#### INDEX OF SHEETS

SEE SHEET 2

VOLUME I

(CONTRACT CSJ: 0106-04-036)

### STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

#### PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT

PROJECT NO.: BR 2022 (829)

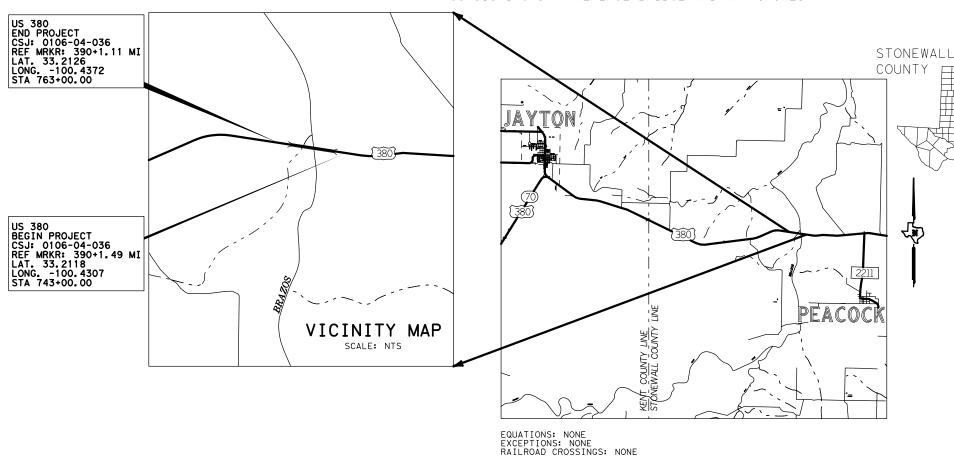
LENGTH OF ROADWAY = 780.00 FT = 0.148 MI LENGTH OF BRIDGE = 1220.00 FT = 0.231 MI TOTAL LENGTH OF PROJECT = 2000.00 FT = 0.379 MI

#### US 380 STONEWALL COUNTY

LIMITS: SALT FORK BRAZOS RIVER

FOR THE CONSTRUCTION OF: BRIDGE REPLACEMENT

CONSISTING OF: REPLACE BRIDGE AND APPROACHES



SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF TRANSPORTATION NOVEMBER 1, 2014 AND SPECIFICATION ITEMS LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION CONTRACTS (FORM FHWA 1273, MAY 1, 2012).

DESIGN SPEED = 60 MPH CURRENT A.D.T. (2020) = 697 VPD PROJECTED A.D.T. (2040) = 976 VPD FUNCTIONAL CLASS = RURAL ARTERIAL EXISTING NBI# = 08-217-0-0106-04-043 PROPOSED NBI# = 08-217-0-0106-04-057

PROJECT No. BR 2022 (829) STATE STATE DIST. COUNTY TEXAS ABILENE STONEWALL CONTROL SECTION JOB HIGHWAY NO. 0106 04 036 US 380

#### FINAL PLANS

LETTING DATE: AUGUST 2022 DATE CONTRACTOR BEGAN WORK: DATE WORK WAS COMPLETED: DATE WORK WAS ACCEPTED: FINAL CONTRACT COST: \$ CONTRACTOR:

#### CERTIFICATION FOR FINAL PLANS

THIS PROJECT WAS BUILT ACCORDING TO THE PLANS AND SPECIFICATIONS. THESE FINAL PLANS REFLECT THE WORK DONE AND THE QUANTITIES SHOWN THEREON AND ON THE FINAL ESTIMATE ARE FINAL QUANTITIES.

AREA ENGINEER

DATE

THE DISTRICT TRAFFIC SAFETY COMMITTEE HAS REVIEWED THE TRAFFIC CONTROL PLAN FOR THIS PROJECT AND IT IS IN COMPLIANCE WITH CURRENT TRAFFIC CONTROL STANDARDS.

-DocuSigned by:

Casey McGes SAMMITITEE CHAIRMAN

5/31/2022 DATE

Texas Department of Transportation

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> RECOMMENDED FOR LETTING: 5/31/2022 -40878C8790864A9! CHAPMAN, P.E. AREA ENGINEER

SUBMITTED FOR LETTING: 05/16/2022

> JENELLE ROMERO, P.E. ATKINS PROJECT MANAGER

RECOMMENDED FOR LETTING: 5/31/2022

DocuSigned by:

-- DDD6B1閏ABEE74€€L MOORE

T×DOT PROJECT MANAGER

RECOMMENDED FOR LETTING: 6/3/2022

DocuSigned by:

Michael Haithcock

-5757₩288Ħ®884FB... HAITHCOCK, P.E. DIRECTOR OF T P & D

APPROVED FOR LETTING: 6/3/2022

DocuSigned by:

-OF6F7世切的記句430...ALLBRITTON, P.E.

DISTRICT ENGINEER

#### INDEX OF SHEETS

	GENERAL	80	BRIDGE
1	TITLE SHEET	81 - 84	STRUCTURE ID DETAILS SIDD-14 BRIDGE LAYOUT
2	INDEX OF SHEETS	85 - 86	TYPICAL SECTION
. 3 _	PROJECT LAYOUT	87 - 92	BORING LOGS
4 - 5	TYPICAL SECTIONS	93	ESTIMATED QUANTITY AND BEARING SEAT ELEVATIONS
6 - 11	GENERAL NOTES	93	CONCRETE WATERPROOFING DETAIL
12 - 12B	ESTIMATE & QUANTITY SHEET	95 - 96	
13	QUANTITY SUMMARIES	97 - 98	ABUTMENT 1 ABUTMENT 11
14	BRIDGE SUMMARY	99	INTERIOR BENTS 2 AND 3
		100	INTERIOR BENT 4
	TRAFFIC CONTROL PLAN	101	INTERIOR BENTS 5 TO 10
15	TRAFFIC CONTROL SEQUENCE OF WORK	102	FRAMING PLAN (SPAN 1)
16 - 17	TRAFFIC CONTROL PLAN TYPICAL SECTIONS	103	FRAMING PLAN (SPANS 2-3)
18 - 19	HAUL ROAD SEQUENCE OF WORK	104	FRAMING PLAN (SPANS 4-6)
20	CRASH CUSHION SUMMARY SHEET	105	FRAMING PLAN (SPANS 7-8)
21	TREATMENT FOR VARIOUS EDGE CONDITIONS	106	FRAMING PLAN (SPANS 9-10)
22	TEMPORARY SUMMARY OF SMALL SIGNS	107 - 108	145.000' PRESTRESSED CONCRETE I-GIRDER UNIT
		109 - 110	295.000' PRESTRESSED CONCRETE I-GIRDER UNIT
	TRAFFIC CONTROL PLAN STANDARDS	111 - 112	360.000' PRESTRESSED CONCRETE I-GIRDER UNIT
23 - 34	#BC(1)-21 TO BC(12)-21	113 - 114	210,000' PRESTRESSED CONCRETE I-GIRDER UNIT
35	#TCP (S-1) -08A	115 - 116	210.000' PRESTRESSED CONCRETE I-GIRDER UNIT
36	#TCP (S-2) -08A	117	IGND
37	#WZ (TD) -17	118	CSAB (MOD)
38	#WZ (STPM) -13		
39	#WZ (BRK) -13		STRUCTURAL STANDARDS
40 41	#WZ(UL)-13 #WZ(RS)-16	119	##BAS-A
42	#TCP(1-1)-18	120 - 121	##FD
43	#TCP (1-2)-18	122 - 123	##IGD
44	#TCP (2-1) -18	124 - 126	##IGEB
45	#TCP (2-2) -18	127 - 128	##IGMS
46	#TCP (2-8) -18	129	##IGSK
47	#TCP (3-1) -13	130	##IGTS
48	#TCP (3-3)-14	131 - 132	##MEBR (C)
49 - 50	#SSCB(2)-10	133 - 136	##PCP ##PCP-FAB
51	#SSCB(5)-10	137 138 - 139	##PCP-FAB ##PCP(0)
52	#ABSORB (M) -19	140 - 141	##PCP(O) -FAB
53	#SLED-19	140 - 141	##PMDF
54 - 55	#CATCB(1)-17	144	##SEJ-M
		145 - 146	##SRR
	ROADWAY DETAILS	147 - 149	##T223
56 - 57	HORIZONTAL & VERTICAL SURVEY CONTROL SHEET	141 143	1223
58	HORIZONTAL ALIGNMENT DATA		TRAFFIC ITEMS
59 - 61	PLAN & PROFILE	150	SUMMARY OF SMALL SIGNS
		151 - 152	SIGNING & PAVEMENT MARKING LAYOUT
	ROADWAY DETAIL STANDARDS	153	SIGN DETAILS
62	#GF (31) -19		
63 - 64	#GF (31) TRTL3-20		TRAFFIC ITEM STANDARDS
65	#GF (31) MS-19	154	#D&OM(1)-20
66	#SGT (11S) 31-18	155	#D&OM(2)-20
67 68	#SGT (15) 31 - 20 #BED-14	156	#D&OM(3)-20
69	#CCCG-21	157	#D&OM(4)-20
70	#TE (HMAC) -11	158	#D&OM(5) -20
10	"TE VIMAO! II	159	#D&OM(VIA)-20
	UTILITIES	160	#TSR (3)-13
71	EXISTING UTILITY NOTES	161	#PM(1) -20 #PM(2) - 20
72 - 74	EXISTING GITELTY NOTES  EXISTING UTILITY LAYOUT	162	#PM(2)-20
12 11	ENIONINO CHIEFFI ENIOCI	163 164	#RS (3) -13 #BS (4) -13
	DRAINAGE DETAILS	165	#RS(4)-13 #SMD(GEN)-08
75	BRIDGE HYDROLOGIC DATA	166	#SMD(GEN)-08
76 - 78	BRIDGE HYDRAULIC DATA	167	#SMD(SLIP-2)-08
79	SCOUR ANALYSIS	168	#SMD(SLIP-3)-08
		100	5.115 (5E2) 57 00

	ENVIRONMENTAL ISSUES
169 - 170	SW3P SITE PLAN
171 - 172	STORM WATER POLLUTION PREVENTION PLAN (SW3P)
173	SW3P NOTIFICATION BOARD DETAIL
174	EPIC
175	TREE AND BRUSH REMOVAL (ABL)
	ENVIRONMENTAL ISSUES STANDARDS
176	#EC(1)-16
177	#EC(2)-16
178	#EC(3)-16
178 - 181	#FC(9)-16



# THE STANDARD SHEETS SPECIFICALLY IDENTIFIED HAVE BEEN SELECTED BY ME OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT



## THE STANDARD SHEETS SPECIFICALLY IDENTIFIED HAVE BEEN SELECTED BY ME OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT

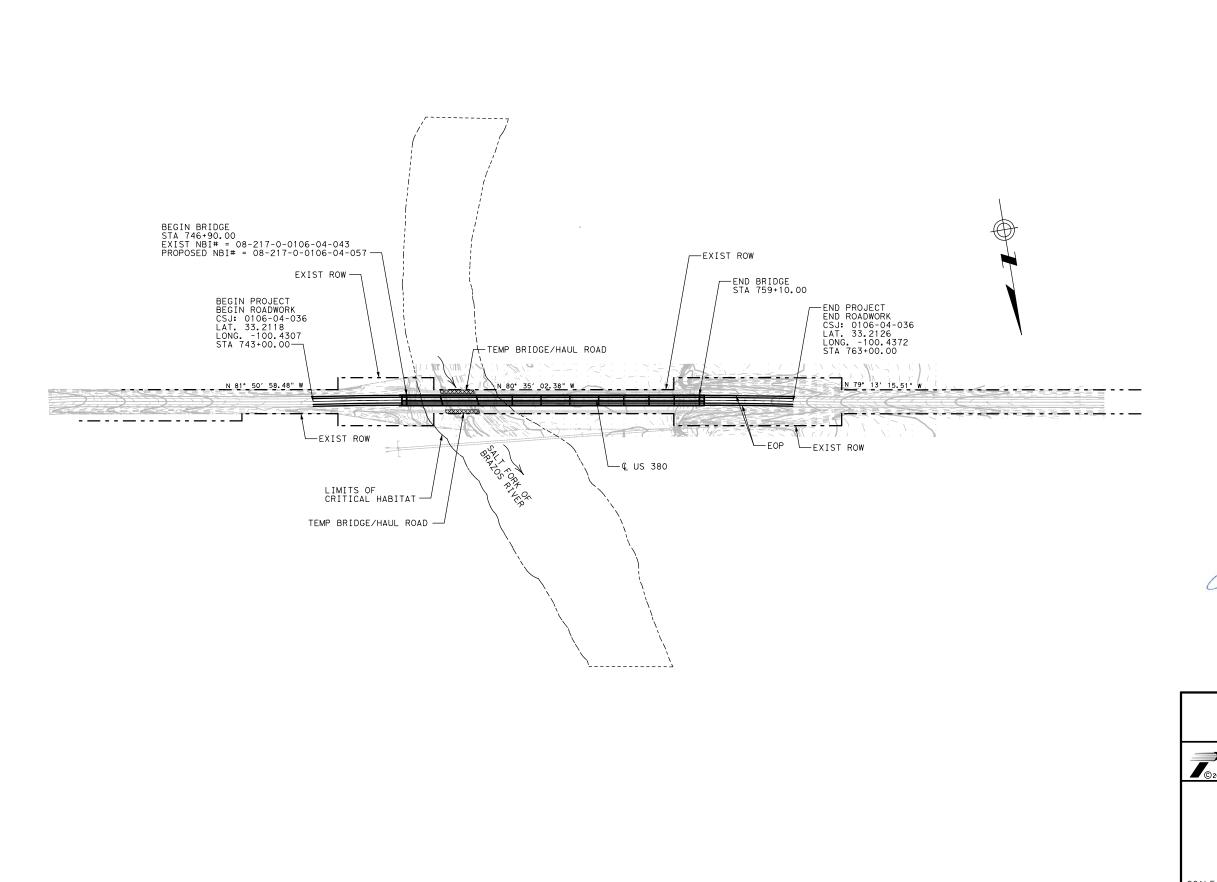


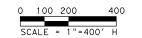
US 380

INDEX OF SHEETS

SHEET 1 OF 1

SIGNED	:SG	FED. RD DIV. No.	STATE	PROJECT No.				HIGHWAY No.		
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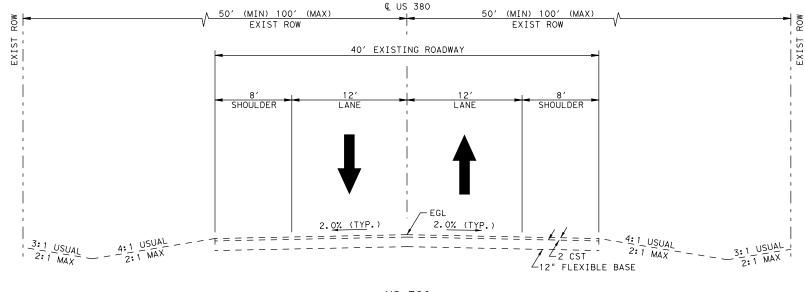


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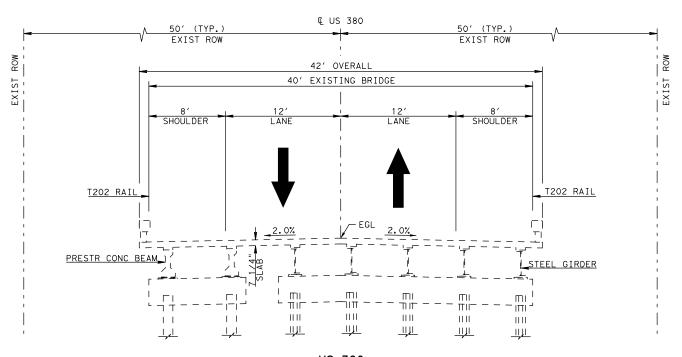


US 380 PROJECT LAYOUT

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US 380 EXISTING ROADWAY TYPICAL STA 743+00.00 TO STA 747+00.00 STA 758+97.00 TO STA 763+00.00



US 380 EXISTING ROADWAY TYPICAL STA 747+00.00 TO STA 758+97.00



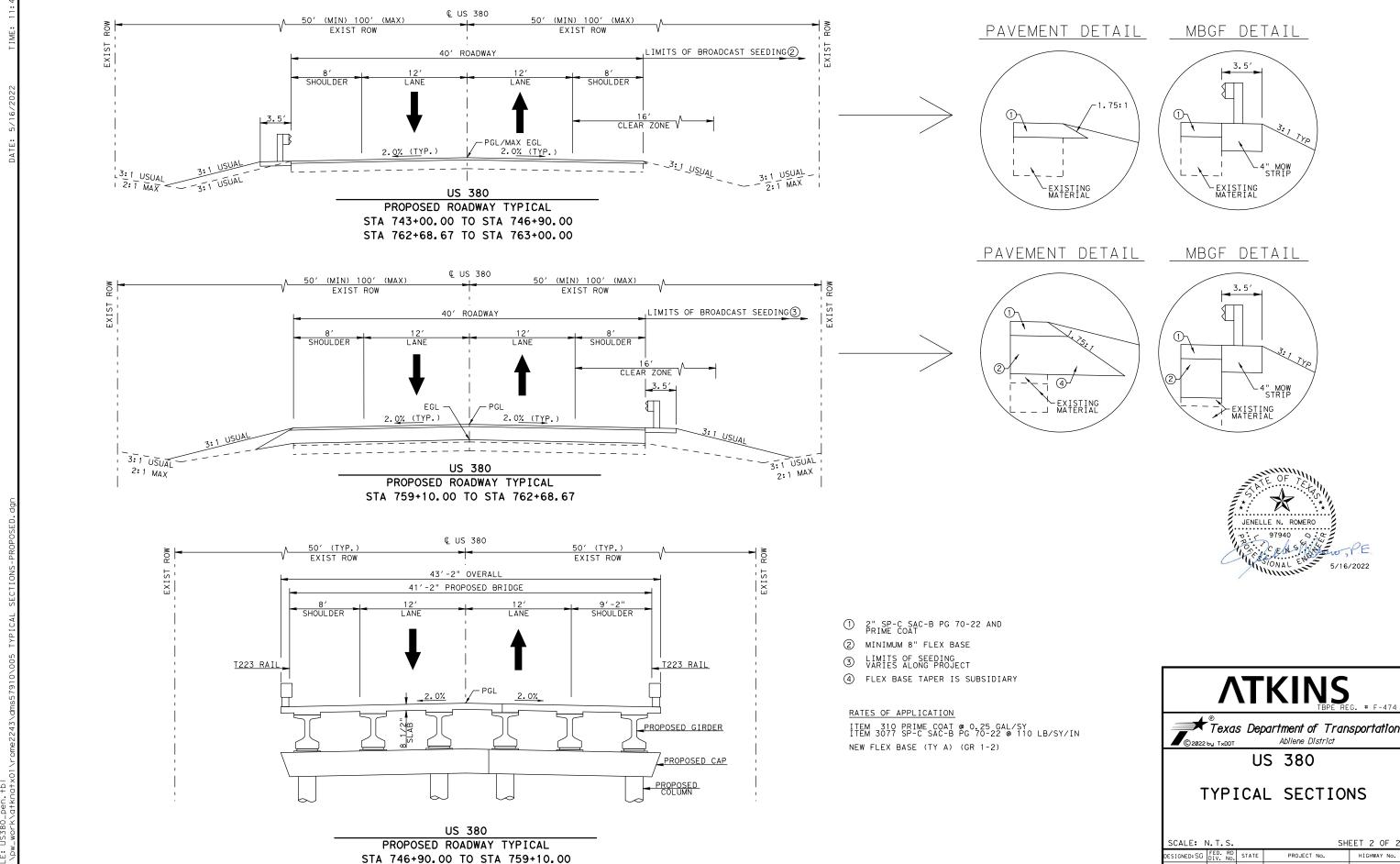




US 380

TYPICAL SECTIONS

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COUNTY

County: Stonewall & Kent Highway: US 380 & SH 208

#### ABILENE DISTRICT GENERAL NOTES **2014 SPECIFICATIONS**

#### General

Contractor questions on this project are to be addressed to the following individual(s):

Stewart Chapman, P.E.: Stewart.Chapman@txdot.gov Maxie Allen, P.E.: Maxie.Allen@txdot.gov (Snyder Area Office)

Contractor questions will be accepted through email, phone, and in person by the above individuals.

All contractor questions will be reviewed by the Engineer. Once a response is developed, it will be posted to TxDOT's Public FTP at the following Address: https://ftp.dot.state.tx.us/pub/txdot-info/Pre-Letting%20Responses/

All questions submitted that generate a response will be posted through this site.

The site is organized by:

District

Project Type (Construction or Maintenance)

Letting Date

CCSJ/Project Name.

Modified Standards: CSAB (MOD) FD(MOD)

Failure to make necessary corrections to SW3P based on SW3P inspections will be cause for withholding the monthly estimate until such corrections have been made.

Failure to make necessary corrections to traffic control items based on barricade inspections will be cause for withholding the monthly estimate until such corrections have been made.

Provide ingress/egress to the adjacent properties in areas under construction. Phased construction of driveways and streets shall be required to provide uninterrupted access to adjacent properties. Coordinate work with the property owners before beginning any construction in the vicinity of the drive.

Cut neat, straight lines with vertical faces along pavement edges or along joints between existing asphalt or concrete pavement and new pavement perpendicular or parallel to the direction of traffic by methods described in applicable bid items, or as directed. Provide clean edges or joints **Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent **Highway:** US 380 & SH 208

without jagged appearance or chunks broken out. This work is considered subsidiary to various

The Contractor must submit a demolition plan.

Construction will not be allowed at the same time on SH 208 and US 380 without approval of the Engineer.

#### **Environmental**

#### **Endangered and Protected Species**

- 1. Migratory Birds
  - a. Bird nesting season is typically 15Feb through 15Sep annually.
  - b. The Contractor will avoid disturbing, destroying, removing, or relocating migratory birds and active nests found in trees, culverts, bridges, on the ground, or anywhere they are encountered.
  - c. Perform all tree trimming and other vegetation clearing activities during the nonbreeding season (typically 15Sep-15Feb annually). Perform any inactive nest removal and bird exclusion methods to prevent birds from establishing nests. Phasing of work during construction may be necessary to stay in compliance.
  - d. When active nests are unexpectedly encountered on-site during construction, the Contractor will stop work and immediately notify the Engineer. Take measures to avoid disturbance of these birds, their occupied nest, eggs, and/or young, in accordance with the Migratory Bird Treaty Act, Texas Parks and Wildlife Code, and TxDOT policy.
  - e. The Engineer will notify the Contractor when work may resume.
  - f. The Contractor should be prepared to prevent migratory birds from building nests by utilizing nest prevention methods, such as bird-deterrent netting and birdrepelling sprays and/or gels, between 15Feb and 15Sep. The Contractor can discuss other preventative measures with the Engineer and/or District Environmental Staff.

#### **Best Management Practices**

- 1. Comply with the SW3P and associated sheets.
- 2. Construction Schedule
  - a. The construction activity schedule is designed to minimize potential impacts to endangered aquatic species. Peak spawning season is Apr – Sep. Work in the water should be avoided during this time. Adhere to the schedule to the maximum extent possible.
  - b. Work will be avoided in the wetted channel during peak spawning season, and especially during flood events. Flow will be monitored with the closest gauge to the Project area with real-time stream data available. Work in the wetted channel

General Notes General Notes Sheet A Sheet B





Texas Department of Transportation Abilene District

US 380

GENERAL NOTES

SHEET 1 OF

ESIGNED	:SG	DIV. No.	STATE		PROJE	CT No.		HIC	SHWAY No.
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HECKED:	JNR	ABL	STONEWA	٩LL	0106	04	0	36	6

County: Stonewall & Kent Highway: US 380 & SH 208

> will be halted if precipitation events in the basin result in an increase in gauge height of 2' or more over the monthly median.

#### 3. Construction Access and Staging

- a. The Critical Habitat Unit (CHU) will be delineated with temporary fencing and
- b. No PSL, laydown area, washout area, portable sanitary facilities, or any other storage area will be placed within 100' of the CHU.
- c. Work within the CHU will be limited to the minimum necessary to complete the
- d. Work in the water should be avoided unless necessary.
- e. Access to the CHU will limited to a temporary haul road from both sides of the bridge. Temporary bridges will be constructed to span the wetted channel; one bridge is allowed on each side of US 380, and one bridge is allowed only on the west side of SH 208. The temporary access bridges will allow access for bridge removal and reconstruction activities. The temporary bridge structures will be supported by up to 4 driven pilings each within the wetted channel. Additional support pilings may be placed within the CHU and outside of the wetted channel.

#### 4. Construction

- a. Work within the CHU will be minimized to that necessary to remove the existing bridge and construct the new bridge.
- b. Any activity involving dewatering (e.g., cofferdams) will need to be cleared by a permitted biologist, supplied by TxDOT. This activity involves clearing endangered fish from the impact area and monitoring the work to ensure aquatic organisms are not impacted by the construction/demolition work.
- c. Access to the bridge from below within the CHU will be limited to the use of the temporary haul road/bridge. Minimize activities off the temporary haul road/bridge to only necessary activities.
- d. When access to under the bridge is no longer required, remove the temporary haul road/bridge and restore the are to pre-construction conditions.
- e. Use district designated seed mix for erosion control and re-seeding.
- f. If erosion control blankets are used on sloped surfaces, use wildlife friendly products from the Approved Product List as designated by the Engineer.
- g. Silt fences and BIOLOGS are to be used in tandem for erosion control, sediment control, and protection of protected fish.
- h. BIOLOGS are to be left in place post construction (TxDOT to remove after one growing season). Silt fence will be removed once post construction erosion control is in place.

#### 5. Bird BMPs

- a. Not disturbing, destroying, or removing active nests, including ground nesting birds, during the nesting season;
- b. Avoiding the removal of unoccupied, inactive nests, as practicable;

**Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent Highway: US 380 & SH 208

- c. Preventing the establishment of active nests during the nesting season on TxDOT owned and operated facilities and structures proposed for replacement or repair;
- d. Not collecting, capturing, relocating, or transporting birds, eggs, young, or active nests without a permit.
- 6. Other Best Management Practices for State Protected Species
  - a. If Black Tailed Prairie Dog (BTPD) burrows or pocket gopher mounds are found near or within the project area, place barrier fencing to discourage the individual animals of moving into or through the construction area.
  - b. While seeding or revegetating, if BTPD or pocket gopher mounds are discovered near or within the planned area, a vegetative barrier should be planted to discourage the dispersal of the species within the TxDOT ROW.
  - c. If animals are within the project area, avoid harming when encountered. Let them leave the area without harassment.
  - d. Avoid or minimize disturbing or removing downed trees, rotting stumps, and leaf litter which may be refugia for terrestrial reptile or amphibians, where feasible.
  - e. For open trenches and excavated pits, install escape ramps at an angle of less than 45 degrees (1:1) in areas left uncovered. Visually inspect excavation areas for the presence of wildlife prior to backfilling.
  - Rubbish found near bridges on TxDOT ROW should be removed and disposed of properly to minimize the risk of pollution. Rubbish does not include brush piles or snags.

#### Item 5, "Control of Work"

Use Method C for construction surveying.

All known utilities are identified in the plans, including the crossing of power lines. Use this information to identify potential issues with power poles and power lines prior to bidding. Make necessary arrangements with utility owners regarding temporary protections such as bracing power poles, and de-energizing power lines. The Department will not reimburse the cost of such temporary protections to the Contractor, unless the Engineer determines that inadequate information was available at the time the project was bid. "Call Before You Dig" "Call 811"

Provide notification to the Traffic Engineering Section by telephone at 325-676-6991 and by email at ABL-TrafficFix@txdot.gov when planning drilling or excavation work in areas where existing TxDOT underground utilities exist. Visual evidence of TxDOT underground utilities in the area include illumination poles, ground boxes, flashing beacons, traffic signals, etc. This notification must be provided 72 hours in advance of performing the work.

Drilled shaft locations or excavation areas must be staked prior to the notification so that the underground utilities can be located in relationship to the proposed work. Preserve and document the marked utility locations to prevent unnecessary secondary notifications. Notify the Engineer of conflicts between proposed work and underground utilities.

General Notes Sheet C General Notes Sheet D





US 380

GENERAL NOTES

SHEET 2 OF 6

SIGNED	:SG	DIV. No.	STATE	PROJE	CT No.		HIC	SHWAY No.	ı
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ECKED:	JNR	ABL	STONEWALI	0106	04	0	36	7	l

County: Stonewall & Kent Highway: US 380 & SH 208

Obtain approval from the Engineer of staked locations for illumination foundations, pull boxes, and power source prior to construction.

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/inside-txdot/formspublications/consultants-contractors/publications/bridge.html#design. Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

#### Item 7, "Legal Relations and Responsibilities"

The total area disturbed for this project is 2.23 acres. The disturbed area in this project, all project locations in the Contract, and the Contractor project specific locations (PSLs), within 1 mile of the project limits, for the Contract will further establish the authorization requirements for storm water discharges. The Department will obtain an authorization to discharge storm water from the Texas Commission on Environmental Quality (TCEQ) for the construction activities shown on the plans. The Contractor is to obtain required authorization from the TCEQ for Contractor PSLs for construction support activities on or off the ROW. When the total area disturbed in the Contract and PSLs within 1 mile of the project limits exceeds 5 acres, provide a copy of the Contractor NOI for PSLs on the ROW to the Engineer and to the government that operates a separate storm sewer system.

Provide one SW3P Notification Board for this project. Notification Boards are to be placed at locations within the right-of-way but outside the clear zone as directed by the Engineer. Consider this work to be subsidiary to the various bid items of the contract. No significant traffic generator events identified.

Hard hats are required at all times during construction when construction personnel are in TxDOT Right-of-Way.

#### **Item 8 "Prosecution and Progress"**

Each contract awarded by the Department stands on its own and as such, is separate from other contracts. A Contractor awarded multiple contracts must be capable and sufficiently staffed to concurrently process and/or execute all contracts at the same time.

The Contractor is hereby authorized to begin work prior to the expiration of the number of calendar days provided in the Special Provision to Item 8, Article 8.1. Notify the Engineer in writing of the date to begin work. Time charges will commence when work begins or on the expiration of the number of calendar days provided, whichever occurs first.

Coordinate and update the work schedule with the project inspector daily. Give a minimum of 24 hours of notice to project inspector if work requiring inspection or testing is to be performed. Failure to do so may cause that work to be delayed or postponed if TxDOT personnel are not

**Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent Highway: US 380 & SH 208

available. Work performed without suitable inspection, as determined by the Engineer, may be ordered removed and replaced at Contractor's expense.

In accordance with SP-000-658, liquidated damages will be increased by \$8,588 per working

#### Item 9, "Measurement and Payment"

The progress payment period shall end on the 25th of each month, unless directed by the Area Office Engineer. Material on Hand (MOH) is due two business days before estimate cut off.

#### Item 100, "Preparing Right of Way"

The Contractor's attention is directed to potential regulations against burning within the project limits. Abide by all local ordinances and county imposed burn bans. When burning is prohibited, dispose of material in accordance with regulations set forth by other regulatory agencies including the Texas Commission for Environmental Quality. The cost of burning or disposal of any product is subsidiary to various bid items.

#### Item 164, "Seed for Erosion Control"

Quantities shown are approximate; limits of the temporary and permanent seeding will be determined during construction.

#### Item 168, "Vegetation Watering"

Water rate for this project shall be 1/4" of water per acre every two weeks for a 3-month period.

#### Item 420, "Concrete Substructures"

The following elements are Plans Quantity Elements.

- Abutment
- Cap
- Column

#### Item 432, "Riprap"

Provide structural fiber reinforced or conventionally reinforced concrete for formed M.B.G.F. concrete mow strip.

Meet the following requirements when using structural fiber reinforcement:

• If slip forming, use an approved method that ensures adequate concrete consolidation. Sprinkle and consolidate the subgrade before the concrete is placed. Finish the surface with a wood float or broom finish as approved. Immediately after finishing operation, cure the riprap according to Item 420, "Concrete Structures".

#### Item 502, "Barricades, Signs and Traffic Handling"

Mobile traffic control in accordance with TPC 3 series will be required for placement of short duration, short term, intermediate term, and long-term traffic control.

General Notes General Notes Sheet E Sheet F

# ΛΤΚΙΝS



Abilene District

US 380

GENERAL NOTES

SHEET 3 OF

SIGNED:SC	DIV. No.	STATE	PROJE	CT No.		HIC	SHWAY No.
ECKED: JNF	₹ 6	TEXAS	SEE TIT	LE SHE	ΕT	U	S 380
awn: SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		0B o.	SHEET No.
ECKED: JNF	ABL	STONEWALL	0106	04	0	36	8

County: Stonewall & Kent Highway: US 380 & SH 208

Provide the Engineer with written notification seven (7) days in advance of major traffic changes. A major traffic change is defined as the temporary (greater than one day) or permanent relocation of traffic lanes typically in an urban setting. The notice will, at a minimum, include the expected date, time and scope of the traffic change. The Department will utilize the information provided to inform the traveling public of the changes. Failure to provide advance notice, or to provide accurate information, will result in delaying the work until such time that the public has been notified.

Additional signs, barricades and traffic handling may be necessary to complete the work shown herein and will be provided by the contractor as required and will be considered subsidiary to this item.

Provide separate attenuators for each work area within a common lane closure as approved or directed by the Engineer.

In sections where traffic is restricted to one lane, two-way traffic, flaggers will be stationed at each end of that section with two-way communication devices and a pilot car will control operations.

Relocate existing roadside signs to temporary supports as approved by the engineer.

All safety appurtenances such as signs, delineators, object markers and route markers will be in place prior to opening each phase of the construction to traffic, unless otherwise directed.

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

The Contractor's person responsible for TCP compliance must be available by local telephone and have a response time within 45 minutes.

Work will not be allowed on both sides of the roadbed at the same time.

Equip all work vehicles within 30 feet of the traveled way with a functioning amber strobe light or rotating beacon visible from all directions.

Repair barricades within the timeline shown on the barricade inspection report. Failure to comply will cease all work until barricades are repaired to the satisfaction of the Department. **Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent Highway: US 380 & SH 208

Replace all damaged traffic control devices immediately. Remove any damaged traffic control devices from the project within 24 hours.

Conflicting guide signs shall be covered as approved by the Engineer.

Pilot car is subsidiary to item 502.

Reduced regulatory speed limit signs should only be posted in the vicinity of ongoing work activity as shown on BC (3)-14 and not throughout the entire project. Removing, relocating or covering speed limit signs shall be considered subsidiary to item 502.

#### Item 504, "Field Office for Laboratory"

#### Field Laboratory:

Furnish a "Type D" structure for the asphalt mix control laboratory for the Engineer's exclusive use. In addition to the requirements of Item 504, furniture and equipment to be furnished by the Contractor shall include:

- eye wash station
- first-aid kit
- two fire extinguishers
- Provide internet connectivity for use by TxDOT lab testing personnel at all laboratory structures on this project.

#### Item 510, "One-way Traffic Control"

The contractor shall use ADDCO PTS-2000 or equivalent, that shall show wait time, as temporary traffic signals. Two (2) temporary traffic signals will be required for this project.

#### Item 512, "Portable Concrete Traffic Barrier

The contractor shall furnish new single slope portable concrete traffic barrier (PCTB) sections using SSCB(2)-10 standard.

Quick-Bolt (SSCB)Type X Joint installation shall be used.

Upon completion of the project, PCTB will become the property of the TxDOT and will be stockpiled as approved by the Engineer at the intersection of US 380 and SH 283, near the community of Old Glory in Stonewall County, approx. 24 miles from the project limits.

#### Item 533, "Milled Rumble Strips"

The milled rumble strips should be placed on shoulder according to rs(1-4)-13 standards and the shoulder widths as shown below.

- Shoulder width of greater than 6 feet the rumble strip will begin 2 feet from the edge line.
- Or as directed by the engineer

General Notes General Notes Sheet G Sheet H





Texas Department of Transportation Abilene District

US 380

GENERAL NOTES

SHEET 4 OF

SIGNED	:SG	DIV. No.	STATE		PROJE	CT No.		HIC	SHWAY No.
ECKED:	JNR	6	TEXAS		SEE TIT	LE SHE	ΞT	υ	S 380
AWN:	SG	STATE DISTRICT	COUNTY		CONTROL No.	SECTION No.		0B o.	SHEET No.
ECKED:	JNR	ABL	STONEWAL	.L	0106	04	0	36	9

County: Stonewall & Kent Highway: US 380 & SH 208

#### Item 540, "Metal Beam Guard Fence"

Core drill 1 1/4 diameter holes through existing slab. Percussion or impact drilling is not permitted. Patch spalls, when directed by the engineer, in accordance with item 429, "Concrete Structure Repair", at the contractor's expense.

#### Item 542, "Removing Metal Beam Guard Fence"

All metal beam guard fence removed from the project will become property of the Contractor.

#### Item 585, "Ride Quality for Pavement Surfaces"

The Engineer reserves the right to prohibit corrective work and assess the penalty for each occurrence of localized roughness per Article 585.3.4.2.3.2.

Use pay adjustment schedule 2 (two) for Ride Quality bonus/penalty calculation.

#### Item 644, "Small Roadside Sign Supports and Assemblies"

Use the latest edition of the "Standard Highway Sign Designs for Texas" for Sign types for which design details are not shown on the plans.

Sign placement shall be in accordance with the latest edition of the TMUTCD & TxDOT's Sign Crew Field Book located at the following addresses.

TMUTCD - https://www.txdot.gov/business/resources/signage/tmutcd.html TxDOT's Sign Crew Field Book - http://onlinemanuals.txdot.gov/txdotmanuals/sfb/index.htm

Before final sign installation, stake all sign locations for approval by the engineer.

All triangle slip base small sign mounts installed under this item shall utilize clamp type bases.

Remove entire small sign foundation.

Deliver and stockpile all signs to be salvaged to the Kent County maintenance yard in Jayton.

#### Item 658, "Delineator and Object Marker Assemblies"

All MBGF delineation shall be GF2 mounted on posts.

Use a minimum 2 inch long lag screws with washers to attach flexible GF2 barrier reflectors to wooden post. For steel posts, use an approved adhesive, or other method approved by Engineer.

Concrete Barrier Reflectors shall be equivalent to Shure-tite CTB "Cup Mount" Delineator (8"). Attach delineators to concrete rail with concrete anchors as approved by the Engineer.

#### Item 662, "Work Zone Pavement Markings"

Dispose of tabs and paper in an approved trash receptacle. (Reference Standard SW3P, waste material)

General Notes

Use traffic paint for non-removable work zone pavement markings.

**Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent Highway: US 380 & SH 208

#### Item 666, "Retro reflectorized Pavement Markings"

Provide a complete system of thermoplastic pavement markings at locations indicated on the plans and as directed by the engineer. The plans are intended to show typical conditions, which can be extended to similar conditions throughout this project as approved or directed.

All longitudinal pavement markings (including profile pavement markings) must meet minimum retro reflectivity requirements.

Contractor is responsible for re-establishing location and alignment for new pavement markings matching pavement marking alignment prior to construction activities. This work will be considered subsidiary.

#### Item 672, "Raised Pavement Markers"

Provide a complete system of raised pavement markers at locations indicated on the plans and as directed by the engineer. The plans are intended to show typical conditions, which can be extended to similar conditions throughout this project as approved or directed.

Bituminous adhesive shall be used on this project.

#### Item 677, "Eliminating Existing Pavement Markings and Markers"

Remove the existing raised pavement markings (RPMs) and profile pavement markings as the work progresses, or as directed by the Engineer. Removal methods shall be approved by the Engineer. Properly dispose of materials removed. Removal of existing profile pavement markings will be paid for directly. Removal of RPMs will not be paid for directly but will be subsidiary to the pertinent bid items.

#### Item 3077, "Superpave Mixtures"

Furnish aggregate for final surfaces with a minimum surface aggregate classification of "B".

Provide an SP-C Fine Mixture with a minimum design VMA of 16.0% and a minimum plantproduced VMA of 15.5%.

The Engineer reserves the right to test all sources even if the source is listed in the Bituminous Source Rated Quality Catalog.

Provide the testing lab samples to calibrate the ignition oven no later than five (5) working days prior to mix design verification.

Meet the minimum Hamburg Wheel Test requirements shown below:

- PG 64 or lower 5,000 passes
- PG 70 10,000 passes
- PG 76 20,000 passes

Sheet I

General Notes Sheet J





Abilene District

US 380

GENERAL NOTES

SHEET 5 OF

SIGNED	:SG	DIV. No.	STATE		PROJE	CT No.		HIG	HWAY No.	
HECKED:	JNR	6	TEXAS		SEE TIT	LE SHEE	ΞT	U	S 380	l
RAWN:	SG	STATE DISTRICT	COUNT	′	CONTROL No.	SECTION No.		ов •	SHEET No.	l
HECKED:	JNR	ABL	STONEWA	۱LL	0106	04	0	36	10	l

County: Stonewall & Kent Highway: US 380 & SH 208

Paving operations will not be allowed to begin until TxDOT has tested and obtained passing Hamburg results on the trial batch.

A maximum of 0.50% anti-stripping agent will be allowed for each specified mix type.

Dilution of tack coat is not allowed.

Do not exceed a laydown width of 16' per pass.

Substitute Binders will not be allowed unless RAP or RAS is used in the production of the

RAS will not be allowed in surface mixes.

A warm mix additive will be required for hotmix hauls over 50 miles.

Unless otherwise directed by the engineer, a warm mix additive will be required when paving during November 1<sup>st</sup> through March 15<sup>th</sup>.

The maximum allowable dust / asphalt ratio that will be allowed is 0.6 to 1.2.

The use of a tapered longitudinal joint will be required for pavement thicker than 2 inches.

Use a self-propelled, wheel-mounted material transfer vehicle (MTV) capable of receiving hot mix from the haul trucks separate from the paver on this project. Minimum requirements for the MTV are a storage capacity of approximately 25 tons, a pivoting discharge conveyor, and a means of completely remixing the ACP prior to placement.

Provide PG 64-22 tack coat at a rate of 0.15 gal/sy.

The Contractor will be required to tack 100% of the surfaces with uniform coverage prior to the subsequent lift. The type and grade of tack will be approved by the Engineer prior to use.

Tack all vertical joints unless otherwise directed.

Cement and kiln dust will not be allowed to be used as mineral fillers.

Shoulders shall not be placed prior to adjoining main lanes.

Final surface of driveway shall not be placed prior to adjoining surface.

#### Item 6185, "Truck Mounted Attenuator (TMA) and Trailer Attenuator (TA)"

Truck Mounted Attenuator (TMA) and Trailer Attenuator (TA) will not be considered a major item of work on this project.

TMA's will only be paid while workers are present or to protect a blunt object.

**Project Number:** BR 2022 (829) **Control:** 0106-04-036 & 2011-02-015

County: Stonewall & Kent **Highway:** US 380 & SH 208

BASIS OF ESTIMATE FOR STATIONARY TMAS										
TMA (Stationary)										
Phase	Standard	Required	Additional	TOTAL						
III	TCP 2-2	2		2						
Basis of	Estimate for Mobil	e TMAs								
		TMA (Mo	bile)							
Phase	Standard	Required	Additional	TOTAL						
III	TCP 3-1	2		2						
	101 5 1	_								

The contractor will be responsible for determining if one or more of these operations will be ongoing at the same time to determine the total number of TMAs needed for the project. The Contractor must get approval from the Engineer for any changes in the number of TMA as shown in the plans.

If a TMA is used for both mobile and stationary traffic control on the same day, it will be paid for as stationary for that day.





US 380

GENERAL NOTES

SIGNED	:SG	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.
ECKED:	JNR	6	TEXAS	SEE TIT	LE SHE	Ξ	U	S 380
AWN:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.
ECKED:	JNR	ABL	STONEWAL	L 0106	04	0	36	11

General Notes General Notes Sheet L Sheet K



# **Estimate & Quantity Sheet**

**CONTROLLING PROJECT ID** 0106-04-036

**DISTRICT** Abilene **HIGHWAY** SH 208, US 380

**COUNTY** Kent, Stonewall

Report Created On: May 23, 2022 2:06:32 PM

		CONTROL SECTION	N JOB	0106-04	4-036	2011-0	2-015		
		PROJ	ECT ID	A00129	9546	A0012	9545		
		Co	OUNTY	Stone	wall	Ker	nt	TOTAL EST.	TOTAL
		HIG	HWAY	US 3	80	SH 2	.08		FINAL
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL	EST.	FINAL		
	100-6002	PREPARING ROW	STA	8.000		15.000		23.000	
	105-6011	REMOVING STAB BASE AND ASPH PAV (2"-6")	SY			6,490.000		6,490.000	
	110-6001	EXCAVATION (ROADWAY)	CY	82.000		860.000		942.000	
	132-6004	EMBANKMENT (FINAL)(DENS CONT)(TY B)	CY	423.000		10,639.000		11,062.000	
	164-6002	BROADCAST SEED (PERM) (RURAL) (SANDY)	AC			1.260		1.260	
	164-6004	BROADCAST SEED (PERM) (RURAL) (CLAY)	AC	0.440				0.440	
	164-6010	BROADCAST SEED (TEMP) (WARM)	AC	0.220		0.630		0.850	
	164-6012	BROADCAST SEED (TEMP) (COOL)	AC	0.220		0.630		0.850	
	168-6001	VEGETATIVE WATERING	MG	17.800		51.400		69.200	
	247-6041	FL BS (CMP IN PLC)(TYA GR1-2)(FNAL POS)	CY	439.000		1,061.000		1,500.000	
	310-6009	PRIME COAT (MC-30)	GAL	874.000		1,592.000		2,466.000	
	354-6021	PLANE ASPH CONC PAV(0" TO 2")	SY	2,297.000				2,297.000	
	400-6005	CEM STABIL BKFL	CY	312.000		179.000		491.000	
	403-6001	TEMPORARY SPL SHORING	SF	1,700.000				1,700.000	
	416-6001	DRILL SHAFT (18 IN)	LF	120.000		182.000		302.000	
	416-6004	DRILL SHAFT (36 IN)	LF	80.000		384.000		464.000	
	416-6005	DRILL SHAFT (42 IN)	LF	1,582.000				1,582.000	
	416-6006	DRILL SHAFT (48 IN)	LF			930.000		930.000	
	420-6014	CL C CONC (ABUT)(HPC)	CY	82.000		59.000		141.000	
	420-6030	CL C CONC (CAP)(HPC)	CY	197.000		115.000		312.000	
	420-6038	CL C CONC (COLUMN)(HPC)	CY	91.000		60.000		151.000	
	422-6002	REINF CONC SLAB (HPC)	SF	52,664.000		23,142.000		75,806.000	
	422-6016	APPROACH SLAB (HPC)	CY	66.000		58.000		124.000	
	425-6038	PRESTR CONC GIRDER (TX46)	LF			2,282.000		2,282.000	
	425-6039	PRESTR CONC GIRDER (TX54)	LF	4,659.000				4,659.000	
	425-6041	PRESTR CONC GIRDER (TX70)	LF	2,631.000		747.000		3,378.000	
	427-6004	SILICONE RESIN PAINT FINISH	SF	4,823.000		2,918.000		7,741.000	
	432-6031	RIPRAP (STONE PROTECTION)(12 IN)	CY	292.000		2,794.000		3,086.000	
	432-6045	RIPRAP (MOW STRIP)(4 IN)	CY	43.100		75.600		118.700	
	450-6007	RAIL (TY T223)(HPC)	LF	2,522.000				2,522.000	
	450-6017	RAIL (TY T552)(HPC)	LF			1,282.000		1,282.000	
	454-6018	SEALED EXPANSION JOINT (4 IN) (SEJ - M)	LF	256.000		157.000		413.000	
	496-6010	REMOV STR (BRIDGE 100 - 499 FT LENGTH)	EA			1.000		1.000	
	496-6012	REMOV STR (BRIDGE 1000 FT OR GREATER)	EA	1.000				1.000	
	500-6001	MOBILIZATION	LS	1.000				1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	МО	13.000		11.000		24.000	
	506-6001	ROCK FILTER DAMS (INSTALL) (TY 1)	LF			60.000		60.000	



DISTRICT	COUNTY	CCSJ	SHEET		
Abilene	Stonewall	0106-04-036	12		



# **Estimate & Quantity Sheet**

CONTROLLING PROJECT ID 0106-04-036

**DISTRICT** Abilene **HIGHWAY** SH 208, US 380

**COUNTY** Kent, Stonewall

Report Created On: May 23, 2022 2:06:32 PM

		CONTROL SECTI	ON JOB	0106-04	-036	2011-02	-015		
		PRO	JECT ID	A00129	546	A00129	545		
			COUNTY	Stonev	<i>r</i> all	Ken	t	TOTAL EST.	TOTAL FINAL
		н	GHWAY	US 38	80	SH 20	08		
\LT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL	EST.	FINAL	1	
	506-6003	ROCK FILTER DAMS (INSTALL) (TY 3)	LF	80.000		80.000		160.000	
	506-6011	ROCK FILTER DAMS (REMOVE)	LF	80.000		140.000		220.000	
	506-6020	CONSTRUCTION EXITS (INSTALL) (TY 1)	SY	230.000		230.000		460.000	
	506-6024	CONSTRUCTION EXITS (REMOVE)	SY	230.000		230.000		460.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	200.000				200.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	200.000				200.000	
	506-6041	BIODEG EROSN CONT LOGS (INSTL) (12")	LF	1,050.000		1,700.000		2,750.000	
	506-6043	BIODEG EROSN CONT LOGS (REMOVE)	LF	1,050.000		1,700.000		2,750.000	
	510-6003	ONE-WAY TRAF CONT (PORT TRAF SIG)	МО	13.000				13.000	
	512-6001	PORT CTB (FUR & INST)(SGL SLOPE)(TY 1)	LF	1,420.000				1,420.000	
	512-6025	PORT CTB (MOVE)(SGL SLP)(TY 1)	LF	1,420.000				1,420.000	
	512-6037	PORT CTB (STKPL)(SGL SLP)(TY 1)	LF	1,420.000				1,420.000	
	533-6001	RUMBLE STRIPS (SHOULDER)	LF	1,360.000		2,784.000		4,144.000	
	533-6002	RUMBLE STRIPS (CENTERLINE)	LF	680.000		1,392.000		2,072.000	
	540-6001	MTL W-BEAM GD FEN (TIM POST)	LF	500.000		1,300.000		1,800.000	
	540-6006	MTL BEAM GD FEN TRANS (THRIE-BEAM)	EA	4.000		4.000		8.000	
	542-6004	RM MTL BM GD FENCE TRANS (THRIE-BEAM)	EA	4.000		4.000		8.000	
	544-6001	GUARDRAIL END TREATMENT (INSTALL)	EA	4.000		4.000		8.000	
	544-6003	GUARDRAIL END TREATMENT (REMOVE)	EA	4.000		4.000		8.000	
	545-6003	CRASH CUSH ATTEN (MOVE & RESET)	EA	2.000				2.000	
	545-6005	CRASH CUSH ATTEN (REMOVE)	EA	2.000				2.000	
	545-6019	CRASH CUSH ATTEN (INSTL)(S)(N)(TL3)	EA	2.000				2.000	
	552-6001	WIRE FENCE (TY A)	LF			2,175.000		2,175.000	
	552-6006	GATE (TY 2)	EA			2.000		2.000	
	644-6001	IN SM RD SN SUP&AM TY10BWG(1)SA(P)	EA			8.000		8.000	
	644-6004	IN SM RD SN SUP&AM TY10BWG(1)SA(T)	EA	4.000		2.000		6.000	
	644-6076	REMOVE SM RD SN SUP&AM	EA	4.000		10.000		14.000	
	658-6014	INSTL DEL ASSM (D-SW)SZ (BRF)CTB (BI)	EA	26.000		14.000		40.000	
	658-6062	INSTL DEL ASSM (D-SW)SZ 1(BRF)GF2(BI)	EA	8.000		15.000		23.000	
	662-6050	WK ZN PAV MRK REMOV (REFL) TY II-A-A	EA	190.000				190.000	
	662-6063	WK ZN PAV MRK REMOV (W)4"(SLD)	LF	5,680.000				5,680.000	
	662-6075	WK ZN PAV MRK REMOV (W)24"(SLD)	LF	80.000				80.000	
	662-6095	WK ZN PAV MRK REMOV (Y)4"(SLD)	LF	7,600.000				7,600.000	
	666-6224	PAVEMENT SEALER 4"	LF	3,120.000		5,964.000		9,084.000	
	666-6303	RE PM W/RET REQ TY I (W)4"(SLD)(100MIL)	LF	4,000.000		4,200.000		8,200.000	
	666-6315	RE PM W/RET REQ TY I (Y)4"(SLD)(100MIL)	LF	4,000.000		4,200.000		8,200.000	
	672-6009	REFL PAV MRKR TY II-A-A	EA	50.000		53.000		103.000	



DISTRICT	COUNTY	CCSJ	SHEET
Abilene	Stonewall	0106-04-036	12A



# **Estimate & Quantity Sheet**

**CONTROLLING PROJECT ID** 0106-04-036

**DISTRICT** Abilene HIGHWAY SH 208, US 380

**COUNTY** Kent, Stonewall

Report Created On: May 23, 2022 2:06:32 PM

		CONTROL SECTI	0106-04	4-036	2011-0	2-015			
		PRO	A00129	9546	A0012	9545			
	COUNTY				wall	Ker	nt	TOTAL EST.	TOTAL FINAL
	HIGHWAY		US 3	80	SH 2	08			
ALT	BID CODE	DESCRIPTION	UNIT		FINAL	EST.	FINAL		
	677-6001	ELIM EXT PAV MRK & MRKS (4")	LF	13,280.000				13,280.000	
	677-6007	ELIM EXT PAV MRK & MRKS (24")	LF	80.000				80.000	
	678-6001	PAV SURF PREP FOR MRK (4")	LF	4,880.000		2,436.000		7,316.000	
	3077-6023	SP MIXESSP-CSAC-B PG70-22	TON	385.000		1,401.000		1,786.000	
	4027-6001	TEMP CONSTRUCTION ACCESS	LS	2.000		1.000		3.000	
	6001-6002	PORTABLE CHANGEABLE MESSAGE SIGN	EA	2.000		2.000		4.000	
	6185-6002	TMA (STATIONARY)	DAY	4.000				4.000	
	6185-6005	TMA (MOBILE OPERATION)	DAY	4.000				4.000	



DISTRICT	COUNTY	CCSJ	SHEET
Abilene	Stonewall	0106-04-036	12B

EN EN	LOI DRIVER: IADOLIF DE LOM: DII	N TABLE: US380_pen.tb!	<pre>ILE: c:\pw_work\atknatx01\rome2243\dms579`</pre>
	- 2	EN TAB	ILE: C

	F REMOVAL ITIES
ITEM NO. DESC. CODE	496-6012
LOCATION	REMOV STR (BRIDGE 1000 FT OR GREATER)
US 380	1
ΤΩΤΔΙ	1 1

	SU	MMARY OF	REMOVAL	QUANTIT	IES
I	ТЕМ	NO. DESC.	CODE	542-6004	544-6003
SHEET	NO.	BEGINNING STATION	ENDING STATION	RM MTL BM GD FENCE TRANS (THRIE- BEAM)	GUARDRAIL END TREATMENT (REMOVE)
				EA	EA
1 of	3	743+00.00	750+00.00	2	2
2 of	3	750+00.00	760+00.00	2	1
3 of	3	760+00.00	763+00.00		1
		TOTAL	4	4	

SUMMARY OF SIGNING AND PAVEMENT MARKING QUANTITIES												
ITEM NO. DESC. CODE 533-6001 533-6002 644-6004 644-6076 658-6014 658-6062 666-6224 666-63	03 666-6315	672-6009	678-6001									
SHEET NO. BEGINNING STATION ST	EQ W/RET REQ W) TY I (Y) D) 4"(SLD)	REFL PAV MRKR TY II-A-A	PAV SURF PREP FOR MRK (4")									
LF LF EA EA EA LF LF	LF	EA	LF									
1 of 2 743+00.00 750+00.00 680 340 1 1 6 4 1560 1400	1400	18	1240									
TOT 2	2000	25	3640									
2 of 2 760+00.00 763+00.00 600 300 2 1200 600	600	8										
2 07 2												
TOTAL 1360 680 2 2 26 8 3120 4000	4000	50	4880									

	SUMMARY OF TRAFFIC CONTROL PLAN QUANTITIES																			
ITEM NO.	403-6001	510-6003	512-6001	512-6025	512-6037	545-6003	545-6005	545-6019	644-6004	644-6076	662-6050	662-6063	662-6075	662-6095	677-6001	677-6007	4027-6001	6001-6002	6185-6002	6185-6005
LOCATION	TEMPORARY SPL SHORING	ONE-WAY TRAF CONT (PORT TRAF SIG)	PORT CTB (FUR & INST) (SGL SLOPE) (TY 1)	PORT CTB (MOVE) (SGL SLP) (TY 1)	PORT CTB (STKPL) (SGL SLP) (TY 1)	CRASH CUSH ATTEN (MOVE & RESET)		CRASH CUSH ATTEN (INSTL)(S) (N)(TL3)	IN SM RD SN SUP&AM TY10BWG(1) SA(T)	REMOVE SM RD SN SUP&AM	WK ZN PAV MRK REMOV (REFL) TY II-A-A	WK ZN PAV MRK REMOV (W) 4" (SLD)	WK ZN PAV MRK REMOV (W) 24" (SLD)	WK ZN PAV MRK REMOV (Y)4"(SLD)	ELIM EXT PAV MRK & MRKS (4")	ELIM EXT PAV MRK & MRKS (24")		PORTABLE CHANGEABLE MESSAGE SIGN		TMA (MOBILE OPERATION)
	SF	MO	LF	LF	LF	EA	EA	EA	EΑ	EA	EA	LF	LF	LF	LF	LF	LS	EA	DAY	DAY
PHASE I	1700	7	1420					2	2	2	95	2840	40	3800	6640	40	1	2	4	4
PHASE II		6		1420	1420	2	2		2	2	95	2840	40	3800	6640	40		2	4	4
PHASE III																		2	4	4
JOB CLOSEOUT																			4	4
TOTAL	1700	13	1420	1420	1420	2	2	2	2	2	190	5680	80	7600	13280	80	1	2	4	4

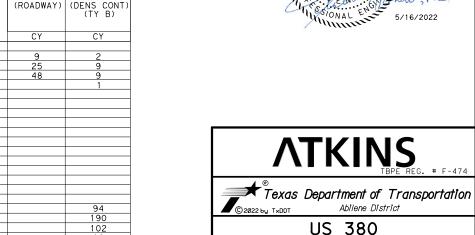
\*\*INCLUDES PAYMENT FOR TEMPORARY HAUL ROAD/BRIDGE SEE SHEET TITLED "HAUL ROAD SEQUENCE OF WORK"

				SUMM	ARY OF ROA	DWAY OLIAN	ITITIES				
I TEM 1	NO. DESC.	CODE	100-6002	247-6041	310	354-6021	432-6045	540-6001	540-6006	544-6001	3077
		ENDING STATION	PREPARING ROW	FL BS (CMP IN PLC) (TYA GR1-2) (FNAL POS)	PRIME COAT (MC-30)	PLANE ASPH CONC PAV (0" TO 2") RIPRAP (MOW STRIP) (4 IN)		MTL W-BEAM GD FEN (TIM POST)	MTL BEAM GD FEN TRANS (THRIE-BEAM)	END TREATMENT	SP MIXES SP-C SAC-B PG70-22
			STA	CY	SY	SY	CY	LF	EA	EA	SY
1 of 3	743+00.00	750+00.00	4		1747	1747	21.4	250	2	2	1747
2 of 3	750+00.00	760+00.00			410	410	6.7	156	2		410
3 of 3	760+00.00	763+00.00	4	439	1336	140	14.9	94		2	1336
	TOTAL		8	439	3493	2297	43.1	500	4	4	3493

	BASIS OF ESTIMATE										
CSJ: 0106-04-036											
ITEM	ITEM DESCRIPTION AREA (SY) RATE QUANTITY UNIT										
310-6009	PRIME COAT (MC-30)	PRIME COAT	3493	0.25	GAL/SY	874	GAL				
3077-6023	SP-C SAC-B PG 70-22	2" OVERLAY	3493	110 L	B/SY/IN	385	TON				

					SUM	MARY OF E	EROSION C	ONTROL Q	UANTITIES					
ITEM	NO. DESC.	CODE	164-6004	164-6010	164-6012	168-6001	506-6003	506-6011	506-6020	506-6024	506-6038	506-6039	506-6041	506-6043
SHEET NO.	BEGINNING STATION	ENDING STATION	BROADCAST SEED (PERM) (RURAL) (CLAY)	BROADCAST SEED (TEMP) (WARM)	BROADCAST SEED (TEMP) (COOL)	VEGETATIVE WATERING	ROCK FILTER DAMS (INSTALL) (TY 3)	ROCK FILTER DAMS (REMOVE)	CONSTRUCTION EXITS (INSTALL) (TY 1)	CONSTRUCTION EXITS (REMOVE)	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDMT CONT FENCE (REMOVE)	BIODEG EROSN CONT LOGS (INSTL) (12")	BIODEG EROSN CONT LOGS (REMOVE)
			AC	AC	AC	MG	LF	LF	SY	SY	LF	LF	LF	LF
1 of 2	743+00.00			0.05	0.05	4.20	40	40	115	115	100	100	500	500
	750+00.00		0.10	0.05	0.05	4.10	40	40					170	170
2 of 2	760+00.00	763+00.00	0.23	0.12	0.12	9.53			115	115	100	100	380	380
	TOTAL		0.44	0.22	0.22	17.8	80	80	230	230	200	200	1050	1050

EARTHW	UMMARY OF ORK QUANT	
ITEM NO. DESC. CODE	110-6001	132-6004
LOCATION	EXCAVATION (ROADWAY)	EMBANKMENT (FINAL) (DENS CONT) (TY B)
	CY	CY
743+00.00		
744+00.00	9	2
745+00.00	25	9
746+00.00	48	9
747+00.00		1
748+00.00		
749+00.00		
750+00.00		
751+00.00		
752+00.00		
753+00.00		
754+00.00		
755+00.00		
756+00.00		
757+00.00		
758+00.00		
759+00.00		
760+00.00		94
761+00.00		190
762+00.00		102
763+00.00		16
TOTAL	82	423
LIOTAL	02	423



<b>*</b>	Texas	Depar	rtment	of	Transporta
©2022	by TxDOT		Abîlene	Dîst	rict
				_	

US 380

QUANTITY SUMMARIES

SHEET	1	ΟF
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SIGNED:SG	DIV. No.	STATE	PROJE	CT No.		HIC	SHWAY No.
ECKED: JNR	6	TEXAS	SEE TIT	LE SHE	ΕT	U	S 380
awn: SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		0B o.	SHEET No.
ECKED: JNR	ABL	STONEWALI	0106	04	0	36	13

SUMMARY OF BRIDGES

CSJ	PLAN PROFILE SHEET	BRIDGE	E NBI #	DESIGN		BRIDGE LOCATION	STA	TION	LENGTH	CLEAR RDWY WIDTH	LOADING	CEM STABIL BKFL	DRILL SHAFT (18 IN)	DRILL SHAFT (36 IN)	DRILL SHAFT (42 IN)
		EXISTING	PROPOSED	EXISTING	PROPOSED		BEGIN	END	FT	FT		CY	LF	LF	LF
0106-04-036	59-60	08-217-0106-04-043	08-217-0-0106-04-057	SIMPLE SPAN DECK STEEL I-BEAM WIDENED WITH PRESTRESS CONCRETE GIRDERS	10 SPANS CONCRETE TX GIRDER SUPPORTED ON MULTI-COLUMN CONCRETE BENT CAPS	US 380 SALT FORK OF BRAZOS RIVER BRIDGE	746+90.0	759+10.0	1220′	41′-2"	HL93	312	120	80	1582
TOTALS												312	120	80	1582

CSJ (CONT'D FROM ABOVE)	CL C CONC (ABUT) (HPC)	CL C CONC (CAP) (HPC)	CL C CONC (COLUMN) (HPC)	REINF CONC SLAB (HPC)	APPROACH SLAB (HPC)	PRESTR CONC GIRDER (TX54)	PRESTR CONC GIRDER (TX70)	SILICONE RESIN PAINT FINISH	RIPRAP (STONE PROTECTION) (12 IN)	RAIL (TY T223) (HPC)	SEALED EXPANSION JOINT (4 IN) (SEJ-M)
	CY	CY	CY	SF	LF	LF	LF	SF	CY	LF	LF
0106-04-036	81.9	196.8	91.0	52664	66.0	4659.00	2631.05	4823	292.0	2522.0	256
TOTALS	81.9	196.8	91.0	52664	66.0	4659.00	2631.05	4823	292.0	2522.0	256



HL93 LOADING

11230	LONDING		
REV. No.	DATE	REVISION	BY
I			

**ATKINS**THE REC.



US 380 BRIDGE SUMMARY

#### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE:	NO SCALE	SHEET 1	OF
	LEED BUI		

SIGNED	:TS	FED. RD DIV. No.	STATE	PROJECT No.				HWAY No.	ı
ECKED:	SK	6	TEXAS :	SEE TIT	LE SHEE	T	U	S 380	l
AWN:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.	ı
ECKED:	TS	ABL	STONEWALL	0106	04	0	36	14	ı

#### **GENERAL**

- 1. PLACE TEMPORARY EROSION CONTROL DEVICES AS SHOWN IN THE PLANS, AND AS DIRECTED BY THE ENGINEER, PRIOR TO BEGINNING ANY OTHER WORK.
- 2. BEFORE THE COMMENCEMENT OF EACH PHASE, INSTALL OR MAINTAIN ADVANCE WARNING SIGNS, TRAFFIC CONTROL DEVICES, WORK ZONE PAVEMENT MARKINGS, AND SIGNS AS SHOWN IN THE STANDARDS.
- 3. PROVIDE ADDITIONAL SIGNS AND BARRICADES AS NECESSARY TO ADDRESS FIELD CONSTRUCTABILITY & VISIBILITY. THESE ADDITIONAL SIGNS WILL BE CONSIDERED SUBSIDIARY TO ITEM 502.
- 4. CONTRACTOR SHALL ONLY ACCESS THE CRITICAL HABITAT AREA AT THE HAUL ROAD.
- 5. CONTRACTOR SHALL SUBMIT A PLAN FOR APPROVAL OF THE CONSTRUCTION LIMITS THAT ENSURES THE LIMITS OF CONSTRUCTION DURING THE DAY CAN BE OPEN TO TRAFFIC AT NIGHT WITHOUT THE USE OF FLAGMEN OR AS DIRECTED BY THE ENGINEER IN THE FIELD.
- 6. UPON COMPLETION OF THE WORK AND BEFORE FINAL ACCEPTANCE AND FINAL PAYMENT IS MADE, THE CONTRACTOR SHALL CLEAR AND REMOVE
  THE SITE OF ALL SURPLUS AND DISCARDED MATERIALS AND DEBRIS OF EVERY KIND AND LEAVE THE ENTIRE PROJECT IN A SMOOTH, NEAT, AND
  SIGHTLY CONDITION.

#### PHASE I - CONSTRUCT WESTBOUND:

- 1. PLACE WORK ZONE SIGNS AT THE BEGINNING AND END OF THE PROJECT LIMITS PRIOR TO BEGINNING WORK. USE TXDOT STANDARDS FOR ALL TRAFFIC CONTROL DEVICES, SIGNS, AND SIGN SPACING.
- 2. CONSTRUCT TEMPORARY HAUL BRIDGES AND ROADS WITH MATERIAL AT CONTRACTOR'S DISCRETION. THE TEMPORARY BRIDGE STRUCTURE
  MAY BE NO MORE THAN 14 FT WIDE OR 140 FT IN LENGTH AND MAYBE SUPPORTED BY NO MORE THAN 4 UNTREATED TIMBERS OR METAL BEAMS
  WITHIN THE SALT FORK BRAZOS RIVER CHANNEL. ADDITIONAL SUPPORT TIMBERS OR BEAMS MAY BE PLACED WITHIN THE CRITICAL HABITAT
  UNIT BUT OUTSIDE OF THE RIVER CHANNEL ABOVE THE NORMAL BANKFULL CONTOUR.
- 3. SHIFT EASTBOUND AND WESTBOUND TRAFFIC TO ONE-LANE, TWO WAY TRAFFIC OPERATIONS ON THE EASTBOUND SIDE USING A TEMPORARY TRAFFIC SIGNAL, PER TXDOT STANDARD TCP (2-8).
- 4. CONSTRUCT TEMPORARY SPECIAL SHORING. PLACE TEMPORARY CONCRETE BARRIER.
- 5. CONSTRUCT WESTBOUND PORTION OF BRIDGE AND ROADWAY. USE LIFTS CONSISTING OF SP-C SAC-B PG 70-22, WITH TACK COAT PLACED BETWEEN LAYERS.
- 6. PLACE PHASE II TEMPORARY CONCRETE BARRIER, CRASH CUSHIONS, AND TEMPORARY STRIPING ALONG WESTBOUND SIDE.

SEE "TRAFFIC CONTROL PLAN TYPICAL SECTIONS" AND "HAUL ROAD SEQUENCE OF WORK" SHEET FOR MORE INFORMATION.

#### PHASE II - CONSTRUCT EASTBOUND:

- 1. PLACE WORK ZONE SIGNS AT THE BEGINNING AND END OF THE PROJECT LIMITS PRIOR TO BEGINNING WORK. USE TXDOT STANDARDS FOR ALL TRAFFIC CONTROL DEVICES, SIGNS, AND SIGN SPACING.
- 2. PHASE II TEMPORARY CONCRETE BARRIER, CRASH CUSHIONS, AND TEMPORARY STRIPING WILL NEED TO BE PLACED BEFORE PHASE I TEMPORARY CONCRETE BARRIER CAN BE SAFELY REMOVED.
- 3. SHIFT EASTBOUND AND WESTBOUND TRAFFIC TO THE NEWLY CONSTRUCTED WESTBOUND PAVEMENT AND BRIDGE WITH ONE-LANE, TWO WAY TRAFFIC OPERATIONS USING A TEMPORARY TRAFFIC SIGNAL, PER TXDOT STANDARD TCP (2-8).
- 4. CONSTRUCT EASTBOUND PORTION OF BRIDGE AND ROADWAY. USE LIFTS CONSISTING OF SP-C SAC-B PG 70-22, WITH TACK COAT PLACED BETWEEN LAYERS.

SEE "TRAFFIC CONTROL PLAN TYPICAL SECTIONS" FOR MORE INFORMATION.

#### PHASE III - CONSTRUCT FINAL PAVEMENT MARKINGS AND MISCELLANEOUS ITEMS

- 1. USING TCP (2-2)-18 AND TCP (3-1)-13, AND TCP (3-3)-14 PLACE PERMANENT PAVEMENT MARKINGS, SIGNS, AND ALL OTHER APPURTENANCES REQUIRED TO COMPLETE US 380 TO THE FINAL CONFIGURATION SHOWN IN THE PLANS AND STANDARDS. CONSTRUCTION SHALL BE LIMITED TO WORK THE CONTRACTOR IS ABLE TO COMPLETE IN ONE WORKING DAY OR AS DIRECTED BY THE ENGINEER. RETURN TRAFFIC TO TWO-LANE OPERATIONS DURING NON-CONSTRUCTION HOURS.
- 2. REMOVE BARRICADES, WORK ZONE SIGNS, AND PORTABLE TRAFFIC SIGNALS.
- 3. OPEN ROADWAY AND BRIDGE TO TRAFFIC.







US 380

TRAFFIC CONTROL SEQUENCE OF WORK

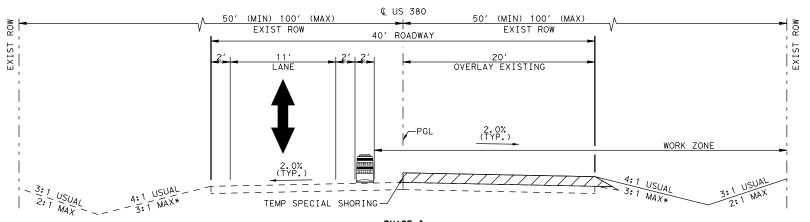
SHEET 1 OF

DESIGNED:	SG	DIV. No.	STATE		PROJE	CT No.		HIC	HWAY No.
CHECKED:	JNR	6	TEXAS		SEE TIT	LE SHEE		U	S 380
DRAWN:	SG	STATE DISTRICT	COUNT	Y	CONTROL No.	SECTION No.		OB 0.	SHEET No.
CHECKED: .	JNR	ABL	STONEW	ALL	0106	04	0	36	15

€ US 380 50' (MIN) 100' (MAX) 50' (MIN) 100' (MAX) EXIST ROW EXIST ROW 40' ROADWAY 20' OVERLAY EXISTING LANE WORK ZONE 2.0% (TYP.) - 3:1 USUAL 2:1 MAX 2:1 MAX 2: 1 MAX PHASE I CONSTRUCT WESTBOUND STA 743+00.00 TO STA 746+90.00

> € US 380 18'
> PHASE I CONSTRUCTION LANE -PROPOSED T223 RAIL -PROPOSED
> PRESTR CONC TX54
> GIRDER (SPANS 4-10)
> PRESTR CONC TX70
> GIRDER (SPANS 1-3) (TYP) піш, 11.11 1111 11.11 1.1 ЩШ

PHASE I CONSTRUCT WESTBOUND STA 746+90.00 TO STA 759+10.00



PHASE I CONSTRUCT WESTBOUND STA 759+10.00 TO STA 763+00.00

#### NOTES:

- 1. SEE PLAN AND PROFILE SHEETS FOR ROADWAY LIMITS AND DETAILS.
- 2. SEE TYPICAL SECTIONS FOR DETAILED PROPOSED TYPICAL SECTIONS.
- 3. SEE TRAFFIC CONTROL SEQUENCE NARRATIVE AND TRAFFIC CONTROL STANDARDS FOR MORE INFORMATION.
- 4. SEE PAVEMENT MARKING LAYOUT SHEETS FOR PERMANANT STRIPING INFORMATION.
- 5. SEE BRIDGE LAYOUT SHEETS FOR BRIDGE DETAILS
- 6. REFERENCE TXDOT TRAFFIC CONTROL STANDARD TCP (2-2), TCP (2-8b) AND TCP (3-1), AND TCP (3-3) FOR ALL TRAFFIC CONTROL DEVICES, SIGNS, AND SIGN SPACINGS.
- 7. TEMPORARY TRAFFIC SIGNALS SHALL BE PLACED IN ACCORDANCE WITH THE TEXAS MUTCD, TCP (2-8), OR AS DIRECTED BY THE ENGINEER.
- 8. PLACE FINAL SURFACE AFTER WORK IS COMPLETE.

#### LEGEND

EXISTING STRUCTURE



UNDER CONSTRUCTION



COMPLETED CONSTRUCTION



TRAFFIC FLOW ARROW



SCALE = N.T.S.

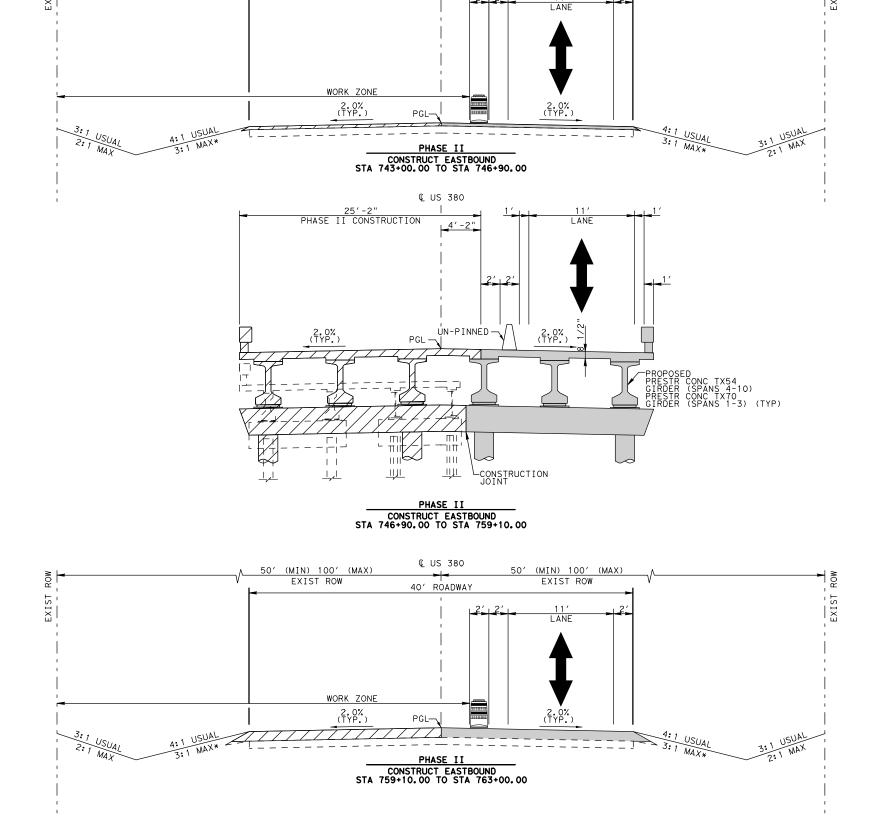


US 380

TRAFFIC CONTROL PLAN
TYPICAL SECTIONS

SHEET 1 OF 2

SIGNED	:SG	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.
ECKED:	JNR	6	TEXAS	SEE TIT	LE SHE	ΞT	U	S 380
AWN:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.
ECKED:	JNR	ABL	STONEWALL	0106	04	0	36	16



€ US 380

40' ROADWAY

50' (MIN) 100' (MAX)

EXIST ROW

50' (MIN) 100' (MAX)

EXIST ROW

#### NOTES:

- 1. SEE PLAN AND PROFILE SHEETS FOR ROADWAY LIMITS AND DETAILS.
- 2. SEE TYPICAL SECTIONS FOR DETAILED PROPOSED TYPICAL SECTIONS.
- 3. SEE TRAFFIC CONTROL SEQUENCE NARRATIVE AND TRAFFIC CONTROL STANDARDS FOR MORE INFORMATION.
- 4. SEE PAVEMENT MARKING LAYOUT SHEETS FOR PERMANANT STRIPING INFORMATION.
- 5. SEE BRIDGE LAYOUT SHEETS FOR BRIDGE DETAILS
- 6. REFERENCE TXDOT TRAFFIC CONTROL STANDARD TCP (2-2), TCP (2-8b) AND TCP (3-1), AND TCP (3-3) FOR ALL TRAFFIC CONTROL DEVICES, SIGNS, AND SIGN SPACINGS.
- 7. TEMPORARY TRAFFIC SIGNALS SHALL BE PLACED IN ACCORDANCE WITH THE TEXAS MUTCD, TCP (2-8), OR AS DIRECTED BY THE ENGINEER.
- 8. PLACE FINAL SURFACE AFTER WORK IS COMPLETE.

