINTS AT BENTS WITHOUT SLAB EXP JOINTS ROADWAY ELEVATION OF RAIL



ELEVATION SHOWING TYPICAL REINFORCING PLACEMENT

TERMINAL CONNECTION DETAILS

SECTION

 \cline{C} 5 ~ 1" Dia holes and 2 $\cline{V_2}$ " Dia x 2" deep recesses. Form or core holes and recesses. Percussion drilling is not permitted. Adjust placement of reinforcing steel as necessary to avoid

bolt holes and recesses. Bolt recesses are only required when pedestrian sidewalks are adjacent to back of rail.

Top of Abut Wingwall

Provide bolts of sufficient length to extend 1/2" to 3/4" beyond

- 1 Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.
- Back of rail offset may, with Engineer's approval, be continued to the end of the railing.
- $\begin{tabular}{l} \hline \end{tabular}$ Increase 2" for structures with overlay.

ELEVATION

4 Place 4 additional Bars R(#4) 3'-8" in length inside Bars S(#4) and centered 2'-0" from end of rail when Terminal Connections are required. Field bend as needed.

Qutside Edge of Slab or Abut Wingwall Quitable Slab Quitable Edge of Slab or Abut Wingwall Quitable Edge of Slab or Abut Wingwall Quitable Edge of Slab Expansion Joint, Quitable Expansion Joint of Slab Expansion Joint, Quitable Expansion Joint of Slab Expansion Joint, Quitable Edge of Slab outside edge. Quitable Edge of Slab of Slab of Slab Expansion Joint, Location of Rail Expansion Joint, Quitable Expansion Joint, Quitable Edge of Slab of Slab of Slab outside edge. Quitable Edge of Slab of Slab of Slab outside edge. Quitable Edge of Slab of Slab outside edge. Cross-hatched area must have \(\frac{1}{2} \) Preformed Bitumuminous \(\frac{1}{2} \) Fiber Material under concrete \(\text{rail}, \text{ as shown}. \)

PLAN OF RAIL AT EXPANSION JOINTS

Example showing Slab Expansion Joints without breakbacks.

SHEET 1 OF 2

Form to here.

Tool V groove

Construction Joint or Controlled Joint

3

INTERMEDIATE WALL JOINT DETAIL

Provide at all interior bents without slab expansion joints.

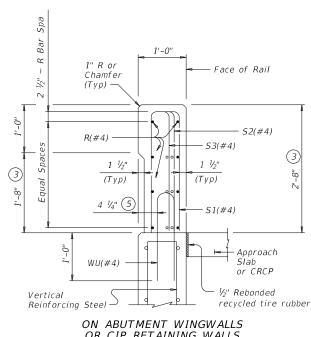
Texas Department of Transportation

TRAFFIC RAIL

Bridge Division Standard

TYPE T221

FILE: 11.	5tu004-19.ugn	DIV: TXL	JU1	CK: TXDUT	DW:	JIN		CK: I XDUI
©TxD0T	September 2019	CONT	SECT	JOB			HIG	HWAY
	REVISIONS	0918	11	100, ET	С.	CR	11	O, ETC.
		DIST		COUNTY				SHEET NO.
		DΔI		KΔIIFM	ΔΝ			108

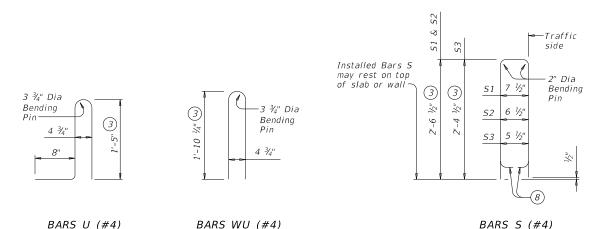


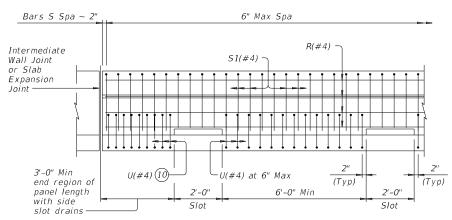
1" R or - Face of Rail Chamfer (Typ)51(#4) R(#4) 1 1/2" 3 (Typ)(Typ) 4 1/4"

OR CIP RETAINING WALLS

ON BRIDGE SLAB

SECTIONS THRU RAIL





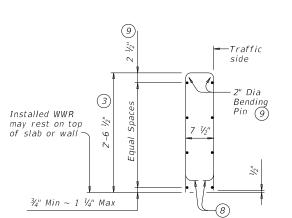
OPTIONAL SIDE SLOT DRAIN DETAIL

Note: Side Slot Drains may be used where shown elsewhere on the plans or as directed by the Engineer. Drains should not be placed over railroad tracks, lower roadways, or sidewalks. When this rail is used as a separator between a roadway surface and a sidewalk surface, side drain slots will not be permitted.

R(#4)Adjust bottom bars R(#4) as required to (Typ)maintain 2 cover over slots. cut bars S(#4) as required at slots.-

SECTION THRU OPTIONAL SIDE SLOT DRAIN

- ③ Increase 2" for structures with overlay.
- cover over horizontal reinforcing in abutment wingwalls or retaining walls on traffic side of wall.
- 6 As an aid in supporting reinforcement, additional longitudinal bars may be used in the slab with the approval of the Engineer. Such bars will be furnished at the Contractors expense.
- (7) Top longitudinal slab bar may be adjusted laterally 3" plus or minus to tie reinforcing.
- 8 Bend or cut as required to clear drain slots.
- 9 No longitudinal wires may be in top center of cage.
- 10 Space U(#4) bars at 4" Max when end region of panel length is less than 6'-0" to side slot drain. Space U(#4) bars at 6" Max when end region of panel length is 6'-0" and greater to side slot drain.



OPTIONAL WELDED WIRE REINFORCEMENT (WWR)

DESCRIPTION	LONGITUDINAL WIRES	VERTICAL WIRES
Minimum (Cumulative Total) Wire Area	1.067 Sq In.	0.267 Sq In. per Ft
	No. of Wires	Spacing
Minimum	8	4"
Maximum	10	8"
Maximum Wire Size Differential	The smaller wire mus of 40% or more of th	

CONSTRUCTION NOTES:

This railing may be constructed by the slipform process when approved by the Engineer, with equipment approved by the Engineer. Provide sensor control for both line and grade. Tack welding to provide bracing for slipform operations is acceptable. Welding may be performed at a minimum spacing of 3 ft between the cage and the anchorage. It is permissible to weld to bars U, WU and S at any location on the cage. If increased bracing is needed, provide additional anchorage devices and weld in the upper two thirds of the cage. Paint welded areas on epoxy coated and/or galvanized reinforcing with an organic zinc rich paint in accordance with Item 445 "Galvanizing".

If rail is slipformed, apply an heavy epoxy bead 1" behind toe of traffic side of rail to concrete deck just prior to slip forming. Provide a $\frac{3}{8}$ " width x $\frac{1}{4}$ " tall heavy epoxy bead with Type III, Class C or a Type V epoxy.

Face of rail and parapet must be vertical transversely unless otherwise shown in the plans or approved by the Engineer. Chamfer all exposed concrete corners.

MATERIAL NOTES:

Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere.

Provide Grade 60 reinforcing steel.

Epoxy coat or galvanize all reinforcing steel if slab bars are

epoxy coated or galvanized.

Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars U and WU unless noted otherwise. Deformed WWR (ASTM 1064) may be substituted for Bars R and S, as shown. Combinations of reinforcing steel and WWR or configurations of WWR other that shown are permitted if conditions in the table are satisfied.

Provide the same laps as required for reinforcing bars. Provide bar laps, where required, as follows:

Uncoated or galvanized $\sim #4 = 1'-7''$

Epoxy coated $\sim #4 = 2'-5''$

GENERAL NOTES:

This rail has been evaluated and accepted to be of equal strength to railings with like geometry, which have been crash tested to meet MASH TL-3 criteria. This rail can be used for speeds of 50 mph and greater when a TL-3 rated guard fence transition is used. When a TL-2 rated guard fence transition is used, this rail can only be used for speeds of 45 mph and less. Do not use this railing on bridges with expansion joints

providing more than 5" movement. Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details elsewhere in plans for these modifications.

Shop drawings are not required for this rail. Average weight of railing with no overlay is 370 plf.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

SHEET 2 OF 2

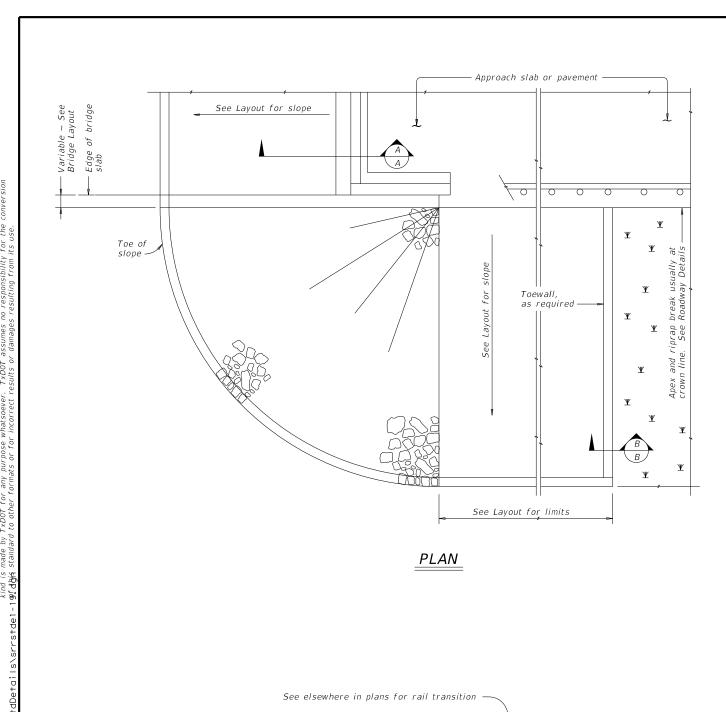


Bridge Division Standard

TRAFFIC RAIL

TYPE T221

E: r/std004-19.dgn	DN: TXL	OOT .	ck: TxD0T	DW:	JTR	ck: TxD0T
TxDOT September 2019	CONT	SECT	JOB			HIGHWAY
REVISIONS	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAI		KAUFM.	ΔN		109

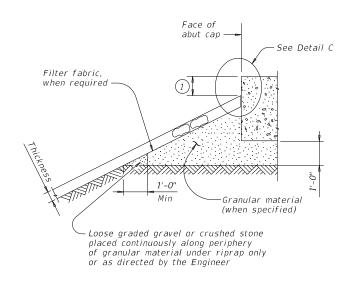


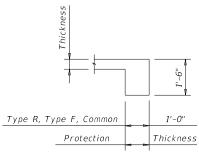
traffic rail -

ELEVATION

 Ψ

2/16/2023 4:58:45 PM N:\P5072-18-19-4\CADD

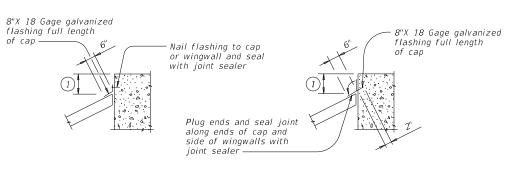




SECTION B-B

Provide toewall when shoulder drain is located adjacent to limits of stone riprap. Omit toewall when thickness of protection riprap is greater than 18".

SECTION A-A AT CAP



CAP OPTION A

CAP OPTION B

DETAIL C

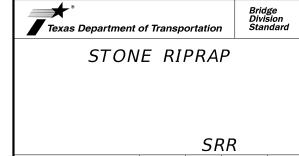
GENERAL NOTES:
Refer to Item 432, "Riprap" for stone size and gradation, and construction details. See Layout for limits and thickness of riprap specified.

See elsewhere in plans for locations and details of

shoulder drains.

1) Top of cap to top of riprap dimension varies as directed by the Engineer. Provide 9" Min for beam/slab type bridges and 1'-6" for slab span, box beam, or slab beam bridges.

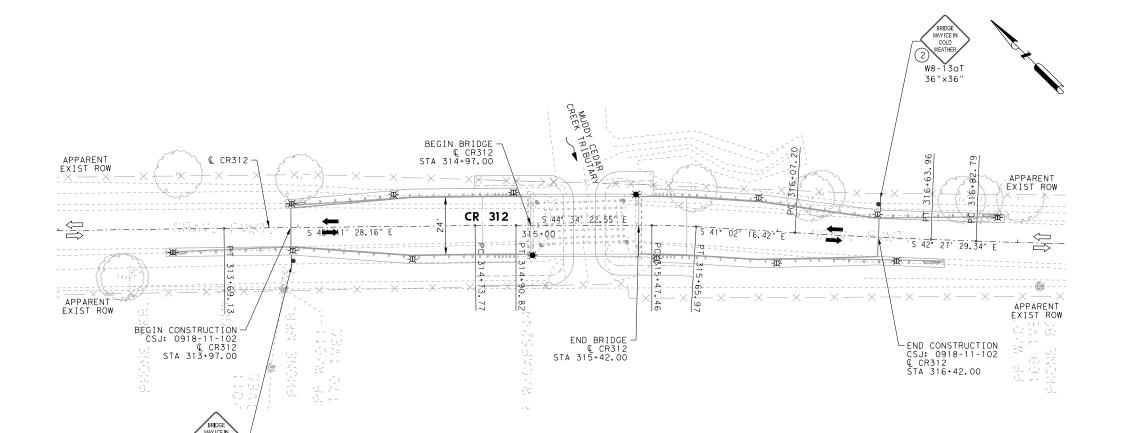
SHEET 1 OF 2



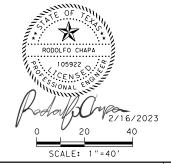
			ЭГ	תו		
FILE: srrstdel-19.dgn	DN: AE	5	ck: JGD	DW:	BWH	CK: AES
©TxDOT April 2019	CONT	SECT	JOB			HIGHWAY
REVISIONS	0918	11	100, ET	С.	CR	110, ETC.
	DIST		COUNTY			SHEET NO.
	DAL		KALLEM	ΛN		110

R110_SPML_01.dgn

CR279_SPML_01.dgr



RMorfin 2/16/2023 N:\P5072-18-19-4\CADD_BR_11-100\DC *.*.nor1-RW-HALF_PDF_pticfg



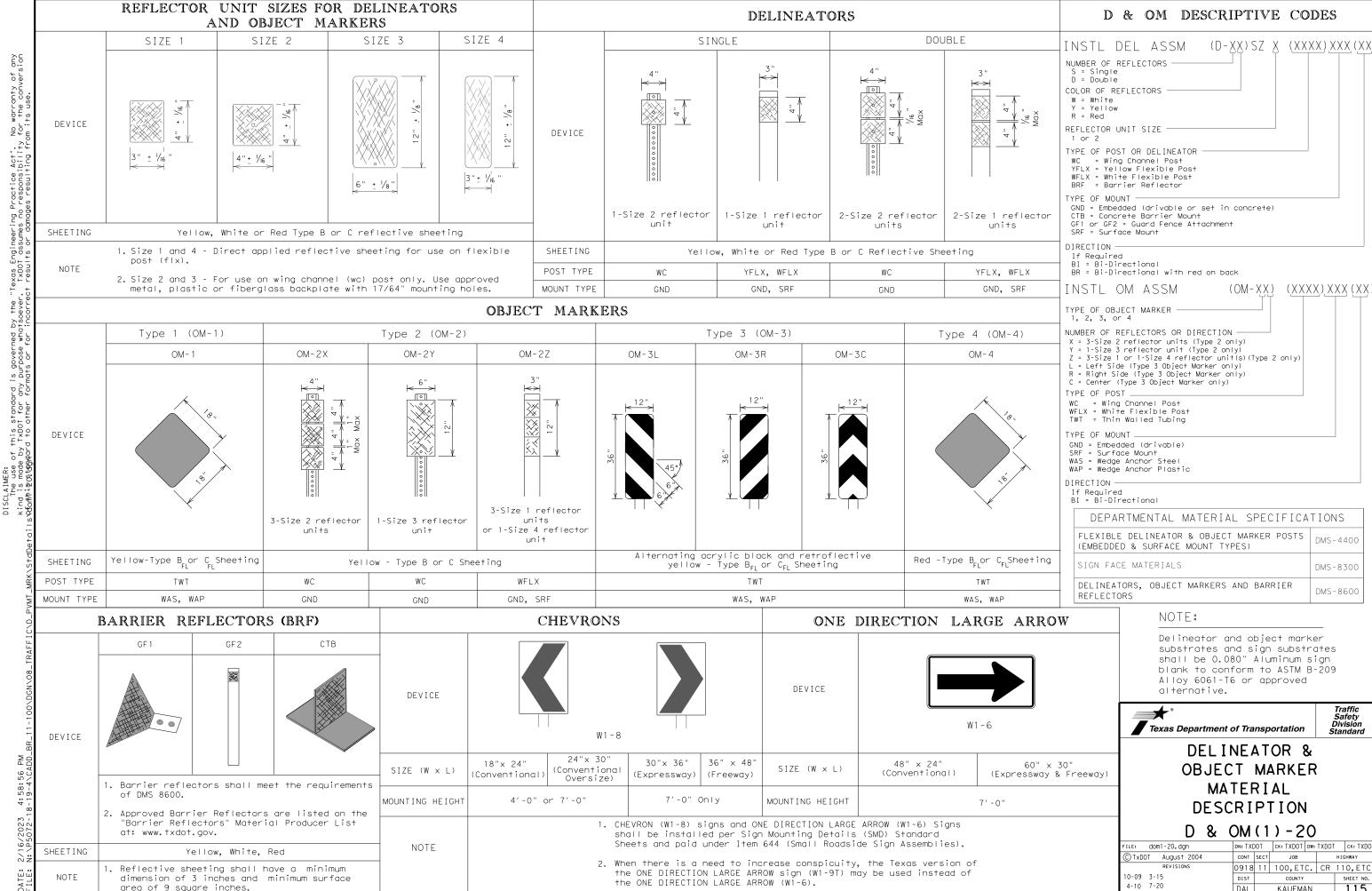
		SCALE: 1"=40'	
NO.	DATE	REVISION	APPRO\
_			





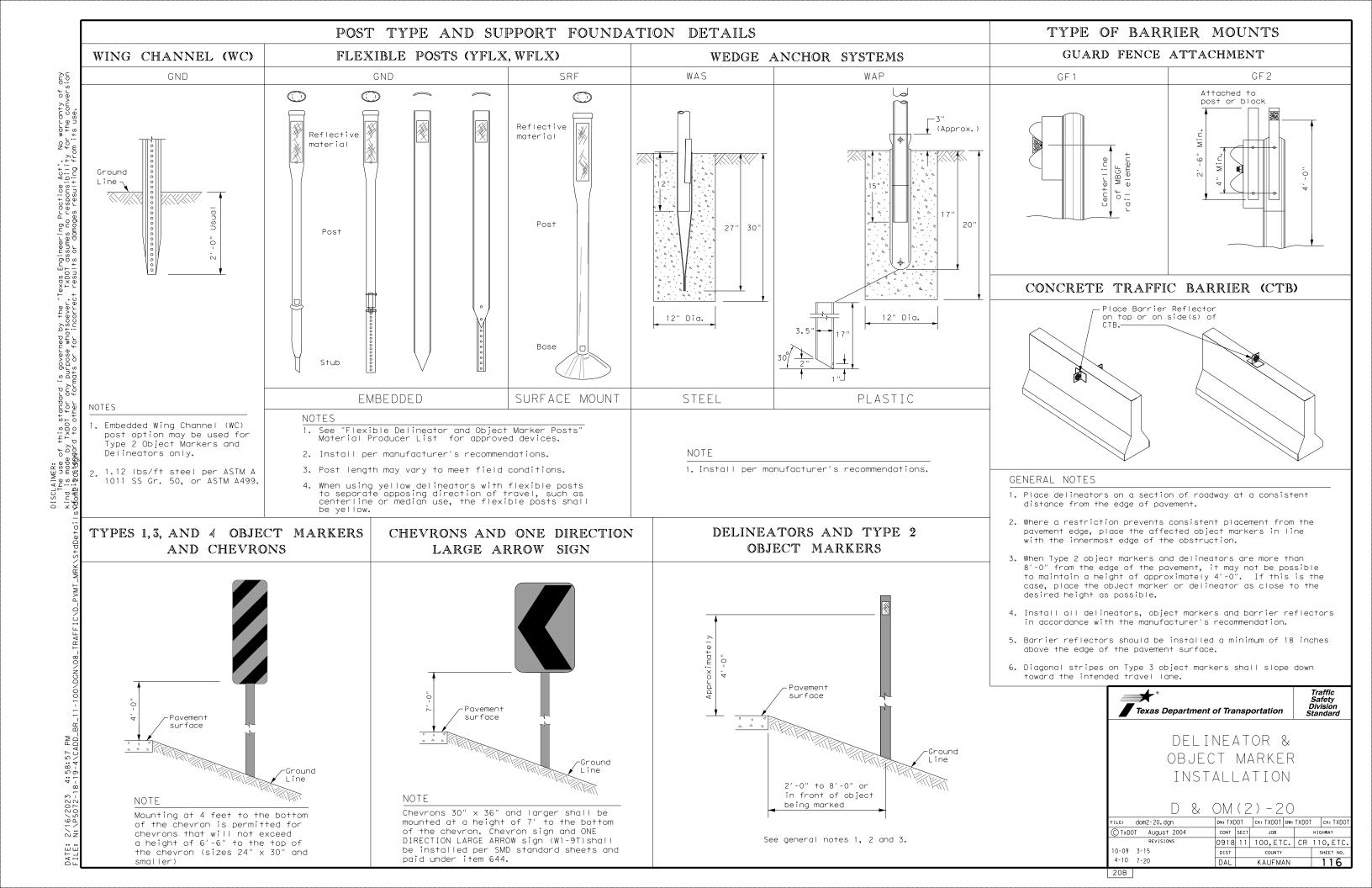
CR 312 SIGNING & PAVEMENT MARKINGS

SHEET 3	3 OF 3	3						
DN:	RM	FED. RD. DIV. NO.	STATE		PROJ	CT NO.		HIGHWAY NO.
CK DN:	RC	6	TEXAS	S SEE TITLE SHEET CR 11				
DW:	RM	STATE DIST.	COUNTY		CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
CK DW:	RC	DAL	KAUFI	MAN	0918	11	100, ETC	114



20A

0918 11 100, ETC. CR 110, ETC KAUFMAN

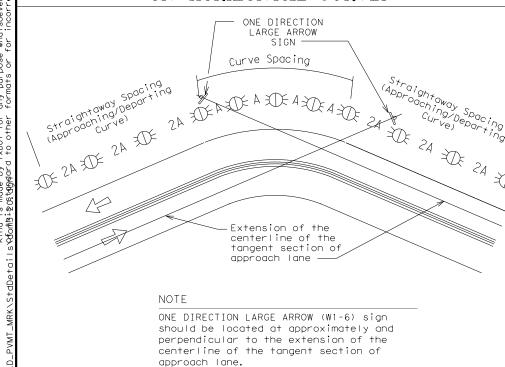


2/16/2023 N:\P5072-1

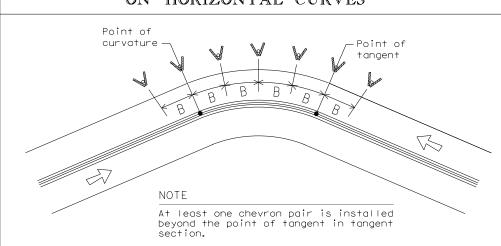
MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

Amount by which Advisory Speed	Curve Advisory Speed					
is less than Posted Speed	Turn (30 MPH or less)	Curve (35 MPH or more)				
5 MPH & 10 MPH	• RPMs	• RPMs				
15 MPH & 20 MPH	 RPMs and One Direction Large Arrow sign 	 RPMs and Chevrons; or RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons. 				
25 MPH & more	RPMs and Chevrons; or RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons	● RPMs and Chevrons				

SUGGESTED SPACING FOR DELINEATORS ON HORIZONTAL CURVES



SUGGESTED SPACING FOR CHEVRONS ON HORIZONTAL CURVES



DELINEATOR AND CHEVRON SPACING

WHEN DEGREE OF CURVE OR RADIUS IS KNOWN

			FEET	
Degree of Curve	Radius of Curve	Spacing in Curve	Spacing in Straightaway	Chevron Spacing in Curve
		А	2A	В
1	5730	225	450	
2	2865	160	320	
3	1910	130	260	200
4	1433	110	220	160
5	1146	100	200	160
6	955	90	180	160
7	819	85	170	160
8	716	75	150	160
9	637	75	150	120
10	573	70	140	120
1 1	521	65	130	120
12	478	60	120	120
13	441	60	120	120
1 4	409	55	110	80
15	382	55	110	80
16	358	55	110	80
19	302	50	100	80
23	249	40	80	80
29	198	35	70	40
38	151	30	60	40
57	101	20	40	40

Curve delineator approach and departure spacing should include 3 delineators spaced at 2A. This spacing should be used during design preparation or when the degree of curve is known.

DELINEATOR AND CHEVRON SPACING

WHEN DEGREE OF CURVE OR RADIUS IS NOT KNOWN

Advisory Speed (MPH)	Spacing in Curve	Spacing in Straightaway	Chevron Spacing in Curve
	А	2×A	В
65	130	260	200
60	110	220	160
55	100	200	160
50	85	170	160
45	75	150	120
40	70	140	120
35	60	120	120
30	55	110	80
25	50	100	80
20	40	80	80
15	35	70	40

If the degree of curve is not known, delineator spacing may be determined based on the Advisory Speed of the curve. Use the delineator curve spacing for each Advisory Speed (MPH).

CONDITION	REQUIRED TREATMENT	MINIMUM SPACING		
Frwy./Exp. Tangent	RPMs	See PM-series and FPM-series standard sheets		
Frwy./Exp. Curve	Single delineators on right side	See delineator spacing table		
Frwy/Exp.Ramp	Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4))	100 feet on ramp tangents Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves)		
Acceleration/Deceleration Lane	Double delineators (see Detail 3 on D&OM(4))	100 feet (See Detail 3 on D & OM (4))		
Truck Escape Ramp	Single red delineators on both sides	50 feet		
Bridge Rail (steel or concrete)and Metal Beam Guard Fence	Bi-Directional Delineators when undivided with one lane each direction Single Delineators when multiple lanes each direction	Equal spacing (100'max) but not less than 3 delineators		
Concrete Traffic Barrier (CTB) or Steel Traffic Barrier	Barrier reflectors matching the color of the edge line	Equal spacing 100' max		
Cable Barrier	Reflectors matching the color of the edge line	Every 5th cable barrier post (up to 100'max)		
Guard Rail Terminus/Impact Head	Divided highway - Object marker on approach end Undivided 2-lane highways - Object marker on approach and departure end	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6)		
Bridges with no Approach Rail	Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail	See D & OM(5)		
Reduced Width Approaches to Bridge Rail	Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end		
Culverts without MBGF	Type 2 Object Markers	See D & OM (5)		
	Double yellow delineators and RPMs	See Detail 2 on D & OM(4) See Detail 1 on D & OM (4)		
Crossovers Pavement Narrowing	Single delineators adjacent	See Seren For B & OW (47)		
(lane merge) on Freeways/Expressway	to affected lane for full length of transition	100 feet		

DELINEATOR AND OBJECT MARKER APPLICATION AND SPACING

NOTES

- 1. Unless indicated otherwise, the delineator or barrier reflector color shall conform to the color of the pavement edge line on the side of the road where the delineators or barrier reflectors are placed.
- 2. Barrier reflectors may be used to replace required delineators.
- 3. Single red delineators may be mounted on the back side of delineator posts for wrong way driver applications

LEGEND					
Bi-directional Delineator					
☐ Delineator					
-	Sign				



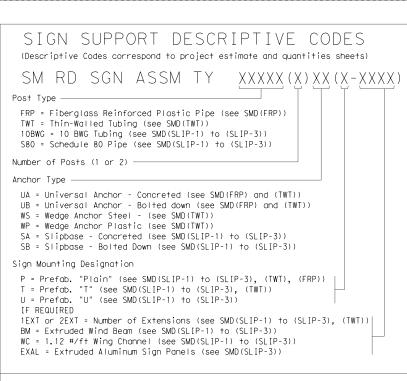
DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

D & OM(3) - 20

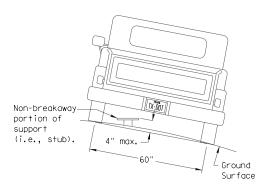
ILE: dom3-20.dgn	DN: TXDOT CK: TXDOT DW: TXDOT		ck: TXDOT			
DTxDOT August 2004	CONT	SECT	JOB		HIGHWAY	
	0918	11	100,ETC.	. CR	110,ETC.	
5-15 8-15	DIST		COUNTY		SHEET NO.	
3-15 7-20	DAL		KAUFMAN	117		

TWO-WAY, TWO LANE ROADWAY TWO-WAY, TWO LANE ROADWAY TWO-WAY, TWO LANE ROADWAY BRIDGE WITH NO APPROACH RAIL WITH REDUCED WIDTH APPROACH RAIL WITH METAL BEAM GUARD FENCE (MBGF) DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TxDOI for any purpose whatsoever. TxDOI assumes no responsibility for the conversion \donnaps@tdgpqard to other formats or for incorrect results or damages resulting from its use. See Note 1 See Note 1 See Note 1 **8** 出一 See Note 25 ft. 25 ft. 3- Type /\times D-SW D-SW delineators delineators spaced 25' spaced 25' $\overset{\sim}{\mathbb{R}}$ apart apart 出 MBGF Type D-SW Type D-SW delineators bidirectional delineators $\stackrel{\wedge}{\bowtie}$ bidirectional One barrier One barrier reflector shall reflector shall be placed $\stackrel{}{\bowtie}$ Steel or concrete-be placed directly behind Bridge rail directly behind each OM-3. each OM-3. The others The others $\stackrel{\times}{\triangleright}$ will have -Steel or concrete will have equal spacing Bridge rail equal spacing (100' max), but (100' max), but not less than 3 Bidirectional not less than 3 bidirectional Bidirectional white barrier bidirectional white barrier white barrier reflectors or white barrier Equal spacing (100′ max), but reflectors reflectors or delineators reflectors Equal spacing delineators not less than (100' max), but 3 bidirectional not less than 3 bidirectional white barrier white barrier reflectors or Equal delineators $\stackrel{\text{\tiny }}{\asymp}$ $\not \boxminus$ reflectors or Equal spacina spacing delineators (100' max), (100' max), but not but not less than less than 3 total. 3- Type $\stackrel{\text{\tiny }}{\succsim}$ \mathbb{R} \mathbb{R} 3 total. 3- Type D-SW D-SW delineators MBGF delineators spaced 25' spaced 25' apart \mathbb{R} apart $\stackrel{\sim}{\bowtie}$ Line Type D-SW <u>↓</u> \ Line \mathbb{R} Type D-SW \Rightarrow delineators delineators bidirectional bidirectional $\stackrel{\wedge}{\bowtie}$ MBGF $\stackrel{\wedge}{\mathbb{A}}$ $\stackrel{\wedge}{\bowtie}$ Traffic Safety Division Standard LEGEND 25 ft. 25 ft. 25 ft. 25 ft. Texas Department of Transportation Bidirectional Delineato DELINEATOR & \mathbb{R} Delineator See Note See Note 1 OBJECT MARKER OM - 3 PLACEMENT DETAILS 2/16/2023 N: \P5072-1 OM-2 D & OM(5) - 201. Terminal ends require reflective 1. Terminal ends require reflective sheeting provided by manufacturer sheeting provided by manufacturer DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDOT FILE: dom5-20.dgn per D & OM (VIA) or a Type 3 per D & OM (VIA) or a Type 3 Terminal End © TxDOT August 2015 JOB Object Marker (OM-3) in front of Object Marker (OM-3) in front 0918 11 100, ETC. CR 110, ETC the terminal end. of the terminal end. Traffic Flow KAUFMAN 20E

≈ ž

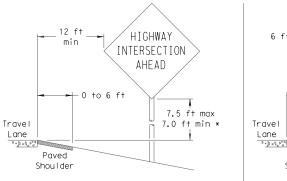


REQUIRED CLEARANCE FOR BREAKAWAY SUPPORT



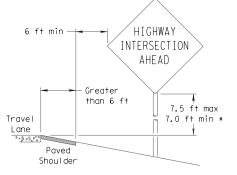
To avoid vehicle undercarriage snagging, any substantial remains of a breakaway support. when it is broken away, should not project more than 4 inches above a 60-inch chord (i.e., typical space between wheel paths).

SIGN LOCATION



LESS THAN 6 FT. WIDE

When the shoulder is 6 ft. or less in width. the sign must be placed at least 12 ft. from the edge of the travel lane.



GREATER THAN 6 FT. WIDE

When the shoulder is greater than 6 ft in width. the sign must be placed at least 6 ft. from the edge of the shoulder.

HIGHWAY

INTERSECTION

AHEAD

Concrete

Barrier

RESTRICTED RIGHT-OF-WAY

(When 6 ft min. is not possible.)

7.5 ft max

7.0 ft min *

Right-of-way restrictions may be created

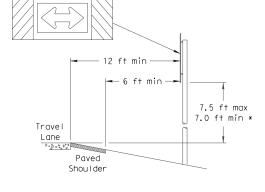
HIGHWAY

INTERSECTION

AHEAD

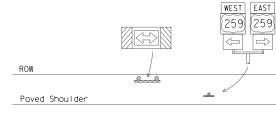
7.5 ft max

7.0 ft min →

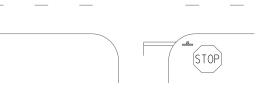


T-INTERSECTION

When this sign is needed at the end of a two-lane, two way roadway, the right edge of the sign should be in line with the centerline of the roadway. Place as close to ROW as practical.



Edge of Travel Lane



- * Signs shall be mounted using the following condition that results in the greatest sign elevation:
- (1) a minimum of 7 to a maximum of 7.5 feet above the edge of the travel lane or
- (2) a minimum of 7 to a maximum of 7.5 feet above the grade at the base of the support when sign is installed on the backslope.

The maximum values may be increased when directed by the Engineer.

See the Traffic Operations Division website for detailed drawings of sign clamps, Triangular Slipbase System components and Wedge Anchor System components.

The website address is: http://www.txdot.gov/publications/traffic.htm



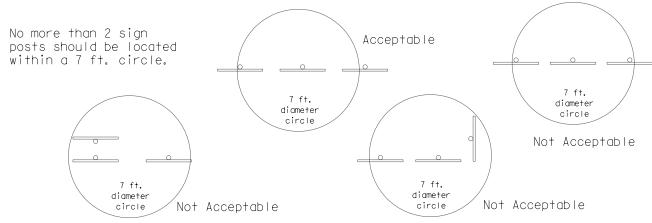
9-08

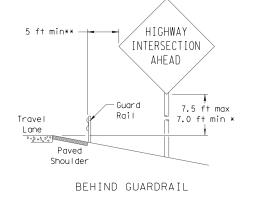
exas Department of Transportation Traffic Operations Division

SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS GENERAL NOTES & DETAILS

SMD (GEN) -08

DN: TXDOT		CK: TXDOT DW: T		TXDOT		CK:	TXDOT
CONT	SECT	JOB			HIG	HWA	Y
0918	11	100,ET	С.	CR	110),	ETC.
DIST	COUNTY		SHEET NO.		T NO.		
DAL		KAUFMA	lΝ			1	19
	CONT 0918 DIST	CONT SECT 0918 11 DIST	CONT SECT JOB 0918 11 100, ET DIST COUNTY	CONT SECT JOB 0918 11 100, ETC. DIST COUNTY	CONT SECT JOB 0918 11 100, ETC. CR DIST COUNTY	CONT SECT JOB HIG 0918 11 100, ETC. CR 110 DIST COUNTY S	CONT SECT JOB HIGHWA 0918 11 100, ETC. CR 110, DIST COUNTY SHEE





BEHIND CONCRETE BARRIER **Sign clearance based on distance required for proper guard rail or concrete barrier performance.

BEHIND BARRIER

2 ft min**

Paved

Shoul der

Travel

Maximum

Travel

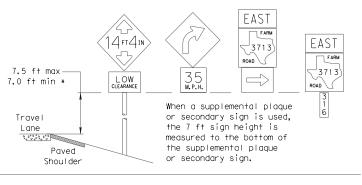
Lane

Shoulder

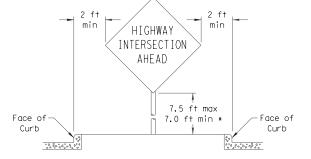
possible

PAVED SHOULDERS

SIGNS WITH PLAQUES









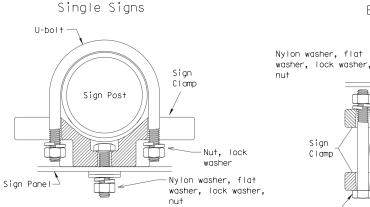
factors. In situations where a lateral restriction prevents the minimum horizontal clearance from the edge of the travel lane, signs should be placed as far from the travel lane as practical.

by rocks, water, vegetation, forest,

buildings, a narrow island, or other

*** Post may be shorter if protected by guardrail or if Engineer determines the post could not be hit due to extreme





Bolts used to mount sign panels to the clamp are 5/16-18 UNC galvanized square head with nut, nylon washer, flat washer and lock washer. The bolt length is 1 inch for aluminum.

When two sign clamps are used to mount signs back-to-back, use a 5/16-18 UNC galvanized hex head per ASTM A307 with nut and helical-spring lock washer. The approximate bolt lengths for various post sizes and sign clamp types are given in the table at right. The bolt length may need to be adjusted depending upon field conditions.

Sign clamps may be either the specific size clamp

26A

Approximate Bolt Length Pipe Diameter Specific Clamp 2" nominal 2 1/2" nominal 3" nominal

Clamp Bolt

Clamp

Nylon washer, flat

washer, lock washer,

3 or 3 1/2" 3 or 3 1/2" 3 1/2 or 4" 3 1/2 or 4"

Signs

Sign Pos-

Sign Panel

 ot Sign Panel

Universal Clamp

4 1/2"

- Sian Bolt

-Nut, lock washer

TRIANGULAR SLIPBASE INSTALLATION GENERAL REQUIREMENTS

10 BWG Tubing or Bolt Keeper Plate Schedule 80 Pipe (See General Note 3) Slip Base 5/8" structural bolts (3), nuts (3), and washers Washers (6) per ASTM A325 if required by or A449 and manufacturer galvanized per Item 445 "Galvanizing." Bolt length is 2 1/2". 3/4 " diameter hole. 36" Provide a 7" x 1/2" diameter rod or #4 rebar. Class A concrete 42 12" min. 24" max. Non-reinforced concrete footing (shall be used unless noted elsewhere in the plans). Foundation should take approx. 2.5 cf of concrete.

SM RD SGN ASSM TY XXXXXX(X)SA(X-XXXX)

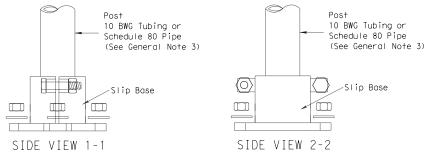
6" min

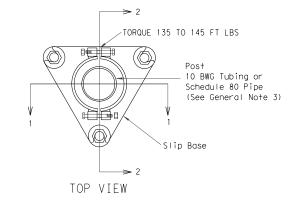
to edge

or joint

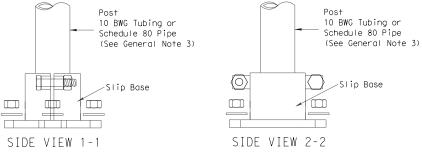
NOTE

The devices shall be installed per manufacturers' recommendations. Installation procedures shall be provided to the Engineer by Contractor.





DETAIL A



GENERAL NOTES:

- 1. Slip base shall be permanently marked to indicate manufacturer. Method, design, and location of marking are subject to approval of the TxDOT Traffic Standards Engineer.
- 2. Material used as post with this system shall conform to the following specifications:

10 BWG Tubing (2.875" outside diameter)

0.134" nominal wall thickness

Seamless or electric-resistance welded steel tubing or pipe

Steel shall be HSLAS Gr 55 per ASTM A1011 or ASTM A1008

Other steels may be used if they meet the following:

55,000 PSI minimum yield strength 70,000 PSI minimum tensile strength

20% minimum elongation in 2"

Woll thickness (uncoated) shall be within the range of 0.122" to 0.138"
Outside diameter (uncoated) shall be within the range of 2.867" to 2.883"
Galvanization per ASTM A123 or ASTM A653 G210. For precoated steel tubing (ASTM A653), recoat tube outside diameter weld seam by metallizing with zinc wire per ASTM B833.

Schedule 80 Pipe (2.875" outside diameter) 0.276" nominal wall thickness

Steel tubing per ASTM A500 Gr C

Other seamless or electric-resistance welded steel tubing or pipe with equivalent

outside diameter and wall thickness may be used if they meet the following:

46,000 PSI minimum yield strength 62,000 PSI minimum tensile strength

21% minimum elongation in 2"

Wall thickness (uncoated) shall be within the range of 0.248" to 0.304" Outside diameter (uncoated) shall be within the range of 2.855" to 2.895"

Galvanization per ASTM A123

3. See the Traffic Operations Division website for detailed drawings of sign clamps and Texas Universal Triangular Slipbase System components. The website address is:

http://www.txdot.gov/publications/traffic.htm

4. Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.