#### LEGEND

----

EXISTING STRUCTURE



UNDER CONSTRUCTION



COMPLETED CONSTRUCTION



TRAFFIC FLOW ARROW



SCALE = N.T.S.

# **ATKINS**

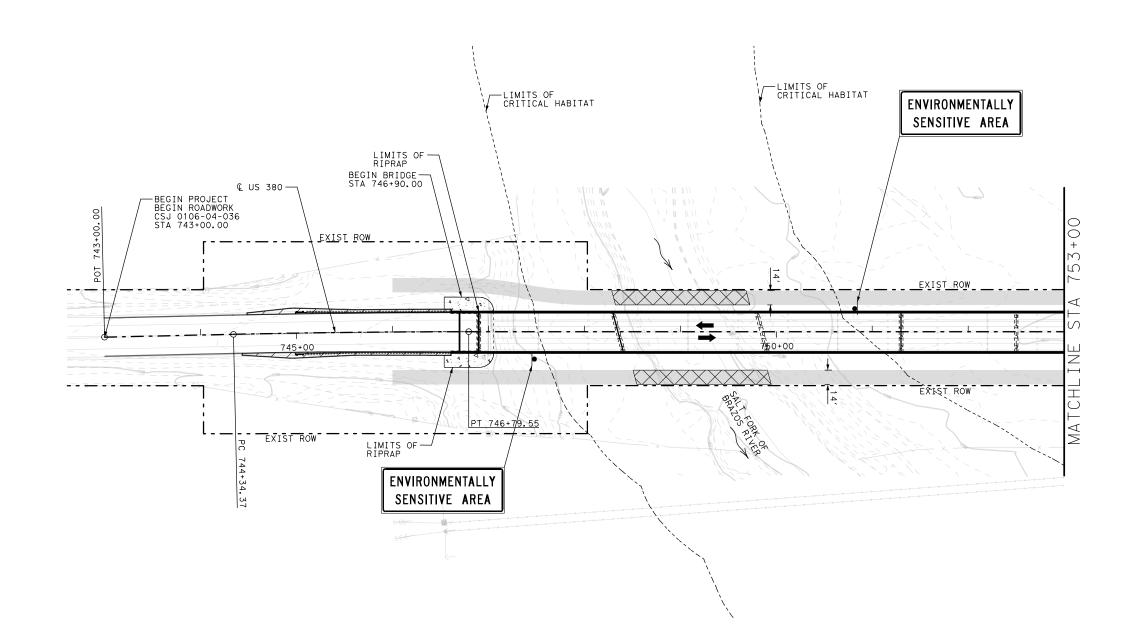


US 380

TRAFFIC CONTROL PLAN
TYPICAL SECTIONS

SHEET 2 OF 2

GNEC	:SG	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	SHWAY No.
KED:	JNR	6	TEXAS	SEE TIT	LE SHE	ΞT	U	S 380
WN:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		о.	SHEET No.
KED:	JNR	ABL	STONEWALL	0106	04	0	36	17





#### **LEGEND**

PROPOSED RIPRAP



TEMPORARY HAUL ROAD



MAXIMUM LIMITS OF TEMPORARY HAUL BRIDGE



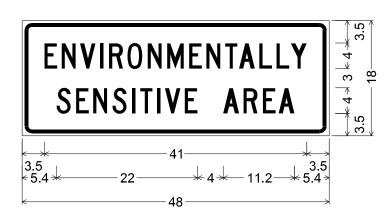
----- LIMITS OF CRITICAL HABITAT





#### SUGGESTED SEQUENCE OF WORK:

- 1. INSTALL BARRICADES, DETOUR SIGNING, AND ADVANCED WARNING SIGNS ACCORDING TO THE PLANS.
- 2. INSTALL SEDIMENT CONTROL DEVICES IN ACCORDANCE WITH SW3P PLANS.
- 3. CONSTRUCT TEMPORARY HAUL BRIDGES AND ROADS WITH MATERIAL AT CONTRACTOR'S DISCRETION. THE TEMPORARY BRIDGE STRUCTURE MAY BE NO MORE THAN 14 FT WIDE OR 140 FT IN LENGTH AND MAYBE SUPPORTED BY NO MORE THAN 4 UNTREATED TIMBERS OR METAL BEAMS WITHIN THE SALT FORK BRAZOS RIVER CHANNEL. ADDITIONAL SUPPORT TIMBERS OR BEAMS MAY BE PLACED WITHIN THE CRITICAL HABITAT UNIT BUT OUTSIDE OF THE RIVER CHANNEL ABOVE THE NORMAL BANKFULL CONTOUR.
- 4. CONSTRUCT ROADWAY AND BRIDGE ACCORDING TO "TRAFFIC CONTROL SEQUENCE OF WORK".
- 5. REMOVE TEMPORARY HAUL ROADS/BRIDGES AND RETURN TO PREVIOUS CONDITIONS.



1.5" Radius, 0.6" Border, 0.4" Indent, Black on White; "ENVIRONMENTALLY", C specified length; "SENSITIVE", C; "AREA", C;

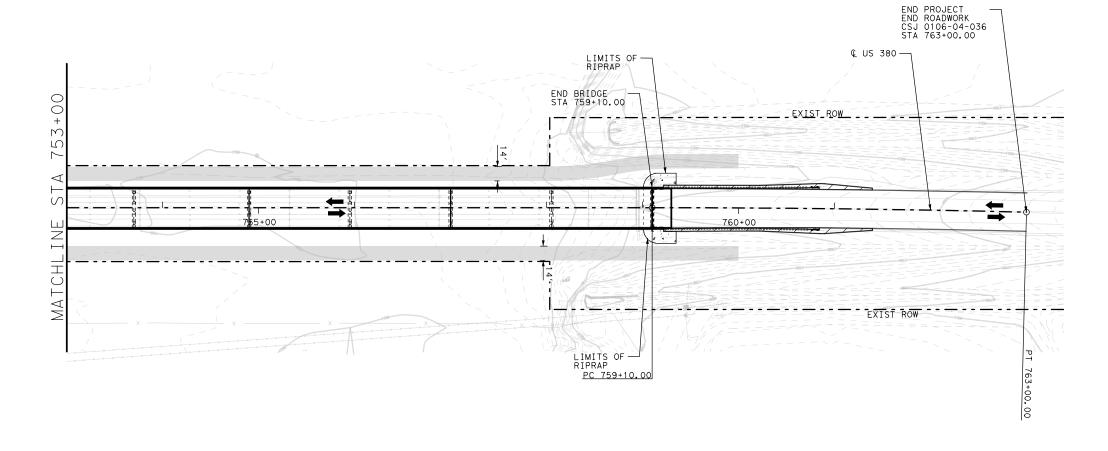


US 380

HAUL ROAD SEQUENCE OF WORK

SHEET 1 OF :

I GNED	:SG	DIV. No.	STATE	PROJE	CT No.		HIC	SHWAY No.
CKED:	JNR	6	TEXAS	SEE TIT	LE SHE	ĒΤ	U	S 380
wn:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		0B o.	SHEET No.
CKED:	JNR	ABL	STONEWALL	0106	04	0	36	18





#### LEGEND

PROPOSED RIPRAP



TEMPORARY HAUL ROAD



MAXIMUM LIMITS OF TEMPORARY HAUL BRIDGE



----- LIMITS OF CRITICAL HABITAT





#### SUGGESTED SEQUENCE OF WORK:

- 1. INSTALL BARRICADES, DETOUR SIGNING, AND ADVANCED WARNING SIGNS ACCORDING TO THE PLANS.
- 2. INSTALL SEDIMENT CONTROL DEVICES IN ACCORDANCE WITH SW3P PLANS.
- 3. CONSTRUCT TEMPORARY HAUL BRIDGES AND ROADS WITH MATERIAL AT CONTRACTOR'S DISCRETION. THE TEMPORARY BRIDGE STRUCTURE MAY BE NO MORE THAN 14 FT WIDE OR 140 FT IN LENGTH AND MAYBE SUPPORTED BY NO MORE THAN 4 UNTREATED TIMBERS OR METAL BEAMS WITHIN THE SALT FORK BRAZOS RIVER CHANNEL. ADDITIONAL SUPPORT TIMBERS OR BEAMS MAY BE PLACED WITHIN THE CRITICAL HABITAT UNIT BUT OUTSIDE OF THE RIVER CHANNEL ABOVE THE NORMAL BANKFULL CONTOUR.
- 4. CONSTRUCT ROADWAY AND BRIDGE ACCORDING TO "TRAFFIC CONTROL SEQUENCE OF WORK".
- 5. REMOVE TEMPORARY HAUL ROADS/BRIDGES AND RETURN TO PREVIOUS CONDITIONS.





Texas Department of Transportation Abilene District

US 380

HAUL ROAD SEQUENCE OF WORK

SHEET 2 OF 2

IGNED	:SG	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	SHWAY No.
CKED:	JNR	6	TEXAS S	SEE TIT	LE SHE	ΞT	U	S 380
WN:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB o.	SHEET No.
CKED:	JNR	ABL	STONEWALL	0106	04	0	36	19

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															CR	ASH CUSHI	ON				
		PLAN				DIRECTION OF	FOUNDA <sup>-</sup>	TION PAD	BACKUP SUPPOR	Т		AVAILABLE			MOVE /	RESET	L	L R	≀ R	S	S
LOC NO.	TCP PHASE	SHEET NUMBER	LOCATION	STA	TEST LEVEL	TRAFFIC (UNI/BI)	PROPOSED MATERIAL	PROPOSED THICKNESS	DESCRIPTION	WIDTH	HEIGHT	SITE LENGTH	INSTALL	REMOVE	MOVE/ RESET	FROM LOC.#	N	w N	v w	N	W
1	1	16	US 380 BRIDGE - EAST END	746+90.00	TL3	ВІ	N/A	N/A	PORTABLE TRAFFIC BARRIER	24"	3′-6"		X							х	
2	1	16	US 380 BRIDGE - WEST END	759+10.00	TL3	BI	N/A	N/A	PORTABLE TRAFFIC BARRIER	24"	3′-6"		Х							Х	
3	2	17	US 380 BRIDGE - EAST END	746+90.00	TL3	BI	N/A	N/A	PORTABLE TRAFFIC BARRIER	24"	3′-6"			Х	X	1				Х	
4	2	17	US 380 BRIDGE - WEST END	759+10.00	TL3	ВІ	N/A	N/A	PORTABLE TRAFFIC BARRIER	24"	3′-6"			X	Х	1				X	
																		_		+	
																				+	
																		_	_		
																		_	_	+	
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																				1	
																			_	1	
												T07::-									
												TOTALS	2	2	2						

LEGEND: L=LOW MAINTENANCE R=REUSABLE S=SACRIFICIAL N=NARROW W=WIDE

FOR DEFINITIONS SEE THE "CRASH CUSHION CATEGORIZATION CHART.PDF" AT THE DESIGN DIVISION (ROADWAY STANDARDS) WEBSITE. USE QUICK LINKS TO ACCESS ATTENUATORS / CRASH CUSHIONS SECTION.

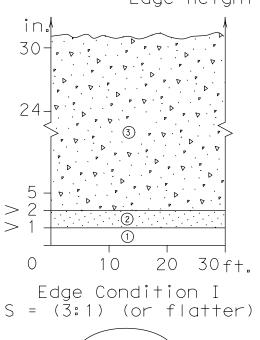
http://www.dot.state.tx.us/insdtdot/orgchart/cmd/cserve/standard/rdwylse.htm

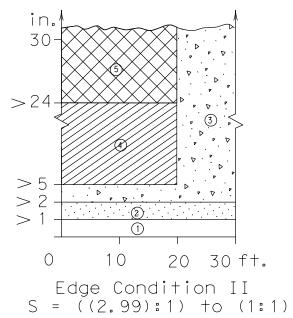
#### CRASH CUSHION SUMMARY SHEET

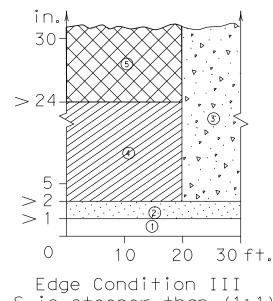
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REVISIONS	0106		04		036
	DIST		COUNTY	ł	HIGHWAY
	ABL	ST	DNEWALL	L	JS 380
	FEDERA	AL A	ID PROJEC	Т	SHEET NO.
					20

## DEFINITION OF TREATMENT ZONES FOR VARIOUS EDGE CONDITIONS

Edge Height (D) in Inches versus Lateral Clearance (Y) in Feet

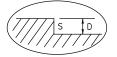


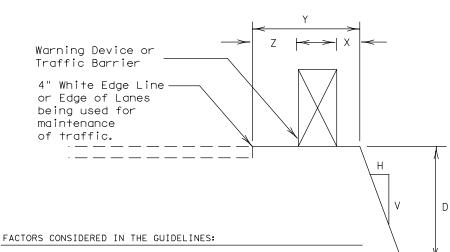




S is steeper than (1:1)







- The "Edge Condition" is the slope (S) of the drop-off (H:V). The "Edge Height is the depth of the drop-off "D".
- Distance "X" is to be the maximum practical under job conditions. Two feet minimum for high speed conditions. Distance "Y" is the lateral clearance from edge of travel lane to edge of dropoff. Distance "Z" does not have a minimum.
- 3. In addition to the factors considered in the guidelines, each construction zone drop-off situation should be analyzed individually, taking into account other variables, such as: traffic mix, posted speed in the construction zone, horizontal curvature, and the practicality of the treatment options.
- 4. The conditions for indicating the use of positive or protective barriers are given by Zone-5 and Figure-1. Traffic barriers are primarily applicable for high speed conditions. Urban areas with speeds of 30 mph or less may have a lesser need for signing, delineation, and barriers. Right-angled edges, however, with "D" greater than 2 inches and located within a lateral offset of 6 feet, may indicate a higher level of treatment.
- 5. If the distance "Y" must be less than 3 feet, the use of a positive barrier may not be feasible. In such a case, consider either: 1) narrowing the lanes to a desired 11 to 12 feet or 10 foot minimum (see CW20-8 sign), or 2) provide an edge slope such as Edge Condition I.

#### Zone Treatment Types Guidelines:

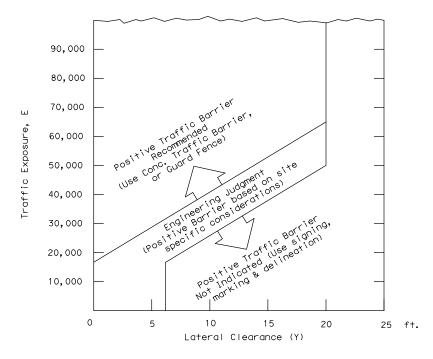
No treatment.

- CW 8-11 "Uneven Lanes" signs.
- CW 8-9a "Shoulder Drop-Off" or CW 8-11 signs plus vertical panels.
- (4) CW 8-9a or CW 8-11, signs plus drums.
  Where restricted space precludes the use of drums, use vertical panels. An edge fill may be provided to change the edge slope to that of the preferable Edge Condition I.
- Check indications (Figure-1) for positive barrier. Where positive barrier is not indicated, the treatment shown above for Zone- 4 may be used after consideration of other applicable factors.

#### Edge Condition Notes:

- Edge Condition I: Most vehicles are able to traverse an edge condition with a slope rate of (3 to 1) or flatter. The slope must be constructed with a compacted material capable of supporting vehicles.
- 2. Edge Condition II: Most vehicles are able to traverse an edge condition with a slope between (2.99 to 1) and (11to 1) so long as "D" does not exceed 5 inches. Under-carriage drag on most automobiles will occur when "D" exceeds 6 inches. As "D" exceeds 24 inches, the possibility for rollover is greater in most vehicles.
- 3. Edge Condition III: When slopes are greater than (1 to 1) and where "D" is greater than 2 inches, a more difficult control factor may exist for some vehicles, if not properly treated. For example, where "D" is greater than 2 inches and up to 24 inches different types of vehicles may experience different steering control at different edge heights. Automobiles might experience more steering control differential when "D" is greater than 2 inches and up to 5 inches. Trucks, particularly those with high loads, have more steering control differential when "D" is greater than 5 inches and up to 24 inches. When "D" exceeds 24 inches, the possibility of rollover is greater for most vehicles.
- 4. Milling or overlay operations that result in Edge Condition III should not be in place without appropriate warning treatments, and these conditions should not be left in place for extended periods of time.

# FIGURE-1: CONDITIONS INDICATING USE OF POSITIVE BARRIER FOR ZONE 5 ( )



- 1 E = ADT x T Where ADT is that portion of the average daily traffic volume traveling within 20 feet (generally two adjacent lanes) of the edge dropoff condition; and, T is the duration time in years of the dropoff condition.
- 2 Figure-1 provides a practical approach to the use of positive barriers for the protection of vehicles from pavement drop-offs. Other factors, such as the presence of heavy machinery, construction workers, or the mix and volume of traffic may make the use of positive barriers appropriate, even when the edge condition alone may not justify the use of a barrier.
- 3 An approved end treatment should be provided for any positive barrier end located within a lateral offset of 20 feet from the edge of the travel lane.

These guidelines apply to temporary traffic control areas or work zones where continuous pavement edges or drop-offs exists parallel and adjacent to a lane used by traffic. The edge conditions may be present between shoulders and travel lanes, between adjacent or opposing travel lanes, or at intermediate points across the width of the paved surface. Due to the variability in construction operations, tolerances in the variables may be allowed by the engineer. These guidelines do not apply to short term operations. These guidelines do not constitute a rigid standard or policy; rather, they are guidance to be used in conjunction with engineering judgement. These guidelines may be updated on the Design Division's on-line manuals.





# TREATMENT FOR VARIOUS EDGE CONDITIONS

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			SUMMARY	OF SN	ΛΑ	\ L	L SIG	N S				
SI AN					(TYPE A)	(TYPE G)					XX (X-XXXX)	BRIDGE MOUNT CLEARANCE
PLAN SHEET	SIGN	SIGN			1		POST TYPE	POSTS	ANCHOR TYPE	MOUN	TING DESIGNATION	SIGNS
NO.	NO.	NOMENCLATURE	SIGN	DIMENSIONS	FLAT ALUMINUM	Ι.	FRP = Fiberglass TWT = Thin-Wall 10BWG = 10 BWG S80 = Sch 80	1 or 2	UB=Universal Bolt		1EXT or 2EXT = # of Ext BM = Extruded Wind Beam WC = 1.12 #/ft Wing Channel EXAL= Extruded Alum Sign Panels	TY = TYPE
SW3P 1	1	MODIFIED SIGN	ENVIRONMENTALLY SENSITIVE AREA  NOTE: TEMPORARY SIGN TO BE REMOVED AFTER CONSTRUCTION	48" × 18"	×		1 OBWG	1	SA	Т		
SW3P 1	2	MODIFIED SIGN	ENVIRONMENTALLY SENSITIVE AREA  NOTE: TEMPORARY SIGN TO BE REMOVED AFTER CONSTRUCTION	48" × 18"	X		1 OBWG	1	SA	Т		

ALUMINUM SIGN BI	ANKS THICKNESS
Square Feet	Minimum Thickness
Less than 7.5	0.080"
7.5 to 15	0.100"
Greater than 15	0.125"

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

http://www.txdot.gov/

#### NOTE:

- 1. Sign supports shall be located as shown on the plans, except that the Engineer may shift the sign supports, within design guidelines, where necessary to secure a more desirable location or to avoid conflict with utilities. Unless otherwise shown on the plans, the Contractor shall stake and the Engineer will verify all sign support locations.
- For installation of bridge mount clearance signs, see Bridge Mounted Clearance Sign Assembly (BMCS)Standard Sheet.
- For Sign Support Descriptive Codes, see Sign Mounting Details Small Roadside Signs General Notes & Details SMD(GEN).



Traffic Operations Division Standard

TEMPORARY SUMMARY OF SMALL SIGNS

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#### BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

- 1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. The development and design of the Traffic Control Plan (TCP) is the responsibility of the Engineer.
- 3. The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop, sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- 6. When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- 7. The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- 9. The temporary traffic control devices shown in the illustrations of the BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES, CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

#### WORKER SAFETY NOTES:

- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel," or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

#### COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

- Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

# THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD) DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS) MATERIAL PRODUCER LIST (MPL) ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)" STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD) TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD) TRAFFIC ENGINEERING STANDARD SHEETS

SHEET 1 OF 12



Safety Division Standard

# BARRICADE AND CONSTRUCTION GENERAL NOTES AND REQUIREMENTS

BC(1)-21

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- The typical minimum signing on a crossroad approach should be a "ROAD WORK AHEAD" (CW20-1D) sign and a (G20-2) "END ROAD WORK" sign, unless noted otherwise in plans.
- 2. The Engineer may use the reduced size 36" x 36" ROAD WORK AHEAD (CW20-1D) sign mounted back to back with the reduced size 36" x 18" "END ROAD WORK" (G20-2) sign on low volume crossroads (see Note 4 under "Typical Construction Warning Sign Size and Spacing"). See the "Standard Highway Sign Designs for Texas" manual for sign details. The Engineer may omit the advance warning signs on low volume crossroads. The Engineer will determine whether a road is low volume as per TMUTCD Part 5. This information shall be shown in the plans.
- Based on existing field conditions, the Engineer/Inspector may require additional signs such as FLAGGER AHEAD, LOOSE GRAVEL, or other appropriate signs. When additional signs are required, these signs will be considered part of the minimum requirements. The Engineer/Inspector will determine the proper location and spacing of any sign not shown on the BC sheets, Traffic Control Plan sheets or the Work Zone Standard Sheets.
- 4. The "ROAD WORK NEXT X MILES"(G20-1aT)sign shall be required at high volume crossroads to advise motorists of the length of construction in either direction from the intersection. The Engineer will determine whether a roadway is considered high volume.
- 5. Additional traffic control devices may be shown elsewhere in the plans for higher volume crossroads.
- 6. When work occurs in the intersection area, appropriate traffic control devices, as shown elsewhere in the plans or as determined by the Engineer/Inspector, shall be in place.

#### BEGIN T-INTERSECTION WORK ZONE **X** ★ G20-9TP ★ ★ R20-5T FINES DOLIBL XX R20-5aTP WORKERS ARE PRESENT ROAD WORK <⇒ NEXT X MILES END ¥ ★ G20-2bT WORK ZONE G20-1bTI $\langle \neg$ INTERSECTED 1000'-1500' - Hwy 1 Block - City 1000'-1500' - Hwy 1 Block - City ROADWAY $\Rightarrow$ BOAD WORK G20-1bTR NEXT X MILES ⇒ 80' WORK ZONE G20-2bT X X l imi+ min BEGIN G20-5T WORK $\times$ $\times$ G20-9TP ZONE TRAFFI G20-6T $\times$ $\times$ R20-5T FINES IDOUBLE → R20-5aTP WHEN WORKERS ARE PRESENT ROAD WORK G20-2

#### CSJ LIMITS AT T-INTERSECTION

- 1. The Engineer will determine the types and location of any additional traffic control devices, such as a flagger and accompanying signs, or other signs, that should be used when work is being performed at or near an intersection.
- 2. If construction closes the road at a T-intersection, the Contractor shall place the "CONTRACTOR NAME" (G20-6T) sign behind the Type 3 Barricades for the road closure (see BC(10) also). The "ROAD WORK NEXT X MILES" left arrow(G20-1bTL) and "ROAD WORK NEXT X MILES" right arrow (G20-1bTR)" signs shall be replaced by the detour signing called for in the plans.

SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING AT THE CSJ LIMITS

#### TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING $^{1,5,6}$

#### SIZE

onventional

48" x 48"

36" x 36"

Expressway/

Freeway

48" × 48'

48" x 48'

Posted Speed	Sign∆ Spacing "X"
MPH	Feet (Apprx.)
30	120
35	160
40	240
45	320
50	400
55	500 <sup>2</sup>
60	600 <sup>2</sup>
65	700 2
70	800 <sup>2</sup>
75	900 <sup>2</sup>
80	1000 <sup>2</sup>
*	* 3

SPACING

CW3, CW4, CW5, CW6, 48" x 48" 48" x 48' CW8-3, CW10, CW12

\* For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.

 $\triangle$  Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

#### GENERAL NOTES

Sign

Number

or Series

CW20' CW21

CW22

CW23

CW25

CW14

CW1, CW2,

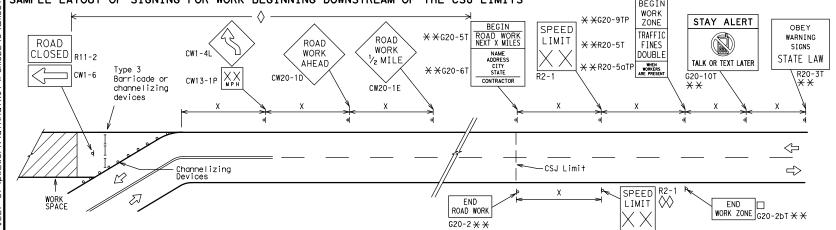
CW7. CW8.

CW9, CW11

- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- 3. Distance between signs should be increased as required to have 1/2 mile or more advance warning.
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D) signs may be used on low volume crossroads at the discretion of the Engineer as per IMUTCD Part 5. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.
- 6. See sign size listing in "TMUTCD". Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design sizes.

WORK AREAS IN MULTIPLE LOCATIONS WITHIN CSJ LIMITS	SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING AT THE CSJ LIMITS	
ROAD WORK AREA AHEAD CW20-1D CW13-1P	** X G20-6T ADDRESS CW13-1P WPH CW20-1D R2-1X X X R20-5aTP ARE PRESENT TALK OR TEXT LATER	OBEY WARNING SIGNS TATE LAW 0-3T **
Channelizing Devices	WORK SPACE    CSJ Limit   Beginning of NO-PASSING   R2-1   LIMIT	**
When extended distances occur between minimal work spaces, the Engineer/I "ROAD WORK AHEAD"(CW20-1D)signs are placed in advance of these work areas		
within the project limits. See the applicable TCP sheets for exact location		
channelizing devices.	The Contractor shall determine the o	appropric

#### SAMPLE LAYOUT OF SIGNING FOR WORK BEGINNING DOWNSTREAM OF THE CSJ LIMITS



The Contractor shall determine the appropriate distance to be placed on the G20-1 series signs and "BEGIN ROAD" WORK NEXT X MILES" (G20-5T) sign for each specific project. This distance shall replace the "X" and shall be rounded

The "BEGIN WORK ZONE" (G20-9TP) and "END WORK ZONE" (G20-2b) shall be used as shown on the sample layout when advance signs are required outside the CSJ Limits. They inform the motorist of entering or leaving a part of the work zone

lying outside the CSJ Limits where traffic fines may double

to the nearest whole mile with the approval of the Engineer

No decimals shall be used.

workers are present.

 $\star\star$  CSJ limit signing is required for highway construction and maintenance work, with the exception of mobile operations.

Area for placement of "ROAD WORK AHEAD" (CW20-1D) sign and other signs or devices as called for on the Traffic

Contractor will install a regulatory speed limit sign at the end of the work zone.

	LEGEND					
$\vdash$	⊢⊣ Type 3 Barricade					
000	Channelizing Devices					
-	Sign					
Х	See Typical Construction Warning Sign Size and Spacing chart or the TMUTCD for sign spacing requirements.					

#### SHEET 2 OF 12

Texas Department of Transportation

Traffic Safety Division

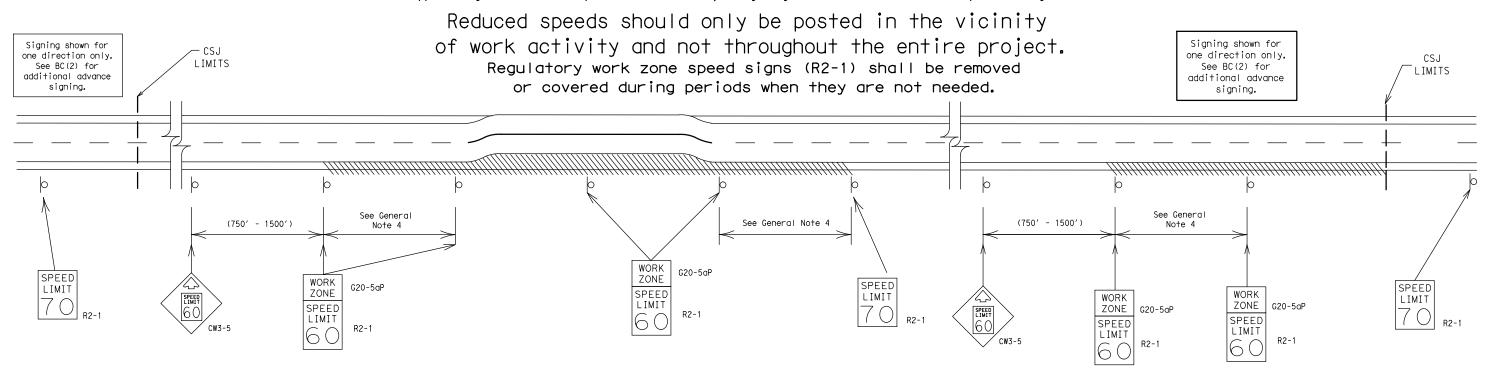
#### BARRICADE AND CONSTRUCTION PROJECT LIMIT

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#### TYPICAL APPLICATION OF WORK ZONE SPEED LIMIT SIGNS

Work zone speed limits shall be regulatory, established in accordance with the "Procedures for Establishing Speed Zones," and approved by the Texas Transportation Commission, or by City Ordinance when within Incorporated City Limits.



#### GUIDANCE FOR USE:

#### LONG/INTERMEDIATE TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit should be included on the design of the traffic control plans when restricted geometrics with a lower design speed are present in the work zone and modification of the geometrics to a higher design speed is not feasible.

Long/Intermediate Term Work Zone Speed Limit signs, when approved as described above, should be posted and visible to the motorist when work activity is present. Work activity may also be defined as a change in the roadway that requires a reduced speed for motorists to safely negotiate the work area, including:

- a) rough road or damaged pavement surface
- b) substantial alteration of roadway geometrics (diversions)
- c) construction detours
- d) grade
- e) width
- f) other conditions readily apparent to the driver

As long as any of these conditions exist, the work zone speed limit signs should remain in place.

#### SHORT TERM WORK ZONE SPEED LIMITS

This type of work zone speed limit may be included on the design of the traffic control plans when workers or equipment are not behind concrete barrier, when work activity is within 10 feet of the traveled way or actually in the traveled way.

Short Term Work Zone Speed Limit signs should be posted and visible to the motorists only when work activity is present. When work activity is not present, signs shall be removed or covered. (See Removing or Covering on BC(4)).

#### GENERAL NOTES

- 1. Regulatory work zone speed limits should be used only for sections of construction projects where speed control is of major importance.
- 2. Regulatory work zone speed limit signs shall be placed on supports at a 7 foot minimum mounting height.
- 3. Speed zone signs are illustrated for one direction of travel and are normally posted for each direction of travel.
- 4. Frequency of work zone speed limit signs should be:

40 mph and greater 0.2 to 2 miles

35 mph and less 0.2 to 1 mile

- 5. Regulatory speed limit signs shall have black legend and border on a white reflective background (See "Reflective Sheeting" on BC(4)).
- 6. Fabrication, erection and maintenance of the "ADVANCE SPEED LIMIT" (CW3-5) sign, "WORK ZONE" (G20-5aP) plaque and the "SPEED LIMIT" (R2-1) signs shall not be paid for directly, but shall be considered subsidiary to Item 502.
- 7. Turning signs from view, laying signs over or down will not be allowed, unless as otherwise noted under "REMOVING OR COVERING" on BC(4).
- 8. Techniques that may help reduce traffic speeds include but are not limited to:
  A. Law enforcement.
  - B. Flagger stationed next to sign.
  - C. Portable changeable message sign (PCMS).
  - D. Low-power (drone) radar transmitter.
  - E. Speed monitor trailers or signs.
- Speeds shown on details above are for illustration only.
   Work Zone Speed Limits should only be posted as approved for each project.
- 10. For more specific guidance concerning the type of work, work zone conditions and factors impacting allowable regulatory construction speed zone reduction see TxDOT form #1204 in the TxDOT e-form system.

SHEET 3 OF 12



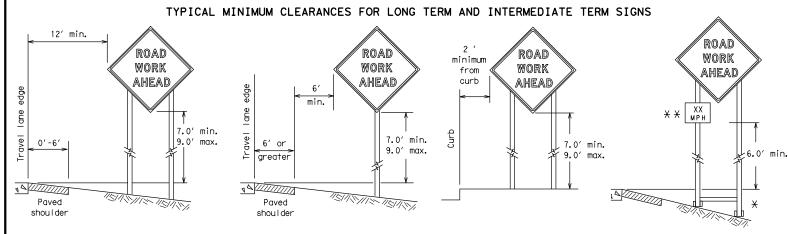
Traffic Safety Division Standard

# BARRICADE AND CONSTRUCTION WORK ZONE SPEED LIMIT

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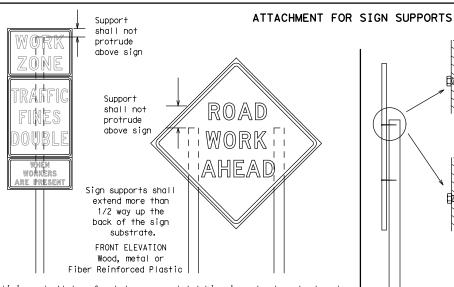
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\* When placing skid supports on unlevel ground, the leg post lengths must be adjusted so the sign appears straight and plumb. Objects shall NOT be placed under skids as a means of leveling.

\* X When plaques are placed on dual-leg supports, they should be attached to the upright nearest the travel lane. Supplemental plaques (advisory or distance) should not cover the surface of the parent sign.



Splicing embedded perforated square metal tubing in order to extend post height will only be allowed when the splice is made using four bolts, two above and two below the spice point. Splice must be located entirely behind the sign substrate, not near the base of the support. Splice insert lengths should be at least 5 times nominal post size, centered on the splice and of at least the same gauge material.

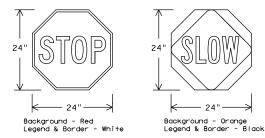
SIDE ELEVATION Wood

Attachment to wooden supports will be by bolts and nuts or screws. Use TxDOT's or manufacturer's recommended procedures for attaching sign substrates to other types of sign supports

Nails shall NOT be allowed. Each sign shall be attached directly to the sign support. Multiple signs shall not be joined or spliced by any means. Wood supports shall not be extended or repaired by splicing or other means.

#### STOP/SLOW PADDLES

- 1. STOP/SLOW paddles are the primary method to control traffic by flaggers. The STOP/SLOW paddle size should be 24" x 24". STOP/SLOW paddles shall be retroreflectorized when used at night.
- 3. STOP/SLOW paddles may be attached to a staff with a minimum length of 6' to the bottom of the sign.
- 4. Any lights incorporated into the STOP or SLOW paddle faces shall only be as specifically described in Section 6E.03 Hand Signaling Devices in the TMUTCD.



SHEETING REQUIREMENTS (WHEN USED AT NIGHT)					
USAGE	COLOR	SIGN FACE MATERIAL			
BACKGROUND	RED	TYPE B OR C SHEETING			
BACKGROUND	ORANGE	TYPE B <sub>FL</sub> OR C <sub>FL</sub> SHEETING			
LEGEND & BORDER	WHITE	TYPE B OR C SHEETING			
LEGEND & BORDER	BLACK	ACRYLIC NON-REFLECTIVE FILM			

#### CONTRACTOR REQUIREMENTS FOR MAINTAINING PERMANENT SIGNS WITHIN THE PROJECT LIMITS

- Permanent signs are used to give notice of traffic laws or regulations, call attention to conditions that are potentially hazardous to traffic operations, show route designations, destinations, directions, distances, services, points of interest, and other geographical, recreational, specific service (LOGO), or cultural information. Drivers proceeding through a work zone need the same, if not better route guidance as normally installed on a roadway without construction.
- When permanent regulatory or warning signs conflict with work zone conditions, remove or cover the permanent signs until the permanent sign message matches the roadway condition. For details for covering large guide signs see the TS-CD standard.
- When existing permanent signs are moved and relocated due to construction purposes, they shall be visible to motorists at all times.
- If existing signs are to be relocated on their original supports, they shall be installed on crashworthy bases as shown on the SMD Standard sheets. The signs shall meet the required mounting heights shown on the BC Sheets or the SMD Standards. This work should be paid for under the appropriate pay item for relocating existing signs.
- If permanent signs are to be removed and relocated using temporary supports. the Contractor shall use crashworthy supports as shown on the BC standard sheets, TLRS standard sheets or the CWZTCD list. The signs shall meet the required mounting heights shown on the BC, or the SMD standard sheets during construction. This work should be paid for under the appropriate pay item for relocating existing signs.
- Any sign or traffic control device that is struck or damaged by the Contractor or his/her construction equipment shall be replaced as soon as possible by the Contractor to ensure proper guidance for the motorists. This will be subsidiary to Item 502.

#### GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports
- All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and guide the traveling public safely through the work zone.
- The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so the Engineer can verify the correct procedures are being followed.
- The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used for identification shall be 1 inch.
- The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

#### <u>DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)</u>

- 1. The types of sign supports, sign mounting height,the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in regard to crashworthiness and duration of work requirements.
  - a. Long-term stationary work that occupies a location more than 3 days.
  - Intermediate-term stationary work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting more than one hour.
  - Short-term stationary daytime work that occupies a location for more than 1 hour in a single daylight period.
  - Short, duration work that occupies a location up to 1 hour.
  - Mobile work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

#### SIGN MOUNTING HEIGHT

- The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except as shown for supplemental plagues mounted below other signs.
- The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above
- the ground. Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to appropriate Long-term/Intermediate sign height.
- Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

#### SIZE OF SIGNS

1. The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

#### SIGN SUBSTRATES

- The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports.
- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave.
- 3. All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6" centers. The Engineer may approve other methods of splicing the sign face.

#### REFLECTIVE SHEETING

- 1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300
- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).
- White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type  $B_{FL}$  or Type  $C_{FL}$ , shall be used for rigid signs with orange backgrounds.

#### **SIGN LETTERS**

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of first class workmanship in accordance with Department Standards and Specifications.

#### REMOVING OR COVERING

- When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any intersections where the sign may be seen from approaching traffic.
- 3. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely covered when not required.
- When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.
- Burlap shall NOT be used to cover signs. Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

#### SIGN SUPPORT WEIGHTS

1. Where sign supports require the use of weights to keep from turning over, the use

- of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used. Rubber ballasts designed for channelizing devices should not be used for
- ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list. Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

#### FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

SHEET 4 OF 12

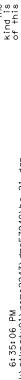


#### BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

Traffic Safety Division Standard

BC(4)-21

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© TxD0T	November 2002	CONT	SECT	JOB		H	HI GHWAY
	REVISIONS	0106	04	036		U	S 380
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13	5-21	ABL		STONEWA	۱LL		26

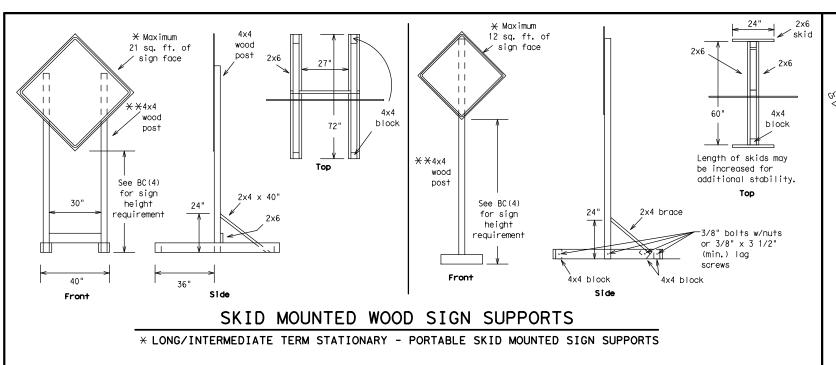


opposite sides going in opposite directions. Minimum

weld, do not

back fill puddle.

- weld starts here

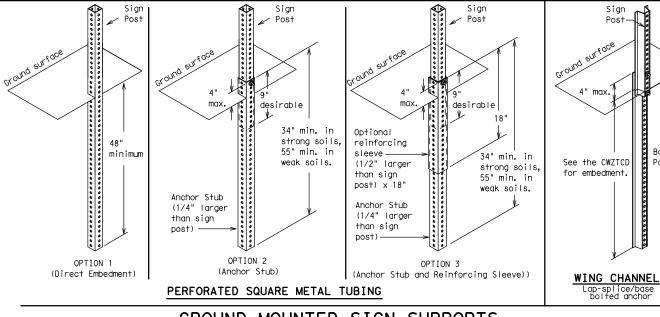


-2" x 2"

12 ga. upright

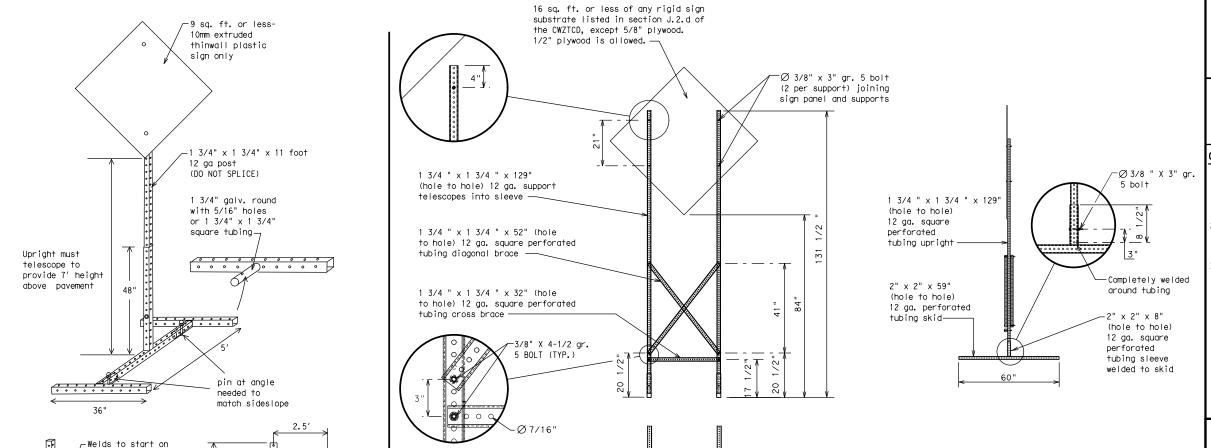
2"

SINGLE LEG BASE



#### GROUND MOUNTED SIGN SUPPORTS

Refer to the CWZTCD and the manufacturer's installation procedure for each type sign support. The maximum sign square footage shall adhere to the manufacturer's recommendation. Two post installations can be used for larger signs.



#### WEDGE ANCHORS

Both steel and plastic Wedge Anchor Systems as shown on the SMD Standard Sheets may be used as temporary sign supports for signs up to 10 square feet of sign face. They may be set in concrete or in sturdy soils if approved by the Engineer. (See web address for "Traffic Engineering Standard Sheets" on BC(1)).

#### OTHER DESIGNS

MORE DETAILS OF APPROVED LONG/INTERMEDIATE AND SHORT TERM SUPPORTS CAN BE FOUND ON THE CWZTCD LIST. SEE BC(1) FOR WEBSITE LOCATION.

#### GENERAL NOTES

- Nails may be used in the assembly of wooden sign supports, but 3/8" bolts with nuts or 3/8" x 3 1/2" lag screws must be used on every joint for final
- No more than 2 sign posts shall be placed within a 7 ft. circle, except for specific materials noted on the CWZTCD List.
- When project is completed, all sign supports and foundations shall be removed from the project site. This will be considered subsidiary to Item 502.
  - See BC(4) for definition of "Work Duration."
- ★★ Wood sign posts MUST be one piece. Splicing will NOT be allowed. Posts shall be painted white.
- ☐ See the CWZTCD for the type of sign substrate that can be used for each approved sign support.

#### SHEET 5 OF 12



Traffic Safety Division Standard

#### BARRICADE AND CONSTRUCTION TYPICAL SIGN SUPPORT

BC(5)-21

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9-07 8-14	DIST		COUNTY			SHEET NO.
7-13 5-21	ABL		STONEWA	ALL.		27

#### SKID MOUNTED PERFORATED SQUARE STEEL TUBING SIGN SUPPORTS

\* LONG/INTERMEDIATE TERM STATIONARY - PORTABLE SKID MOUNTED SIGN SUPPORTS

32′

WHEN NOT IN USE. REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

#### PORTABLE CHANGEABLE MESSAGE SIGNS

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- 2. Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO, "FOR." "AT." etc.
- 3. Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by
- 4. Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., "EXIT CLOSED." Do not use the term "RAMP."
- 5. Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- 7. The message term "WEEKEND" should be used only if the work is to start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- 8. The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- 11. Do not use the word "Danger" in message.
- 12. Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- 15. PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE	ABBREVIATION
Access Road	ACCS RD	Major	MAJ
Alternate	ALT	Miles	MI
Avenue	AVE	Miles Per Hour	MPH
Best Route	BEST RTE	Minor	MNR
Boulevard	BLVD	Monday	MON
Bridge	BRDG	Normal	NORM
Canno+	CANT	North	N
Center	CTR	Northbound	(route) N
Construction Ahead	CONST AHD	Parking	PKING
CROSSING	XING	Road	RD
Detour Route	DETOUR RTE	Right Lane	RT LN
Do Not	DONT	Saturday	SAT
East	F	Service Road	SERV RD
Eastbound	(route) E	Shoulder	SHLDR
Emergency	EMER	Slippery	SLIP
Emergency Vehicle		South	S
Entrance, Enter	ENT	Southbound	(route) S
Express Lane	EXP LN	Speed	SPD
Expressway	EXPWY	Street	ST
XXXX Feet	XXXX FT	Sunday	SUN
Fog Ahead	FOG AHD	Telephone	PHONE
Freeway	FRWY, FWY	Temporary	TEMP
Freeway Blocked	FWY BLKD	Thursday	THURS
Friday	FRI	To Downtown	TO DWNTN
Hazardous Driving		Traffic	TRAF
Hazardous Material		Travelers	TRVLRS
High-Occupancy	HOV	Tuesday	TUES
Vehicle		Time Minutes	TIME MIN
Highway	HWY	Upper Level	UPR LEVEL
Hour (s)	HR. HRS	Vehicles (s)	VEH, VEHS
Information	INFO	Warning	WARN
It Is	ITS	Wednesday	WED
Junction	JCT	Weight Limit	WT LIMIT
Left	LFT	West	W
Left Lane	LFT LN	Westbound	(route) W
Lane Closed	LN CLOSED	Wet Pavement	WET PVMT
Lower Level	LWR LEVEL	Will Not	WONT
Maintenance	MAINT		

designation # IH-number, US-number, SH-number, FM-number

#### RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES

(The Engineer may approve other messages not specifically covered here.)

#### Phase 1: Condition Lists

oaa/Lane/Ramp	o Closure List	Other Cond	dition List
FREEWAY CLOSED X MILE	FRONTAGE ROAD CLOSED	ROADWORK XXX FT	ROAD REPAIRS XXXX FT
ROAD CLOSED AT SH XXX	SHOULDER CLOSED XXX FT	FLAGGER XXXX FT	LANE NARROWS XXXX FT
ROAD CLSD AT FM XXXX	RIGHT LN CLOSED XXX FT	RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE
RIGHT X LANES CLOSED	RIGHT X LANES OPEN	MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT
CENTER LANE CLOSED	DAYTIME LANE CLOSURES	LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT
NIGHT LANE CLOSURES	I-XX SOUTH EXIT CLOSED	DETOUR X MILE	ROUGH ROAD XXXX FT
VARIOUS LANES CLOSED	EXIT XXX CLOSED X MILE	ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN
EXIT CLOSED	RIGHT LN TO BE CLOSED	BUMP XXXX FT	US XXX EXIT X MILES
MALL DRIVEWAY CLOSED	X LANES CLOSED TUE - FRI	TRAFFIC SIGNAL XXXX FT	LANES SHIFT
XXXXXXXX			

# LANE

#### Phase 2: Possible Component Lists

mp Closure List	Other Cond	dition List		Effect on Travelist	Location List	Warning List	* * Advance Notice List
FRONTAGE ROAD CLOSED	ROADWORK XXX FT	ROAD REPAIRS XXXX FT	MERGE RIGHT	FORM X LINES RIGHT	AT FM XXXX	SPEED LIMIT XX MPH	TUE-FRI XX AM- X PM
SHOULDER CLOSED XXX FT	FLAGGER XXXX FT	LANE NARROWS XXXX FT	DETOUR NEXT X EXITS	USE XXXXX RD EXIT	BEFORE RAILROAD CROSSING	MAXIMUM SPEED XX MPH	APR XX- XX X PM-X AM
RIGHT LN CLOSED XXX FT	RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE	USE EXIT XXX	USE EXIT I-XX NORTH	NEXT X MILES	MINIMUM SPEED XX MPH	BEGINS MONDAY
RIGHT X LANES OPEN	MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT	STAY ON US XXX SOUTH	USE I-XX E TO I-XX N	PAST US XXX EXIT	ADVISORY SPEED XX MPH	BEGINS MAY XX
DAYTIME LANE CLOSURES	LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT	TRUCKS USE US XXX N	WATCH FOR TRUCKS	XXXXXXX TO XXXXXXX	RIGHT LANE EXIT	MAY X-X XX PM - XX AM
I-XX SOUTH EXIT CLOSED	DETOUR X MILE	ROUGH ROAD XXXX FT	WATCH FOR TRUCKS	EXPECT DELAYS	US XXX TO FM XXXX	USE CAUTION	NEXT FRI-SUN
EXIT XXX CLOSED X MILE	ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN	EXPECT DELAYS	PREPARE TO STOP		DRIVE SAFELY	XX AM TO XX PM
RIGHT LN TO BE CLOSED	BUMP XXXX FT	US XXX EXIT X MILES	REDUCE SPEED XXX FT	END SHOULDER USE		DRIVE WITH CARE	NEXT TUE AUG XX
X LANES CLOSED TUE - FRI	TRAFFIC SIGNAL XXXX FT	LANES SHIFT *	USE OTHER ROUTES	WATCH FOR WORKERS			TONIGHT XX PM- XX AM
X LANES SHIFT in Phas	se 1 must be used with	n STAY IN LANE in Phase 2	STAY IN		<b>* *</b> Se	e Application Guidelin	es Note 6.

#### APPLICATION GUIDELINES

- 1. Only 1 or 2 phases are to be used on a PCMS.
- 2. The 1st phase (or both) should be selected from the "Road/Lane/Ramp Closure List" and the "Other Condition List".
- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- 4. A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- 5. If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

#### WORDING ALTERNATIVES

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- 2. Roadway designations IH, US, SH, FM and LP can be interchanged as appropriate.
- 3. EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- 5. ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary.
- 7. FT and MI. MILE and MILES interchanged as appropriate.
- 8. AT. BEFORE and PAST interchanged as needed.
- 9. Distances or AHEAD can be eliminated from the message if a location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

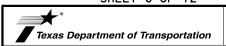
#### FULL MATRIX PCMS SIGNS

BLVD

CLOSED

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 under "PORTABLE CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol"(CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of the Engineer, it shall maintain the legibility/visibility requirement listed above.
- 3. When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and shall not substitute for, or replace that sign.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC(7), for the same size arrow.

#### SHEET 6 OF 12



#### BARRICADE AND CONSTRUCTION PORTABLE CHANGEABLE MESSAGE SIGN (PCMS)

BC(6)-21

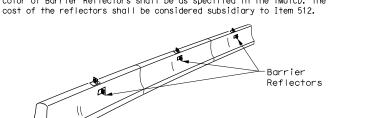
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© TxD0T	November 2002	CONT	SECT	JOB		н	IGHWAY
	REVISIONS	0106	04	036		U:	380
9-07	8-14	DIST		COUNTY			SHEET NO.
7-13 5-21		ABL	STONEWALL				28

6:35:07

Warning reflector may be round or square. Must have a yellow reflective surface area of at least 30 square inches

#### 1. Barrier Reflectors shall be pre-qualified, and conform to the color and reflectivity requirements of DMS-8600. A list of pregualified Barrier Reflectors can be found at the Material Producer List web address shown on BC(1).

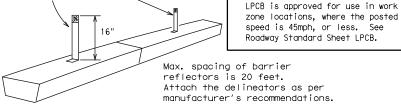
2. Color of Barrier Reflectors shall be as specified in the TMUTCD. The



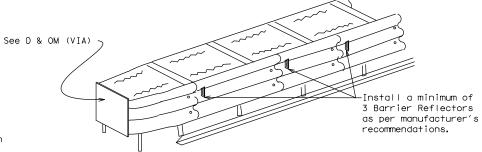
#### CONCRETE TRAFFIC BARRIER (CTB)

- Where traffic is on one side of the CTB, two (2) Barrier Reflectors shall be mounted in approximately the midsection of each section of CTB. An alternate mounting location is uniformly spaced at one end of each CTB. This will allow for attachment of a barrier grapple without damaging the reflector. The Barrier Reflector mounted on the side of the CTB shall be located directly below the reflector mounted on top of the barrier, as shown in the detail above.
- Where CTB separates two-way traffic, three barrier reflectors shall be mounted on each section of CTB. The reflector unit on top shall have two yellow reflective faces (Bi-Directional) while the reflectors on each side of the barrier shall have one yellow reflective face, as shown in the detail above.
- 5. When CTB separates traffic traveling in the same direction, no barrier reflectors will be required on top of the CTB.
- 6. Barrier Reflector units shall be yellow or white in color to match the edgeline being supplemented.
- 7. Maximum spacing of Barrier Reflectors is forty (40) feet.
- 8. Pavement markers or temporary flexible-reflective roadway marker tabs shall NOT be used as CTB delineation.
- 9. Attachment of Barrier Reflectors to CTB shall be per manufacturer's
- 10.Missing or damaged Barrier Reflectors shall be replaced as directed
- 11. Single slope barriers shall be delineated as shown on the above detail.

#### LOW PROFILE CONCRETE BARRIER (LPCB) USED Barrier Reflector on 16" tall plastic bracket IN WORK ZONES



#### LOW PROFILE CONCRETE BARRIER (LPCB)



#### DELINEATION OF END TREATMENTS

#### END TREATMENTS FOR CTB'S USED IN WORK ZONES

End treatments used on CTB's in work zones shall meet the apppropriate crashworthy standards as defined in the Manual for Assessing Safety Hardware (MASH), Refer to the CWZTCD List for approved end treatments and manufacturers.

#### BARRIER REFLECTORS FOR CONCRETE TRAFFIC BARRIER AND ATTENUATORS

#### WARNING LIGHTS

- 1. Warning lights shall meet the requirements of the TMUTCD.
- 2. Warning lights shall NOT be installed on barricades.
- 3. Type A-Low Intensity Flashing Warning Lights are commonly used with drums. They are intended to warn of or mark a potentially hazardous area. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "FL". The Type A Warning Lights shall not be used with signs manufactured with Type  $B_{FL}$  or  $C_{FL}$  Sheeting meeting the requirements of Departmental Material Specification DMS-8300.
- 4. Type-C and Type D 360 degree Steady Burn Lights are intended to be used in a series for delineation to supplement other traffic control devices. Their use shall be as indicated on this sheet and/or other sheets of the plans by the designation "SB".
- 5. The Engineer/Inspector or the plans shall specify the location and type of warning lights to be installed on the traffic control devices.
- 6. When required by the Engineer, the Contractor shall furnish a copy of the warning lights certification. The warning light manufacturer will certify the warning lights meet the requirements of the latest ITE Purchase Specifications for Flashing and Steady-Burn Warning Lights. 7. When used to delineate curves, Type-C and Type D Steady Burn Lights should only be placed on the outside of the curve, not the inside.
- 8. The location of warning lights and warning reflectors on drums shall be as shown elsewhere in the plans.

#### WARNING LIGHTS MOUNTED ON PLASTIC DRUMS

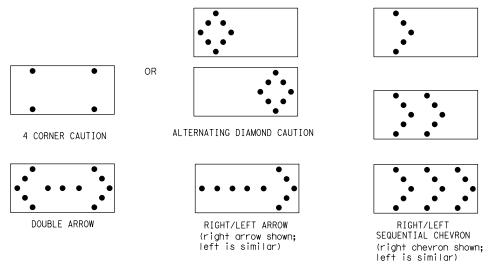
- 1. Type A flashing warning lights are intended to warn drivers that they are approaching or are in a potentially hazardous area.
- 2. Type A random flashing warning lights are not intended for delineation and shall not be used in a series.
- 3. A series of sequential flashing warning lights placed on channelizing devices to form a merging taper may be used for delineation. If used, the successive flashing of the sequential warning lights should occur from the beginning of the taper to the end of the merging taper in order to identify the desired vehicle path. The rate of flashing for each light shall be 65 flashes per minute, plus or minus 10 flashes.
- 4. Type C and D steady-burn warning lights are intended to be used in a series to delineate the edge of the travel lane on detours, on lane changes, on lane closures, and on other similar conditions.
- 5. Type A, Type C and Type D warning lights shall be installed at locations as detailed on other sheets in the plans.
- 6. Warning lights shall not be installed on a drum that has a sign, chevron or vertical panel.
- 7. The maximum spacing for warning lights on drums should be identical to the channelizing device spacing.

#### WARNING REFLECTORS MOUNTED ON PLASTIC DRUMS AS A SUBSTITUTE FOR TYPE C (STEADY BURN) WARNING LIGHTS

- 1. A warning reflector or approved substitute may be mounted on a plastic drum as a substitute for a Type C, steady burn warning light at the discretion of the Contractor unless otherwise noted in the plans.
- 2. The warning reflector shall be yellow in color and shall be manufactured using a sign substrate approved for use with plastic drums listed
- 3. The warning reflector shall have a minimum retroreflective surface area (one-side) of 30 square inches.
- 4. Round reflectors shall be fully reflectorized, including the area where attached to the drum.
- 5. Square substrates must have a minimum of 30 square inches of reflectorized sheeting. They do not have to be reflectorized where it
- 6. The side of the warning reflector facing approaching traffic shall have sheeting meeting the color and retroreflectivity requirements for DMS 8300-Type B or Type C.
- 7. When used near two-way traffic, both sides of the warning reflector shall be reflectorized.
- 8. The warning reflector should be mounted on the side of the handle nearest approaching traffic.
- 9. The maximum spacing for warning reflectors should be identical to the channelizing device spacing requirements.

Arrow Boards may be located behind channelizing devices in place for a shoulder taper or merging taper, otherwise they shall be delineated with four (4) channelizing devices placed perpendicular to traffic on the upstream side of traffic.

- 1. The Flashing Arrow Board should be used for all lane closures on multi-lane roadways, or slow moving maintenance or construction activities on the travel lanes.
- 2. Flashing Arrow Boards should not be used on two-lane, two-way roadways, detours, diversions or work on shoulders unless the "CAUTION" display (sée detail below) is used.
- The Engineer/Inspector shall choose all appropriate signs, barricades and/or other traffic control devices that should be used in conjunction with the Flashing Arrow Board.
- 4. The Flashing Arrow Board should be able to display the following symbols:



- 5. The "CAUTION" display consists of four corner lamps flashing simultaneously, or the Alternating Diamond Caution mode as shown.
- The straight line caution display is NOT ALLOWED.
- The Flashing Arrow Board shall be capable of minimum 50 percent dimming from rated lamp voltage. The flashing rate of the lamps shall not be less than 25 nor more than 40 flashes per minute.
- Minimum lamp "on time" shall be approximately 50 percent for the flashing arrow and equal intervals of 25 percent for each sequential phase of the flashing chevron.
- 9. The sequential arrow display is NOT ALLOWED.
  10. The flashing arrow display is the TxDOT standard; however, the sequential chevron display may be used during daylight operations.
- 11. The Flashing Arrow Board shall be mounted on a vehicle, trailer or other suitable support.
  12. A Flashing Arrow Board SHALL NOT BE USED to laterally shift traffic.
  13. A full matrix PCMS may be used to simulate a Flashing Arrow Board provided it meets visibility,
- flash rate and dimming requirements on this sheet for the same size arrow.
- 14. Minimum mounting height of trailer mounted Arrow Boards should be 7 feet from roadway to bottom of panel.

	REQUIREMENTS							
TYPE	MINIMUM SIZE	MINIMUM NUMBER OF PANEL LAMPS	MINIMUM VISIBILITY DISTANCE					
В	30 × 60	13	3/4 mile					
С	48 × 96	15	1 mile					

ATTENTION Flashing Arrow Boards shall be equipped with automatic dimmina devices.

WHEN NOT IN USE, REMOVE THE ARROW BOARD FROM THE RIGHT-OF-WAY OR PLACE THE ARROW BOARD BEHIND CONCRETE TRAFFIC BARRIER OR GUARDRAIL.

#### FLASHING ARROW BOARDS

SHEET 7 OF 12

#### TRUCK-MOUNTED ATTENUATORS

- 1. Truck-mounted attenuators (TMA) used on TxDOT facilities must meet the requirements outlined in the Manual for Assessing Safety Hardware (MASH).
- Refer to the CWZTCD for the requirements of Level 2 or Level 3 TMAs.
- 3. Refer to the CWZTCD for a list of approved TMAs.
- 4. TMAs are required on freeways unless otherwise noted n the plans.
- 5. A TMA should be used anytime that it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the work performance.
- 6. The only reason a TMA should not be required is when a work area is spread down the roadway and the work crew is an extended distance from the TMA.



Traffic Safety Division Standard

#### BARRICADE AND CONSTRUCTION ARROW PANEL, REFLECTORS, WARNING LIGHTS & ATTENUATOR

BC(7)-21

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#### GENERAL NOTES

- For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CMUTCD).
- Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

#### GENERAL DESIGN REQUIREMENTS

Pre-qualified plastic drums shall meet the following requirements:

- Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- to be held down while separating the drum body from the base.

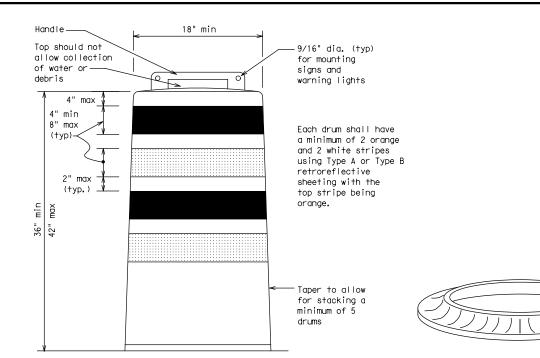
  8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

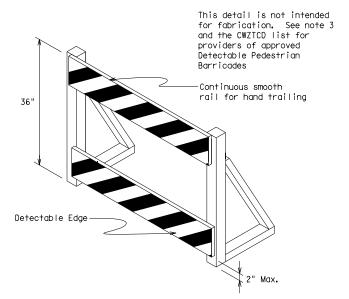
#### RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

#### **BALLAST**

- 1. Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- . Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.





#### DETECTABLE PEDESTRIAN BARRICADES

- 1. When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
- Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- 3. Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian path.
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian movements.
- Warning lights shall not be attached to detectable pedestrian barricades.
- 6. Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.



18" x 24" Sign (Maximum Sign Dimension) Chevron CW1-8, Opposing Traffic Lane Divider, Driveway sign D70a, Keep Right R4 series or other signs as approved by Engineer

See Ballast



12" x 24"
Vertical Panel
mount with diagonals
sloping down towards
travel way

Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums

SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS

- Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD.
- 2. Chevrons and other work zone signs with an orange background shall be manufactured with Type  $\rm B_{FL}$  or Type  $\rm C_{FL}$  Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans.
- 3. Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A or Type B. Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane.
- 4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below.
- Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection
- Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts.
- 7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations, they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans.
- R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer.

SHEET 8 OF 12

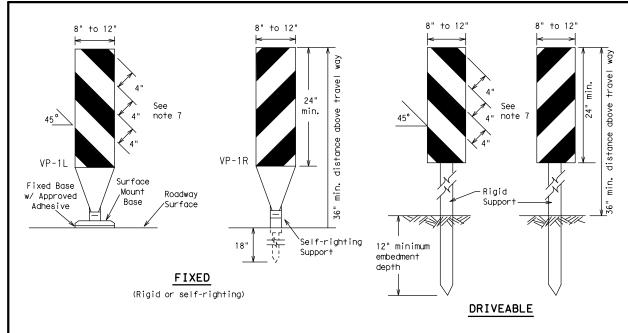


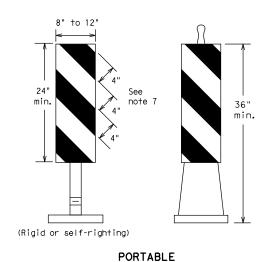
Traffic Safety Division Standard

# BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

BC(8)-21

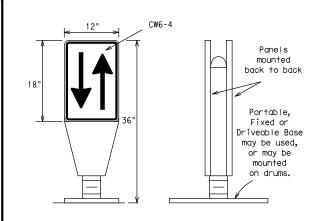
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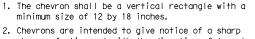
- 1. Vertical Panels (VP's) are normally used to channelize traffic or divide opposing lanes of traffic.
- 2. VP's may be used in daytime or nighttime situations. They may be used at the edge of shoulder drop-offs and other areas such as lane transitions where positive daytime and nighttime delineation is required. The Engineer/Inspector shall refer to the Roadway Design Manual for additional requirements on the use VP's for drop-offs.
- 3. VP's should be mounted back to back if used at the edge of cuts adjacent to two-way two lane roadways. Stripes are to be reflective orange and reflective white and should always slope downward toward the travel lane.
- 4. VP's used on expressways and freeways or other high speed roadways, may have more than 270 square inches of retroreflective area facing traffic.
- Self-righting supports are available with portable base.
   See "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- Sheeting for the VP's shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300, unless noted otherwise.
- Where the height of reflective material on the vertical panel is 36 inches or greater, a panel stripe of 6 inches shall be used.

#### VERTICAL PANELS (VPs)



- 1. Opposing Traffic Lane Dividers (OTLD) are delineation devices designed to convert a normal one-way roadway section to two-way operation. OTLD's are used on temporary centerlines. The upward and downward arrows on the sign's face indicate the direction of traffic on either side of the divider. The base is secured to the pavement with an adhesive or rubber weight to minimize movement caused by a vehicle impact or wind gust.
- The OTLD may be used in combination with 42" cones or VPs.
- Spacing between the OTLD shall not exceed 500 feet. 42" cones or VPs placed between the OTLD's should not exceed 100 foot spacing.
- 4. The OTLD shall be orange with a black non-reflective legend. Sheeting for the OTLD shall be retroreflective Type B<sub>FL</sub> or Type C<sub>FL</sub> conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.

OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

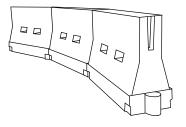


- Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the outside of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type B<sub>FL</sub> or Type C<sub>FL</sub> conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

#### CHEVRONS

#### **GENERAL NOTES**

- Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- 6. Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.



#### LONGITUDINAL CHANNELIZING DEVICES (LCD)

36"

Fixed Base w/ Approved Adhesive

(Driveable Base, or Flexible

Support can be used)

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- 2. LCDs may be used instead of a line of cones or drums.
- LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- $\hbox{4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers. } \\$
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

#### WATER BALLASTED SYSTEMS USED AS BARRIERS

- Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings.
- 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- 5. When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

Posted Speed	Formula	Desirable Taper Lengths <del>X</del> <del>X</del>			Spacing of Channelizing Devices				
		10′ Offset	11' Offset	12' Offset	On a Taper	On a Tangent			
30	, WS <sup>2</sup>	150′	165′	180′	30′	60′			
35	L= WS	205′	225′	245′	35′	70′			
40	80	265′	295′	320′	40′	80′			
45		450′	495′	540′	45′	90′			
50		500′	550′	600′	50′	100′			
55	L=WS	550′	605′	660′	55′	110′			
60	L 113	600′	660′	720′	60′	120′			
65		650′	715′	780′	65 <i>′</i>	130′			
70		700′	770′	840′	70′	140′			
75		750′	825′	900′	75′	150′			
80		800′	880′	960′	80′	160′			
V V Taner Lengths have been reunded off									

\*\*X\*Taper lengths have been rounded off.
L=Length of Taper (FT.) W=Width of Offset (FT.)
S=Posted Speed (MPH)

# SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12



Traffic Safety Division Standard

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# BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

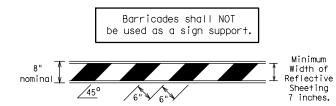
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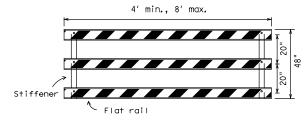
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#### TYPE 3 BARRICADES

- 1. Refer to the Compliant Work Zone Traffic Control Devices List (CWZTCD) for details of the Type 3 Barricades and a list of all materials used in the construction of Type 3 Barricades.
- Type 3 Barricades shall be used at each end of construction projects closed to all traffic.
- 3. Barricades extending across a roadway should have stripes that slope downward in the direction toward which traffic must turn in detouring When both right and left turns are provided, the chevron striping may slope downward in both directions from the center of the barricade. Where no turns are provided at a closed road, striping should slope downward in both directions toward the center of roadway.
- 4. Striping of rails, for the right side of the roadway, should slope downward to the left. For the left side of the roadway, striping should slope downward to the right.
- Identification markings may be shown only on the back of the barricade rails. The maximum height of letters and/or company logos used for identification shall be 1".
- 6. Barricades shall not be placed parallel to traffic unless an adequate clear zone is provided.
- Warning lights shall NOT be installed on barricades.
- 8. Where barricades require the use of weights to keep from turning over. the use of sandbags with dry, cohesionless sand is recommended. The sandbags will be tied shut to keep the sand from spilling and to maintain a constant weight. Sand bags shall not be stacked in a manner that covers any portion of a barricade rails reflective sheeting. Rock, concrete, iron, steel or other solid objects will NOT be permitted. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs. Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall not be used for sandbags. Sandbags shall only be placed along or upon the base supports of the device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners.
- Sheeting for barricades shall be retroreflective Type A or Type B conforming to Departmental Material Specification DMS-8300 unless otherwise noted.

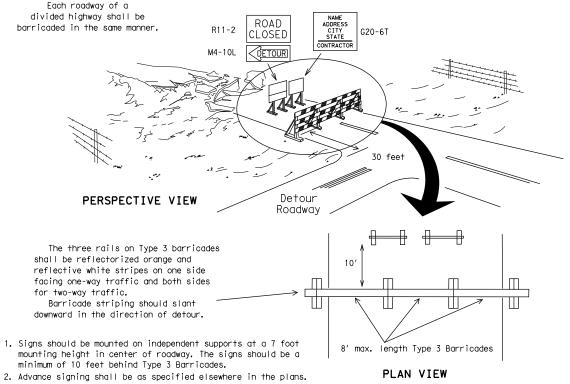


#### TYPICAL STRIPING DETAIL FOR BARRICADE RAIL



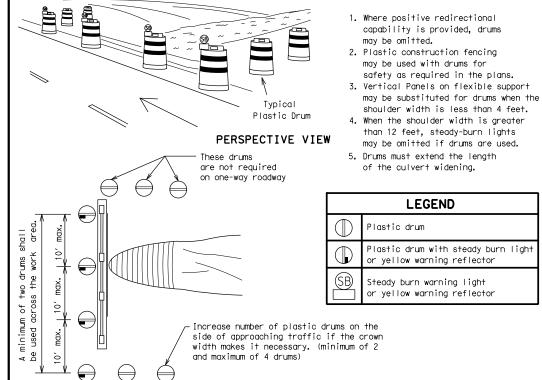
Stiffener may be inside or outside of support, but no more than 2 stiffeners shall be allowed on one barricade.

### TYPICAL PANEL DETAIL



TYPE 3 BARRICADE (POST AND SKID) TYPICAL APPLICATION

Two-Piece cones



CONES \_ 4" min. orange 2" min. white 2" min. [6" min. 4" min. orange \_2" min. 2" min. 4" min. white 42" min. 28' min.

4" min. 28"

PLAN VIEW

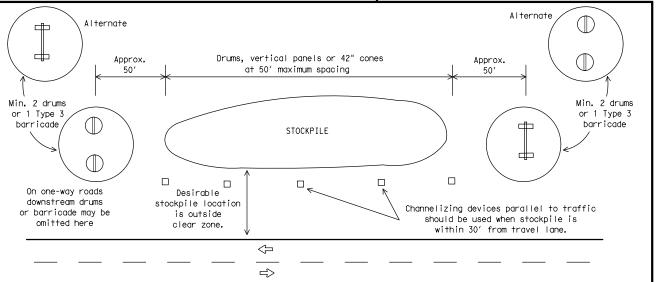
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CULVERT WIDENING OR OTHER ISOLATED WORK WITHIN THE PROJECT LIMITS

One-Piece cones

Tubular Marker

FOR SKID OR POST TYPE BARRICADES



TRAFFIC CONTROL FOR MATERIAL STOCKPILES

28" Cones shall have a minimum weight of 9 1/2 lbs.

42" 2-piece cones shall have a minimum weight of 30 lbs. including base.

- 1. Traffic cones and tubular markers shall be predominantly orange, and meet the height and weight requirements shown above.
- 2. One-piece cones have the body and base of the cone molded in one consolidated unit. Two-piece cones have a cone shaped body and a separate rubber base, or ballast, that is added to keep the device upright and in place.
- 3. Two-piece cones may have a handle or loop extending up to 8" above the minimum height shown, in order to aid in retrieving the device.
- 4. Cones or tubular markers shall have white or white and orange reflective bands as shown above. The reflective bands shall have a smooth, sealed outer surface and meet the requirements of Departmental Material Specification DMS-8300 Type A or Type B.
- 5. 28" cones and tubular markers are generally suitable for short duration and short-term stationary work as defined on BC(4). These should not be used for intermediate-term or long-term stationary work unless personnel is on-site to maintain them in their proper upright position.
- 6. 42" two-piece cones, vertical panels or drums are suitable for all work zone durations.
- 7. Cones or tubular markers used on each project should be of the same size and shape.

SHEET 10 OF 12



BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

Traffic Safety Division Standard

BC(10)-21

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#### WORK ZONE PAVEMENT MARKINGS

#### GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- Additional supplemental pavement marking details may be found in the plans or specifications.
- Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- 7. All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

#### RAISED PAVEMENT MARKERS

- Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

#### PREFABRICATED PAVEMENT MARKINGS

- Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

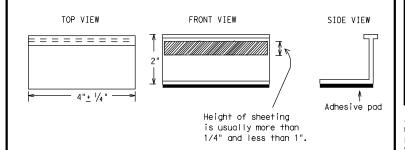
#### MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

#### REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- 5. Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10. Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

#### Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECURE TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARKER TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guidemarks shall meet the requirements of DMS-8242.
- 2. Tabs detailed on this sheet are to be inspected and accepted by the Engineer or designated representative. Sampling and testing is not normally required, however at the option of the Engineer, either "A" or "B" below may be imposed to assure quality before placement on the roadway.
  - A. Select five (5) or more tabs at random from each lot or shipment and submit to the Construction Division, Materials and Pavement Section to determine specification compliance.
  - B. Select five (5) tabs and perform the following test. Affix five (5) tabs at 24 inch intervals on an asphaltic pavement in a straight line. Using a medium size passenger vehicle or pickup, run over the markers with the front and rear tires at a speed of 35 to 40 miles per hour, four (4) times in each direction. No more than one (1) out of the five (5) reflective surfaces shall be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. See Standard Sheet TCP(7-1) for tab placement on seal coat work.

#### RAISED PAVEMENT MARKERS USED AS GUIDEMARKS

- Raised pavement markers used as guidemarks shall be from the approved product list, and meet the requirements of DMS-4200.
- 2. All temporary construction raised pavement markers provided on a project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applied or butyl rubber pad for all surfaces, or thermoplastic for concrete surfaces.
- Guidemarks shall be designated as:
  YELLOW (two amber reflective surfaces with yellow body).
  WHITE (one silver reflective surface with white body).

DEPARTMENTAL MATERIAL SPECIFICATIO	NS
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
TRAFFIC BUTTONS	DMS-4300
EPOXY AND ADHESIVES	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS	DMS-8241
TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242

A list of prequalified reflective raised pavement markers, non-reflective traffic buttons, roadway marker tabs and other pavement markings can be found at the Material Producer List web address shown on BC(1).

SHEET 11 OF 12



Traffic Safety Division Standard

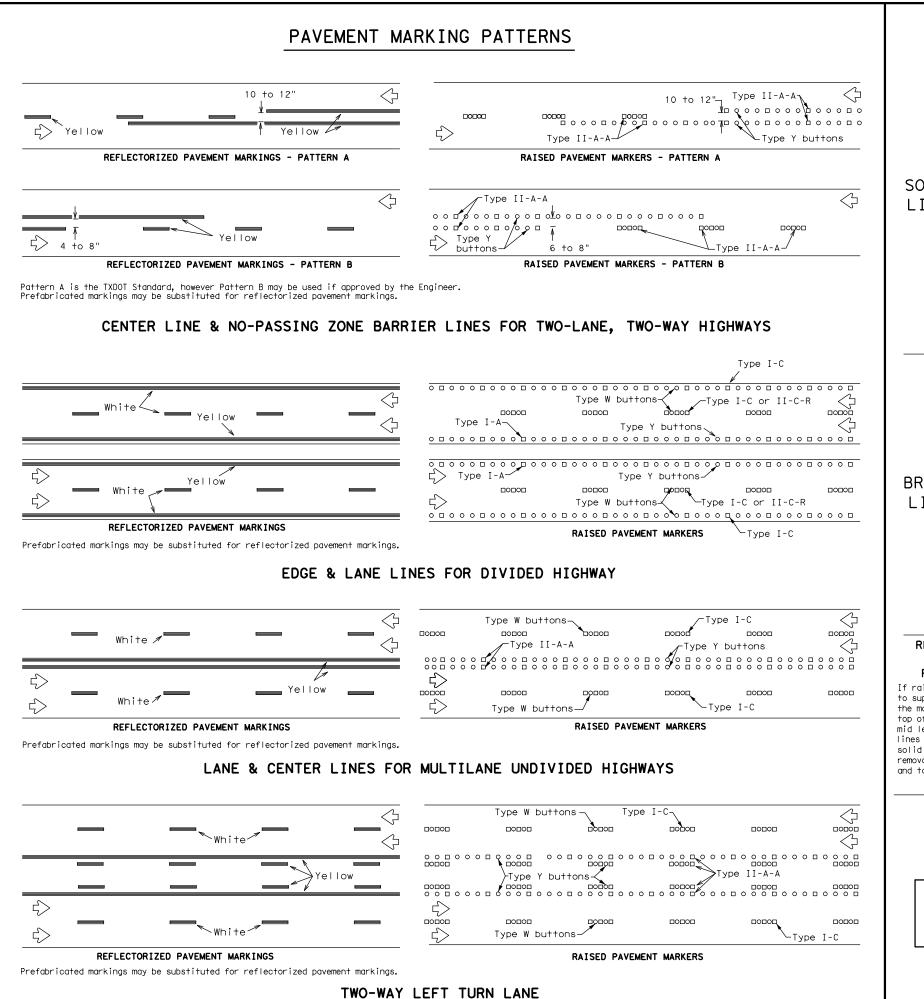
# BARRICADE AND CONSTRUCTION PAVEMENT MARKINGS

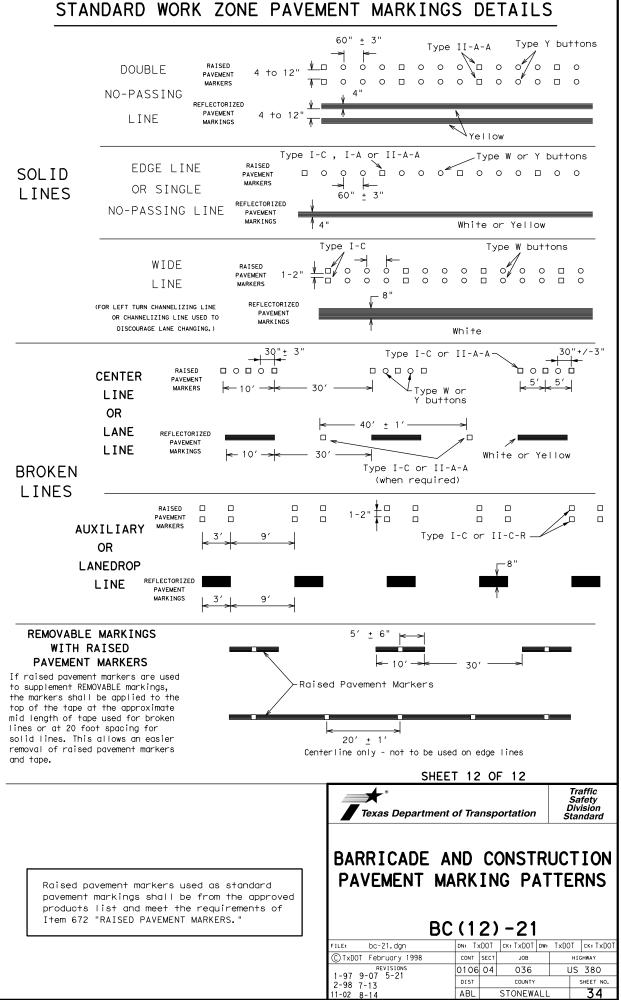
BC(11)-21

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© TxDOT February 1998		SECT	JOB		н	HIGHWAY	
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2-98 9-07 5-21 1-02 7-13	DIST		COUNTY			SHEET NO.	
11-02 8-14	ABL	STONEWALL 33				33	

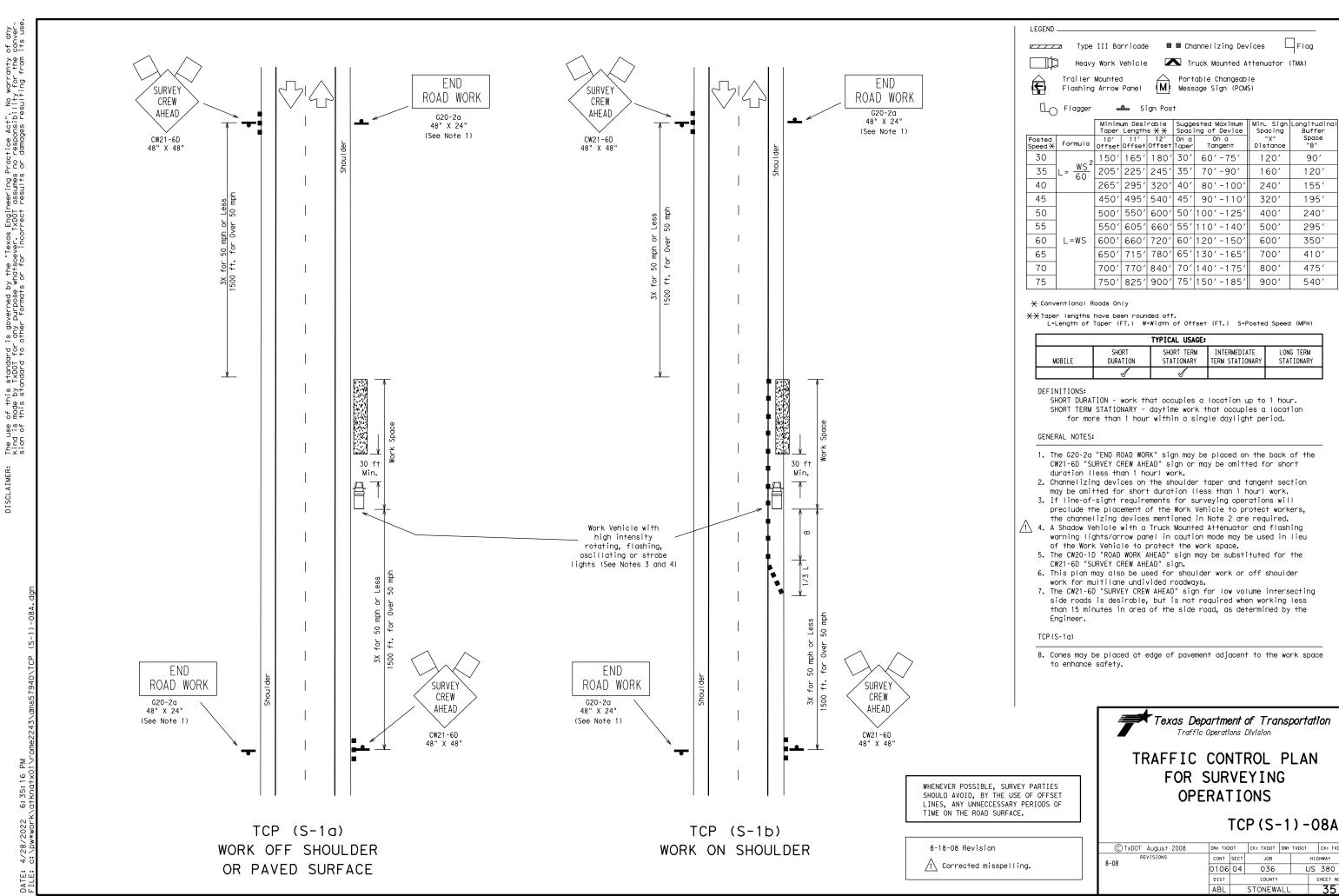
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1-02 E





STONEWALL

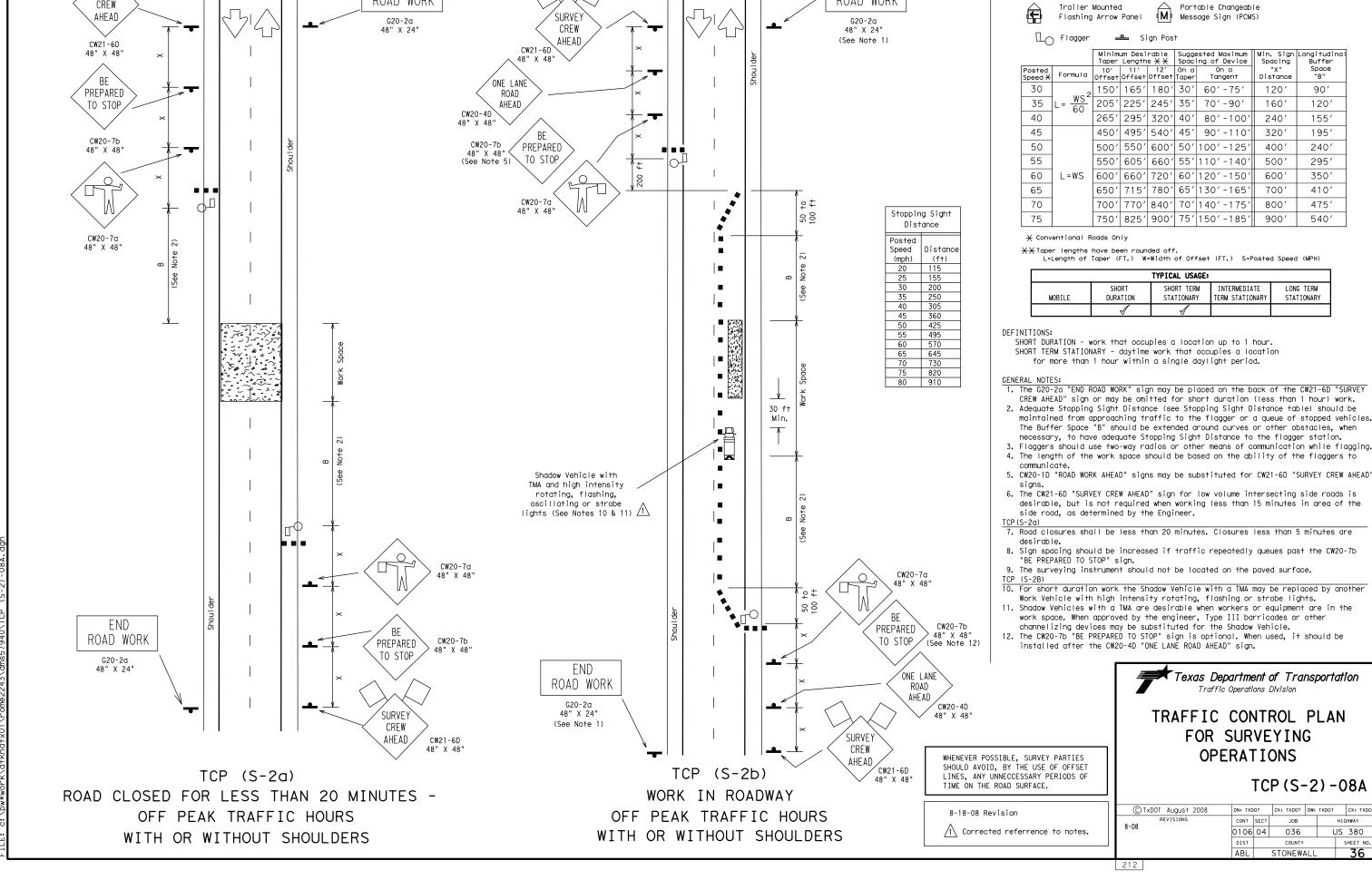


DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO US 380



CREW





END

ROAD WORK

Type III Barricade

Trailer Mounted

Heavy Work Vehicle

Flag

■ Channelizing Devices

Portable Changeable

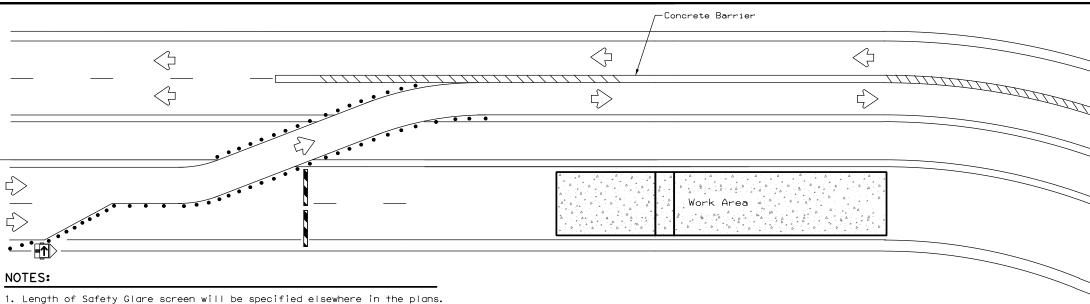
Truck Mounted Attenuator (TMA)

LEGEND

FND

ROAD WORK

6:35:35 0+kpd+v0



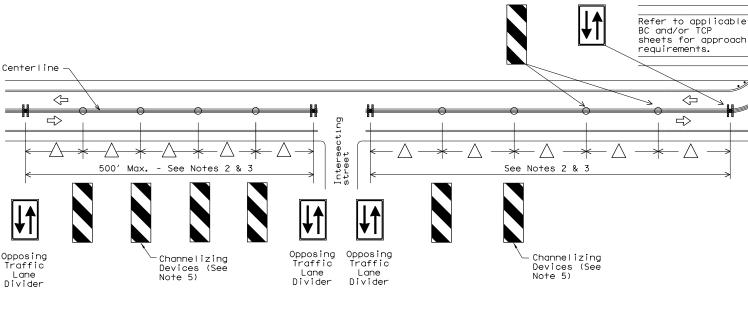
# BARRIER DELINEATION WITH MODULAR GLARE SCREENS

	LEGEND					
	Type 3 Barricade					
• • •	• • • Channelizing Devices					
<b>E</b>	Trailer Mounted Flashing Arrow Board					
_	<b>♣</b> Sign					
\\\\ Safety glare screen						

DEPARTMENTAL MATERIAL SPECIFICA	ATIONS
SIGN FACE MATERIALS	DMS-8300
DELINEATORS AND OBJECT MARKERS	DMS-8600
MODULAR GLARE SCREENS FOR HEADLIGHT BARRIER	DMS-8610

Only pre-qualified products shall be used. A copy of the Compliant Work Zone Traffic Control Devices List" CWZTCD)describes pre-qualified products and their sources and may be found at the following web address:

http://www.txdot.gov/business/resources/producer-list.html



VERTICAL PANELS & OPPOSING TRAFFIC LANE DIVIDERS (OTLD) SEPARATING TWO-WAY TRAFFIC ON NORMALLY DIVIDED HIGHWAYS

traffic barrier on which they are installed so the joint between barrier sections will not be spanned by any one safety glare screen unit.

to the edge of the panel/blade. The sheeting shall be attached to one glare screen panel/blade per section of concrete barrier not to exceed a spacing of 30 feet. Barrier reflectors are not necessary when panel/blades

4. Payment for these devices will be under statewide Special Specification

are installed with reflective sheeting as described.

'Modular Glare Screens for Headlight Barrier.'

be as shown elsewhere in the plans.

#### NOTES:

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- When two-lane, two way traffic control must be maintained on one roadway of a normally divided highway, opposing traffic shall be separated with either temporary traffic barriers, channelizing devices, or a temporary raised island throughout the length of the two way operation. The above Typical Application is intended to show the appropriate application of channelizing devices when they are used for this purpose. This is not a traffic control plan. If this detail is to be used for other types of roads or applications, those locations should be stated elsewhere in the
- Space devices according to the Tangent Spacing shown on the Device Spacing table on BC(9) but not exceeding 100'.
- Every fifth device should be an OTLD except when spaced closer to accommodate an intersection. An OTLD should be the first device on each side of intersecting streets or roads.
- 4. Locations where surface mount bases with adhesives or self-righting devices will be required in order to maintain them in their proper position should be noted elsewhere in the plans.
- 5. Channelizing devices are to be vertical panels, 42" cones or tubular markers that are at least 36" tall. Tubular markers used to separate traffic should have a rubber base weighing at least 30 pounds. Tubular markers that are 42" tall or more shall have four bands of reflective material as detailed for 42" cones on BC(10). Tubular markers less than 42" but at least 36" tall shall have three bands of 3" wide white reflective material spaced 2" apart. Reflective material shall meet DMS-8300, Type A.



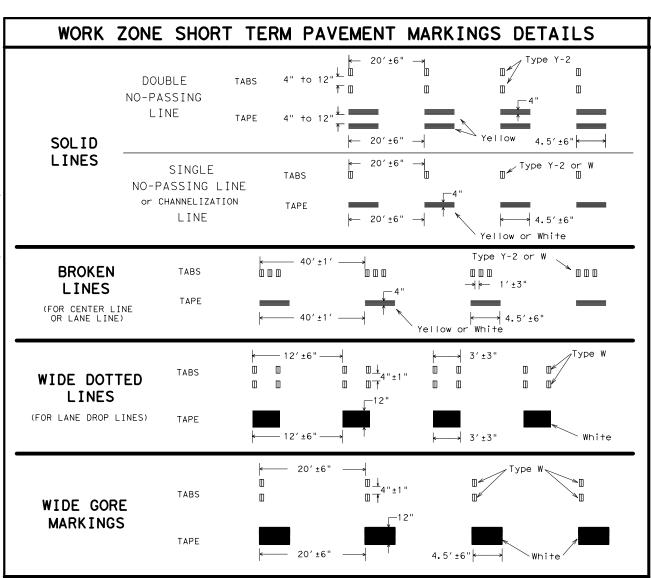
Traffic Operations Division Standard

# TRAFFIC CONTROL PLAN TYPICAL DETAILS

WZ(TD)-17

FILE:	wztd-17.dgn	DN: T:	×DOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
© TxD0T	February 1998	CONT	SECT	JOB		H	HIGHWAY
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3-03	2-11	DIST		COUNTY			SHEET NO.
7-13		ABL		STONEWA	٩LL		37
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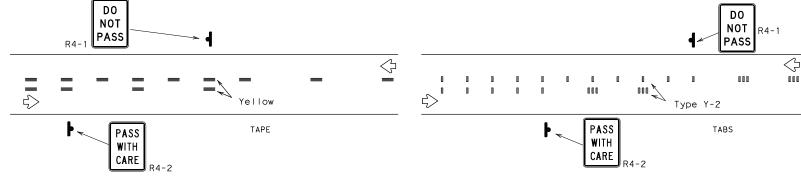
#### NOTES:

- 1. Short term pavement markings may be prefabricated markings (stick down tape) or temporary flexiblereflective roadway marker tabs unless otherwise specified elsewhere in plans.
- 2. Short term payement markings shall NOT be used to simulate edge lines.
- 3. Dimensions indicated on this sheet are typical and approximate. Variations in size and height may occur between markers or devices made by manufacturers, by as much as 1/4 inch, unless otherwise noted.
- 4. Temporary flexible-reflective roadway marker tabs will require normal maintenance replacement when used on roadways with an ADT per lane of up to 7500 vehicles with no more than 10% truck mix. When roadways exceed these values, additional maintenance replacement of devices should be planned.
- 5. No segment of roadway open to traffic shall remain without permanent pavement markings for a period greater than 14 calendar days. The Contractor will be responsible for maintaining short term pavement markings until permanent pavement markings are in place. When the Contractor is responsible for placement of permanent pavement markings, no segment of roadway shall remain without permanent pavement markings for a period greater than 14 calendar days unless weather conditions prohibit placement. Permanent pavement markings shall be placed as soon as weather permits.
- For two lane, two-way roadways, DO NOT PASS signs shall be erected to mark the beginning of sections where passing is prohibited and PASS WITH CARE signs shall be erected to mark the beginning of sections where passing is permitted. Signs shall be in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and may be used to indicate the limits of no-passing zones for up to 14 calendar days. Permanent pavement markings should then be placed.
- For low volume two lane, two-way roadways of 4000 ADT or less, no-passing lines may be omitted when approved by the Engineer. DO NOT PASS and PASS WITH CARE signs shall be erected (see note 6).
- For exit gores where a lane is being dropped place wide gore markings or retroreflective channelizing devices to guide motorist through the exit. If channelizing devices are to be used it should be noted elsewhere in the plans. One piece cones are not allowed for this purpose.

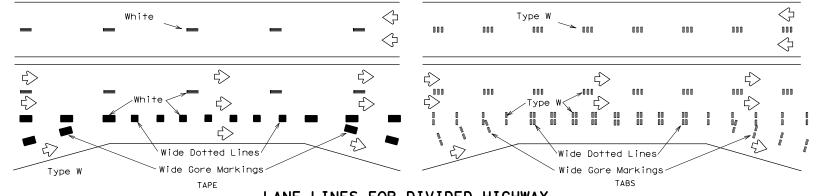
#### TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS (TABS)

- Temporary flexible-reflective roadway marker tabs detailed on this sheet will be designated Type Y-2 (two amber reflective surfaces with yellow body); Type Y (one amber reflective surface with yellow body); and Type W (one white or silver reflective surface with white body). Additional details may be found on BC(11).
- Tabs shall meet requirements of Departmental Material Specification DMS-8242.
- 3. When dry, tabs shall be visible for a minimum distance of 200 feet during normal daylight hours and when illuminated by automobile low-beam head light at night, unless sight distance is restricted by roadway
- No two consecutive tabs nor four tabs per 1000 feet of line shall be missing or fail to meet the visual performance requirements of Note 3.

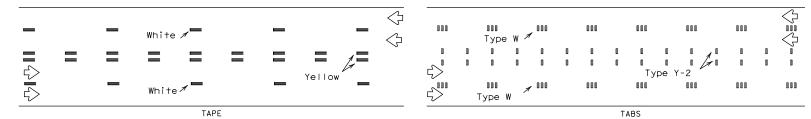
# WORK ZONE SHORT TERM PAVEMENT MARKINGS PATTERNS



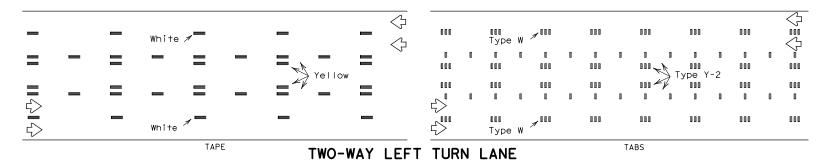
# CENTER LINE & NO-PASSING ZONE BARRIER LINES FOR TWO LANE TWO-WAY HIGHWAYS



### LANE LINES FOR DIVIDED HIGHWAY



# LANE & CENTER LINES FOR MULTILANE UNDIVIDED HIGHWAYS



Removable Raised Short Term Pavement Pavement Marker Marking (Tape)

If raised pavement markers are used to supplement REMOVABLE short term markings, the markers shall be applied to the top of the tape at the approximate mid length of the tape. This allows an easier removal of raised markers and tape.

# Texas Department of Transportation

Operation Division Standard

#### PREFABRICATED PAVEMENT MARKINGS

- 1. Temporary Removable Prefabricated Pavement Markings shall meet the requirements of DMS-8241.
- 2. Non-removable Prefabricated Pavement Markings shall meet the requirements of either DMS-8240 "Permanent Prefabricated Pavement Markings" or DMS-8243 "Temporary Costruction-Grade Prefabricated Pavement Markings."

#### RAISED PAVEMENT MARKERS

1. All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and DMS-4200.

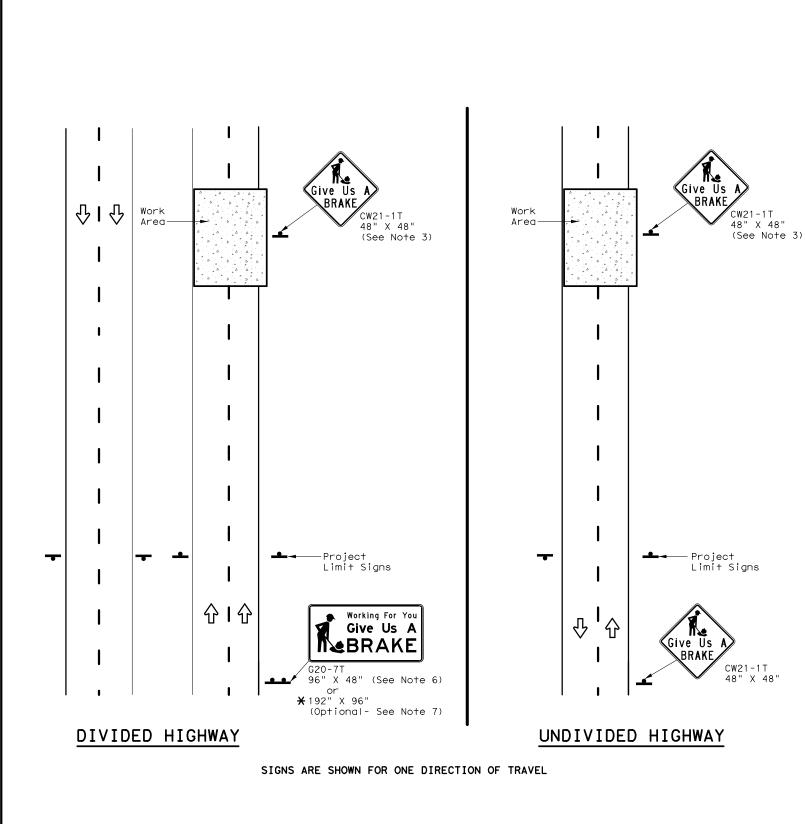
#### DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS) & MATERIAL PRODUCER LISTS (MPL)

1. DMSs referenced above can be found along with embedded links to their respective MPLs at the following website: http://www.txdot.gov/business/contractors\_consultants/material\_specifications/default.htm

# **WORK ZONE SHORT TERM** PAVEMENT MARKINGS

# WZ (STPM) -13

FILE:	wzstpm-13.dgn	DN: T:	xDOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
© TxDOT	April 1992	CONT	SECT	JOB		ніс	CHWAY
1-97	REVISIONS	0106	04	036		US	380
3-03		DIST		COUNTY			SHEET NO.
7-13		ABL		STONEWA	LL		38



\* When the optional larger WORKING FOR YOU GIVE US A BRAKE (G20-7T) 192" x 96" sign is required, the locations shall be noted elsewhere in the plans.

SUMMARY OF LARGE SIGNS											
BACKGROUND COLOR	STON		SIGN REFLECTIVE DIMENSIONS SHEETING		CICN		SQ FT	GALVA STRUC ST		-	DRILLED SHAFT
COLOR	DESIGNATION	DESIGNATION DIMENSIONS SHEETING			Size	(L	F)	24" DIA. (LF)			
Orange	G20-7T	Working For You Give Us A	96" X 48"	Type B <sub>FL</sub> or C <sub>FL</sub>	32	<b>A</b>	<b>A</b>	<b>A</b>	<b>A</b>		
Orange	G20-7T	Working For You Give Us A	192" X 96"	Type B <sub>FL</sub> or C <sub>FL</sub>	128	W8×18	16	17	12		

▲ See Note 6 Below

LEGEND				
•	Sign			
Large Sign				
₽	Traffic Flow			

DEPARTMENTAL MATERIAL SPEC	IFICATIONS
PLYWOOD SIGN BLANKS	DMS-7100
ALUMINUM SIGN BLANKS	DMS-7110
SIGN FACE MATERIALS	DMS-8300

COLOR	USAGE	SHEETING MATERIAL
ORANGE	BACKGROUND	TYPE B <sub>FL</sub> OR TYPE C <sub>FL</sub>
BLACK	LEGEND & BORDERS	NON-REFLECTIVE ACRYLIC FILM

#### GENERAL NOTES

- 1. See BC and SMD sheets for additional sign support details.
- 2. Sign locations shall be approved by the Engineer.
- 3. For projects more than two miles in length, Give Us a BRAKE signs should be repeated halfway through the project. The Give Us a Brake (CW21-1T) may be used for this purpose.
- 4. Work zone speed limits are sometimes used in conjunction with GIVE US A BRAKE signing. See BC(3) for location and spacing of construction speed zone signing when required.
- 5. Give Us a Brake (CW21-1T) signs and supports shall be considered subsidiary to Item 502, "Barricades, Signs and Traffic Handling."
- 6. The 96" X 48" Working For You Give Us A BRAKE (G20-7T) may use a 1/2" or 5/8" plywood substrate or 0.125" aluminum sheeting substrate and may be supported by two 4" x 6" wood posts with drilled holes for breakaway as per BC(5) and will be subsidiary to Item 502.
- 7. The Working For You Give Us A BRAKE (G20-7T) 192" X 96" sign shall be paid for under the following specification items:

Item 636 - Aluminum Signs

Item 647 - Large Roadside Sign Supports and Assemblies.

Item 416 - Drilled Shaft Foundations

8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.

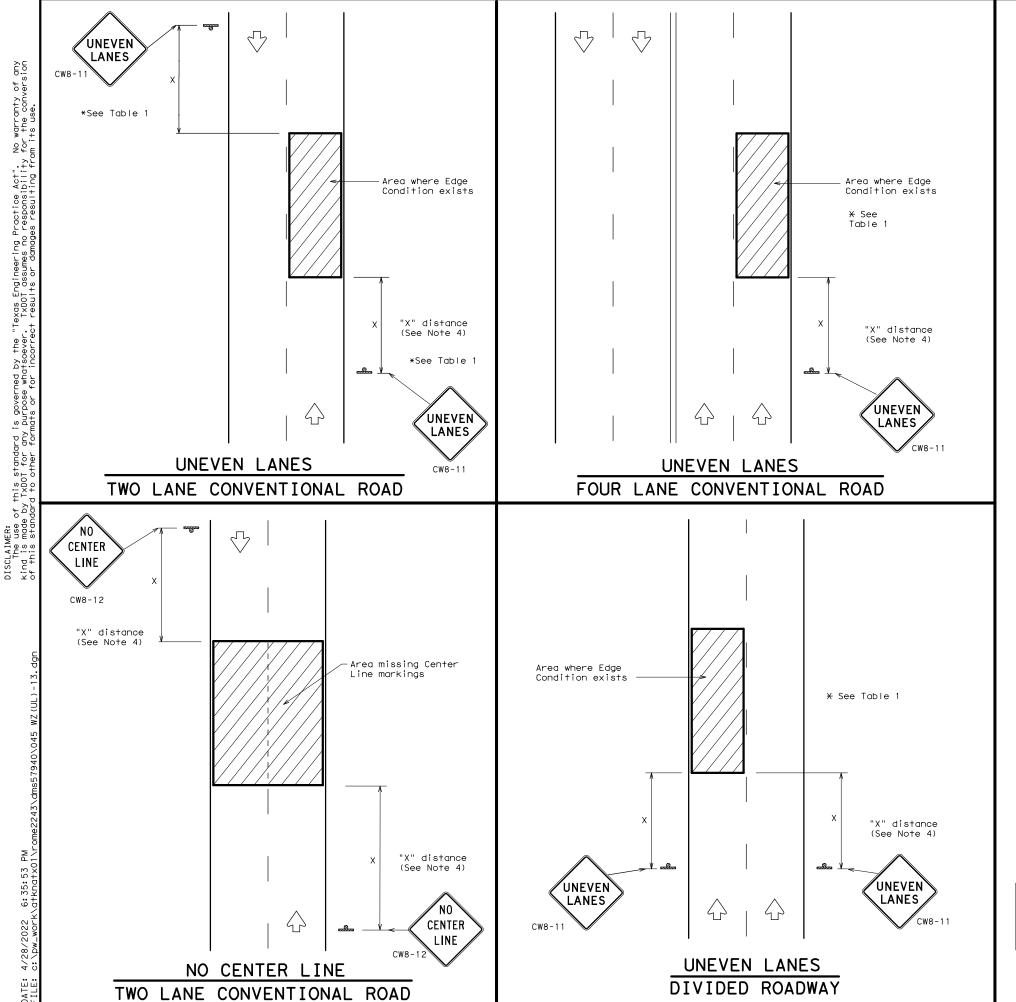


Traffic Operations Division Standard

WORK ZONE
"GIVE US A BRAKE"
SIGNS

WZ (BRK) -13

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)TxDOT August 1995	CONT	SECT	JOB		ні	GHWAY
REVISIONS	0106	04	036		US	380
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-96 3-03	ABL		STONEWA	٩LL		39



DEPARTMENTAL MATERIAL SPECIFICAT	IONS
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
TEMPORARY (REMOVABLE) PREFABRICATED PAVEMENT MARKINGS	DMS-8241
SIGN FACE MATERIALS	DMS-8300

COLOR	USAGE	SHEETING MATERIAL
ORANGE	BACKGROUND	TYPE B <sub>FL</sub> OR TYPE C <sub>FL</sub> SHEETING
BLACK	LEGEND & BORDERS	ACRYLIC NON-REFLECTIVE SHEETING

#### **GENERAL NOTES**

- 1. If spalling or holes occur, ROUGH ROAD (CW8-8) signs should be placed in advance of the condition and be repeated every two miles where the condition persists.
- UNEVEN LANES (CW8-11) signs shall be installed in advance of the condition and repeated every mile. Signs installed along the uneven lane condition may be supplemented with the NEXT XX MILES (CW7-3aP) plaque or Advisory Speed (CW13-1P) plaque.
- 3. NO CENTER LINE (CW8-12) signs and temporary pavement markings as per the WZ(STPM) standard shall be installed if yellow centerlines separating two way traffic are obscured or obliterated. Repeat NO CENTER LINE signs every two miles where the center line markings are not in place. The signs and markings shall remain in place until permanent pavement markings are
- 4. Signs shall be spaced at the distances recommended as per BC standards.
- Additional signs may be required as directed by the Engineer. Signs shall remain in place until final surface is applied. Signs shall be considered subsidiary to Item 502 "BARRICADES, SIGNS AND TRAFFIC HANDLING."
- 6. Signs shall be fabricated and mounted on supports as shown on the BC  $\,$ standards and/or listed on the "Compliant Work Zone Traffic Control Devices"
- 7. Short term markings shall not be used to simulate edge lines.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition.

,	TABLE 1								
Edge Condition	Edge Height (D)	* Warning Devices							
①	Less than or equal to: $1\frac{1}{4}$ " (maximum-planing) $1\frac{1}{2}$ " (typical-overlay)	Sign: CW8-11							
7/// 🛧 D	Distance "D" may be a maximum of 1 1/4 " for planing operations and 2" for overlay operations if uneven lanes with edge condition 1 are open to traffic after work operations cease.								
② >3	Less than or equal to 3"	Sign: CW8-11							
3 0" to 3/4" 7 D Notched Wedge Joint	Distance "D" may be a maximum of 3" if uneven lanes with edge condition 2 or 3 are open to traffic after work operations cease. Uneven lanes should not be open to traffic when "D" is greater than 3".								

TRAFFIC CONTROL DURING PLANING, OVERLAY AND LEVELING OPERATIONS ARE SHOWN ELSEWHERE IN THE PLANS.

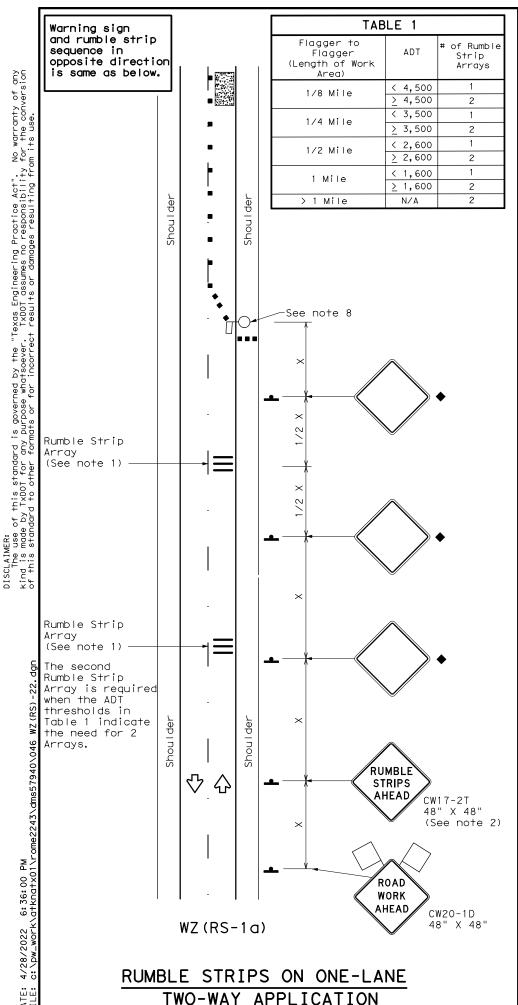
MINIMUM	WARNING	SIGN	SIZE
Convention	nal roads	36" :	x 36"
Freeways/ex divided	48" >	< 48"	

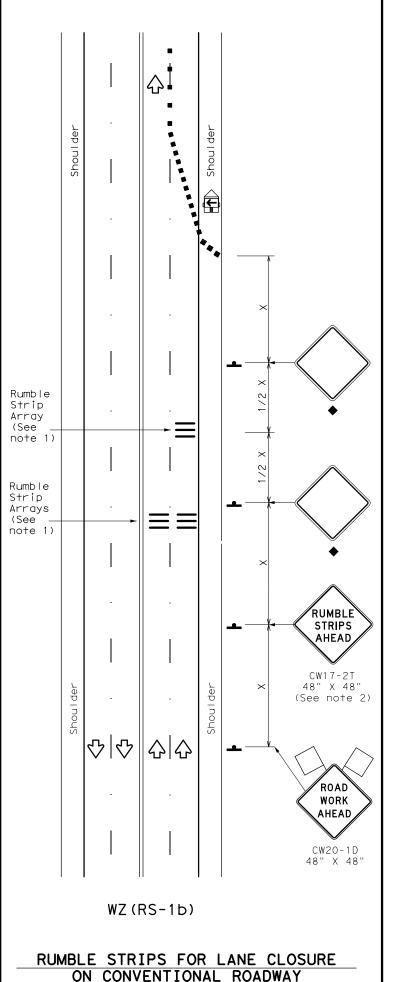


# SIGNING FOR UNEVEN LANES

WZ (UL) -13

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) T×DOT	April 1992	CONT	SECT	JOB		н	CHWAY
	REVISIONS	0106	04	036		US	380
95 2-98	7-13	DIST COUNTY SHEE		SHEET NO.			
97 3-03		ABL STONEWALL 40		40			
2							





#### **GENERAL NOTES**

- 1. Each Rumble Strip Array should consist of three rumble strips spaced center to center at the spacing shown in Table 2, placed transverse across the lane at locations shown.
- 2. The CW17-2T "RUMBLE STRIPS AHEAD" sign should be located after the CW20-1D "ROAD WORK AHEAD sign and spaced as shown. If traffic is observed to be queuing, or is expected to queue beyond the Rumble Strips, the CW17-2T sign and the first Rumble Strip Array may be located upstream of the CW20-1D sign as necessary to provide needed warning.
- 3. Temporary Rumble Strips will be considered subsidiary to Item 502, and shall be a product listed on the Compliant Work Zone Traffic Control
- 4. Remove Temporary Rumble Strips before removing the advanced warning signs.
- 5. Temporary Rumble Strips should not be used on horizontal curves, loose gravel, soft or bleeding asphalt, heavily rutted pavements or unpaved
- 6. Temporary Rumble Strips shall be installed and maintained as per manufacturer's recommendations.
- 7. This standard sheet shall be used in conjunction with other appropriate TCP standard, TMUTCD typical application or project specific detail for the project.
- The one-lane two-way application may utilize a flagger, an Automated Flagger Assistance Device (AFAD) or a Portable Traffic Signal (PTS).
- 9. Replace defective Temporary Rumble Strips as directed by the Engineer.
- 10. Temporary Rumble Strips may be used on freeways or expressways based on engineering judgment and written direction from the Engineer.

LEGEND								
	Type 3 Barricade		Channelizing Devices					
	Heavy Work Vehicle		Truck Mounted Attenuator (TMA)					
	Trailer Mounted Flashing Arrow Panel	M	Portable Changeable Message Sign (PCMS)					
4	Sign	♡	Traffic Flow					
$\Diamond$	Flag		Flagger					

Posted Speed	Formula	D	Minimur esirab er Len <del>XX</del>	le	Suggested Maximum Spacing of Channelizing Devices		Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space	
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"	
30	, WS <sup>2</sup>	150′	165′	180′	30′	60′	120′	90′	
35	L= WS	205′	225′	245′	35′	70′	160′	120′	
40	80	265′	295′	320′	40′	80′	240′	155′	
45		450′	495′	540′	45′	90′	320′	195′	
50		500′	550′	600′	50′	100′	400′	240′	
55	L=WS	550′	605′	660′	55′	110′	500′	295′	
60	L #3	600′	660′	720′	60′	120′	600′	350′	
65		650′	715′	780′	65′	130′	700′	410′	
70		700′	770′	840′	70′	140′	800′	475′	
75		750′	825′	900′	75′	150′	900′	540′	

- \* Conventional Roads Only
- \*\* Taper lengths have been rounded off. L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

TYPICAL USAGE							
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE LONG TEI TERM STATIONARY STATIONA				
	✓	✓					

- Signs are for illustrative purposes only. Signs required may vary depending on the TCP, TMUTCD Typical Application, or project specific details for the project.
- For posted speeds in excess of 65 MPH, it is recommended that spacing is increased as speed limits increase. Increasing space between rumble strips will improve effectiveness.

TABLE 2							
Speed	Approximate distance between strips in an array						
<u>≤</u> 40 MPH	10′						
> 40 MPH & <u>&lt;</u> 55 MPH	15′						
= 60 MPH	20′						
≥ 65 MPH	<del>*</del> 35′+						

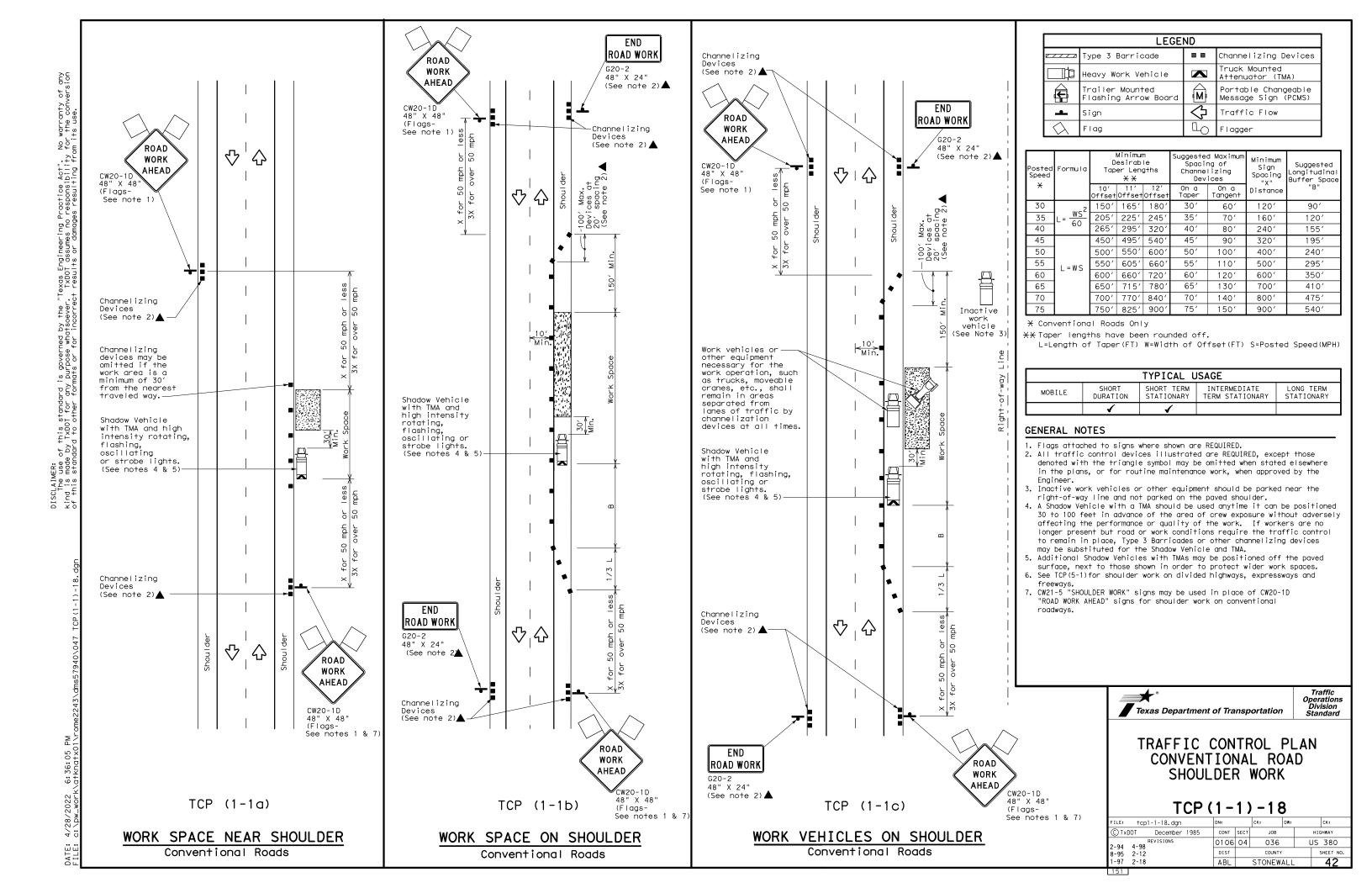
Texas Department of Transportation

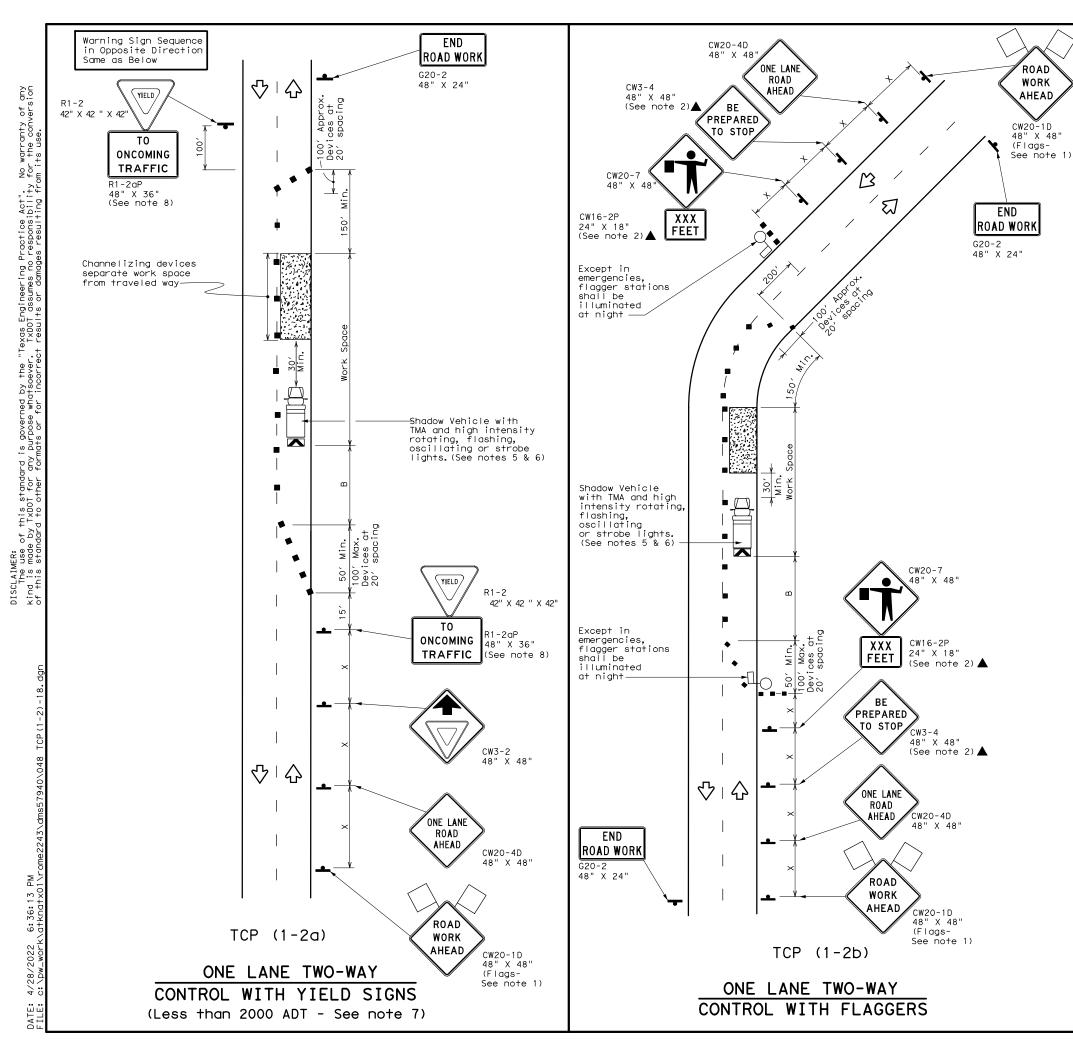
TEMPORARY RUMBLE STRIPS

Traffic Safety Division Standard

WZ(RS) - 22

ILE: wzrs22.dgn	DN: Tx	DOT	ck: TxDOT	DW:	TxDOT	ck: TxDOT
C)TxDOT November 2012	CONT	SECT	JOB		н	IGHWAY
REVISIONS	0106	04	036		US 380	
2-14 1-22 4-16	DIST		COUNTY			SHEET NO.
4-16	ABL	STONEWALL				41





	LEGEND							
	Type 3 Barricade		Channelizing Devices					
	Heavy Work Vehicle		Truck Mounted Attenuator (TMA)					
<b>F</b>	Trailer Mounted Flashing Arrow Board	M	Portable Changeable Message Sign (PCMS)					
•	Sign	♡	Traffic Flow					
$\triangle$	Flag		Flagger					

Posted Speed	Formula	Minimum Desirable Taper Lengths XX		Suggested Maximum Spacing of Channelizing Devices		Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space	Stopping Sight Distance	
*		10' Offset	11' Offset	12' Offset	0n a Taper	On a Tangent	Distance	"B"	
30	WS <sup>2</sup>	150′	165′	180′	30′	60′	120′	90′	200′
35	$L = \frac{WS}{60}$	205′	225′	245′	35′	70′	160′	120′	250′
40	80	265′	295′	320′	40′	80′	240′	155′	305′
45		450′	495′	540′	45′	90′	320′	195′	360′
50		500′	550′	600′	50′	100′	400′	240′	425′
55	L=WS	550′	605′	660′	55′	110′	500′	295′	495′
60	L-#3	600′	660′	720′	60′	120′	600′	350′	570′
65		650′	715′	780′	65′	130′	700′	410′	645′
70		700′	770′	840′	70′	140′	800′	475′	730′
75		750′	825′	900′	75′	150′	900′	540′	820′

X Conventional Roads Only

\*X Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

TYPICAL USAGE							
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY			
	1	1					

#### **GENERAL NOTES**

- 1. Flags attached to signs where shown are REQUIRED.
- 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved by the Engineer.
- 3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4D "ONE LANE ROAD AHEAD" sign, but proper sign spacing shall be maintained.
- 4. Sign spacing may be increased or an additional CW20-1D "ROAD WORK AHEAD" sign may be used if advance warning ahead of the flagger or R1-2 "YIELD" sign is less than 1500 feet.
- 5. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.
- 6. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect wider work spaces.

#### TCP (1-2a)

- 7. R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work spaces should be no longer than one half city block. In rural areas on roadways with less than 2000 ADT, work spaces should be no longer than 400 feet.
- 8. R1-2 "YIELD" sign with R1-2aP "TO ONCOMING TRAFFIC" plaque shall be placed on a support at a 7 foot minimum mounting height.

- 9. Flaggers should use two-way radios or other methods of communication to control traffic. 10. Length of work space should be based on the ability of flaggers to communicate.
- 11. If the work space is located near a horizontal or vertical curve, the buffer distances should be increased in order to maintain adequate stopping sight distance to the flagger and a queue of stopped vehicles (see table above).
- 12. Channelizing devices on the center-line may be omitted when a pilot car is leading traffic and approved by the Engineer.
- 3. Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to emergency situations.

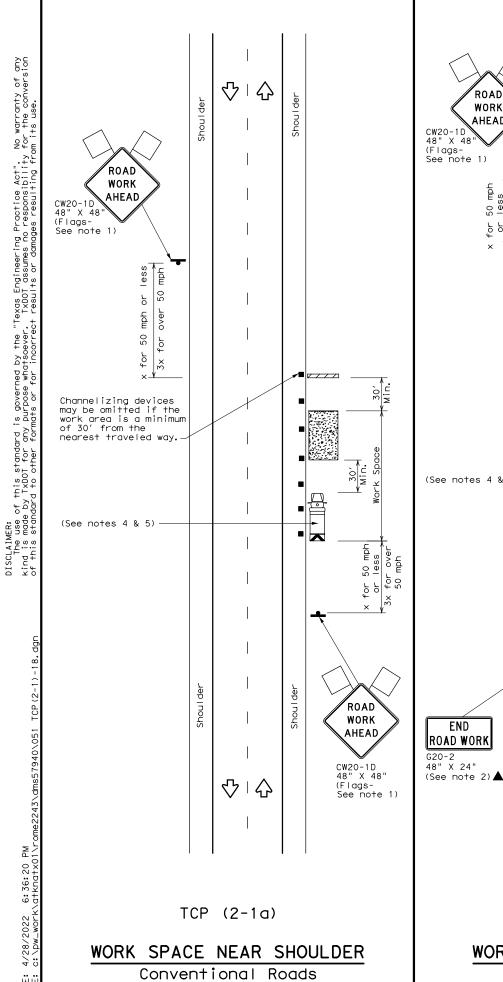


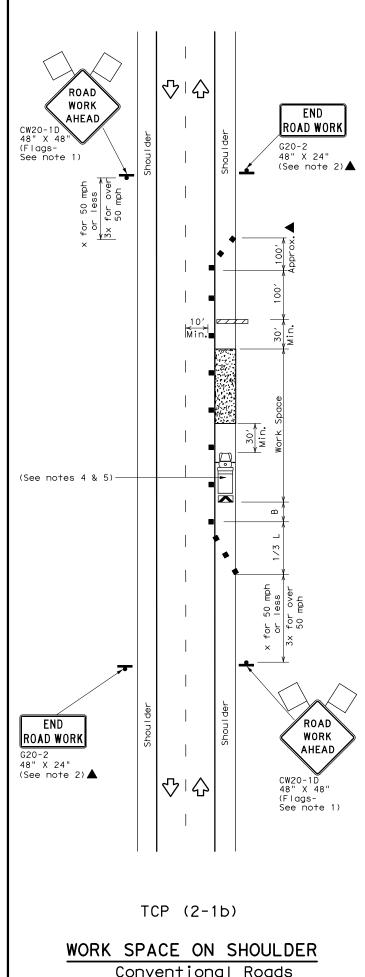
Traffic Operations Division Standard

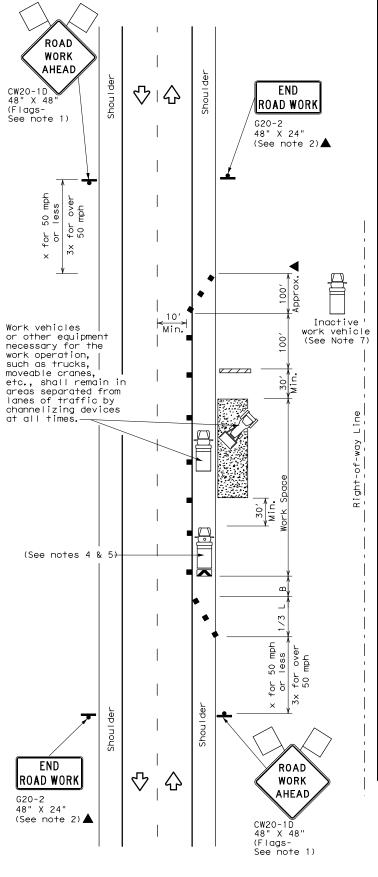
TRAFFIC CONTROL PLAN ONE-LANE TWO-WAY TRAFFIC CONTROL

TCP (1-2)-18

FILE: tcp1-2-18.dgn	DN:	CK: DW:		DW:	ck:
©⊺xDOT December 1985	CONT	SECT	JOB		HIGHWAY
4-90 4-98 REVISIONS	0106	04	036 l		JS 380
2-94 2-12	DIST		COUNTY		SHEET NO.
1-97 2-18	ABL	STONEWALL			43







TCP (2-1c)

WORK VEHICLES ON SHOULDER Conventional Roads

	LEGEND							
	Type 3 Barricade	ype 3 Barricade ■■						
	Heavy Work Vehicle		Truck Mounted Attenuator (TMA)					
	Trailer Mounted Flashing Arrow Board	M	Portable Changeable Message Sign (PCMS)					
-	Sign	♡	Traffic Flow					
$\Diamond$	Flag	LO	Flagger					

Speed	Formula	D		sirable Spacing of Channelizing Spacing X X Sign		Channelizing Devices		Suggested Longitudinal Buffer Space
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"
30	, WS <sup>2</sup>	150′	165′	180′	30′	60′	120′	90′
35	L = WS	205′	225′	245′	35′	70′	160′	120′
40	80	265′	295′	320′	40′	80′	240′	155′
45		450′	495′	540′	45′	90′	320′	195′
50		500′	550′	600′	50′	100′	400′	240′
55	L=WS	550′	605′	660′	55′	110′	500′	295′
60	L-W3	600′	660′	720′	60′	120′	600′	350′
65		650′	715′	780′	65′	130′	700′	410′
70		700′	770′	840′	70′	140′	800′	475′
75		750′	825′	900′	75′	150′	900′	540′

imes Conventional Roads Only

\*X Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

	TYPICAL USAGE						
MOBILE	MOBILE SHORT SHORT TERM INTERMEDIATE LONG TERM DURATION STATIONARY TERM STATIONARY STATIONARY						
	1	1	1	1			

#### **GENERAL NOTES**

- 1. Flags attached to signs where shown, are REQUIRED.
- 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated in the plans, or for routine maintenance work, when approved by the Engineer.
- 3. Stockpiled material should be placed a minimum of 30 feet from
- nearest traveled way.

  4. Shadow Vehicle with TMA and high intensity rotating, flashing, oscillating or strobe lights. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.
- 5. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect a wider work space.
- 6. See TCP(5-1) for shoulder work on divided highways, expressways and
- 7. Inactive work vehicles or other equipment should be parked near the right-of-way line and not parked on the paved shoulder.
- 8. CW21-5 "SHOULDER WORK" signs may be used in place of CW20-1D "ROAD WORK AHEAD" signs for shoulder work on conventional roadways.

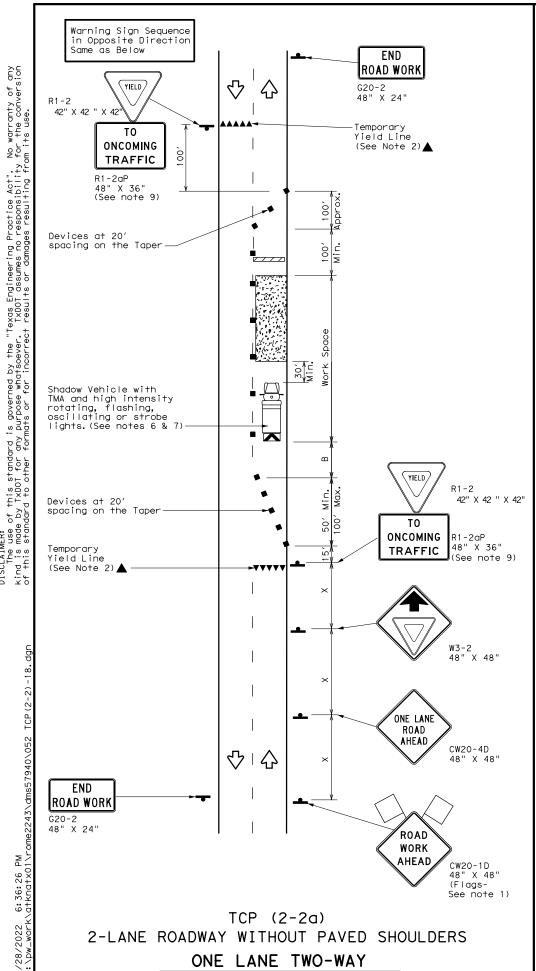
Texas Department of Transportation

Traffic Operations Division Standard

TRAFFIC CONTROL PLAN CONVENTIONAL ROAD SHOULDER WORK

TCP (2-1)-18

: tcp2-1-18.dgn	DN:		CK:	DW:	CK:
TxDOT December 1985	CONT	SECT	JOB		HIGHWAY
REVISIONS 34 4-98	0106	04	036	Į	JS 380
94 4-96 95 2-12	DIST		COUNTY		SHEET NO.
7 2-18	ABL		STONEW	ALL	44



CONTROL WITH YIELD SIGNS

(Less than 2000 ADT - See Note 9)

CW20-4 ONE LANE ROAD ROAD WORK XXX FT 48" X 48" AHEAD BE PREPARED CW20-1D 48" X 48" TO STOP (Flags-See note 1) CW20-7  $\angle 3$ XXX FEET  $\overline{\mathcal{U}}$ END CW16-2P ROAD WORK 24" X 18" G20-2 48" X 24" Except in emergencies, flagger stations shall be illuminated Temporary 24" Stop Line (See Note 2)▲ 100' Approx. Devices at 20' spacing Shadow Vehicle with TMA and high intensity rotating, flashing, oscillating or strobe lights. (See notes 6 & 7 48" X 48" Devices at 20' spacing XXX FEET on the Taper Except in emergencies, flagger stations BE illuminated PREPARED at night TO STOP CW3-4 48" X 48" Temporary (See note 2)▲ 24" Stop Line (See Note 2) ONE LANE 公 ROAD XXX FT CW20-4 48" X 48" END ROAD ROAD WORK WORK AHEAD CW20-1D 48" X 48" (Flags-See note 1) TCP (2-2b) 2-LANE ROADWAY WITHOUT PAVED SHOULDERS

ONE LANE TWO-WAY

CONTROL WITH FLAGGERS

**LEGEND** Type 3 Barricade Channelizing Devices ruck Mounted Heavy Work Vehicle Attenuator (TMA) railer Mounted Portable Changeable Flashing Arrow Board Message Sign (PCMS) • Traffic Flow Sign Flag Flagger

Posted Speed	Formula	D	Minimur esirab er Leng <del>XX</del>	le	Spacii Channe		Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space	Stopping Sight Distance
*		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"	
30	WS <sup>2</sup>	150′	165′	180′	30′	60′	120′	90′	200′
35	L= WS	205′	225′	245′	35′	70′	160′	120′	250′
40	80	265′	295′	320′	40′	80′	240′	155′	305′
45		450′	495′	540′	45′	90′	320′	195′	360′
50		500′	550′	600′	50′	100′	400′	240′	425′
55	L=WS	550′	605′	660′	55′	110′	500′	295′	495′
60	L-W5	600′	660′	720′	60′	120′	600′	350′	570′
65		650′	715′	780′	65′	130′	700′	410′	645′
70		700′	770′	840′	70′	140′	800′	475′	730′
75		750′	825′	900′	75′	150′	900′	540′	820′

\* Conventional Roads Only

XX Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

	TYPICAL USAGE						
MOBILE	MOBILE SHORT SHORT TERM INTERMEDIATE LONG TERM DURATION STATIONARY TERM STATIONARY STATIONARY						
		_/					

#### GENERAL NOTES

- 1. Flags attached to signs where shown, are REQUIRED.
- 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved
- 3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4 "ONE LANE ROAD XXX FT" sign, but proper sign spacing shall be maintained.
- 4. Flaggers should use two-way radios or other methods of communication to control traffic.
- 5. Length of work space should be based on the ability of flaggers to communicate.
- 6. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow Vehicle and TMA.
- 7. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown in order to protect a wider work space.

#### TCP (2-2a)

8. The R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work space should be no longer than one half city block. In rural areas, roadways with less than 2000 ADT, work space should be no longer than 400 feet.

9. The R1-2aP "YIELD TO ONCOMING TRAFFIC" sign shall be placed on a support at a 7 foot minimum mounting height.

#### TCP (2-2b)

- 10.Channelizing devices on the center line may be omitted when a pilot car is leading traffic and approved by the Engineer.
- 11.If the work space is located near a horizontal or vertical curve, the buffer distances should be increased in order to maintain stopping sight distance to the flagger and a queue of stopped vehicles.
- 12.Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to emergency situtations.

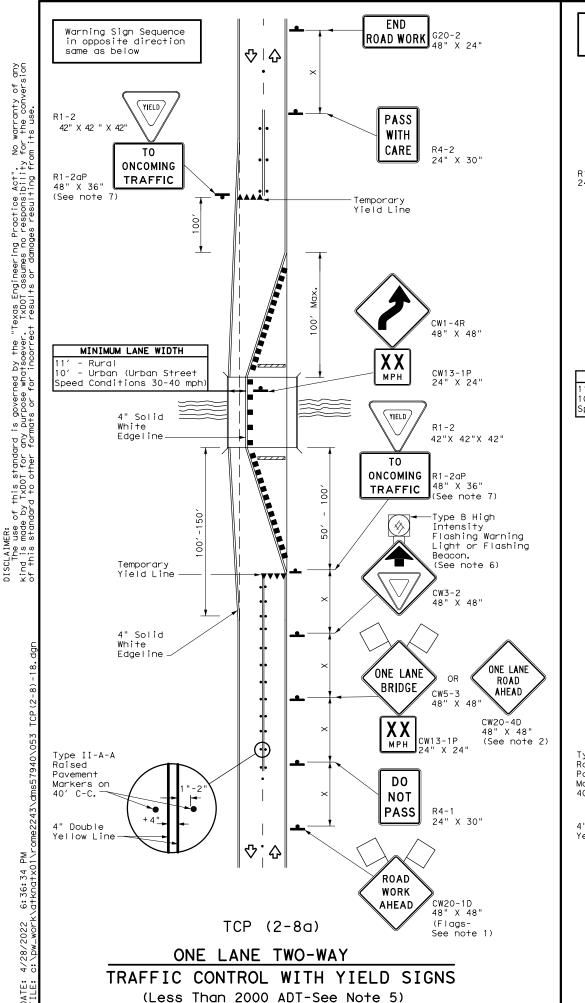


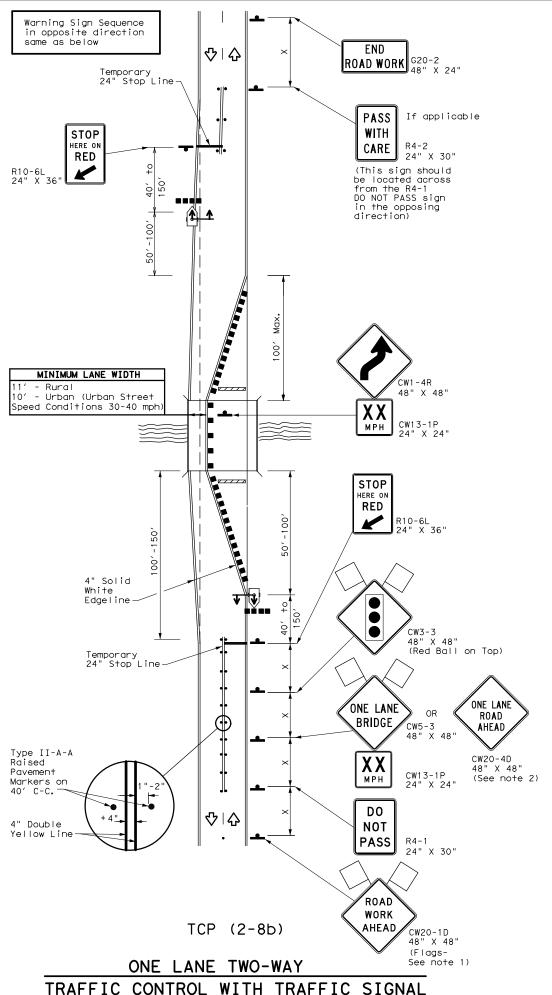
Traffic Operations Division Standard

TRAFFIC CONTROL PLAN ONE-LANE TWO-WAY TRAFFIC CONTROL

TCP (2-2) -18

FILE: tcp2-2-18.dgn	DN:		CK:	DW:	CK:
© TxDOT December 1985	CONT	SECT	JOB		HIGHWAY
REVISIONS 8-95 3-03	0106	04	036		JS 380
1-97 2-12	DIST		COUNTY		SHEET NO.
4-98 2-18	ABL		STONEW	ALL	45





	LEGEND						
	Type 3 Barricade		Channelizing Devices				
-	Sign	♡	Traffic Flow				
$\Diamond$	Flag		Flagger				
••••	Raised Pavement Markers Ty II-AA	<b>*</b>	Temporary or Portable Traffic Signal				

Posted Speed	Formula	D	Minimum esirab er Leng <del>X</del> <del>X</del>	le	Spacir Channe		Minimum Sign Spacing "X"	Suggested Longitudinal Buffer Space	Stopping Sight Distance
<del>*</del>		10' Offset	11' Offset	12' Offset	On a Taper	On a Tangent	Distance	"B"	
30	WS <sup>2</sup>	150′	165′	180′	30′	60′	120′	90′	200′
35	L = WS	205′	225′	245′	35′	70′	160′	120′	250′
40	60	265′	295′	320′	40′	80′	240′	155′	305′
45		450′	495′	540′	45′	90′	320′	195′	360′
50		500′	550′	600′	50′	100′	400′	240′	425′
55	L=WS	550′	605′	660′	55′	110′	500′	295′	495′
60	L #3	600′	660′	720′	60′	120′	600′	350′	570′
65		650′	715′	780′	65′	130′	700′	410′	645′
70		700′	770′	840′	70′	140′	800′	475′	730′
75		750′	825′	900′	75′	150′	900′	540′	820′

\* Conventional Roads Only

\*\* Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

	TYPICAL USAGE							
MOBILE	MOBILE SHORT SHORT TERM INTERMEDIATE LONG TERM DURATION STATIONARY TERM STATIONARY STATIONARY							
	1 1							

#### GENERAL NOTES

- 1. Flags attached to signs where shown are REQUIRED.
- 2. When this TCP is used at a location which does not involve a bridge, a 48" x 48" CW20-4D "ONE LANE ROAD AHEAD" signs should be used in lieu of the CW5-3 "ONE LANE BRIDGE" signs. The CW13-1P Advisory Speed Plaque is required with either warning sign.
- Raised pavement markers shall be placed 40 feet c-c on centerline between DO NOT PASS signs and stop or yield lines.
- 4. For intermediate term situations, when it is not feasible to remove and restore pavement markings, the channelization must be made dominant by using a very close spacing. This is especially important in locations of conflicting information, such as where traffic is directed over a double yellow centerline. In such locations a maximum channelizing device spacing of 20 feet is recommended. The 20 foot channelizing device spacing recommendation is intended for the area of conflicting information and not the entire work zone.

#### TCP (2-8a

- 5. Traffic control by CW3-2 "YIELD AHEAD" symbol signs for one lane two-way traffic control operations should be limited to work spaces less than 400 feet long and roadways with less than 2000 ADT. Otherwise, portable traffic signals should be used.
- If power is available, a flashing beacon should be attached to the CW3-2 "YIELD AHEAD" symbol sign for emphasis.
- 7. The R1-2 "YIELD" and R1-2aP "TO ONCOMING TRAFFIC" signs and other regulatory signs shall be installed at 7 foot minimum mounting height.

#### TCP (2-8b

- 8. A list of approved Portable Traffic Signals can be found in the "Compliant Work Zone Traffic Control Devices" list.
- Portable traffic signals should be located to provide adequate stopping sight distance for approaching motorist (See table above).



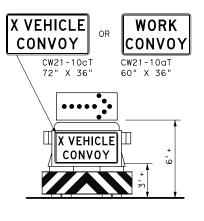
Traffic Operations Division Standard

TRAFFIC CONTROL PLAN LONG TERM ONE-LANE TWO-WAY CONTROL

TCP(2-8)-18

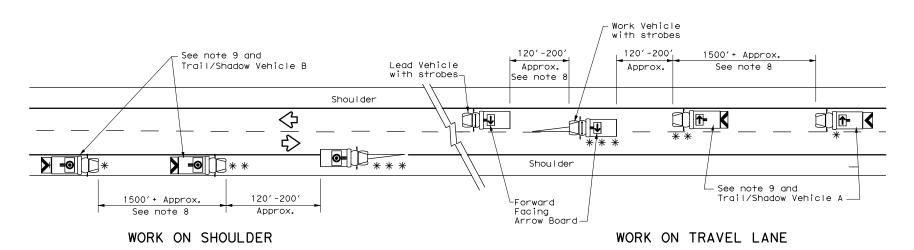
FILE: †cp2-8-18.dgn	DN:		CK:	DW:		CK:
© TxDOT December 1985	CONT	SECT	JOB		ніс	CHWAY
REVISIONS 8-95 3-03	0106	04	036		US	380
1-97 2-12	DIST		COUNTY			SHEET NO.
4-98 2-18	ABL		STONEW	ALL		46

168



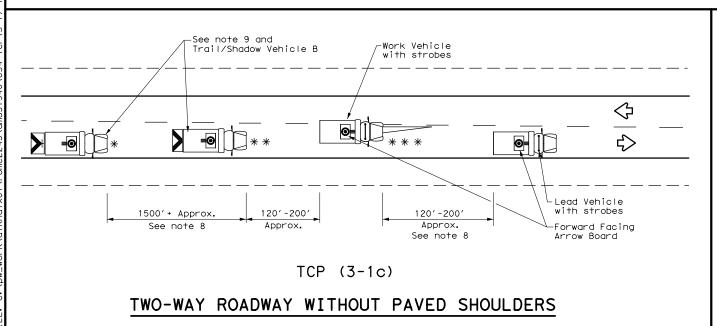
# TRAIL/SHADOW VEHICLE A

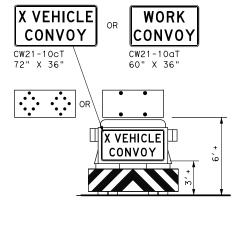
with RIGHT Directional display Flashing Arrow Board



TCP (3-1b)

# TWO-WAY ROADWAY WITH PAVED SHOULDERS





TRAIL/SHADOW VEHICLE B

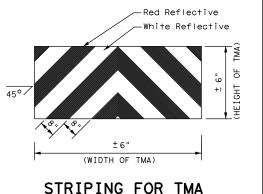
with Flashing Arrow Board in CAUTION display

	LEGEND					
*	Trail Vehicle		ARROW BOARD DISPLAY			
* *	Shadow Vehicle		ARROW BOARD DISPLAT			
* * *	Work Vehicle	RIGHT Directional				
	Heavy Work Vehicle	<b>—</b>	LEFT Directional			
	Truck Mounted Attenuator (TMA)	₩	Double Arrow			
♡	Traffic Flow	<b>©</b> =	CAUTION (Alternating Diamond or 4 Corner Flash)			

TYPICAL USAGE						
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY		
1						

#### GENERAL NOTES

- TRAIL, SHADOW, and LEAD vehicles shall be equipped with arrow boards as illustrated. When a LEAD vehicle is not used the WORK vehicle must be equipped with an arrow board. The Engineer will determine if the LEAD VEHICLE and/or TRAIL VEHICLE are required based on prevailing roadway conditions, traffic volume, and sight distance restrictions.
- 2. The use of amber high intensity rotating, flashing, oscillating, or strobe lights on vehicles are required. Blue high intensity rotating, flashing, oscillating or strobe lights when mounted on the driver's side of the vehicle may be operated simultaneously with the amber beacons or strobe lights.
- 3. The use of truck mounted attenuators (TMA) on the SHADOW VEHICLE and TRAIL VEHICLE are required.
- Reflective sheeting on the rear of the TMA shall meet or exceed the reflectivity and color requirements of DEPARTMENTAL MATERIAL SPECIFICATION DMS 8300, Type A.
- Flashing arrow boards shall be Type B or Type C as per the Barricade and Construction (BC) standards. The board shall be controlled from inside the vehicle.
- Each vehicle shall have two-way radio communication capability.
- When work convoys must change lanes, the TRAIL VEHICLE should change lanes first to shadow the other convoy vehicles.
- Vehicle spacing between the TRAIL VEHICLE and the SHADOW VEHICLE will vary depending on sight distance restrictions. Motorists approaching the work convoy should be able to see the TRAIL VEHICLE in time to slow down and/or change lanes as they approach the TRAIL VEHICLE. Vehicle spacing between the WORK VEHICLE and SHADOW VEHICLE and vehicle spacing between WORK VEHICLE and LEAD VEHICLE may vary according to terrain, work activity and other factors.
- "X VEHICLE CONVOY" (CW21-10cT) or "WORK CONVOY" (CW21-10aT) signs shall be used on TRAIL VEHICLES and SHADOW VEHICLES as shown. As an option 48" X 48" diamond shaped "WORK CONVOY"(CW21-10T) or "X VEHICLE CONVOY" (CW21-10bT) signs may be used where adequate mounting space exists. When used, the X VEHICLE CONVOY sign shall have the number of the convoy vehicles displayed on the sign in the number designation "X" location. The "X VEHICLE CONVOY" sign shall not be used on the SHADOW VEHICLE if a TRAIL VEHICLE is used.
- 10. On two-lane two-way roadways, the work and protection vehicles should pull over periodically to allow motor vehicle traffic to pass. If motorists are not allowed to pass the work convoy, a "DO NOT PASS" (R4-1) sign should be placed on the back of the rearmost protection vehicle.