ASSEMBLY PROCEDURE

Foundation

- 1. Prepare 12-inch diameter by 42-inch deep hole. If solid rock is encountered, the depth of the foundation may be reduced such that it is embedded a minimum of 18 inches into the solid rock.
- 2. The Engineer may permit batches of concrete less than 2 cubic yards to be mixed with a portable, motor-driven concrete mixer. For small placements less than 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Concrete shall be Class A.
- 3. Push the pipe end of the slip base stub into the center of the concrete. Rotate the stub back and forth while pushing it down into the concrete to assure good contact between the concrete and stub.
- Continue to work the stub into the concrete until it is between 2 to 4 inches above the ground. 4. Plumb the stub. Allow a minimum of 4 days to set, unless otherwise directed by the Engineer.
- 5. The triangular slipbase system is multidirectional and is designed to release when struck from any direction.

- 1. Cut support so that the bottom of the sign will be 7 to 7.5 feet above the edge of the travelway (i.e., edge of the closest lane) when slip plate is below the edge of pavement or 7 to 7.5 feet above slip plate when the slip plate is above the edge of the travelway. The cut shall be plumb and
- 2. Attach sign to support using connections shown. When multiple signs are installed on the same support, ensure the minimum clearance between each sign is maintained. See SMD(SLIP-2) for clearances based on sign types.

ADDED DETAIL A FOR CLAMP BASE 10-2010



SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

SMD (SLIP-1) - 08 (DAL)

© TxDOT July 2002	DN: TXDOT		CK: TXDOT DW:		TXDOT		CK:	TXDO
9-08 REVISIONS	CONT	SECT	JOB			HIG	HWAY	′
12-10 (DISTRICT) ADDED CLAMP BASE DETAIL FOR SLIP	0918	11	100,ET	c.	CR 1	10),	ETO
	DIST	COUNTY				SHEET NO.		
BASE INSTALLATION	DAL	KAUFMAN					12	20

26B



SM RD SGN ASSM TY XXXXX(X)SB(X-XXXX)

CONCRETE ANCHOR

expansion or adhesive type.

5/8" diameter Concrete Anchor

8 places (embed a minimum of

5 1/2" and torque to min. of

50 ft-lbs), Anchor may be

weight concrete with a 5 1/2"

yield and ultimate tensile strength of 50 and 75 KSI, respectively. Nuts, bolts and washers shall be galvanized per Item 445, "Galvanizing." Adhesive type anchors shall have stud bolts installed with Type III epoxy per DMS-6100, "Epoxies and Adhesives." Adhesive anchors may be loaded after adequate epoxy cure time per the manufacturer's recommendations. Top of bolt shall extend at least flush with top of the nut when installed. The anchor, when installed in 4000 psi normal-

minimum embedment, shall have a

minimum allowable tension and shear

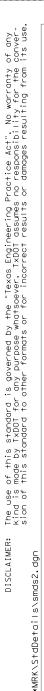
of 3900 and 3100 psi, respectively.

Concrete anchor consists of 5/8"

diameter stud bolt with UNC series bolt threads on the upper end. Heavy hex nut per ASTM A563, and

hardened washer per ASTM F436. The

stud bolt shall have a minimum



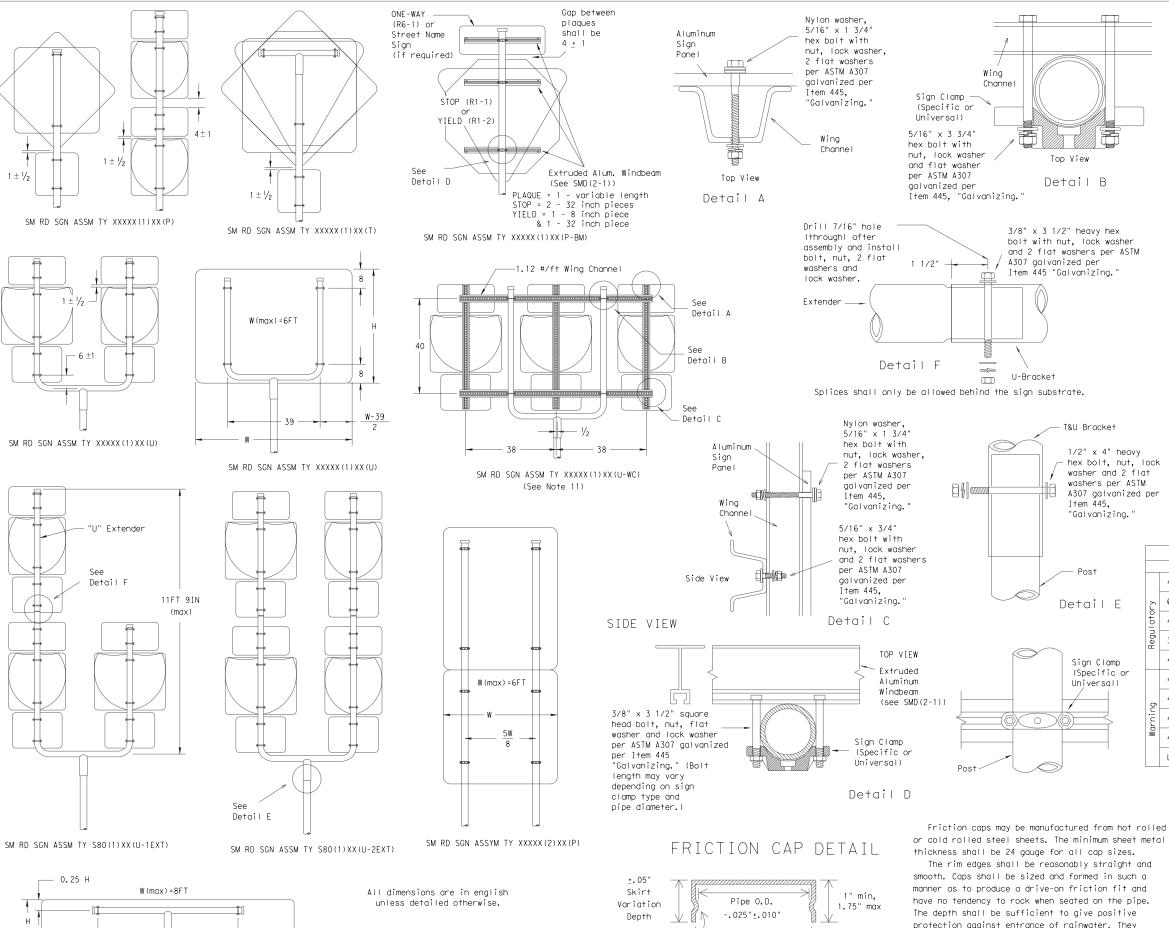


1-100/DGN/08*TRAFFIC/D*PVMT

2/16/2023 4:59:04 PM N:\P5072-18-19-4\CADD

- 0.2W

0.6W



Rolled Crimp to

engage pipe O.D.

Pipe O.D.

+.025" ±.010"

SM RD SGN ASSM TY XXXXX(1)XX(T)

(* - See Note 12)

GENERAL NOTES:

1.	SIGN SUPPORT	# OF POSTS	MAX. SIGN AREA
	10 BWG	1	16 SF
	10 BWG	2	32 SF
	Sch 80	1	32 SF
	Sch 80	2	64 SF

2. The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.

3. Sign supports shall not be spliced except where shown.

Sign support posts shall not be spliced.

4. Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.

5. Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.

6. For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of greater height.

7. When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly" connected to each other except through the sign panel. This will allow each support to act independently

when impacted by an errant vehicle.

8. Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.

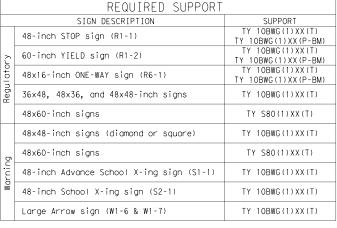
9. Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing.

10. Additional route markers may be added vertically, provided the total sign area does not exceed the maximum allowable amount per Note 1.

11. Additional sign clamp required on the "T-bracket" post for 24 inch height signs. Place the clamp 3 inches above bottom of sign when possible.

12. Post open ends shall be fitted with Friction Caps.

13. Sign blanks shall be the sizes and shapes shown on the plans.



exas Department of Transportation Traffic Operations Division

SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

SMD(SLIP-2)-08

© TxDOT July 2002	DN: TXD	тоот	CK: TXDOT	DW:	TXDO	T	CK:	TXDOT	
9-08 REVISIONS	CONT	SECT	JOB	JOB HIGHWA			HWA'	r	
	0918	11	100,ET	С.	CR 110, ETC			ETC.	
	DIST	COUNTY				SHEET NO.			
	DAL	KAUFMAN					121		

shall be free of sharp creases or indentations

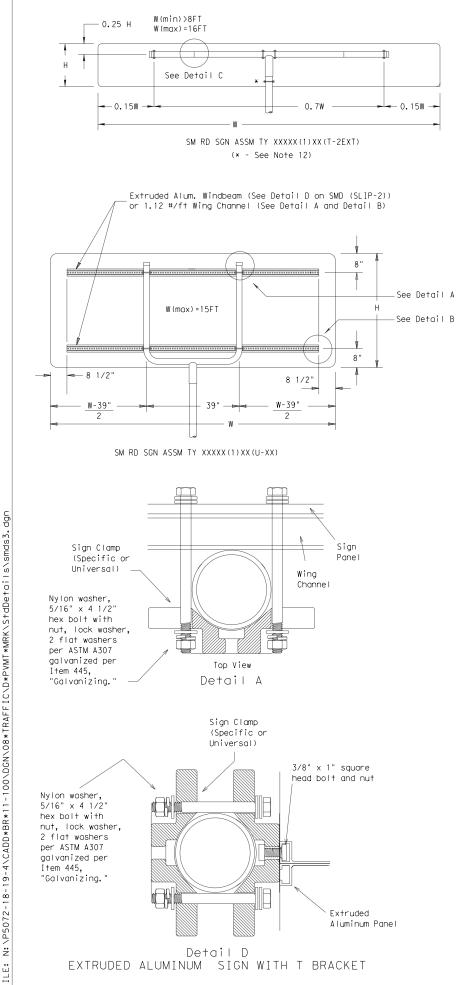
Caps shall have an electrodeposited coating of

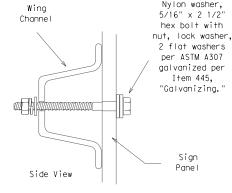
zinc in accordance with the requirements of ASTM

and show no evidence of metal fracture.

B633 Class FE/ZN 8.

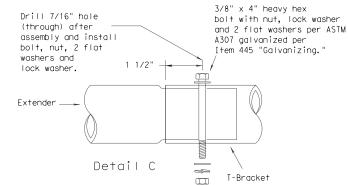
26C







w variable



Splices shall only be allowed behind the sign substrate.

Sign

Clamps

(Specific or

Universal)

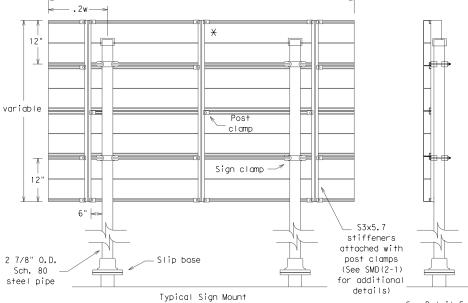
3/8" x 4 1/2'

square head bolt, nut, flat washer and lock washer per ASTM A307 galvanized

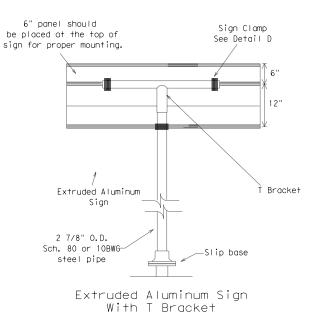
per Item 445.

"Galvanizing.

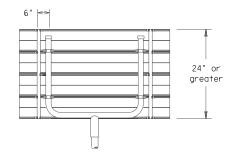
Detail E



SM RD SGN ASSM TY S80(2)XX(P-EXAL) imes Additional stiffener placed at approximate center of signs when sign width is greater than 10'.







Use Extruded Alum. Windbeam as stiffeners See SMD (2-1) for additional details

See Detail E for clamp installation

GENERAL NOTES:

1.	SIGN SUPPORT	# OF POSTS	MAX. SIGN AREA
	10 BWG	1	16 SF
	10 BWG	2	32 SF
	Sch 80	1	32 SF
	Sch 80	2	64 SF

2. The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.

3. Sign supports shall not be spliced except where shown.

Sign support posts shall not be spliced.
4. Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.

5. Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.

6. For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of areater height.

7. When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly" connected to each other except through the sign panel. This will allow each support to act independently when impacted by an errant vehicle.

8. Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.

 Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing.

10. Sign blanks shall be the sizes and shapes shown on

11. Additional sign clamp required on the "T-bracket" post for 24 inch high signs. Place the clamp 3 inches above bottom of sign when possible.

12. Post open ends shall be fitted with Friction Caps.

	REQUIRED SUPPORT					
	SIGN DESCRIPTION	SUPPORT				
	48-inch STOP sign (R1-1)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
آ ح	60-inch YIELD sign (R1-2)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
latory	48x16-inch ONE-WAY sign (R6-1)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
Regul	36x48, 48x36, and 48x48-inch signs	TY 10BWG(1)XX(T)				
	48x60-inch signs	TY S80(1)XX(T)				
	48x48-inch signs (diamond or square)	TY 10BWG(1)XX(T)				
υĝ	48x60-inch signs	TY S80(1)XX(T)				
Warnin	48-inch Advance School X-ing sign (S1-1)	TY 10BWG(1)XX(T)				
	48-inch School X-ing sign (S2-1)	TY 10BWG(1)XX(T)				
	Large Arrow sign (W1-6 & W1-7)	TY 10BWG(1)XX(T)				



SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

SMD(SLIP-3)-08

© TxDOT July 2002	DN: TXD	тот	CK: TXDOT	DW:	TXDOT		CK: TXDOT
9-08 REVISIONS	CONT	SECT	JOB			ніс	SHWAY
	0918	11	100,ETC. CR 110,		O,ETC.		
	DIST		COUNTY			,	SHEET NO.
	DAL		KAUFMA	ΙN			122

SCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any and is made by TxD01 for any purpose whotsoever. TxD01 assumes no responsibility for the conversion od is astandard to other formats or for incorrect results or damages resulting from its use.

REQUIREMENTS FOR INDEPENDENT MOUNTED ROUTE SIGNS

SH	SHEETING REQUIREMENTS				
USAGE	COLOR	SIGN FACE MATERIAL			
BACKGROUND	WHITE	TYPE A SHEETING			
BACKGROUND	ALL OTHERS	TYPE B OR C SHEETING			
LEGEND & BORDERS	WHITE	TYPE A SHEETING			
LEGEND & BORDERS	BLACK	ACRYLIC NON-REFLECTIVE FILM			
LEGEND & BORDERS	ALL OTHERS	TYPE B or C SHEETING			



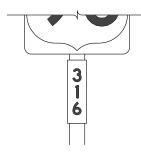




TYPICAL EXAMPLES

REQUIREMENTS FOR BLUE, BROWN & GREEN D AND I SERIES GUIDE SIGNS

SHEETING REQUIREMENTS					
USAGE	COLOR	SIGN FACE MATERIAL			
BACKGROUND	ALL	TYPE B OR C SHEETING			
LEGEND & BORDERS	WHITE	TYPE D SHEETING			
LEGEND, SYMBOLS & BORDERS	ALL OTHERS	TYPE B OR C SHEETING			













TYPICAL EXAMPLES

GENERAL NOTES

- Signs to be furnished shall be as detailed elsewhere in the plans and/or as shown on sign tabulation sheet. Standard sign designs and arrow dimensions can be found in the "Standard Highway Sign Designs for Texas" (SHSD).
- 2. White legend shall use the Clearview Alphabet. The following Clearview fonts shall be used to replace the existing white Federal Highway Administration (FHWA) Standard Highway Alphabets, when not specified in the SHSD, or in the plans.

В	CV-1W
С	CV-2W
D	CV-3W
E	CV-4W
Emod	CV-5WR
F	CV-6W

- 3. Route sign legend (ie. IH, US, SH and FM shields) shall use the Federal Highway Administration (FHWA) Standard Highway Alphabets B, C, D, E, Emod or F).
- 4. Lateral spacing between letters and numerals shall conform with the SHSD, and any approved changes thereto. Lateral spacing of legend shall provide a balanced appearance when spacing is not shown.
- 5. Independent mounted route sign with white or colored legend and borders shall be applied by screening process with transparent color ink, transparent colored overlay film to white background sheeting or cut-out white sheeting to colored background sheeting, or combination thereof. White legend, symbols and borders on all other signs shall be cut-out white sheeting applied to colored background sheeting.
- 6. Information regarding borders and radii for signs is found in the "Standard Highway Sign Designs for Texas". Dimensions shown and described for borders and corner radii on parent sign are nominal. Borders may vary in width as much as 1/2 inch. Corner radii above 3 inches may vary in width as much as 1 inch. Borders and corner radii within a parent sign must be of matching widths. The sign area outside the corner radius should be trimmed or rounded.
- 7. Sign substrate shall be any material that meets the Departmental Material Specification requirements of DMS-7110 or approved alternative.
- 8. Mounting details of roadside signs are shown in the "SMD series" Standard Plan Sheets.

L	DEPARTMENTAL MATERIAL SPEC	IFICATIONS
	ALUMINUM SIGN BLANKS	DMS-7110
	SIGN FACE MATERIALS	DMS-8300

ALUMINUM SIGN BLANKS THICKNESS		
Square Feet	Minimum Thickness	
Less than 7.5	0.080	
7.5 to 15	0.100	
Greater than 15	0.125	

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

http://www.txdot.gov/



TYPICAL SIGN
REQUIREMENTS

Traffic Operations Division Standard

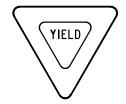
TSR(3)-13

FILE:	tsr3-13.dgn	DN: To	OOT	ck: TxDOT	DW:	TxD0	T CK:	TXDOT
© TxD0T	October 2003	CONT	SECT	JOB			HIGHWAY	
	REVISIONS	0918	11	100,ET	С.	CR	110,E	TC.
12-03 7-13		DIST		COUNTY			SHEET	NO.
9-08		DAL		KAUFMA	٩N		12	3

2/16/2023 4:59:06 N:\P5072-18-19-4\C

REQUIREMENTS FOR RED BACKGROUND REGULATORY SIGNS (STOP, YIELD, DO NOT ENTER AND WRONG WAY SIGNS)









REQUIREMENTS FOR FOUR SPECIFIC SIGNS ONLY

SHEETING REQUIREMENTS				
USAGE	COLOR	SIGN FACE MATERIAL		
BACKGROUND	RED	TYPE B OR C SHEETING		
BACKGROUND	WHITE	TYPE B OR C SHEETING		
LEGEND & BORDERS	WHITE	TYPE B OR C SHEETING		
LEGEND	RED	TYPE B OR C SHEETING		

REQUIREMENTS FOR WARNING SIGNS





TYPICAL EXAMPLES

	SHEETING REQU	IREMENTS
USAGE COLOR		SIGN FACE MATERIAL
BACKGROUND FLOURESCENT YELLOW		TYPE B _{FL} OR C _{FL} SHEETING
LEGEND & BORDERS	BLACK	ACRYLIC NON-REFLECTIVE FILM
LEGEND & SYMBOLS	ALL OTHER	TYPE B OR C SHEETING

REQUIREMENTS FOR WHITE BACKGROUND REGULATORY SIGNS

(EXCLUDING STOP, YIELD, DO NOT ENTER AND WRONG WAY SIGNS)





TYPICAL EXAMPLES

SHEETING REQUIREMENTS				
USAGE	COLOR	SIGN FACE MATERIAL		
BACKGROUND	WHITE	TYPE A SHEETING		
BACKGROUND	ALL OTHERS	TYPE B OR C SHEETING		
LEGEND, BORDERS AND SYMBOLS	BLACK	ACRYLIC NON-REFLECTIVE FILM		
LEGEND, BORDERS AND SYMBOLS	ALL OTHER	TYPE B OR C SHEETING		

REQUIREMENTS FOR SCHOOL SIGNS





TYPICAL EXAMPLES

	SHEETING REQUIREMENTS					
USAGE	COLOR	SIGN FACE MATERIAL				
BACKGROUND	WHITE	TYPE A SHEETING				
BACKGROUND	FLOURESCENT YELLOW GREEN	TYPE B _{FL} OR C _{FL} SHEETING				
LEGEND, BORDERS AND SYMBOLS BLACK		ACRYLIC NON-REFLECTIVE FILM				
SYMBOLS	RED	TYPE B OR C SHEETING				

GENERAL NOTES

- Signs to be furnished shall be as detailed elsewhere in the plans and/or as shown on sign tabulation sheet. Standard sign designs and arrow dimensions can be found in the "Standard Highway Sign Designs for Texas" (SHSD).
- 2. Sign legend shall use the Federal Highway Administration (FHWA) Standard Highway Alphabets (B, C, D, E, Emod or F).
- 3. Lateral spacing between letters and numerals shall conform with the SHSD, and any approved changes thereto. Lateral spacing of legend shall provide a balanced appearance when spacing is not shown.
- Black legend and borders shall be applied by screening process or cut-out acrylic non-reflective black film to background sheeting, or combination thereof.
- 5. White legend and borders shall be applied by screening process with transparent colored ink, transparent colored overlay film to white background sheeting or cut-out white sheeting to colored background sheeting, or combination thereof.
- 6. Colored legend shall be applied by screening process with transparent colored ink, transparent colored overlay film or colored sheeting to background sheeting, or combination thereof.
- 7. Sign substrate shall be any material that meets the Departmental Material Specification requirements of DMS-7110 or approved alternative.
- 8. Mounting details for roadside mounted signs are shown in the "SMD series" Standard Plan Sheets.