Texas Department of Transportation

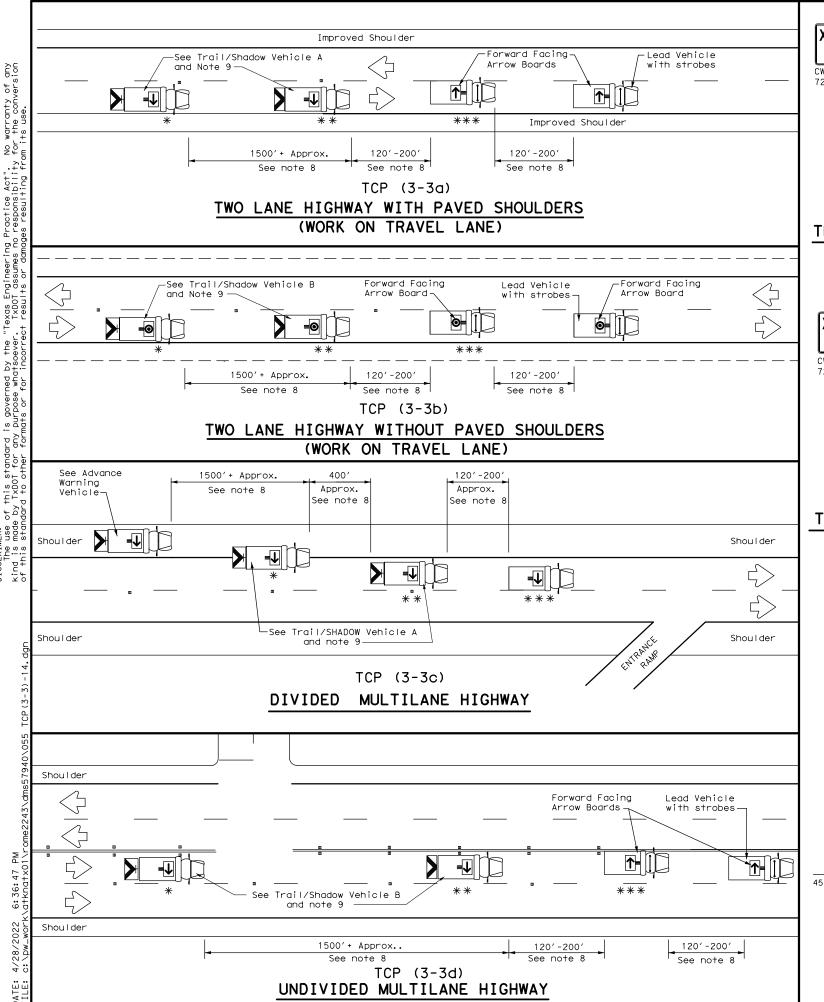
TRAFFIC CONTROL PLAN MOBILE OPERATIONS

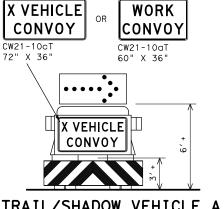
Division Standard

TCP(3-1)-13

DN: TxDOT CK: TxDOT DW: TxDOT CK: TxDO tcp3-1.dgn C) TxDOT December 1985 0106 04 036 US 380 2-94 4-98 STONEWALL

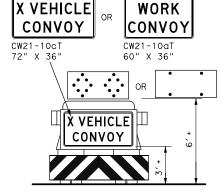
UNDIVIDED HIGHWAYS





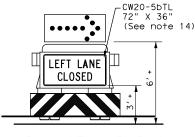
TRAIL/SHADOW VEHICLE A

with RIGHT Directional display Flashing Arrow Board

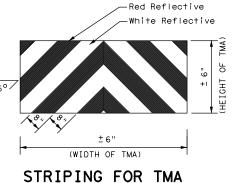


## TRAIL/SHADOW VEHICLE B

with Flashing Arrow Board in Caution Mode



ADVANCE WARNING VEHICLE



LEGEND							
*	Trail Vehicle		ARROW BOARD DISPLAY				
* *	Shadow Vehicle	ARROW BOARD DISPLAT					
* * *	Work Vehicle	<b>→</b>	RIGHT Directional				
	Heavy Work Vehicle	<b>—</b>	LEFT Directional				
	Truck Mounted Attenuator (TMA)	$\Theta$	Double Arrow				
₹	Traffic Flow	0	CAUTION (Alternating Diamond or 4 Corner Flash)				

TYPICAL USAGE								
MOBILE	SHORT DURATION	SHORT TERM STATIONARY	INTERMEDIATE TERM STATIONARY	LONG TERM STATIONARY				
1								

#### GENERAL NOTES

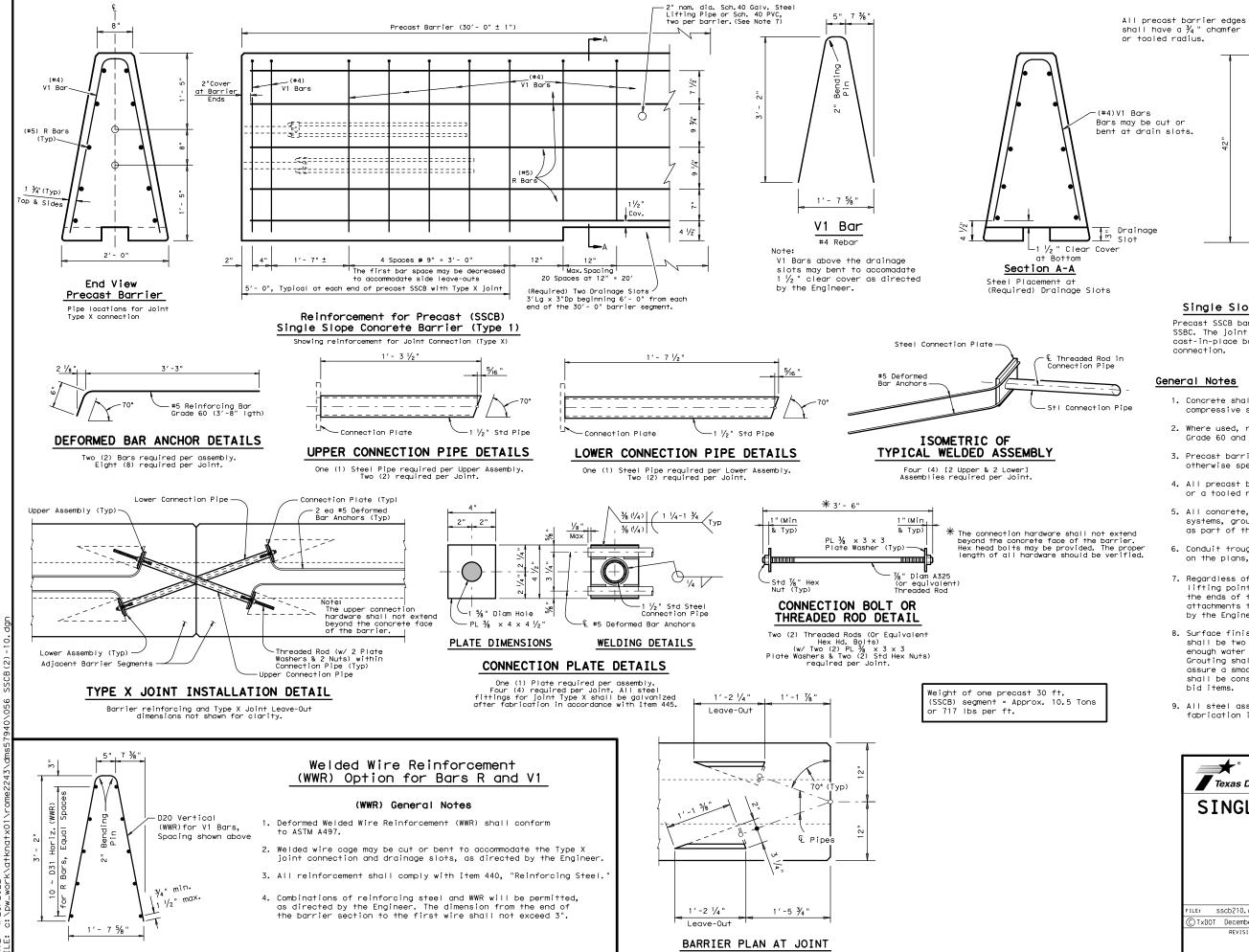
- 1. TRAIL, SHADOW, and LEAD vehicles shall be equipped with arrow boards as illustrated. When a LEAD vehicle is not used on two way roads the WORK vehicle must have an arrow board. For divided roadways, the arrow board on the WORK vehicle is optional based on the type of work being performed. The Engineer will determine if the LEAD vehicle and/or TRAIL vehicle are required based on
- prevailing roadway conditions, traffic volume, and sight distance restrictions. The use of amber high intensity rotating, flashing, oscillating, or strobe lights on vehicles are required. Blue high intensity rotating, flashing, oscillating, or strobe lights when mounted on the driver's side of the vehicle may be operated simultaneously with the amber beacons or strobe lights.
- The use of truck mounted attenuators (TMA) on the SHADOW VEHICLE, ADVANCE WARNING and TRAIL VEHICLE are required.
- 4. Reflective sheeting on the rear of the TMA shall meet or exceed the reflectivity and color requirements of DEPARTMENTAL MATERIAL SPECIFICATION DMS 8300, Type A.
- Flashing arrow boards shall be Type B or Type C as per the Barricade and Construction (BC) standards. The board shall be controlled from inside the
- Each vehicle shall have two-way radio communication capability.
  When work convoys must change lanes, the TRAIL VEHICLE should change lanes
- which work convoys must change ranes, the TRAIL VEHICLE should change ranes first to shadow the other convoy vehicles. Vehicle spacing between the TRAIL VEHICLE and the SHADOW VEHICLE will vary depending on sight distance restrictions. Motorists approaching the convoy should be able to see the TRAIL VEHICLE in time to slow down and/or change lanes as they approach the TRAIL VEHICLE. Vehicle spacing between the WŎRK VEHICLE and SHADOW VEHICLE and vehicle spacing between WORK VEHICLE and LEAD VEHICLE may vary according to terrain, work activity and other factors. X VEHICLE CONVOY (CW21-10cT) or WORK CONVOY (CW21-10aT) signs shall be used on
- TRAIL VEHICLES and SHADOW VEHICLES as shown. As an option 48" x 48" diamond shaped WORK CONVOY (CW21-10T) or X VEHICLE CONVOY (CW21-10bT) signs may be used where adequate mounting space exists. When used, the X VEHICLE CONVOY sign shall have the number of the convoy vehicles displayed on the sign in the number designation "X" location. The X VEHICLE CONVOY sign shall not be used on the SHADOW VEHICLE if a TRAIL VEHICLE is used.
- 10.For divided highways with two or three lanes in one direction, the appropriate LEFT LANE CLOSED (CW20-5bTL), RIGHT LANE CLOSED (CW20-5bTR), or CENTER LANE CLOSED (CW20-5dT) sign should be used on the Advance Warning Vehicle. As an option, a portable changeable message sign (PCMS) or truck mounted changeable message sign (TMCMS) with a minimum character height of 12", and displaying the same legend may be substituted for these signs. An appropriate directional arrow display, simulating the size and legibility of the flashing arrow board may be used in the second phase of the PCMS/TMCMS message. When this is done, the arrow board will not be required on the Advance Warning Vehicle.
- 11.A double arrow shall not be displayed on the arrow board on the Advance Warning
- 12. For divided highways with three or four lanes in each direction, use TCP(3-2). 13. Standard diamond shape versions of the CW20-5 series signs may be used as an
- option if the rectangular signs shown are not available.
- 14. The Advance Warning Vehicle may straddle the edgeline when Shoulder width makes it necessary.
- 15. On two-lane two-way roadways, the work and protection vehicles should pull over periodically to allow motor vehicle traffic to pass. If motorists are not allowed to pass the work convoy, a DO NOT PASS (R4-1) sign should be placed on the back of the rearmost protection vehicle.



Traffic Operation Division Standard

TRAFFIC CONTROL PLAN MOBILE OPERATIONS RAISED PAVEMENT MARKER INSTALLATION/ **REMOVAL** TCP(3-3)-14

FILE:	tcp3-3.dgn	DN: T>	OOT	ck: TxDOT	DW:	TxDO	CK: TxDOT
© TxD0T	September 1987	CONT	SECT	JOB			HIGHWAY
REVISIONS 2-94 4-98 8-95 7-13		0106	04	036		US 380	
		DIST		COUNTY			SHEET NO.
1-97 7-1	4	ABL		STONEWA	٩LL		48



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Engineering Practice Act". of this standard to other

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Single Slope Concrete Traffic Barrier

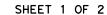
(Optional) Conduit

Trough (See General

Precast SSCB barrier may be connected to cast-in-place SSBC. The joint connection "Types" may be used in the cast-in-place barrier, to match the precast barrier connection.

#### General Notes

- 1. Concrete shall be Class H with a minimum compressive strength of 3,600 psi.
- 2. Where used, rebar reinforcement shall be Grade 60 and conform to ASTM A615.
- 3. Precast barrier length shall be 30 ft. unless otherwise specified on the plans.
- 4. All precast barrier edges shall have a 3/4 " chamfer or a tooled radius.
- 5. All concrete, reinforcement, joint connection systems, grout etc. as shown, are considered as part of the barrier payment.
- 6. Conduit trough when required shall be shown elsewhere on the plans, or as directed by the Engineer.
- 7. Regardless of the method of handling, barrier lifting points shall be approx. 7.5 feet from the ends of the barrier. Lifting devices and attachments to barrier sections shall be approved by the Engineer.
- 8. Surface finishing and grouting (where required) shall be two parts sand one part cement with enough water to make the mixture plastic. Grouting shall be done in a manner that will assure a smooth surface. Surface finishing shall be considered subsidiary to the various
- 9. All steel assemblies shall be galvanized after fabrication in accordance with Item 445, "Galvanizing.



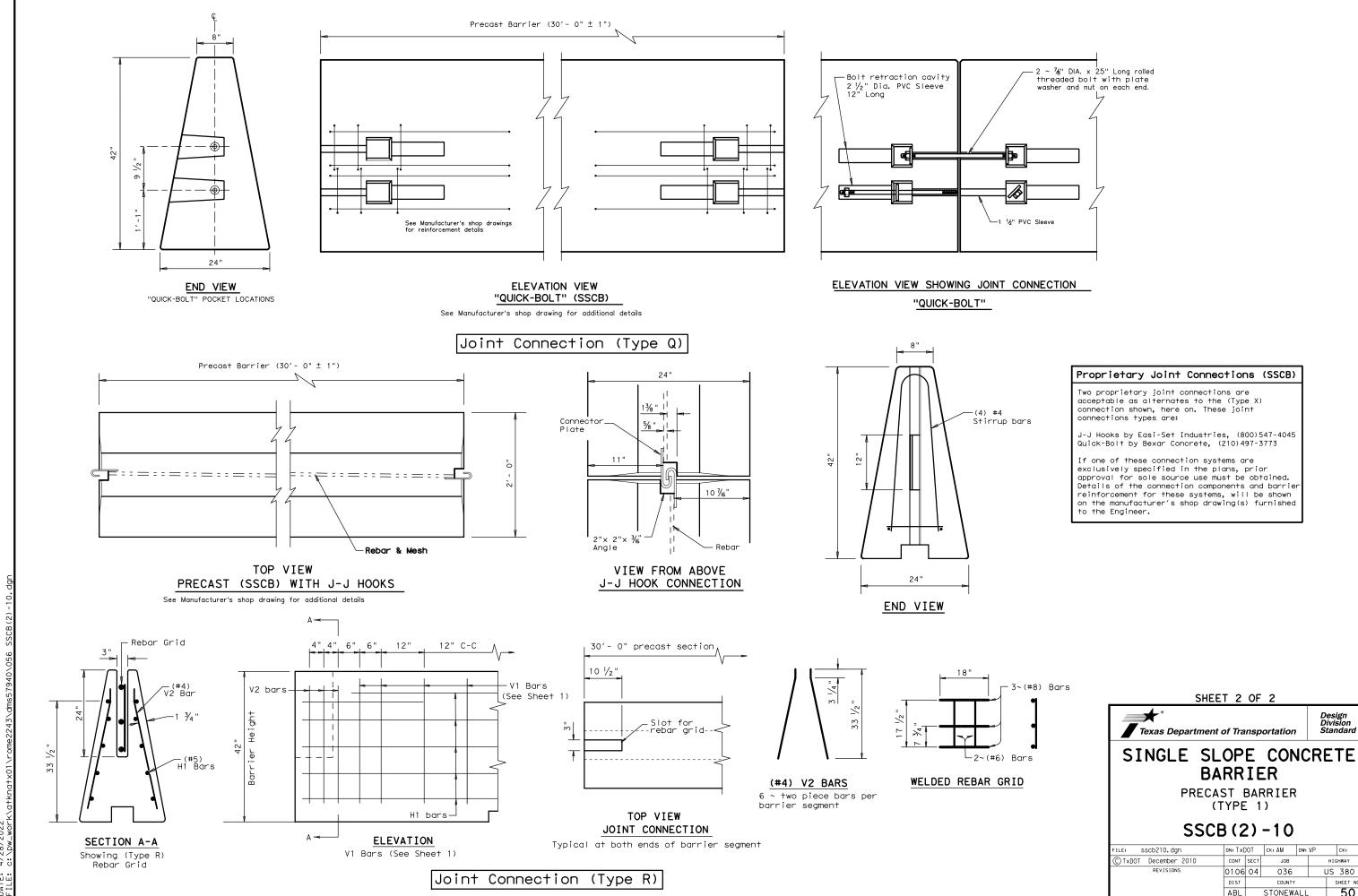


# SINGLE SLOPE CONCRETE BARRIER

PRECAST BARRIER (TYPE 1)

SSCB(2)-10

DN: TxDOT CK: AM DW: BD sscb210.dgn C)TxDOT December 2010 CONT SECT JOB HIGHWAY 0106 04 036 US 380 ABL STONEWALL 49

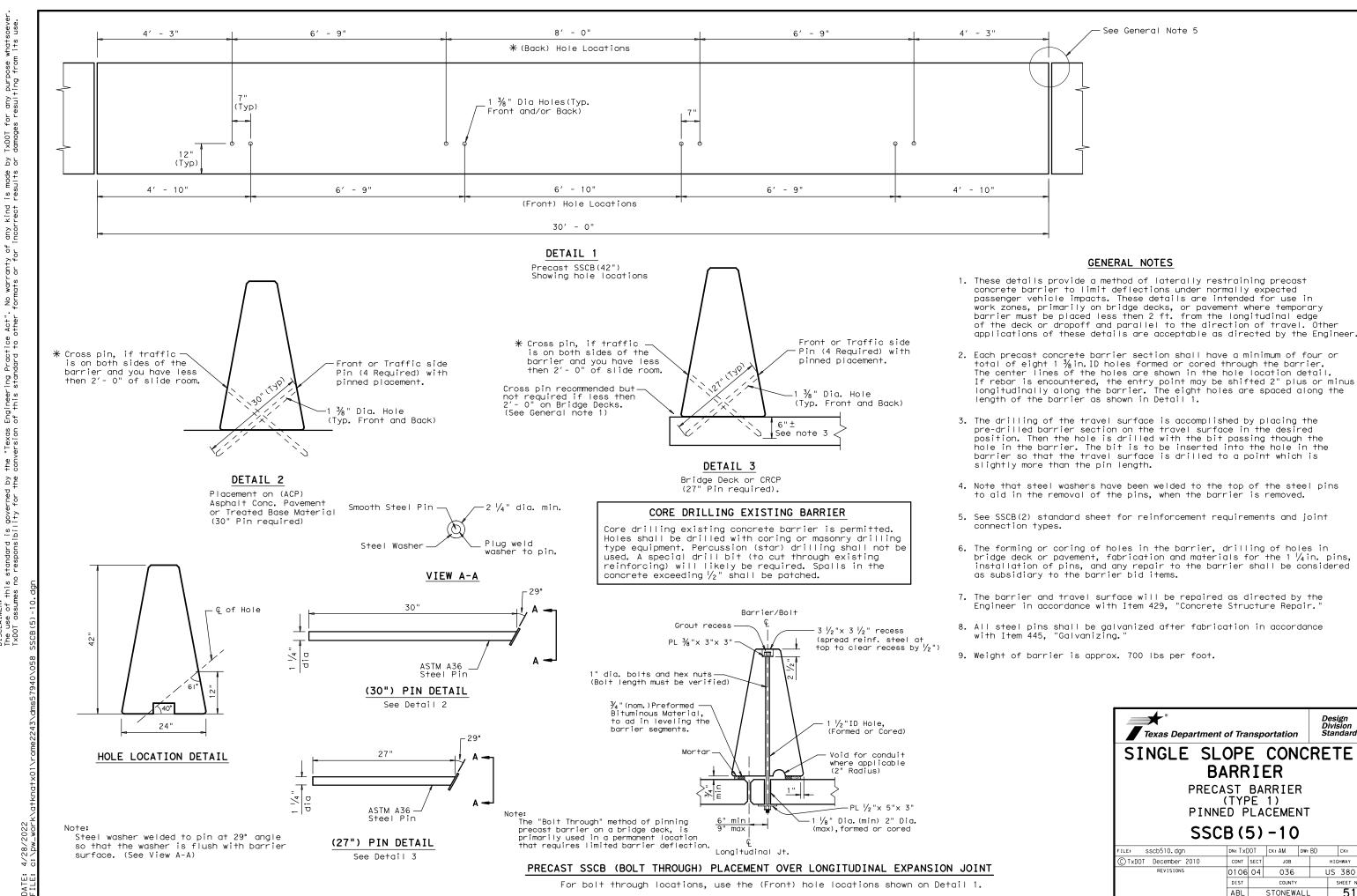


HIGHWAY US 380

50

DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind TXDOI assumes no responsibility for the conversion of this standard to other formats or for incorrect

is made by TxDOT for any purpose whatsoever results or damages resulting from its use.

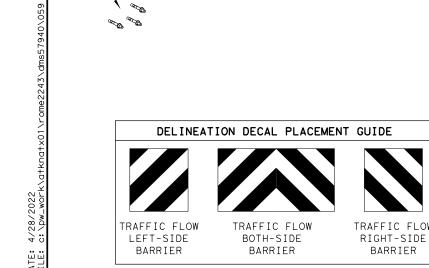


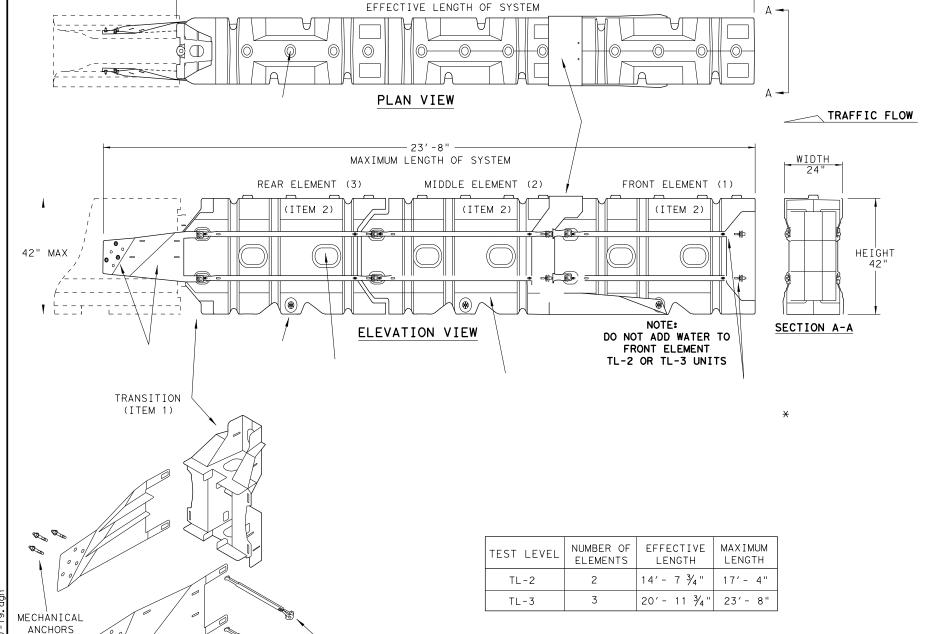
HIGHWAY

US 380

Engineering Practice Act". of this standard to other "Texas the conv this standard is governed by es no responsibility for the

(ITEM 13)





SYSTEM SHOWN - ABSORB-M TL-3

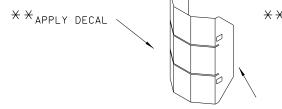
- 20′ -11 ¾" —

#### GENERAL NOTES

- 1. FOR SPECIFIC INFORMATION REGARDING THE INSTALLATION AND TECHNICAL GUIDANCE, CONTACT: LINDSAY TRANSPORTATION SOLUTIONS (LTS) - BARRIER SYSTEMS, INC. AT (707) 374-6800. 180 RIVER ROAD, RIO VISTA, CA 94571
- 2. THE ABSORB-M SYSTEM IS ONLY APPROVED FOR USE IN (TEMPORARY WORK ZONE) LOCATIONS.
- 3. THE ABSORB-M IS A WATER FILLED NON-REDIRECTIVE, GATING CRASH CUSHION THAT DOES NOT NEED TO BE ATTACHED TO A FOUNDATION AND CAN BE INSTALLED ON TOP OF CONCRETE, ASPHALT, OR ANY SURFACE CAPABLE OF BEARING THE WEIGHT OF THE SYSTEM.
- 4. MAXIMUM PERMISSIBLE CROSS-SLOPE IS 8%.
- 5. THE INSTALLATION AREA SHOULD BE FREE FROM CURBS, ELEVATED OBJECTS, OR DEPRESSIONS.
- 6. THE ABSORB-M SHOULD BE LOCATED APPROXIMATELY PARALLEL WITH THE BARRIER.
- 7. THE USE OF THE ABSORB-M IS RESTRICTED TO A BARRIER HEIGHT OF UP TO 42 INCHES.
- 8. DO NOT ADD WATER TO FRONT ELEMENT (TL-2 OR TL-3 UNIT).

	BILL	OF MATERIALS	(BOM) ABSORB-M TL-3 & TL-2 SYSTEMS	QTY	QTY
	ІТЕМ #	PART NUMBER	PART DESCRIPTION	TL-2 SYSTEM	TL-3 SYSTEM
	1	BSI-1809036-00	TRANSITION-(GALV)	1	1
Г	2	BSI-1808002-00	PRE-ASSEMBLED ABSORBING (ELEMENTS)	2	3
	3	BSI-4004598	FILL CAPS	8	12
×	4	BSI-4004599	DRAIN PLUGS	2	3
~	5	BSI-1809053-00	TENSION STRAP-(GALV)	8	12
	6	BSI-2001998	C-SCR FH 3/8-16 X 1 1/2 GR5 PLT	8	12
L	7	BSI-2001999	C-SCR FH 3/8-16 X 1 GR5 PLT	8	12
	8	BSI-1809035-00	MIDNOSE-(GALV)	1	1
	9	BSI-1808014-00	NOSE PLATE	1	1
	10	BSI-1809037-00	TRANSITION STRAP (LEFT-HAND)-(GALV)	1	1
	11	BSI-1809038-00	TRANSITION STRAP (RIGHT-HAND)-(GALV)	1	1
	12	BSI-1808005-00	PIN ASSEMBLY	8	10
	13	BSI-2002001	ANC MECH 5/8-11X5 (GALV)	6	6
	14	ABSORB-M	INSTALLATION AND INSTRUCTIONS MANUAL	1	1

<sup>\*</sup>COMPONENTS PRE-ASSEMBLED WITH ELEMENT ASSEMBLY



\*\* NOTE: (PROVIDED BY OTHERS) ENGINEER OR CONTRACTOR SHALL COORDINATE WITH THE MANUFACTURER FOR THE CORRECT DECAL PER TRAFFIC FLOW, LEFT, RIGHT OR BOTH-SIDES.

TRAFFIC FLOW

NOSE PLATE

APPLY A HIGH REFLECTIVE DECAL TO THE NOSE PLATE. DELINEATION DECAL ORIENTATION IS SHOWN ON THE CONSTRUCTION PLAN SET AND SHALL BE IN ACCORDANCE WITH THE TEXAS MUTCD FOR (TRAFFIC CONTROL DEVICES). DECALS ARE AVAILABLE FOR TRAFFIC FLOW ON THE LEFT-SIDE, BOTH -SIDES AND RIGHT-SIDE.

THIS STANDARD IS A BASIC REPRESENTATION OF THE ABSORB-M, IT IS NOT INTENDED TO REPLACE THE INSTALLATION INSTRUCTIONS MANUAL.

(MASH TL-3 & TL-2) TEMPORARY - WORK ZONE ABSORB (M) -19 DN: TxDOT CK: KM DW: VP CK: ILE: absorbm19

Texas Department of Transportation

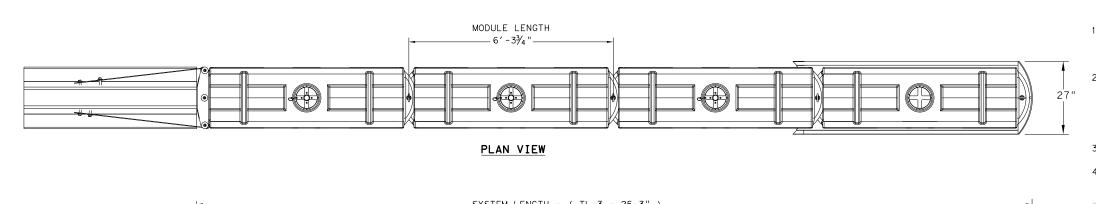
C) TxDOT: JULY 2019 CONT SECT JOB HIGHWAY 0106 04 036 US 380 STONEWALL

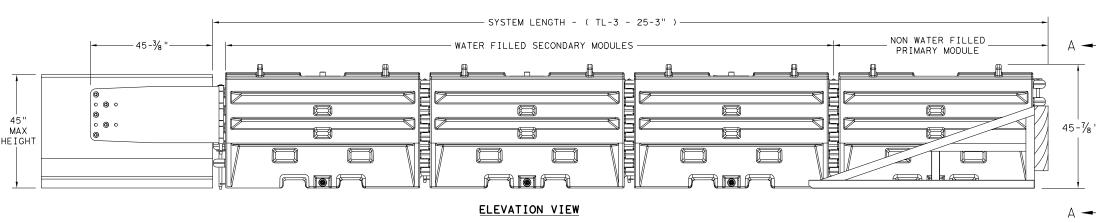
LINDSAY TRANSPORTATION SOLUTIONS

CRASH CUSHION

SACRIFICIAL





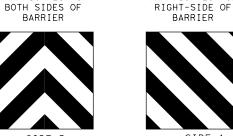




SECTION A-A



TRAFFIC FLOW ON





TRAFFIC FLOW ON

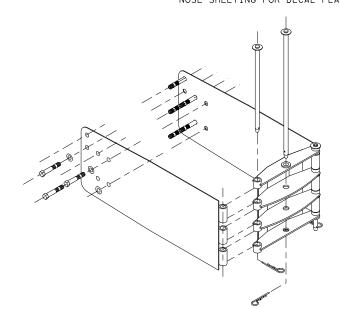


TRAFFIC FLOW ON

LEFT-SIDE OF

ROTATED 90 DEGREES

NOSE SHEETING PANEL DELINEATION SEE INSTALLATION MANUAL FOR CUSTOMIZED DELINEATION NOSE SHEETING FOR DECAL PLACEMENT.



# TRANSITION OPTIONS SLED TRANSITION TO CONCRETE TRAFFIC BARRIER (TEMPORARY OR PERMANENT)

TEST LEVEL

TL-3

NUMBER OF

SECONDARY MODULES

SYSTEM LENGTH

25′ 3"

SLED TRANSITION TO STEEL TRAFFIC BARRIER (CONTACT MFGR FOR PROPER TRANSITION)

SLED TRANSITION TO PLASTIC TRAFFIC BARRIER (CONTACT MFGR FOR PROPER TRANSITION)

SLED TRANSITION TO W-BEAM OR THRIE BEAM GUARD RAIL (CONTACT MFGR FOR PROPER TRANSITION)

SLED TRANSITION TO CONCRETE BRIDGE ABUTMENT

#### SLED TRANSITION COMPONENTS FOR ATTACHMENT TO CMB

SEE MANUFACTURER'S INSTALLATION MANUAL FOR FURTHER DETAILS.

THIS STANDARD IS A BASIC REPRESENTATION OF THE SLED. IT IS NOT INTENDED TO REPLACE THE INSTALLATION INSTRUCTIONS MANUAL.

#### **GENERAL NOTES**

- 1. REFER TO THE INSTALLATION MANUAL FOR SPECIFIC SYSTEM ASSEMBLY AND MODULE ORIENTATION. FOR ADDITIONAL INFORMATION, CONTACT TRAFFIX, INC. AT (949) 361-5663.
- 2. THE SLED SYSTEM IS A MASH APPROVED TEST LEVEL 3 (TL-3) CRASH CUSHION APPROVED FOR USE IN TEMPORARY WORK ZONES. THE SLED SYSTEM IS A NON-REDIRECTIVE, GATING CRASH CUSHION THAT DOES NOT NEED TO BE ATTACHED TO THE GROUND AND CAN BE INSTALLED ON CONCRETE, ASPHALT, GRAVEL OR COMPACTED SOIL.
- 3. MAXIMUM PERMISSIBLE CROSS SLOPE IS 8° (DEGREES) (14%).
- 4. THE INSTALLATION AREA SHOULD BE FREE FROM CURBS, ELEVATED OBJECTS, OR DEPRESSIONS.
- 5. THE SLED SYSTEM CAN BE ATTACHED TO:
  - CONCRETE BARRIER, TEMPORARY OR PERMANENT, 45" MAXIMUM HEIGHT
  - .STEEL BARRIER
  - . PLASTIC BARRIER
  - CONCRETE BRIDGE ABUTMENTS
  - .W-BEAM GUARD RAIL
  - THRIE BEAM GUARD RAIL

BILL OF MATERIAL							
PART NUMBER	DESCRIPTION	QTY: TL-3					
45131	TRANSITION FRAME, GALVANIZED	1					
45150	TRANSITION PANEL, GALVANIZED	2					
45147-CP	TRANSITION SHORT DROP PIN W/ KEEPER PIN, GALVANIZED	2					
45148-CP	TRANSITION LONG DROP PIN W/ KEEPER PIN, GALVANIZED	1					
45050	ANCHOR BOLTS	9					
12060	WASHER, 3/4" ID X 2" OD	9					
45044-Y	SLED YELLOW WATER FILLED MODULE	3					
45044-YH	SLED YELLOW "NO FILL" MODULE	1					
45044-S	CIS (CONTAINMENT IMPACT SLED), GALVANIZED	1					
45043-CP	T-PIN W/ KEEPER PIN	4					
18009-B-I	FILL CAP W/ "DRIVE BY" FLOAT INDICATOR	3					
45033-RC-B	DRAIN PLUG	3					
45032-DPT	DRAIN PLUG REMOVAL TOOL	1					



SLED CRASH CUSHION TL-3 MASH COMPLIANT (TEMPORARY, WORK ZONE)

SLED-19

DN: TxDOT CK: KM DW: VP ILE: sled19.dgn C) TxDOT: DECEMBER 2019 CONT SECT JOB HIGHWAY 0106 04 036 US 380 STONEWALL 53

SACRIFICIAL

SACRIFICIAL

ABL STONEWALL

ty fc		
TXDOT assumes no responsibility fo		
no re		
assumes		⊆
TxDOT		tcb17.dg
		57940\ca
		243\dms
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	DATE: 4/28/2022 6:37:24 PM	FILE: c:\pw_work\atknatx01\rome2243\dms57940\catcb17.dgn
	DATE:	FILE:

CATCB FRONT SECTION (POSTS 1 THRU 6) BILL OF MATERIAL Code DESCRIPTION 1 Nose Plate (10 Ga) 983G 984G 2 Side Plate (10 Ga) 31G 2 "W" Beam 12 Ga x 13′-6 1/2 130A 2 "W" Beam 10 Ga x 13′-6 ½ 9852A | 1 | Channel Strut x 6'-6" 740G | 6 | Steel Foundation Tube 766G 6 Soil Plate 18" x 24" Wood Post  $5\frac{1}{2}$ " x  $7\frac{1}{2}$ " (Notched) 3075B (Post 1) 3074B 5 Wood Post 5½" x 7½"(Post 2-6) 3100B 2 Wood Block  $5\frac{1}{2}$  x  $7\frac{1}{2}$  (Post 1) 3101B | 10 | Wood Block  $5\frac{1}{2}$ " x  $7\frac{1}{2}$ " (Post 2-6) 9916A 1 Sleeve (Post 1) 9915A | 1 | Spacer Channel (Post 2) 9921G 2 Steel Tube (Posts 4 & 6) 19271G | 1 | Pipe Sleeve (Post 1) 1 Pipe Sleeve (Post 2) 19261G 2 Post Plate (Post 4) 782G | 1 | Bearing Plate (Post 1) 1 Cable Assembly(Posts 1 to 2) 3275G 2 3%" Restraint Rod(Post 3 & 5 19259G 32 Plate Washer (Posts 4 & 6) HARDWARE 3263G 4 3/8" x 2" Lg Lag Screw 4252G 8 3/8" Hex Nut 4258G 4  $\frac{3}{8}$ " Lock Washer 4257G | 4 | 3/8" Flat Washer 3320G 4 Rectangular Washer 3395G 32  $\frac{5}{8}$ " ×  $1\frac{3}{4}$ " H.H. Splice Bolt 3650G 2  $\frac{5}{8}$ " x 25" Lg H.G.R. Bolt 46406 8 5%" × 24" Lg H.H. Bolt 3478G 13 5%" × 7½" Lg H.H. Bolt 3380G 8 5%" × 1½" Lg H.H. Bolt 3360G 16 5%" × 1½" Lg H.H. Bolt 3340G 85 5%" × 1¼" Lg H.G.R. Bolt 3300G 8 1%" Flat Washer 34976 6  $\frac{5}{8}$ " x  $9\frac{1}{2}$ " Lg H.H. Bolt 39106 4 1" Hex Nut 3900G 2 1"Flat Washer

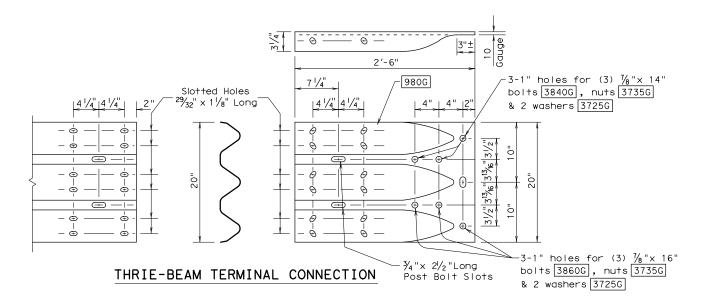
### CATCB GUARDRAIL TERMINAL END SECTION (POSTS 7 & 8) BILL OF MATERIAL Mfr DESCRIPTION Code # 4064B | 2 | Wood Post 5 $\frac{1}{2}$ " x 7 $\frac{1}{2}$ " x 6' 3101B 4 Wood Block 5 $\frac{1}{2}$ " x 7 $\frac{1}{2}$ " "W" Beam Guard Rail (12 Ga) 1 "W" Beam Guard Rail (12 Ga) 701A 1 Bracket Bearing Plate 705G 1 Pipe Sleve 3000G | 1 | Cable Assembly 3320G 2 Rectangular Washer HARDWARE 3360G 24 $\frac{5}{8}$ " × $\frac{1}{4}$ " H.G.R. Splice Bolt 3400G | 4 $\frac{5}{8}$ " × 25" H.G.R. Post Bolt 3380G 8 $\frac{5}{8}$ " x $1\frac{1}{2}$ " Hex Hd Bolt 3340G 28 $\frac{5}{8}$ " H.G.R. Nut 3300G 8 1/8" Washer 3910G 4 1" Hex Nut 3900G 2 1" Washer

#### CATCB TRANSITION SECTION (POST 9 THRU END SHOE) BILL OF MATERIAL Mfr DESCRIPTION Code # 211G 4 Thrie beam 12'-6"(12 Ga) 974G 2 Trans panel 6'-3"(12 Ga) 2 | Special Thrie beam end shoe 3078B 3 Wood Post 6" x 8" x 6', (Posts11&12) 3320G | 20 | Rectangular Washer 3340G 62 5/8" H.G.R. Nut 3400G 52 5/8" x 2" Splice Bolt 3406B 2 22 ½" Block 6"x 3 ½" (Post 12) 3407B 2 22 ½" Block 6"x 4 ½" (Post 11) 3408B 2 22 $\frac{1}{2}$ " Block 6" x 5 $\frac{1}{2}$ " (Post 10) 3409B | 2 | 22 $\frac{1}{2}$ " Block 6" x 6 $\frac{1}{2}$ " (Post 9) 3412B | 1 | Wood Post 6" x 8" x 6', (Posts 9) 3560G 2 5/8" × 16" Bolt 4406G 8 $\frac{5}{8}$ " x 3 $\frac{3}{4}$ " Expansion Bolts w/Nuts 3580G 2 $\frac{5}{8}$ " x 18" Post Bolt (Post 12) 3600G 2 5%" x 20" Post Bolt (Post 11) 3620G 2 5%" x 22" Post Bolt (Post 10) 3640G 2 5%" x 24" Post Bolt (Post 9) 37256 12 7/8" Washer (End Shoe Bolts) 37356 6 7/8" Hex Nuts (End Shoe Bolts) 38406 3 7/8" x 14" Hex Bolt (End Shoe) 3860G 3 $\frac{7}{8}$ " x 16" Hex Bolt (End Shoe) 9606A 2 Spacer Bracket Delineation 3177B | 2 | Object Marker 18" x 18" (Cut to fit) Optional Hardware for Single Slope Barrier-42"

\* Expansion or through bolts may be used with optional bracket installation.

#### **GENERAL NOTES**

- 1. For specific information regarding installation and technical guidance of the system, contact: Trinity Highway at 1(888)323-6374. 70 W. Madison St. Suite 2350. Chicago, IL 60602
- 2. Crown will be widened to accommodate the CAT system. The crown should extend at least 3 feet beyond the inside face of rail. The ground line at posts should be an extension of the roadway surface crown.
- 3. All bolts, nuts, washers, cable assemblies, cable anchors, post tubes, backup plates, and soil plates shall be galvanized.
- 4. The exposed end segment of an "End Section" should be evaluated as a potential obstacle in the determination of the need of MBGF for the opposing direction of traffic.
- 5. For placement at curb sections, the height from gutter pan to post bolt will be 21", and the front section shall be flared (See Detail 2).
- 6. The wood blockouts shall be "toe nailed" to the rectangular wood posts to prevent them from turning when the wood shrinks.
- 7. Either 6"- 8" or  $5 \frac{1}{2}$ " x  $7 \frac{1}{2}$ " wood blocks may be used at posts 1 thru 8 as supplied by the manufacturer.
- 8. If a "single sided" transition section is required for the attachment to a rigid concrete rail, see the MBGF transition standards for the proper installation.
- 9. Object markers shall be installed on the front of the terminal as detailed on the D&OM(VIA).



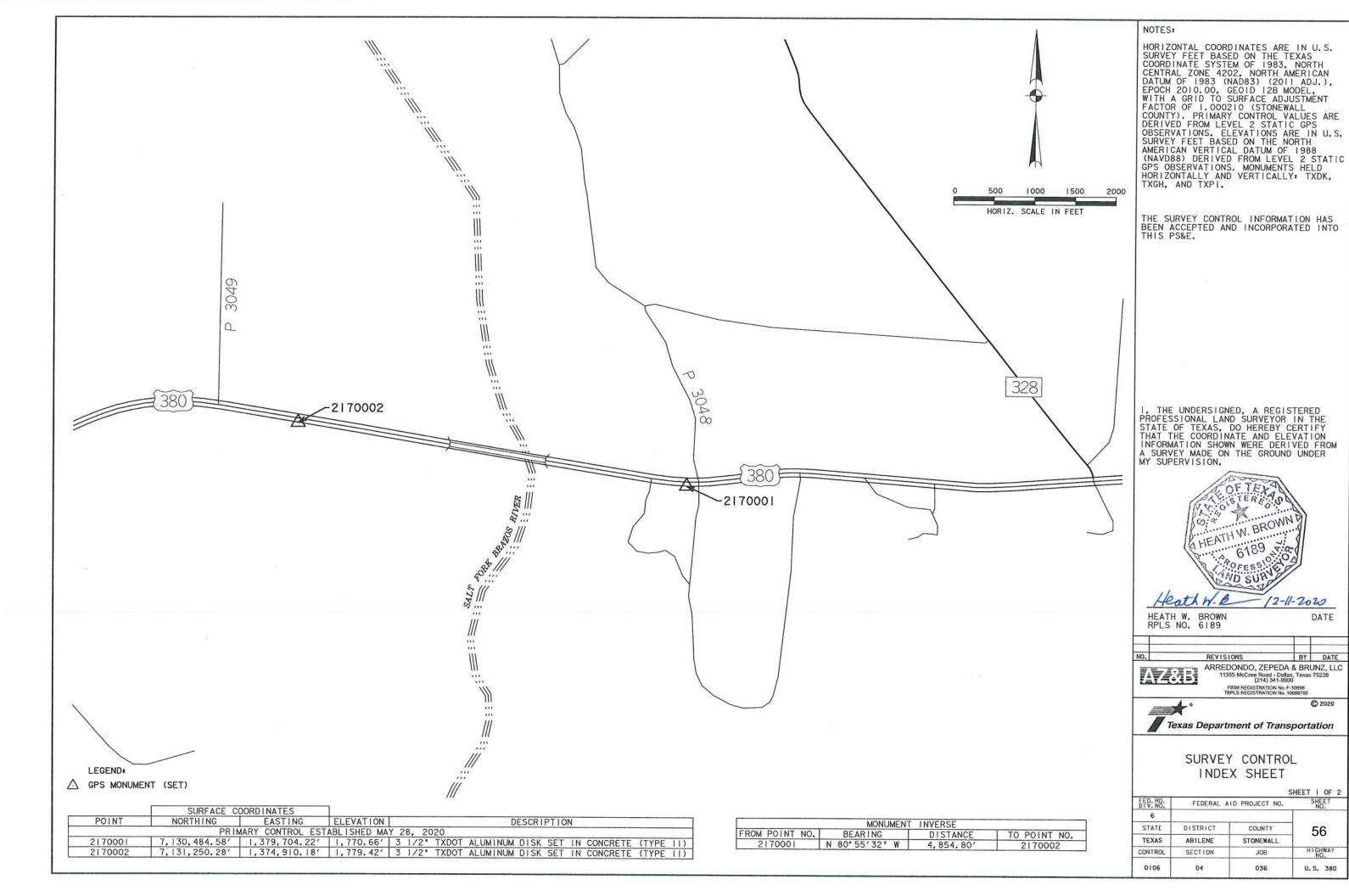
SHEET 2 OF 2



TRINITY HIGHWAY ENERGY ABSORPTION CRASH CUSHION (CONCRETE BARRIER)

CATCB(1) - 17

: catcb17.dgn	DN: Tx[	)OT	ck: KM	ow: BD		Dw: BD			ck: VP
TxDOT: 1997	CONT	SECT	JOB	JOB		HIGHWAY			
REVISIONS ISED 03,2016 VP	0106	04	036		U	S	380		
ISED 03,2017 KM	DIST		COUNTY			S	HEET NO.		
	ΔRI		STONEWA	1.12			55		



12-11-2020

DATE

© 2020

SHEET I OF 2

COUNTY

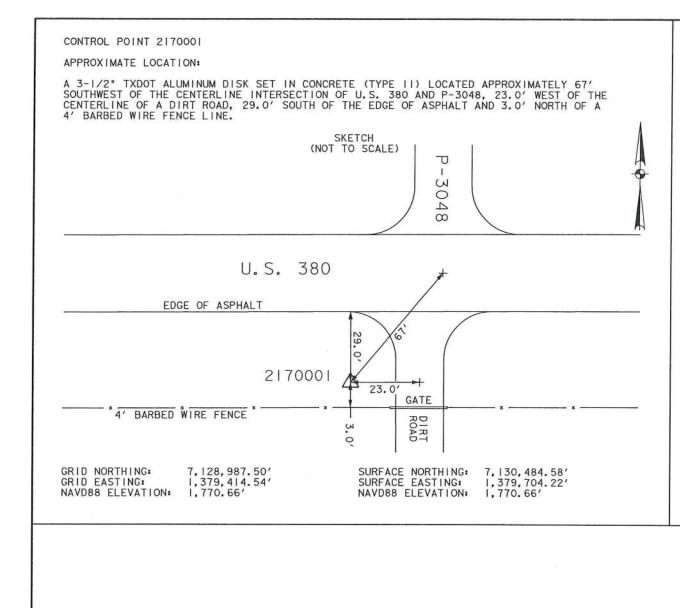
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036

SHEET NO.

56

U.S. 380

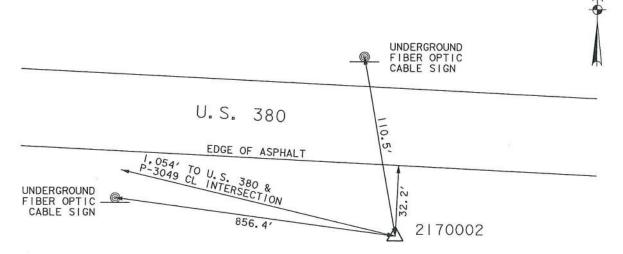


CONTROL POINT 2170002

#### APPROXIMATE LOCATION:

A 3-1/2" TXDOT ALUMINUM DISK SET IN CONCRETE (TYPE II) LOCATED APPROXIMATELY 1,054' SOUTHEAST OF THE CENTERLINE INTERSECTION OF U.S. 380 AND P-3049, II0.5' SOUTHEAST OF AN UNDERGROUND FIBER OPTIC CABLE SIGN, 32.2' SOUTHWEST OF THE EDGE OF ASPHALT AND 856.4' SOUTHEAST OF ANOTHER UNDERGROUND FIBER OPTIC CABLE SIGN.

SKETCH (NOT TO SCALE)



GRID NORTHING: GRID EASTING: NAVD88 ELEVATION:

7, 129, 753.03' 1,374,621.51' 1,779.42'

SURFACE NORTHING: 7, 131, 250, 28' SURFACE EASTING: 1,374,910.184 NAVD88 ELEVATION: 1,779.42'

NOTES:

HORIZONTAL COORDINATES ARE IN U.S. SURVEY FEET BASED ON THE TEXAS COORDINATE SYSTEM OF 1983, NORTH CENTRAL ZONE 4202, NORTH AMERICAN DATUM OF 1983 (NAD83) (2011 ADJ.), EPOCH 2010.00, GEOID 12B MODEL, WITH A GRID TO SURFACE ADJUSTMENT FACTOR OF 1.000210 (STONEWALL COUNTY). PRIMARY CONTROL VALUES ARE DERIVED FROM LEVEL 2 STATIC GPS OBSERVATIONS. ELEVATIONS ARE IN U.S. SURVEY FEET BASED ON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD88) DERIVED FROM LEVEL 2 STATIC CRS. OBSERVATIONS. GPS OBSERVATIONS. MONUMENTS HELD HORIZONTALLY AND VERTICALLY: TXDK. TXGH, AND TXPI.

THE SURVEY CONTROL INFORMATION HAS BEEN ACCEPTED AND INCORPORATED INTO

I, THE UNDERSIGNED, A REGISTERED PROFESSIONAL LAND SURVEYOR IN THE STATE OF TEXAS, DO HEREBY CERTIFY THAT THE COORDINATE AND ELEVATION INFORMATION SHOWN WERE DERIVED FROM A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION.



REVISIONS

HEATH W. BROWN RPLS NO. 6189

© 2020

ARREDONDO, ZEPEDA & BRUNZ, LLC 11355 McCree Road - Dallas, Texas 75238 (214) 341-9900

FIRM REGISTRATION No. F-10098 TBPLS REGISTRATION No. 10088700

Texas Department of Transportation

HORIZONTAL AND VERTICAL

CONTROL SHEET

SHEET 2 OF 2 FED. RD. FEDERAL AID PROJECT NO. SHEET NO. STATE DISTRICT COUNTY 57 TEXAS ABILENE STONEWALL HIGHWAY CONTROL SECTION JOB 0106 04 036 U.S. 380

PROPOSED US 380 -	HOR	RIZONTAL	ALI	GNMENT
Chain 380CL contains: CUR 380CL_1 CUR 380CL_2 CUR 380CL_	.5			
Beginning chain 380CL description Fedture: Road_Centerline Point 36 N 7,130,72 Course from 36 to PC 380CL_1 N 81°	9.3432 E 50′58.	======================================	====== 44 S+a 3654	743+00.00
	Curve	Data *		
Curve 380CL_1 2.I. Station 745+56.96 2.I. Station 745+56.96 2.I. Station 745+56.96 2.I. Station 745+56.96 3.0 58.24" 3.0	(RT)	7,130,765.7733	E	1,378,099.3476
External = 10.6770 cong Chard = 245.1789 Mid. Ord. = 245.1789 P.C. Station 744+34.37 P.T. Station 746+79.55 C.C. = N 81° 50′ 58.48" W Ahead = N 80° 35′ 02.38" W Chord Bear = N 81° 13′ 00.43" W	N N N	7,130,748.3925 7,130,785.8304 7,141,736.2745	FE	1,378,220.7062 1,377,978.4024 1,379,794.3800
Course from PT $380CL_1$ to PC $380CL$	_2 N 80°	35′ 02.38" W D	ist 1,23	30.4551
	Curve	Data *		
Curve 380CL_2 P.I. Station 761+05.01 Delta = 1°21'46.86" Degree = 0°20'58.17" Tangent = 195.0092 Eength = 390.0000 Radius = 16.394.0359	(RT)	7,131,019.0386	E	1,376,572.1442

7,130,987.1348 E 7,131,055.5095 E 7,147,160.2855 E

1,376,764.5259 1,376,380.5758 1,379,446.6161







Texas Department of Transportation

\*\*Department of Transportation Abilene District\*\*

US 380

HORIZONTAL ALIGNMENT DATA

ED: JNR	ABL	STONEWA	۱LL	0106	04	0	36	58
: SG	STATE DISTRICT	COUNTY	,	CONTROL No.	SECTION No.		0B 0.	SHEET No.
ED: JNR	6	TEXAS	SEE TITLE SHEET				US 380	
NED: SG	FED. RD DIV. No.	STATE		PROJE	HIC	SHWAY No.		

① 1 GET 150 LF MBGF 1 MBGF TRANS (THRIE-BM) ② 1 GET 150 LF MBGF 1 MBGF TRANS (THRIE-BM) SUBMITTAL -10.5 CY RIPRAP MOW STRIP (4 IN) -LIMITS OF RIPRAP -BEGIN PROJECT BEGIN ROADWORK BEGIN MILL AND 2" OVERLAY CSJ 0106-04-036 STA 743+00.00 -BEGIN BRIDGE STA 746+90.00 -@ US 380 -380CL\_1 R=11,100 00% 20+00 LEGEND (T) EXISTING ROW <u>EXIST</u>ROW FULL RECONSTRUCTION MOW STRIP PROPOSED RIPRAP FLOW DIRECTION EXIST ROW MATCHL LIMITS OF PT 746+79.55 -END TAPER STA 746+60.97 OFF 21'-2" RT 10.9 CY RIPRAP MOW STRIP (4 IN) BEGIN TAPER STA 745+10.69 OFF 20.00' RT 380CL\_1 P.I. STATION = 745+56.96
DELTA = 1° 15′ 56.11″ (RT)
DEGREE = 0° 30′ 58.24″
TANGENT = 122.60
LENGTH = 245.18
RADIUS = 11,100.00
P.C. STATION = 744+34.37
P.T. STATION = 746+79.55 1760 1760 0 1755 1755 -BEGIN BRIDGE STA 746+90.00 PROPOSED Q US 380 - BEĞIN PROJECT BEĞIN ROADWORK BEĞIN MILL AND 2" OVERLAY CSJ 0106-04-036 - STA-743+00-00---50 1750 1750 (+)0.75 %  $\triangleleft$ (+)0.50 % 1745 1745 EXIST-GROUND-Q US: 380 \\_\_\_TX-7¦0 Y STA = 744+62.53EL = 1,748.30" 1740 1740 HW (50 YR) WSEL = 1741.31 - HW (100 YR) WSEL = 1741.67 1735 PROPOSED GRADE . ∢-1730 1730 ELEV ELEV 751.58 SCALE: 1"=100'H, 1"=10'V 750. SIGNED: SG FED. RD DIV. No. STATE TEXAS HECKED: JNR 1725 745+00 750+00

- 1. SEE "HORIZONTAL ALIGNMENT DATA" FOR ADDITIONAL INFORMATION.
- 2. SEE "BRIDGE LAYOUT" SHEETS FOR ADDITIONAL INFORMATION.
- 3. SEE "EXISTING UTILITY LAYOUTS" SHEETS FOR ADDITIONAL INFORMATION.
- 4. SEE "SIGNING AND PAVEMENT MARKING LAYOUT" SHEETS FOR ADDITIONAL INFORMATION.

TRAFFIC DIRECTION

MILL AND OVERLAY

PROPOSED MBGF



# **ATKINS**

Texas Department of Transportation Abilene District

US 380

PLAN & PROFILE BEGIN TO STA 750+00

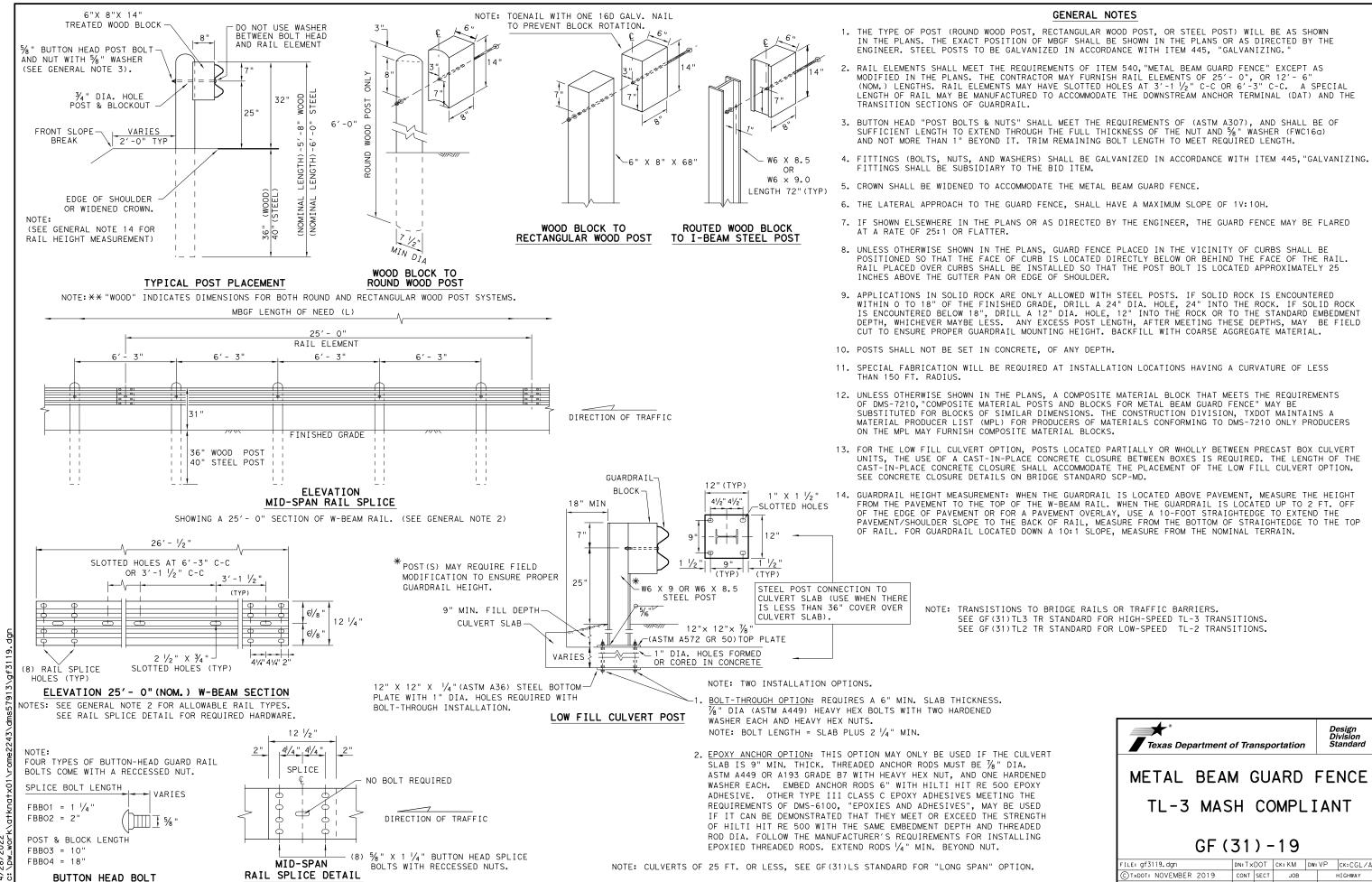
PROJECT No. SEE TITLE SHEET US 380 RAWN: SG STATE CONTROL SECTION JOB No. COUNTY STONEWALL 0106 04

1. SEE "HORIZONTAL ALIGNMENT DATA" FOR ADDITIONAL INFORMATION. (3) 1 GET (4) 1 GET 150 LF MBGF 1 MBGF TRANS (THRIE-BM) 1 MBGF TRANS (THRIE-BM) SUBMITTAL 2. SEE "BRIDGE LAYOUT" SHEETS FOR ADDITIONAL INFORMATION. 3.4 CY RIPRAP MOW STRIP (4 IN) 3. SEE "EXISTING UTILITY LAYOUTS" SHEETS FOR ADDITIONAL INFORMATION. LIMITS OF -4. SEE "SIGNING AND PAVEMENT MARKING LAYOUT" SHEETS FOR ADDITIONAL INFORMATION. END BRIDGE STA 759+10.00 00% LEGEND - © US 380 + EXISTING ROW 09 TRAFFIC DIRECTION Ω FULL RECONSTRUCTION MILL AND OVERLAY PROPOSED MBGF N 80° 35′ 02.38" W N 80° 35′ 02.38" MOW STRIP PROPOSED RIPRAP FLOW DIRECTION EXIST ROW LIMITS OF — RIPRAP PC 759+10.00
BEGIN TAPER STA 759+30.03
OFF 21'-2" RT @\_\_\_ EOP -3.4 CY RIPRAP MOW STRIP (4 IN) 1765 1765 STA\_=\_753+10.00 END BRIDGE ----STA 759+10.00 EL = 1,754.65' SSD = 1079'  $ex = -\dot{q}.54'$ 0 1760 1760 K = 300 L = 360.00' 0 09 -PROPOSED L US 380 2 1755 1755 **ATKINS** 1750 1750 Texas Department of Transportation <del>├</del> TX-54 1745 1745 Abilene District 751+30.00 US 380 王 1740 **₹** PLAN & PROFILE VPC EL. -RIPRAP STONE PROTECTION STA 750+00 TO STA 760+00 1735 1735 -PROPOSED GRADE ROP ELEV XST ELEV 752.14 SCALE: 1"=100'H, 1"=10'V SIGNED: SG FED. RD DIV. No. STATE PROJECT No. TEXAS SEE TITLE SHEET US 380 HECKED: JNR 1730 RAWN: SG STATE CONTROL SECTION JOB No. COUNTY 750+00 755+00 760+00 STONEWALL 0106 04

1. SEE "HORIZONTAL ALIGNMENT DATA" FOR ADDITIONAL INFORMATION. SUBMITTAL 2. SEE "BRIDGE LAYOUT" SHEETS FOR ADDITIONAL INFORMATION. -END FULL REONSTRUCTION BEGIN MILL AND 2" OVERLAY STA 762+68.67 3. SEE "EXISTING UTILITY LAYOUTS" SHEETS FOR ADDITIONAL INFORMATION. -7.4 CY RIPRAP MOW STRIP (4 IN) —€ US 380 4. SEE "SIGNING AND PAVEMENT MARKING LAYOUT" SHEETS FOR ADDITIONAL INFORMATION. -END PROJECT END ROADWORK END MILL AND 2" OVERLAY CSJ 0106-04-036 STA 763+00.00 -EOP 00% 00 LEGEND EXISTING ROW  $\bigcirc$ 9 TRAFFIC DIRECTION FULL RECONSTRUCTION MILL AND OVERLAY PROPOSED MBGF MOW STRIP PROPOSED RIPRAP FLOW DIRECTION EXIST ROW -END TAPER STA 760+80.22 OFF 20.00' RT -7.5 CY RIPRAP MOW STRIP (4 IN) -380CL\_2 380CL\_2 P.I. STATION = 761+05.01
DELTA = 1° 21′ 46.86″ (RT)
DEGREE = 0° 20′ 58.17″
TANGENT = 195.01
LENGTH = 390.00
RADIUS = 16,394.04
P.C. STATION = 763+00.00 JENELLE N. ROMERO 97940 9: 00 PE 1775 1775 1770 1770 0 9 1765 1765 **ATKINS** 1760 1760 Texas Department of Transportation  $\equiv$ 1755 1755 Abilene District -END-PROJECT-----END ROADWORK END WILL AND 2" OVERLAY CSJ 0106-04-036 STA 763+00.00 US 380 1-10.45 1/0 (+) 2. 1750 STA = 760+62.991750 -END-FULL RECONSTRUCTION-BEGIN MILL AND OVERLAY CSJ 0106-04-036 STA 762+68.67 PLAN & PROFILE EL = 1,751.26 ex = 1.85' STA 760+00 TO END K = 136 L = 448.80' 1745 1745 ELEV ELEV SCALE: 1"=100'H, 1"=10'V 753. 752. STATE SIGNED: SG RST. TEXAS HECKED: JNR 1740 RAWN: SG STATE 760+00

PROJECT No. HIGHWAY No. SEE TITLE SHEET US 380 CONTROL SECTION JOB No. COUNTY STONEWALL 0106 04

SHEET 3 OF



0106 04

036

STONEWAL

US 380

CCCC, CC. 7

NOTE: SEE GENERAL NOTE 3 FOR

SPLICE & POST BOLT DETAILS.

NOTE: GF(31), MID-SPAN RAIL SPLICES ARE

REQUIRED WITH 6'-3" POST SPACINGS.

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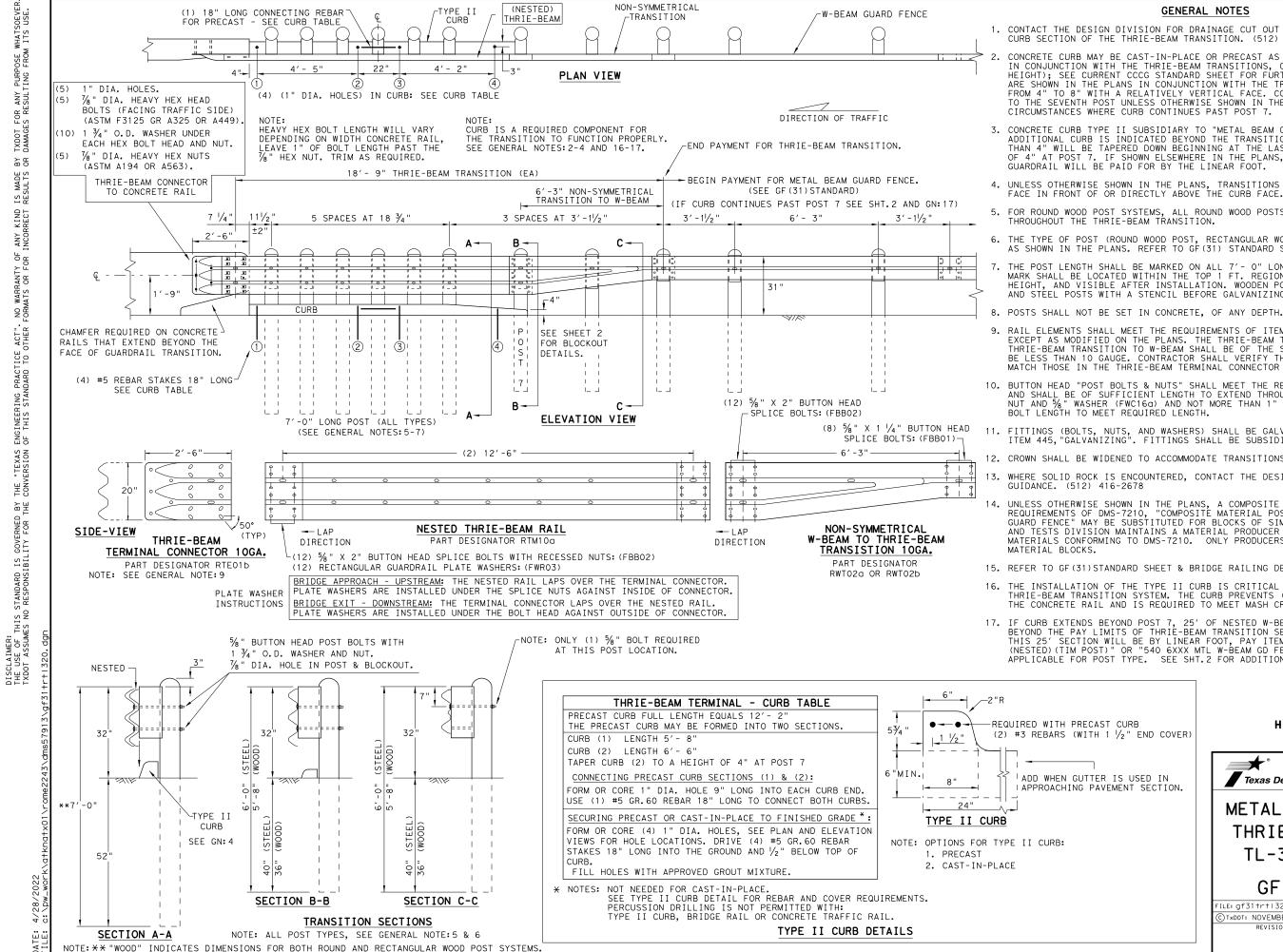
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**GENERAL NOTES** 

1. CONTACT THE DESIGN DIVISION FOR DRAINAGE CUT OUT OPTIONS NEEDED WITHIN THE CURB SECTION OF THE THRIE-BEAM TRANSITION. (512) 416-2678

CONCRETE CURB MAY BE CAST-IN-PLACE OR PRECAST AS SHOWN ON THIS SHEET. WHEN USED IN CONJUNCTION WITH THE THRIE-BEAM TRANSITIONS, CURB SHALL BE TYPE II (5- 3/4" HEIGHT); SEE CURRENT CCCG STANDARD SHEET FOR FURTHER DETAILS. IF OTHER CURB HEIGHTS ARE SHOWN IN THE PLANS IN CONJUNCTION WITH THE TRANSITION, THE CURB HEIGHT MAY BE FROM 4" TO 8" WITH A RELATIVELY VERTICAL FACE. CONCRETE CURB SHALL BE CONTINUOUS TO THE SEVENTH POST UNLESS OTHERWISE SHOWN IN THE PLANS. SEE GENERAL NOTE: 17 FOR CIRCUMSTANCES WHERE CURB CONTINUES PAST POST 7.

3. CONCRETE CURB TYPE II SUBSIDIARY TO "METAL BEAM GUARD FENCE TRANSITION". IF NO ADDITIONAL CURB IS INDICATED BEYOND THE TRANSITION, THEN ANY CURB HEIGHT GREATER THAN 4" WILL BE TAPERED DOWN BEGINNING AT THE LAST 7 FT. POST TO A MAXIMUM HEIGHT OF 4" AT POST 7. IF SHOWN ELSEWHERE IN THE PLANS, ADDITIONAL CURB UNDERNEATH GUARDRAIL WILL BE PAID FOR BY THE LINEAR FOOT.

4. UNLESS OTHERWISE SHOWN IN THE PLANS, TRANSITIONS SHALL BE PLACED WITH THE BLOCKOUT FACE IN FRONT OF OR DIRECTLY ABOVE THE CURB FACE. SEE SECTION A-A.

5. FOR ROUND WOOD POST SYSTEMS, ALL ROUND WOOD POSTS SHALL BE 7  $1\!\!/_2$  " DIA. MINIMUM THROUGHOUT THE THRIE-BEAM TRANSITION.

6. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. REFER TO GF(31) STANDARD SHEET.

THE POST LENGTH SHALL BE MARKED ON ALL 7'- O" LONG POSTS BY THE MANUFACTURER. THE MARK SHALL BE LOCATED WITHIN THE TOP 1 FT. REGION OF THE POST, AT LEAST  $\frac{5}{8}$ " IN HEIGHT, AND VISIBLE AFTER INSTALLATION. WOODEN POSTS SHALL BE MARKED WITH A BRAND, AND STÉEL POSTS WITH A STENCIL BEFORE GALVANIZING.

9. RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED ON THE PLANS. THE THRIE-BEAM TERMINAL CONNECTOR AND THE THRIE-BEAM TRANSITION TO W-BEAM SHALL BE OF THE SAME MATERIAL, BUT SHALL NOT BE LESS THAN 10 GAUGE. CONTRACTOR SHALL VERIFY THAT THE LOCATIONS OF BOLT HOLES MATCH THOSE IN THE THRIE-BEAM TERMINAL CONNECTOR PRIOR TO ORDERING MATERIALS.

10. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND  $\frac{5}{6}$ " WASHER (FWC16a) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING

11. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.

12. CROWN SHALL BE WIDENED TO ACCOMMODATE TRANSITIONS.

13. WHERE SOLID ROCK IS ENCOUNTERED, CONTACT THE DESIGN DIVISION FOR ADDITIONAL GUIDANCE. (512) 416-2678

UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. TXDOT'S MATERIALS AND TESTS DIVISION MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210. ONLY PRODUCERS ON THE MPL CAN FURNISH COMPOSITE

15. REFER TO GF (31) STANDARD SHEET & BRIDGE RAILING DETAILS FOR ADDITIONAL DETAILS.

16. THE INSTALLATION OF THE TYPE II CURB IS CRITICAL FOR THE PERFORMANCE OF THE THRIE-BEAM TRANSITION SYSTEM. THE CURB PREVENTS (VEHICLE WHEEL SNAGGING) AT THE CONCRETE RAIL AND IS REQUIRED TO MEET MASH CRASH TEST CRITERIA.

17. IF CURB EXTENDS BEYOND POST 7, 25' OF NESTED W-BEAM GUARDRAIL SHALL BE INSTALLED BEYOND THE PAY LIMITS OF THRIE-BEAM TRANSITION SECTION, (SEE SHT.2). PAYMENT FOR THIS 25' SECTION WILL BE BY LINEAR FOOT, PAY ITEM "0540 6XXX MTL W-BEAM GD FEN (NESTED) (TIM POST)" OR "540 6XXX MTL W-BEAM GD FEN (NESTED) (STEEL POST)" AS APPLICABLE FOR POST TYPE. SEE SHT.2 FOR ADDITIONAL INFORMATION.

# **HIGH-SPEED TRANSITION** SHEET 1 OF 2



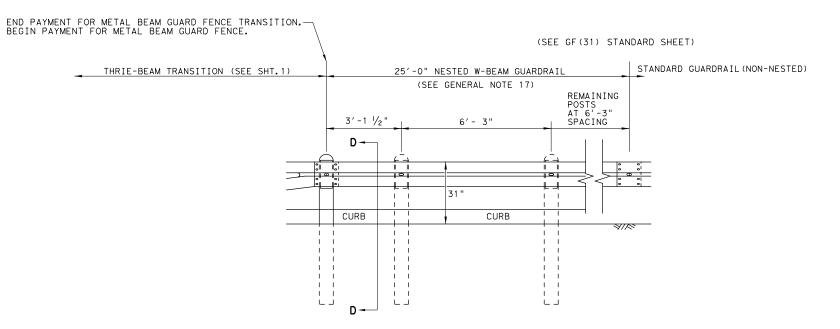
METAL BEAM GUARD FENCE THRIE-BEAM TRANSITION

TL-3 MASH COMPLIANT

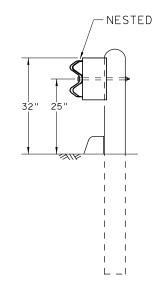
GF (31) TR TL3-20

DN:TxDOT CK: KM DW: VP CK:CGL/A ILE: gf31trt1320.dgn C)TxDOT: NOVEMBER 2020 CONT SECT JOB HIGHWAY 0106 04 036 US 380 STONEWAL

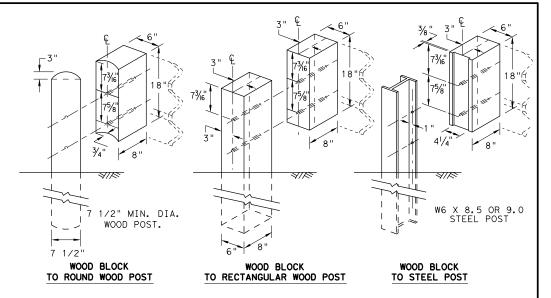
725 vork\atknatx01\rome2243\dms57913\gf31+r+1320.dgn REQUIRED ALTERNATIVE FOR CONTINUOUS CURB EXTENDING PAST POST 7 (SEE SHT. 1 GENERAL NOTE 17)



#### **ELEVATION VIEW**



SECTION D-D



# THRIE BEAM TRANSITION BLOCKOUT DETAILS

# HIGH-SPEED TRANSITION

SHEET 2 OF 2



Design Division Standard

METAL BEAM GUARD FENCE THRIE-BEAM TRANSITION TL-3 MASH COMPLIANT

GF(31)TR TL3-20

FILE: gf31trtl320.dgn	DN: Tx	DOT	ck: KM	DW:	KM	CK:CGL/AG
©TxDOT: NOVEMBER 2020	CONT	SECT	JOB			HIGHWAY
REVISIONS	0106	04	036		Į	JS 380
	DIST		COUNTY			SHEET NO.
	ABL		STONEWA	\LL		64

Curb shown on top of mow strip

HIGHWAY

ABL

STONEWALL

US 380

65

embedment throughout the system.

#### GENERAL NOTES

- FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: LINDSAY TRANSPORTATION SOLUTIONS (LTS) - BARRIER SYSTEMS, INC. AT (707) 374-6800
- FOR INSTALLATION, REPAIR, & MAINTENANCE REFER TO THE; MAX-TENSION INSTALLATION INSTRUCTION MANUAL. P/N MANMAX REV D (ECN 3516).
- 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURE'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- 5. ALL STEEL COMPONENTS ARE GALVANIZED PER ASTM A123 OR EQUIVALENT UNLESS OTHERWISE STATED.
- 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POST WITH COMPOSITE BLOCKOUTS.
- 7. COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- 8. REFER TO INSTALLATION MANUAL FOR SPECIFIC PANEL LAPPING GUIDANCE.
- 9. IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL FOR INSTALLATION GUIDANCE.
- 10. POSTS SHALL NOT BE SET IN CONCRETE.
- 11. A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POST TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST
- 12. MAX-TENSION SYSTEM SHALL NEVER BE INSTALLED WITHIN A CURVED SECTION OF GUARDRAIL.
- IF A DELINEATION MARKER IS REQUIRED, MARKER SHALL BE IN ACCORDANCE WITH TEXAS MUTCD.
- 14. THE SYSTEM IS SHOWN WITH 12'-6" MBGF PANELS, 25'-0" MBGF PANELS ARE ALSO ALLOWED.
- 15. A MINIMUM OF 12'-6" OF 12GA. MBGF IS REQUIRED IMMEDIATELY DOWNSTREAM OF THE MAX-TENSION SYSTEM.

ITEM#	PART NUMBER	DESCRIPTION	QTY
1	BSI-1610060-00	SOIL ANCHOR - GALVANIZED	1
2	BSI-1610061-00	GROUND STRUT - GALVANIZED	1
3	BSI-1610062-00	MAX-TENSION IMPACT HEAD	1
4	BSI-1610063-00	W6×9 I-BEAM POST 6FTGALVANIZED	1
5	BSI-1610064-00	TSS PANEL - TRAFFIC SIDE SLIDER	1
6	BSI-1610065-00	ISS PANEL - INNER SIDE SLIDER	1
7	BSI-1610066-00	TOOTH - GEOMET	1
8	BSI-1610067-00	RSS PLATE - REAR SIDE SLIDER	1
9	B061058	CABLE FRICTION PLATE - HEAD UNIT	1
10	BSI-1610069-00	CABLE ASSEMBLY - MASH X-TENSION	2
11	BSI-1012078-00	X-LITE LINE POST-GALVANIZED	8
12	B090534	8" W-BEAM COMPOSITE-BLOCKOUT XT110	8
13	BSI-4004386	12'-6" W-BEAM GUARD FENCE PANELS 12GA.	4
14	BSI-1102027-00	X-LITE SQUARE WASHER	1
15	BSI-2001886	5/8" X 7" THREAD BOLT HH (GR.5)GEOMET	1
16	BSI-2001885	3/4" X 3" ALL-THREAD BOLT HH (GR.5)GEOMET	4
17	4001115	5/8" X 1 1/4" GUARD FENCE BOLTS (GR.2)MGAL	48
18	2001840	5/8" X 10" GUARD FENCE BOLTS MGAL	8
19	2001636	5/8" WASHER F436 STRUCTURAL MGAL	2
20	4001116	5/8" RECESSED GUARD FENCE NUT (GR. 2)MGAL	59
21	BSI-2001888	5/8" X 2" ALL THREAD BOLT (GR.5)GEOMET	1
22	BSI-1701063-00	DELINEATION MOUNTING (BRACKET)	1
23	BSI-2001887	1/4" X 3/4" SCREW SD HH 410SS	7
24	4002051	GUARDRAIL WASHER RECT AASHTO FWR03	1
25	SEE NOTE BELOW	HIGH INTENSITY REFLECTIVE SHEETING	1
26	4002337	8" W-BEAM TIMBER-BLOCKOUT, PDB01B	8
27	BSI-4004431	25' W-BEAM GUARDRAIL PANEL, 8-SPACE, 12GA.	2
28	MANMAX Rev-(D)	MAX-TENSION INSTALLATION INSTRUCTIONS	1

Texas Department of Transportation

Design Division Standard

MAX-TENSION END TERMINAL

MASH - TL-3

SGT (11S) 31-18

LE: sg+11s3118.dgn	DN: TxE	ОТ	CK: KM DW:		T×DOT	CK: CL
TxDOT: FEBRUARY 2018	CONT	SECT	JOB		ΗI	GHWAY
REVISIONS	0106	04	036		US	380
	DIST		COUNTY			SHEET NO.
	ABL		STONEWA	٩LL		66

 $\mbox{$\star$}$  NOTE: GUARDRAIL PANELS 2 & 3 (ITEM C) MAY BE SUBSTITUTED WITH ONE 25'-0" GUARDRAIL PANEL (ITEM D). NOTE: THERE ARE NO SUBSTITUTE GUARDRAIL PANELS FOR (MODIFIED PANEL 4) END OF LENGTH OF NEED PANEL 1 TXDOT FOR ANY PURPOSE DAMAGES RESULTING FROM MODIFIED MODIFIED PANEL 2 PANEL 3 9'-4 1/2' (b, (2d), e, f) 12'-6" 12'-6" 12'-6" −3′ 1½ <del>" −| −</del>3′ 1½ <del>" −</del> (a, d, f) FIELDSIDE FACE -(H)STRUT GR PANEL -(B2) GR PANEL C GR PANEL POST 3 (E)-PLAN VIEW  $_{\rm PR}^{\rm BY}$ LENGTH OF NEED -BGR PANEL COMPOSITE BLOCKOUTS (ITEM F) MAY BE SUBSTITUTED WITH (ITEM G) WOOD BLOCKOUTS. MADE SULTS NOTE: CONFIRM ALL POST OFFSET'S AS SHOWN POST 2 POST ON THE PRODUCT DESCRIPTION ASSEMBLY MANUAL. END PAYMENT FOR SGT 1S RES DO NOT BOLT MODIFIED (PANEL 4) TO WOOD POST TRAFFIC-SIDE VIEW OFFSET DISTANCE 3 TO POST 2 = 8 3 TO POST 1 = 6 → BEGIN STANDARD 31 MBGF TRAFFIC FLOW GRABBER HARDWARE NOTE: RAIL SPLICE HARDWARE LAP GUARDRAIL SPLICES IN DIRECTION OF TRAFFIC FLOW GRABBER TEETH LOCKED ONTO FRONT (h, (2i), e, f (8) \%" X 1 \/4" GR BOLTS YIELDING POST HARDWARE OF THE MODIFIED GUARDRAIL PANEL NO WARRANTY OF FORMATS OR FOR WITH 5/8" GR HEX NUTS (1) \%"\times 10" GR BOLT NO BOLTS IN BREAKAWAY WITH 5/8" GR HEX NUT REAR TWO HOLES POST ()-(c, f) (c, f) ) A HEAD  $(\mathsf{I},\mathsf{m})$ (b, f) (b, f)--(b, f) -(b, f) - RFID CHIP ITEM QTY 4 BCT CĂBLE Q-YIELDING E-POST HEIGHT -(1,m)<sup>3</sup>/<sub>8</sub>" X 3" GR5 LAG SCREWS ∽FINISHED \HSTRUT 1/2" YIELDING (g, (2i), j, k) GRADE BEARING ALTERNATIVE ITEMS 11 1.1 -11 POST PLATE HOLES AT 41" II 11 NOTE: DEPTH (TYP 8-2) l<sub>l</sub>—(b, (2d), e, f 11 11 11 11 HARDWARE SEE PLAN VIEW 11 -11 1.1 1.1 11 Hj П Пj "TEXAS POST 5 POST POST 8 POST 7 POST 6 POST 4 POST 3 POST 2 STRUT POST **ELEVATION VIEW** ITEM (E) (YIELDING POST 8 THRU 2) ARE MODIFIED W6X8.5 STEEL THE POST WITH FOUR 1/2" YIELDING HOLES, TWO HOLES PER FLANGE. POST 1 DISCLAIMER: THE USE OF THIS STANDARD IS GOVERNED BY TXDOT ASSUMES NO RESPONSIBILITY FOR THE TRAFFIC SIDE VIEW 5 1/2" X 7 1/2" X 50" WOOD BREAKAWAY POST WOOD STRIKE BLOCK (K)-FIELD SIDE TRAFFIC 6" X 8" X 14" W6X8.5 I-BEAM POST WITH YEILDING HOLES COMPOSITE BLOCKOUT STRIKE PLATE (L) NO BOLTS IN \_SIDE\_ 17" GUARDRAIL N-MODIFIED B-REINFORCEMENT REAR TWO HOLES RAIL M PLATE ITEM 🕞 Æ ITEM S REFLECTIVE SHEETING PROVIDED BY COMPANY SGET (A)-N GUARDRAII GRABBER IMPACT HEAD SEE (GENERAL NOTE 3) (h, (2i), J, K (1) 5/8" X 10" GR BOL BEARING (1) ⊸Q BCT CABLE (1) 5/8" GR NUT BEARING O HSTRUT PLATE PPIPE SLEEVE  $(2) \frac{1}{2}$ (6h)  $\frac{1}{2}$ " X 1  $\frac{1}{4}$ " BOLTS STRUT (H)-MAXIMUM TUBE HEIGHT (b, (2d), e, f) YEILDING HOLE (12i)  $\frac{1}{2}$ " FLAT WASHER (6j)  $\frac{1}{2}$ " LOCK WASHER 5/8" × 10" GR BOLT 5/8" FLAT WASHER 3" X 3" X 80" POST LENGTH ABOVE GROUND 1/4" THICKNESS (2) YEILDING -FINISHED 5/8" HEX NUT (6k) " LOCK WASHER POST GRADE GR NUT TÜBE Œ) TUBE LENGTH TWO FLAT WASHERS EMBE PER BOLT, ONE EACH SIDE OF PANEL. POST 2 DEPTH —(I) FOUNDATION TUBE STRUT POST 6" X 8" X 72" 3/6" THICKNESS (I)-/ SIDE VIEW SIDE VIEW REINFORCEMENT PLATE POST 1 FIELD SIDE VIEW POST 1 POST 8 - POST 3 (TYP) FRONT END VIEW WITH GUARDRAIL GRABBER 50' APPROACH GRADING APPROX 5'-10" SGET MAXIMUM (OFFSET), HORIZONTAL FLARE STANDARD OVER THE FIRST 50 FEET = 1 FOOT. EDGE OF PAVEMENT-APPROACH GRADING -2'-0" MAX. (1V: 10H OR FLATTER) RAIL OFFSET NOTE: ADJUST WIDTH ACCORDINGLY WHEN OFFSET IS USED. (OFFSET "OPTION" SHOWN) THIS STANDARD IS A BASIC REPRESENTATION OF THE SGET TERMINAL SYSTEM AND IS NOT INTENDED

APPROACH GRADING AT GUARDRAIL END TREATMENTS

GENERAL NOTES

- 1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202
- 2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL.
- 3. MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER' TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.
- 4. THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.
- 5. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.
- (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS.
- 7. POSTS SHALL NOT BE SET IN CONCRETE.

TO REPLACE THE MANUFACTURER'S ASSEMBLY MANUAL

- IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.
- HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.
- A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.
- THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.



MAIN SYSTEM COMPONENTS



ITEM #

SPIG INDUSTRY, LLC SINGLE GUARDRAIL TERMINAL SGET - TL-3 - MASH SGT (15) 31-20

DN: Tx	ОТ	CK: KM	DW:\	/P	CK: VP
CONT	SECT	JOB		H	HIGHWAY
0106	04 036		U	S 380	
DIST	ST COUNTY				SHEET NO.
ABL STONEWALL				67	
	CONT 0106 DIST	DIST	CONT SECT JOB 0106 04 036 DIST COUNTY	CONT SECT JOB 0106 04 036 DIST COUNTY	CONT SECT JOB H 0106 04 036 U DIST COUNTY

#### **GENERAL NOTES**

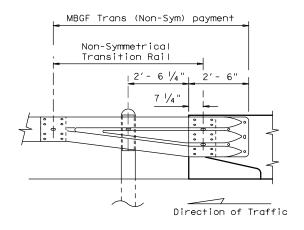
- 1. For more detail: See GF(31), SGT()31, GF(31)TR, and GF(31)TL2 standard sheets.
- 2. Quantities of metal beam guard fence (MBGF) at individual bridge ends are as shown in the plans.
- 3. Use average daily traffic (ADT) for the current year to determine MBGF length of need in accordance with the Roadway Design Manual unless otherwise specified. Where significant traffic volume growth is anticipated on low volume (0-750 ADT) highways, use length determinations for the higher volume
- 4. MBGF may not be required to shield departure end of bridge unless other obstacles within the horizontal clearance limits or opposing traffic indicate a MBGF consideration.
- 5. Downstream anchor terminals (DAT) are only for downstream end anchorage use, outside the horizontal clearance area of opposing traffic.
- 6. Direct connection of MBGF to concrete rails are only for downstream rail connections outside the horizontal clearance area of opposing traffic. (This requires a minimum of three standard line posts plus the DAT terminal,
- 7. The crown shall be widened to accommodate MBGF. Typically the "front slope" break should be 2' 0" from the back of the MBGF post. This applies to new construction on new alignment or where existing roadway cross section is to be widened to increase roadway width. This does not apply to rehabilitation work where existing roadway crown width is to be retained (See Typical Cross Section at MBGF).
- 8. For restrictive bridge widths: The MBGF should be properly transitioned from the existing bridge rail to the adjoining MBGF (See MBGF Transition Standards). Metal beam guard fence at these bridge location(s) shall be flared at the rate of 25:1 or flatter, and be of the length necessary to locate the terminal end at the 2 ft. "maximum" offset from the shoulder edge in the approach direction.
- 9. Transition length and post spacing will vary depending on the transition type. Transition type will be shown elsewhere in the plans.
- 10. A minimum 25' length of MBGF will be required.

See GF(31) standard

for post types.

Edge of shoulder

or widened crown



TYPICAL CROSS SECTION AT MBGF

All rail elements shall be lapped in the direction of adjacent traffic.

#### DETAIL A

Showing Downstream Rail Attachment



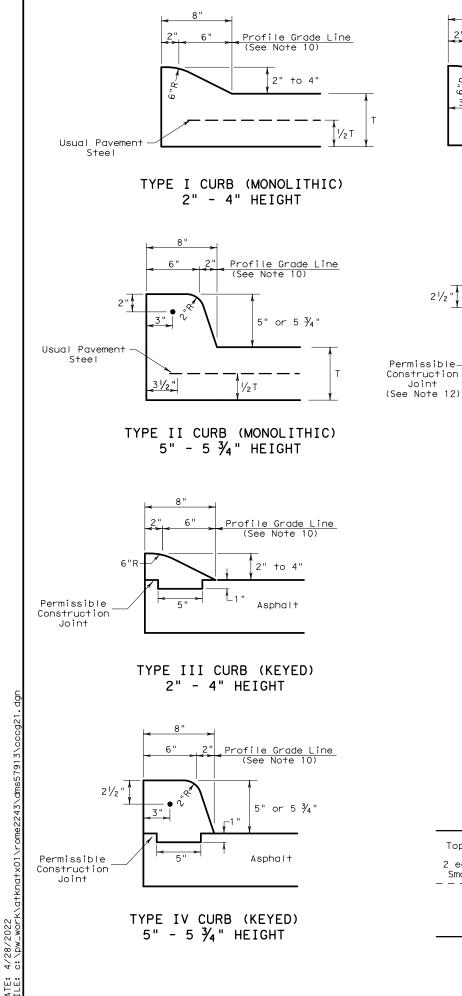
# BRIDGE END DETAILS

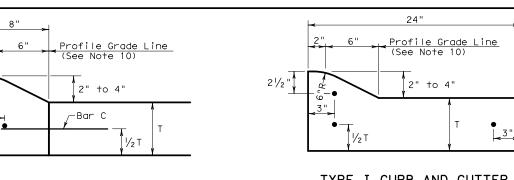
(METAL BEAM GUARD FENCE APPLICATIONS TO RIGID RAILS)

BED-14

ILE: bed14.dgn	DN: Tx[	OT.	ск: АМ	ow: BD	/VP	ck: CGL	
CTxDOT: December 2011	CONT	SECT	JOB		HIC	HIGHWAY	
REVISIONS EVISED APRIL 2014 EE (MEMO 0414)	0106	04	036		US 380		
	DIST		COUNTY			SHEET NO.	
	ABL	STONEWALL				68	

Engineering Practice Act". of this standard to other "Texas ersion this standard is governed by es no responsibility for the





Profile Grade Line

5" or  $5\frac{3}{4}$ 

 $\frac{1}{2}$ T

Profile Grade Line (See Note 10)

For Curb Height= 5

For Curb Height= 5 3/4"

5" or 5 3/4'

1/2 T

Use 2 layers of roofing felt

to wrap bars and plug end

11/2

−Bar C

TYPE IIa CURB

5" - 5 ¾" HEIGHT

Top of Curb

14"

EXPANSION JOINT DETAIL

-Bar C

TYPE II CURB

5" - 5 3/4" HEIGHT

Permissible -Construction

Joint

 $\frac{1}{2}$ " Wide Expansion

Joint Material

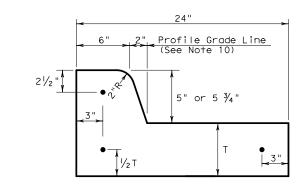
Top of Pavement

 $2 ea \sim \frac{7}{8}$ " x 24"

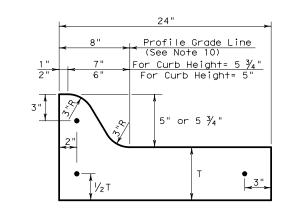
1/2 T

Smooth Dowels-

TYPE I CURB AND GUTTER TYPE I CURB 2" - 4" HEIGHT 2" - 4" HEIGHT

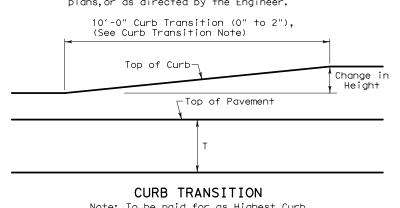


TYPE II CURB AND GUTTER 5" - 5 3/4" HEIGHT



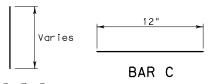
TYPE IIa CURB AND GUTTER 5" - 5 3/4" HEIGHT

Field conditions may require a longer or shorter transition, and shall be shown elsewhere in the



#### **GENERAL NOTES**

- 1. All materials and construction shall be in accordance with Item 529, "Concrete Curb, Gutter, and Combined
- 2. Concrete shall be Class A.
- When reinforcing bars are used, they shall be No.4 unless otherwise shown. The use of fiber reinforced concrete in lieu of reinforcing steel is acceptable. Use fibers meeting the requirements of DMS 4550, "Fibers for Concrete," and dose fibers in accordance with Material Producers List (MPL) "Fibers for Class A and B Concrete Applications.
- Round exposed sharp edges with a rounding tool, to a minimum radius of  $\frac{1}{4}$  inch.
- 5. All existing curbs and driveways to be removed shall be sawed or removed at existing joints.
- 6. Where concrete curb is to be placed on existing concrete pavement, Bar B may be drilled and the grouted in place, or may be inserted into fresh concrete.
- 7. Expansion and contraction joints shall be constructed to match pavement joints in all curbs and curb and gutter adjacent to jointed concrete pavement. Where placement of curb or curb and gutter is not adjacent to concrete pavement, expansion joints shall be provided at structures, curb returns at streets, and at locations directed by The Engineer.
- Vertical and horizontal dowel bars and transverse reinforcing bars shall be placed at four feet C~C.
- 9. Dimension 'T' shown is the thickness of concrete pavement. When curb is installed adjacent to flexible pavement dimension 'T' is 8" maximum.
- 10. Usual profile grade line. Refer to typical sections and plan-profile sheets for exact locations.
- 11. One-half inch expansion joint material shall be provided where curb or curb and gutter is adjacent to sidewalk
- 12. When horizontal permissible construction joints are used, the longitudinal pavement steel shall be placed in accordance with pavement details shown elsewhere in the plans. Reinforcing steel for curb section shall then conform to that required for concrete curb.
- 13. Bar B used as needed to support curb reinforcing steel during concrete placement.



BAR B



CONCRETE CURB AND CURB AND GUTTER

CCCG-21

	_	•			
ILE: cccg21.dgn	DN: TX[	)OT	ck: AN	DW: SS	ck: KM
C)TxDOT: FEBRUARY 2021	CONT	SECT	JOB		HIGHWAY
REVISIONS	0106	04	4 036 L		JS 380
	DIST				SHEET NO.
	ABL		STONEWA	69	

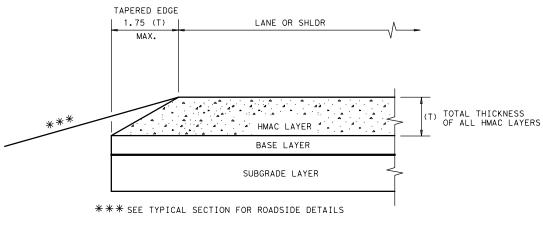
# CURB TRANSITION NOTE: plans, or as directed by the Engineer.

Note: To be paid for as Highest Curb

LANE OR SHLDR NO TAPERED EDGE REQUIRED . HMAC LAYER TOTAL THICKNESS 2.5" OR LESS EXIST. PVMT OR BASE LAYER SUBGRADE LAYER

\*\*\* SEE TYPICAL SECTION FOR ROADSIDE DETAILS

# CONDITION - 1 THIN HMAC SURFACES OR HMAC OVERLAY WITH THICKNESS OF 2.5" OR LESS



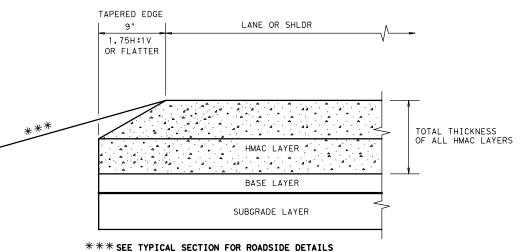
#### CONDITION - 3

NEW OR RECONSTRUCTED PAVEMENT HMAC THICKNESS 2.5" TO 5"

TAPERED EDGE 1.75 (T) LANE OR SHLDR MAX. TOTAL THICKNESS OF ALL HMAC LAYERS EXISTING PAVEMENT \*\* EXISTING ROADSIDE EMBANKMENT TO BE GRADED TO PRODUCE A SMOOTH LEVEL SURFACE FOR PLACEMENT OF TAPERED EDGE. THIS WORK IS SUBSIDIARY TO THE VARIOUS BID ITEMS.

### CONDITION - 2 OVERLAY OF EXISTING PAVEMENT HMAC THICKNESS 2.5" TO 5"

\*\*\* SEE TYPICAL SECTION FOR ROADSIDE DETAILS



#### CONDITION - 4

NEW OR RECONSTRUCTED PAVEMENT HMAC THICKNESS 5" OR GREATER

HMAC PAVEMENT TE(HMAC)-11

TAPERED EDGE DETAILS

Texas Department of Transportation

GENERAL NOTES

1. UNLESS OTHERWISE SHOWN IN THE PLANS, A VERTICAL EDGE IS PERMISSIBLE FOR HMAC PLACED GREATER THAN 5" BELOW THE EDGE OF PAVEMENT AND FOR THICKNESS OF HMAC LESS

2. FOR FURTHER INFORMATION REGARDING THE ROADSIDE AND

3. PAYMENT FOR TAPERED EDGE WILL BE IN ACCORDANCE WITH APPLICABLE ITEMS IN THE CONTRACT.

4. THE SLOPE OF THE TAPERED EDGE SHALL BE 1.75H:1V OR

5. THE TAPERED EDGE SHALL BE PRODUCED BY USE OF A SCREED ATTACHMENT CAPABLE OF PRODUCING A SMOOTH COMPACTED SURFACE. ADDITIONAL COMPACTING EFFORT BEHIND THE

PAVEMENT DETAILS, SEE TYPICAL SECTIONS.

SCREED IS NOT REQUIRED.

ILE: tehmac11.dgn	DN: Tx[	TO(	ck: RL	DW: KB	CK:	
©TxDOT January 2011	CONT	SECT	JOB		HIGHWAY	
REVISIONS	0106	04	036 l		US 380	
	DIST		COUNTY		SHEET NO.	
	ABL		STONEWA	70		

# (NOT TO SCALE)

#### UTILITY QUALITY LEVELS

(OBTAINED FROM ASCE PUBLICATION CI/ASCE STANDARD 38-02)

- UTILITY QUALITY LEVEL D (QL D): INFORMATION DERIVED FROM EXISTING RECORDS OR ORAL RECOLLECTIONS.
- UTILITY QUALITY LEVEL C (QL C): INFORMATION OBTAINED BY SURVEYING AND PLOTTING VISIBLE ABOVE-GROUND UTILITY FEATURES AND BY USING PROFESSIONAL JUDGEMENT IN CORRELATING THIS INFORMATION TO QUALITY LEVEL D INFORMATION.
- UTILITY QUALITY LEVEL B (QL B): INFORMATION OBTAINED THROUGH THE APPLICATION OF APPROPRIATE SURFACE GEOPHYSICAL METHODS TO DETERMINE THE EXISTENCE AND APPROXIMATE HORIZONTAL POSITION OF SUBSURFACE UTILITIES. QUALITY LEVEL B DATA SHOULD BE REPRODUCIBLE BY SURFACE GEOPHYSICS AT ANY POINT OF THEIR DEPICTION. THIS INFORMATION IS SURVEYED TO APPLICABLE TOLERANCES DEFINED BY THE PROJECT AND REDUCED ONTO PLAN DOCUMENTS.
- UTILITY QUALITY LEVEL A (QL A): PRECISE HORIZONTAL AND VERTICAL LOCATION OF UTILITIES OBTAINED BY THE ACTUAL EXPOSURE (OR VERIFICATION OF PREVIOUSLY EXPOSED AND SURVEYED UTILITIES) AND SUBSEQUENT MEASUREMENT OF SUBSURFACE UTILITIES, USUALLY AT A SPECIFIC POINT. MINIMALLY INTRUSIVE EXCAVATION EQUIPMENT IS TYPICALLY USED TO MINIMIZE THE POTENTIAL FOR UTILITY DAMAGE. A PRECISE HORIZONTAL AND VERTICAL LOCATION, AS WELL AS OTHER UTILITY ATTRIBUTES, IS SHOWN ON PLAN DOCUMENTS. ACCURACY IS TYPICALLY SET TO 15-MM VERTICAL AND TO APPLICABLE HORIZONTAL SURVEY AND MAPPING ACCURACY AS DEFINED OR EXPECTED BY THE PROJECT OWNER.

#### GENERAL NOTES

- THE UTILITIES DEPICTED WERE INVESTIGATED BY LAMB-STAR ENGINEERING, ALL OTHER PLAN INFORMATION, NOTABLY THE BACKGROUND INFORMATION, WERE PROVIDED BY OTHERS AND LAMB-STAR ENGINEERING DISCLAIMS RESPONSIBILITY FOR ITS
- UTILITY LOCATIONS ON THESE DRAWINGS ARE INTENDED FOR DESIGN PURPOSES AND NOT CONSTRUCTION. THEY REFLECT SUBSURFACE UTILITIES FROM RECORDS RECEIVED. CALL TEXAS 811 FOR UTILITY LOCATIONS 48-HOURS PRIOR TO ANY WORK.
- UTILITIES ON THESE DRAWINGS HAVE BEEN IDENTIFIED TO ASCE STANDARD 38-02. QUALITY LEVEL D INFORMATION IS SHOWN AS NOTED IN THE LEGEND.
- UTILITIES ON THESE DRAWINGS HAVE BEEN IDENTIFIED TO ASCE STANDARD 38-02. QUALITY LEVEL C INFORMATION IS SHOWN AS NOTED IN THE LEGEND.



REV. No.	DATE	REVISION	BY
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LAMB-STAR ENGINEERING, L.L.C. 5700 W. PLANO PARKWAY, SUITE 1000 PLANO, TEXAS 75093 (214) 440-3600 TEXAS REGISTERED ENGINEERING FIRM F-9073 TBPE REG. # F-9073

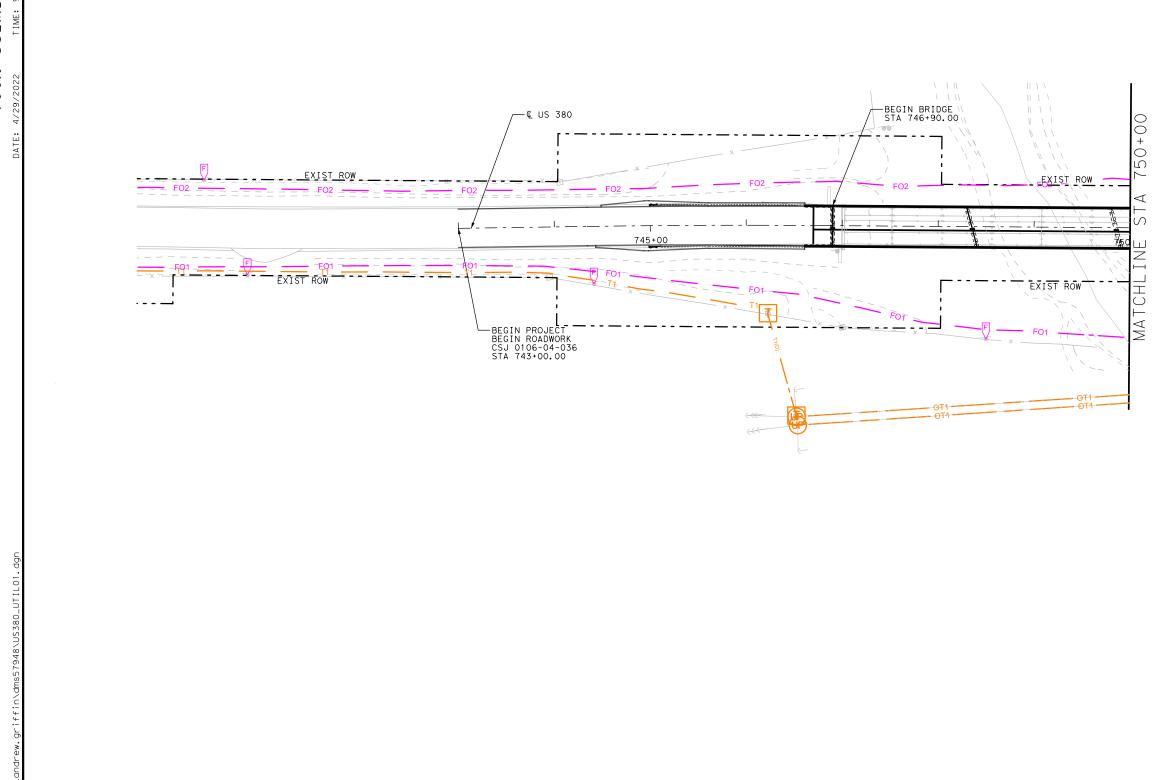
Texas Department of Transportation Abilene District

US 380

## EXISTING UTILITY NOTES

CUEET 4 OF 4

	SHEET 1 OF 1									
IGNED	:JVH	FED. RD DIV. No.	STATE	PROJECT No.				SHWAY No.		
CKED:	ASG	6	TEXAS	SEE TITLE SHEET			U	S 380		
wn:	ASG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.	JOB No.		SHEET No.		
CKED:	JVH	ABL S	TONEWAL	L 0106	04	0	36	71		



# LEGEND

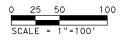
- FO1(D) — FO2 — - FO2(D)

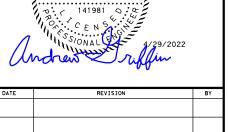
FIBER - CAPROCK (QLB) FIBER - CAPROCK (QLD) FIBER - CENTURYLINK (QLB)
FIBER - CENTURYLINK (QLD) T1— TELEPHONE - CAPROCK (QLB)
T1(D) - TELEPHONE - CAPROCK (QLD)
OT1 — OT1 — OH TELEPHONE - CAPROCK (QLC)

TELEPHONE PEDESTAL

TELEPHONE UTILITY POLE

UNDERGROUND FIBER MARKER







LAMB-STAR ENGINEERING, L.L.C.
5700 W. PLANO PARKWAY, SUITE 1000
PLANO, TEXAS 75093 (214) 440-3600
TEXAS REGISTERED ENGINEERING FIRM F-9073
TBPE REG. # F-9073



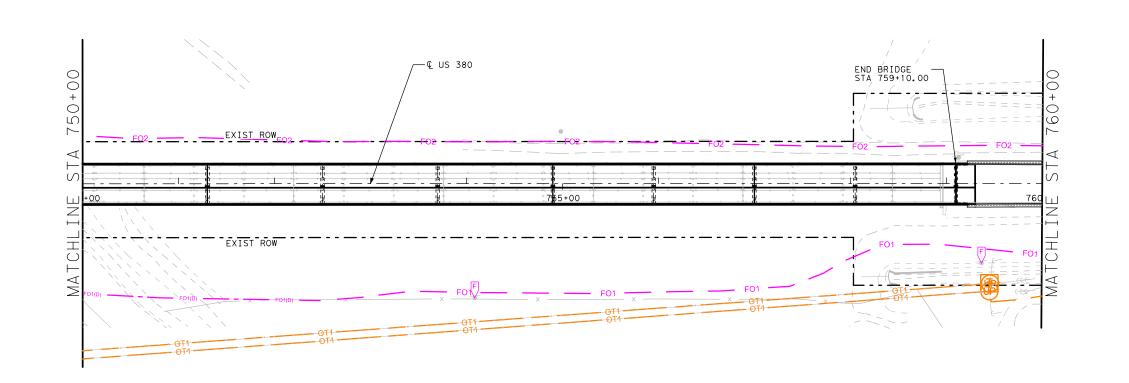
US 380

UTILITY LAYOUT BEGIN TO STA 750+00

SCALE: 1"=100'

SHEET 1 OF

SCALE		-100				эп	EEI	I OF	J
SIGNED	:JVH	FED. RD DIV. No.	STATE	PROJE	ECT No.		ніс	HWAY No.	
ECKED:	ASG	6	TEXAS	SEE TIT	LE SHE	ΕT	U	S 380	
AWN:	ASG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB o.	SHEET No.	
ECKED:	JVH	ABL S	TONEWAI	L 0106	04	0	36	72	



# LEGEND

- FO1(D) -

FIBER - CAPROCK (QLB) FIBER - CAPROCK (QLD) FO1(D) - FIBER - CAPROCK (QLD)

FO2 — FIBER - CENTURYLINK (QLB)

FO2(D) - FIBER - CENTURYLINK (QLD)

T1 — TELEPHONE - CAPROCK (QLB)

T1(D) - TELEPHONE - CAPROCK (QLD)

OT1 — OH TELEPHONE - CAPROCK (QLC)

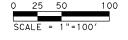


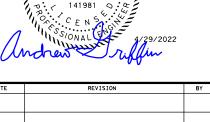
TELEPHONE PEDESTAL



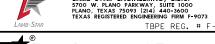
TELEPHONE UTILITY POLE

UNDERGROUND FIBER MARKER











US 380

UTILITY LAYOUT STA 750+00 TO STA 760+00

SCALE: 1"=100' SHEET									
DESIGNED	:JVH	FED. RD DIV. No.	STATE		PROJI	ECT No.		ніс	SHWAY No.
CHECKED:	ASG	6	TEXAS		SEE TIT	LE SHE	ΕT	L	IS 380
DRAWN:	ASG	STATE DISTRICT	COUN.	ГҮ	CONTROL No.	SECTION No.		OB Io.	SHEET No.
CHECKED:	JVH	ΔRI '	TONE	/Δ1	0106	0.4	0	36	73

# LEGEND

- FO1(D) -

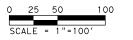
— FO1 — FIBER - CAPROCK (QLB) FIBER - CAPROCK (QLD)

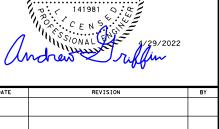
- FO1(D) - FIBER - CAPROCK (QLD)
- FO2 - FIBER - CENTURYLINK (QLB)
- FO2(D) - FIBER - CENTURYLINK (QLD)
- T1 - TELEPHONE - CAPROCK (QLB)
- T1(D) - TELEPHONE - CAPROCK (QLD)
- OT1 - OH TELEPHONE - CAPROCK (QLC)

TELEPHONE PEDESTAL

TELEPHONE UTILITY POLE

UNDERGROUND FIBER MARKER





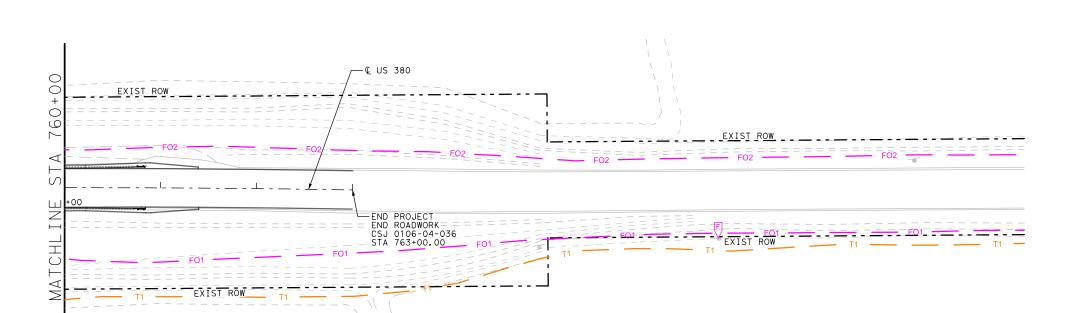




US 380

UTILITY LAYOUT STA 760+00 TO END

CALE	: 1	"=100′				SH	EET	3 OF	3
SIGNED	:JVH	FED. RD DIV. No.	STATE	PROJI	ECT No.		ніс	SHWAY No	٠.
ECKED:	ASG	6	TEXAS	SEE TITLE SHEET			U	IS 380	
AWN:	ASG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEE No	
ECKED.			TONEWAL	0106	04	0	36	7.4	



SUBMITTAL CURRY COUNTY US 60 -PARMER COUNT SWISHER COUNTY COUNTY 00% Al=Area of watershed 1 A2=Area of watershed 2 COUNTY 500001 100000 SILVERTON LLANO ESTACADO TX 194-PLAINVIEW FLOYD ROOSEVEL COUNTY BAILEY COUNTY LAMB MOTLEY COUNTY NOTES: COUNT WHITE RIVER COUNTY STREAM GAUGE 08082000 US 380 @ SALT FORK BRAZOS RIVER DRAINAGE AREA = 4597 SQ MI BRIDGE LOCATION LUBBOCK COUNTY HOCKLEY RESERVOIR COUNTY CROSBY COUNTY KING COUNTY CAPROCK TERRY KENT COUNTY LEGEND US 380 BRIDGE LOCATION GARZA COUNTY — DRAINAGE AREA 1 - PROJECT SITE HASKELL COUNTY KÉNT ■ DRAINAGE AREA 2 - STREAM GAUGE 08082000 COUNTY STONE WALL - - - DRAINAGE AREA - WHITE RIVER RESERVOIR FLOW DIRECTION JONES COUNTY FISHER

50000' 100000

SCURRY

From TxDOT Hydraulic Design Manual, Equation 4-10

Q1=Estimated AEP discharge at ungauged watershed 1 Q2=Known AEP discharge at gauged watershed 2

% Chance Exceedance	Storm Frequency	Q1	Q2	A 1	A2
	i requericy	CFS	CFS	sq. mi.	sq. mi.
50	2	20500	21600	4597	5130
20	5	27700	29200	4597	5130
10	10	32500	34300	4597	5130
4	25	37300	39300	4597	5130
2	50 (DESIGN)	43400	45800	4597	5130
1	100 (CHECK)	48100	50800	4597	5130
0.5	200 (CHECK)	53000	55900	4597	5130
0.2	500	59500	62800	4597	5130

- PEAK FLOWS WERE COMPUTED USING STATISTICAL ANALYSIS OF STREAM GAUGE DATA (USGS STREAM GAUGE 08082000, "SALT FK BRAZOS RV NR ASPERMONT, TX) BASED ON BULLETIN 17C PROCEDURES USING HEC-SSP VERSION 2.2. WITH THE CONSTRUCTION OF THE WHITE RIVER RESERVOIR IN 1963, MUCH OF THIS WATERSHED BECAME REGULATED AND UNUSABLE IN THE STATISTICAL ANALYSIS. THEREFORE, ONLY RECORDED AND HISTORICAL PEAK Q DATA FROM 1939 TO 1962 WAS UTILIZED IN THE HYDROLOGIC ANALYSIS. A FULL POOL LAKE LEVEL CONDITION IS ASSUMED.
- FEMA HAS NOT COMPLETED A STUDY TO DETERMINE FLOOD HAZARD FOR THE BRIDGE LOCATION; THEREFORE, A FLOOD MAP WAS NOT PUBLISHED AT THE TIME OF THIS ANALYSIS.
- THE PROJECT LOCATION LIES IN AN AREA NOT MAPPED BY FEMA. INFORMAL COORDINATION WAS COMPLETED ON 06/09/2021 WITH THE HONORABLE RONNIE MOORHEAD, STONEWALL COUNTY.



REV. No.	DATE	REVISION	BY
			1



LAMB-STAR ENGINEERING, L.L.C. 5700 W. PLANO PARKWAY, SUITE 1000 PLANO, TEXAS 75093 (214) 440-3600 TEXAS REGISTERED ENGINEERING FIRM F-9073

Texas Department of Transportation Abilene District

US 380

BRIDGE HYDROLOGIC DATA

SALT FORK BRAZOS RIVER BRIDGE

SHEET 1 OF

IGNED: JVH FE	D. RD V. No.	STATE	PROJE	HIGHWAY No.			
KED: ASG	6 T	EXAS	SEE TIT	LE SHEE	US 380		
	STATE STRICT	COUNTY	CONTROL No.	SECTION No.		0B 0.	SHEET No.
CKED: JVH	ABL S	TONEWAL	_ 0106	04	0	36	75

SUBMITTAL 100%

> 1750-1745-1740-1735-

> > 1730-

1725 0

----1127

1000 FEET

Brazos River Salt Fork

Salt Fork Plan: 1) EXIST 12/14/2021 2) PROP 12/15/2021

-WSEL PR 100-YR

-WSEL EX 100-YR

WSEL PR 50-YR

-WSEL EX 50-YR

Main Channel Distance (ft)

CRITICAL HABITAT \_\_EXIST ROW

-SALT FORK BRAZOS RIVER

-HEC-RAS MODEL CROSS SECTION

US 380-



Legend WS 1% AEP - EXIST WS 1% AEP - PROP WS 2% AEP - PROP WS 2% AEP - EXIST

Ground

- HYDRAULIC MODELING WAS PERFORMED USING HEC-RAS VERSION 5.0.7 WITH STEADY FLOW ANALYSIS.
- THE PROJECT LOCATION LIES IN AN AREA NOT MAPPED BY FEMA. INFORMAL COORDINATION WAS COMPLETED ON 06/09/2021 WITH THE HONORABLE RONNIE MOORHEAD, STONEWALL COUNTY.



REV. No.	DATE	REVISION	BY
	_		



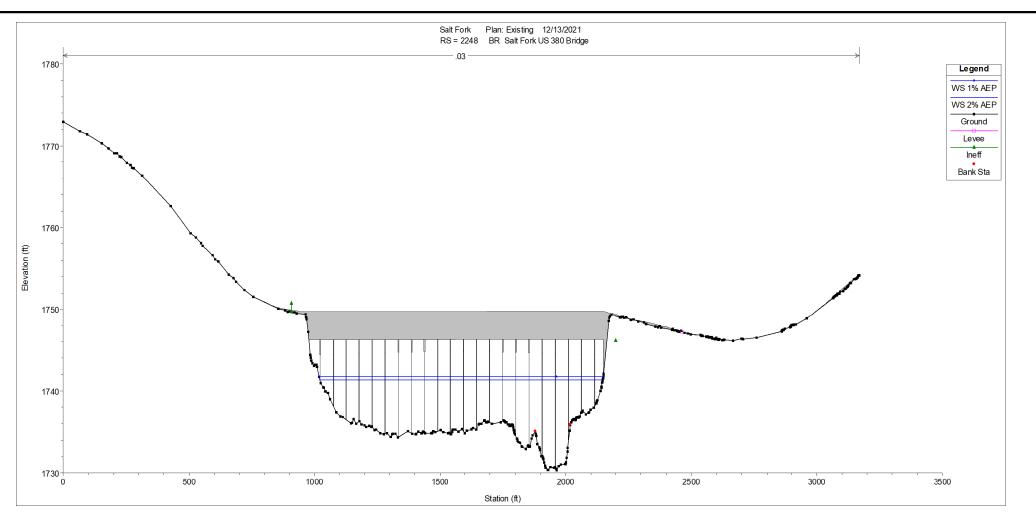
Texas Department of Transportation

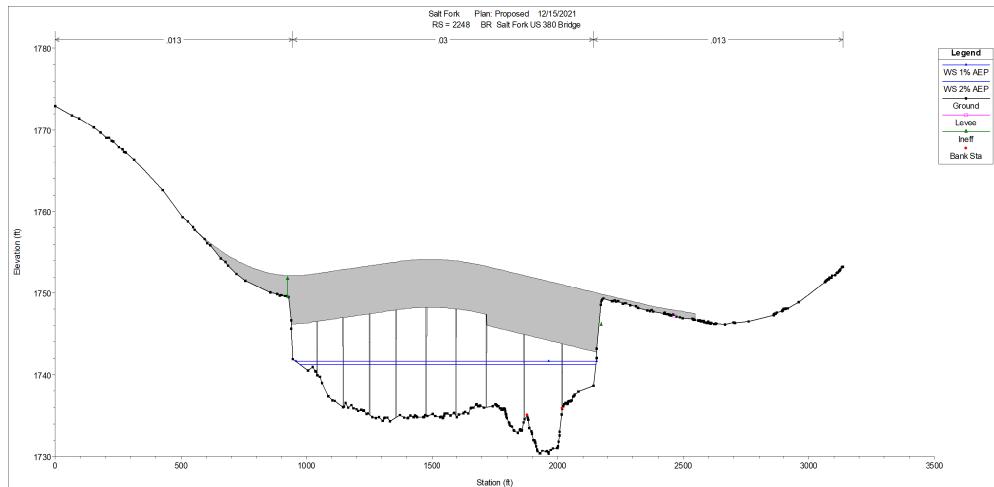
Abilene District

US 380 BRIDGE HYDRAULIC DATA

DESIGNED:	:JVH	FED. RD DIV. No.	STATE		PROJE	ECT No.		ніс	SHWAY No.
CHECKED:	ASG	6	TEXAS		SEE TIT	LE SHE	Ξ	U	S 380
DRAWN:	ASG	STATE DISTRICT	COUNT	Υ	CONTROL No.	SECTION No.		0B o.	SHEET No.
CHECKED:	JVH	ABL	STONEW	ΑL	L 0106	04	0	36	76

SUBMITTAL 100%





- 1. THE PROJECT LOCATION LIES IN AN AREA NOT MAPPED BY FEMA. INFORMAL COORDINATION WAS COMPLETED ON 06/09/2021 WITH THE HONORABLE RONNIE MOORHEAD, STONEWALL COUNTY.

  2. BOTH EXISTING AND PROPOSED ELEVATIONS ON THIS SHEET FACE DOWNSTREAM (NORTH).



REV. No.	DATE	REVISION	BY





US 380

BRIDGE HYDRAULIC DATA

SHEET	2	OF	

GNED:	JVH	FED. RD DIV. No.	STATE	PROJECT No.					HIGHWAY No.		
KED:	ASG	6	TEXAS	SEE TITLE SHEET					US 380		
N:	ASG	STATE DISTRICT	COUNT	Y CONTROL No.		SECTION No.	JOB No.		SHEET No.		
KED:	JVH	ABL	TONE W.	A L L	. 0106	04	036		77		

Reach

Salt Fork

4437

4437

4437

3827

3827

3827

3827

3252

3252

3252

3252

2882

2882

2882

2882

2514

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2% AFP

1% AEP

1% AFP

2% AEP

1% AEP

(f+)

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1731 91

1731.91

1731.91

1731.62

1731.62

1731.62

1730.93

1730.93

1730.93

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1730.01

1729.69

1729.69

1729.69

1729.69

1729.40

1729.40

1729.32

1729.32

1729.32

1728.94

1728, 94

1728.94

1728.16

(f+)

1743.23

1743.20

1743.66

1742.85

1743.31

1743.28

1742.63

1743.05

1742.59

1743.01

1742.31

1742.27

1742.68

1741.93

1741.89

1742.33

1742.27

1741.38

1741.31

1741.74

1741.67

1740.88

1740.88

1741.21

1741.2

1740.71

1740.71

1741.04

1741.04

1740.15

1740.47

1739.61

1739.61

1739.93

1738.84

1739, 15

1739.15

1737.13

1729.40 | 1740.15

1729.40 1740.47

1729.32 1739.93

1728.94 1738.84

1728.16 1736.93

1728, 16 1736, 93

1728, 16 1737, 13

1730.67 1742.73

1743.63

1731.62 | 1742.88

(f+)

1740.06

1740.23

1740.23

1739.43

1739.43

1739.60

1739.60

1739.16

1739.16

739.35

1739.35

1738.84

1738.84

739.06

738.65

1738.65

1738.90

1738.90

1739.19

1739.19

1739.45

1739.35

1739.35

1739.61

1739.61

1738.57

1738.57

1738.85

1738.85

1738.07

1738.07

1738.31

1738.31

1737, 76

1737.97

1737.97

737.43

1737.43

1737.67

1736.93

1736, 93

1737.13

1737.13

739.45

1739.06

1740 06

Plan Q Total Min Ch El W.S. Elev Crit W.S. E.G. Elev E.G. Slope Vel Chnl Flow Area Top Width Froude # Chl

(f+/f+)

0.000424

0 000430

0.000413

0.000419

0.000495

0.000502

0.000492

0.000500

0.000591

0.000601

0.000593

0.000604

0.000728

0.000742

0.000735

0.000751

0.000833

0.000852

0.000856

0.000878

0.001679

0.001738

0.001715

0.001782

0.002133

0.002133

0.002196

0.002196

0.001378

0.001378

0.001431

0.001431

0.001541

0.001541

0.001575

0.001575

0.001436

0.001436

0.001454

0.001454

0.001918

0.001918

0.001956

0.001956

0.004802

0.004802

0.005007

0.005007

(ft/s)

4.22

4.24

4.30

4.32

4.64

4.66

4.77

4.80

5.04

5.07

5.20

5.23

5.20

5.24

5.37

5.41

6.25

6.30

6.48

6.54

8.31

8.41

8.61

8.73

9.70

9.70

10.06

10.06

8.06

8.06

8.41

8.41

7.82

7.82

8.12

8.12

7.43

7.43

7.68

7.68

8.68

8.68

8.99

12.31

12.31

12.81

12.81

(sa ft)

13004.42

12948, 28

3980.59

13912.5

11382.21

12148.2

12084.69

10322.29

10267.8

10998.53

10934.31

9396.9

9340.4

9983.58

9917.37

8370.17

8311.93

8851.06

8782.81

6605.89

6532.55

7013.58

6927.05

5955.38

5955.38

6312.06

6312.06

6831.67

6831.67

7212.76

7212.76

6986.39

6986.39

7439.72

7439.72

7494.75

7494, 75

7983.05

7983.05

6843.32

6843.32

7292, 12

7292.12

5048.27

5048, 27

5340.24

5340.24

1329.04

(f+)

2265.93

2265 84

2267.32

2267.23

1792.55

1792.44

1794.21

1794.08

1600.03

1599.72

1603.99

1603.62

1419.98

1419.61

1423.83

1423.40

1217.42

1216.97

1221.13

1220.60

1111.08

1109.96

1116.95

1115.71

1067.27

1067.27

1073.35

1073.35

1146.17

1146.17

1150.21

1150.21

1389.96

1389.96

1407.79

1407.79

1516.94

1516.94

1528.32

1528.32

1474.74

1474.74

1518.55

1518.55

1463.22

1463, 22

1472.57

1472.57

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0.54

0.54

0.55

0.55

0.83

0.83

0.85

0.85

(f+)

1743.41

1743 39

1743.85

1743.83

1743.12

1743.09

1743.57

1743.53

1742.91

1742.88

1743.36

1743.32

1742.66

1742.62

1743.10

1743.06

1742.37

1742.33

1742.81

1742.76

1742.11

1742.06

1742.54

1742.48

1741.82

1741.82

1742.24

1742.24

1741.41

1741.41

1741.81

1741.81

1740.83

1740.83

1741.21

1741.21

1740.20

1740,20

1740.56

1740.56

1739.57

1739.57

1739.92

1739.92

1738.39

1738.39

1738.71

1738.71

Salt Fork Salt Fork Salt Fork Salt Fork Salt Fork Salt Fork Salt Forl Salt Fork Salt Fork Salt Fork Existing Conditions Hydraulic Results

2% AEP Event E.G. US. (ft) W.S. US. (ft) 1741.38 Elev (ft) 1741.36 1739.11 Total (cfs) W.S. Elev (ft) Q Bridge (cfs) 43400 Crit W.S. (ft) 11.03 6.58 6592.44 Max Chi Dpth (ft) Vel Total (ft/s) Weir Sta Lft (ft Weir Sta Rgt (ft) low Area (sq ft) Weir Submerg Froude # Chl Specif Force (cu ft) 0.45 31148.79 Weir Max Depth (ft) Min El Weir Flow (ft) Hydr Depth (ft)
W.P. Total (ft) Min El Prs (ft) 1746.28 972089.7 1097.05 Delta EG (ft) 0.28 Conv. Total (cfs) 942356.6 Delta WS (ft) Top Width (ft) 1095.18 BR Open Area (sq ft)
BR Open Vel (ft/s) 12064.93 C & E Loss (ft) 6.73 Sluice Coef Shear Total (lb/sq ft) Energy only Power Total (lb/ft s)

		1% AEP Event		
E.G. US. (f+)	1742.54	Element	Inside BR US	Inside BR DS
W.S. US. (f+)	1741.74	E.G. Elev (ft)	1742.49	1742.39
Q Total (cfs)	48100	W.S. Elev (ft)	1741.73	1741.59
Q Bridge (cfs)	48100	Crit W.S. (ft)	1739.38	1739.38
Q Weir (cfs)		Max Chl Dpth (ft)	11.4	11.26
Weir Sta Lft (ft)		Vel Total (ft/s)	6.88	7.02
Weir Sta Rgt (ft)		Flow Area (sq ft)	6995.18	6847
Weir Submerg		Froude # Chi	0.37	0.38
Weir Max Depth (ft)		Specif Force (cu ft)	35039.11	34338.42
Min El Weir Flow (ft)	1748.01	Hydr Depth (ft)	6.35	6.22
Min El Prs (ft)	1746.28	W.P. Total (ft)	1380.84	1373.77
Delta EG (ft)	0.3	Conv. Total (cfs)	1058869	1026635
Delta WS (ft)	0.53	Top Width (ft)	1101.01	1099.93
BR Open Area (sq ft)	12064.93	Frctn Loss (ft)	0.09	0.08
BR Open Vel (ft/s)	7.02	C & E Loss (ft)	0.01	0.07
BR Sluice Coef		Shear Total (lb/sq ft)	0.65	0.68
BR Sel Method	Energy only	Power Total (lb/ft s)	4.49	4.8

Proposed Conditions Hydraulic Results

		2% AEP Event		
E.G. US. (f+)	1742.06	Element	Inside BR US	Inside BR DS
W.S. US. (f+)	1741.31	E.G. Elev (ft)	1742.03	1741.95
Q Total (cfs)	43400	W.S. Elev (ft)	1741.28	1741.19
Q Bridge (cfs)	43400	Crit W.S. (ft)	1739.25	1739.23
Q Weir (cfs)		Max Chl Dpth (ft)	10.95	10.86
Weir Sta Lft (ft)		Vel Total (ft/s)	6.62	6.75
Weir Sta Rgt (ft)		Flow Area (sq ft)	6552.88	6434.27
Weir Submerg		Froude # Chl	0.37	0.37
Weir Max Depth (ft)		Specif Force (cu ft)	30937.7	30349.85
Min El Weir Flow (ft)	1748.01	Hydr Depth (ft)	5.69	5.61
Min El Prs (ft)	1748.21	W.P. Total (ft)	1253.38	1255.32
Delta EG (ft)	0.24	Conv. Total (cfs)	1059255	1015641
Delta WS (ft)	0.43	Top Width (ft)	1151.11	1146.63
BR Open Area (sq ft)	12511	Frctn Loss (ft)	0.07	0.07
BR Open Vel (ft/s)	6.75	C & E Loss (ft)	0	0.05
BR Sluice Coef		Shear Total (lb/sq ft)	0.55	0.58
BR Sel Method	Energy only	Power Total (lb/ft s)	3.63	3.94

		1% AEP Event		
E.G. US. (ft)	1742.48	Element	Inside BR US	Inside BR DS
W.S. US. (ft)	1741.67	E.G. Elev (ft)	1742.45	1742.37
Q Total (cfs)	48100	W.S. Elev (ft)	1741.64	1741.55
Q Bridge (cfs)	48100	Crit W.S. (ft)	1739.52	1739.49
Q Weir (cfs)		Max Chl Dpth (ft)	11.31	11.22
Weir Sta Lft (ft)		Vel Total (ft/s)	6.9	7.02
Weir Sta Rgt (ft)		Flow Area (sq ft)	6971.16	6849.35
Weir Submerg		Froude # Chl	0.38	0.38
Weir Max Depth (ft)		Specif Force (cu ft)	34778.16	34144.76
Min El Weir Flow (ft)	1748.01	Hydr Depth (ft)	5.97	5.89
Min El Prs (ft)	1748.21	W.P. Total (ft)	1277.96	1279.31
Delta EG (ft)	0.24	Conv. Total (cfs)	1158531	1112418
Delta WS (ft)	0.45	Top Width (ft)	1167.15	1163.1
BR Open Area (sq ft)	12511	Frctn Loss (ft)	0.08	0.08
BR Open Vel (ft/s)	7.02	C & E Loss (ft)	0	0.06
BR Sluice Coef		Shear Total (lb/sq ft)	0.59	0.62
BR Sel Method	Energy only	Power Total (lb/ft s)	4.05	4.39

1. THE PROJECT LOCATION LIES IN AN AREA NOT MAPPED BY FEMA. INFORMAL COORDINATION WAS COMPLETED ON 06/09/2021 WITH THE HONORABLE RONNIE MOORHEAD, STONEWALL COUNTY.



REV. No. DATE	REVISION	BY



LAMB-STAR ENGINEERING, L.L.C. 5700 W. PLANO PARKWAY, SUITE 1000 PLANO, TEXAS 75093 (214) 440-3600 TEXAS REGISTERED ENGINEERING FIRM F-9073 TRPF REG. # F-907



US 380

BRIDGE HYDRAULIC DATA

	SH	EE1 3 OF 3
STATE	PROJECT No.	HIGHWAY No.
TEXAS	SEE TITLE SHEET	US 380

CHECKED: ASG 6 TEXAS SEE TITLE SHEET US 380  CRAWN: ASG STATE OCUNTY CONTROL SECTION NO. NO. NO. NO. CHECKED: JVH ABL STONEWAL 0106 04 036 78	DESIGNED: J	٧Н	FED. RD DIV. No.	STATE		PROJE	ECT No.		HIGHWAY No.			
DRAWN: ASG DISTRICT COUNTY No. No. No. No.	CHECKED: A	SG	6	TEXAS		SEE TIT	LE SHE	ΞT	U	S 380		
CHECKED: JVH ABL STONEWAL 0106 04 036 78	DRAWN: A	SG	STATE DISTRICT	COUNT	ГҮ							
	CHECKED: J	۷Н	ABL :	TONEW	/AL	L 0106	04	036		78		

# <u>US380 Contraction Scour Data</u>

		_					
Con	traction	Scour					
Innut Darameter	Units	West 0	verbank	Main C	hanne I	East 0	/erbank
Input Parameter	UIIITS	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr
Average Depth Upstream of Contraction	f†	5.99	6.33	9.94	10.31	3.89	4.17
D50	f†			0.00	1083		
Average Velocity Upstream	ft/s	6.84	7.15	8.73	9.01	3.23	3.41
Temperature of Water	° F				0		
Slope of Energy Grade Line at Approach Section	ft/ft	0.001782	0.001810	0.001782	0.001810	0.001782	0.001810
Flow in Contracted Section	cfs	32798.45	36360.13	48100.00	53000.00	2577.68	3028.98
Flow Upstream that is Transporting Sediment	cfs	35856.84	39762.18	48100.00	53000.00	1464.40	1695.48
Width in Contracted Section	f†	896.05	907.44	1167.15	1180.02	133.80	135.29
Width Upstream that is Transporting Sediment	f+	874.64	878.00	1115.71	1121.49	116.86	119.29
Depth Prior to Scour in Contracted Section	f†	7.29	7.66	11.31	11.68	5.72	6.09
Unit Weight of Water	lb/ft^3			62	. 4		
Unit Weight of Sediment	lb/ft^3			1 1	65		
	•	•					
	Result	s					
Recommended Scour Depth	f+	-1.83	-1.93	-1.67	-1.73	0.03	0.20
Critical velocity above which bed material of size D and smaller will be transported	ft/s	1.55	1.56	1.68	1.69	1.44	1.46
Equation	N/A			Live	Bed		

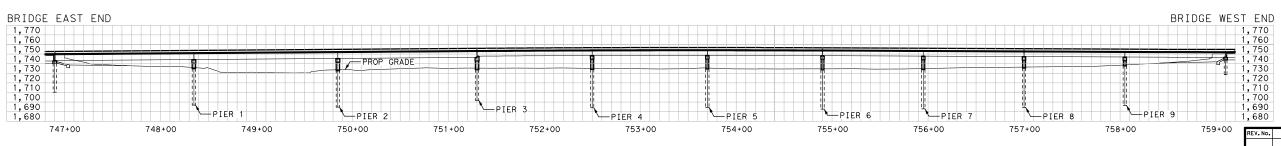
Riprap Desig	gn Dato	]						
Input Parameter	Units	West At	outment	East At	outment			
Input Furdilleter	011115	100-yr	200-yr	100-yr	200-yr			
Average Water Depth in Channel	f†	5.67	5.97	3.81	4.13			
Average Channel Veolocity	ft/s	6.45	6.71	5.06	5.42			
Abutment Sideslope	N/A	2	: 1	4:	: 1			
Angle of Sideslope to Horizontal	*	26	. 6	14.	. 04			
Factor of Safety	N/A		1.	. 5				
Gravitational Acceleration	ft/s²		32	2.2				
Riprap Specific Gravity	N/A	2.65						
Stability Coefficient, Cs	N/A		0.	. 3				
Vertical Velocity Distribution Coefficient, Cv	N/A	1						
Sideslope Correction Factor, K	N/A	1						
D30	f†	0.26	0.28	0.13	0.15			
Result	s							
Calculated D50	in	3.7	4.0	1.9	2.2			
Recommended D50	in	12	12	12	12			
Riprap Thickness	in	18	18	18	18			
Abutment Toe Width	f†	3	3	3	3			
Abutment Toe Depth	f†	3	3	3	3			

- NOTES:
  1. Scour calculations performed using
  Hydraulic Toolbox version 5.0
- based on HEC-18 (FHWA, 2012).

  2. Refer to Drainage Report for information on scour envelope.

### US380 Pier Scour Data

						Pier S	cour (Pier	s Numbere	d from Eas	t to West)									
Input Parameter	Units	Pi∈	Pier 1 Pier 2 Pier 3 Pier 4 Pier 5 Pier 6 Pier 7 Pier 8 Pier 9										∍r 9						
Triput Parameter	UIIIIS	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr	100-yr	200-yr
Computation Method	N/A									HEC	-18								
Pier Shape	N/A									Group of	Cylinders								
Bed Condition	N/A									Clear-Wa	ter Scour								
Depth Upstream of Pier	f†	5.69	6.06	7.94	8.31	6.24	6.61	6.62	6.99	6.17	6.54	7.07	7.44	6.75	7.12	5.14	5.54	2.99	3.36
Velocity Upstream of Pier	ft/s	9.21	9.51	6.45	6.71	6.45	6.71	6.45	6.71	6.45	6.71	6.45	6.71	6.45	6.71	6.45	6.71	6.45	6.71
Width of Column	f†	3.	. 5	3	. 5		3	,	3	,	3		3		3		3		3
Angle of Attack	Degrees									1	5								
Spacing between Columns of Piers	f†	11.	. 67	11	. 67	10.	. 67	10.	. 67	10.	. 67	10	. 67	10	. 67	10	. 67	10	. 67
Number of Columns	N/A										4								
Angle of Repose	Degrees									4	4								
Resul†s																			
Pier Scour Depth	f†	17.76	18.04	14.27	14.52	14.61	14.85	14.31	14.55	14.67	14.92	13.94	14.18	14.20	14.44	15.44	15.69	16.96	17.25
Total Scour at Pier	f†	17.76	18.04	14.27	14.52	14.61	14.85	14.31	14.55	14.67	14.92	13.94	14.18	14.20	14.44	15.44	15.69	16.96	17.25
Froude Number Upstream	N/A	0.68	0.68	0.40	0.41	0.46	0.46	0.44	0.45	0.46	0.46	0.43	0.43	0.44	0.44	0.50	0.50	0.66	0.65





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REV. No.	DATE	REVISION	BY
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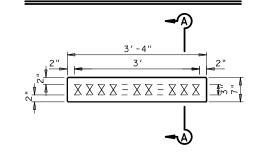
Texas Department of Transportation Abilene District

> US 380 SCOUR ANALYSIS

SHEE	T 1	ΩF

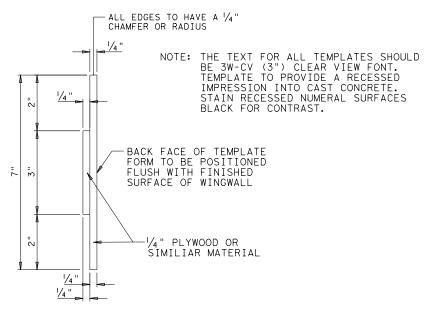
IGNED	:JVH	FED. RD DIV. No.	STATE	PROJECT No.			HIGHWAY No.			
CKED:	ASG	6	TEXAS		SEE TITLE SHEET			US 380		
WN:	ASG	STATE DISTRICT	COUNT	Υ	CONTROL No.	SECTION No.		OB 0.	SHEET No.	
CKED:	JVH	ABL S	TONEW	ΑL	L 0106	04	0	36	79	

#### STRUCTURE ID TEMPLATES



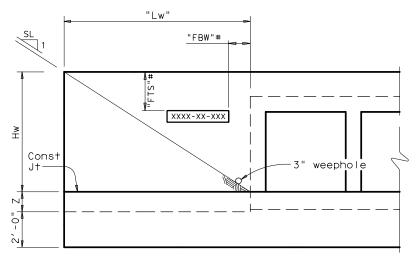
NOTE: THE SYMBOLS XXXX-XX-XXX REPRESENT THE STRUCTURE NUMBER WHICH IS SHOWN IN THE TABLE TO THE RIGHT.

ALL CHARACTERS ARE REQUIRED, AND ARE TO BE FORMATTED EXACTLY AS SHOWN IN THE STRUCTURE NUMBER COLUMN TO THE RIGHT.

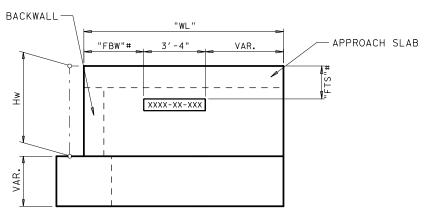


	STRUCTURE ID TEMPLATE NUMBERS										
NBI NUMBER	LOCATION	STRUCTURE NUMBER	"WL"	"Lw"	"Hw"	"FBW"#	"FTS"#				
08-217-0-0106-04-057	SALT FORK OF BRAZOS RIVER BRIDGE	CSJ 0106-04-036	ABUT 1: 29' ABUT 11: 12'	NA	ABUT 1: 7'-2¾" ABUT 11: 5'-11"	VARIOUS	VARIOUS				

STRUCTURE ID

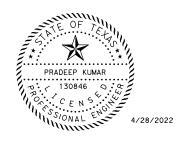


PARALLEL WING ELEVATION

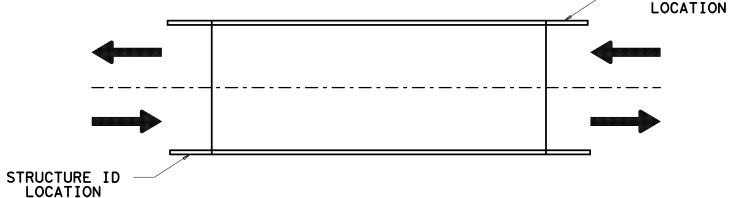


#### WINGWALL ELEVATION

# FIELD LOCATE TO AVOID CONFLICT WITH REINFORCEMENT AND RIPRAP. THE ENGINEER SHALL APPROVE INSTALLATION LOCATION PRIOR TO PLACEMENT.



SECTION A-A



THE STRUCTURE ID'S ARE USUALLY PLACED ON THE RIGHT HAND SIDE OF APPROACHES. THIS PLACES THE ID'S ON DIAGONAL CORNERS. THE STRUCTURE ID'S WILL NOT BE PAID FOR DIRECTLY, BUT WILL BE CONSIDERED SUBSIDIARY TO THE VARIOUS BRIDGE ITEMS.