ALUMINUM SIGN	BLANKS THICKNESS
Square Feet	Minimum Thickness
Less than 7.5	0.080
7.5 to 15	0.100
Greater than 15	0.125

DEPARTMENTAL MATERIAL SPEC	IFICATIONS
ALUMINUM SIGN BLANKS	DMS-7110
SIGN FACE MATERIALS	DMS-8300

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

http://www.txdot.gov/



Traffic Operations Division Standard

TYPICAL SIGN REQUIREMENTS

TSR(4)-13

.E: †	sr4-13.dg	ın	DN: T>	DOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
)TxDOT C	otober)	2003	CONT	SECT	JOB		н	SHWAY
REVISIONS		0918	11	100, ETC. CR		CR 11	O,ETC.	
1-03 7-13 1-08)		DIST		COUNTY			SHEET NO.
- 00			DAL		KAUFMA	٩N		124

Δ

	9	g	ゔ゙゙゙゙゙゙゙゙゙゙	
	3	Xe.	e Se	
	ટુ	elat	ğ	
	žg	as needed for proportioning and readability but do not relocate from its relative pos	3. All areas should be addressed thoroughly and verify the necessary pay liems are set up	
	<u>ts</u>	2	₹ ^	
	ק	ź,	8	
	g g	ate	Sar	•
	ð	ğ	Ses	
	g	さす	9	
	ž,	<u>ح</u>	##	
•	ğ	o さ	ž	•
	A S	タタ	7 8	
	Ø,	į	Š	
	かり	ğ	Ę	
	0	2	Ş	
	ō,	Š	5	
	8	į	Sed	
	ğ	<i>‡</i>	<i>Tes</i>	ું
•	ยร	ğ	gg	8
	_ 8	ğ	28	8
	Spa	ğ	B	કુ
	B	B	g	ઇ
	8€	8	Sg	Ž
	g	S	g	ğ
	=	_	₹	-,
	Ŋ		M	
_				

I. STORMWATER POLLUTION PR	EVENTION PLAN-CLEAN WAT	ER ACT SECTION 402	III. CULTURAL RESOURCES	١
TPDES TXR 150000: Stormwater required for projects with 1 or me	Discharge Permit or Canstruction Coore acres disturbed soil, Projects	eneral Permit with any	Refer to TxDOT Standard Specifications in the event historical issues or archeological artifacts are found during construction. Upon discovery of archeological artifacts (bones, burnt rock, flint, poltery, etc.) cease	G
Item 506.	osion and sedimentation in accorda	nce with	work in the immediate area and contact the Engineer immediately.	'n
	hat receive discharges from this p	project.	No Action Required	Pi
They need to be notified prior to (Note: Leave blank only if no adi	o construction activities. acent MS 4 Operator(s) are affect	(ed.)	K to total refer to	0
			Action Number:	P
1, Kaufman County Phase II MS4	cantact Kathy Morris, Public Works	Director	i,	c
2.			IV. VEGETATION RESOURCES	P
2.			Preserve notive vegetation to the extent practical.	In
No Action Required	Required Action		Contractor must adhere to Construction Specification Requirements Specs 162, 164, 192, 193, 506, 730, 751 & 752 in order to comply with requirements for invasive species, beneficial landscoping and tree/brush removal commitments.	in in
ACTION NUMBER			☑ No Action Required ☐ Required Action	l
accordance with TPDES Perm	controlling erosion and sedimenta it TXR 150000. vise when necessary to controlpoli		Action Number:	
required by the Engineer.	•		1.	l
the site, accessible to the pu 4. When Contractor project specif	(CSN) with SW3P information on ar blic and TCEQ, EPA ar other inspec fic locations (PSL's) increase distur bmit NOI to TCEQ and the Engineer	tors. bed soil	V. FEDERAL LISTED, PROPOSED THREATENED, ENDANGERED SPECIES, CRITICAL HABITAT, STATE LISTED SPECIES, CANDIDATE SPECIES AND MIGRATORY BIRDS TREATY ACT.	
II WADY IN AD NEAD STOFALE	C WATERDOONIEC AND WET:	NOC CIEAN WATER	_	l
II. WORK IN OR NEAR STREAMS ACT SECTIONS 401 AND		WILL WAILK	☐ No Action Required ☐ Required Action	l
		di ta ann	Action Number:	l
	g, dredging, excavaling or olher wa eams, wellands or wel areas. No ea	•	1. The following species could occur in the project area: Monarch butterfly,	l
allowed in any sream channel be	elow the ardinary High Water Mark	= =	black bear,trocalored bat, eastern tiger salamander, sauthern crawfish frog, Strecker's chorus frog, Woodhouse's tood, eastern spotted skunk,long-tailed	l
approved temporary stream co	rossings or drill pods.		weasel,mountain lion, swamp robbit, eastern box turtle,slender glass lizard,	l
The Contractor must adhere to the following permit(s):	all of the terms and conditions as	socioled with	and western chicken turtle.Follow the special notes and BMPs listed below to protect these species.	
No Permit Required			2. Contractor to implement the fallowing BMPs from "Beneficial Management	1
Nationwide Permit 14 - PCN	not Required (less than 1/10th acr	e waters or	Practices: Avoiding, Minimizing, and Mitigating Impacts of Transportation Projects on State Natural Resources" available at	l
wellands affected)			https://ftp.txdot.gov/pub/txdot-info/env/toolkit/300-01-bmp.pdf	l
☐ Nationwide Permit 14 - PCN	Required (1/10 to <1/2 ocre, 1/3	in lidal waters)	a. Minimize impacts to wetland habitats including isolated ephemeral	1
Individual 404 Permit Require	d		pools. b. Section 2.6.1 Aquatic Amphibian and Reptile BMP (barrier fencing	1
Other Nationwide Permit Req	uired: NWP= 3(a)		nol required)	l
_			c. Section 2.6.2 Terrestrial Amphibian and Reptile BMP d. Section 1.4 Water Quality BMP	l
	the US Permit opplies to, location ctices planned to control erosion, s		e. Section 1.2 Vegetation BMP	
1. Bridge - STA 114 · 20 - Jones Cr	reek - Stream Impacts - NWP 14		Special Notes:	l
	,		Avoid harming all wildlife species if encountered and allow them to safely leave the project site. Due diligence should be used to avoid killing or horming any wildlife species in the implementation of transportation projects.	
			2. If any of the listed species are observed, cease work in the immediate area,	٧
			do not disturb species or habitat and contact the Engineer immediately. The	l
	h water marks of any areas required the US requiring the use of a n	-	work may not remove active nests from bridges and ather structures during nesting season of the birds associated with the nests. If caves or sinkholes	i
permit can be found on the Bridg		J	ore discovered, cease work in the immediated area, and contact the	l
Rest Management Proctices	for applicable 401 General Cond	titions:	Engineer immediately.	l
(Note: If CORP Permit not rec	• •		3. The Migratory Bird Act of 1918 states that it is unlawful to kill, capture, callect, possess, buy, sell, trade or transport any migratory bird, nest,	l
			young, feather or egg in part or in whole, without a federalpermit issued in	l
Erosion	Sedimentation	Post-Construction TSS	accordance within the Act's policies and regulations. The contractor would remove all old migratory bird nests from any structure or trees where work would be done from October 1 to February 15. In addition, the contractor would be prepared	
▼ Temporary Vegetation	Sit Fence	Vegetative Filter Strips	to prevent migratory birds from building nest(s) between February 15 to October 1.	l
Blankets/Matting	Rock Berm	Retention/Irrigation Systems	In the event that migratary birds are encountered on-site during project construction, efforts to avoid adverse impacts on protected birds, active nests, eggs and/or young	l
☐ Mulch	Triongulor Filler Dike	Extended Detention Bosin	would be observed.	l
Sodding	Sond Bog Berm	Constructed Wetlands	LIST OF ABBREVIATIONS	l
Interceptor Swale	Strow Bale Dike	Wet Basin	BMP: Best Management Practice SPCC Spill Prevention Control and Countermeasure	l
Diversion Dike	Brush Berms	Erosion Control Compost	COP: Construction General Permit SWSP: Storm Water Pallution Prevention Plan	l
Erosion ControlCompost	Erosion Control Compost	Mulch Filler Berm and Socks	DS-S: Texos Department of State Health Services PON: Pre-Construction Notification FHMP: Federal Highway Administration PSL: Project Specific Location	l
☐ Mulch Filter Berm and Socks	☐ Wulch Filter Berm and Socks	Compost Filler Berm and Socks	MOA: Memor and um of Agreement TOEO: Texas Commission on Environmental Quality MOU: Memor and um of Understanding TPDES: Texas Pall ut ant Discharge Elimination System	l
Compost Filter Berm and Socks	Compost Filter Berm and Socks	Vegetation Lined Ditches	MS4: Municipal Separate Starmater Sever System TPVD: Texas Parks and Wildlife Department	l
	Slone Outlet Sediment Traps	Sand Filter Systems	MBTA: Migratory Bird Treaty Act Tx00T: Texas Department of Transportation NOT: Notice of Termination T&E: Threatened and Endangered Species	l
	Sediment Bosins	Grassy Swales	NVP: Notionwide Permit USACE: U.S. Army Corp of Engineers NO: Notice of Intent USFWS: U.S. Fish and Wildlife Service	l
				_

VI, HAZARDOUS MATERIALS OR CONTAMINATION ISSUES

Seneral (applies to all projects):

Comply with the Hazard Communication Act (the Act) for persannel who will be working with nazardous materials by conducting safety meetings prior to beginning construction and nakina warkers aware of polenlial hazards in the workplace. Ensure that all workers are rovided with personal protective equipment appropriate for any hazardous materials used.

Obtain and keep an-site Safety Data Sheets (SDS) for all hazardous products sed on the project, which may include, but are not limited to the following categories: aints, acids, solvents, asphalt products, chemical additives, fuels and concrete curing compounds or additives. Provide protected storage, off bare ground and covered, for roducts which may be hazardous. Maintain product labelling as required by the Act. laintain an adequate supply of on-site spill response materials, as indicated in the SDS. the event of a spill, take actions to mitigate the spill as indicated in the SDS, accordance with safe work practices, and contact the District Spill Coordinator nmediately. The Contractor shall be responsible for the proper containment and cleanup

Contact the Engineer if any of the following are detected:

- Dead or distressed vegetation (not identified as normal)
- Trash piles, drums, canisters, barrels, etc.
- Undesirable smells or odors
- Evidence of leaching or seepage of substances

Does the project involve any bridge class structure rehabilitation(s) or replacement(s) (bridge class structures not including box culverts)?

X Yes □ No

If "No", then no further action is required.

If "Yes", then TxDOT is responsible for completing asbestos assessment/inspection.

Are the results of the asbestos inspection positive (is asbestos present)?

X No ☐ Yes

If "Yes", then TxDOT must retain a DSHS licensed asbestos consultant to assist with the natification, develop abatement/mitigation procedures, and perform management activities as necessary. The notification form to DSHS must be postmarked at least 15 working days prior to scheduled demalition.

If "No", then TxDOT is still required to notify DSHS 15 working days prior to any scheduled demolition.

In either case, the Contractor is responsible for providing the date(s) for abotement activities and/or demolition with careful coordination between the Engineer and asbestos consultant in order to minimize construction delays and subsequent claims

Any other evidence indicating possible hazardous materials or contamination discovered on site. Hazardous Materials or Contamination Issues Specific to this Project:

☐ No Action Required

X Required Action

Action Number:

1.CR 110 over Jones Creek (NBI 181300AA0347001) at STA 1114-21 silver/block LBP (700ppm) on steel beams. LBP abotement on steel beams required only at cut locations prior to demolition activities.

VII. OTHER ENVIRONMENTAL ISSUES

Any change orders and/or deviations from the final design must be reported to the

Engineer prior to commencement of construction activities, as additional environmental clearance may be required.

(includes regionalissues such as Edwards Aquifer District, etc.)

X No Action Required

Required Action

Action Number

GENERAL NOTE:

CR 110 at Jones Creek

©²⁰²² Texas Department of Transportation

Dallas District

ENVIRONMENTAL PERMITS.

ISSUES AND COMMITMENTS (EPIC)

FED.RD. DIV.NO.	FE	HIGHWAY NO.	
6	SEE	TITLE SHEET	CR 110, etc.
STATE	DISTRICT	COUNTY	011 110, 610.
TEXAS	DALLAS	Kaufman	SHEET
CONTROL	SECTION	JOB	NO.
0918	11	100, etc.	125

LAST REVISION:1/15/15

' '	1. Do not after Sheet Design or Font style, size or weight - match text attributes.	
<i>ا</i> ن	2. If additional space is needed for a numbered section, fence and adjust sections up or down	DISCLAI
	as needed for proportioning and readability but do not relocate from its relative position.	The use
m,	All areas should be addressed thoroughly and verify the necessary pay items are set up to	No warr
	support actions needed.	TxD07
=======================================	Filled Out: XX/XX/XXXX	formats

VI. HAZARDOUS MATERIALS OR CONTAMINATION ISSUES STORMWATER POLLUTION PREVENTION PLAN-CLEAN WATER ACT SECTION 402 III. CULTURAL RESOURCES Refer to TxDOT Standard Specifications in the event historical issues or General (applies to all projects): TPDES TXR 150000: Stormwater Discharge Permit or Construction General Permit Comply with the Hazard Communication Act (the Act) for personnel who will be working with archeological artifacts are found during construction. Upon discovery of required for projects with 1 or more acres disturbed soil. Projects with any archeological artifacts (bones, burnt rock, flint, pottery, etc.) cease hazardous materials by conducting safety meetings prior to beginning construction and disturbed soil must protect for erosion and sedimentation in accordance with work in the immediate area and contact the Engineer immediately. making workers aware of potential hazards in the workplace. Ensure that all workers are List adjacent MS 4 Operator(s) that receive discharges from this project. provided with personal protective equipment appropriate for any hazardous materials used. X No Action Required Required Action They need to be notified prior to construction activities. Obtain and keep on-site Safety Data Sheets (SDS) for all hazardous products (Note: Leave blank only if no adjacent MS 4 Operator(s) are affected.) used on the project, which may include, but are not limited to the following categories: Action Number: Paints, acids, solvents, asphalt products, chemical additives, fuels and concrete curing 1. Kaufman County Phase II MS4 contact Kathy Morris, Public Works Director compounds or additives. Provide protected storage, off bare ground and covered, for products which may be hazardous. Maintain product labelling as required by the Act. IV. VEGETATION RESOURCES Maintain an adequate supply of on-site spill response materials, as indicated in the SDS. In the event of a spill, take actions to mitigate the spill as indicated in the SDS, Preserve native vegetation to the extent practical. in accordance with safe work practices, and contact the District Spill Coordinator X Required Action Contractor must adhere to Construction Specification Requirements Specs 162, ■ No Action Required immediately. The Contractor shall be responsible for the proper containment and cleanup 164, 192, 193, 506, 730, 751 & 752 in order to comply with requirements for of all product spills. invasive species, beneficial landscaping and tree/brush removal commitments. Action Number: Contact the Engineer if any of the following are detected: X No Action Required Required Action 1. Prevent stormwater pollution by controlling erosion and sedimentation in Dead or distressed vegetation (not identified as normal) Trash piles, drums, canisters, barrels, etc. accordance with TPDES Permit TXR 150000. Action Number: Undesirable smells or odors 2. Comply with the SW3P and revise when necessary to control pollution or Evidence of leaching or seepage of substances required by the Engineer. 3. Post Construction Site Notice (CSN) with SW3P information on or near Does the project involve any bridge class structure rehabilitation(s) or the site, accessible to the public and TCEQ, EPA or other inspectors. V. FEDERAL LISTED, PROPOSED THREATENED, ENDANGERED SPECIES, replacement(s) (bridge class structures not including box culverts)? 4. When Contractor project specific locations (PSL's) increase disturbed soil CRITICAL HABITAT, STATE LISTED SPECIES, CANDIDATE SPECIES area to 5 acres or more, submit NOI to TCEQ and the Engineer. AND MIGRATORY BIRDS TREATY ACT. If "No", then no further action is required. II. WORK IN OR NEAR STREAMS, WATERBODIES AND WETLANDS CLEAN WATER If "Yes", then TxDOT is responsible for completing asbestos assessment/inspection. ☐ No Action Required X Required Action ACT SECTIONS 401 AND 404 Are the results of the asbestos inspection positive (is asbestos present)? USACE Permit required for filling, dredging, excavating or other work in any 1. The following species could occur in the project grea: Mongrob butterfly water bodies, rivers, creeks, streams, wetlands or wet areas. No equipment is triccolored bat, eastern tiger salamander, southern crawfish frog, Strecker's If "Yes", then TxDOT must retain a DSHS licensed asbestos consultant to assist with allowed in any sream channel below the ordinary High Water Mark except on chorus frog, Woodhouse's toad, eastern spotted skunk, long-tailed weasel, the notification, develop abatement/mitigation procedures, and perform management approved temporary stream crossings or drill pads. swamp rabbit, eastern box turtle, slender glass lizard, western box turtle, activities as necessary. The notification form to DSHS must be postmarked at least and western chicken turtle. Follow the special notesand BMPs listed below 15 working days prior to scheduled demolition. The Contractor must adhere to all of the terms and conditions associated with to protect these species. the following permit(s): If "No", then TxDOT is still required to notify DSHS 15 working days prior to any scheduled demolition. 2. Contractor to implement the following BMPs from "Beneficial Management ☐ No Permit Required Practices: Avoiding, Minimizing, and Mitigating Impacts of Transportation In either case, the Contractor is responsible for providing the date(s) for abatement Nationwide Permit 14 - PCN not Required (less than 1/10th acre waters or) Projects on State Natural Resources" available at activities and/or demolition with careful coordination between the Engineer and https://ftp.txdot.gov/pub/txdot-info/env/toolkit/300-01-bmp.pdf asbestos consultant in order to minimize construction delays and subsequent claims. a. Minimize impacts to wetland habitats including isolated ephemeral Nationwide Permit 14 - PCN Required (1/10 to <1/2 acre, 1/3 in tidal waters) Any other evidence indicating possible hazardous materials or contamination discovered ☐ Individual 404 Permit Required on site. Hazardous Materials or Contamination Issues Specific to this Project: b. Section 2.6.1 Aquatic Amphibian and Reptile BMP (barrier fencing Other Nationwide Permit Required: NWP# 3(a) X No Action Required Required Action c. Section 2.6.2 Terrestrial Amphibian and Reptile BMP d. Section 1.4 Water Quality BMP Action Number: Required Actions: List Waters of the US Permit applies to, location in project e. Section 1.2 Vegetation BMP and check Best Management Practices planned to control erosion, sedimentation and post-project TSS. 1. Bridge - STA 2282+20 - Bachelor Creek Relief - Stream Impacts - NWP 14 Special Notes: 1. Avoid harming all wildlife species if encountered and allow them to safely leave the project site. Due diligence should be used to avoid killing or harming any wildlife species in the implementation of transportation projects. VII. OTHER ENVIRONMENTAL ISSUES 2. If any of the listed species are observed, cease work in the immediate area, do not disturb species or habitat and contact the Engineer immediately. The (includes regional issues such as Edwards Aquifer District, etc.) The elevation of the ordinary high water marks of any areas requiring work work may not remove active nests from bridges and other structures during Required Action X No Action Required to be performed in the waters of the US requiring the use of a nationwide nesting season of the birds associated with the nests. If caves or sinkholes permit can be found on the Bridge Layouts. are discovered, cease work in the immediated area, and contact the Action Number: Engineer immediately. Best Management Practices for applicable 401 General Conditions: 3. The Migratory Bird Act of 1918 states that it is unlawful to kill, (Note: If CORP Permit not required, do not check boxes.) capture, collect, possess, buy, sell, trade or transport any migratory bird, nest, young, feather or egg in part or in whole, without a federal permit issued in accordance within the Act's policies and regulations. The contractor would Sedimentation Post-Construction TSS Erosion remove all old migratory bird nests from any structure or trees where work would be CR 279 at Bachelor Creek Relief done from October 1 to February 15. In addition, the contractor would be prepared X Silt Fence ☐ Vegetative Filter Strips to prevent migratory birds from building nest(s) between February 15 to October 1. ▼ Temporary Vegetation © 2022 Texas Department of Transportation In the event that migratory birds are encountered on-site during project construction, Rock Berm ☐ Blankets/Matting Retention/Irrigation Systems efforts to avoid adverse impacts on protected birds, active nests, eggs and/or young Mulch ☐ Triangular Filter Dike Extended Detention Basin would be observed. Sand Bag Berm Constructed Wetlands Sodding GENERAL NOTE: LIST OF ABBREVIATIONS ☐ Interceptor Swale Straw Bale Dike ₩et Basin Any change orders and/or deviations from Best Management Practice SPCC: Spill Prevention Control and Countermeasure the final design must be reported to the ☐ Diversion Dike ☐ Brush Berms Erosion Control Compost Construction General Permit SW3P: Storm Water Pollution Prevention Plan DSHS: Texas Department of State Health Services Pre-Construction Notification Engineer prior to commencement of Erosion Control Compost Erosion Control Compost Mulch Filter Berm and Socks FHWA: Federal Highway Administration Project Specific Location construction activities, as additional Texas Commission on Environmental Quality MOA: Memorandum of Agreement Mulch Filter Berm and Socks ☐ Mulch Filter Berm and Socks ☐ Compost Filter Berm and Socks environmental clearance may be required. 6 Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System TPWD: Texas Parks and Wildlife Department Municipal Separate Starmwater Sewer System STATE ☐ Compost Filter Berm and Socks ☐ Compost Filter Berm and Socks ☒ Vegetation Lined Ditches MBTA: Migratory Bird Treaty Act TxDOT: Texas Department of Transportation Stone Outlet Sediment Traps Sand Filter Systems NOT: Notice of Termination T&E: Threatened and Endangered Species USACE: U.S. Army Corp of Engineers
USFWS: U.S. Fish and Wildlife Service Notionwide Permit Sediment Basins