HL93 LOADING REV. No. DATE REVISION

Texas Department of Transportation Abilene District

US 380 STRUCTURE ID DETAILS SIDD-14

GNED	:TS	DIV. No.	STATE	PROJECT No.				HIGHWAY No.		
KED:	SK	6	TEXAS	SEE TIT	LE SHE	ΕT	U	S 380		
N:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		0B o.	SHEET No.		
KED:	TS	ABL	STONEWAL	L 0106	04	0	36	80		

DESIGN SPEED: 50 MPH ADT (2020): 697 VPD EX ROW ADT (2040): 976 VPD BORE HOLE: B-6 FUNCTION CLASS: RURAL ARTERIAL STA: 746+91.54 EXISTING NBI: 08-217-0106-04-043 PROPOSED NBI: 08-217-0106-04-057 CRITICAL HABITAT LIMITS OF OFFSET: 66.50' LT CRITICAL HABITAT ELEV: 1746.22 BEGIN BRIDGE **GENERAL NOTES:** FACE OF BACKWALL ₹ EXISTING FIBER 1. DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATION, 8TH EDITION (2017), AS MODIFIED ABUTMENT NO.1 CENTURYLINK (QLB) SUBMITT STA 746+90.00 LIMITS OF RIPRAP
(STONE PROTECTION) 12 IN
THICKNESS = 18 IN (TYP) EL = 1749.99' BY 2020 TXDOT BDM. EX ROW EXISTING BRIDGE. \_\_ (SEE RDWŶ 2. FOR VERTICAL PROFILE AND HORIZONTAL ALIGNMENT SEE ROADWAY PLAN AND PROFILE SHEETS. 0 TO BE REMOVED **Q** US 380 & PGL PLANS) (TYP) 4 € BENT 3 € BENT NO. 2 50+ 3. ALL DIMENSIONS ARE EITHER HORIZONTAL OR VERTICAL AND MUST BE CORRECTED FOR GRADE, STA 749+85.00 STA 748+35.00 CROWN AND/OR SUPERELEVATION. 00% ~ 4. THE CONTRACTOR IS RESPONSIBLE FOR FIELD VERIFYING THE LOCATION OF ALL UTILITIES AND NOM FACE (1) -@ GIRDER #1 ⋖ CSAB (MOD) EXISTING STRUCTURES PRIOR TO ORDERING OF T223 RAIL N80° 35′ 02 "W MATERIALS. NOTIFY ENGINEERING IN WRITING S OF ANY DISCREPANCIES OR CONFLICTS. 748+00-749+00-750+00 -CURVE 380CL\_1 -€ GIRDER #6 INE SEJ-M (4") 15° 00′ 00" 5. THE H VALUES SHOWN ARE ESTIMATED COLUMN HEIGHTS. THE CONTRACTOR IS RESPONSIBLE FOR CALCULATING THE ACTUAL COLUMN HEIGHTS 7′-0" BASED ON FIELD CONDITIONS. \_ آب SEJ-M (4") 20'-0 NOM 6. THE EXISTING 23 SPAN BRIDGE IS TO BE REMOVED BY THE CONTRACTOR. STEEL I-BEAMS AND MATCH BAS-A BORE HOLE: B-5 <u>2</u>1 15° 00′ 00" STA: 748+20.88 PRESTRESSED CONCRETE BEAMS SUPPORTED ON OFFSET: 52.57' RT \ ELEV: 1737.70 36'-7 -PHASE CAST-IN-PLACE SUBSTRUCTURE ON PRECAST CONCRETE CONSTRUCTION PILES FOUNDATION. THIS BRIDGE IS TO BE REMOVED AND DISPOSED OF IN ACCORDANCE WITH STANDARD JOINT SPECIFICATION ITEM 496. CONTRACTOR IS REQUIRED -EXISTING FIBER CURVE 380CL\_ TO SUBMIT SIGN AND SEALED DEMO PLAN TO THE ENGINEER FOR APPROVAL. EX ROW HORIZONTAL ALIGNMENT DATA CAPROCK (QLB) PI STATION = 745+56.96
DELTA = 1° 15′ 56.11″ (RT)
DEGREE OF CURVE = 0° 30′ 58.24″
TANGENT = 122.5969′
LENGTH = 245.1839′
RADIUS = 11,100′ 1) PLACE WATER BARRIER PER 1223 STANDARD. LEFT: STA 747+57 TO STA 750+60 RIGHT: STA 747+74 TO 751+04 7. "D" DENOTES DOWEL IN OUTSIDE GIRDER. 8. SEE TYPICAL SECTION FOR PHASED CONSTRUCTION DETAILS. RADIUS PC STATION PT STATION = 744+34.37 = 746+79.55 = N 81°50′ 58.48" W I 9. SHEAR KEYS REQUIRED AT ABUTMENT 1 AND BENTS. BACK BEARING AHEAD BEARING 2 - 3 SEE "IGSK" STANDARD AND DETAILS SHEETS. PLAN 10. FOUND DRILLED SHAFTS AT LENGTHS SHOWN
OR LONGER TO OBTAIN A MINIMUM ONE DRILLED SHAFT EX ROW DIAMETER PENETRATION INTO HARD ROCK. HYDRAULIC DATA 50 YEAR EL = 1741.31 Q = 43400 CFS V = 6.62 FPS ALL BENTS ON BEARING N 9°24′58" E (UNO) 100 YEAR EL = 1741.67 Q = 48100 CFS V = 6.90 FPS PRADEEP KUMAR The Stonal English 4/20 1800 OVERALL LENGTH OF BRIDGE = 1220.00' 1800 4/28/2022 BEGIN BRIDGE 145.00' PRESTR CONC GIRDER UNIT (TY TX70) 295.00' PRESTR CONC GIRDER UNIT (TY TX70) RAIL FOR PAYMENT 0 (SPAN 1) (145.00') (SPANS 2 - 3) (150.00' - 145.00') HL93 LOADING 4 1780 1780 BEGIN BRIDGE OVERALL LENGTH OF TYPE T223 RAIL = 1261.00' (EACH SIDE) REV. No. FACE OF BACKWALL ABUTMENT NO. 1 -HW (100 YR) EL = 1741.67 50 STA 746+90.00 EL = 1749.99'~ -HW (50 YR) 1760 1760 TYPE T223 RAIL -PGL MBGF (SEE RDWY EL = 1741.31 (+)0.7500% PLANS) (TYP) ⋖ S 1740 1740 INE PROPOSED GRADE 🚩 Texas Department of Transportation RIPRAP (STONE PROTECTION) 12 IN, -EXISTING GROUND 1720 1720 Abilene District THICKNESS = 18 IN 4 - 18" DIA MIN TOE DIMENSIONS =  $36 \text{ IN} \times 36 \text{ IN}$ -5 - 42" DIA US 380 DRILLED SHAFTS x 30 MATCH 4 - 42" DIA -4 - 42" DIA DRILLED SHAFTS x 30' DRILLED SHAFTS x 38' DRILLED SHAFTS x 38 BRIDGE LAYOUT 1700 1700 (3)  $\langle 1 \rangle$  $\langle 2 \rangle$ 1680 1680 SALT FORK OF BRAZOS RIVER BRIDGE ELEVATION SIGNED:TS HIGHWAY No HECKED: SK SEE TITLE SHEET US 380 TEXAS 1660 CONTROL SECTION JOB No. SHEET No. RAWN: DN COUNTY 747+00 748+00 749+00 750+00 746+00 ECKED: TS ABL TONEWALL 0106

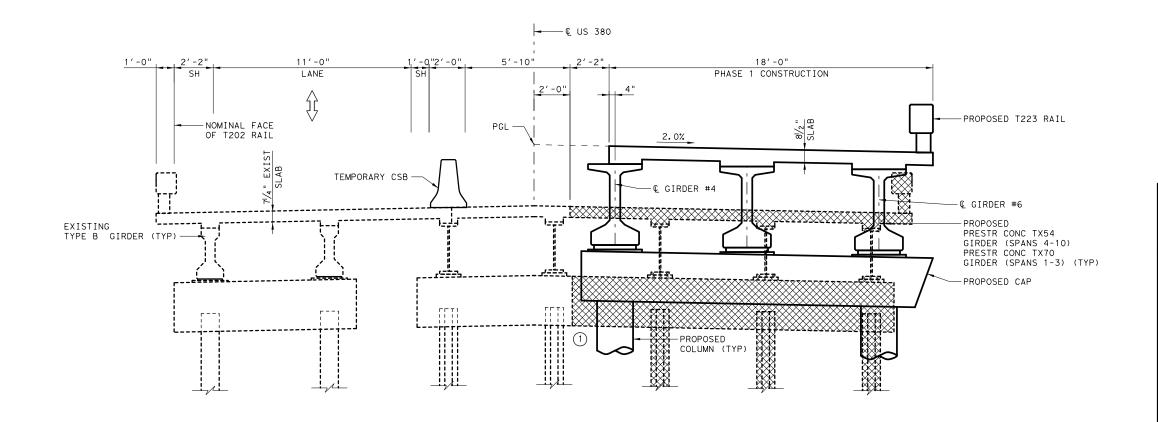
NOTES: 1. FOR GENERAL NOTES SEE BRIDGE LAYOUT 1 OF 4. - BORE HOLE: B-3 STA: 752+82.85 OFFSET: 35.38′ LT ELEV: 1735.86 -EXISTING FIBER CENTURYLINK (QLB) -BORE HOLE: B-4 STA: 750+93.85 SUBMITTAL OFFSET: 38.80' LT EXISTING BRIDGE ELEV: 1737.79 TO BE REMOVED — € US 380 & PGL 4+00 50+40 € BENT NO.4 € BENT NO.5 € BENT NO.6 1) PLACE WATER BARRIER PER T223 STANDARD. LEFT: STA 747+57 TO STA 750+60 RIGHT: STA 747+74 TO 751+04 STA 751+30.00 STA 752+50.00 STA 753+70.00 Š 00% OVERALL 41'-2" -ROADWAY - Q GIRDER #1 NOM FACE 1 OF T223 RAIL ⋖ ⋖ SEJ-M (4") S S -€ GIRDER #6 뵘 INE 9′ -2" SH 1,-0 NOM MATCH MATCH -PHASE CONSTRUCTION JOINT LIMITS OF CRITICAL HABITAT-EX ROW PLAÑ ALL BENTS ON BEARING N 9° 24′ 58" E (UNO) PRADEEP KUMAR 1 SOONAL ENGLANDS / 130846 1800 1800 OVERALL LENGTH OF BRIDGE = 1220.00' 4/28/2022 295.00' PRESTR CONC GIRDER UNIT (TY TX70) 360.00' PRESTR CONC GIRDER UNIT (TY TX54) 0 (SPANS 2-3) (150.00'-145.00') (SPANS 4 - 6) (120.00' - 120.00' - 120.00') HL93 LOADING 1780 4 Ŏ 1780 OVERALL LENGTH OF TYPE T223 RAIL = 1261.00' (EACH SIDE) REV. No. DATE 50+ VPI STA = 753+10.00 EL = 1,754.65' ex = -0.54' K = 300 HW (100 YR) EL = 1741.67 4 2 VPC STA 751+30.00 TYPE T223 RAIL 1760 1760 = 360.00' EL = 1,753.30' (+) 0. 7500%—<u>(-)</u> (-) 0. 4500%· ⋖ ⋖ S S 1740 1740 INE Texas Department of Transportation -EXISTING GROUND 1720 1720 Abilene District US 380 MATCH  $\mathcal{F}$ 4 - 42" DIA 4 - 42" DIA 4 - 42" DIA DRILLED SHAFTS x 32' DRILLED SHAFTS x 43' DRILLED SHAFTS × 43' BRIDGE LAYOUT 1700 1700 MAT (5)  $\langle 4 \rangle$  $\langle 6 \rangle$ 1680 1680 SALT FORK OF BRAZOS RIVER BRIDGE ELEVATION SIGNED:TS PROJECT No. HIGHWAY No. SEE TITLE SHEET HECKED: SK TEXAS US 380 1660 CONTROL SECTION JOB No. No. No. RAWN: DN COUNTY 750+00 751+00 752+00 753+00 754+00 ECKED: TS ABL TONEWALL 0106

NOTES: 1. FOR GENERAL NOTES, SEE BRIDGE LAYOUT 1 OF 4. SUBMITTAL BORE HOLE: B-2 STA: 755+85.21 OFFSET: 37.89' LT -EXISTING FIBER CENTURYLINK (QLB) ELEV: 1735.07 54+00 20 STA 754+90.00 STA 755+95.00 STA 757+00.00 757 00% ⋖ ⋖ - 90° 00′ 00' (TYP) NOM FACE ST OF T223 RAIL S -SEJ-M (4") - & GIRDER #6 SEJ-M (4") LINE INE — € US 380 & PGL EXISTING BRIDGE TO BE REMOVED -PHASE MATCH MATCH CONSTRUCTION EX ROW PLAN ALL BENTS ON BEARING N 9° 24′ 58" E (UNO) 210.00' PRESTR CONC GIRDER This sound and the same of the UNIT (TY TX54) (SPANS 9 - 10) (105.00' - 105.00') 1800 1800 OVERALL LENGTH OF BRIDGE = 1220.00' 4/28/2022 60.00' PRESTR CONC GIRDER UNIT (TY TX54) 210.00' PRESTR CONC GIRDER UNIT (TY TX54) 00+ +20 (SPANS 4-6) (120.00'-120.00'-120.00') (SPANS 7 - 8) (105.00' - 105.00') HL93 LOADING 1780 1780 OVERALL LENGTH OF TYPE T223 RAIL = 1261.00' (EACH SIDE) REV. No. DATE -HW (100 YR) EL = 1741.67 75 5 VPT STA 754+90.90 -HW (50 YR) -TYPE T223 RAIL +PGL 1760 1760 EL = 1,753.84' EL = 1741.31 (-)0.4500% STA⋖ S 1740 1740 INE Texas Department of Transportation -EXISTING GROUND 1720 1720 Abilene District \_\_\_ US 380 MATCH 4 - 42" DIA 4 - 42" DIA 4 - 42" DIA DRILLED SHAFTS x 41' DRILLED SHAFTS  $\times$  41 DRILLED SHAFTS x 41' BRIDGE LAYOUT 1700 1700 MAT  $\langle 7 \rangle$ 8 9 1680 1680 SALT FORK OF BRAZOS RIVER BRIDGE ELEVATION SIGNED:TS SEE TITLE SHEET US 380 HECKED: SK TEXAS 1660 CONTROL SECTION JOB No. No. No. RAWN: DN COUNTY 754+00 755+00 756+00 757+00 758+00 HECKED: TS ABL

NOTES: 1. FOR GENERAL NOTES, SEE BRIDGE LAYOUT 1 OF 4. BORE HOLE: B-1 STA: 758+88.84 OFFSET: 30.95' LT ELEV: 1746.240 SUBMITTAL END BRIDGE FACE OF BACKWALL EXISTING FIBER ABUTMENT NO.11 CENTURYLINK (QLB) STA 759+10.00 Ñ MBGF (SEE RDWY € BENT NO.10 + PLANS) (TYP) STA 758+05.00 S 00% / ⋖ -CSAB (MOD) T223 RAIL S 760+00 - & GIRDER #6 岁 - CURVE 380CL\_2 -----90°00′00" (TYP) 20′-0" - Q US 380 & PGL MATCH BAS-A -PHASE CONSTRUCTION JOINT LIMITS OF RIPRAP 19'-2" (TYP) -EXISTING BRIDGE TO BE REMOVED (STONE PROTECTION) 12 IN THICKNESS = 18 IN (TYP) EX ROW -EXISTING FIBER CAPROCK (QLB) CURVE 380CL\_2 HORIZONTAL ALIGNMENT DATA PI STATION = 761+05.01
DELTA = 1° 21′ 46.86" (RT)
DEGREE OF CURVE = 0° 20′ 58.17"
TANGENT = 195.0092'
LENGTH = 390.0000'
RADIUS = 16,394.0359'
PC STATION = 759+10.00 N
PT STATION = 763+00.00 N
BACK BEARING = N 80° 35′ 02.38" W
AHEAD BEARING = N 79° 13′ 15.51" W PLAN ALL BENTS ON BEARING N 9° 24′ 58" E (UNO) PRADEEP KUMAR 130846 1800 1800 OVERALL LENGTH OF BRIDGE = 1220.00' 4/28/2022 -END BRIDGE RAIL FOR PAYMENT 210.00' PRESTR CONC GIRDER UNIT (TY TX54) (SPANS 9 - 10) (105.00' - 105.00') END BRIDGE HL93 LOADING 1780 1780 S OVERALL LENGTH OF TYPE T223 RAIL = 1261.00' (EACH SIDE) FACE OF BACKWALL REV. No. ABUTMENT NO. 11 -HW (100 YR) EL = 1741.67 VPC STA 758+38.60 STA 759+10.00 2 EL = 1,752.27' EL = 1752.136' -HW (50 YR) - PGL MBGF (SEE RDWY 1760 TYPE T223 RAIL 1760 EL = 1741.31 (-)0.4500% (-)0.4500%<u>(+)2.8500%</u> VPI STA = 760+62.99 S 1740 1740 = 1,751.26° ex 빙 PROPOSED GRADE -5 - 36" DIA = 448.80′ -EXISTING GROUND Texas Department of Transportation DRILLED SHAFT x 16' 1720 1720 Abilene District US 380 SH -4 - 42" DIA DRILLED SHAFTS x 41' BRIDGE LAYOUT 1700 1700 MAT RIPRAP (STONE PROTECTION) 12 IN, THICKNESS = 18 IN MIN TOE DIMENSIONS = 36 IN x 36 IN 10  $\langle 11 \rangle$ SALT FORK OF BRAZOS RIVER BRIDGE 1680 1680 ELEVATION SIGNED:TS HIGHWAY No. SEE TITLE SHEET HECKED: SK TEXAS US 380 1660 CONTROL SECTION JOB No. RAWN: DN COUNTY 757+00 758+00 759+00 760+00 761+00 ECKED: TS ABL STONEWALL 0106

**Q** US 380 42'-0" OVERALL 40'-0" 1'-0" 1′-0" ROADWAY 8'-0" 12'-0" 12'-0" 8'-0" SHOULDER LANE LANE SHOULDER NOMINAL FACE NOMINAL FACE OF T202 RAIL OF T202 RAIL --EXISTING PGL 2.0% 2.0% EXISTING TYPE B GIRDER (TYP) -EXISTING STEEL GIRDER (TYP) - 45<u>5</u>51------

## EXISTING TYPICAL SECTION



PHASE 1

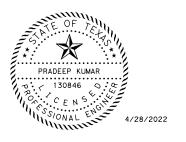


PORTION OF EXISTING BRIDGE
TO BE REMOVED IN PHASE INDICATED

AT PROPOSED TX70 (SPANS 1-3, EXISTING BENTS 2-8), THERE IS NO CLEARANCE BETWEEN BOTTOM OF GIRDERS AND TOP OF EXISTING BENT CAP. PHASED REMOVAL REQUIRED. AT PROPOSED SPANS 8-10 (EXISTING BENTS 20-23), THERE IS MINIMAL CLEARANCE BETWEEN BOTTOM OF THE TX54 GIRDERS AND TOP OF EXISTING BENT CAP. PHASED REMOVAL REQUIRED. REQUIRED.

#### NOTE:

1. CONTRACTOR SHALL FIELD VERIFY EXISTING STRUCTURE DIMENSIONS AND ELEVATIONS PRIOR TO COMMENCING DEMOLITION AND NEW CONSTRUCTION.



#### HI OZ I OADINO

HL93	LUADING		
REV. No.	DATE	REVISION	BY

# **ATKINS**

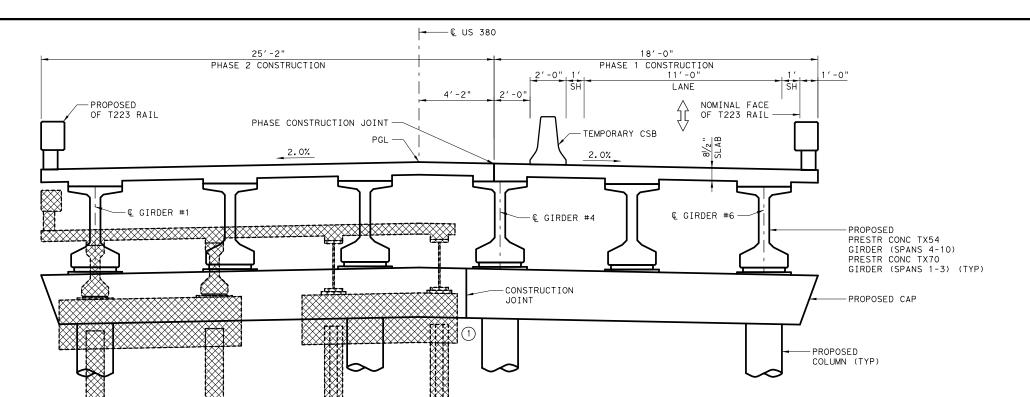


US 380

TYPICAL SECTION

SALI	FURN UF	DRAZUS	KIAEK	DK 1	DGE
SCALE:	3/ <sub>16</sub> "=1'-0"		SHE	ET 1	I OF

IGNED	:TS	FED. RD DIV. No.	STATE	PROJE	HIGHWAY No.			
CKED:	SK	6	TEXAS :	SEE TIT	LE SHE	ΞT	U	S 380
wn:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB o.	SHEET No.
CKED:			STONEWALL	0106	04	0	36	85

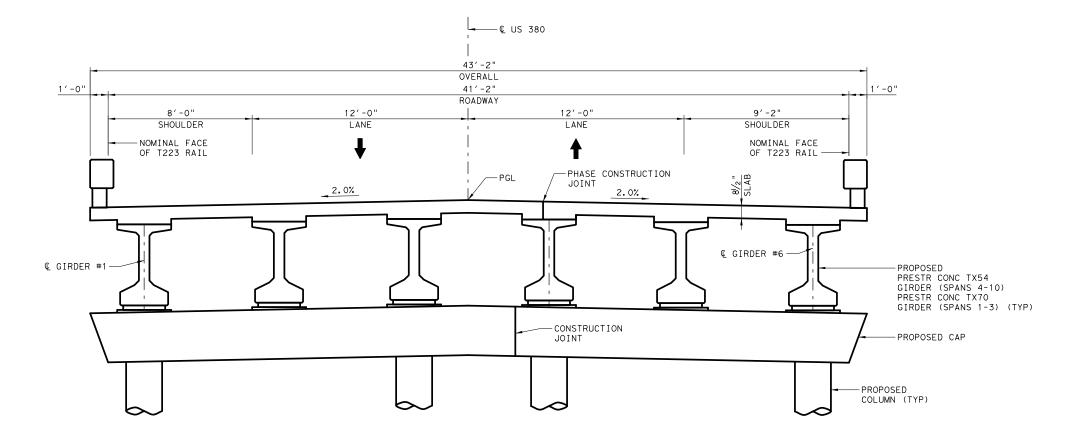


PORTION OF EXISTING BRIDGE
TO BE REMOVED IN PHASE INDICATED

AT PROPOSED TX70 (SPANS 1-3, EXISTING BENTS 2-8), THERE IS NO CLEARANCE BETWEEN BOTTOM OF GIRDERS AND TOP OF EXISTING BENT CAP. PHASED REMOVAL REQUIRED. AT PROPOSED SPANS 8-10 (EXISTING BENTS 20-23), THERE IS MINIMAL CLEARANCE BETWEEN BOTTOM OF THE TX54 GIRDERS AND TOP OF EXISTING BENT CAP. PHASED REMOVAL REQUIRED. REQUIRED.

# PHASE 2

FINAL





HL93 LOADING

11233	LOADING		
REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380

TYPICAL SECTION

SALI	FORK	OF	BKAZUS	KIAFK	BRIDGE	
SCALE: 3				SH	EET 2 OF 2	2
DESIGNED: <b>TS</b>	FED. RD DIV. No.	STATE	PROJEC	CT No.	HIGHWAY No.	

ESIGNED	NED:TS FED. RD STATE PROJECT NO.					HIGHWAY No.			
HECKED:	SK	6	TEXAS	SEE TIT	LE SHE	US 380			
RAWN:	DN	STATE DISTRICT	COUNTY	CONTROL No.			ов •	SHEET No.	
HECKED:			STONEWALL	0106			0106 04 036		86

DRILLING LOG

County Stonewall

0106-04-036

Highway US 380

CSJ

WinCore

Version 3.3

1 of 2

District Abilene Structure 07/17/20 to 07/18/20 Bridge Date Grnd. Elev. 1746.24 ft Station 758+88.84 Offset 30.95' LT GW Elev. 1726.84 ft

L Texas Cone		Triaxial Test		Prop	ertie	s	
C Texas Cone O Penetrometer	Strata Description	Lateral Deviator Press. Stress (psi) (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
	SAND, Clayey with Gravel, dry, brown, fine grained, trace roots	(F37) (F37)	1			AF = -7	SSS@0', N=29
	CLAY, Sandy, Silty, soft, moist, reddish brown (CL-ML)		4	20	6		SSS@3', N=8, -#200=53.3%
6 (6) 9 (6)							
	CLAY, Lean, hard, moist, reddish		15				SSS@7.5', N=55
50 (5.5) 50 (2.5)	brown and gray, trace Gravei (CL)		15	37	20		SSS@10.8', N=24, 38, 50/5,
							-#200=98.9%
50 (1.5) 50 (0.5)	SHALE, hard, reddish brown, trace Gypsum crystals		9				SSS@15.3', N=50/5.75
50 (1.25) 50 (1.5)							222 200 0' N 04 00 50/0 00
							SSS@20.3', N=24, 38, 50/2.25 Sulfate Content=8,960 ppm
E0 (1) 50 (0 5)	SHALE very hard reddish brown						
50 (1) 50 (0.5)	trace Gypsum crystals to 35.6						SSS@25.2', N=30/4, 10/0
50 (1) 50 (0.25)			16				SSS@30.2', N=30/4, 10/0
50 (1) 50 (0.5)			13	30	14		SSS@35.3', N=30/3.25, 10/0
50 (0.75) 50 (0.25							
	6 (6) 9 (6)  50 (5.5) 50 (2.5)  50 (1.5) 50 (0.5)  50 (1) 50 (0.5)	G Penetrometer Strata Description  SAND, Clayey with Gravel, dry, brown, fine grained, trace roots (SC) CLAY, Sandy, Silty, soft, moist, reddish brown (CL-ML)  CLAY, Lean, hard, moist, reddish brown and gray, trace Gravel (CL)  SO (1.5) 50 (0.5)  SHALE, hard, reddish brown, trace Gypsum crystals  SHALE, very hard, reddish brown, trace Gypsum crystals to 35.6	Strata Description  Strata Description  Press. Stress (psi)  SAND, Clayey with Gravel, dry, brown, fine grained, trace roots (SC) CLAY, Sandy, Silty, soft, moist, reddish brown (CL-ML)  CLAY, Lean, hard, moist, reddish brown and gray, trace Gravel (CL)  SO (1.5) 50 (0.5)  SHALE, hard, reddish brown, trace Gypsum crystals  SHALE, very hard, reddish brown, trace Gypsum crystals to 35.6  SHALE, very hard, reddish brown, trace Gypsum crystals to 35.6	Strata Description   Press. Stress (psi)   MC	Strata Description	Sand   Sand	Strata Description

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Luis Salgado

Organization: Corsair Consulting LLC

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**DRILLING LOG** 

WinCore Version 3.3

County Stonewall Highway US 380 CSJ 0106-04-036 Structure Bridge 758+88.84 Station Offset 30.95' LT

Date 07/17/20 to 07/18/20 Grnd. Elev. 1746.24 ft GW Elev. 1726.84 ft

District

Abilene

2 of 2

	11	Texas Cone			ai iesi		LIO	ertie		
Elev. (ft)	L O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SHALE, very hard, reddish brown, trace Gypsum crystals to 35.6	,	<b>Y</b> /				<b>1</b>	SSS@40.2', N=20/2, 10/0
45 - - -		50 (0.75) 50 (0.12	5)			14				SSS@45.2', N=25/2.5, 10/0
50 - - -		50 (0.25) 50 (0.25)								SSS@50.1', N=50/3.5
 692.2 55 		50 (2.25) 50 (0.75	SHALE, hard to very hard, reddish brown							SSS@55.3', N=50/4.75
60 - -		50 (2.75) 50 (0.5)								SSS@60.5', N=50/4.5
- - 681. <sup>65</sup> - -		50 (1.5) 50 (0.125	,							Boring Terminated at 65.2'
70 –										
- - 75 -	-									
- - - 80 -										
	_			-						

Remarks: Drill Rig: CME 75 with TxDOT 170-pound Automatic Hammer; SSS: Split Spoon Sample; PTS: Push Tube Sample; PP: Pocket Penetrometer Reading (tsf); Drilling Method: CFA to 25', then Mud Rotary; Northing: 7130953.14, Easting: 1376780.34

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

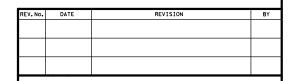
Logger: Luis Salgado

Organization: Corsair Consulting LLC

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#### NOTE:

1. BORING LOGS CONDUCTED BY CORSAIR CONSULTING LLC ARE SHOWN HERE FOR INFORMATIONAL PURPOSES ONLY. SEE GEOTECHNICAL MEMORANDUM US 380 AND SH 208 BRIDGE REPLACEMENT, STONEWALL AND KENT COUNTIES, TEXAS, CORSAIR PROJECT NO. 2000515, DATED AUGUST 20,







US 380 BORING LOGS

### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: N	NO SCA	LE				SH	EET	1 OF 6
DESIGNED: XX	FED. RD DIV. No.	STATE		PROJE	CT No.		ніс	SHWAY No.
CHECKED: SK	6	TEXAS		SEE TIT	LE SHE	US 380		
DRAWN: DN STATE COUNTY CONTROL S					SECTION	J	ОВ	SHEET

**DRILLING LOG** 

County Stonewall

0106-04-036

Highway US 380

CSJ

WinCore

Version 3.3

1 of 2

District Abilene 07/18/20 to 07/19/20 Structure Bridge Date Grnd. Elev. 1735.07 ft 755+85.21 Station Offset 37.89' LT GW Elev. 1731.27 ft

	L			Triax	al Test		Prop	ertie	s	
Elev. (ft)	O G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
- - -			SAND, Clayey, moist, brown, fine grained, traces Gravel and ferrous staining (SC)	(190.7	(40.7	9			(60.7	SSS@0', N=13 SSS@3', N=9
1731.3 _ 5 - -		13 (6) 18 (6)	SAND, Poorly Graded, slightly compact, wet, brown, fine to medium grained, few Gravel; SM layer to 5' (SP)			17				SSS@6.5', N=10, -#200=4.2%
1727.1 – – 10 –		5 (6) 3 (6)	SAND, Poorly Graded, loose, wet, brown, fine to medium grained, few Gravel (SP)							
- - -	-					_17				SSS@11.5', N=8
1721.1 - 15 - - -		11 (6) 8 (6)	CLAY, Lean, soft, moist, brown and gray, trace Gravel, little ferrous deposits (CL)			15	34	17		SSS@16.5', N=33, -#200=95.7% Sulfate Content=720 ppm
1716.1 - 20 - - -		24 (6) 22 (6)	CLAY, Lean, very stiff, moist, brown and gray, few ferrous deposits (CL)			19				SSS@21.5', N=16
1711.1 - 25 - - -		34 (6) 42 (6)	CLAY, Silty, very stiff, moist, brown and gray, few ferrous deposits (CL-ML)				20	6		SSS@26.5', N=36, -#200=88.4%
1706.1 - 30 - -		50 (6) 50 (5.75)	CLAY, Lean, hard, moist, brown and gray, few ferrous deposits (CL)			17				SSS@31.2', N=72
1701.1 - 35 - - -		50 (1.5) 50 (0.5)	SHALE, hard to very hard, reddish brown, trace Gypsum crystals							SSS@35.3', N=50/4.75
40 -		50 (1.25) 50 (0.5)								
Remarks			ith TxDOT 170-pound Automatic Hamme ing (tsf); Drilling Method: CFA to 5', HSA							

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Luis Salgado

Organization: Corsair Consulting LLC

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**DRILLING LOG** 

WinCore Version 3.3

Elev. (ft)

County Stonewall Highway US 380 CSJ 0106-04-036

Texas Cone

Bridge Structure 755+85.21 Station Offset 37.89' LT

District Abilene Date Grnd. Elev. 1735.07 ft GW Elev. 1731.27 ft

07/18/20 to 07/19/20

2 of 2

GEOTECHNICAL MEMORANDUM US 380 AND SH 208 BRIDGE REPLACEMENT, STONEWALL AND KENT COUNTIES, TEXAS, CORSAIR PROJECT NO. 2000515, DATED AUGUST 20,

1. BORING LOGS CONDUCTED BY CORSAIR CONSULTING LLC ARE SHOWN HERE FOR

INFORMATIONAL PURPOSES ONLY. SEE

NOTE:

Lateral Deviator Press. Stress (psi) (psi) MC LL PI Den. (pcf) SSS@40.3', N=50/4.5 SHALE, hard to very hard, reddish 50 (1.75) 50 (0.5) 45 -SSS@45.4', N=20/2.75, 10/0 50 (0.75) 50 (0.25) SHALE, very hard, reddish brown trace Gypsum crystals to 55.5' 50 13 39 23 SSS@50.2', N=50/4.75 50 (1) 50 (0.5) SSS@55.2', N=50/4 50 (0.25) 50 (0.125) SSS@60.1', N=50/2.25 60 50 (0.25) 50 (0.125) SSS@65.1', N=10/0.5, 10/0 50 (0.5) 50 (0.125) Boring Terminated at 70.1'

Remarks: Drill Rig: CME 75 with TxDOT 170-pound Automatic Hammer; SSS: Split Spoon Sample; PTS: Push Tube Sample; PP: Pocket Penetrometer Reading (tsf); Drilling Method: CFA to 5', HSA to 45', then Mud Rotary; Northing: 7130896.62, Easting: 1377078.74

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Logger: Luis Salgado

Organization: Corsair Consulting LLC

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REV. No.	DATE	REVISION	BY





US 380 BORING LOGS

#### SALT FORK OF BRAZOS RIVER BRIDGE

	. •	•					•••		
SCALE: N	NO SCA	LE				SH	EET	2 OF 6	
DESIGNED: XX	FED. RD DIV. No.	STATE		PROJE	CT No.		ніс	SHWAY No.	
CHECKED: SK	6	TEXAS		SEE TIT	LE SHE	ΕT	US 380		
DRAWN: DN	STATE DISTRICT	COUNT	Υ	CONTROL No.	SECTION No.	0B 0.	SHEET No.		

WinCore Version 3.3

County Stonewall Highway US 380 CSJ 0106-04-036

0106-04-036

### DRILLING LOG

Bridge

752+82.85

35.38' LT

Structure

Station

Offset

District Abilene 07/19/20 to 07/20/20 Date Grnd. Elev. 1735.86 ft GW Elev. 1729.86 ft

	L	T 0		Triaxi	al Test		Prop	ertie	s	
Elev. (ft)	O G	Texas Cone Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SAND, Clayey, moist, brown, fine grained, traces Gravel and ferrous staining (SC)		``	7			ч ,	SSS@0', N=6
32.9 - - 5 -		6 (6) 5 (6)	CLAY, Sandy Lean, soft, moist, brown, traces ferrous staining, 3" GP with Sand seam at 7.8'				22	8		SSS@3', N=7, -#200=60.3%
-			(CL)			20				SSS@6.5', N=10
27.9			SAND, Silty, loose, wet, brown,			10				SSS@8', N=6 -#200=4.9%
10 -		5 (6) 7 (6)	fine grained; SP layers to 9.5' and below 13.8' (SM)							
-						19				SSS@11.5', N=1
-						19				SSS@13', N=1
'21.4 15 - -		15 (6) 14 (6)	SAND, Poorly Graded with Silt, slightly compact, wet, brown, fine to medium grained, trace Gravel; CL layer below 17.7' (SP-SM)			14				SSS@16.5', N=25, -#200=7.5%
- 16.9 -	- (	(2) (2)	SAND, Clayey, compact, wet, brown,							
20 -	-	26 (6) 20 (6)	fine to coarse grained, trace Gravel (SC)			17				SSS@21.3', N=4
13.9 - - -			CLAY, Lean, very soft, moist, brown and gray, traces calcareous deposits and Gypsum crystals (CL)			21	27	12		-#200=93.2%
<b>25</b> - -		4 (6) 3 (6)				20				SSS@26.5', N=3, 17, 50/1.5
- - 06.9 - 30		50 (0.5) 50 (0.25)	SHALE, very hard, reddish brown	_						SSS@30.2', N=49, 50/3.5
-										
01.9 - 35 -		50 (2.5) 50 (1)	SHALE, hard, reddish brown	_						SSS@35.5', N=45, 50/3
-										
40 -		50 (1) 50 (0.5)								

Penetrometer Reading (tsf); Drilling Method: CFA to 5', HSA to 30', then Mud Rotary; Northing: 7130849.63, Easting: 1377377.44

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Luis Salgado

Organization: Corsair Consulting LLC

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1 of 2

### **DRILLING LOG**

WinCore Version 3.3

County Stonewall Highway US 380 CSJ 0106-04-036

Bridge Structure 752+82.85 Station Offset 35.38' LT

District Date

07/19/20 to 07/20/20 Grnd. Elev. 1735.86 ft GW Elev. 1729.86 ft

Abilene

2 of 2

NOTE:

1. BORING LOGS CONDUCTED BY CORSAIR CONSULTING LLC ARE SHOWN HERE FOR INFORMATIONAL PURPOSES ONLY. SEE GEOTECHNICAL MEMORANDUM US 380 AND SH 208 BRIDGE REPLACEMENT, STONEWALL AND KENT COUNTIES, TEXAS, CORSAIR PROJECT NO. 2000515, DATED AUGUST 20,

	L	Texas Cone			al Test		Prop	ertie		
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press.	Deviator Stress (psi)	мс	LL	PI	Wet Den. (pcf)	Additional Remarks
			SHALE, hard, reddish brown	(20.7	(F-0.)				(100.)	SSS@40.2', N=50/4
	Ē									
_	Ē									
691.9 -	Ξ									
45 -	≣	50 (0.5) 50 (0.5)	SHALE, very hard, reddish brown, trace Gypsum crystals			16				SSS@45.1', N=50/3
-	≣		шас сурсані стускаю							
-	≣									
-	≣									
	≣	50 (0.75) 50 (0.25)	•							200
50 -		, ,								SSS@50.2', N=50/3.5
_										
_	≣									
_	≣									
55 -	₽	50 (1.25) 50 (0.5)				12	43	24		SSS@55.3', N=50/6 Sulfate Content=9,200 ppm
-	≣									Suitate Content=9,200 ppm
-	≣									
-	≣									
60 -	≣	50 (0.5) 50 (0.25)								SSS@60.1', N=28/1.75, 10/0
- 00										
-	ቜ									
_	░									
-	≣	50 (0 75) 50 (0 10	-,							
65 -	░	50 (0.75) 50 (0.12	5)							SSS@65.1', N=10/0.25, 10/0
-	≣									
-	≣									
665.870 -	≣	50 (0.5) 50 (0.125)								Boring Terminated at 70.1'
-	- 1									
-	-									
-	$\{\ \  $									
-										
75 -	1									
-										
_										
-										
80 -										

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Luis Salgado

Organization: Corsair Consulting LLC

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REV. No.	DATE	REVISION	BY





US 380 BORING LOGS

#### SALT FORK OF BRAZOS RIVER BRIDGE

				—				
SCALE: N	NO SCA	LE				SHI	EET	3 OF 6
DESIGNED: XX	FED. RD DIV. No.	STATE		PROJE	CT No.		ніс	HWAY No.
CHECKED: SK	6	TEXAS		SEE TIT	LE SHE	Ξ	J	S 380
DRAWN: DN	STATE DISTRICT	COUNT	Y	CONTROL No.	SECTION No.		0B 0.	SHEET No.

WinCore

Version 3.3

35 -

50 (1.5) 50 (0.5)

Highway US 380

0106-04-036

CSJ

**DRILLING LOG** County Stonewall

Structure

Station

Offset

Bridge

750+93.85

38.80' LT

1 of 2

District Abilene 07/23/20 to 07/24/20 Date Grnd. Elev. 1737.79 ft GW Elev. 1728.79 ft

SSS@35.2', N=50/4.5

	L	Texas Cone		Triaxi	al Test		Prop	ertie	es	
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	PI	Wet Den. (pcf)	Additional Remarks
			SAND, Silty, dry, light brown, fine grained (SM)		ч ,	4			<u>, , , , , , , , , , , , , , , , , , , </u>	SSS@0', N=10
	-	6 (6) 12 (6)				2				SSS@3', N=5
1732.85		6 (6) 12 (6)	SAND, Silty, loose, wet, light brown, fine grained (SM)			17				SSS@6.5', N=18
1729.8			SAND, Poorly Graded with Silt and Gravel, loose, wet, brown,			12				SSS@8', N=4
10	-	4 (6) 8 (6)	fine to coarse grained; SM layer to 10' (SP-SM)			9				SSS@11.5', N=12, -#200=6.4%
1723.3 15	-	18 (6) 18 (6)	CLAY, Lean, stiff, moist, reddish							
			brown and gray (CL)			21				SSS@16.5', <b>N=</b> 7
1717.820		50 (2) 50 (1.5)	SHALE, hard, reddish brown, trace							SSS@20.5', N=40, 50/5.75
			calcareous deposits and Gypsum crystals							Sulfate Content=8,080 ppm
1713.8 25		50 (0.5) 50 (0.75)	SHALE, very hard, reddish brown, trace Gypsum crystals to 25.6'							SSS@25.2', N=50/5
30		50 (0.5) 50 (0.25)								SSS@30.1', N=50/5

40 50 (0.25) 50 (0.75) Remarks: Drill Rig: CME 75 with TxDOT 170-pound Automatic Hammer; SSS: Split Spoon Sample; PTS: Push Tube Sample; PP: Pocket Penetrometer Reading (tsf); Drilling Method: HSA to 20', then Mud Rotary; Northing: 7130815.33, Easting: 1377563.33

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA Logger: Jose Flores Organization: Corsair Consulting LLC

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**DRILLING LOG** 

WinCore Version 3.3

Elev. (ft)

45 -

50 -

60

1667.770

County Stonewall Highway US 380 CSJ 0106-04-036

Bridge Structure Station 750+93.85 Offset 38.80' LT

District Abilene 07/23/20 to 07/24/20 Date Grnd. Elev. 1737.79 ft

GW Elev. 1728.79 ft

2 of 2

Texas Cone Lateral Deviator Press. Stress (psi) (psi) MC LL PI Den. (pcf) SSS@40.2', N=50/3.5 SHALE, very hard, reddish brown, trace Gypsum crystals to 25.6' 50 (0.5) 50 (0.25) SSS@45.1', N=50/3 50 (1.25) 50 (0.5) SSS@50.2', N=20/2.5, 10/0 50 (1) 50 (0.125) SSS@55.2', N=20/1, 10/0 50 (0.5) 50 (0.25) SSS@60.2', N=50/2.5 50 (0.5) 50 (0.5) SSS@65.2', N=20/0.75, 10/0 50 (0.5) 50 (0.25) Boring Terminated at 70.1'

Remarks: Drill Rig: CME 75 with TxDOT 170-pound Automatic Hammer; SSS: Split Spoon Sample; PTS: Push Tube Sample; PP: Pocket eter Reading (tsf); Drilling Method: HSA to 20', then Mud Rotary; Northing: 7130815.33, Easting: 1377563.33

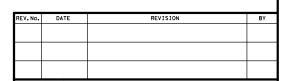
Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA Logger: Jose Flores Organization: Corsair Consulting LLC

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#### NOTE:

1. BORING LOGS CONDUCTED BY CORSAIR CONSULTING LLC ARE SHOWN HERE FOR INFORMATIONAL PURPOSES ONLY. SEE GEOTECHNICAL MEMORANDUM US 380 AND SH 208 BRIDGE REPLACEMENT, STONEWALL AND KENT COUNTIES, TEXAS, CORSAIR PROJECT NO. 2000515, DATED AUGUST 20,







US 380 BORING LOGS

#### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: NO SCALE SHEET 4 OF 6											
DESIGNED: XX FED. RD. STATE PROJECT NO. HIGHWAY NO.											
CHECKED: SK	6	TEXAS	SEE TITLE SHEET US 380								
DRAWN: DN	STATE	COUNT	CONTROL SECTION JO								

DRILLING LOG

County Stonewall

0106-04-036

Highway US 380

CSJ

WinCore

Version 3.3

1 of 2

District Abilene 07/22/20 to 07/23/20 Structure Bridge Date Grnd. Elev. 1737.70 ft 748+20.88 Station Offset 52.57' RT GW Elev. 1729.20 ft

	L	Texas Cone		Triaxi	al Test		Prop	ertie	s	
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL	ΡI	Wet Den. (pcf)	Additional Remarks
-			SAND, Clayey, dry, reddish brown, fine grained, few Gravel (SC)	(100.7	(50.)	4			(100.7	SSS@0', N=20
1735.2 - - 5 -		10 (6) 9 (6)	SAND, Silty, loose, dry to 4.5', wet below 6.5', brown, fine grained, trace CL lenses (SM)			5				SSS@3', N=12
-	_					17				SSS@6.5', N=17
1729.7			SAND, Poorly Graded, loose, wet,			17				SSS@8', N=7
10 - - -	-	6 (6) 7 (6)	brown, fine to coarse grained, trace CL lenses, few Gravel; SM layer to 10' (SP)			_14				SSS@11.5', N=11, -#200=3.6%
- - 1722.715 -		22 (6) 23 (6)	SAND, Poorly Graded, compact,							
-			wet, brown, fine to coarse grained, trace CL lenses, few Gravel (SP)			14				SSS@16.5', N=13
1718.7 - 20 - - - -		20 (6) 18 (6)	SAND, Poorly Graded with Gravel, slightly compact, wet, brown, fine to coarse grained, trace CL lenses (SP)			13				SSS@21.5', N=22, -#200=4.6%
- 1712.725 - - -		50 (1.5) 50 (1)	SHALE, hard, reddish brown, trace Gypsum crystals							SSS@25.3', N=50/4.75
30 - -		50 (1.25) 50 (1.5)								SSS@30.5', N=50/4.5
1703.7 - 35 - -		50 (1) 50 (0.5)	SHALE, very hard, reddish brown, trace Gypsum crystals							SSS@35.3', N=50/4.5
40 -	s: Dr	50 (1) 50 (0.5)	ith TxDOT 170-pound Automatic Hamm	er; SSS:	Split Spo	on Sai	mple;	PTS	S: Push	Tube Sample; PP: Pocket
	Pe	enetrometer Read	ing (tsf); Drilling Method: CFA to 5', HSA	to 35', t	hen Mud	Rotary	; No	rthin	g: 7130	860.80, Easting: 1377847.57

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Jose Flores

Organization: Corsair Consulting LLC

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**DRILLING LOG** 

WinCore Version 3.3

County Stonewall Highway US 380 CSJ 0106-04-036 Structure Bridge 748+20.88 Station Offset 52.57' RT

District Abilene Date 07/22/20 to 07/23/20 Grnd. Elev. 1737.70 ft GW Elev. 1729.20 ft

2 of 2

NOTE:

1. BORING LOGS CONDUCTED BY CORSAIR CONSULTING LLC ARE SHOWN HERE FOR

INFORMATIONAL PURPOSES ONLY. SEE

GEOTECHNICAL MEMORANDUM US 380 AND SH 208 BRIDGE REPLACEMENT, STONEWALL AND KENT COUNTIES, TEXAS, CORSAIR PROJECT NO. 2000515, DATED AUGUST 20,

DATE REVISION



US 380 BORING LOGS

### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: N	NO SCA	LE				SH	EET	5 OF 6
DESIGNED: XX	ніс	SHWAY No.						
CHECKED: SK	6	TEXAS		S 380				
DRAWN: DN	STATE	COUNT	Y	CONTROL	SECTION		ОВ	SHEET

CHECKED: XX ABL STONEWALL 0106 04 036

Texas Cone Lateral Deviator Press. Stress (psi) (psi) MC LL PI Den. (pcf) Elev. (ft) SSS@40.3', N=50/3.5 SHALE, very hard, reddish brown, trace Gypsum crystals 50 (1) 50 (0.5) 45 SSS@45.2', N=20/4.5, 10/0 50 (1) 50 (0.5) 50 -SSS@50.2', N=20/3.5, 10/0 50 (0.75) 50 (1.25) SSS@55.3', N=50/5.5 Sulfate Content=11,120 ppm 50 (1) 50 (0.25) SSS@60.1', N=50/2 60 50 (0.25) 50 (0.25) SSS@65.1', N=20/0.75, 10/0 50 (0.5) 50 (0.25) Boring Terminated at 70.1' 1667.670

Remarks: Drill Rig: CME 75 with TxDOT 170-pound Automatic Hammer; SSS: Split Spoon Sample; PTS: Push Tube Sample; PP: Pocket Penetrometer Reading (tsf); Drilling Method: CFA to 5', HSA to 35', then Mud Rotary; Northing: 7130860.80, Easting: 1377847.57

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA

Logger: Jose Flores

Organization: Corsair Consulting LLC

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DRILLING LOG

County Stonewall

0106-04-036

Highway US 380 CSJ 0106-04-0

WinCore

Version 3.3

1 of 2

District Abilene 07/21/20 Structure Bridge Date Grnd. Elev. 1746.22 ft 746+91.54 Station Offset 66.50' LT GW Elev. 1728.22 ft

	L	Texas Cone		Triaxi	al Test		Prop	ertie	s	
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	MC	LL	PI	Wet Den. (pcf)	Additional Remarks
-			SAND, Clayey, dry, reddish brown, fine grained, traces Gravel and roots (SC)	(10.7)	(POI)	4			(100)	SSS@0', N=21 SSS@3', N=7
743.2 - - 5 -		4 (6) 3 (6)	SAND, Clayey, very loose, dry to moist, reddish brown, fine grained, trace Gravel (SC)			_				000@0 , N=1
-						7				SSS@6.5', N=3
-						6				SSS@8', N=2
736.210 -		12 (6) 10 (6)	CLAY, Sandy Lean, stiff, moist,							
-			reddish brown (CL)			13	23	8		SSS@11.5', N=5, -#200=58.1%
732.2 - 15 -		11 (6) 10 (6)	SAND, Poorly Graded with Silt and Gravel, slightly compact, wet, brown, fine to coarse grained (SP-SM)			14				SSS@16.5', N=8, -#200=7.0%
- - 727.2 - 20 -		14 (6) 12 (6)	SAND, Poorly Graded with Silt, slightly compact, wet, brown, fine to medium grained (SP-SM)			17				SSS@21.5', N=3
- - 721.225 -		48 (6) 46 (6)								
-			SAND, Poorly Graded with Silt, dense, wet, brown, fine to medium grained (SP-SM)			19				SSS@26.2', N=33, -#200=5.29
717.2 - 30 -		40 (6) 50 (3.5)	CLAY, Lean, hard, moist, reddish brown and gray, trace Gypsum crystals (CL)			25	35	19		SSS@31.1', N=16, 50/4.5, -#200=91,4% SSS@32', N=50/4
714.2 - -			SHALE, hard, reddish brown, traces calcareous deposits and Gypsum crystals			21				SSS@32', N=50/4
35 - - -		50 (1.5) 50 (0.75)								SSS@35.3', N=20/3.5, 10/0, Sulfate Content=10,216 ppm
706.240 -		50 (0.75) 50 (0.5)								

Penetrometer Reading (tsf); Drilling Method: CFA to 5', HSA to 20', then Mud Rotary; Northing: 7130722.19, Easting: 1377955.72

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Driller: CoreCo USA Logger: Jose Flores Organization: Corsair Consulting LLC

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### **DRILLING LOG**

0106-04-036

County Stonewall WinCore Highway US 380 Version 3.3

Bridge Structure 746+91.54 Station Offset 66.50' LT

Triaxial Test

District Abilene Date 07/21/20 Grnd. Elev. 1746.22 ft GW Elev. 1728.22 ft

2 of 2

# NOTE:

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	L	Texas Cone		Triaxial Test Properties						
Elev. (ft)	O G	Penetrometer	Strata Description	Lateral Press. (psi)	Deviator Stress (psi)	МС	LL P	Wet Den. (pcf)	Additional Remarks	
-			SHALE, very hard, reddish brown, traces calcareous deposits and Gypsum crystals to 40.5'						SSS@40.3', N=25/2, 10/0	
45 - - -		50 (0.5) 50 (0.25)							SSS@45.1', N=30/3, 10/0	
50 -		50 (0.5) 50 (0.5)							SSS@50.2', N=25/4, 10/0	
- - 55 -		50 (0.5) 50 (0.25)				14			SSS@55.1', N=50/3	
687.2 - 60 -		50 (2) 50 (0.75)	SHALE, hard, reddish brown			15	35 18	ı	SSS@60.3', N=30/5, 10/0	
- 682.2 - 65 - -		50 (0.5) 50 (0.5)	SHALE, very hard, reddish brown, trace Gypsum crystals						SSS@65.4', N=50/5	
70 – 70 – 675.6 _		50 (0.25) 50 (0.25							Boring Terminated at 70.6'	
- 75 - -										
- - 80 -										

Any ground water elevation information provided on this boring log is representative of conditions existing on the day and for the specific location where this information was collected. The actual groundwater elevation may fluctuate due to time, climatic conditions, and/or construction activity.

Logger: Jose Flores

Organization: Corsair Consulting LLC

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REV. No.	DATE	REVISION	BY





US 380 BORING LOGS

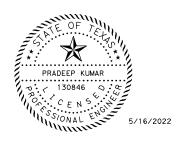
#### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: N	NO SCA	LE				SH	EET	6 OF 6
DESIGNED: XX	FED. RD DIV. No.	STATE		PROJE	CT No.		ніс	SHWAY No.
CHECKED: SK	6	TEXAS		SEE TIT	LE SHE	ΕT	J	S 380
DRAWN: DN	STATE DISTRICT	COUNT	Y	CONTROL No.	SECTION No.		0B 0.	SHEET No.

					SI	UMMARY C	F ESTIM	ATED QU	ANTITIES							
BID CODES		400	416	416	416	420	420	420	422	422	425	425	427	432	450	454
BRIDGE	BID ITEM DESCRIPTION	CEM STABIL BKFL	DRILL SHAFT (18 IN)	DRILL SHAFT (36 IN)	DRILL SHAFT (42 IN)	CL C CONC (ABUT) (HPC)	CL C CONC (CAP) (HPC)	CL C CONC (COLUMN) (HPC)	REINF CONC SLAB (HPC)	APPROACH SLAB (HPC)	PRESTR CONC GIRDER (TX54)	PRESTR CONC GIRDER (TX70)	SILICONE RESIN PAINT FINISH	RIPRAP (STONE PROTECTION) (12 IN)	RAIL (TY T223) (HPC)	SEALED EXPANSION JOINT (4 IN) (SEJ-M)
ELEMENT		CY	LF	LF	LF	CY	CY	CY	SF	CY	LF	LF	SF	CY	LF	LF
2 ~ ABUTMENTS		312	120	80	150	81.9				66.0			601	292		86
9 ~ INTERIOR BENTS					1432		196.8	91.0					4222			170
1 ~ 145.00' PRESTR CONC	GIRDER UNIT								6260			868.09			348.0	
1 ~ 295.00' PRESTR CONC	GIRDER UNIT								12734			1762.96			590.0	
1 ~ 360.00' PRESTR CONC	GIRDER UNIT								15540		2151.00				720.0	
1 ~ 210.00' PRESTR CONC	GIRDER UNIT								9065		1254.00				420.0	
1 ~ 210.00' PRESTR CONC	GIRDER UNIT								9065		1254.00				444.0	
TOTAL		312	120	80	1582	81.9	196.8	91.0	52664	66.0	4659.00	2631.05	4823	292	2522.0	256

# BEARING SEAT ELEVATION

BENT	1	(FWD)	BEAM 1 1742.548	BEAM 2 1742.698	BEAM 3 1742.848	BEAM 4 1742.818	BEAM 5 1742.671	BEAM 6 1742.524
BENT	2	(BK) (FWD)	BEAM 1 1743.583 1743.598	BEAM 2 1743.748 1743.763	BEAM 3 1743.913 1743.928	BEAM 4 1743.898 1743.913	BEAM 5 1743.766 1743.781	BEAM 6 1743.634 1743.649
BENT	3	(BK) (FWD)	BEAM 1 1744.707 1744.701	BEAM 2 1744.872 1744.866	BEAM 3 1745.037 1745.031	BEAM 4 1745.022 1745.016	BEAM 5 1744.890 1744.884	BEAM 6 1744.758 1744.753
BENT	4	(BK) (FWD)	BEAM 1 1745.809 1747.261	BEAM 2 1745.959 1747.411	BEAM 3 1746.109 1747.561	BEAM 4 1746.079 1747.531	BEAM 5 1745.932 1747.385	BEAM 6 1745.786 1747.238
BENT	5	(BK) (FWD)	BEAM 1 1747.910 1747.917	BEAM 2 1748.060 1748.067	BEAM 3 1748.210 1748.217	BEAM 4 1748.180 1748.187	BEAM 5 1748.033 1748.040	BEAM 6 1747.886 1747.893
BENT	6	(BK) (FWD)	BEAM 1 1748.093 1748.092	BEAM 2 1748.243 1748.242	BEAM 3 1748.393 1748.392	BEAM 4 1748.363 1748.362	BEAM 5 1748.216 1748.215	BEAM 6 1748.070 1748.069
BENT	7	(BK) (FWD)	BEAM 1 1747.797 1747.767	BEAM 2 1747.947 1747.917	BEAM 3 1748.097 1748.067	BEAM 4 1748.067 1748.037	BEAM 5 1747.920 1747.890	BEAM 6 1747.773 1747.744
BENT	8	(BK) (FWD)	BEAM 1 1747.303 1747.294	BEAM 2 1747.453 1747.444	BEAM 3 1747.603 1747.594	BEAM 4 1747.573 1747.564	BEAM 5 1747.427 1747.417	BEAM 6 1747.280 1747.271
BENT	9	(BK) (FWD)	BEAM 1 1746.830 1746.821	BEAM 2 1746.980 1746.971	BEAM 3 1747.130 1747.121	BEAM 4 1747.100 1747.091	BEAM 5 1746.954 1746.945	BEAM 6 1746.807 1746.798
BENT	10	(BK) (FWD)	BEAM 1 1746.358 1746.265	BEAM 2 1746.508 1746.415	BEAM 3 1746.658 1746.565	BEAM 4 1746.628 1746.535	BEAM 5 1746.481 1746.389	BEAM 6 1746.334 1746.242
BENT	11	(BK)	BEAM 1 1745.984	BEAM 2 1746.134	BEAM 3 1746.284	BEAM 4 1746.254	BEAM 5 1746.107	BEAM 6 1745.961



HL93 LOADING

REV. No.	DATE	REVISION	BY

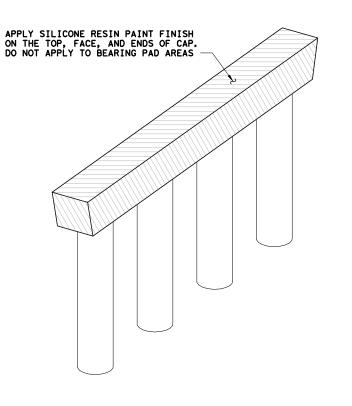
**ATKINS**THE REC.



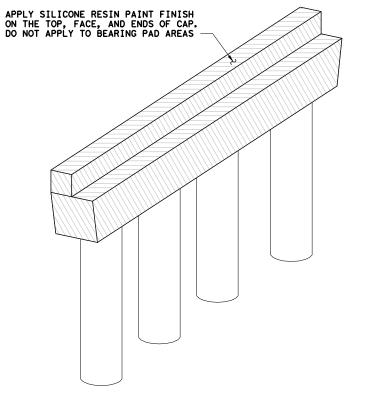
US 380 ESTIMATED QUANTITIES AND

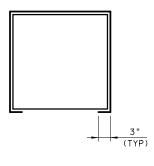
BEARING SEAT ELEVATIONS SALT FORK OF BRAZOS RIVER BRIDGE

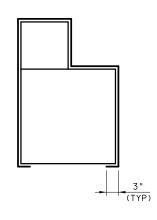
CALL. I	NO SCA	LL	SHEET TOLL							
SIGNED:TS	FED. RD DIV. No.	PROJE	CT No.	HIGHWAY No.						
ECKED: SK	6	TEXAS	SEE TITLE SHEET				US 380			
awn: DN	STATE DISTRICT	COUNTY				OB 0.	SHEET No.			
ECKED: TS	ABI	STONEWALL	0106	04	0	36	93			



CAP ISOMETRIC

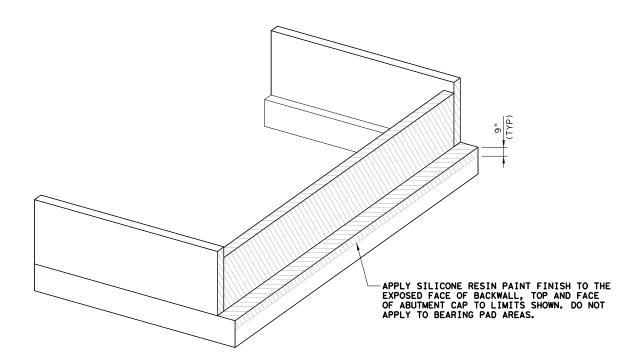






CAP SECTION

# CAP WATERPROOFING DETAIL



ABUTMENT WATERPROOFING DETAIL



HL93 LOADING

REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380 CONCRETE WATERPROOFING DETAIL

### SALT FORK OF BRAZOS RIVER BRIDGE

CALE: NO SCALE SHEET 1 OF 1										
IGNED:TS	FED. RD DIV. No.	STATE	PROJECT No. HIGHWAY No.							
CKED: <b>SK</b>	6	TEXAS	SEE TITLE SHEET US 380							
011	STATE		CONTROL SECTION JOB SHEET							

SUBMITTAL % -€ US 380 & PGL Ŏ -10 1/2 (TYP) 2'-6" BEGIN BRIDGE -LEVEL FOR BRG SEAT (TYP) (TYP) (TYP) FACE OF BKWL, © CAP AND © DR SHAFTS -SHEAR KEY 90°00′00' (UPSTREAM SIDE) ルナー 1 & LEVEL W/ WOOD © GIRDER #6 € GIRDER 1'-0" FLOAT FINISH DOWEL D (TY € BRG SEE FRAMING (TYP) (TYP) TOP OF CAP PLAN FOR 3'-0" ANGLE (TYP) (TYP) -PHASE CONSTRUCTION JOINT GIRDER SPA 3.000 2 SPA AT 7.333' = 14.667' 3 SPA AT 7.500' = 22.500 3.000′ DS SPA 4.167 11.000' 3 SPA AT 8.000' = 24.000' 4.000' 22.167 21.000 BEARING SEAT DETAIL 19.667' PHASE 1 CONSTRUCTION 23.500' PHASE 2 CONSTRUCTION (BEARING SURFACE MUST BE CLEAN AND FREE OF ALL LOOSE MATERIAL BEFORE 43.167 PLACING BEARING PAD) PLAN PHASE 1 PHASE 2 -UNIFORM SLOPE BETWEEN BEARINGS PHASE CONSTRUCTION JOINT BARS V ~ SPA AT 1'-0" MAX (3" FROM WINGWALL PARALLEL -EXTEND BARS H & A 1'-0" MIN FOR MECHANICAL TO ROADWAY SURFACE (TYP) COUPLERS (SHOWN) OR PROVIDE 3'-3" BAR LAPS FOR BARS H ONLY. & CONSTUCTION JT) — € US 380 & PGL € BRG -L (TYP) -SHEAR KEY - (UPSTREAM SIDE) U (TYP) -ss CONST JT (TYP) (TYP) (TYP) 7 SPA \_AT 7 3/4" MAX AT 7 3/4" MAX 12 SPA AT 7 1/2 " MAX BARS S SPA = 7'-6" = 4'-6" = 4'-6" 4 SPA AT 7 1/2" MAX = 2'-6" — 3 SPA AT -3 SPA AT 6" MAX = 1'-6' -2 SPA AT 9" MAX = 1'-6" 7" MAX = 1'-8" **ELEVATION** 

### **GENERAL NOTES:**

— € GIRDER

-DOWEL D ~ GALVANIZED (#9) x 1'-8" AT OUTSIDE GIRDERS ONLY

APPROACH SLAB

(FLUSH WITH TOP OF SLAB)

 $2\frac{1}{2}$ " (TYP UNLESS

OTHERWISE NOTED)

CONST JT

CONST JT

(BUNDLED) (TYP)

1'-6" 1'-6"

4'-0"

SECTION A-A

-0"

- 1. DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.
- 2. CONCRETE SHALL BE CLASS C (HPC), f'c = 3600 PSI.
- 3. ALL REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED)
- 4. SEE FRAMING PLAN FOR GIRDER ANGLES.
- 5. SEE BRIDGE LAYOUT FOR HEADER SLOPE, FOUNDATION TYPE, SIZE AND LENGTH.
- 6. SEE COMMON FOUNDATION DETAILS, FD, STANDARD SHEET FOR ALL FOUNDATION DETAILS AND NOTES.
- 7. SEE STONE RIPRAP, SRR, STANDARD SHEET FOR RIPRAP ATTACHMENT DETAILS.
- 8. SEE SHEAR KEY DETAILS, IGSK, STANDARD SHEET FOR ALL SHEAR KEY DETAILS AND NOTES.
- 9. SEE RAIL DETAILS FOR RAIL ANCHORAGE CAST IN WINGWALLS.
- 10. COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED
- 11. REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR UNLESS NOTED OTHERWISE.
- 12. CALCULATED FOUNDATION LOADS = 142 TONS/DS
- 13.ALL MECHANICAL COUPLERS SHALL BE IN ACCORDANCE WITH ITEM 440, "REINFORCING STEEL".





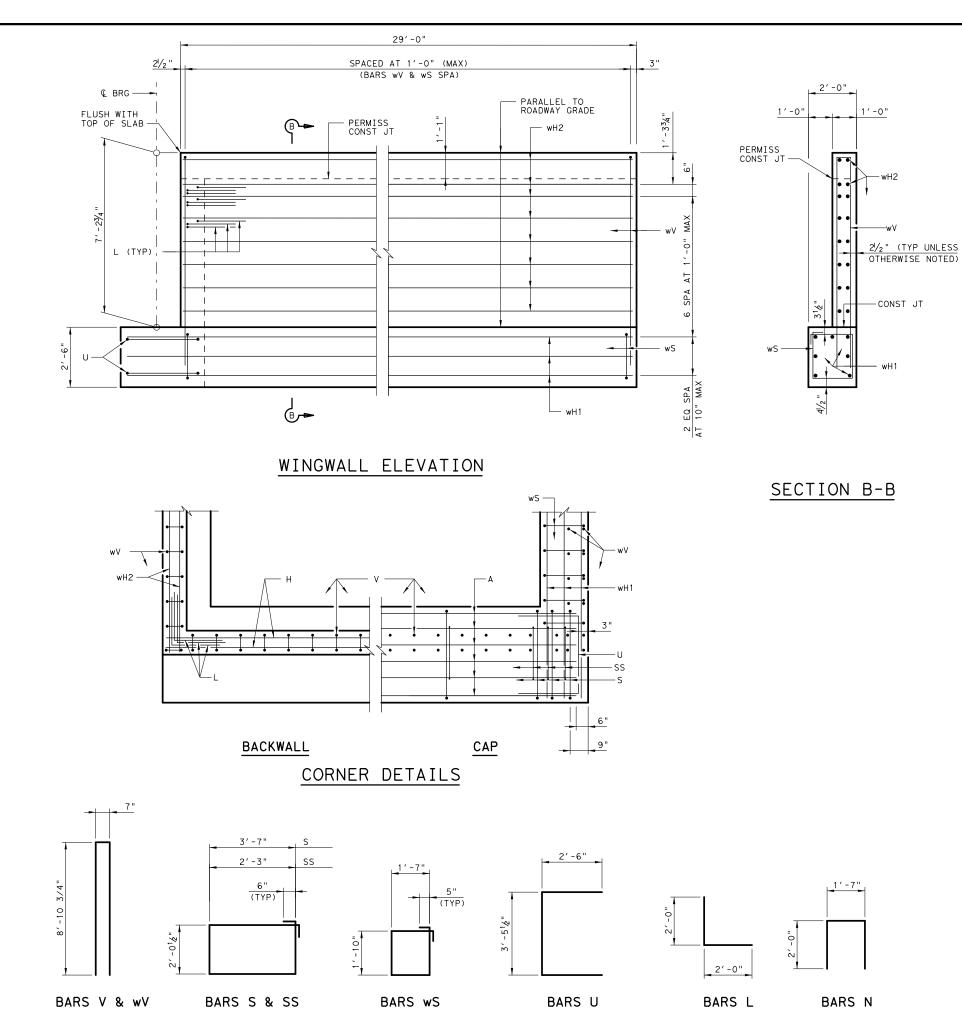
REV. No.	DATE	REVISION	BY
l			

# **NTKINS**



## US 380 ABUTMENT 1

ALE: $\frac{3}{6}$ " = 1'-0" SHEET 1 OF 2											
GNED:TS	FED. RD DIV. No.	STATE	PROJE	CT No.	HIGHWAY No.						
KED: SK	6	TEXAS	SEE TIT	LE SHE	US 380						
n: DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.				
KED: TS	ABL	STONEWALL	0106	04	0	36	95				



# TABLE OF ESTIMATED QUANTITIES

	BAR	NO.	SIZE	LENGTH		WEIGHT			
	Α	16	#11	42′-2"		3585			
	D	2	#9	1′-8"		12			
1	Н	14	#6	42′-9"		899			
	L	18	#6	4′-0"		109			
2	М	2	#5	4′-5"		10			
2	N	8	#5	5′-7"		47			
	S	45	#5	12′-3"		575			
	SS	45 #5 9'-7"		450					
	U	J 4 #6 8'-6		8′-6"		52			
	٧	43	#5	18′-5"		826			
	wH1	14	#6	30′-7"		644			
	wH2	32	#6	28′-7"		1374			
	wS	60	#4	7′-8"		308			
	wV	60	#5	18′-5"		1153			
	REINFO	REINFORCING STEEL LB							
	CLASS	"C" CO	NCRETE	(ABUT) (HPC)	CY	51.3			

- 1 WHERE BAR LAPS ARE PROVIDED IN LIEU OF COUPLERS, MAKE THE FOLLOWING ADJUSTMENTS FOR THE ADDITIONAL STEEL REQUIRED:

  BARS H LENGTH: 3'-3"
  REINFORCING STEEL: 68 LBS
- 2) SEE "IGSK" STANDARD SHEET FOR BARS M AND N.

#### NOTES:

1. FOR GENERAL NOTES SEE "ABUTMENT 1" SHEET 1 OF 2.



HL93 LOADING

TILJU	LOADING		
REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380 ABUTMENT 1

SCALE: 1/4" = 1'-0" SHEET 2 OF 2									
DESIGNED: <b>T</b>	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.		
CHECKED: SI	6	TEXAS	SEE TIT	LE SHE	ΕT	U	S 380		
DRAWN: DI	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		0B 0.	SHEET No.		
CHECKED: TS	ABL	STONEWALL	0106	04	0	36	96		

GENERAL NOTES: 1. DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM. SUBMITTAL 2. CONCRETE SHALL BE CLASS C (HPC), f'c = 3600 PSI. 3. ALL REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED). 4. SEE FRAMING PLAN FOR GIRDER ANGLES. 5. SEE BRIDGE LAYOUT FOR HEADER SLOPE, FOUNDATION TYPE, SIZE AND LENGTH. - PHASE CONSTRUCTION JOINT € US 380 & PGL -6. SEE COMMON FOUNDATION DETAILS, FD, STANDARD SHEET FOR ALL FOUNDATION DETAILS AND NOTES. % END BRIDGE -LEVEL FOR 7. SEE STONE RIPRAP, SRR, STANDARD SHEET FOR RIPRAP FACE OF BKWL, Q CAP AND Q DR SHAFTS Ŏ BRG SEAT (TYP) ATTACHMENT DETAILS. 8. SEE SHEAR KEY DETAILS, IGSK, STANDARD SHEET FOR ALL 90° 00′ 00' SHEAR KEY DETAILS AND NOTES. 9. SEE RAIL DETAILS FOR RAIL ANCHORAGE CAST IN WINGWALLS. 10. COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE. 11. REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR - € GIRDER UNLESS NOTED OTHERWISE. 12. CALCULATED FOUNDATION LOADS = 104 TONS/DS 1'-6"|1'-6" € GIRDER #1 € GIRDER +6 SEE FRAMING-13. ALL MECHANICAL COUPLERS SHALL BE IN ACCORDANCE WITH ITEM 440, "REINFORCING STEEL". PLAN FOR ANGLE (TYP) 1'-0" C BRG (TYP) LEVEL W/ WOOD FLOAT FINISH 4'-6" 3'-0" (TYP) TOP OF CAP -GIRDER SPA 3.000′ 3 SPA AT 7.500' = 22.500' 2 SPA AT 7.333' = 14.667' 3.000 DS SPA 4.000 3 SPA AT 8.000' = 24.000' 11.000' 4.167 21.000' 22.167 23.500' PHASE 2 CONSTRUCTION 19.667' PHASE 1 CONSTRUCTION BEARING SEAT DETAIL 43.167 (BEARING SURFACE MUST BE CLEAN AND FREE OF ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD) PLAN PHASE 2 PHASE 1 SPA AT 1'-0" MAX (3" FROM WINGWALL 3'-6" @ US 380 & PGL --PHASE CONSTRUCTION JOINT 1'-9" 1'-0" -PARALLEL TO ROADWAY SURFACE (TYP) -EXTEND BARS H & A 1'-0" MIN FOR MECHANICAL COUPLERS (SHOWN) OR PROVIDE 3'-3" BAR LAPS FOR BARS H ONLY. -0", UNIFORM SLOPE BETWEEN BEARINGS HL93 LOADING APPROACH SLAB REV. No. (FLUSH WITH TOP OF SLAB) L (TYP) -21/2" (TYP UNLESS € BRG OTHERWISE NOTED) -CONST JT CONSTR JT (TYP) (TYP) 10 SPA BARS S SPA 16 SPA AT 6" MAX AT 6" MAX AT 6" MAX = 5'-0" = 5'-0" = 8'-0" SALT FORK OF BRAZOS RIVER BRIDGE -3 SPA AT 7" MAX = 1'-9" 3 SPA AT 7" MAX = 1'-9" -6 SPA AT 5  $\frac{1}{2}$ " MAX = 2'-9" 3 SPA AT CONST JT 8" MAX = 1'-11"

ELEVATION

SECTION A-A

PRADEEP KUMAR

130846

DATE

SIGNED:TS

CHECKED: SK

RAWN: DN

HECKED: TS ABL

This Stonal English 4/20

REVISION

Texas Department of Transportation Abilene District

PROJECT No.

SEE TITLE SHEET

04

CONTROL SECTION

US 380

JOB No.

US 380

ABUTMENT 11

STATE

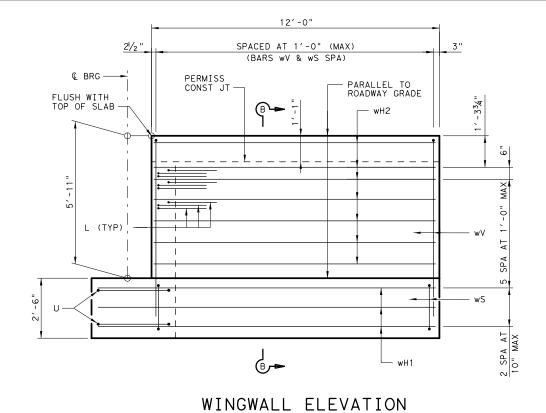
TEXAS

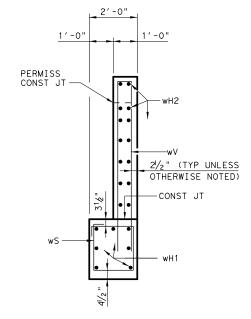
COUNTY

STONEWALL 0106

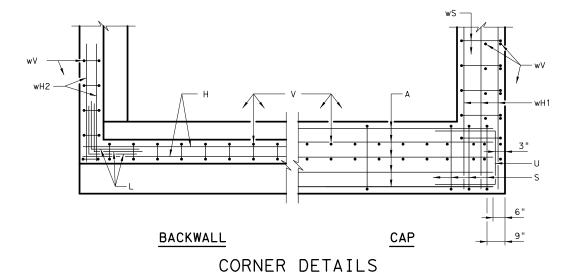
4/28/2022

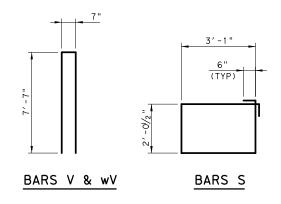


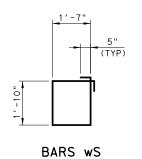


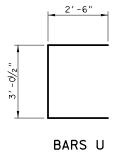


SECTION B-B









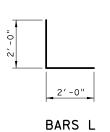


TABLE OF ESTIMATED QUANTITIES

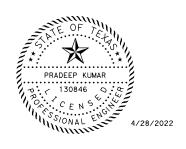
	BAR	NO.	SIZE	LENGTH		WEIGHT				
	Α	12	#11	42′-2"		2689				
1	Н	12	#6	42′-9"		771				
	L	18	#6	4′-0"		109				
	S	58	#5	11'-2"		676				
	U	4	#6	8′-1"	8′-1"					
	٧	43	#5	15′-9"		707				
	wH1	14	#6	13'-4"		281				
	wH2	28	#6	11′-7"		488				
	wS	26	#4	7′-8"		134				
	wV	26	#5	15′-9"		427				
	REINFO	REINFORCING STEEL LB								
	CLASS	"C" CO	NCRETE	(ABUT) (HPC)	CY	30.6				

1) WHERE BAR LAPS ARE PROVIDED IN LIEU OF COUPLERS, MAKE THE FOLLOWING ADJUSTMENTS FOR THE ADDITIONAL STEEL REQUIRED:

BARS H LENGTH: 3'-3"
REINFORCING STEEL: 59 LBS

#### NOTES:

1. FOR GENERAL NOTES SEE "ABUTMENT 11" SHEET 1 OF 2.



HL93 LOADING

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REV. No.	DATE	REVISION	BY

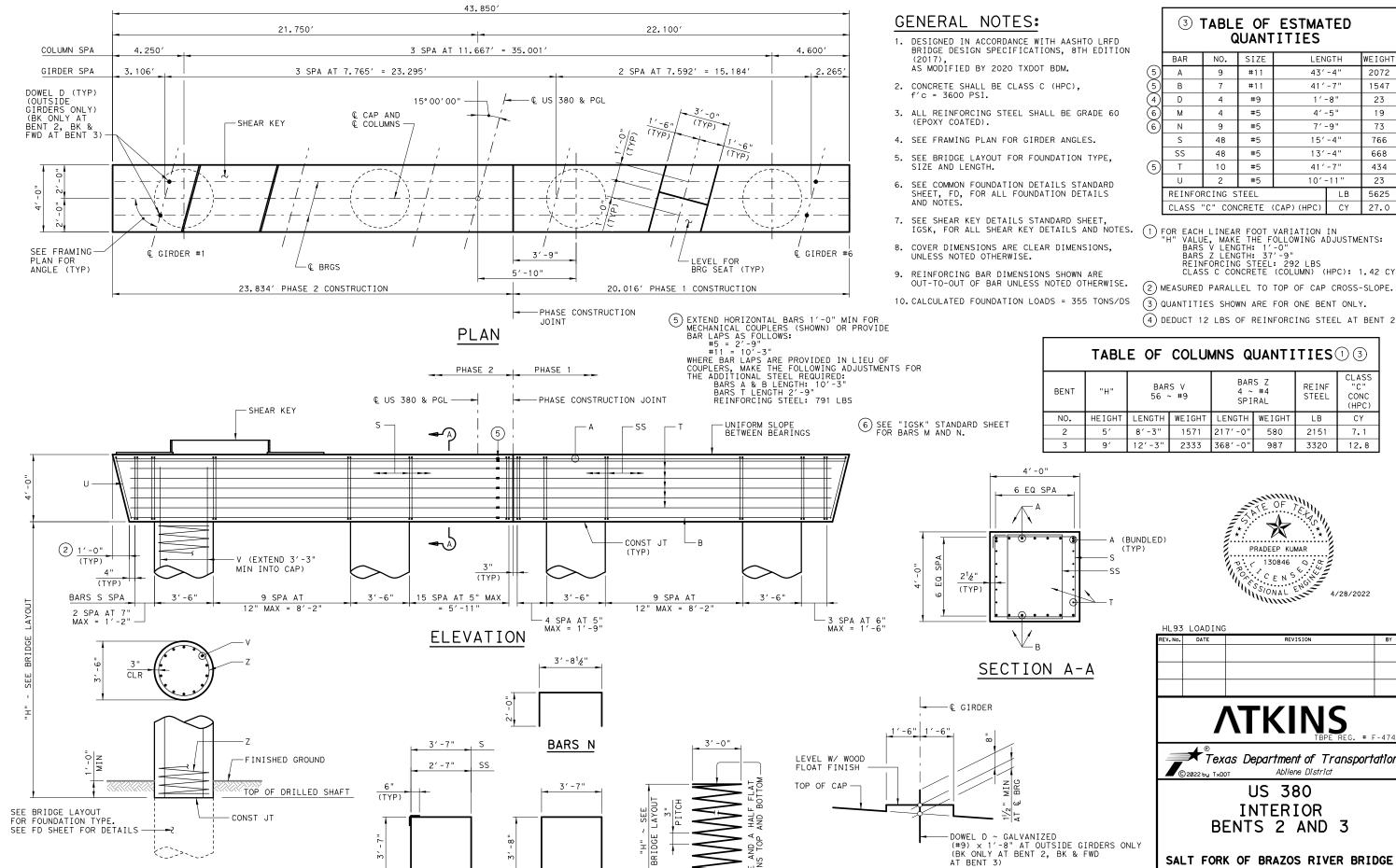
# **ATKINS**



US 380 **ABUTMENT 11** 

SCALE: $\frac{1}{4}$ " = 1'-0" SHEET 2 OF 2									
DESIGNED: <b>TS</b>	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.		
CHECKED: SK	6	TEXAS :	SEE TIT	LE SHE	ΕT	U	S 380		
DRAWN: <b>DN</b>	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.		
CHECKED: TS	ABL	STONEWALL	0106	04	0	36	98		





BARS Z

BARS S & SS

BARS U

BEARING SEAT DETAIL

(BEARING SURFACE MUST BE CLEAN AND

FREE OF ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD)

_	BAR	NO.	SIZE	LEN	GTH	WEIGHT
(5)	А	9	#11	43′	-4"	2072
(5)	В	7	#11	41 ′	-7"	1547
4	D	4	#9	1′-	-8"	23
6	М	4	#5	4′-	-5"	19
6	N	9	#5	7′-	73	
_	S	48	#5	15′	-4"	766
	SS	48	#5	13′	-4"	668
(5)	Т	10	#5	41 ′	-7"	434
_	U	2	#5	10′-	23	
	REINFO	RCING S	LB	5625		
	01.100		(0.0) (110.0)	0.17	07.0	

THE FOR EACH LINEAR FOOT VARIATION IN "H" VALUE, MAKE THE FOLLOWING ADJUSTMENTS:

BARS V LENGTH: 1'-0"

BARS Z LENGTH: 37'-9"

REINFORCING STEEL: 292 LBS

CLASS C CONCRETE (COLUMN) (HPC): 1.42 CY

- (3) QUANTITIES SHOWN ARE FOR ONE BENT ONLY.