Grassy Swales

NOI: Notice of Intent

LAST REVISION: 1/15/15

ENVIRONMENTAL PERMITS. ISSUES AND COMMITMENTS (EPIC)

Dallas District

FEDERAL AID PROJECT NO. SEE TITLE SHEET CR 110, etc TEXAS DALLAS Kaufman SHEET CONTROL SECTION 0918 100, etc. 11

00	Nores 10 Designer:
'	Do not alter Sheet Design or Font style, si.
2	If additional space is needed for a number
	as needed for proportioning and readabili
3.	All areas should be addressed thoroughly a

I. STORMWATER POLLUTION P	REVENTION PLAN-CLEAN	WATER ACT SECTION 402	III.	CULTURAL RESOURCES			VI. HAZARDOUS MATERIALS OR CONTAMI	NATION ISSUES
TPDES TXR 150000: Stormwater required for projects with disturbed soil must protect Item 506. List adjacent MS 4 Operator They need to be notified pr	or more acres disturbed s for erosion and sedimental (s) that receive discharges	soil. Projects with any tion in accordance with s from this project.		Refer to TxDOT Standard Specification archeological artifacts are found archeological artifacts (bones, but work in the immediate area and continuous Required	luring construction. Upon disc int rock, flint, pottery, etc.	covery of .) cease	hazardous materials by conducting safety me making workers aware of potential hazards i	n the workplace. Ensure that all workers are appropriate for any hazardous materials used.
(Note: Leave blank only if				Action Number:			used on the project, which may include, but	are not limited to the following categories: chemical additives, fuels and concrete curing storage, off bare ground and covered, for
2. No Action Requirements Action Number:	red 🛛 Required Act	ion	IV.	VEGETATION RESOURCES Preserve native vegetation to the Contractor must adhere to Construint 164, 192, 193, 506, 730, 751 & 75 invasive species, beneficial land	ction Specification Requireme ? in order to comply with req	uirements for	Maintain an adequate supply of on-site spil In the event of a spill, take actions to mi in accordance with safe work practices, and	I response materials, as indicated in the SDS. tigate the spill as indicated in the SDS.
1. Prevent stormwater polludecordance with TPDES Per 2. Comply with the SWSP and required by the Engineer. 3. Post Construction Site Not the site, accessible to 4. When Contractor projects.	mit TXR 150000. revise when necessary to c ptice (CSN) with SW3P infor the public and TCEQ, EPA or	control pollution or rmation on or near r other inspectors, increase disturbed soil		X No Action Required Action Number: 1. FEDERAL LISTED, PROPOSED THR CRITICAL HABITAT, STATE LIST AND MIGRATORY BIRDS TREATY A	Required Action EATENED, ENDANGERED SPECED SPECIES, CANDIDATE SI	CIES,	Contact the Engineer if any of the follow * Dead or distressed vegetation (not i * Trash piles, drums, canisters, barre * Undesirable smells or odors * Evidence of leaching or seepage of s Does the project involve any bridge class replacement(s) (bridge class structures not to the project involve and the project	dentified as normal) els, etc. substances structure rehabilitation(s) or of including box culverts)?
II. WORK IN OR NEAR STREA ACT SECTIONS 401 AND USACE Permit required for water bodies, rivers, cree allowed in any sream chann approved temporary stream	404 filling, dredging, excavat ks, streams, wetlands or w el below the ordinary High	ing or other work in any et areas. No equipment is		No Action Required Action Number: 1. The following species could occ triccolored bat, eastern tiger sal chorus frog, Woodhouse's toad, eas swamp rabbit, eastern box turtle,s	omander, southern crawfish fr tern spotted skunk,long-taile	og,Strecker's d weasel,	the notification, develop abatement/mitiga	ompleting asbestos assessment/inspection. In positive (is asbestos present)? I icensed asbestos consultant to assist with ation procedures, and perform management
The Contractor must adhere the following permit(s): No Permit Required Nationwide Permit 14 - 1 wetlands affected)				Follow the special notesand BMPs I 2. Contractor to implement the fol Practices: Avoiding, Minimizing, a Projects on State Natural Resource https://ftp.txdot.gov/pub/txdot-in	isted below to protect these lowing BMPs from "Beneficial nd Mitigating Impacts of Tran s" available at	species. Management sportation	activities as necessary. The notification 15 working days prior to scheduled demolit If "No", then TxDOT is still required to scheduled demolition. In either case, the Contractor is responsi activities and/or demolition with careful	notify DSHS 15 working days prior to any ble for providing the date(s) for abatement
Nationwide Permit 14 - I Individual 404 Permit R Other Nationwide Permit	equired	acre, 1/3 in tidal waters)		 a. Minimize impacts to wetlan pools. b. Section 2.6.1 Aquatic Amphotorrequired) c. Section 2.6.2 Terrestrial d. Section 1.4 Water Quality 	ibian and Reptile BMP (barrie Amphibian and Reptile BMP		asbestos consultant in order to minimize of Any other evidence indicating possible haz on site. Hazardous Materials or Contamina No Action Required	ardous materials or contamination discovered
Required Actions: List Water and check Best Management Franch on the post-project TSS.	ractices planned to contro	l erosion, sedimentation		e. Section 1.2 Vegetation BMP			Action Number:	
1. Bridge - STA 315+20 - Mud The elevation of the ordina	ry high water marks of any	areas requiring work	1. Lec har 2. do wor	Avoid harming all wildlife species ave the project site. Due diligence rming any wildlife species in the im If any of the listed species are obnot disturb species or habitat and rk may not remove active nests from	should be used to avoid killi plementation of transportatio served, cease work in the imm contact the Engineer immediat pridges and other structures	ng or n projects. mediate area, ely. The during	2. VII. OTHER ENVIRONMENTAL ISSUES (includes regional issues such as Edv X No Action Required	vards Aquifer District, etc.)
to be performed in the wate permit can be found on the Best Management Practic (Note: If CORP Permit no	es for applicable 401 (General Conditions:	are Eng	sting season of the birds associated e discovered, cease work in the imme gineer immediately. The Migratory Bird Act of 1918 states to bture, collect, possess, buy, sell, trade	diated area, and contact the nat it is unlawful to kill,		Action Number:	
	Sedimentation	Post-Construction TSS	you acc rem	ung, feather or egg in part or in whole, cordance within the Act's policies and re nove all old migratory bird nests from a	without a federal permit issued egulations. The contractor would by structure or trees where work	in would be		CR 312 at Muddy Cedar Creek Tributary
∑ Temporary Vegetation ☐ Blankets/Matting ☐ Mulch	X Silt Fence ☐ Rock Berm ☐ Triangular Filter Dike	☐ Vegetative Filter Strips ☐ Retention/Irrigation Systems ☐ Extended Detention Basin	to In eff	ne from October 1 to February 15. In add prevent migratory birds from building no the event that migratory birds are enco forts to avoid adverse impacts on protec- uld be observed.	est(s) between February 15 to Oc untered on-site during project co	tober 1. onstruction,		© 2022 Texas Department of Transportation Dallas District
Sodding Interceptor Swale Diversion Dike Erosion Control Compost Mulch Filter Berm and Socks Compost Filter Berm and Socks	_	Constructed Wetlands Wet Basin Erosion Control Compost Mulch Filter Berm and Socks Compost Filter Berm and Socks X Yegetation Lined Ditches	CGP: DSHS: FHWA: MOA: MOU: MS4:	Best Management Practice Construction General Permit Texas Department of State Health Services Federal Highway Administration Memorandum of Agreement Memorandum of Understanding Municipal Separate Stormwater Sewer System Migratory Bird Treaty Act	SPCC: Spill Preventian Control and SWDP: Storm Water Pollution Prever PON: Pre-Construction Natification PSL: Project Specific Location TCCQ: Texas Commission on Environm TPDES: Texas Pollutant Discharge E	ntion Plan on mental Quality Limination System continent	GENERAL NOTE: Any change orders and/or deviations from the final design must be reported to the Engineer prior to commencement of construction activities, as additional environmental clearance may be required.	ENVIRONMENTAL PERMITS, ISSUES AND COMMITMENTS (EPIC) FED. RD. DIV. NO. FEDERAL AID PROJECT NO. FEDERA
	Stone Outlet Sediment Traps Sediment Basins	☐ Sand Filter Systems ☐ Grassy Swales	NOT: NWP:	Notice of Termination	TRE: Threatened and Endangered Sp USACE: U.S. Army Corp of Engineers USFWS: U.S. Fish and Wildlife Servi	pecies		TEXAS DALLAS Kaufman CONTROL SECTION JOB NO. 11 100 etc 127

LAST REVISION: 1/15/15

0918

11

127

100, etc.

This SWP3 has been developed in accordance with TxDOT policy for projects disturbing less than 1 acre of soil, and not part of a larger common plan of development.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept at the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans, and the project's environmental permits, issues, and commitments (EPICs).

1.0 SITE/PROJECT DESCRIPTION

1.1 PROJECT CONTROL SECTION JOB (CSJ):

0918-11-100, etc.

1.2 PROJECT LIMITS:

From: CR 110 at Jones Creek

To:_

1.3 PROJECT COORDINATES:

BEGIN: (Lat) 32.538756 ,(Long) -96.159448

1.4 TOTAL PROJECT AREA (Acres): 0.54

END: (Lat) 32.539058 ,(Long)

1.5 TOTAL AREA TO BE DISTURBED (Acres): 0.51

-96.158260

1.6 NATURE OF CONSTRUCTION ACTIVITY:

Construction of bridge replacement consisting of replacing bridge and approaches.

1.7 MAJOR SOIL TYPES:

Soil Type	Description				
Gowen clay loam, 0 to 1% Slopes	100% Gowen clay, well drained, negligible rate of runoff				
	Poor grass throughout. 77% vegetative density				

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below:

PSLs determined during preconstruction meeting

DOL and a township and about a process from the second structure.

□ PSLs determined during construction

☐ No PSLs planned for construction

Туре	Sheet #s)
)
)
)
)
		-
		L
		-

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.3.)

- ▼ Mobilization
- X Install sediment and erosion controls
- □ Blade existing topsoil into windrows, prep ROW, clear and grub
- Remove existing pavement
- x Grading operations, excavation, and embankment
- X Excavate and prepare subgrade for proposed pavement widening
- ☐ Remove existing culverts, safety end treatments (SETs)
- ☐ Remove existing metal beam guard fence (MBGF), bridge rail
- X Install proposed pavement per plans
- ☐ Install culverts, culvert extensions, SETs
- Install mow strip, MBGF, bridge rail
- X Place flex base

Other:

- ▼ Blade windrowed material back across slopes
- ▼ Revegetation of unpaved areas
- Achieve site stabilization and remove sediment and erosion control measures

Other:			
•			

Other:			

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- ▼ Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment, and storage
- Solvents, paints, adhesives, etc. from various construction activities
- ▼ Transported soils from offsite vehicle tracking
- ☑ Construction debris and waste from various construction activities
- X Contaminated water from excavation or dewatering pump-out water
- Sanitary waste from onsite restroom facilities
- Trash from various construction activities/receptacles

Other:

☐ Long-term stockpiles of material and waste

X Other. _	Concrete pouring and washout
☐ Other: _	

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

Tributaries	Classified Waterbody			
Jones Creek	Flows into Cedar Creek (0818B*)			
	then Cedar Creek			
	Reservoir(0818*)			
* Add (*) for impaired waterbodies with pollutant in ().				

* Add (*) for impaired waterbodies with pollutant in ()
*Impaired by Bacteria in water (Recreation use)

control measures

1.12 ROLES AND RESPONSIBILITIES: TxDOT

X Development of plans and specifications

X Perform SWP3 inspections

X Maintain SWP3 records and update to reflect daily operations

Other:			

1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

☐ Other:	-		
□ Other:			



CR 110 at Jones Creek
STORMWATER POLLUTION
PREVENTION PLAN (SWP3)
(Less Than 1 Acre)



Sheet 1 of 2

FED. RD. DIV. NO.		SHEET NO.			
6		SEE TITLE SHEET			
STATE	STATE STATE DIST.		COUNTY		
TEXAS DAL		KAUFMAN			
CONT.		SECT.	JOB	HIGHWAY NO.	
0918	3	1 1	100, ETC. CR 110, E		ETC.

2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND **MAINTENANCE**

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

2.1 EROSION CONTROL AND SOIL

STABILIZATION BMPs:
T/P
□ □ Protection of Existing Vegetation
□ □ Vegetated Buffer Zones
□ □ Soil Retention Blankets
□ □ Geotextiles
□ □ Mulching/ Hydromulching
□ □ Soil Surface Treatments
X □ Temporary Seeding
□ X Permanent Planting, Sodding or Seeding
⊠ □ Biodegradable Erosion Control Logs
X □ Rock Filter Dams/ Rock Check Dams
X □ Vertical Tracking
☐ ☐ Interceptor Swale
│ □ X Riprap │ □ □ Diversion Dike
□ □ Temporary Pipe Slope Drain
□ □ Embankment for Erosion Control
□ □ Paved Flumes
X X Other: Preservation of natural resources
☐ X Other: Compost manufactured topsoil
☐ X Other: Vegetation lined ditches
□ □ Other:
2.2 SEDIMENT CONTROL BMPs:
T/P
X □ Biodegradable Erosion Control Logs
□ □ Inlet Protection
X Rock Filter Dams/ Rock Check Dams
□ □ Sandbag Berms
X □ Sediment Control Fence
X □ Stabilized Construction Exit
□ □ Floating Turbidity Barrier
□ □ Vegetated Buffer Zones
□ □ Vegetated Filter Strips
X □ Other: Rock bedding at construction exits
□ □ Other:
□ □ Other:
□ □ Other:
Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets

located in Attachment 1.2 of this SWP3

2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Туре	Stationing			
	From	То		
No permanent controls are planned.				
Defende the Environmental Leve		\		

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.4 OFFSITE VEHICLE TRACKING CONTROLS:

X Excess dirt/mud on road removed daily

☐ Haul roads dampened for dust control

X Loaded haul trucks to be covered with tarpaulin

X Stabilized construction exit

X Other: Dampen disturbed soil areas as needed for dust control.

Other:	
Other:	
·-	
Other:	

2.5 POLLUTION PREVENTION MEASURES:

- X Chemical Management
- X Concrete and Materials Waste Management
- X Debris and Trash Management
- X Dust Control
- X Sanitary Facilities
- X Other: Avoid storing portable sanitary units, concrete washouts or chemicals within 50-ft upgradient of a receiving water without adequate pollution controls.

X Other: Capture saw cutting debris and concrete slurry for proper disposal.

V	Other:	
	Ouiei.	

2.6 VEGETATED BUFFER ZONES:

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Type	Stationing		
J.	From	То	
Jones Creek: Vegetative Buffer Not feasible due to type of work which includes excavation at bridge near creek to remove exisiting bridge, build new bridge and installing of stone riprap.			
Sediment fence, rock filter dams,	1112+20	1115+87	

& erosion control logs.	1112+20	1115+87

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.



CR 110 at Jones Creek

STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 2 of 2

FED. RD. DIV. NO.		PROJECT NO.				
6		SEE	TITLE SHE	EET	129	
STATE		STATE DIST.	C	OUNTY		
TEXAS		DAL	KAUFMAN			
CONT.		SECT.	JOB	HIGHWAY NO.		
0918	3	1 1	100,ETC.	CR 110,	ETC.	

This SWP3 has been developed in accordance with TxDOT policy for projects disturbing less than 1 acre of soil, and not part of a larger common plan of development.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept at the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans, and the project's environmental permits, issues, and commitments (EPICs).

1.0 SITE/PROJECT DESCRIPTION

1.1 PROJECT CONTROL SECTION JOB (CSJ):

0918-11-100, etc.

1.2 PROJECT LIMITS:

From: CR 279 at Bachelor Creek Relief

1.3 PROJECT COORDINATES:

BEGIN: (Lat) 32.656816 ,(Long) -96.280390

END: (Lat) 32.657647 ,(Long) -96.279275

1.4 TOTAL PROJECT AREA (Acres): 0.52

1.5 TOTAL AREA TO BE DISTURBED (Acres): ____0.49

1.6 NATURE OF CONSTRUCTION ACTIVITY:

Construction of bridge replacement consisting of replacing bridge and approaches.

1.7 MAJOR SOIL TYPES:

Soil Type	Description		
Axtell fine sandy loam 2 to 5% slopes 8.5% of area	90% axtell sandy loam, moderately well drained. high runoff		
Gowen clay loam 0 to 1% slopes 91.5% of area	100% Gowen clay, well drained, negligible rate of runoff		
	Average grass,Pasture throughout. 73% vegetative density		

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below: ☑ PSLs determined during preconstruction meeting

- □ PSLs determined during construction
- ☐ No PSLs planned for construction

Туре	Sheet #s	activities
		 X Contaminated water from excavation or devented water ✓ Sanitary waste from onsite restroom facilities ✓ Trash from various construction activities/re □ Long-term stockpiles of material and waste ✓ Other: Concrete pouring and washout
		☐ Other: ☐ Other:

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.3.)

- X Mobilization
- Install sediment and erosion controls
- Blade existing topsoil into windrows, prep ROW, clear and grub
- Remove existing pavement
- X Grading operations, excavation, and embankment
- X Excavate and prepare subgrade for proposed pavement widenina
- ☐ Remove existing culverts, safety end treatments (SETs)
- □ Remove existing metal beam guard fence (MBGF), bridge rail
- X Install proposed pavement per plans
- □ Install culverts, culvert extensions, SETs
- X Install mow strip, MBGF, bridge rail
- X Place flex base

□ Other

- ▼ Blade windrowed material back across slopes
- ▼ Revegetation of unpaved areas
- X Achieve site stabilization and remove sediment and erosion control measures

Other:			

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment,
- Solvents, paints, adhesives, etc. from various construction
- ▼ Transported soils from offsite vehicle tracking
- ☑ Construction debris and waste from various construction activities
- X Contaminated water from excavation or dewatering pump-out
- ☒ Sanitary waste from onsite restroom facilities
- ▼ Trash from various construction activities/receptacles
- □ Long-term stockpiles of material and waste

X Other:	Concrete pouring and washout
Other:	

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

Tributaries	Classified Waterbody
Bachelor Creek Relief	Flows into Kings Creek (0818C*) then Cedar Creek Reservoir (0818*)
* Add (*) for impaired waterhodie	s with pollutant in ()

- Add (*) for impaired waterbodies with pollutant in ().
- *Impaired by Bacteria in water (Recreation use)

1.12 ROLES AND RESPONSIBILITIES: TxDOT

X Development of plans and specifications

X Perform SWP3 inspections

X Maintain SWP3 records and update to reflect daily operations

Other:			

1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

□ Other:	-					
□ Other:						



CR 279 at Bachelor Creek Relief STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 1 of 2

FED. RD. DIV. NO.	PROJECT NO.				SHEET NO.
6		SEE	TITLE SHE	EET	130
STATE		STATE DIST.	C	OUNTY	
TEXAS		DAL	KAUFMAN		
CONT.		SECT.	JOB	HIGHWAY N	١0.
0918		11	100,ETC.	CR 110,	ETC.