TABLE OF COLUMNS QUANTITIES ① ③ CLASS "C" REINE CONC STEEL (HPC) CY LB 2151 7.1

REVISION

3320

12.8

4/28/2022

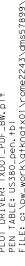
🗯 Texas Department of Transportation Abilene District

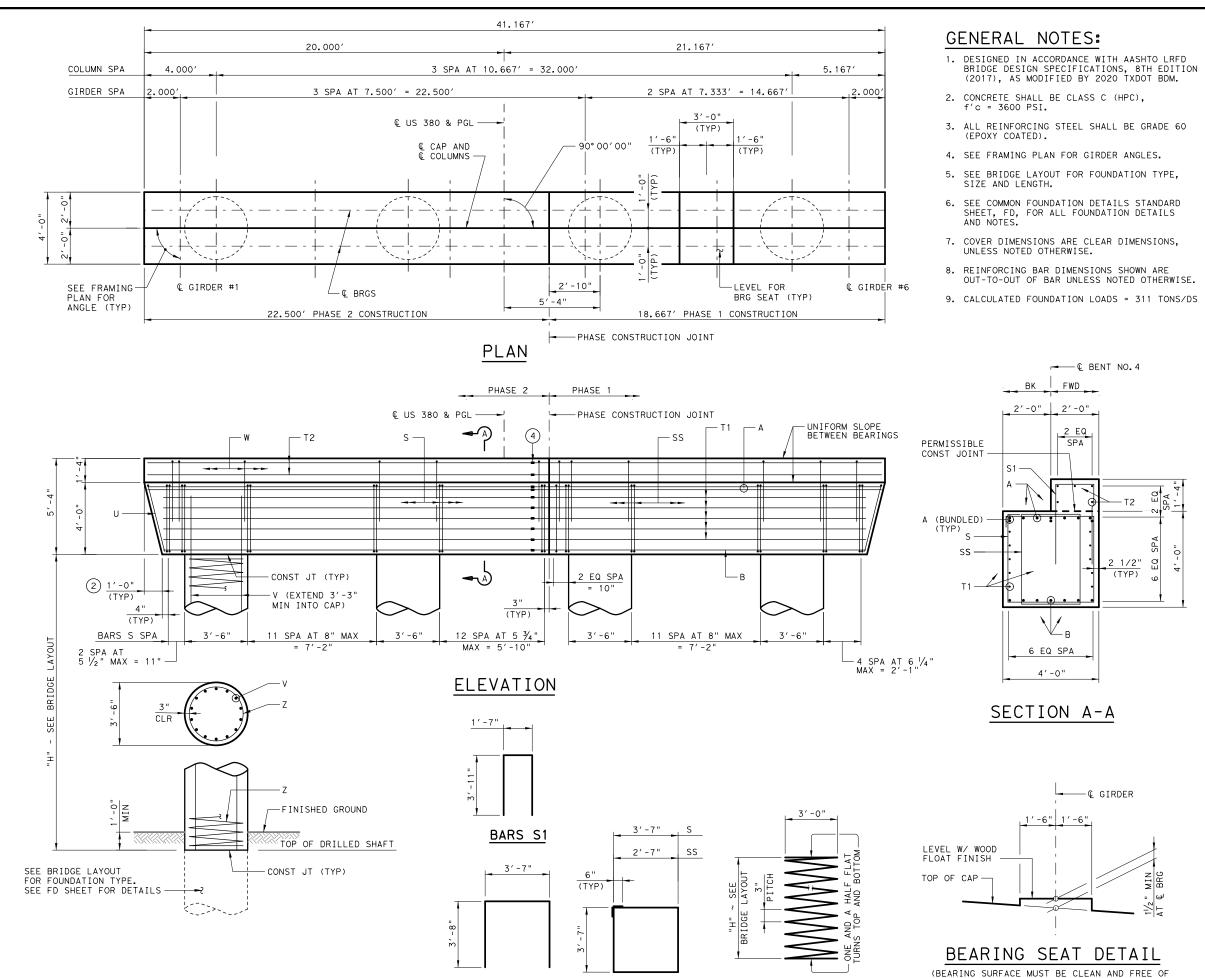
> US 380 INTERIOR BENTS 2 AND 3

SALT FORK OF BRAZOS RIVER BRIDGE

SHEET 1 OF 1 SCALE:  $\frac{3}{16}$ " = 1'-0"

IGNED	:TS	FED. RD DIV. No.	STATE	PROJECT No.			HIGHWAY No.			
CKED:	SK	6	TEXAS	SEE TITLE SHEET				US 380		
wn:	DN	STATE DISTRICT	COUNTY				0B 0.	SHEET No.		
CKED:			STONEWALL	0106	04	036		99		





BARS U

BARS S

# 3 TABLE OF ESTMATED QUANTITIES

	BAR	NO.	SIZE	LENGTH		WEIGHT
4)	А	10	#11	40′-8"		2161
4)	В	7	#11	38′-11'	1	1448
	S	42	#5	15′-4"		672
	SS	42	#5	13′-4"		585
	S1	42	#5	9′-5"	413	
4)	T1	10	#5	38′-11′	406	
	T2	5	#5	40′-8"		213
	С	2	#5	10′-11'	1	23
	٧	56	#9	11′-3"		2142
	Z	4	#4	330′-3'	1	883
	REINFO	DRCING	STEEL		LB	8946
	CLASS	"C" CO	NCRETE	(CAP) (HPC)	CY	30.7
	CLASS	"C" CO	NCRETE	(COL)(HPC)	CY	11.4

- 1 BASED ON "H" VALUE SHOWN ON BRIDGE LAYOUT.
  FOR EACH LINEAR FOOT VARIATION IN
  "H" VALUE, MAKE THE FOLLOWING ADJUSTMENTS:
  BARS V LENGTH: 1'-0"
  BARS Z LENGTH: 37'-9"
  REINFORCING STEEL: 292 LBS
  CLASS C CONCRETE (COLUMN) (HPC): 1.42 CY
- (2) MEASURED PARALLEL TO TOP OF CAP CROSS-SLOPE.
- (3) QUANTITIES SHOWN ARE FOR ONE BENT ONLY.
- 4 BARS T2 SHALL BE SPLICED WITH A MECHANICAL COUPLER ONLY. BARS A, T1, AND B SHALL BE EXTENDED 1'-0" MIN FOR MECHANICAL COUPLERS (SHOWN) OR PROVIDE BAR LAPS AS FOLLOWS:

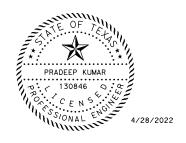
  #5 = 2'-9"

WHERE BAR LAPS ARE PROVIDED IN LIEU OF COUPLERS, MAKE THE FOLLOWING ADJUSTMENTS FOR THE ADDITIONAL STEEL REQUIRED:

BARS A & B LENGTH: 10'-3"

BARS TI LENGTH: 2'-9"

REINFORCING STEEL: 791 LBS



HL93 LOADING

REV.No. DATE REVISION BY

# **NIKINS**



US 380

### INTERIOR BENT 4

# SALT FORK OF BRAZOS RIVER BRIDGE

ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD)

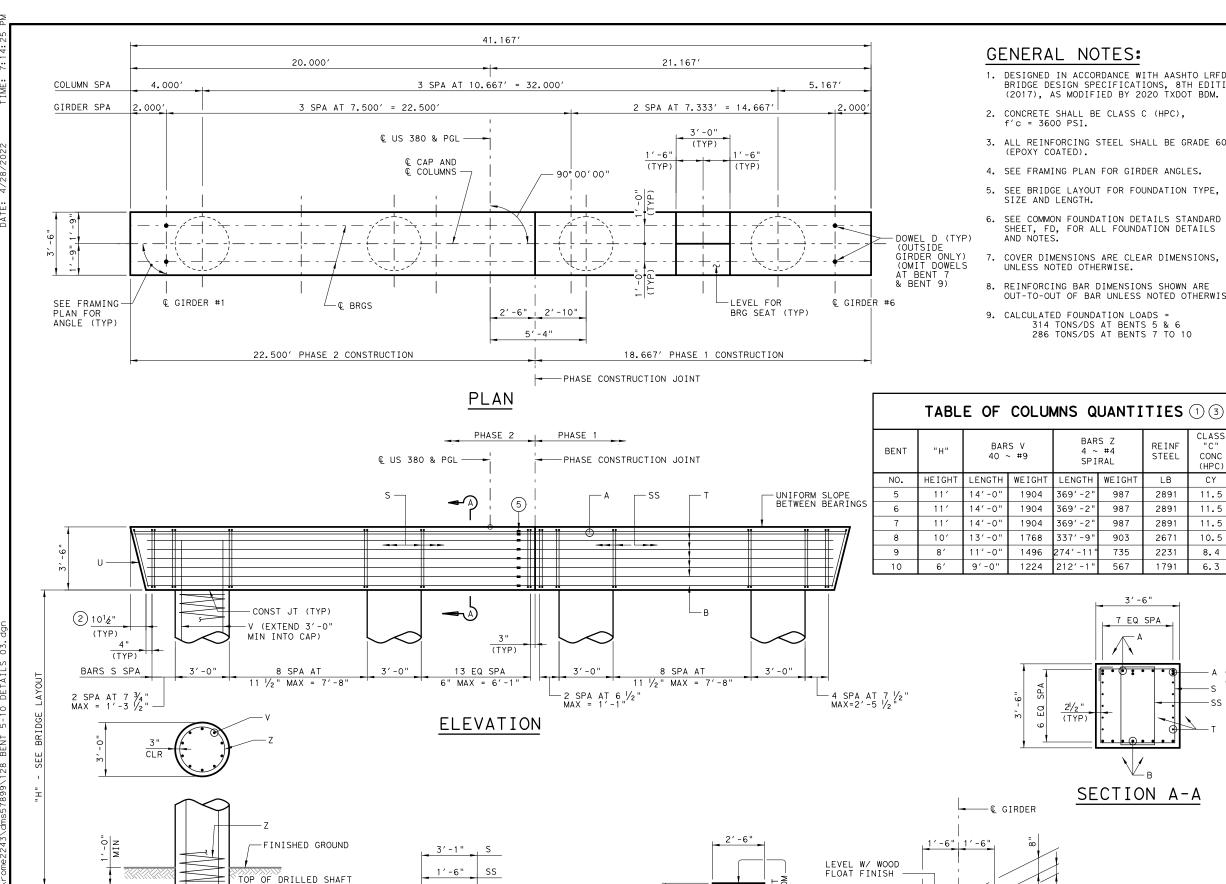
BARS Z

CALE:	ALE: %6 = 1'-0" SHEET TOF T											
SIGNED: <b>TS</b>	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.					
ECKED: SK	6	TEXAS :	SEE TIT	LE SHEE	ΞT	U	S 380					
AWN: DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.					
ECKED: TS	ABL	STONEWALL	0106	04	0	36	100					



SEE BRIDGE LAYOUT FOR FOUNDATION TYPE.

SEE FD SHEET FOR DETAILS -



3'-1"

BARS U

BARS Z

(TYP)

BARS S

- CONST JT

### GENERAL NOTES:

- 1. DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.
- 2. CONCRETE SHALL BE CLASS C (HPC). f'c = 3600 PSI.
- 3. ALL REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).
- 4. SEE FRAMING PLAN FOR GIRDER ANGLES.
- 5. SEE BRIDGE LAYOUT FOR FOUNDATION TYPE, SIZE AND LENGTH.
- 6. SEE COMMON FOUNDATION DETAILS STANDARD SHEET, FD, FOR ALL FOUNDATION DETAILS AND NOTES.
- 7. COVER DIMENSIONS ARE CLEAR DIMENSIONS, UNLESS NOTED OTHERWISE.
- 8. REINFORCING BAR DIMENSIONS SHOWN ARE OUT-TO-OUT OF BAR UNLESS NOTED OTHERWISE.
- 9. CALCULATED FOUNDATION LOADS = 314 TONS/DS AT BENTS 5 & 6 286 TONS/DS AT BENTS 7 TO 10

BARS Z

SPIRAL

WEIGHT

987

987

987

903

735

567

3'-6"

7 EQ SPA

LENGTH

369′-2

<del>369′-</del>2

369'-2

337'-9

274'-11

212'-1

1904

1904

1904

1768

1496

1224

ğ

### **3 TABLE OF ESTMATED** QUANTITIES

N I						
` _	BAR	NO.	SIZE	LENGTH	1	WEIGHT
(5)	А	11	#11	40′-8'	'	2377
(5)	В	8	#11	38′-11"		1655
$\overline{(4)}$	D	4	4 #9 1′-8"		23	
_	S	43	#5	13′-4"		598
	SS	43	#5	#5 10'-2"		456
(5)	Т	10	#5	38′-11	ш	406
	U	2	#5	9′-5"		20
	REINFO	5535				
	CLASS	19.0				

T FOR EACH LINEAR FOOT VARIATION IN

"H" VALUE, MAKE THE FOLLOWING ADJUSTMENTS:

BARS V LENGTH: 1'-0"

BARS Z LENGTH: 31'-5"

REINFORCING STEEL: 220 LBS

CLASS C CONCRETE (COLUMN) (HPC): 1.05 CY

- (2) MEASURED PARALLEL TO TOP OF CAP CROSS-SLOPE.
- (3) QUANTITIES SHOWN ARE FOR ONE BENT ONLY.
- 4 DEDUCT 23 LBS OF REINFORCING STEEL AT BENT 7 AND BENT 9.
- (S) HORIZONTAL BARS IN BENTS 5-9 SHALL BE SPLICED WITH A MECHANICAL COUPLER ONLY. HORIZONTAL BARS IN BENT 10 MAY BE SPLICED WITH A MECHANICAL COUPLER OR BAR LAPS. EXTEND BARS 1'-0" MIN FOR MECHANICAL COUPLERS (SHOWN) OR PROVIDE BAR LAPS

  (AT BENT 10 ONLY) AS FOLLOWS:

  #5 = 2'-9"

  #11 = 10'-3"
  - #5 = 2'-9"
    #11 = 10'-3"
    AT BENT 10, WHERE BAR LAPS ARE PROVIDED IN
    LIEU OF COUPLERS, MAKE THE FOLLOWING
    ADJUSTMENTS FOR THE ADDITIONAL STEEL REQUIRED:
    BARS A & B LENGTH: 10'-3"
    BARS T LENGTH: 2'-9"
    REINFORCING STEEL: 900 LBS



HL93 LOADING DATE REVISION



US 380 INTERIOR BENTS 5 TO 10

#### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: 3/16 " = 1'-0" SHEET 1 OF 1

DESIGNED: <b>TS</b> FED. RD DIV. No.		STATE	PROJECT No.			HIGHWAY No.			
CHECKED:	SK	6	TEXAS		SEE TIT	LE SHEE	T	U	S 380
DRAWN:	DN	STATE DISTRICT	COUNTY	Y	CONTROL No.	SECTION No.		OB 0.	SHEET No.
CHECKED:	TS	ABL	STONEWA	4LL	0106	04	0	36	101

SECTION A-A

CLASS

CONC

(HPC)

CY

11.5

11.5

11.5

10.5

8.4

6.3

(BUNDLED)

(TYP)

RETNE

STEEL

LB

2891

2891

2891

2671

2231

1791

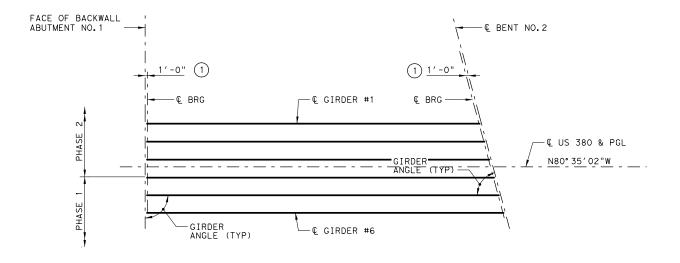
TOP OF CAP

DOWEL D ~ GALVANIZED (#9) x 1'-8" AT OUTSIDE GIRDERS ONLY (OMIT DOWELS AT BENT 7 & BENT 9)

## BEARING SEAT DETAIL

(BEARING SURFACE MUST BE CLEAN AND FREE OF ALL LOOSE MATERIAL BEFORE PLACING BEARING PAD)





SPAN 1 (TX70 GIRDERS)

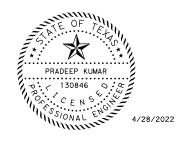
- 1) SEE ELASTOMERIC BEARING AND GIRDER END DETAILS (IGEB) STANDARD SHEET FOR ORIENTATION OF DIMENSIONS.
- (2) GIRDER LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.

# BENT REPORT

## GIRDER REPORT

DISTANCE	BENT BETWEEN  1 BEAM BEAM BEAM BEAM TOTA	STATION BEAI (C.L 1 2 3 4 5 6	LINE AND M SPAC.	57.61 E) BEAM 1, BEAM D M 90 0 90 0 90 0 90 0 90 0	ANGLE S O O O O O	18.000 L
DISTANCE	BENT BETWEEN 1 BEAM BEAM BEAM BEAM BEAM TOTA	STATION BEAI (C.L 1 2 3 4 5 6		2.39 W) BEAM 1, BEAM 0 75 0 75 0 75 0 75 0 75 0	ANGLE S O O O O O	18.635 L

		BEAM REPORT HORIZONTAL C-C BENT		TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM	1	140.177	138.177	139.68 (2)	0.0075
BEAM	2	142.187	140.187	141.69	0.0075
BEAM	3	144.196	142.196	143.70	0.0075
BEAM	4	146.206	144.206	145.71	0.0075
BEAM	5	148.171	146.171	147.67	0.0075
BEAM	6	150.136	148.136	149.64	0.0075



HL93 LOADING

REV. No.	DATE	REVISION	BY





US 380 FRAMING PLAN (SPAN 1)

### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: 1	I" = 4	0'-0"	SH	EET 1 OF 5
SIGNED: <b>TS</b>	FED. RD DIV. No.	STATE	PROJECT No.	HIGHWAY No.
ECKED: SK	6	TEXAS	SEE TITLE SHEET	US 380

RAWN: DN STATE COUNTY CONTROL SECTION NO. NO. HECKED: TS ABL STONEWALL 0106 04

SPAN 2 (TX70 GIRDERS)

18.635 L

18.000 L

SPAN 3 (TX70 GIRDERS)

- 1) SEE ELASTOMERIC BEARING AND GIRDER END DETAILS (IGEB) STANDARD SHEET FOR ORIENTATION OF DIMENSIONS.
- (2) GIRDER LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.

## BENT REPORT

# BENT NO. 2 (N 5 35 2.39 W) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C. L. BENT) D M S SPAN 2 BEAM 1 0.000 75 0 0 BEAM 2 7.765 75 0 0 BEAM 3 7.765 75 0 0 BEAM 4 7.765 75 0 0 BEAM 4 7.765 75 0 0 BEAM 5 7.592 75 0 0 BEAM 6 7.593 75 0 0 TOTAL 38.478 SPAN 2 BEAM BEAM BEAM BEAM BEAM TOTAL BENT NO. 3 (N 5 35 2.39 W) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C. L. BENT) D M S SPAN 2 BEAM 1 0000 75 0 0 BEAM 2 7.765 75 0 0 BEAM 3 7.765 75 0 0 BEAM 4 7.765 75 0 0 BEAM 4 7.765 75 0 0 BEAM 5 7.592 75 0 0 BEAM 6 7.593 75 0 0 TOTAL 38.478 18.635 L TOTAL 0.000 7.765 7.765 7.765 7.592 7.593 38.478 BEAM BEAM BEAM BEAM 75 0 75 0 75 0 75 0 75 0 75 0 SPAN 3 BEAM BEAM TOTAL BENT NO. 4 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C.L. BENT) D M S SPAN 3 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 TOTAL 37.167

SPAN 3 BEAM BEAM BEAM BEAM BEAM TOTAL

### GIRDER REPORT

			RT, SPAN 2 _ DISTANCE C-C BRG.	TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM	1	150.000	148.000	149.50 (2)	0.0075
BEAM	2	150.000	148.000	149.50	0.0075
BEAM	3	150.000	148.000	149.50	0.0075
BEAM	4	150.000	148.000	149.50	0.0075
BEAM	5	150.000	148.000	149.50	0.0075
BEAM	6	150.000	148.000	149.50	0.0075
			RT, SPAN 3 _ DISTANCE C-C BRG.	TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM	1	149.823	147.823	149.33 (2)	0.0075
BEAM	2	147.813	145.813	147.32	0.0075
BEAM	3	145.804	143.804	145.31	0.0075
BEAM	4	143.794	141.794	143.30	0.0075
BEAM	5	141.829	139.829	141.33	0.0075
BEAM	6	139.864	137.864	139.37	0.0075



HL93 LOADING

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REV. No.	DATE	REVISION	BY

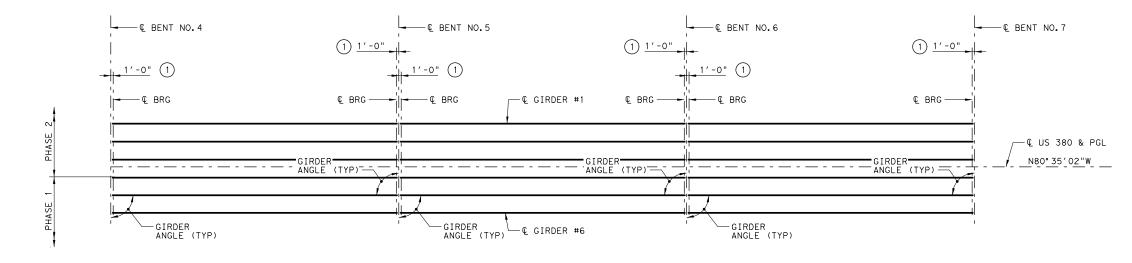
**ATKINS** 



US 380 FRAMING PLAN (SPANS 2-3)

#### SALT FORK OF BRAZOS RIVER BRIDGE

	. •	•							
SCALE:	l" = 4	0'-0"				SH	EET	2 OF	5
DESIGNED: <b>TS</b>	FED. RD DIV. No.	STATE		PROJE	CT No.		HIG	HWAY No.	
CHECKED: SK	6	TEXAS	9,	SEE TIT	LE SHE	ΞT	U	S 380	
DRAWN: <b>DN</b>	STATE DISTRICT	COUNT	Υ	CONTROL No.	SECTION No.		OB 0.	SHEET No.	



SPAN 4 (TX54 GIRDERS)

SPAN 5 (TX54 GIRDERS)

SPAN 6 (TX54 GIRDERS)

## BENT REPORT

0.000 7.500 7.500 7.500 7.333 7.334 37.167

BENT NO. 7 (N 9 24 57.61 E)

DISTANCE BETWEEN STATION LINE AND BEAM 1,

BEAM SPAC.

(C.L. BENT) D M S

(C.L. BENT) D M S

BEAM 1 0.000 90 0 0

BEAM 2 7.500 90 0 0

BEAM 3 7.500 90 0 0

BEAM 4 7.500 90 0 0

BEAM 4 7.500 90 0 0

BEAM 5 7.333 90 0 0

BEAM 6 7.334 90 0 0

TOTAL 37.167

18.000 L

BEAM BEAM BEAM BEAM

BEAM BEAM TOTAL

## GIRDER REPORT

BENT NO. 4 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1,	18.000 L		BEAM REPORT HORIZONTAL	DISTANCE	TRUE DISTANCE	ВЕАМ
BEAM SPAC. BEAM ANGLE (C.L. BENT) D M S SPAN 4 BEAM 1 0000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0		BEAM 1 BEAM 2 BEAM 3 BEAM 4	C-C BENT 120.000 120.000 120.000 120.000	C-C BRG.  118.000  118.000  118.000  118.000	BOT. BM. FLG.  119.50  119.50  119.50  119.50	SLOPE 0.0055 0.0055 0.0055 0.0055
BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 TOTAL 37.167		BEAM 5 BEAM 6	120.000 120.000 BEAM REPORT	118.000 118.000	119.50 119.50	0.0055 0.0055
BENT NO. 5 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. BEAM ANGLE	18.000 L		HORIZONTAL C-C BENT	ĎISTANCE C-C BRG.	TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
SPAN 4 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0		BEAM 1 BEAM 2 BEAM 3 BEAM 4 BEAM 5 BEAM 6	120.000 120.000 120.000 120.000 120.000 120.000	118.000 118.000 118.000 118.000 118.000 118.000	119.50 119.50 119.50 119.50 119.50 119.50	0.0015 0.0015 0.0015 0.0015 0.0015 0.0015
TOTAL 37.167  SPAN 5 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0			BEAM REPORT HORIZONTAL C-C BENT	, SPAN 6 DISTANCE C-C BRG.	TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM 3 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0		BEAM 1 BEAM 2 BEAM 3 BEAM 4 BEAM 5 BEAM 6	120.000 120.000 120.000 120.000 120.000 120.000	118.000 118.000 118.000 118.000 118.000		-0.0025 -0.0025 -0.0025 -0.0025 -0.0025 -0.0025
BENT NO. 6 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. BEAM ANGLE	18.000 L	DEAW 0	120.000	110.000	113.30	0.0023
SPAN 5 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 BEAM 6 7.334 90 0 0						

- 1) SEE ELASTOMERIC BEARING AND GIRDER END DETAILS (IGEB) STANDARD SHEET FOR ORIENTATION OF DIMENSIONS.
- (2) GIRDER LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.



LILOZ LOADING

REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380 FRAMING PLAN (SPANS 4-6)

### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE:	" = 4	0'-0"	SH	EET 3 OF 5
DESIGNED:TS	FED. RD DIV. No.	STATE	PROJECT No.	HIGHWAY No.
CHECKED. CK	6	TEVAC	SEE TITLE SHEET	115 300

DRAWN: DN STATE COUNTY CONTROL SECTION NO. No. HECKED: TS ABL STONEWALL 0106 04

SPAN 7

SPAN 8 (TX54 GIRDERS)

- 1) SEE ELASTOMERIC BEARING AND GIRDER END DETAILS (IGEB) STANDARD SHEET FOR ORIENTATION OF DIMENSIONS.
- (2) GIRDER LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.

# BENT REPORT

# GIRDER REPORT

— € BENT NO.9

-€ US 380 & PGL

N80° 35′02"W\_

DISTANCE	7 BEAM BEAM BEAM BEAM BEAM BEAM TOTA	BEA (C.L 1 2 3 4 5 6	(N 9 24 LINE AND M SPAC. BENT) 0.000 7.500 7.500 7.500 7.500 7.333 7.334 7.167		E) M AN M O O O O O	GLE S 0 0 0 0	18.000 L
DISTANCE	BENT BETWEEN	STATION	LINE AND M SPAC.		E) 1, M AN M		18.000 L
SPAN	7 BEAM BEAM BEAM BEAM BEAM TOTA	1 2 3 4 5 6	7.500 7.500 7.500 7.500 7.333 7.334	90 90 90 90 90	00000	S 0 0 0 0 0 0	
SPAN	8 BEAM BEAM BEAM BEAM BEAM TOTA	2 3 4 5 6	0.000 7.500 7.500 7.500 7.333 7.334	90 90 90 90 90	0 0 0 0 0	0 0 0 0 0	
DISTANCE	BENT BETWEEN	STATION BEA	(N 9 24 LINE AND M SPAC.	BEAM BEAI	E) 1, M.AN		18.000 L
SPAN	8 BEAM BEAM BEAM BEAM BEAM TOTA	1 2 3 4 5 6	BENT) 0.000 7.500 7.500 7.500 7.333 7.334	90 90 90 90 90	M 0 0 0 0 0 0	S 0 0 0 0 0 0	

		BEAM REPOR HORIZONTAL C-C BENT		TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM BEAM BEAM BEAM BEAM BEAM	1 2 3 4 5 6	105.000 105.000 105.000 105.000 105.000 105.000	103.000 103.000 103.000 103.000 103.000 103.000	104.50 104.50 104.50 104.50 104.50 104.50	-0.0045 -0.0045 -0.0045 -0.0045 -0.0045 -0.0045
		BEAM REPOR HORIZONTAL C-C BENT		TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM BEAM BEAM BEAM BEAM BEAM	1 2 3 4 5 6	105.000 105.000 105.000 105.000 105.000 105.000	103.000 103.000 103.000 103.000 103.000 103.000	104.50 104.50 104.50 104.50 104.50 104.50	-0.0045 -0.0045 -0.0045 -0.0045 -0.0045



HL93 LOADING

REV. No.	DATE	REVISION	BY

# **ATKINS**



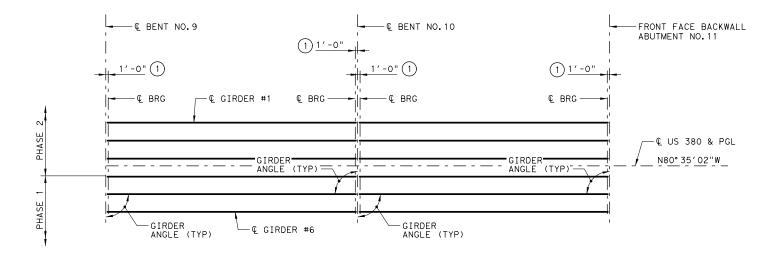
US 380 FRAMING PLAN (SPANS 7-8)

#### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: 1" = 40'-0" SHEET 4 OF 5									5	
DESIGNED: TS FED. RD STATE					PROJECT No. HIGHWAY No				SHWAY No.	
CHECKED:	SK	6	TEXAS		SEE TIT	LE SHE	ΞT	U	S 380	
DRAWN:	DN	STATE	COUNT	Y	CONTROL	SECTION	J	ОВ	SHEET	

CHECKED: TS ABL STONEWALL 0106 04 036 105





SPAN 9

18.000 L

SPAN 10

# BENT REPORT

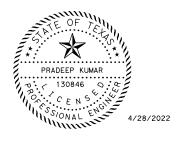
# BENT NO. 9 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C.L. BENT) D M S SPAN 9 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 TOTAL 37.167 BENT NO. 10 (N 9 24 57.61 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C.L. BENT) D M S SPAN 9 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 TOTAL 37.167 18.000 L 90 0 90 0 90 0 90 0 90 0 SPAN 10 BEAM BEAM BEAM BEAM BEAM BENT NO. 11 (N 9 24 57.63 E) DISTANCE BETWEEN STATION LINE AND BEAM 1, BEAM SPAC. (C.L. BENT) D M S SPAN 10 BEAM 1 0.000 90 0 0 BEAM 2 7.500 90 0 0 BEAM 3 7.500 90 0 0 BEAM 4 7.500 90 0 0 BEAM 5 7.333 90 0 0 BEAM 6 7.334 90 0 0 TOTAL 37.167 18.000 L

### GIRDER REPORT

		BEAM REPOR HORIZONTAL C-C BENT		TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM	1	105.000	103.000	104.50	-0.0045
BEAM	2	105.000	103.000	104.50	-0.0045
BEAM	3	105.000	103.000	104.50	-0.0045
BEAM	4	105.000	103.000	104.50	-0.0045
BEAM	5	105.000	103.000	104.50	-0.0045
BEAM	6	105.000	103.000	104.50	-0.0045
		BEAM REPOR HORIZONTAL C-C BENT		TRUE DISTANCE BOT. BM. FLG.	BEAM SLOPE
BEAM	1	105.000	103.000	104.50	-0.0027
BEAM	2	105.000	103.000	104.50	-0.0027
BEAM	3	105.000	103.000	104.50	-0.0027
BEAM	4	105.000	103.000	104.50	-0.0027
BEAM	5	105.000	103.000	104.50	-0.0027
BEAM	6	105.000	103.000	104.50	-0.0027



(2) GIRDER LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.



HL93 LOADING

REV. No.	DATE	REVISION	BY

# **ATKINS**

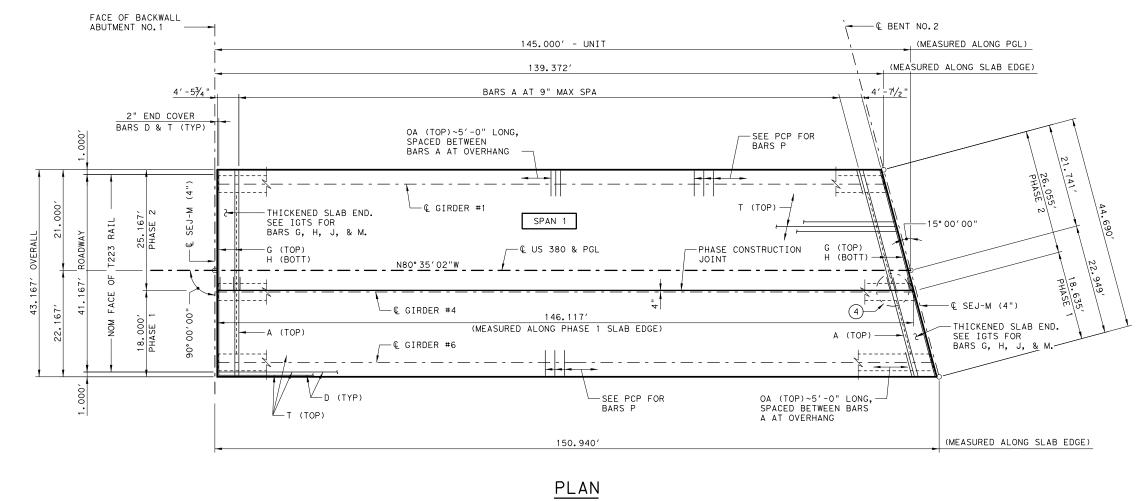


US 380 FRAMING PLAN (SPANS 9-10)

### SALT FORK OF BRAZOS RIVER BRIDGE

DRAWN: DN	STATE	COLINIT	v .	CONTROL	SECTION	J	ов	SHEET	
CHECKED: SK	6	TEXAS		SEE TITLE SHEET			US 380		
DESIGNED: <b>TS</b>	FED. RD DIV. No.		PROJECT No.			HIGHWAY No.			
SCALE: 1" = 40'-0" SHEET 5 OF 5									

HECKED: TS ABL STONEWALL 0106 04 036 106



**GENERAL NOTES:** 

DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.

CONCRETE SHALL BE CLASS S (HPC), f'c = 4000 PSI.

PROVIDE GFRP BARS, CONFORMING TO ASTM D7957/7957M, EXCEPT PROVIDE A MINIMUM MODULUS OF ELASTICITY OF 7500 KSI.

PROVIDE GFRP BARS FOR TOP MAT ONLY. ALL OTHER REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).

SEE RAIL STANDARD FOR ANCHORAGE IN SLAB.

SEE GFRP SLAB (IGFRP) STANDARD FOR ADDITIONAL NOTES AND TOP MAT OF SLAB REINFORCEMENT AT THE THICKENED SLAB END.

SEE PCP AND PCP-FAB FOR PANEL DETAILS NOT SHOWN.

SEE PCP(O) AND PCP(O)-FAB FOR PRECAST OVERHANG PANEL DETAILS IF THIS OPTION IS USED.

SEE IGMS STANDARD FOR MISCELLANEOUS DETAILS.

SEE PMDF STANDARD FOR DETAILS AND QUANTITY ADJUSTMENTS IF THIS OPTION IS USED.

BAR LAPS, WHERE REQUIRED SHALL BE AS FOLLOWS: EPOXY COATED  $\sim$  #4 = 2'-5" GFRP  $\sim$  #5 = 2'-9"

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.



HL93 LOADING

REV. No.	DATE	REVISION	BY

**ATKINS** 



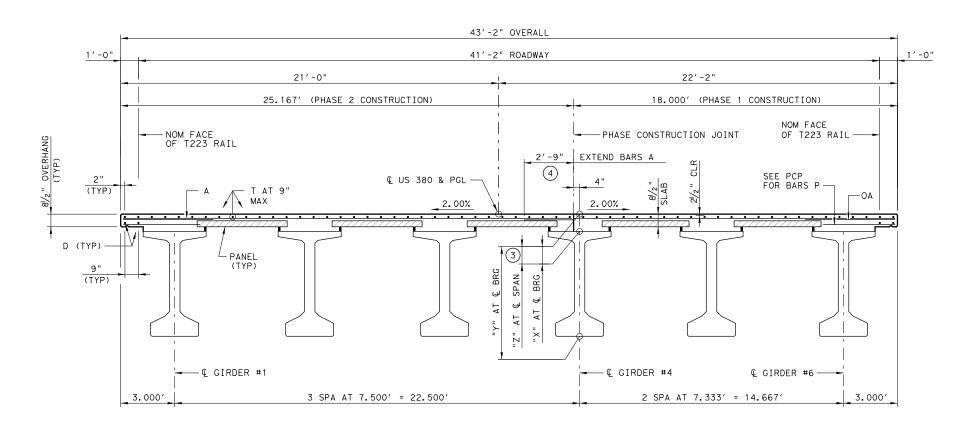
US 380 145.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: NONE

SHEET 1 OF 2

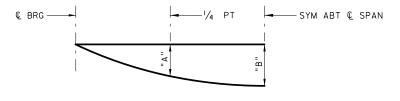
SIGNED: <b>TS</b>	FED. RD DIV. No.	STATE	PROJECT No.			HIG	HWAY No.
ECKED: <b>SK</b>	6	TEXAS :	SEE TIT	LE SHE	ΞT	U	S 380
AWN: DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		о.	SHEET No.
ECKED: TS		STONEWALL	0106	04	0	36	107



### TYPICAL TRANSVERSE SECTION

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.

TABLE OF DEFLECTIONS							
SPAN NO.	GIRDER NO.	"A"	"B"				
SPAN NO.	GIRDER NO.	FT	FT				
	1	0.110	0.156				
	2	0.129	0.184				
	3	0.137	0.194				
1	4	0.077	0.110				
	5	0.149	0.212				
	6	0.143	0.203				



### DEAD LOAD **DEFLECTION DIAGRAM**

DEFLECTIONS SHOWN ARE DUE TO CONCRETE SLAB ONLY. (EG = 5,000 KSI). CALCULATED DEFLECTIONS SHOWN ARE THEORETICAL AND ACTUAL DEFLECTIONS MAY BE LESS. ADJUST VALUES AS REQUIRED IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERIFICATION.

TABLE OF SECTION DEPTHS							
SPAN NO.	GIRDER NO.	"X" AT & BRG	"Y" AT & BRG	"Z" AT & SPAN 3			
	1	12 1/2 "	6'-10 1/2"	10"			
	2	12 1/2 "	6'-10 1/2"	10 1/4 "			
1	3	12 1/2 "	6'-10 1/2"	10 3/8"			
ı	4	12 1/2 "	6'-10 1/2"	9 5/8 "			
	5	12 1/2 "	6′-10 ½"	10 %"			
	6	12 1/2 "	6'-10 1/2"	10 %"			

(3) THEORETICAL DIMENSION

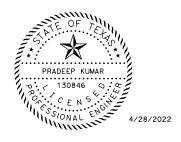
BAR	TABLE
BAR	SIZE
Α	#5
D	#4
G	#5
Н	#4
J	#4
М	#4
OA	#5
Ρ	#4

	TABLE OF ESTIMATED QUANTITIES							
	SPAN	REINFORCED CONCRETE SLAB	PRESTR CONCRETE GIRDER (TX70)	REINF STEEL 1				
┨	NO.	SF	LF	LB				
1	1	6260	868.09	14398				
1	TOTAL	6260	868.09	14398				

- 1) REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.
- 2 LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.

### NOTES:

- FOR GENERAL NOTES SEE 145.000' PRESTRESSED CONCRETE I-GIRDER UNIT SHEET 1 OF 2.
- 2. THE HAUNCH DEPTH REQUIRES ADDITIONAL REINFORCING, SEE PCP OR IGMS FOR HAUNCH REINFORCING DETAILS.



HL93 LOADING

REV. No.	DATE	REVISION	BY

**ATKINS** 

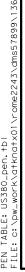


US 380 145.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: 3/16 " = 1'-0" SHEET 2 OF 2

DESIGNED	:TS	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	HWAY No.
CHECKED:	SK	6	TEXAS	SEE TIT	LE SHE	ΞT	U	S 380
DRAWN:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		о.	SHEET No.
CHECKED:			STONEWALL	0106	04	0	36	108



← © BENT NO.4 295.000' - UNIT (MEASURED ALONG PGL) 150.000′ 145.000' (MEASURED ALONG SLAB EDGE) 150.000' 150.627' BARS A AT 9" MAX SPA 4'-53/4" 4'-71/2" 2" END COVER & BENT NO.2-@ BENT NO.3 -BARS D & T (TYP) OA (TOP)~5'-0" LONG, -SEE PCP FOR BARS P SPACED BETWEEN BARS A AT OVERHANG CONST JT OR - C GIRDER #1 T (TOP) SPAN 3 15° 00′ 00' H (BOTT) CONTROLLED JT RAIL -15° 00′ 00" THICKENED SLAB END. -PHASE SPAN 2 SEE IGTS FOR CONSTRUCTION JOINT G (TOP) -Ç US 380 & PGL 1.741 (TYP) BARS G, H, J, & M. H (BOTT) N80° 35′ 02"W R - © GIRDER #4 (4) 150.000′ 143.884 (MEASURED ALONG PHASE 1 SLAD EDGE) (MEASURED ALONG PHASE 1 SLAD EDGE) 22. -A (TOP) A (TOP) -\_\_ € GIRDER #6 22.949' (TYP) € SEJ-M (4") ∠D (TYP) -SEE PCP FOR OA (TOP)~5'-0" LONG, BARS P SPACED BETWEEN BARS T (TOP) A AT OVERHANG (MEASURED ALONG SLAB EDGE) 150.000' 139.060'

#### **GENERAL NOTES:**

DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.

CONCRETE SHALL BE CLASS S (HPC), f'c = 4000 PSI.

PROVIDE GFRP BARS, CONFORMING TO ASTM D7957/7957M, EXCEPT PROVIDE A MINIMUM MODULUS OF ELASTICITY OF 7500 KSI.

PROVIDE GFRP BARS FOR TOP MAT ONLY. ALL OTHER REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).

SEE RAIL STANDARD FOR ANCHORAGE IN SLAB.

SEE GFRP SLAB (IGFRP) STANDARD FOR ADDITIONAL NOTES AND TOP MAT OF SLAB REINFORCEMENT AT THE THICKENED SLAB END.

SEE PCP AND PCP-FAB FOR PANEL DETAILS NOT SHOWN.

SEE PCP(O) AND PCP(O)-FAB FOR PRECAST OVERHANG PANEL DETAILS IF THIS OPTION IS USED.

SEE IGMS STANDARD FOR MISCELLANEOUS DETAILS.

SEE PMDF STANDARD FOR DETAILS AND QUANTITY ADJUSTMENTS IF THIS OPTION IS USED.

BAR LAPS, WHERE REQUIRED SHALL BE AS FOLLOWS: EPOXY COATED ~ #4 = 2'-5" GFRP ~ #5 = 2'-9"

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.



HL93 LOADING

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REV. No.	DATE	REVISION	BY

**ATKINS** 

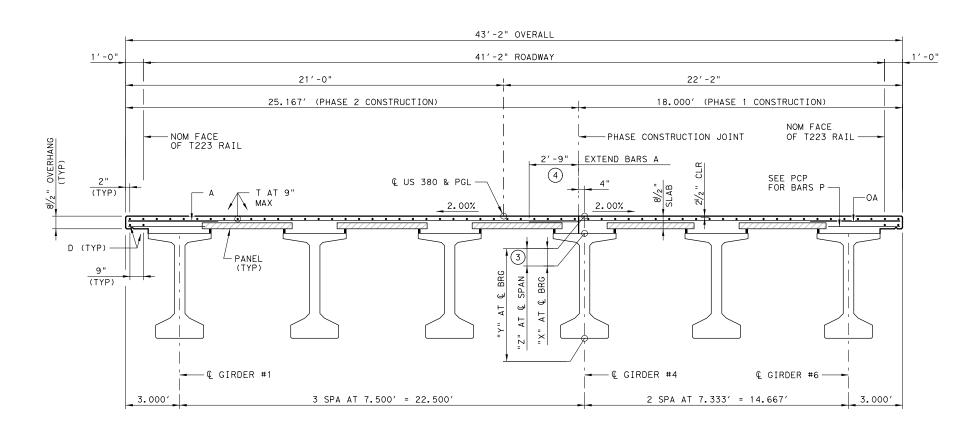


US 380 295.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: NONE SHEET 1 OF 2

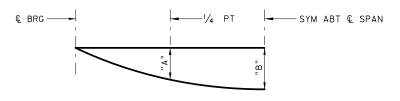
DESIGNED	:12	DIV. No.	STATE		PROJE	.CI NO.		HIL	HWAT NO.	
CHECKED:	SK	6	TEXAS		SEE TIT	LE SHE	ΞT	U	S 380	
DRAWN:	DN	STATE DISTRICT	COUNT	Y	CONTROL No.	SECTION No.		OB 0.	SHEET No.	
CHECKED:	TS	ABL	STONEWA	ALL	0106	04	0	36	109	l



### TYPICAL TRANSVERSE SECTION

(4) EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.

TABLE OF DEFLECTIONS						
SPAN NO.	GIRDER NO.	"A"	"B"			
SPAN NO.	GIRDER NO.	FT	FT			
	1	0.144	0.205			
	2	0.160	0.228			
2	3	0.160	0.228			
	4	0.086	0.112			
	5	0.157	0.223			
	6	0.143	0.203			
	1	0.144	0.204			
	2	0.151	0.215			
3	3	0.143	0.203			
	4	0.072	0.102			
	5	0.125	0.178			
	6	0.107	0.153			



### DEAD LOAD DEFLECTION DIAGRAM

DEFLECTIONS SHOWN ARE DUE TO CONCRETE SLAB ONLY. (Ec = 5,000 KSI). CALCULATED DEFLECTIONS SHOWN ARE THEORETICAL AND ACTUAL DEFLECTIONS MAY BE LESS. ADJUST VALUES AS REQUIRED IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERIFICATION.

TABLE OF SECTION DEPTHS								
SPAN NO.	GIRDER NO.	"X" AT & BRG	"Y" AT & BRG	"Z" AT © SPAN 3				
	1	12 1/2 "	6'-10 1/2"	10 % "				
	2	12 1/2 "	6'-10 1/2"	10 1/8"				
2	3	12 1/2 "	6'-10 1/2"	10 %"				
2	4	12 1/2 "	6'-10 1/2"	9 %"				
	5	12 1/2 "	6'-10 1/2"	10 3/4"				
	6	12 1/2 "	6'-10 1/2"	10 5/8"				
	1	12 3/4"	6'-10 3/4"	10 1/8"				
	2	12 3/4"	6′-10 3/4"	10 1/8"				
3	3	12 ¾"	6'-10 3/4"	10 3/4"				
3	4	12 ¾"	6'-10 3/4"	9 ¾"				
	5	12 ¾"	6′-10 ¾"	10 ½"				
	6	12 ¾"	6′-10 3/4"	10 1/4"				

(3) THEORETICAL DIMENSION

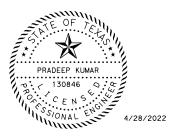
BAR	TABLE
BAR	SIZE
Α	#5
D	#4
G	#5
Н	#4
J	#4
М	#4
OA	#5
Р	#4
Т	#5

_	TABLE OF ESTIMATED QUANTITIES						
	SPAN REINFORCE CONCRETE SLAB		PRESTR CONCRETE GIRDER (TX70)	REINF STEEL 1			
1	NO.	SF	LF	LB			
1	2	6476	897.00	14895			
ł	3	6260	865.96	14398			
1	TOTAL	12390	1762.96	29293			

- 1 REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.
- 2 LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER SLOPE.

### NOTES:

- 1. FOR GENERAL NOTES SEE 295.000' PRESTRESSED CONCRETE I-GIRDER UNIT SHEET 1 OF 2.
- 2. THE HAUNCH DEPTH REQUIRES ADDITIONAL REINFORCING, SEE PCP OR IGMS FOR HAUNCH REINFORCING DETAILS.



HL93 LOADING

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REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380 295.000' PRESTRESSED

SALT FORK OF BRAZOS RIVER BRIDGE

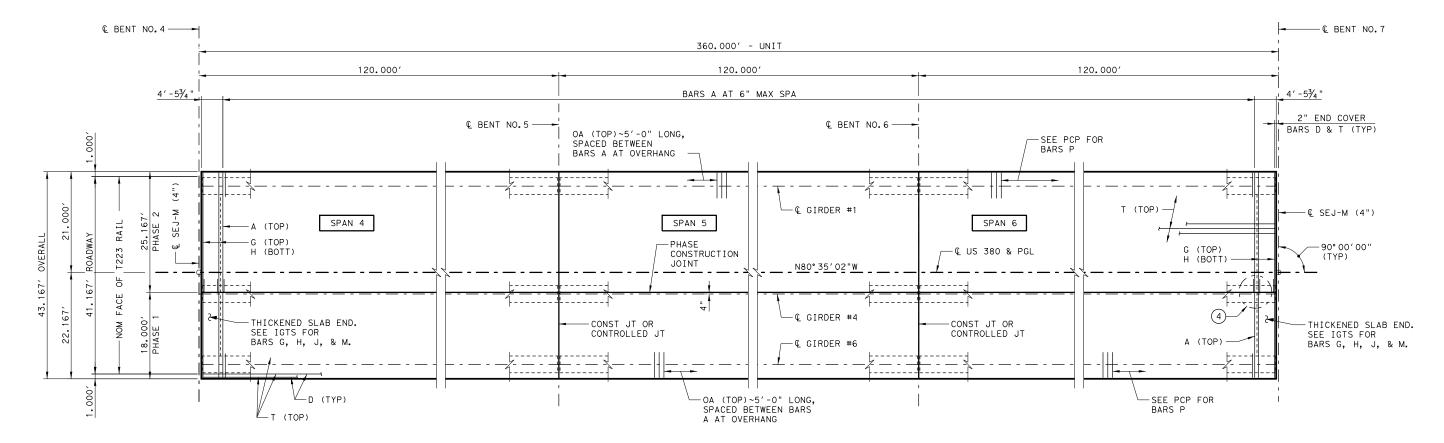
CONCRETE I-GIRDER UNIT

SCALE: 3/16 " = 1'-0"

SHEET 2 OF 2

SIGNED	:TS	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.
ECKED:	SK	6	TEXAS	SEE TIT	LE SHE	ΞT	U	IS 380
AWN:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.
ECKED:			STONEWALL	0106	04	0	36	110





## PLAN

#### **GENERAL NOTES:**

DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.

CONCRETE SHALL BE CLASS S (HPC), f'c = 4000 PSI.

PROVIDE GFRP BARS, CONFORMING TO ASTM D7957/7957M, EXCEPT PROVIDE A MINIMUM MODULUS OF ELASTICITY OF 7500 KSI.

PROVIDE GFRP BARS FOR TOP MAT ONLY. ALL OTHER REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).

SEE RAIL STANDARD FOR ANCHORAGE IN SLAB.

SEE GFRP SLAB (IGFRP) STANDARD FOR ADDITIONAL NOTES AND TOP MAT OF SLAB REINFORCEMENT AT THE THICKENED SLAB END.

SEE PCP AND PCP-FAB FOR PANEL DETAILS NOT SHOWN.

SEE PCP(O) AND PCP(O)-FAB FOR PRECAST OVERHANG PANEL DETAILS IF THIS OPTION IS USED.

SEE IGMS STANDARD FOR MISCELLANEOUS DETAILS.

SEE PMDF STANDARD FOR DETAILS AND QUANTITY ADJUSTMENTS IF THIS OPTION IS USED.

BAR LAPS, WHERE REQUIRED SHALL BE AS FOLLOWS: EPOXY COATED ~ #4 = 2'-5" GFRP ~ #5 = 2'-9"

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.



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REV. No.	DATE	REVISION	

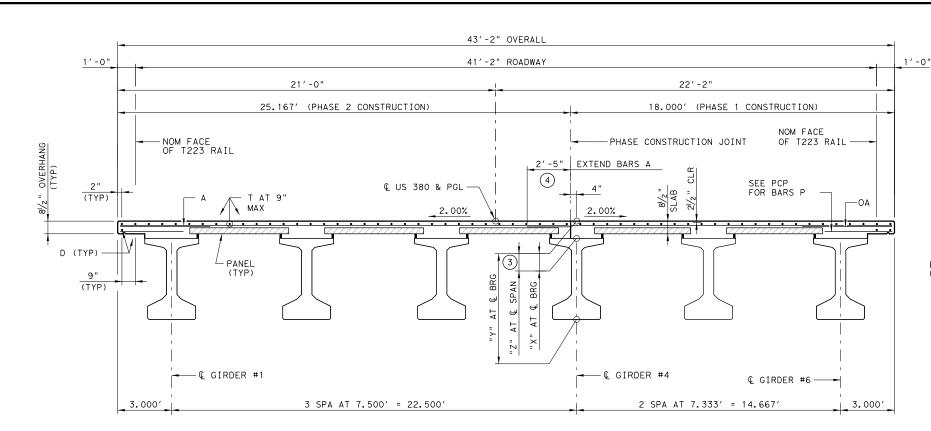
# **ATKINS**



US 380 360.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE SCALE: NONE SHEET 1 OF 2

IGNED	:TS	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	HWAY No.
CKED:	SK	6	TEXAS S	SEE TIT	LE SHE	Ξ	U	S 380
wn:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		о.	SHEET No.
CKED:			STONEWALL	0106	04	0	36	111



BAR TABLE BAR SIZE #5 #4 D G #5 #4 #4 М #4 OΑ #5 #4 #5

	TABLE OF ESTIMATED QUANTITIES						
SPAN		REINFORCED CONCRETE SLAB	PRESTR CONCRETE GIRDER (TX54)	REINF STEEL 1			
	NO.	SF	LF	LB			
	4	5180	717.00	11914			
	5	5180	717.00	11914			
	6	5180	717.00	11914			
	TOTAL	15540	2151.00	35742			

- 1 REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.
- (2) LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER

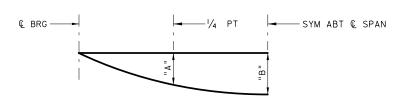
(X) END BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.

### NOTES:

1. FOR GENERAL NOTES SEE 360.000' PRESTRESSED CONCRETE I-GIRDER UNIT SHEET 1 OF 2.

### TYPICAL TRANSVERSE SECTION

TABLE OF DEFLECTIONS						
SPAN NO.	GIRDER NO.	"A"	"B"			
SPAN NO.	GIRDER NO.	FT	FT			
	1	0.122	0.174			
	2	0.136	0.193			
4-6	3	0.136	0.193			
4-6	4	0.073	0.103			
	5	0.133	0.189			
	6	0.121	0.172			



### DEAD LOAD DEFLECTION DIAGRAM

DEFLECTIONS SHOWN ARE DUE TO CONCRETE SLAB ONLY. (Ec = 5,000 KSI). CALCULATED DEFLECTIONS SHOWN ARE THEORETICAL AND ACTUAL DEFLECTIONS MAY BE LESS. ADJUST VALUES AS REQUIRED IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERIFICATION.

TABLE OF SECTION DEPTHS							
SPAN NO.	GIRDER NO.	"X" AT & BRG	"Y" AT & BRG	"Z" AT © SPAN 3			
	1	11 1/2"	5'-5 1/2"	10 1/4"			
	2	11 1/2 "	5'-5 1/2"	10 1/2 "			
4-6	3	11 1/2 "	5'-5 1/2"	10 1/2 "			
4-6	4	11 1/2 "	5'-5 1/2"	9 5/8"			
	5	11 1/2 "	5'-5 1/2"	10 3/8"			
	6	11 1/2 "	5′-5 1/2"	10 1/4"			

(3) THEORETICAL DIMENSION



HL93 LOADING

11233	LOADING		
REV. No.	DATE	REVISION	BY

# **ATKINS**

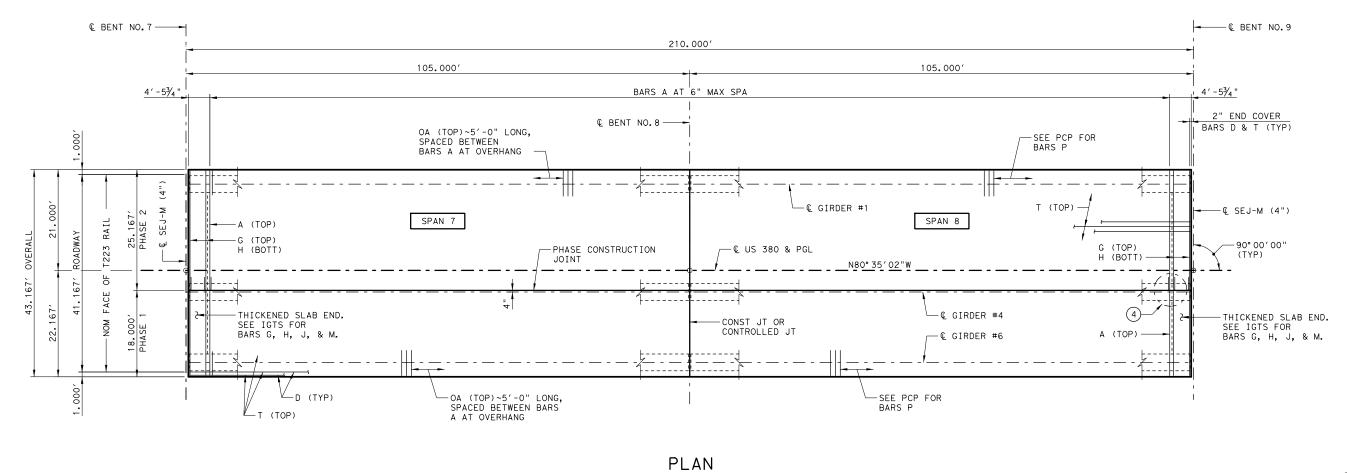


US 380 360.000' PRESTRESSED CONCRETE I-GIRDER UNIT

### SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: 3/16 " = 1'-0" SHEET 2 OF 2

ESIGNED:	TS	FED. RD DIV. No.	STATE	PROJE	CT No.		ніс	SHWAY No.
HECKED:	SK	6	TEXAS	SEE TIT	LE SHE	Τ	U	S 380
RAWN:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.
HECKED:	TS	ABL	STONEWALL	0106	04	0	36	112



#### **GENERAL NOTES:**

DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.

CONCRETE SHALL BE CLASS S (HPC), f'c = 4000 PSI.

PROVIDE GFRP BARS, CONFORMING TO ASTM D7957/7957M, EXCEPT PROVIDE A MINIMUM MODULUS OF ELASTICITY OF 7500 KSI.

PROVIDE GFRP BARS FOR TOP MAT ONLY. ALL OTHER REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).

SEE RAIL STANDARD FOR ANCHORAGE IN SLAB.

SEE GFRP SLAB (IGFRP) STANDARD FOR ADDITIONAL NOTES AND TOP MAT OF SLAB REINFORCEMENT AT THE THICKENED SLAB END.

SEE PCP AND PCP-FAB FOR PANEL DETAILS NOT SHOWN.

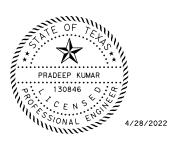
SEE PCP(O) AND PCP(O)-FAB FOR PRECAST OVERHANG PANEL DETAILS IF THIS OPTION IS USED.

SEE IGMS STANDARD FOR MISCELLANEOUS DETAILS.

SEE PMDF STANDARD FOR DETAILS AND QUANTITY ADJUSTMENTS IF THIS OPTION IS USED.

BAR LAPS, WHERE REQUIRED SHALL BE AS FOLLOWS: EPOXY COATED ~ #4 = 2'-5" GFRP ~ #5 = 2'-9"

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.



HL93 LOADING

REV. No.	DATE	REVISION	BY

**ATKINS** 

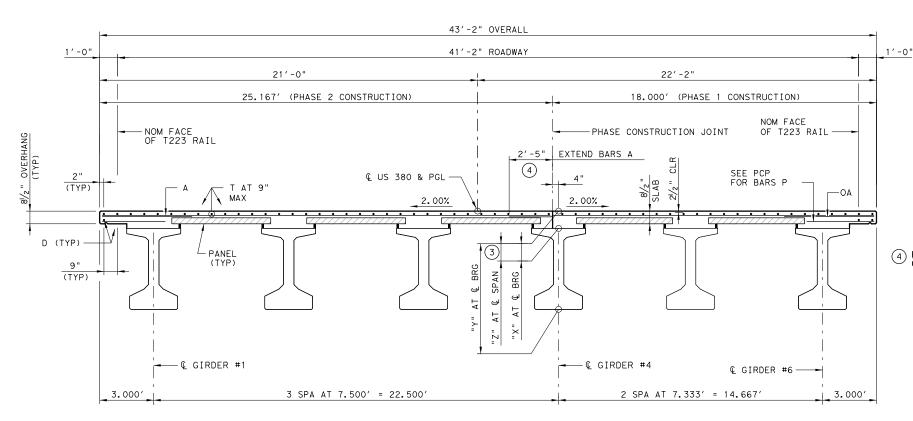


US 380 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: NONE SHEET 1 OF 2

IGNED	:TS	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	HWAY No.
CKED:	SK	6	TEXAS :	SEE TIT	LE SHE	ΞT	U	S 380
wn:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		о.	SHEET No.
CKED:			STONEWALL	0106	04	0	36	113



BAR TABLE BAR SIZE #5 #4 D G #5 #4 #4 М #4 OΑ #5 #4

TABLE OF ESTIMATED QUANTITIES							
SPAN	REINFORCED CONCRETE SLAB	PRESTR CONCRETE GIRDER (TX54)	REINF STEEL 1				
NO.	SF	LF	LB				
7	4533	627.00	10426				
8	4533	627.00	10426				
TOTAL	9066	1254.00	20852				

- 1 REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.
- 2 LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.

### NOTES:

#5

1. FOR GENERAL NOTES SEE 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT SHEET 1 OF 2.

# PRADEEP KUMAR 130846 MINNING ENGL 4/28/2022

REV. No.	DATE	REVISION	BY

# **ATKINS**



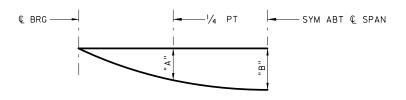
US 380 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE SCALE: 3/16" = 1'-0" SHEET 2 OF 2

IGNED	:TS	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	HWAY No.
CKED:	SK	6	TEXAS :	SEE TIT	LE SHE	ΞT	U	S 380
wn:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.
CKED:			STONEWALL	0106	04	0	36	114

### TYPICAL TRANSVERSE SECTION

TABLE OF DEFLECTIONS						
SPAN NO.	GIRDER NO.	"A"	"B"			
SPAN NO.	GIRDER NO.	FT	FT			
	1	0.071	0.101			
	2	0.079	0.112			
7-8	3	0.079	0.112			
7-8	4	0.042	0.060			
	5	0.077	0.110			
	6	0.070	0.100			

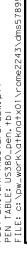


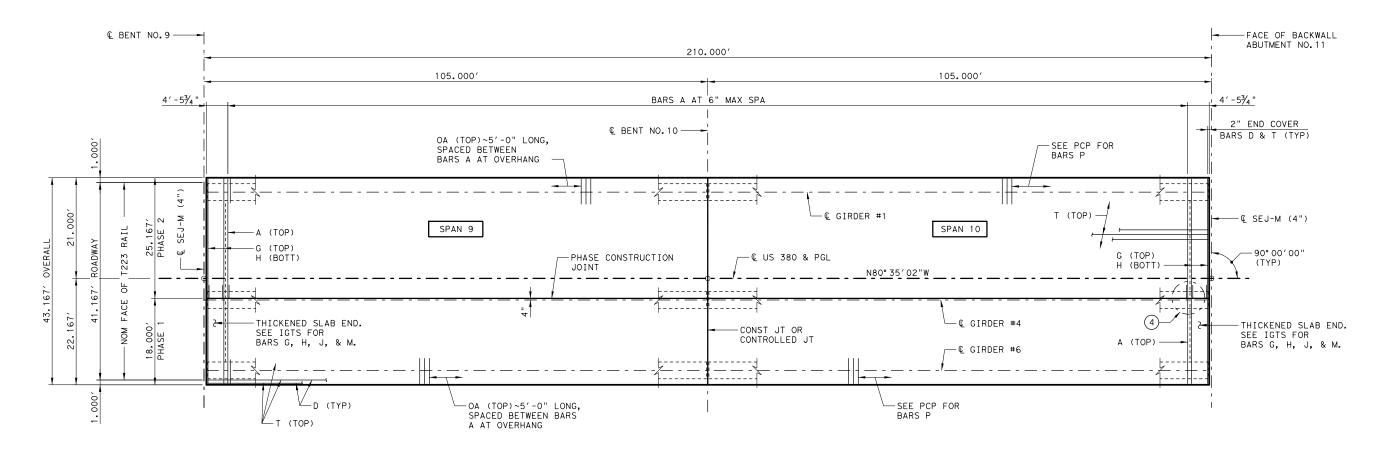
### DEAD LOAD **DEFLECTION DIAGRAM**

DEFLECTIONS SHOWN ARE DUE TO CONCRETE SLAB ONLY. (EG = 5,000 KSI). CALCULATED DEFLECTIONS SHOWN ARE THEORETICAL AND ACTUAL DEFLECTIONS MAY BE LESS. ADJUST VALUES AS REQUIRED IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERICATION.

TABLE OF SECTION DEPTHS						
SPAN NO.	GIRDER NO.	"X" AT & BRG	"Y" AT & BRG	"Z" AT & SPAN 3		
	1	11 3/4"	5′-5 ¾"	9 1/8"		
	2	11 3/4"	5′-5 1/8"	10"		
7 0	3	11 3/4"	5′-5 1/8"	10"		
7-8	4	11 3/4"	5′-5 1/8"	9 ½"		
	5	11 3/4"	5′-5 %"	10"		
	6	11 3/4"	5′-5 7⁄8"	9 7/8"		

(3) THEORETICAL DIMENSION





PLAN

DESIGNED IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 8TH EDITION (2017), AS MODIFIED BY 2020 TXDOT BDM.

CONCRETE SHALL BE CLASS S (HPC), f'c = 4000 PSI.

PROVIDE GFRP BARS, CONFORMING TO ASTM D7957/7957M, EXCEPT PROVIDE A MINIMUM MODULUS OF ELASTICITY OF 7500 KSI.

PROVIDE GFRP BARS FOR TOP MAT ONLY. ALL OTHER REINFORCING STEEL SHALL BE GRADE 60 (EPOXY COATED).

SEE RAIL STANDARD FOR ANCHORAGE IN SLAB.

**GENERAL NOTES:** 

SEE GFRP SLAB (IGFRP) STANDARD FOR ADDITIONAL NOTES AND TOP MAT OF SLAB REINFORCEMENT AT THE THICKENED SLAB END.

SEE PCP AND PCP-FAB FOR PANEL DETAILS NOT SHOWN.

SEE PCP(O) AND PCP(O)-FAB FOR PRECAST OVERHANG PANEL DETAILS IF THIS OPTION IS USED.

SEE IGMS STANDARD FOR MISCELLANEOUS DETAILS.

SEE PMDF STANDARD FOR DETAILS AND QUANTITY ADJUSTMENTS IF THIS OPTION IS USED.

BAR LAPS, WHERE REQUIRED SHALL BE AS FOLLOWS: EPOXY COATED ~ #4 = 2'-5" GFRP ~ #5 = 2'-9"

4 EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.



HL93 LOADING

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REV. No.	DATE	REVISION	BY

**ATKINS** 

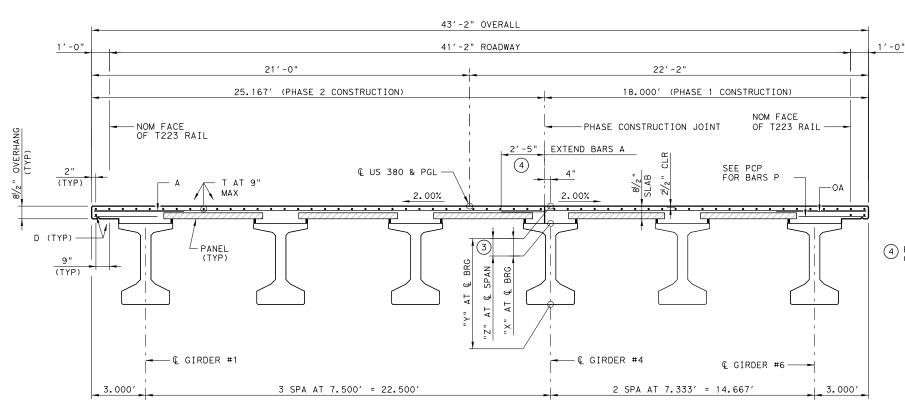


US 380 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT

SALT FORK OF BRAZOS RIVER BRIDGE

SCALE: NONE SHEET 1 OF 2

GIGNED:	TS	FED. RD DIV. No.	STATE	PROJE	CT No.		HIG	HWAY No.
CKED:	SK	6	TEXAS :	SEE TIT	LE SHE	ΞT	U	S 380
wn:	DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB o.	SHEET No.
CKED:			STONEWALL	0106	04	0	36	115



BAR TABLE BAR SIZE #5 #4 D G #5 #4 #4 М #4 OΑ #5 #4 #5

TABLE OF ESTIMATED QUANTITIES							
SPAN	REINFORCED CONCRETE SLAB	PRESTR CONCRETE GIRDER (TX54)	REINF STEEL 1				
NO.	SF	LF	LB				
9	4533	627.00	10426				
10	4533	627.00	10426				
TOTAL	9066	1254.00	20852				

- 1 REINFORCING STEEL WEIGHT IS CALCULATED USING AN APPROXIMATE FACTOR OF 2.3 LBS/SF.
- 2 LENGTHS SHOWN ARE BOTTOM GIRDER FLANGE LENGTHS WITH ADJUSTMENTS MADE FOR GIRDER

(4) EXTEND BARS A, G & H ACROSS PHASED CONSTRUCTION JOINT.

### NOTES:

1. FOR GENERAL NOTES SEE 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT SHEET 1 OF 2.

# PRADEEP KUMAR 130846 MINNINGENONAL ENGL 4/28/2022

REV. No.	DATE	REVISION	BY

# **ATKINS**



US 380 210.000' PRESTRESSED CONCRETE I-GIRDER UNIT

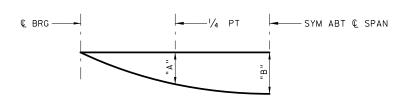
SALT FORK OF BRAZOS RIVER BRIDGE

SCALE:  $\frac{3}{6}$ " = 1'-0"

710										
SIGNED:TS	FED. RD DIV. No.	STATE	PROJE	CT No.	HIGHWAY No.					
ECKED: SK	6	TEXAS	SEE TITLE SHEET				US 380			
AWN: DN	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB 0.	SHEET No.			
ECKED: TS	ABI	STONEWALL					0106 04 036		116	

### TYPICAL TRANSVERSE SECTION

TABLE OF DEFLECTIONS										
SPAN NO.	GIRDER NO.	"A"	"B"							
SPAN NO.	GIRDER NO.	FT	FT							
	1	0.071	0.101							
	2	0.079	0.112							
9-10	3	0.079	0.112							
9-10	4	0.042	0.060							
	5	0.077	0.110							
	6	0.070	0.100							



### DEAD LOAD DEFLECTION DIAGRAM

DEFLECTIONS SHOWN ARE DUE TO CONCRETE SLAB ONLY. (EG = 5,000 KSI). CALCULATED DEFLECTIONS SHOWN ARE THEORETICAL AND ACTUAL DEFLECTIONS MAY BE LESS. ADJUST VALUES AS REQUIRED IF OPTIONAL SLAB FORMING IS USED. THESE VALUES MAY REQUIRE FIELD VERICATION.

	TABLE OF SECTION DEPTHS										
SPAN NO.	GIRDER NO.	"X" AT & BRG	"Y" AT Q BRG	"Z" AT © SPAN 3							
	1	11 3/4"	5′-5 1/8"	9 1/8"							
	2	11 3/4"	5′-5 1/8"	10"							
9	3	11 3/4"	5′-5 1/8"	10"							
9	4	11 3/4"	5′-5 1/8"	9 1/2 "							
	5	11 3/4"	5′-5 1/8"	10"							
	6	11 3/4"	5′-5 1/8"	9 %"							
	1	12 ¾"	5′-6 ¾"	10"							
	2	12 ¾"	5′-6 ¾"	10 1/8"							
10	3	12 ¾"	5'-6 3/4"	10 1/8"							
10	4	12 3/4"	5′-6 3/4"	9 5/8 "							
	5	12 3/4"	5′-6 ¾"	10"							
	6	12 3/4"	5'-6 3/4"	10"							

(3) THEORETICAL DIMENSION

SUBMITTA 00%

G F E D C B A A B C D E F C 13 Spa at 2" *TYPE Tx46 & Tx54* 

DEPRESSED

STRAND

**PATTERN** 

NO.

ÉND

64.5

50.5

50.5

"e" END

17.58

12.51

11.71

CONCRETE

28 DAY COMP STRGTH

7.500

7.500

6.000

TRGTH

6.000

5.500

5.000

DESIGN LOAD

STRESS

(TOP Q) (SERVICE I,

4.365

4.239

3.254

GEEDCBAABCDEEG 13 Spa at 2"

OPTIONAL DESIGN

MINIMUM

MOMENT

STRENGTH (kip-ft)

12171

6280

LIVE LOAD DISTRIBUTION FACTOR

2

Moment Shear

0.812

0.779

0.779

0.602

0.595

0.617

DESIGN LOAD

TENSILE

STRESS

(SERVICE II

-4.352

-4.069

-3.185



NON	N-STANDARD STRAND PATTERNS
PATTERN	STRAND ARRANGEMENT AT € OF GIRDER

1) Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

Tension =  $0.24 \sqrt{f'ci}$ 

Optional designs must likewise conform.

(2) Portion of full HL93.

#### DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Optional designs for girders 120 feet or longer must have a calculated residual camber equal to or greater than that of the designed girder.

Prestress losses for the designed girders have been calculated for a relative humidity of 60 percent. Optional designs must

#### **FABRICATION NOTES:**

Provide Class H concrete.

Provide Grade 60 reinforcing steel bars.

Use low relaxation strands, each pretensioned to 75 percent of

Strand debonding must comply with Item 424.4.2.2.2.4. Full-length debonded strands are only permitted in positions marked . Double wrap full-length debonded strands in outer most position of each

When shown on this sheet, the Fabricator has the option of furnishing either the designed girder or an approved optional design. All optional design submittals must be signed, sealed and dated by a Professional Engineer registered in the State of Texas.

Seal cracks in girder ends exceeding 0.005" in width as directed by the Engineer. The fabricator is permitted to decrease the spacing of Bars R and S by providing additional bars to help limit crack width provided the decreased spacing results in no less than 1" clear between bars. The fabricator must take an approved corrective action if cracks greater than 0.005" form on a repetitive

#### DEPRESSED STRAND DESIGNS:

Locate strands for the designed girder as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc., beginning each row in the "A" position and working outward until the required number of strands is reached. All strands in the "A" position must be depressed, maintaining the 2" spacing so that, at the girder ends, the upper two strands are in the position shown in the table.