2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND MAINTENANCE

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

2.1 EROSION CONTROL AND SOIL STABILIZATION BMPs:

31	ADILIZATION DIVIPS.
T/P	
□ □ Pi	rotection of Existing Vegetation
	egetated Buffer Zones
□ □ S	oil Retention Blankets
□ □ G	eotextiles
\square \square M	ulching/ Hydromulching
	oil Surface Treatments
	emporary Seeding
	ermanent Planting, Sodding or Seeding
	iodegradable Erosion Control Logs
X R	ock Filter Dams/ Rock Check Dams
X U	ertical Tracking
	terceptor Swale
□ X R	iprap iversion Dike
	emporary Pipe Slope Drain
	mbankment for Erosion Control
	aved Flumes
	ther: Preservation of natural resources
	ther: Compost manufactured topsoil
	ther: Vegetation lined ditches
	ther:
2.2 SE	DIMENT CONTROL BMPs:
T/P	
	odegradable Erosion Control Logs
	ewatering Controls
	let Protection
X 🗆 Ro	ock Filter Dams/ Rock Check Dams
	andbag Berms
X Se	ediment Control Fence
X St	abilized Construction Exit
	oating Turbidity Barrier
□ □ Ve	egetated Buffer Zones
□ □ Ve	egetated Filter Strips
X - O1	ther: Rock bedding at construction exits
	ther:
	ther:
□ □ Ot	ther:
Refer to	the Environmental Layout Sheets/ SWP3 Layout She

located in Attachment 1.2 of this SWP3

2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Typo	Stationing		
Туре	From	То	
No permanent controls are planned.			
Refer to the Environmental Layo located in Attachment 1.2 of this		3 Layout Sheets	

2.4 OFFSITE VEHICLE TRACKING CONTROLS:

★ Excess dirt/mud on road removed daily

☐ Haul roads dampened for dust control

X Loaded haul trucks to be covered with tarpaulin

X Other: Dampen disturbed soil areas as needed for dust control.

Other:	
Other:	
Other:	
-	-

2.5 POLLUTION PREVENTION MEASURES:

- X Chemical Management
- X Concrete and Materials Waste Management
- X Debris and Trash Management
- X Dust Control
- X Sanitary Facilities
- X Other: Avoid storing portable sanitary units, concrete washouts or chemicals within 50-ft upgradient of a receiving water without adequate pollution controls.

X Other:	Capture saw cutting debris and concrete slurry for	
proper o	lisposal.	

X	Other:

Other:	

2.6 VEGETATED BUFFER ZONES:

Type

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Stationing

To

From

2279+97	2284+55
	2279+97

Bachelor Creek Relief: Vegetative Buffer Not feasible due to type

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.



CR 279 at Bachelor Creek Relief

STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 2 of 2

FED. RD. DIV. NO.		PROJECT NO. SHEET NO.			
6		SEE TITLE SHEET 1			131
STATE		STATE DIST.	C	OUNTY	
TEXAS	3	DAL	KAUFMAN		
CONT.		SECT.	JOB	HIGHWAY N	٧٥.
0918	3	11	100,ETC. CR 110,ET		ETC.

This SWP3 has been developed in accordance with TxDOT policy for projects disturbing less than 1 acre of soil, and not part of a larger common plan of development.

For all projects with any soil disturbing activities, TxDOT will maintain a SWP3 with all pertinent records, correspondence, environmental documents, etc. at the project field office. If no field office is available, then this SWP3 shall be kept at the appropriate TxDOT Area Office.

This SWP3 is consistent with requirements specified in applicable stormwater plans, and the project's environmental permits, issues, and commitments (EPICs).

1.0 SITE/PROJECT DESCRIPTION

1.1 PROJECT CONTROL SECTION JOB (CSJ):

0918-11-100, etc.

1.2 PROJECT LIMITS:

From: CR 312 at Muddy Cedar Creek Tributary

1.3 PROJECT COORDINATES:

BEGIN: (Lat) 32.694053 ,(Long) -96.159410

END: (Lat) 32.693557 ,(Long) -96.158874

1.4 TOTAL PROJECT AREA (Acres): 0.31

1.5 TOTAL AREA TO BE DISTURBED (Acres): ____0.30

1.6 NATURE OF CONSTRUCTION ACTIVITY:

Construction of bridge replacement consisting of replacing bridge and approaches.

1.7 MAJOR SOIL TYPES:

Soil Type	Description
Crockett , 2 to 5% slopes 82% of Area	Moderately well drained, High runoff
Kemp, fequently flooded 18% of Area	Bars on flood plains, moderately well drained, neglible runoff
	Average grass,Pasture throughout. 73% vegetative density

1.8 PROJECT SPECIFIC LOCATIONS (PSLs):

PSLs must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. PSLs may be identified during preconstruction meetings or during the construction process. Please choose from the options below: ☑ PSLs determined during preconstruction meeting

- □ PSLs determined during construction
- ☐ No PSLs planned for construction

Туре	Sheet #s	activities
		 X Contaminated water from excavation or dewatering water X Sanitary waste from onsite restroom facilities X Trash from various construction activities/recepta □ Long-term stockpiles of material and waste
		▼ Other: Concrete pouring and washout
		□ Other:
		☐ Other:

All off-ROW PSLs required by the Contractor are the Contractor's responsibility. The Contractor shall secure all permits required by local, state, federal laws for off-ROW PSLs. The contractor shall provide diagrams, areas of disturbance, acreage, and BMPs for all off-ROW PSLs within one mile of the project.

1.9 CONSTRUCTION ACTIVITIES:

(Use the following list as a starting point when developing the Construction Activity Schedule and Ceasing Record in Attachment 2.3.)

- X Mobilization
- Install sediment and erosion controls
- Blade existing topsoil into windrows, prep ROW, clear and grub
- Remove existing pavement
- X Grading operations, excavation, and embankment
- X Excavate and prepare subgrade for proposed pavement widenina
- ☐ Remove existing culverts, safety end treatments (SETs)
- □ Remove existing metal beam guard fence (MBGF), bridge rail
- X Install proposed pavement per plans
- □ Install culverts, culvert extensions, SETs
- X Install mow strip, MBGF, bridge rail
- X Place flex base
- ▼ Blade windrowed material back across slopes
- ▼ Revegetation of unpaved areas
- X Achieve site stabilization and remove sediment and erosion control measures

☐ Other:				
□ Other				

_ C.1.O1.							
	-						
	Other:						

1.10 POTENTIAL POLLUTANTS AND SOURCES:

- Sediment laden stormwater from stormwater conveyance over disturbed area
- X Fuels, oils, and lubricants from construction vehicles, equipment,
- Solvents, paints, adhesives, etc. from various construction
- ▼ Transported soils from offsite vehicle tracking
- ▼ Construction debris and waste from various construction activities
- Contaminated water from excavation or dewatering pump-out
- Sanitary waste from onsite restroom facilities
- Trash from various construction activities/receptacles
- Long-term stockpiles of material and waste

Other: _	Concrete pouring and washout
□ Other:	
_	

1.11 RECEIVING WATERS:

Receiving waters must be depicted on the Environmental Layout Sheets in Attachment 1.2 of this SWP3. Include Segment # for receiving waters.

Tributaries	Classified Waterbody
Muddy Cedar Creek	Flows into Cedar Creek (0818B*) then Cedar Creek Reservoir (0818*)
* Add (*) for impaired waterbodies	s with pollutant in ().

- *Impaired by Bacteria in water (Recreation use)

1.12 ROLES AND RESPONSIBILITIES: TxDOT

- X Development of plans and specifications
- X Perform SWP3 inspections
- X Maintain SWP3 records and update to reflect daily operations

	·
□ Other:	
⊔ Omer	

1.13 ROLES AND RESPONSIBILITIES: CONTRACTOR

X Day To Day Operational Control

□ Other:

X Maintain schedule of major construction activities

X Install, maintain and modify BMPs

Other:			



CR 312 at Muddy Cedar Creek Tributary

STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 1 of 2

FED. RD. DIV. NO.		PROJECT NO.			SHEET NO.
6		SEE TITLE SHEET			
STATE		STATE COUNTY			
TEXAS	3	DAL KAUFMAN			
CONT.		SECT.	JOB HIGHWAY NO.		١0.
0918	3	11	100,ETC.	CR 110,	ETC.

2.0 BEST MANAGEMENT PRACTICES (BMPs) AND CONTROLS, INSPECTION, AND **MAINTENANCE**

The Contractor shall be the responsible party for implementing the BMPs described herein and for complying with the SWP3 for control of erosion and sedimentation during day-to-day operations. The Contractor shall implement changes to this SWP3 approved by TxDOT within the times specified in this SWP3 or the CGP.

	OSION CONTROL AND SOIL ABILIZATION BMPs:
T/P	
	rotection of Existing Vegetation egetated Buffer Zones
	oil Retention Blankets eotextiles
	ulching/ Hydromulching
	oil Surface Treatments
	emporary Seeding ermanent Planting, Sodding or Seeding
	iodegradable Erosion Control Logs
	ock Filter Dams/ Rock Check Dams
X 🗆 Ve	ertical Tracking
□□In □XRi	terceptor Swale
	iversion Dike
	emporary Pipe Slope Drain
	mbankment for Erosion Control aved Flumes
	ther: Preservation of natural resources
	ther: Compost manufactured topsoil
	ther: Vegetation lined ditches
	DIMENT CONTROL BMPs:
T/P	odegradable Erosion Control Logs
	ewatering Controls
	let Protection
	ock Filter Dams/ Rock Check Dams
	andbag Berms ediment Control Fence
	abilized Construction Exit
	oating Turbidity Barrier
	egetated Buffer Zones
	egetated Filter Strips
X □ Ot □ Ot	ther: Rock bedding at construction exits
	ther:
Refer to	the Environmental Layout Sheets/ SWP3 Layout Sheets

located in Attachment 1.2 of this SWP3

2.3 PERMANENT CONTROLS:

(Coordinate post-construction BMPs with appropriate TxDOT maintenance sections.)

BMPs To Be Left In Place Post Construction:

Type	Stationing			
Туре	From	То		
No permanent controls are				
planned.				
Defer to the Environmental Lave	ut Chapta/ CM/D2	Lavand Obaada		

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.4 OFFSITE VEHICLE TRACKING CONTROLS:

X Excess dirt/mud on road removed daily

☐ Haul roads dampened for dust control

X Loaded haul trucks to be covered with tarpaulin

X Stabilized construction exit

X Other: Dampen disturbed soil areas as needed for dust control.

Other:
Other:
Other:

2.5 POLLUTION PREVENTION MEASURES:

- X Chemical Management
- X Concrete and Materials Waste Management
- X Debris and Trash Management
- X Dust Control
- X Sanitary Facilities
- X Other: Avoid storing portable sanitary units, concrete washouts or chemicals within 50-ft upgradient of a receiving water without adequate pollution controls.

X Other:	Capture saw cutting debris and concrete slurry for	
<u>proper</u>	disposal.	

∏ X Oth	er:
---------	-----

Other:	

2.6 VEGETATED BUFFER ZONES:

Type

Natural vegetated buffers shall be maintained as feasible to protect adjacent surface waters. If vegetated natural buffer zones are not feasible due to site geometry, the appropriate additional sediment control measures have been incorporated into this SWP3.

Stationing

From

	1 10111	10
Muddy Cedar Creek Tributary: Ve to type of work which includes ex- remove exisiting bridge, build new riprap.	egetative Buffer N cavation at bridge bridge and insta	lot feasible due e near creek to alling of stone
Sediment fence, rock filter dams, & erosion control logs.	313+50	317+00

Refer to the Environmental Layout Sheets/ SWP3 Layout Sheets located in Attachment 1.2 of this SWP3

2.7 ALLOWABLE NON-STORMWATER DISCHARGES:

- X Fire hydrant flushings
- X Irrigation drainage
- X Pavement washwater (where spills or leaks have not occurred, and detergents are not used)
- X Potable water sources
- X Springs
- X Uncontaminated groundwater
- X Water used to wash vehicles or control dust
- X Other allowable non-stormwater discharges as allowed by TPDES GP TXR150000.

2.8 INSPECTIONS:

All disturbed areas and erosion and sediment control devices shall be inspected at least once every seven (7) days. Inspections shall be performed by TxDOT as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.

2.9 MAINTENANCE:

Control measures shall be properly installed according to specifications. If it is determined that a BMP or control measure is not operating effectively, maintenance must be accomplished as soon as possible and before the next anticipated rain event, but in no case later than 7 calendar days after being able to access the site. Maintenance shall be performed by the Contractor as indicated on the Field Inspection and Maintenance Report Form 2118 and retained in Attachment 2.3 of this SWP3.



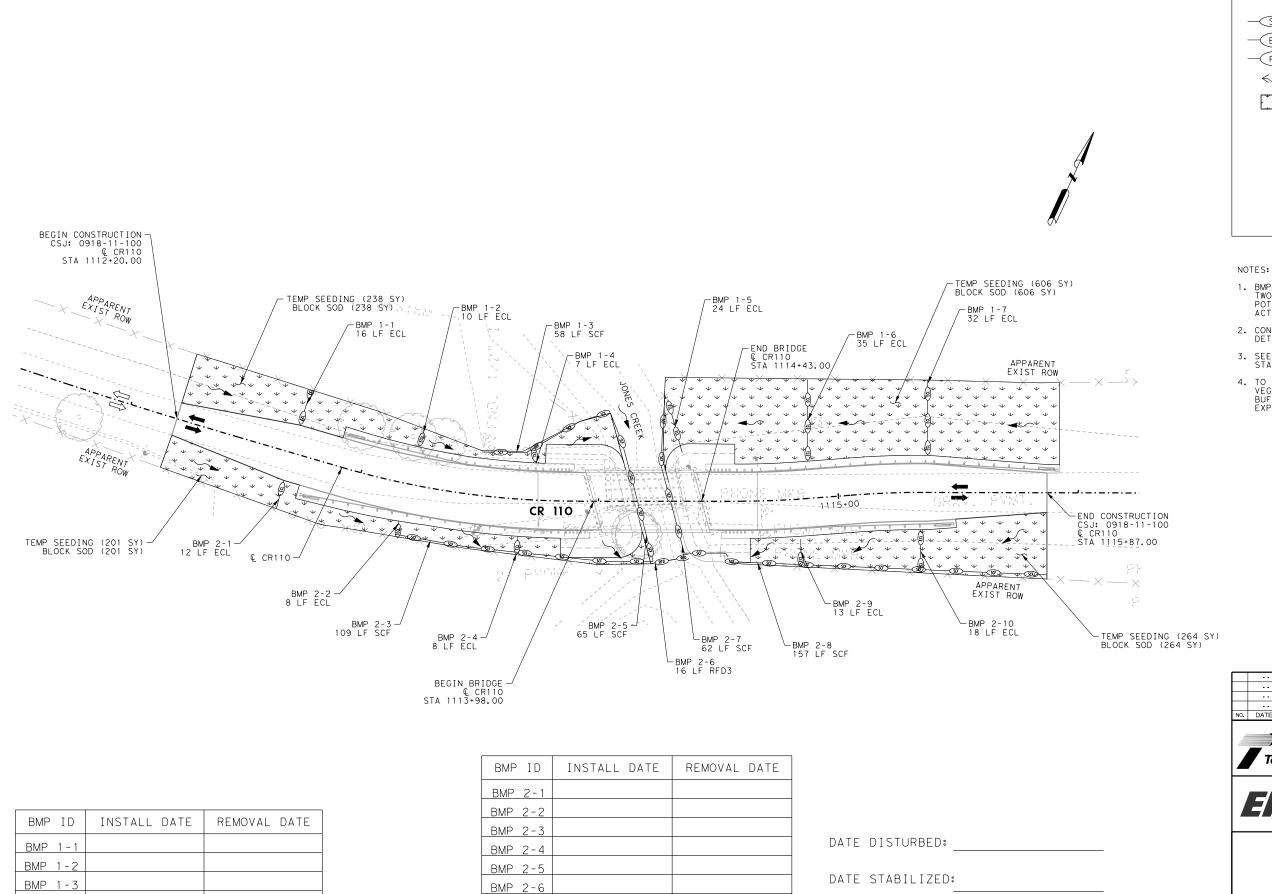
CR 312 at Muddy Cedar Creek Tributary

STORMWATER POLLUTION PREVENTION PLAN (SWP3) (Less Than 1 Acre)



Sheet 2 of 2

FED. RD. DIV. NO.	PROJECT NO.			SHEET NO.	
6		SEE TITLE SHEET			
STATE		STATE DIST.	COUNTY		
TEXAS	3	DAL	KAUFMAN		
CONT.		SECT.	JOB HIGHWAY NO.		٠0.
0918	3	1 1	100,ETC.	CR 110,	ETC.



BMP 2-7

BMP 2-8

BMP 2-9

BMP 2-10

4:59:15 PM 18-19-4\CADD_BR_11-

BMP 1-4

<u>BMP</u>_1-5

BMP 1-6

BMP 1-7

LEGEND:

SCF SEDIMENT CONTROL FENCE



RFD — ROCK FILTER DAM DIRECTION OF FLOW



- BMP'S SHALL BE INSTALLED NO SOONER THAN TWO WEEKS PRIOR TO SOIL DISTURBANCE OR POTENTIAL POLLUTANT-GENERATING ACTIVITIES IN THEIR CONTROL AREA.
- 2. CONSTRUCTION EXITS TO BE FIELD DETERMINED AND APPROVED BY ENGINEER.
- 3. SEE DAILY WORK REPORTS FOR INITIAL STABILIZATION TIMEFRAMES.
- 4. TO THE EXTENT FEASIBLE, PRESERVE EXISTING VEGETATION AND MAINTAIN A VEGETATIVE BUFFER ALONG STREAM BANK TO MINIMIZE EXPOSURE OF DISTURBED SOILS.



		SCALE: 1 =40	
NO.	DATE	REVISION	APPROV.
		1	

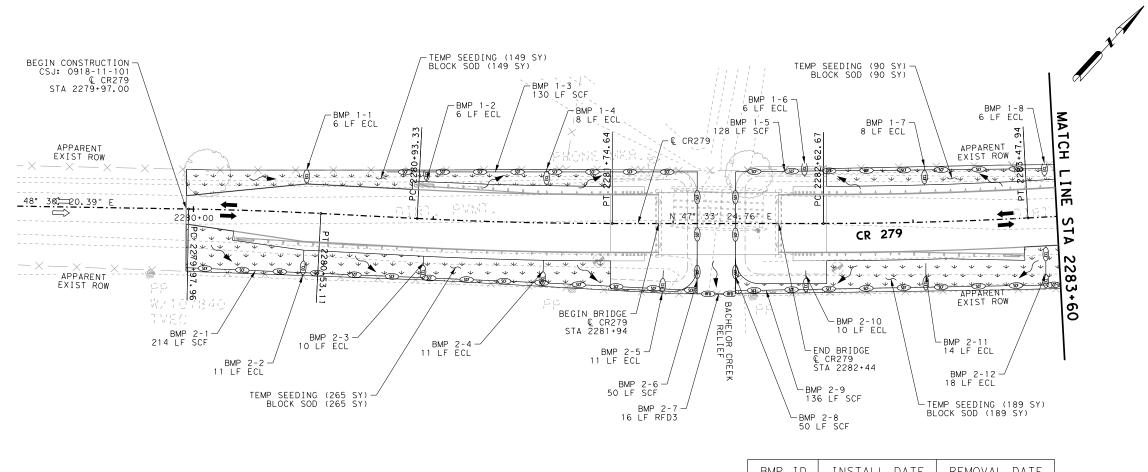




SW3P LAYOUT CR 110

SHEET 1 OF 3

	OI J								
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		H[GHWAY NO.	
K DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ET	C
W:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.	
K DW:	RC	DAL	KAUF	MAN	0918	11	100,ETC	134	_



MATCH LINE STA 2285-80	2285+00 PC 2285+00 PC 2285+00 PC END CONSTRUCTION CSJ: 0918-11-101 Q CR279 STA 2284+55.00	
•	STA 2284+55 00	

TEMP SEEDING (210 SY) BLOCK SOD (210 SY)

RMorfin 2/16/2023 N:\P5072-18-19-4\CADD_BR_11-N:\P5074-18-19-4\CADD_BR_11-

DATE DISTURBED:

DATE STABILIZED:

BMP ID	INSTALL DATE	REMOVAL DATE
BMP 1-1		
BMP 1-2		
BMP 1-3		
BMP 1-4		
BMP 1-5		
BMP 1-6		
BMP 1-7		
BMP 1-8		
BMP 1-9		

BMP ID	INSTALL DATE	REMOVAL DATE
BMP 2-1		
BMP 2-2		
BMP 2-3		
BMP 2-4		
BMP 2-5		
BMP 2-6		
BMP 2-7		
BMP 2-8		
BMP 2-9		
BMP 2-10		
BMP 2-11		
BMP 2-12		
BMP 2-13		
BMP 2-14		

<u>LEGEND:</u>

SCF SEDIMENT CONTROL FENCE

ECL EROSION CONTROL LOG

— ECL — EROSION CONTROL — RFD — ROCK FILTER DAM

DIRECTION OF FLOW

TEMP SEEDING & BLOCK SOD

NOTES:

- BMP'S SHALL BE INSTALLED NO SOONER THAN TWO WEEKS PRIOR TO SOIL DISTURBANCE OR POTENTIAL POLLUTANT-GENERATING ACTIVITIES IN THEIR CONTROL AREA.
- 2. CONSTRUCTION EXITS TO BE FIELD DETERMINED AND APPROVED BY ENGINEER.
- 3. SEE DAILY WORK REPORTS FOR INITIAL STABILIZATION TIMEFRAMES.
- 4. TO THE EXTENT FEASIBLE, PRESERVE EXISTING VEGETATION AND MAINTAIN A VEGETATIVE BUFFER ALONG STREAM BANK TO MINIMIZE EXPOSURE OF DISTURBED SOILS.



NO.	DATE	REVISION	APPROV.



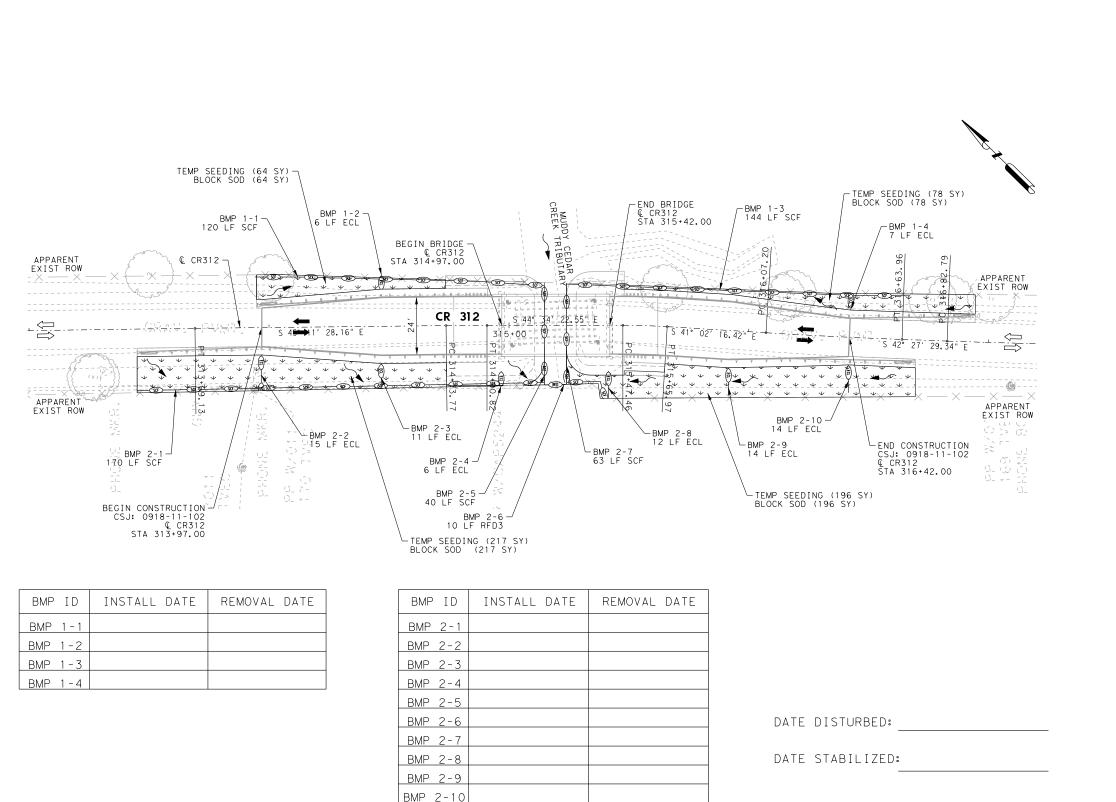


F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

SW3P LAYOUT CR 279

SHEET 2 OF 2

TILL I Z	- 01 2							
N:	RM	FED.RD. DIV.NO.	STATE		PROJE	CT NO.		HIGHWAY NO.
K DN:	RC	6	TEXAS	S	EE TIT	LE SHE	ET	CR 110,ETC
W:	RM	STATE DIST.	COUN	ITY	CONTROL NO.	SECTION NO.	JOB NO.	SHEET NO.
K DW:	RC	DAL	KAUF	MAN	0918	11	100.ETC	135



<u>LEGEND:</u>

— SCF — SEDIMENT CONTROL FENCE



RFD ROCK FILTER DAM

DIRECTION OF FLOW



TEMP SEEDING & BLOCK SOD

NOTES:

- BMP'S SHALL BE INSTALLED NO SOONER THAN TWO WEEKS PRIOR TO SOIL DISTURBANCE OR POTENTIAL POLLUTANT-GENERATING ACTIVITIES IN THEIR CONTROL AREA.
- 2. CONSTRUCTION EXITS TO BE FIELD DETERMINED AND APPROVED BY ENGINEER.
- 3. SEE DAILY WORK REPORTS FOR INITIAL STABILIZATION TIMEFRAMES.
- 4. TO THE EXTENT FEASIBLE, PRESERVE EXISTING VEGETATION AND MAINTAIN A VEGETATIVE BUFFER ALONG STREAM BANK TO MINIMIZE EXPOSURE OF DISTURBED SOILS.



		SCALE: 1 10	
NO.	DATE	REVISION	APPROV.



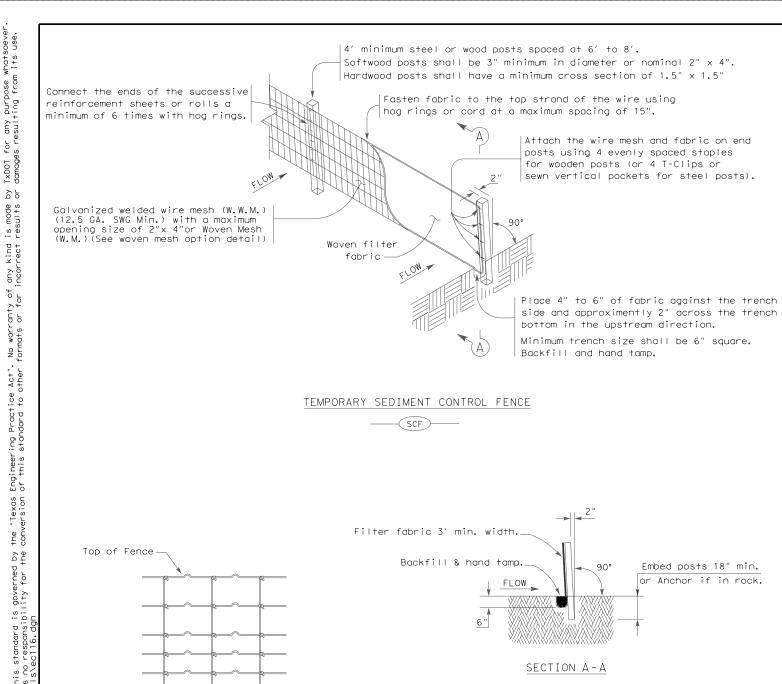


F-6932 15021 Katy Freeway, Suite 500 Houston, Texas, 77094 281-945-0069 PH 281-945-0081 FX

SW3P LAYOUT CR 312

SHEET 3 OF 3

HEEL	3 OF 3						
N:	RM	FED.RD. DIV.NO.	STATE	PRO	JECT NO.		HIGHWAY NO.
K DN:	RC	6	TEXAS	SEE TI	TLE SH	ET	CR 110,ETC
W:	RM	STATE DIST.	COUN	ITY CONTRO	L SECTION NO.	JOB NO.	SHEET NO.
K DW:	RC	DAL	KAUF	MAN 0918	- 11	100, ETC	136



HINGE JOINT KNOT WOVEN MESH (OPTION) DETAIL

Galvanized hinge joint knot woven mesh (12.5 GA.SWG Min.) requires a minimum of five horizontal wires spaced at a maximum of 12 inches apart and all vertical wires spaced at a maximum of 12 inches apart.

SEDIMENT CONTROL FENCE USAGE GUIDELINES

A sediment control fence may be constructed near the downstream perimeter of a disturbed area along a contour to intercept sediment from overland runoff. A 2 year storm frequency may be used to calculate the flow rate to be filtered.

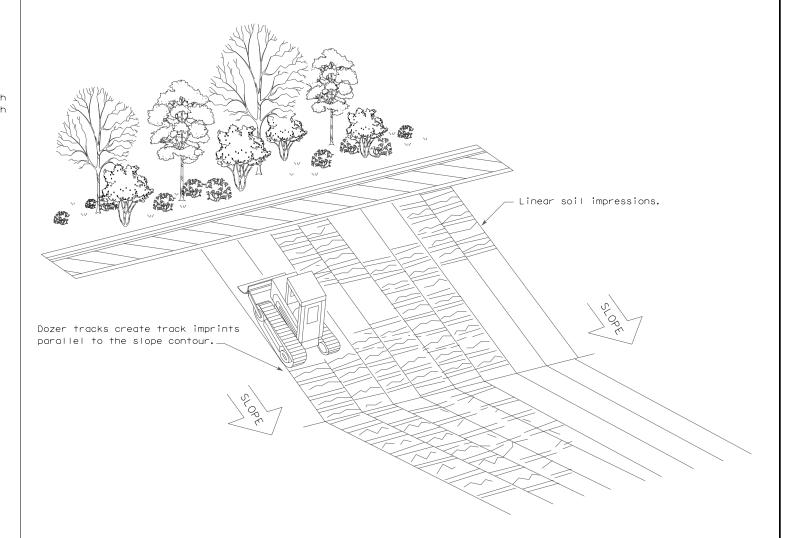
Sediment control fence should be sized to filter a maximum flow through rate of 100 GPM/FT². Sediment control fence is not recommended to control erosion from a drainage area larger than 2 acres.

LEGEND

Sediment Control Fence

GENERAL NOTES

- 1. Vertical tracking is required on projects where soil distributing activities have occurred unless otherwise approved.
- 2. Perform vertical tracking on slopes to temporarily stabilize soil.
- 3. Provide equipment with a track undercarriage capable of producing linear soil impressions measuring a minimum of 12" in length by 2" to 4" in width by 1/2" to 2" in depth.
- 4. Do not exceed 12" between track impressions.
- 5. Install continous linear track impressions where the minimum 12" length impressions are perpendicular to the slope or direction of water flow.



VERTICAL TRACKING



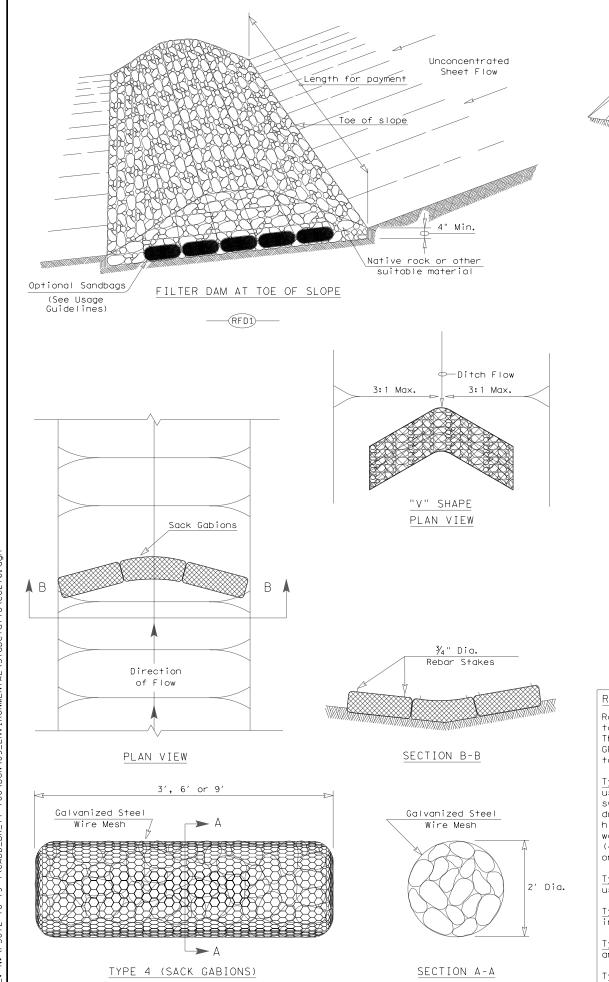
Design Division Standard

TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

FENCE & VERTICAL TRACKING

EC(1)-16

ILE: ec116	DN: TxD	OT	ck: KM	DW:	VP DN/CK: LS		
TxDOT: JULY 2016	CONT	SECT	JOB		HIGHWAY		
REVISIONS	0918	11	100,ET	TC. CR		110,ETC.	
	DIST	COUNTY			SHEET NO.		
	DAL		KAUFMA	٩N		137	



by P made s∪l†s

kind rect

ranty of or for

Engineering Practice Act". of this standard to other

"Texas

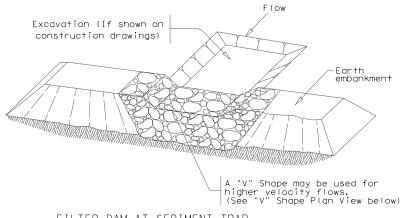
for †

gover |ity 1

standard is responsibil

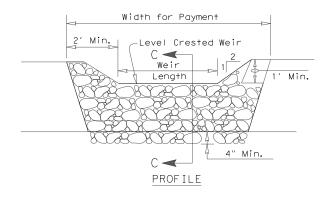
2/16/2023 N:\P5072-

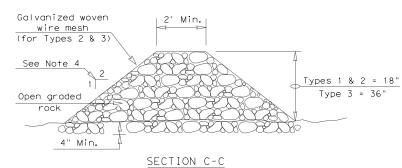
____(RFD4)____



FILTER DAM AT SEDIMENT TRAP







ROCK FILTER DAM USAGE GUIDELINES

Rock Filter Dams should be constructed downstream from disturbed areas to intercept sediment from overland runoff and/or concentrated flow. The dams should be sized to filter a maximum flow through rate of 60 ${\sf GPM/FT^2}$ of cross sectional area. A 2 year storm frequency may be used to calculate the flow rate.

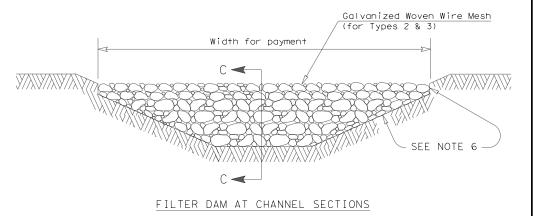
Type 1 (18" high with no wire mesh) (3" to 6" aggregate): Type 1 may be used at the toe of slopes, around inlets, in small ditches, and at dike or swale outlets. This type of dam is recommended to control erosion from a drainage area of 5 acres or less. Type 1 may not be used in concentrated high velocity flows (approximently 8 Ft/Sec or more) in which aggregate wash out may occur. Sandbags may be used at the embedded foundation (4" deep min.) for better filtering efficiency of low flows if called for on the plans or directed by the Engineer.

Type 2 (18" high with wire mesh) (3" to 6" aggregate): Type 2 may be used in ditches and at dike or swale outlets.

Type 3 (36" high with wire mesh) (4" to 8" aggregate): Type 3 may be used in stream flow and should be secured to the stream bed.

Type 4 (Sack gabions) (3" to 6" aggregate): Type 4 May be used in ditches and smaller channels to form an erosion control dam.

Type 5: Provide rock filter dams as shown on plans.



GENERAL NOTES

1. If shown on the plans or directed by the Engineer, filter dams should be placed near the toe of slopes where erosion is anticipated, upstream and/or downstream at drainage structures, and in roadway ditches and channels to collect sediment.

- 2. Materials (aggregate, wire mesh, sandbags, etc.) shall be as indicated by the specification for "Rock Filter Dams for Erosion and Sedimentation
- 3. The rock filter dam dimensions shall be as indicated on the SW3P plans.
- 4. Side slopes should be 2:1 or flatter. Dams within the safety zone shall have sideslopes of 6:1 or flatter.
- 5. Maintain a minimum of 1' between top of rock filter dam weir and top of embankment for filter dams at sediment traps.
- 6. Filter dams should be embedded a minimum of 4" into existing ground.
- 7. The sediment trap for ponding of sediment laden runoff shall be of the dimensions shown on the plans.
- 8. Rock filter dam types 2 & 3 shall be secured with 20 gauge galvanized woven wire mesh with 1" diameter hexagonal openings. The aggregate shall be placed on the mesh to the height & slopes specified. The mesh shall be folded at the upstream side over the aggregate and tightly secured to itself on the downstream side using wire ties or hog rings. For in stream use, the mesh should be secured or staked to the stream bed prior to aggregate placement.
- 9. Sack Gabions should be staked down with $\frac{3}{4}$ " dia. rebar stakes, and have a double-twisted hexagonal weave with a nominal mesh opening of 2 $\frac{1}{2}$ " x 3 $\frac{1}{4}$ "
- 10. Flow outlet should be onto a stabilized area (vegetation, rock, etc.).
- 11. The guidelines shown hereon are suggestions only and may be modified by

PLAN SHEET LEGEND

Type 1 Rock Filter Dam — Type 2 Rock Filter Dam Type 3 Rock Filter Dam

Type 4 Rock Filter Dam RFD4



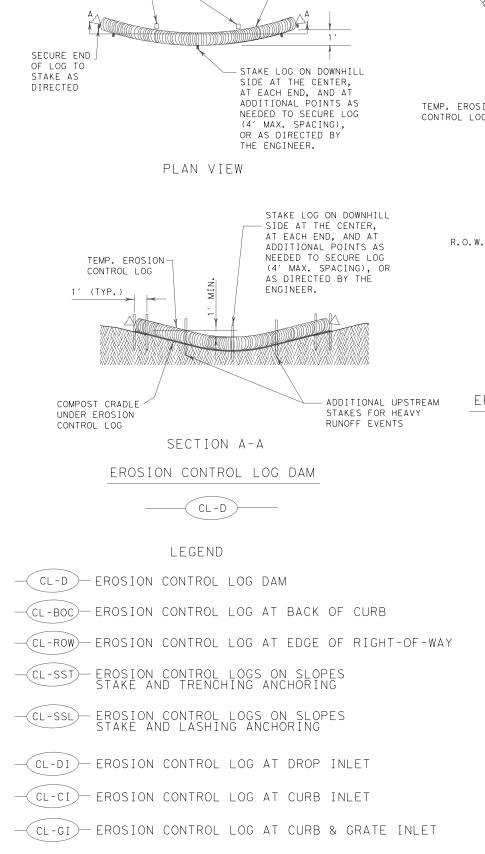
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

ROCK FILTER DAMS

EC(2)-16

ILE: ec216	DN: TxD	ОТ	ск: КМ	DW:	VP DN/CK: LS		
TxDOT: JULY 2016	CONT	SECT	JOB	В		HIGHWAY	
REVISIONS	0918	11	100,ET	С.	CR 1	10,ETC.	
DIST		COUNTY			SHEET NO.		
	dal		KALIEMA	١N١		1 7 8	

DATE: FILE:



FLOW

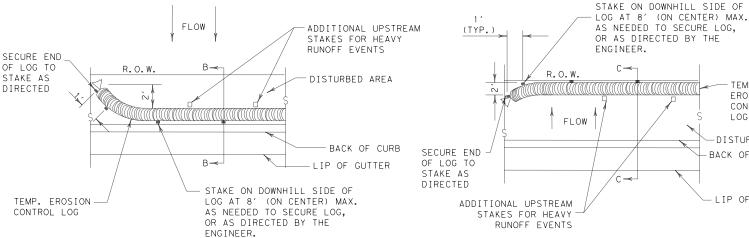
ADDITIONAL UPSTREAM

STAKES FOR HEAVY

RUNOFF EVENTS

TEMP. EROSION

CONTROL LOG



TEMP. EROSION

COMPOST CRADIE

UNDER EROSION

CONTROL LOG

CONTROL LOG

PLAN VIEW

SECTION B-B

EROSION CONTROL LOG AT BACK OF CURB

CL-BOC

1/2 " ±

REBAR STAKE DETAIL

PLAN VIEW

TEMPORARY

-DISTURBED AREA

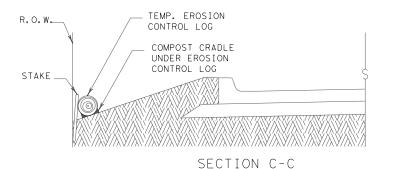
LIP OF GUTTER

EROSION

CONTROL

LOG

BACK OF CURB



EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY



SEDIMENT BASIN & TRAP USAGE GUIDELINES

An erosion control log sediment trap may be used to filter sediment out of runoff draining from an unstabilized area.

The drainage area for a sediment trap should not exceed 5 acres. The trap capacity should be 1800 CF/Acre (0.5" over

Control logs should be placed in the following locations:

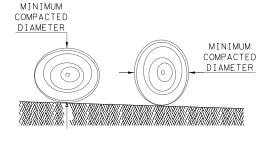
- 1. Within drainage ditches spaced as needed or min. 500' on center
- 3. Just before the drainage enters a water course
- limits where drainage flows away from the project.

The logs should be cleaned when the sediment has accumulated to a

will not be paid for separately.

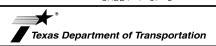
GENERAL NOTES:

- 1. EROSION CONTROL LOGS SHALL BE INSTALLED IN ACCORDANCE WITH MANFACTURER'S RECOMMENDATIONS, OR AS DIRECTED BY THE ENGINEER.
- 2. LENGTHS OF EROSION CONTROL LOGS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND AS REQUIRED FOR THE PURPOSE INTENDED.
- UNLESS OTHERWISE DIRECTED, USE BIODEGRADABLE OR PHOTODEGRADABLE CONTAINMENT MESH ONLY WHERE LOG WILL REMAIN IN PLACE AS PART OF A VEGETATIVE SYSTEM. FOR TEMPORARY INSTALLATIONS, USE RECYCLABLE CONTAINMENT MESH.
- FILL LOGS WITH SUFFICIENT FILTER MATERIAL TO ACHIEVE THE MINIMUM COMPACTED DIAMETER SPECIFIED IN THE PLANS WITHOUT EXCESSIVE DEFORMATION.
- STAKES SHALL BE 2" X 2" WOOD OR #3 REBAR, 2'-4' LONG, EMBEDDED SUCH THAT 2" PROTRUDES ABOVE LOG, OR AS DIRECTED BY THE ENGINEER.
- 6. DO NOT PLACE STAKES THROUGH CONTAINMENT MESH.
- 7. COMPOST CRADLE MATERIAL IS INCIDENTAL & WILL NOT BE PAID FOR SEPARATELY.
- SANDBAGS USED AS ANCHORS SHALL BE PLACED ON TOP OF LOGS & SHALL BE OF SUFFICIENT SIZE TO HOLD LOGS IN PLACE.
- TURN THE ENDS OF EACH ROW OF LOGS UPSLOPE TO PREVENT RUNOFF FROM FLOWING AROUND THE
- 10. FOR HEAVY RUNOFF EVENTS, ADDITIONAL UPSTREAM STAKES MAY BE NECESSARY TO KEEP LOG FROM FOLDING IN ON ITSELF.



DIAMETER MEASUREMENTS OF EROSION CONTROL LOGS SPECIFIED IN PLANS

SHEET 1 OF 3



TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

EROSION CONTROL LOG

EC(9) - 16

ILE: ec916	DN: TxD	OT	ск: КМ	DW:	: LS/PT CK: LS		ck: LS
TxDOT: JULY 2016	CONT	SECT	JOB		HIGHWAY		
REVISIONS	0918	11	100,ET	С.	CR	11	O,ETC.
	DIST	COUNTY				S	HEET NO.
	DAL		KAUFMA	٩N			139

the drainage area).

- 2. Immediately preceding ditch inlets or drain inlets
- 4. Just before the drainage leaves the right of way
- 5. Just before the drainage leaves the construction

depth of 1/2 the log diameter.

Cleaning and removal of accumulated sediment deposits is incidental and

SECURE END OF LOG TO STAKE AS

TEMP. EROSION-CONTROL LOG

FLOW

2/16/2023 N:\P5072-1

DATE: FILE:

EROSION CONTROL LOG AT CURB & GRADE INLET

MIN.

SANDBAG

TEMPORARY EROSION CONTROL LOG USE STAKES ON DOWNSTREAM SIDE OF LOGS, AT ENDS, MIDPOINT, & AS NEEDED OR SANDBAGS TO HOLD IN PLACE.

EROSION CONTROL LOG AT DROP INLET

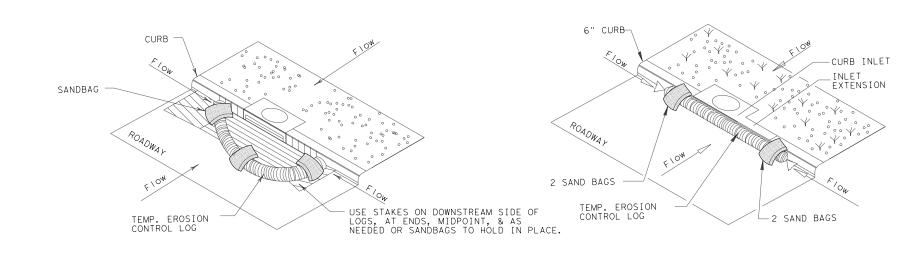
CURB AND GRATE INLET

OVERLAP ENDS TIGHTLY 24" MINIMUM

- FLOW

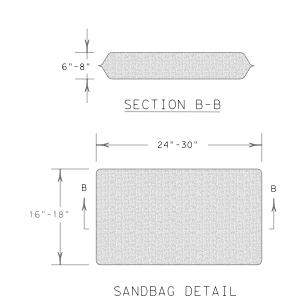
-STAKE OR USE SANDBAGS ON DOWNHILL SIDE OF LOG AS NEEDED TO HOLD IN PLACE (TYPICAL)

COMPLETELY SURROUND DRAINAGE ACCESS TO AREA DRAIN INLETS WITH EROSION CONTROL LOG



EROSION CONTROL LOG AT CURB INLET

NOTE: EROSION CONTROL LOGS USED AT CURB INLETS SHOULD ONLY BE USED IF THEY WILL NOT IMPEDE TRAFFIC OR FLOOD THE ROADWAY OR WHEN THE STORM SEWER SYSTEM IS NOT FULLY FUNCTIONAL.



SHEET 3 OF 3



EROSION CONTROL LOG AT CURB INLET

TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

EROSION CONTROL LOG

EC(9)-16

FILE: ec916	DN: TxD	OT	CK: KM DW: LS/PT		/PT	ck: LS
C TxDOT: JULY 2016	CONT	SECT	JOB		HIGHWAY	
REVISIONS	0918	11	100,ETC. CR		R 11	O,ETC.
	DIST	COUNTY SHEE		HEET NO.		
	ΠΔΙ	KALIEMAN		1 4 1		

SURFACE PREPARATION ITEM 160* TOPSOIL SY / ITEM 161* COMPOST MANUF. TOPSOIL (BOS) (4") SY

SURFACE PREPARATION

Prepare planting area surface BEFORE placing Topsoil, Compost, Fertilizer, Seed and/or Sod.

Once project area has been completed to final lines, grade and compaction, remove objectionable materials from planting area surface and cultivate existing surface to a depth of 4 inches, unless otherwise specified or directed.

Refer to Items 160 and 161 of TxDOT 2014 Standard Specifications* for specifications, dimensions, volumes, and measurements that have been modified or not shown in plans. Materials and construction shall meet all specifications.

- TOPSOIL NOTES:

 1. When Topsoil is specified under Item 160, use suitable material salvaged from the project ROW in accordance with Item 160 specifications, and/or secure additional good material from approved sources.

 2. Topsoil shall include only the top 6 inches of its native surface, and be easily cultivated, fertile, erosion-resistant
- and free of objectionable materials.
- 3. Topsoil obtained from sites outside of the ROW must come from approved sources and have a pH between 5.5 and 8.5 su.
 4. Place Topsoil on pre-cultivated surface, spread to a uniform loose cover at thickness specified, and shape per plans.
 Water and roll the finished surface with a light roller or other suitable equipment per Item 160.3; do not over-compact.

- 1. When Compost Manufactured Topsoil (4") is specified under Item 161, use compost meeting all requirements of Item 161.2 and Table 1. Provide quality control (QC) documentation and obtain Engineer approval prior to compost delivery.

 2. Contractor shall provide tickets/invoices that document material type, quantity and placement for all compost delivered.

 3. Additional topsoil may be required to be imported to achieve the compost/topsoil mix ratio. Topsoil must meet Item 160
- specifications.

APPLICATION OF COMPOST MANUFACTURED TOPSOIL (4")

AFTER Surface Preparation, uniformly spread a 1-inch layer of compost on-grade with 3 inches topsoil over pre-cultivated planting area. (25% compost and 75% topsoil = 1" compost and 3" topsoil.)

Then mix compost and topsoil together by cultivating the compost into the topsoil (by till or disk) to a 4-inch (4") depti Roll the finished surface with a light corrugated drum; do not over-compact.

FERTILIZER ITEM 166* FERTILIZER AC

ANALYSIS FOR FERTILIZER APPLICATION RATE

Unless otherwise stated in the plans. Contractor shall perform at least one soil analysis on each project before fertilization, and submit results to Engineer with recommended fertilizer rates based on soil analysis. Engineer may direct sample location(s). Soil analysis may be waived if both compost and sod are used on entire project.

RECOMMENDED

Sept 16th, Oct,

Nov, Dec, Jan, Feb, Mar 14th

- 1. Refer to Item 166 of TxDOT 2014 Standard Specifications* for specifications, dimensions, volumes, and measurements that have been modified or not shown in plans. Materials and construction shall meet all specifications.
 2. Apply fertilizer BEFORE seeding, or AFTER placing sod.
 3. Use fertilizer containing nitrogen (N), phosphoric acid (P) and potash (K) nutrients, unless otherwise specified. At least 50% of the Nitrogen component shall be a slow-release sulfur-coated urea as described in Item 166.3. Do not apply more than 60 lbs Nitrogen per acre without Engineer concurrence.
 4. Deliver fertilizer in bags, clearly labeled to show contents, unless otherwise specified or approved prior to delivery. When non-bagged, loose fertilizer is approved, provide documentation for each load of material delivered, to validate authenticity of the material.
 5. Apply fertilizer uniformly, as a dry, granular material, essentially dust-free, and do not mix with water for application as a slurry.

- application as a slurry.

 6. When both temporary and permanent seeding are specified for the same area, apply half of the required fertilizer before the temporary seeding operation and the other half before the permanent seeding operation.

PERMANENT RURAL SEED MIX

SEEDING FOR EROSION CONTROL ITEM 164* DRILL SEEDING AC

PLANTING SEASON ITEM 164 - DRILL SEEDING (PERM) (URBAN) (CLAY) ITEM 164 - DRILL SEEDING (PERM) (RURAL) (CLAY) ITEM 164 - DRILL SEEDING (TEMP) (WARM OR COOL) Pure Live Seed Rate** Pure Live Seed Rate** Pure Live Seed Rate** Green Sprangletop (Leptochloa dubia) Sideoats Grama (El Reno) (Bouteloua curtipendula) Buffalograss (Texoka) (Buchloe dactyloides) - 0.3 lbs/AC - 3.6 lbs/AC - 1.6 lbs/AC Green Sprangletop (Van Horn) Sideoats Grama (Haskell) - 1.0 lbs/AC - 1.0 lbs/AC Foxtail Millet (Setaria italica) - 34 lbs/AC WARM SEASON - 1.0 lbs/AC - 0.4 lbs/AC Texas Grama (Atascosa) Mar.15th, April, Hairy Grama (Chaparral) Bermudagrass (Cynodon dactylon) - 2.4 lbs/AC Shortspike Windmillgrass (Welder) Little Bluestem (OK Select) Purple Prairie Clover (Cuero) May, June, July, August, Sept. 15th - 0.2 lbs/AC - 0.8 lbs/AC - 0.6 lbs/AC Engelmann Daisy (Eldorado) Illinois Bundleflower - 1.3 lbs/AC - 0.2 lbs/AC Awnless Bushsunflower (Plateau) Pure Live Seed Rate* COOL SEASON

- 1. When seeding is specified under Item 164, refer to TxDOT 2014 Standard Specifications* for specifications, dimensions, volumes, and measurements that have been modified or not shown. Materials and construction shall meet specifications.

 2. Conduct seeding upon completion of each applicable construction stage (dependent upon planting season requirements),
- Conduct seeding upon completion of each applicable construction stage (dependent upon planting season requirements), without compensation for additional move ins.
 Place seed AFTER preparing planting area surface. Refer to Surface Preparation detail this sheet, as well as Topsoil Item 160 and Compost Manufactured Topsoil Item 161 when specified. Apply fertilizer per Item 166 BEFORE seeding, per specifications and this sheet, to help drill the fertilizer into the soil.
 When temporary grasses are well-established and more than 2 inches tall, mow planting area before seeding permanent grasses; mowing for this purpose will be subsidiary. When vegetation is not already well-established, cultivate planting area to a depth as described in Item 164m3, before temporary seeding and before permanent seedingm
 Seed material must be appropriate to the location, soil type and season. Use the seed mix species and pure live seed rates designated in Tables 1-4 of the TxDOT 2014 Standard Specifications* for Item 164, unless otherwise specified.
 All seed shall meet labeling, delivery, analysis, and testing requirements described in Item 164m2.1. Deliver seed in labeled, unopened bags or containers to Engineer prior to planting.
 Uniformly plant seed over the designated planting area, along the contour of slopes, and drill seed to a depth as described in Item 164.3.4.
 Hydroseeding may be allowed, when specified or Engineer concurs.

- 8. Hydroseeding may be allowed, when specified or Engineer concurs.
 9. Implement and continue Vegetative Watering per the schedule, rate and volume specified under Item 168.

TXDOT REFERENCE MATERIALS:

- * "STANDARD SPECIFICATIONS FOR CONSTRUCTION AND MAINTENANCE OF HIGHWAYS, STREETS, AND BRIDGES" 2014
 "A GUIDANCE TO ROADSIDE VEGETATION ESTABLISHMENT" 2004
 ONLINE TRAINING COURSE: MNT415 REVEGETATION DURING CONSTRUCTION

- DALLAS DISTRICT "VEGETATION ESTABLISHMENT GUIDELINES'

SODDING FOR EROSION CONTROL ITEM 162* BLOCK SOD (BERMUDA) SY

BLOCK OR ROLL SOD	COMMON NAME	BOTANICAL NAME		
	Common Bermuda Grass	Cynodon dactylon		

- SODDING NOTES:

 1. Refer to Item 162 of TxDOT 2014 Standard Specifications* for specifications, dimensions, volumes, and measurements that have been modified or not shown in plans. Materials and construction shall meet all specifications.

 2. Place sod between the average date of the last freeze in the Spring and 6 weeks before the average date of the first freeze in the Fall, per the Texas Almanac for the project area.

 3. Place sod only AFTER soil surface preparation is complete as detailed in this sheet. Dry soil may require pre-watering.

 4. Place all sod (blocks or rolls) within 24 hours of delivery to the site, and keep moist from the time it is dug up until it is planted. Sod with dried roots will not be accepted.

 5. Place sod with joints alternating on each row to prevent all joints from lining up, and place blocks firmly against adjacent blocks. Roll, tamp and trim sod per Item 162.3.

 6. Place fertilizer promptly AFTER sodding operation is complete in each area.

 7. Water sod immediately following placement, and continue Vegetative Watering per Item 168.

VEGETATIVE WATERING FOR ESTABLISHING SEED AND SOD ITEM 168* VEGETATIVE WATERING MG

WATERING SCHEDULE							
RATE	TIME SCHEDULE	TOTAL WATER ESTIMATE					
7,000 gallons/acre per working day	Vegetative watering for seed shall begin on the day after rainfall described below and continue for 60 consecutive working days;	420,000 gallons/acre (60 working days)					
12,000 gallons/acre per working day	vegetative watering for sod shall begin on the day the sod is placed and continue for a minimum of 15 consecutive working days.	720,000 gallons/acre (60 working days)					
1,000 gallons/acre per working day	Vegetative watering for seed and/or sod shall begin on the day after placement for 15 consecutive working days	15,000 gallons/acre (15 working days)					
	7,000 gallons/acre per working day 12,000 gallons/acre per working day 1,000 gallons/acre	7,000 gallons/acre per working day 12,000 gallons/acre per working day 12,000 gallons/acre per working days; vegetative watering for seed shall begin on the day after rainfall described below and continue for 60 consecutive working days; vegetative watering for sod shall begin on the day the sod is placed and continue for a minimum of 15 consecutive working days. 1,000 gallons/acre Vegetative watering for seed shall begin on the day after placement for shall begin on the day after placement for					

Notes: Rate and frequency may be adjusted, with the approval of the Engineer, to meet site conditions (especially with sod). For informational purposes only: 1,000 gallons equals 1 MG

VEGETATIVE WATERING NOTES:

PERMANENT URBAN SEED MIX

- 1. Refer to Item 168 of TxDOT 2014 Standard Specifications* for specifications, dimensions, volumes, and measurements that have been modified or not shown in plans. Materials and construction shall meet all specifications.

 2. Use clean water free of industrial waste and other substances harmful to vegetation growth, per Item 168.2.

 3. Use Vegetative Watering to keep the seed bed moist during germination; not to provide initial watering. After drill seeding, postpone watering operations until site receives at least 1/2-inch of natural rainfall in a single day. Delay watering operations for warm season grasses until soil temperature exceeds 70 degrees F.
- 4. For sod, water immediately.

 5. All water distribution equipment shall be furnished and operated to provide water at a uniform and controllable rate.

- 5. All water distribution equipment shall be furnished and operated to provide water at a uniform and controllable rate. Use a metering device on all watering equipment.
 6. Evenly distribute water over entire area designated for seeding and/or sodding, using even spray patterns that do not disturb seed bed and/or dislodge seed from seed bed.
 7. Do not water between the hours of 12:00 p.m. and 6:00 p.m. when daytime temperatures exceed 95 degrees F.
 8. After initial establishment period, continue intermittent watering of newly established seed or sod at a rate of approximately 1-inch water/week, during summer months until end of contract.
 9. If 1/4-inch or more of rainfall occurs on site on any given working day, no vegetative watering will be needed on that working day. (Note: 1/4-inch rain equals 7,000 gallons of water per acre.)
 10. Should the Contractor fail to apply the specified amount of water within the time allowed, any seed or sod in poor condition shall be replaced, fertilized, and watered at Contractor's expense.

TEMPORARY DRILL SEED MIX

Tall Fescue (Festuca arundinaceae) Western Wheatgrass (Agropyron smithii) Red Winter Wheat (Triticum aestivum) Cereal Rye

- 4.5 |bs/AC - 5.6 |bs/AC - 34 |bs/AC - 34 lbs/AC

**Note: The amount of Pure Live Seed (PLS) in one pound of bulk seed is based on three factors: % Purity, % Germination, and % Dormant.

Use the following formula to calculate PLS in bulk seed: PLS = % Purity X (% Germination + % Dormant)

Ensure that the specified amount of pure live seed is placed.

ROADSIDE MOWING ITEM 730* PROJECT MAINTENANCE AC

MOWING NOTES:

- 1. During project construction, once seed is established, use mowing to promote permanent grasses by mowing any remaining temporary grasses.

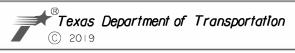
 2. Also mow established turf and ROW grasses in designated areas of project limits as specified or directed by Engineer.

 3. Remove litter and debris prior to mowing.

- 4. Do not mow on wet ground when soil rutting can occur.
 5. Hand-trim around obstructions and stormwater control devices as needed.
 6. Maintain paved surfaces free of tracked soils and clipped vegetation.

SEQUENCE OF WORK:

- CULTIVATE SURFACE SOIL.
- PREPARE / PLACE TOPSOIL, OR
- PREPARE / PLACE COMPOST MANUFACTURED TOPSOIL.
- APPLY FERTILIZER AND THEN PLACE SEEDING, OR
- PLACE SOD AND THEN APPLY FERTILIZER.
- CONDUCT VEGETATIVE WATERING.
- CONDUCT ROADSIDE MOWING, AS DIRECTED.



VEGETATION ESTABLISHMENT SHEET (DALLAS DISTRICT)

TEMPLATE REVISION DATE: 02/21/19

Ì	DESIGN CPB	FED. RD. DIV. NO.	FEDER	HIGHWAY NO.			
	GRAPHICS	6	(See	Title Sheet)	CR 110, etc.		
	XXX	STATE	DISTRICT	COUNTY	SHEET NO.		
	CHECK	TEXAS	DALLAS	KAUFMAN			
	CHECK	CONTROL	SECTION	JOB	142		
ı	XXX	0918	11	100, ETC.	Ī		