HL93 LOADING

EV. No.	DATE	REVISION	BY

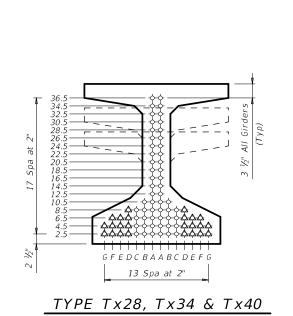
# **ATKINS**



US 380 PRESTRESSED CONCRETE I-GIRDER DESIGNS (NON-STANDARD SPANS)

SALT FORK OF BRAZOS RIVER BRIDGE

DESIGNED: <b>TS</b> FED. R	STATE		PROJE	HIGHWAY No.			
CHECKED: SK 6	TEXAS		SEE TIT	LE SHE	ΞΤ	U	S 380
DRAWN: DN STATE	T COUNT	Y	CONTROL No.	SECTION No.		OB 0.	SHEET No.
CHECKED: TS ABL	STONEW	ALL	0106	04	0	36	117



DESIGNED GIRDERS

NO.

40

34

NON-STD STRAND PATTERN

SPAN NO.

7-10

STRUCTURE

US380 - SALT FORK OF BRAZOS RIVER

GIRDER

ALL

ALL

GIRDER TYPE

Tx54

Tx54

PRESTRESSING STRANDS

STRGTH

270 270

19.11

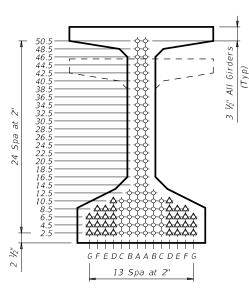
19.48

SIZE

0.6

0.6

0.6



*TYPE Tx62 & Tx70* 

1 Usual limit of Cement Stabilized Backfill is at end of wingwall. However, extend limits to the end of the Approach Slab.

Other materials can be used as a bond breaker if permitted by the Engineer. 2 layers of 30 Lb roofing felt or 2 layers of heavy mil polyethylene sheeting are examples.

Provide Cement Stabilized Backfill meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments.

Details are drawn showing left forward skew.
See Bridge Layout for actual skew direction.
These details do not apply when MSE or
Concrete Block retaining walls are used in



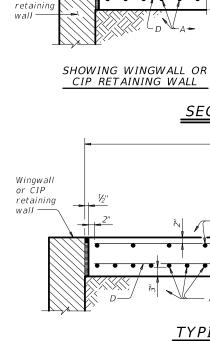


CEMENT STABILIZED ABUTMENT BACKFILL BRIDGE ABUTMENT

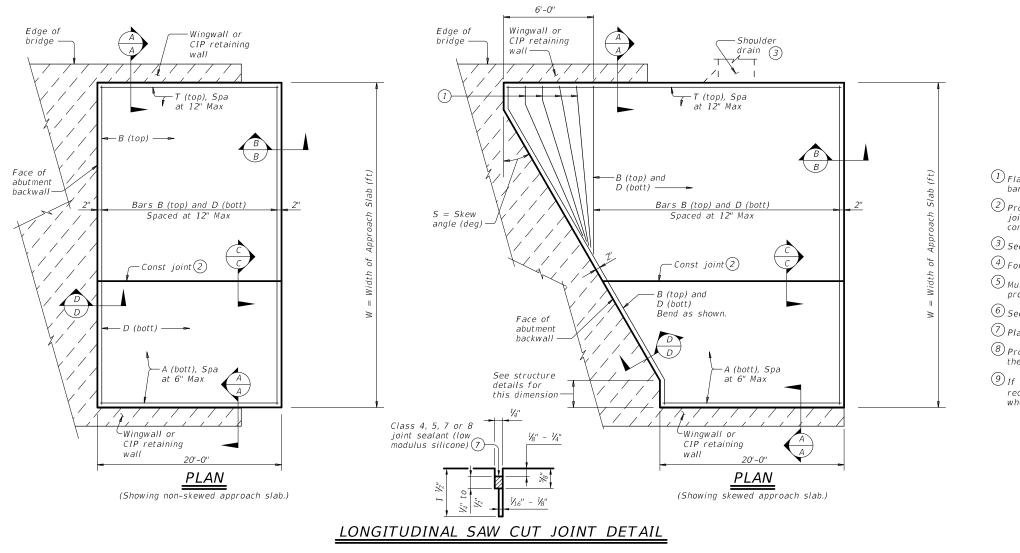
CSAB (MOD)

					٠,		,	
	csabste1.dgn	DN: TXDOT CK: TXDOT		ск: ТхD0Т	DW: TXDOT		ck: TxD0T	
ОТ	January 2015	CONT	SECT	JOB	HI		HIGHWAY	
	REVISIONS	0106	04	036	036 L		380	
		DIST		COUNTY	COUNTY		SHEET NO.	
		ABL		STONEWALL			118	





7:16:32 F



SHOWING MSE WALL

– € Structure

6

SECTION A-A

6

W = Width of Approach Slab (ft)

TYPICAL TRANSVERSE SECTION

*TABLE* BAR SIZE #8 #5 #5

## APPROXIMATE QUANTITIES 4

Reinf steel weight = 8.5 Lbs/SF of Approach Slab

Volume of Appr Slab Conc (CY) =  $0.802W + 0.02W^2$  Tan S

W = Width of Approach Slab (ft)

S = Skew Angle (deg)

- ① Flare Bars B and D in this region (1'-6" Max Spa, 3" Min Spa). Minimum flared bar length = 2'-6". Bend bars as necessary.
- 2) Provide longitudinal construction joints that align with longitudinal construction joints in the bridge slab with bridges built in stages. Other longitudinal construction joints must receive approval of the Engineer.
- (3) See details elsewhere in plans for shoulder drain location and details.
- 4 For Contractor's information only. Quantities shown are for one approach slab.
- (5) Multiple piece tie bars are acceptable at longitudinal construction joints provided minimum laps shown are achieved.
- 6 See details elsewhere in plans for required cross-slope.
- 7 Place in accordance with Item 438.

BAR

#5

Α

В

D

- $\fbox{8}$  Provide backer rod that is 25% larger than joint opening and compatible with the sealant.
- (9) If bridge rail is present at the wingwall or CIP retaining wall, place ½" rebonded recycled tire rubber between concrete railing and top of approach slab as shown when concrete railing projects over the approach slab.

#### GENERAL NOTES:

Construct approach slab in accordance with Item 422. Provide Class "S" concrete with a minimum compressive strength of 4,000 psi.

Provide Grade 60 reinforcing steel.

Provide longitudinal joints as shown on the Longitudinal Saw Cut Joint Detail at lane lines and shoulders when width between longitudinal construction joints or edges of approach slab exceeds 16 feet. Saw cut joints within 24 hours of concrete placement to a depth of 1  $\frac{1}{2}$ " and seal in accordance with Item 438. Alternately, provide a controlled joint consisting of 1  $\frac{1}{2}$  vinyl or plastic joint former (Stress Cap, Zip Strip, Stress Lock, or equal as approved by the Engineer.)

Provide rebonded recycled tire rubber joint filler that meets the requirements of DMS-6310. "Joint Sealants and Fillers:

Construct the subgrade or subbase away from the bridge for a minimum distance of 100 feet prior to the approach slab, unless otherwise indicated on the plans.

Compact and finish the subgrade or foundation for the

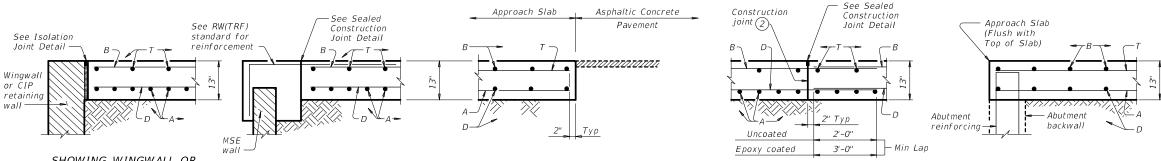
approach slab to the typical cross-section and to the lines and grades shown on the plans.

Cure for 4 days using water or membrane curing per Item 422.

All details shown herein are subsidiary to bridge approach slab.

Cover dimensions are clear dimensions, unless noted otherwise.

Texas Department of Transportation



SECTION B-B

Class 4, 5, 7, or 8 joint sealant

(low modulus

silicone) (7)

Wingwall or

wall

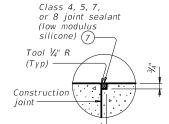
See Isolation

Joint Detail (Typ)

> or ČIP retaining

wall

CIP retaining



SECTION D-D

SECTION C-C 5

Backer rod (8)

Rebonded recycled

ISOLATION JOINT DETAIL

**SEALED** CONSTRUCTION JOINT DETAIL

BRIDGE APPROACH SLAB ASPHALTIC CONCRETE PAVEMENT

BAS-A

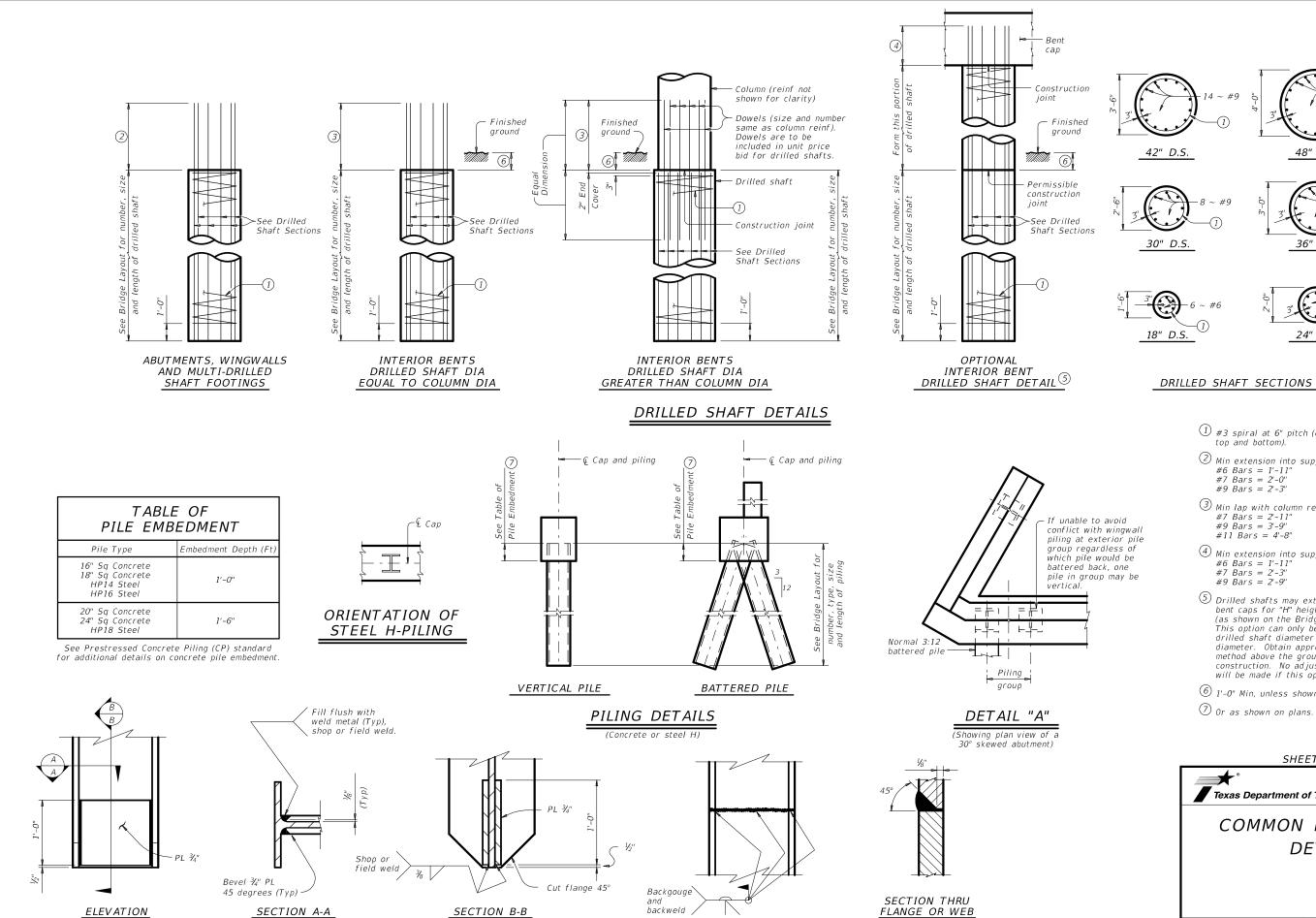
	_,,					
FILE: basaste1-20.dgn	DN: TXE	DOT	ск: ТхD0Т	DW:	TxD0T	ck: TxD0T
©TxD0T April 2019	CONT	SECT	JOB			HIGHWAY
REVISIONS	0106	04	036		U	IS 380
02-20: Removed stress relieving pad.	DIST		COUNTY			SHEET NO.
	ABL		STONEW	4LL	.	119



STEEL H-PILE TIP REINFORCEMENT

See Item 407 "Steel Piling" to determine when tip reinforcement

is required and for options to the details shown.



STEEL H-PILE SPLICE DETAIL

Use when required

1) #3 spiral at 6" pitch (one and a half flat turns top and bottom).

48" D.S.

36" D.S.

24" D.S.

18 ~ #9

Min extension into supported element: #6 Bars = 1'-11" #7 Bars = 2'-0" #9 Bars = 2'-3"

3 Min lap with column reinf. #7 Bars = 2'-11" #9 Bars = 3'-9"  $#11 \; Bars = 4'-8''$ 

4 Min extension into supported element: #6 Bars = 1'-11"  $\#7 \; Bars = 2'-3''$ 

(5) Drilled shafts may extend to the bottom of bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the drilled shaft diameter equals the column diameter. Obtain approval of the forming method above the ground line prior to construction. No adjustments in payment will be made if this option is used.

6 1'-0" Min, unless shown otherwise on plans.

7 Or as shown on plans.



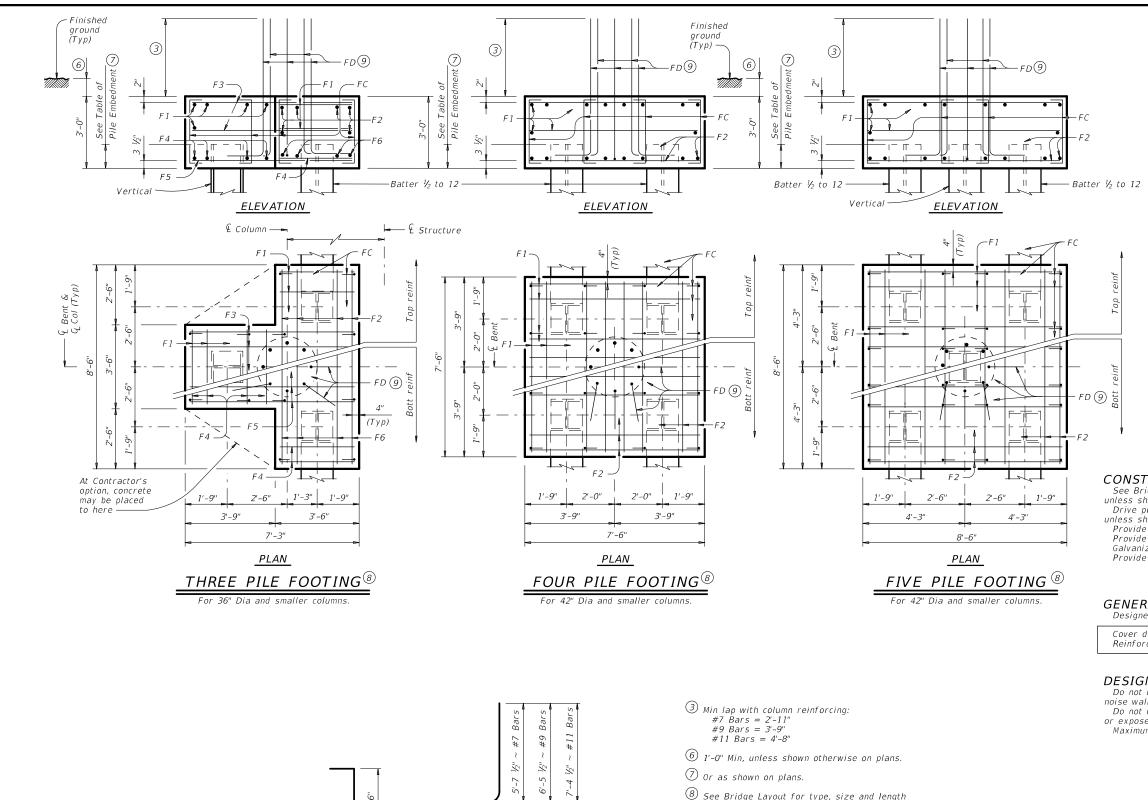


### COMMON FOUNDATION **DETAILS**

FDCK: TXDOT DW: TXDOT CK: TXDO 036

fdstde01-20.dgr N: TXDOT C)TxDOT April 2019 0106 04 US 380 01-20: Added #11 bars to the FD bars STONEWALL





1'-2" #7 Bars

1'-7" #9 Bars

2'-0" #11 Bars

BARS FD 9

6"

BARS FC

- $\fbox{8}$  See Bridge Layout for type, size and length of piling.
- Number and size of FD bars must match column reinforcing. Tie FD bars to the top of the bottom reinforcing mat.
- 10 Adjust FD quantity, size and weight as needed to match column reinforcing.

### TABLE OF FOOTING QUANTITIES FOR 30" COLUMNS

		30 (	COLUN	כ עוו	l
		ONE 3	PILE FOOT	rING	
Bar	No.	Size	Lengt	h	Weight
F 1	11	#4	3'- 2	II .	23
F2	6	#4	8'- 2	п	33
F3	6	#4	6'- 17	!"	28
F4	8	#9	3'- 2	ıı .	86
F5	4	#9	6'- 11	!"	94
F6	4	#9	8'- 2	ıı	111
FC	12	#4	3'- 6	ıı	28
FD 10	8	#9	8'- 1	ıı .	220
Reinf	orcing	Steel		Lb	623
Class	"C" Cc	ncrete		CY	4.8
		ONE 4	PILE FOOT	ING	
Bar	No.	Size	Lengt	h	Weight
F 1	20	#4	7'- 2		96
F2	16	#8	7'- 2	II .	306
FC	16	#4	3'- 6	ıı	37
FD [10]	8	#9	8'- 1	п	220
Reinf	orcing	Steel		Lb	659
Class	"C" Cc	ncrete		CY	6.3
		ONE 5	PILE FOOT	「ING	
Bar	No.	Size	Lengt	h	Weight
F 1	20	#4	8'- 2	ıı .	109
F2	16	#9	8'- 2	II .	444
FC	24	#4	3'- 6	п	56
FD [10]	8	#9	8'- 1	п	220
Reinf	orcing	Steel		Lb	829
Class	"C" Cc	ncrete		CY	8.0

### CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise.

Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile unless shown otherwise.

Provide Class C Concrete (f'c = 3,600 psi), unless shown otherwise. Provide Grade 60 reinforcing steel. Galvanize reinforcing if shown elsewhere in the plans.

Provide bar laps for drilled shaft reinforcing, where required, as follows:

Uncoated or galvanized (#6) ~ 2'-6"

Uncoated or galvanized (#7) ~ 2'-11"

Uncoated or galvanized (#9) ~ 3'-9"

GENERAL NOTES:
Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

DESIGNER NOTES:

Do not use the drilled shaft details shown on this standard for retaining wall,

noise wall, barrier, or sign foundations without structural evaluation.

Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.

Maximum allowable pile loads for the footings shown are:
72 Tons/Pile with 24" Dia Columns
80 Tons/Pile with 30" Dia Columns
100 Tons/Pile with 36" Dia Columns 120 Tons/Pile with 42" Dia Columns

SHEET 2 OF 2



Bridge Division Standard

### COMMON FOUNDATION **DETAILS**

FD

				_			
E: fdstde01-20.dgn	DN: TXE	OT.	ск: ТхD0Т	DW: T.	xD0T	ck: TxD0T	
TXDOT April 2019	CONT	SECT	JOB		HIG	HWAY	
REVISIONS	0106	04	04 036 US 380				
11-20: Added #11 bars to the FD bars.	DIST	COUNTY				SHEET NO.	
	ABL		STONEW	ALL		121	



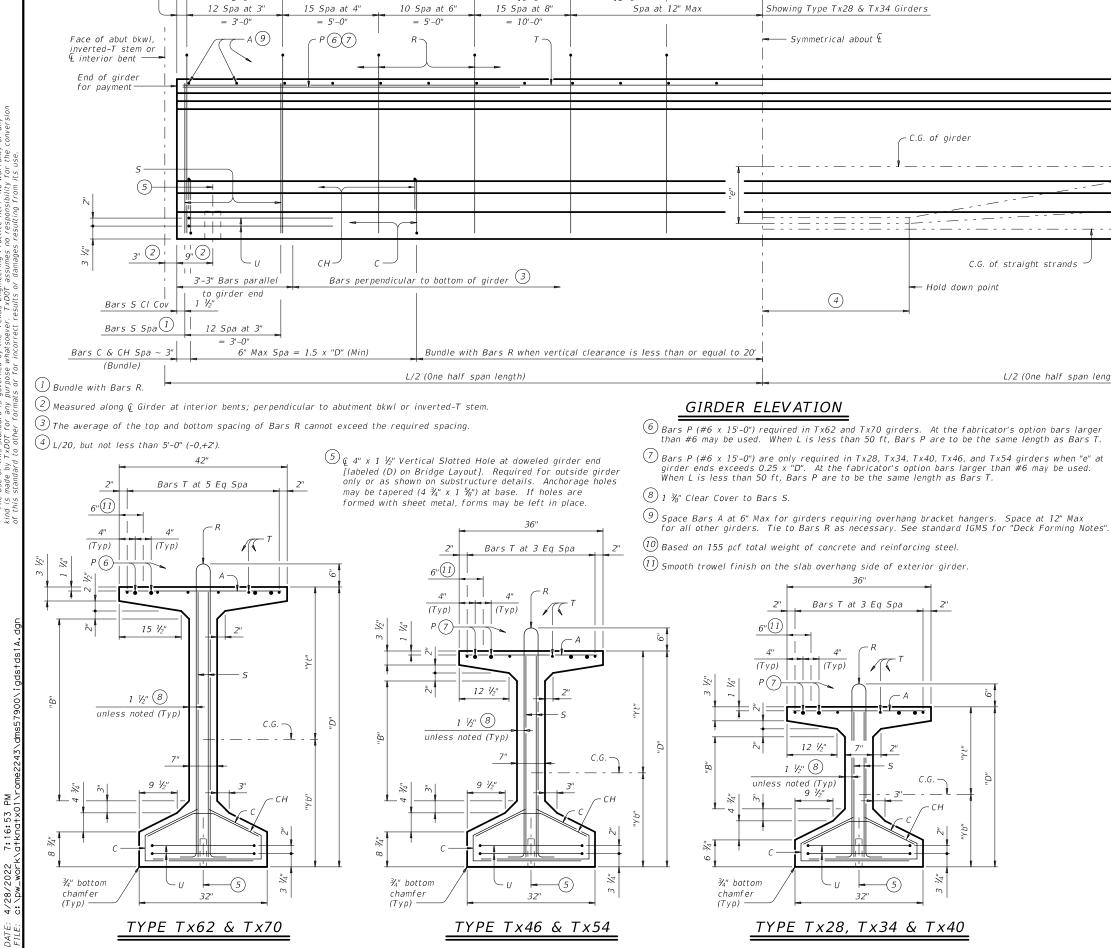
12 Spa at 3"

= 3'-0''

12 Spa at 3"

= 3'-0"

Bars R Spa ~ 2 1/3"



30 Spa at 8'' = 20'-0''

20 Spa at 6" = 10'-0"

15 Spa at 12"

= 15'-0"

15 Spa at 12"

= 15'-0"

15 Spa at 8"

= 10'-0"

Spa at 18" Max

Spa at 18" Max

Showing Type Tx62 & Tx70 Girders

Showing Type Tx40, Tx46 & Tx54 Girders

#### GIRDER DIMENSIONS AND SECTION PROPERTIES Weight Girder Type (in.) (in.) (in.) (in.) $(in.^2)$ (in.4) (in.4) (plf) 630 Tx28 28 15.02 12.98 585 52,772 40.559 34 12 18.49 15.51 627 88,355 40,731 675 Tx34 18.10 720 Tx40 40 18 21.90 669 134,990 40.902 T x 46 46 22 25.90 20.10 761 198,089 46,478 819 880 Tx54 54 30 30.49 23.51 817 299,740 46,707 Tx62 62 37 1/2" 33.72 28.28 910 463,072 57,351 980

- Face of abut bkwl,

interior bent

inverted-T stem or

End of girder for payment Ontional ¾" Chamfer

vertically (Typ)

90° at int bents, plumb ends at abut bkwl & inverted-T

#### GENERAL NOTES:

70

Tx70

Designed according to AASHTO LRFD Bridge Design Specifications. Provide Class H concrete.

31.91

966

628,747

57,579

1,040

45 1/2"

Do not blockout

C.G. of depressed strands

C.G. of all strands

L/2 (One half span length)

top of girders for

thickened slab ends.

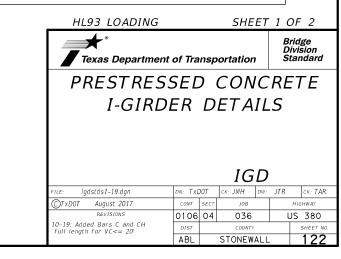
Provide Grade 60 reinforcing steel

38.09

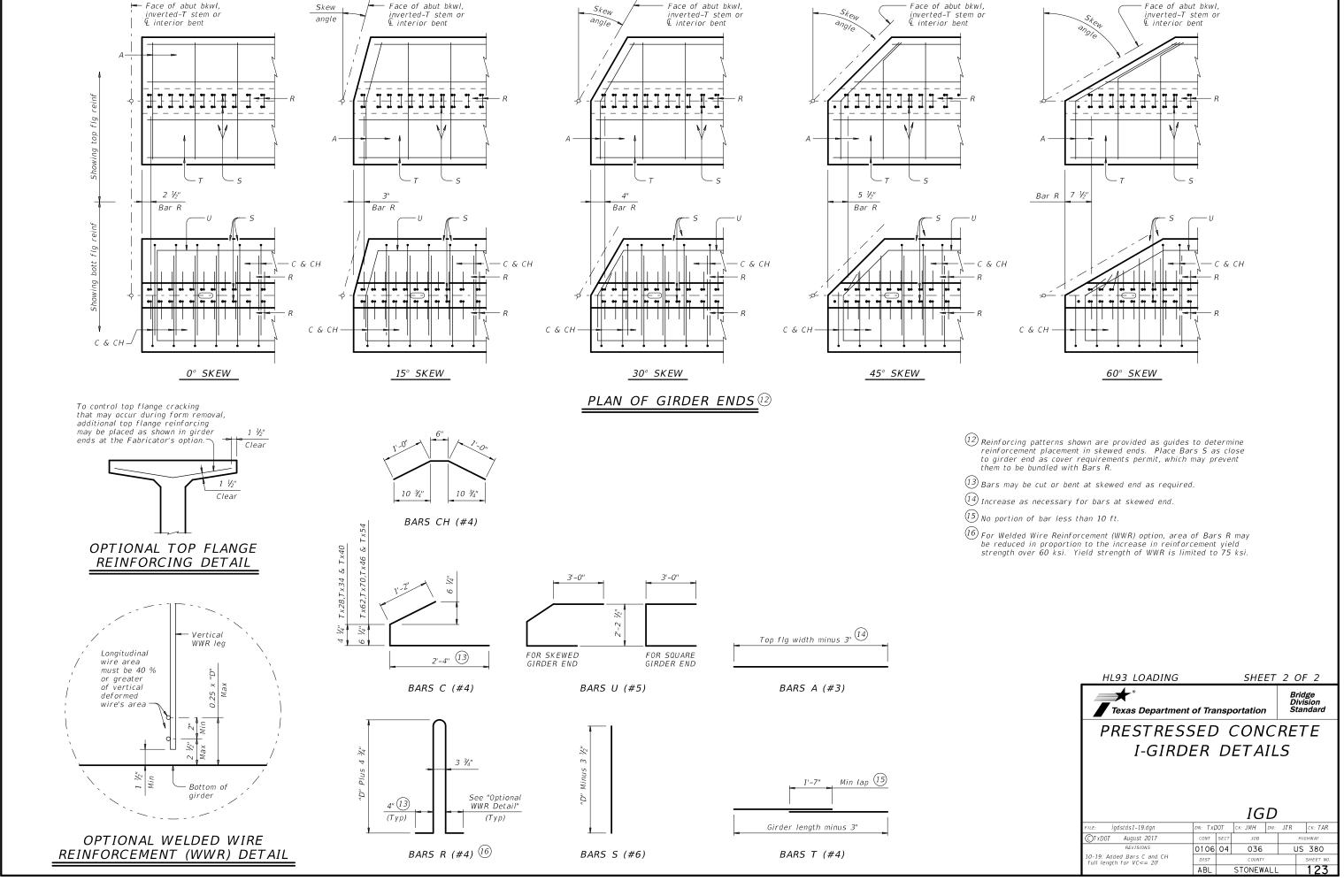
An equal area of deformed Welded Wire Reinforcement (WWR) (ASTM A1064) may be substituted for Bars A, C, R or T unless otherwise noted.

It is permissible for bars or strands to come in contact with materials used in forming anchor holes.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.







STONEWALL



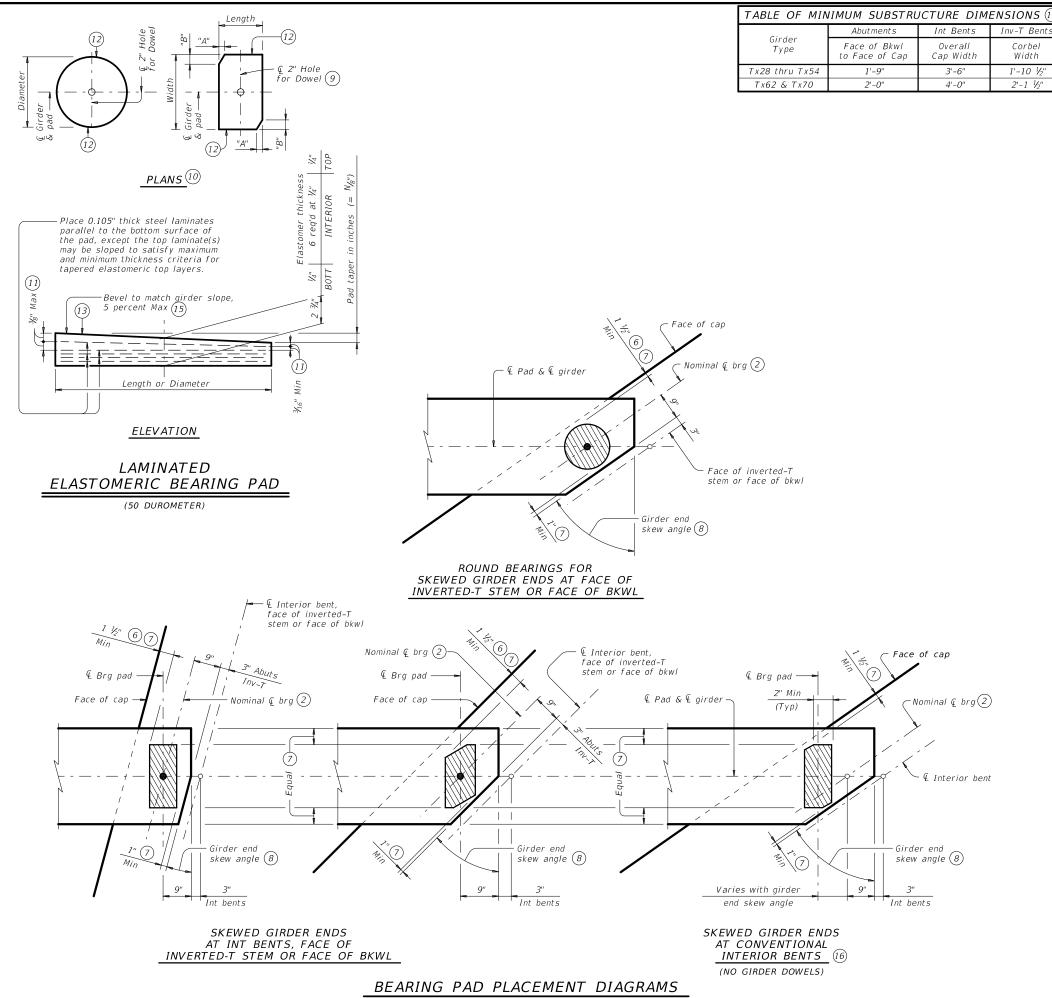


TABLE OF BEARING PAD DIMENSIONS Girder End Pad Clip Girder Pad Size Bent Type (13) Skew Angle Dimensions Lgth x Wdth Туре Range G-1-"N" 0° thru 21° 8" x 21' Tx28,Tx34, 21°+ thru 30° 8" x 21" ABUTMENTS. INVERTED-T G-3-"N"30°+ thru 45° 9" x 21" 4 1/2" AND TRANSITION G-4-"N" 45°+ thru 60° 15" Dia G-5-"N" 0° thru 21° 9" x 21" BENTS Tx62 G-6-"N" 21°+ thru 30° 9" x 21" 1 1/5" BACKWALLS G-7-"N" 30°+ thru 45° 10" x 21" 4 1/3" Tx70 7 1/4" 45°+ thru 60° 10" x 21" Tx28,Tx34, CONVENTIONAL Tx40, Tx46INTERIOR & Tx54 G-1-"N" 8" x 21" 0° thru 60° BENTS Tx62 & Tx70 G-5-"N" 0° thru 60° 9" x 21" G-1-"N" 0° thru 18° 8" x 21" CONVENTIONAL INTERIOR Tx28,Tx34, G-2-"N"18°+ thru 30° 8" x 21" G-9-"N" 30°+ thru 45° 8" x 21" WITH& Tx54 SKEWED G-10-"N" 45°+ thru 60° 9" x 21" GIRDER G-5-"N" 0° thru 18° 9" x 21" Tx62 G-5-"N" 9" x 21" 18°+ thru 30° (GIRDER CONFLICTS) 30°+ thru 45° G-11-"N"9" x 21" 1 1/2" Tx70 (16) 45°+ thru 60° 9" x 21"

- 2) For purposes of computing bearing seat elevations, nominal centerline of bearing must be defined as shown. The actual center of bearing pad may vary from this line.
- 6 3" for inverted-T.
- 7 Place centerline pad as near nominal centerline bearing as possible between limits shown.
- (8) Girder end skew angle is equal to  $90^\circ$  minus the girder angle except at some conflicting girders.
- (9) Provide 2" dia hole only at locations required. See Substructure details for location.
- (10) See Table of Bearing Pad Dimensions for dimensions.
- (1) Maximum and minimum layer thicknesses shown are for elastomer only, on tapered layers.
- (12) Locate Permanent Mark here.
- 13 Indicate BEARING TYPE on all pads. For tapered pads, locate BEARING TYPE on the high side. The Fabricator must include the value of "N" (amount of taper in 1/8" increments) in this mark.

Examples: N=0, (for 0" taper) N=1, (for  $\frac{1}{8}$ " taper)

N=2, (for  $\frac{1}{4}$ " taper) (etc.)

Fabricated pad top surface slope must not vary from plan girder slope by more than  $\begin{pmatrix} 0.0625'' \\ Length \text{ or Dia} \end{pmatrix}$  IN/IN.

- (14) Substructure dimensions must satisfy the minimums provided to accommodate the elastomeric bearings shown on this standard.
- (15) See sheet 3 of 3 for beveled plate use when slopes exceed 5 percent.
- (16) If girder end is skewed for a girder conflict at an interior bent and a beveled sole plate is required, use bearing type for abutments at this location. Location of bearing centerline is to be set as for abutments in this case.

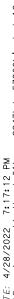
HL93 LOADING SHEET 2 OF 3

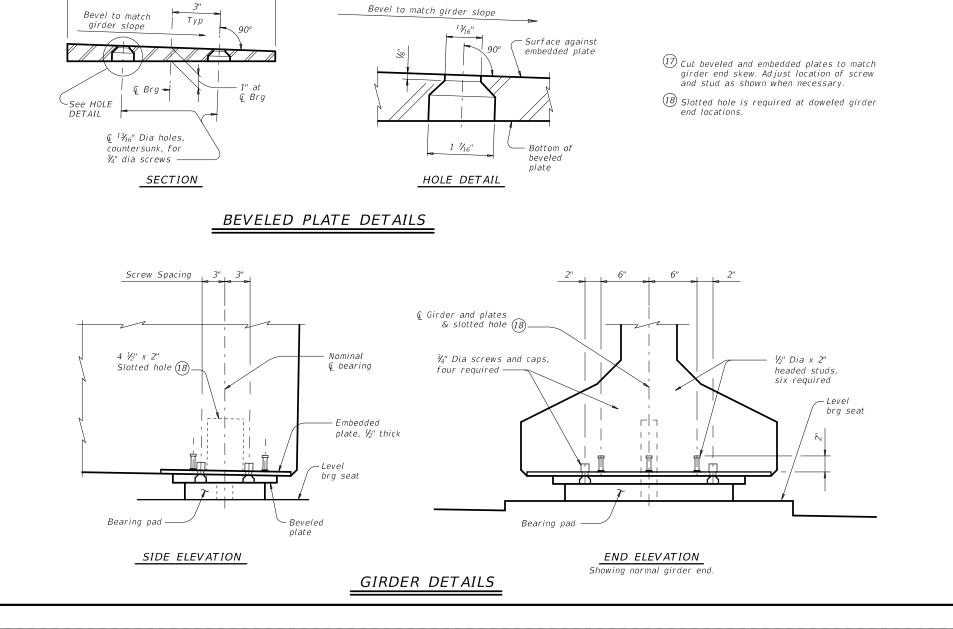


ELASTOMERIC BEARING AND GIRDER END DETAILS PRESTR CONCRETE I-GIRDERS

IGEB

E: igebsts1-17.dgn	DN: AE	Έ	ск: ЈМН	DW:	JTR	ck: TxD0T
TxD0T August 2017	CONT	SECT	JOB		HI	HWAY
REVISIONS	0106	04	036		US	380
	DIST		COUNTY			SHEET NO.
	ABL		STONEW	ALL		125





Embedded plate, 1/2" thick

Ç ½" Dia studs

Ç ¾" Dia

screws and caps

4 ½" x 2"

Slotted hole (18)

Beveled plate

Bearing pad

€ bearing

© Girder and plates -

& slotted hole

Pad L + 3"

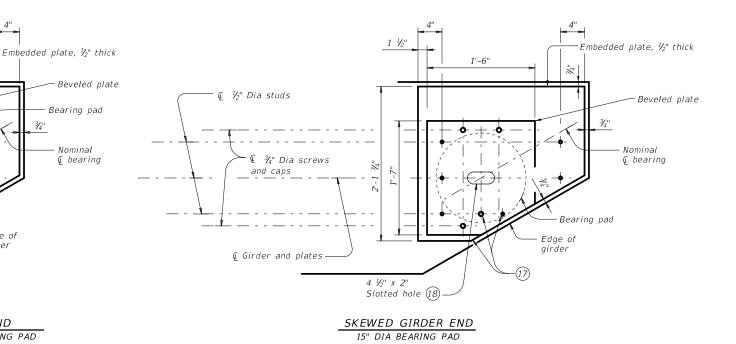
NORMAL GIRDER END

RECTANGULAR BEARING PAD

Length

4 ½" x 2"

Slotted hole (18)-



### PLAN VIEW OF SOLE PLATE DETAILS

SKEWED GIRDER END

CLIPPED RECTANGULAR BEARING PAD

3/4"

Edge of

Pad L + 3"

#### SOLE PLATE NOTES:

Provide constant thickness elastomeric bearings with beveled and embedded steel sole plates in accordance with these details when the girder slope exceeds 5 percent or if otherwise required in the plans. Provide for all girders in the span.

On the shop drawings, dimension sole plates to the nearest  $\mathcal{V}_{16}$ " based on required thickness at centerline of bearing and slope of girder. Thickness tolerance variation from the approved shop drawings is  $V_{16}$ "+/-, except variation from a plane parallel to the theoretical top surface can not exceed  $V_{16}$ " total. Bearing surface tolerances listed in

Item 424 apply to embedded and beveled plates. Steel plate must conform to ASTM A36, A572 Gr 50, or A709 Gr 36 or Gr 50. Hot dip galvanize both the embedded plate and beveled sole plate after fabrication. Seal weld caps to embedded plate before

When determining if relocation of screw holes and studs are necessary for skewed girder ends, minimum clearance from screw or stud centerline

Tap threads in the embedded plate only. Drill and tap prior to galvanizing.

34" Dia screws must be electroplated, socket flat head countersunk cap screws conforming to ASTM F835. Electroplating must conform to ASTM B633, SC 2, Type I. Provide screws long enough to maintain a 34" minimum embedment into the embedded plate and galvanized cap. Provide galvanized steel caps (16 ga Min) with a nominal 1" inside diameter and deep enough to accommodate the screws, but not less than ½" deep or

Install beveled sole plates prior to shipping girders. Installed screw heads must not protrude below the bottom of the beveled plate.

OTxDOT August 2017

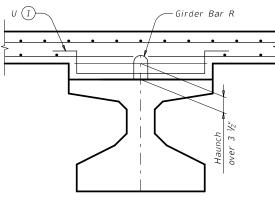


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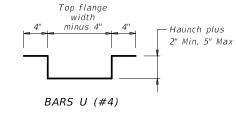
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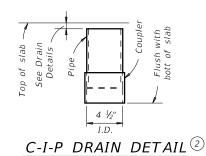
STONEWALL

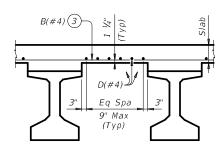
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### HAUNCH REINFORCING DETAIL

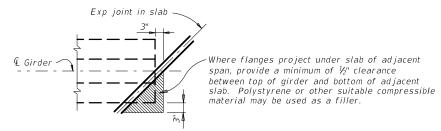




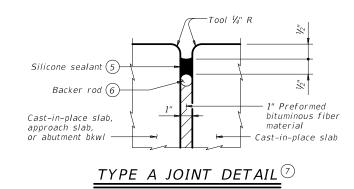


TYPICAL PART TRANSVERSE (4) SLAB SECTION WITHOUT PCP

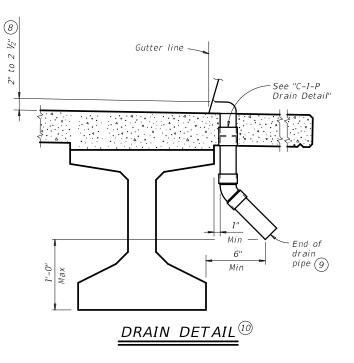
Top reinforcing steel not shown for clarit



### TREATMENT AT GIRDER END FOR SKEWED SPANS



- 1 Space Bars U with girder Bars R in all areas where measured haunch exceeds 3 ½".
- 2 Roughen outside of PVC with coarse rasp or equal to ensure bond with cast-in-place concrete.
- 3 Bars B(#4) spaced at 9" Max with 2" end cover. Overhang option, Contractor's may end alternating bars B(#4) at centerline outside girder.
- 4 Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows:  $\begin{array}{c} \text{Uncoated} \sim \#4 = 1'-7'' \\ \text{Epoxy coated} \sim \#4 = 2'-5'' \end{array}$
- (5) Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- 6 1 ¼" backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- The maximum distance between Type A expansion joints is 100'. See Bridge Layout for location of joints
- 8 Drain entrance formed in rail or sidewalk.
- Water may not be discharged onto girders.
- (10) All drain pipe and fittings to be 4" diameter (Sch 40) PVC. See Item 481 "Pipe for Drains" for pipe, connections and solvent welding. Bend reinforcing steel to clear PVC 1". Drain length and location is as directed by the Engineer. Drains are not permitted over roadways or railways, or within 10'-0" of bent caps. Degrease outside of exposed PVC, apply acrylic water base primer, then coat with same surface finishing material as used for outside girder face. Variations of the above designs, as required for the type of rail used and its location on the structure, may be installed with the approval and direction of the Engineer.



#### GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.
Payment for Type A joint will be as per Item 454, "Bridge Expansion Joints."
All other items (reinforcing steel, drains, etc.) shown on this sheet are subsidiary to other bid items.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are

Reinforcing bar dimensions shown are out-to-out of bar.

### DECK FORMWORK NOTES:

Overhang bracket hangers are limited to a safe working load of 3,600 lbs, applied to and along the axis of a coil rod at 45 degrees from vertical, regardless of higher loads permitted by hanger manufacturers. Do not place a hanger less than 12" from girder end. Space hangers accordingly.

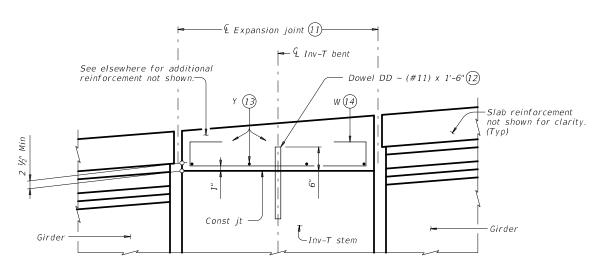
SHEET 1 OF 2



MISCELLANEOUS
SLAB DETAILS
PRESTR CONCRETE I-GIRDERS

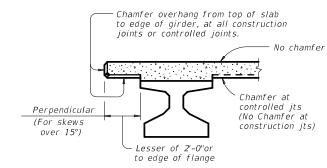
*IGMS* 

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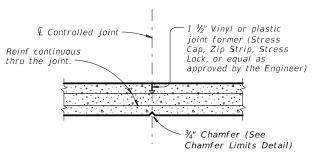


## ¾" Continuous drip bead (both sides of struct)

### DRIP BEAD DETAIL



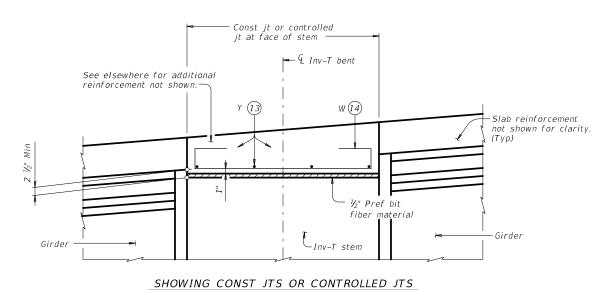
### CHAMFER LIMITS DETAIL (15)



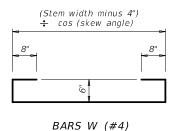
### CONTROLLED JOINT DETAIL

(Saw-cutting is not allowed)

### SHOWING EXPANSION JOINTS



## REINFORCEMENT OVER INV-T BENTS



Dowels DD (#11) spaced at 5 Ft Max. See Inv-T bents for quantity and location.

11) See Layout for joint type.

(13) Space Bars Y (#4) at 12" Max. Use 2" end cover. Number of Bars Y must satisfy spacing limit. Place parallel to bent. Space Bars W at 12" Max (3" from end of cap). Tilt if necessary to maintain cover requirements. Place parallel to longitudinal slab

15 See Span details for type of joint and joint locations.





*MISCELLANEOUS* SLAB DETAILS PRESTR CONCRETE I-GIRDERS

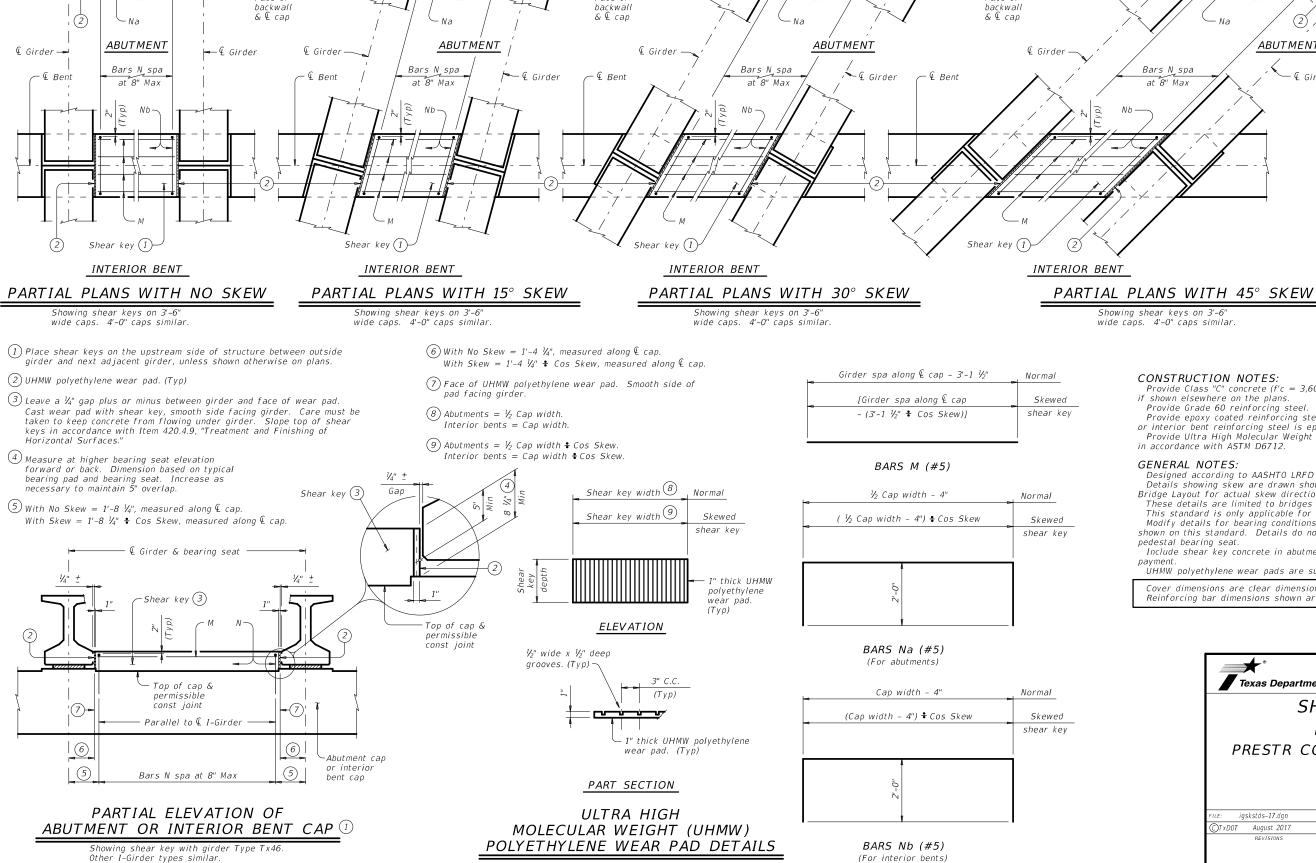
Bridge Division Standard

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* *	ABL	STONEWALL				128



Shear key (1)

Face of



Face of

Shear key (1)

Face of

Shear key (1)

### CONSTRUCTION NOTES:

Provide Class "C" concrete ( $f'c=3,600\,$  psi). Provide Class "C" (HPC) if shown elsewhere on the plans.

Shear key (1)-

Face of

backwall

Provide Grade 60 reinforcing steel

Provide epoxy coated reinforcing steel for shear key if abutment or interior bent reinforcing steel is epoxy coated.

Provide Ultra High Molecular Weight (UHMW) polyethylene wear pads

ABUTMENT

· & Girder

in accordance with ASTM D6712.

Designed according to AASHTO LRFD Bridge Design Specifications. Details showing skew are drawn showing right forward skew. See Bridge Layout for actual skew direction.

These details are limited to bridges skewed 45 degrees and less.

This standard is only applicable for I-Girders.

Modify details for bearing conditions, and girder spacing not shown on this standard. Details do not account for sole plate or pedestal bearing seat.

Include shear key concrete in abutment or bent concrete for

UHMW polyethylene wear pads are subsidiary to Class "C" concrete.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

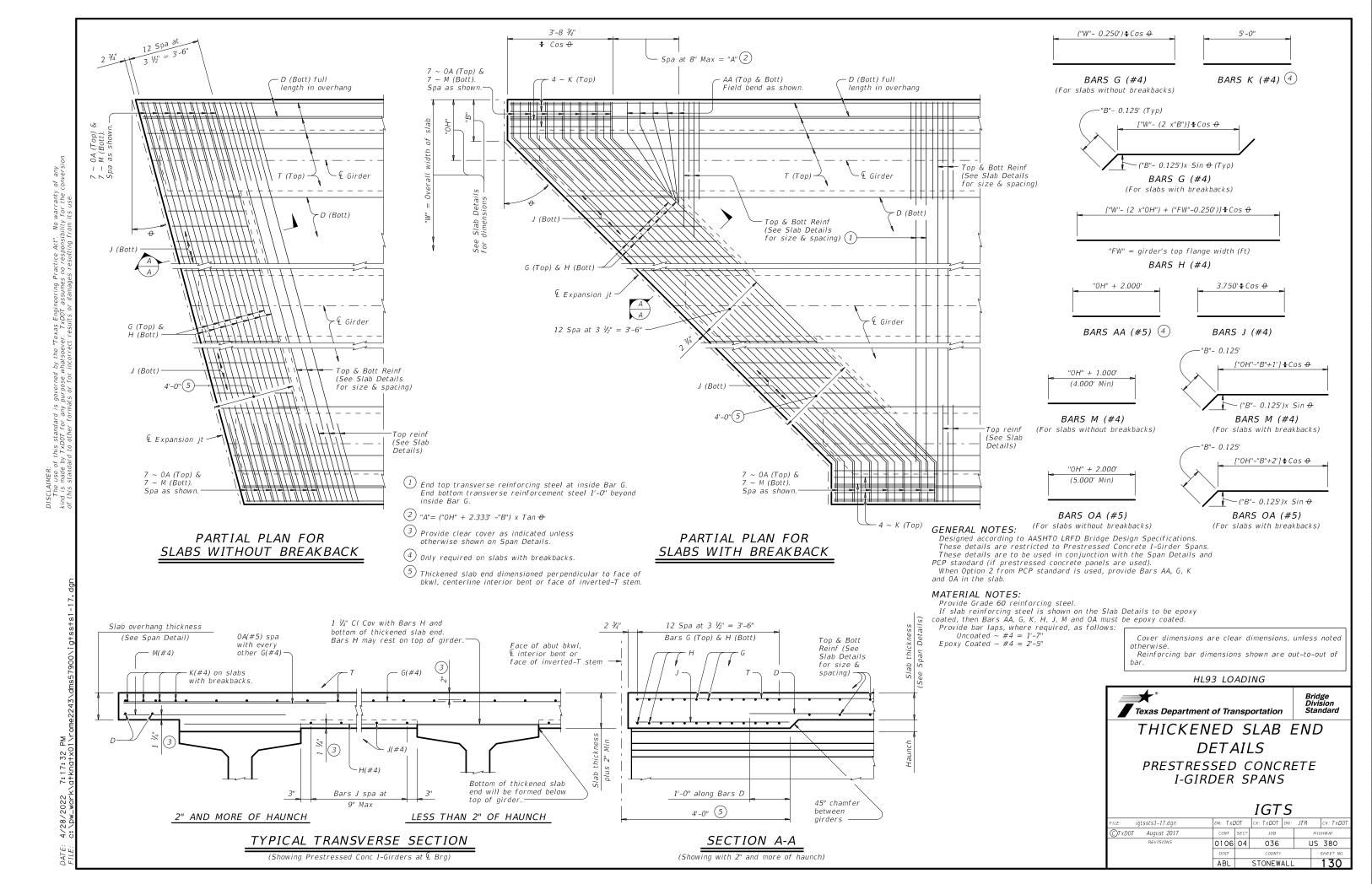


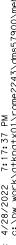
SHEAR KEY **DETAILS** PRESTR CONCRETE I-GIRDERS

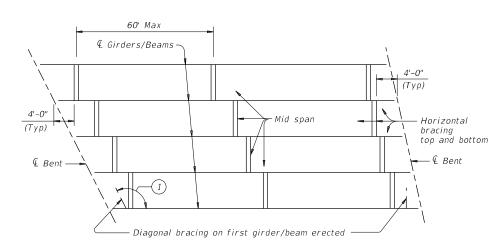
**IGSK** 

Bridge Division Standard

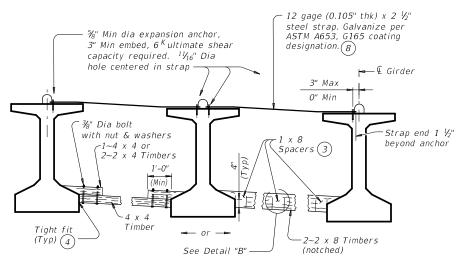
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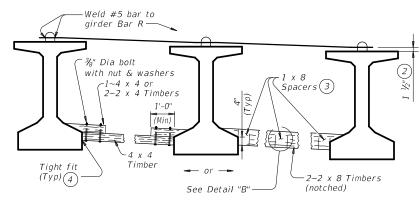


### **ERECTION BRACING**



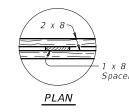
#### FOR ERECTION BRACING, OPTION 1

(This option is not allowed when slab is formed with PMDF or plywood.)

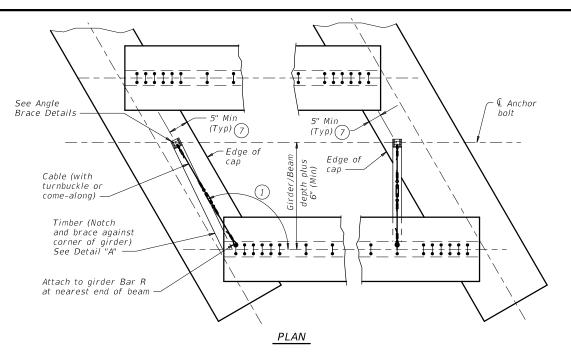


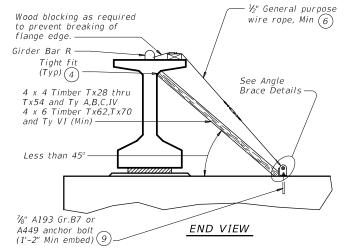
#### FOR ERECTION BRACING, OPTION 2

### HORIZONTAL BRACING DETAILS (5)



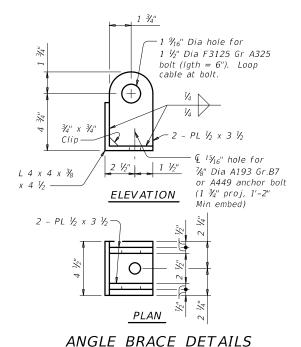
DETAIL "B"





## DIAGONAL BRACING DETAILS (5)

(To be used on both ends of the first girder/beam erected in the span in each phase.)



#### HAULING & ERECTION:

The Contractor's attention is directed to the possible lateral instability of prestressed concrete girders and beams over 130' long, especially during hauling and erection. The use of the following methods to improve stability is encouraged: Locate lifting devices at the maximum practical distance from girder ends; use external lateral stiffening devices during hauling and erection; lift with vertical lines using two machines; and take care in handling to minimize inertial and impact forces.

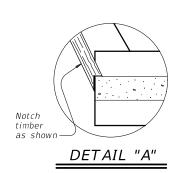
#### **ERECTION BRACING:**

Erection bracing details shown are considered the minimum for fulfilling the bracing requirements of Item 425.

Required erection bracing must be placed immediately after erection of each girder and remain in place until additional bracing as required for slab placement is in place. This standard is needed in all cases to meet requirements for Slab Placement Bracing.

#### PHASED CONSTRUCTION:

Place erection and slab placement bracing for all girders in a phase as shown in these details. For phases after first, also place erection and slab placement bracing between outer girder of completed phase and adjacent girder of current phase. When the phase construction joint is between girders, top bracing can be



- 1) If angle shown exceeds 120 degrees, move diagonal brace to other side of girder/beam and place square to girder/beam. This may prevent exterior girder from being erected first.
- 2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R (See Sheet 2 of 2).
- 3 Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- (4) Use wedges as necessary to obtain tight fit. Nail wedges
- (5) Pressure treated landscape timbers can not be used.
- 6 All hardware used with cable must be able to develop a minimum 25 kips breaking strength. Use thimbles at all loops in cable. Install cable clamps with saddles bearing against the live end and U-bolts bearing aginst the dead end.
- (7) It is acceptable to tie anchor bolts to cap reinforcement.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- (9) Anchor bolt may be drilled and epoxied in place. Provide 25k

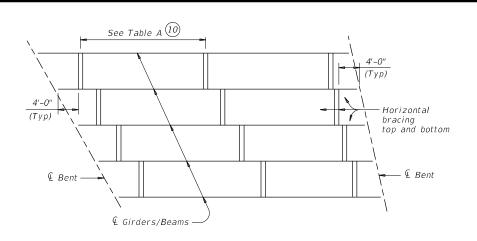
### SHEET 1 OF 2



MINIMUM ERECTION AND BRACING REQUIREMENTS PRESTRESSED CONCRETE I-GIRDERS AND I-BEAMS

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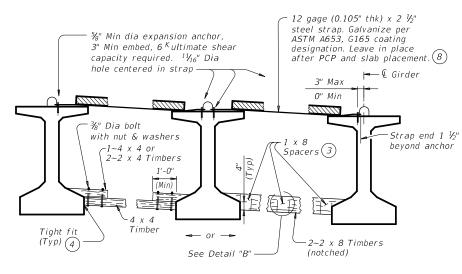


SLAB PLACEMENT BRACING

OPTION 1-RIGID BRACING (STEEL STRAP)							
	Maximum Bracing Spacing						
Girder or Beam Type	Slab Overhang less than 4'-0" (11)	Slab Overhang 4'-0" and greate					
Tx28	⅓ points	½ points					
Tx34	¼ points	1/4 points					
T x 40	¼ points	√ <sub>8</sub> points					
Tx46	⅓ points	⅓ points					
Tx54	⅓ points	½ points					
Tx62	⅓ points	½ points					
Tx70	¼ points	⅓ points					
А	⅓ points	⅓ points					
В	⅓ points	½ points					
С	∜a points	⅓ points					
IV	⅓ points	⅓ points					
VI	V₄ points	1/2 noints					

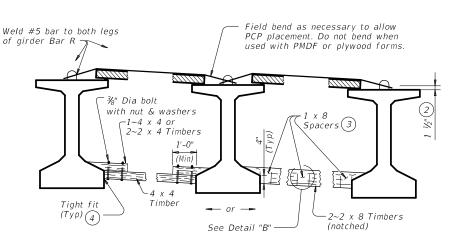
EL STRAP)	OPTION 2-FLEXIBLE BRACING (NO. 5 OVER PCP)							
ng Spacing		Maximum Bracing Spacing						
Slab Overhang 4'-0" and greater (11)	Girder or Beam Type	Slab Overhang less than 4'-0" (11)	Slab Overhang 4'-0" and greater (11)					
⅓ points	T x 28	${\cal V}_{\!\!4}$ points	${}^{\!$					
⅓ points	Tx34	¼ points	$\mathcal{V}_{\!\scriptscriptstyle{\mathcal{B}}}$ points					
$lat{V}_8$ points	T x 40	¼ points	⅓ points					
⅓ points	T x 46	¼ points	$lay{1}{8}$ points					
⅓ points	T x 54	¼ points	⅓ points					
$lay{1}{8}$ points	Tx62	¼ points	$lay{1}{8}$ points					
½ points	T x 7 0	¼ points	$V_8$ points					
√ <sub>8</sub> points	A	2.0 ft	1.5 ft					
⅓ points	В	3.0 ft	2.0 ft					
$lay{1}{8}$ points	С	4.5 ft	2.0 ft					
⅓ points	IV	¼ points	4.0 ft					
⅓ points	VI	¼ points	4.0 ft					

TABLE A



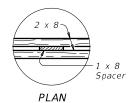
### FOR SLAB PLACEMENT BRACING, OPTION 1 - RIGID

(Showing slab formed with PCP. This option is not allowed when slab is formed with PMDF or plywood.)



FOR SLAB PLACEMENT BRACING, OPTION 2 - FLEXIBLE (Showing slab formed with PCP.)

HORIZONTAL BRACING DETAILS (5)



DETAIL "B"

- 2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R.
- 3 Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- 10 Bracing spacing ( 14 and 16 points ) measured between first and last typical brace location.
- (1) Measure slab overhang from centerline of girder or beam. When overhang varies in span, determine bracing spacing based on largest overhang.

#### SLAB PLACEMENT BRACING:

The details for slab placement bracing are considered minimum for fulfilling the requirements of Specification Items 422 and 425. Required slab placement bracing must remain in place until slab concrete has attained a compressive strength of 3000 psi.

#### GENERAL NOTES:

Bracing details for spans longer than 150' are not provided. The Contractor must submit proposed bracing details for such conditions to the Engineer for approval prior to erection.

Systems equal to or better than those shown may be used provided details of such systems are submitted to and approved by the Engineer prior to erection.

Use of these systems or details does not relieve the Contractor of the responsibility for the adequacy of the bracing and the safety of the structure.

Removal of bracing for short periods of time to align girders and beams is permissible.

All turn-buckles, come-alongs, anchors and other connections must be capable of developing the full strength of the cable

Furnish anchor bolts and nuts in accordance with Item 449, "Anchor Bolts".

SHEET 2 OF 2



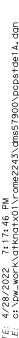
MINIMUM ERECTION AND BRACING REQUIREMENTS

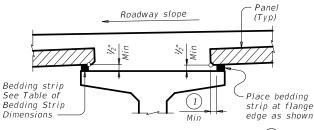
Bridge Division Standard

PRESTRESSED CONCRETE I-GIRDERS AND I-BEAMS

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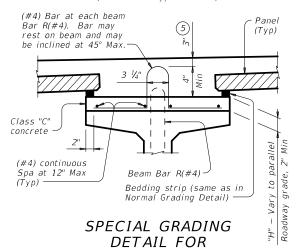
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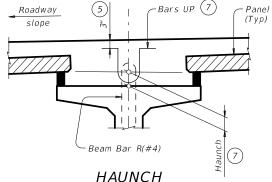


### NORMAL GRADING DETAIL 3

Showing prestressed concrete I-girders (Other beam types similar)

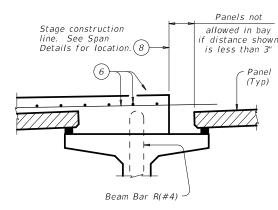


CONCRETE BEAMS
Showing prestressed concrete I-girders.
(Other beam types similar)



## REINFORCING DETAIL

Showing prestressed concrete I-girders. (Other beam types similar)



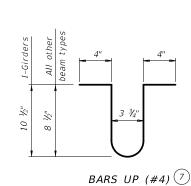


TABLE OF BEDDING STRIP

**DIMENSIONS** 

Min

1/2"

1/2"

1/2"

1/2"

1/2"

1/2"

1/2"

1/2"

1/3"

WIDTH

1" (Min.

1 1/4"

1 1/2"

1 3/4"

2 1/4"

2 1/2"

2 3/4"

3" (Max

HEIGHT (4)

Max

2"

2 1/2"

3 1/2"

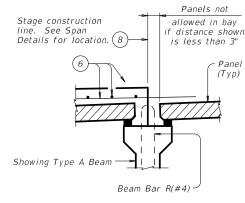
4"

4 1/2" (.

5" (2

5 1/2" (2

6"



PRESTR CONC I-GIRDERS

PRESTR CONC I-BEAMS

### STAGE CONSTRUCTION LIMITATIONS

(Other beam types similar)

 $1 \ 2$ " Min for I-giders,  $1 \ \frac{1}{2}$ " Min for all other beam types.

(2) Allowed for I-girders, not allowed on other beam types.

3 To reduce the quantity of cast-in-place concrete, bedding strip thickness may be increased in '\mathcal{N}'' increments. Bedding strips must be comprised of one layer. Bond bedding strips to the beams with an adhesive compatible with bedding strips. Bedding strips over 2.5" high may need to be bonded to panels. The same thickness strip must be used under any one panel edge and the maximum change in thickness between adjacent panels is '\mathcal{V}''. Alternatively, bedding strips may be cut to grade. Panels may be supported by an alternate method, using a commercial product, if approved by the Engineer of Bridge Design, Bridge Division. If bedding strips exceed 6" high for I-Girders, 4" high for all other beam types, use Special Grading Detail for Concrete Beams or submit an alternate method to the Bridge Division for approval.

(4) Height must not exceed twice the width.

(5) Provide clear cover as indicated unless otherwise shown on Span Details.

(6) See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.

7 Space Bars UP(#4) with Beam Bars R(#4) in all areas where measured haunch exceeds 3 ½" with I-girders, and 3" for all other beam types. Epoxy coating for Bars UP is not required.

(8) Do not locate construction joints on top of a panel.

9 Butt adjacent bedding strips together with adhesive. Cut v-notches, approx 1/4" deep, in the top of the bedding strips at 8' o.c..

Seal joint between panels when gap exceeds ¼" with polyurethane sealant or expanding foam sealer.

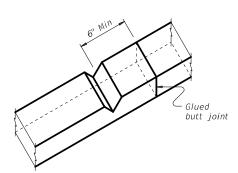
Make seal flush with top of panel.

Allowable Gap

### PANEL JOINTS

(Panel reinforcing not shown for clarity.

The gap cannot be considered as a panel fabrication tolerance. Adjust panel placement to minimize joint openings.)



BEDDING STRIP DETAIL 9

#### CONSTRUCTION NOTES:

Erected panels must bear uniformly on bedding strips of extruded polystyrene placed along top flange edges.

Placing panels to minimize joint openings is recommended.

If additional blocking is needed, special grading details for supporting the panels and extra reinforcing between beam and slab will be considered subsidiary to deck construction.

Bars U, shown on PCP-FAB, may be bent over or cut off if necessary.

Care must be taken to ensure proper cleaning of

Care must be taken to ensure proper cleaning of construction debris and consolidation of concrete material under the edges of the panels. Bedding strips must be placed at beam flange edges so that adequate space is provided for the mortar to flow a minimum of  $1 \frac{1}{2}$ " under the panels as the slab concrete is placed. To allow the proper amount of mortar to flow between

To allow the proper amount of mortar to flow between beam and panel, the minimum vertical opening must be at least ½". Roadway cross-slope reduces the opening available for entry of the mortar. Bedding strips varying in thickness across the beam are therefore required.

For clear span between U-beams less than or equal to 18", see Permissible Slab Forming Detail on Miscellaneous Slab Detail sheets, UBMS.

#### MATERIAL NOTES:

Provide Grade 60 reinforcing steel in the cast-in-place slab. See Table of Reinforcing Steel for size and spacing of reinforcement.

If the top and bottom layer of reinforcing steel is shown on the Span Details to be epoxy coated, then the D, E, P, & Z bars must be epoxy coated.

Provide bar Laps, where required, as follows: Uncoated ~ #4 = 1'-7" Epoxy Coated ~ #4 = 2'-5"

### GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

Panel placement may follow either Option 1 or Option 2 except Option 1 must be used if the skew exceeds 45 degrees.

Use of Prestressed Concrete Panels is not permitted for horizontally curved steel plate or tub girders. See Span Details for other possible restrictions on their use.

These details are to be used in conjunction with the Span Details, PCP-FAB and other applicable standard drawings.

When panel support (bedding strips) deviates from what is shown herein, provide details signed and sealed by a professional Engineer.

Any additional reinforcement or concrete required on this standard is considered subsidiary to the bid Item "Reinforced Concrete Slab".

Cover dimensions are clear dimensions, unless noted otherwise.

Reinforcing bar dimensions shown are out-to-out of

HL93 LOADING

SHEET 1 OF 4

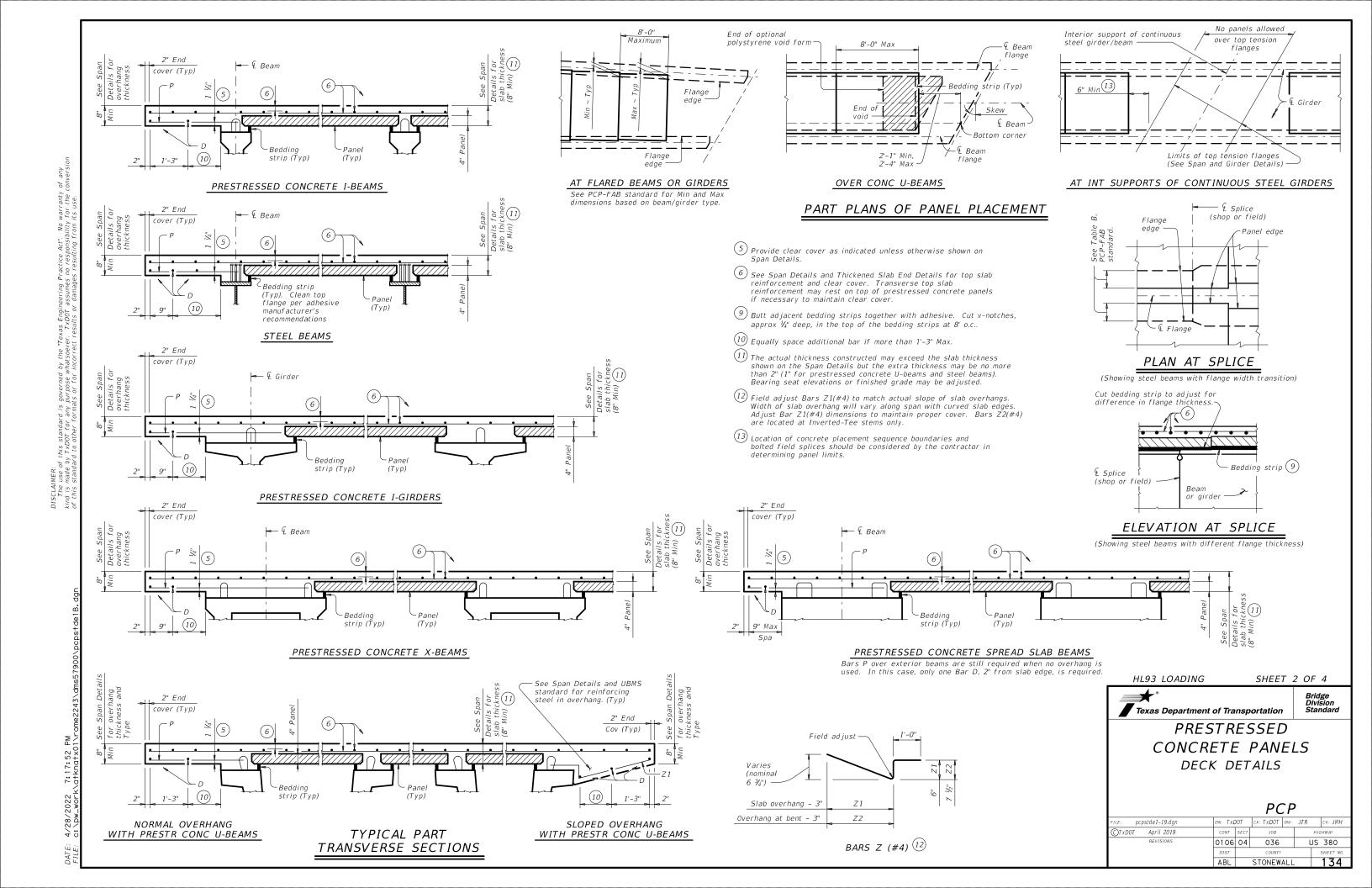


Bridge Division Standard

PRESTRESSED
CONCRETE PANELS
DECK DETAILS

PCP

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©TxDOT April 2019	CONT	SECT	JOB			HIGH	HWAY
REVISIONS	0106	04	036		US 380		380
	DIST	COUNTY			5	SHEET NO.	
	ABL	L STONEWALL		133			



& Bent-

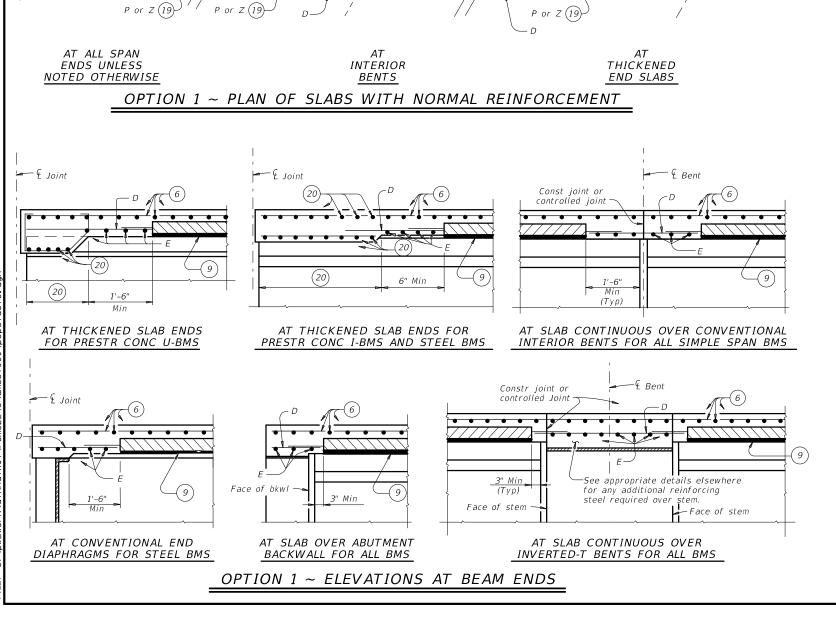
Prestressed

 $Panel \sim (Typ)$ 

Concrete

E(17)

Place one bar E parallel to edge of slab



P or Z (19)

controlled ioint (See Span Details) P or Z (19)

£ Beam

Flange

Showing thickened slab

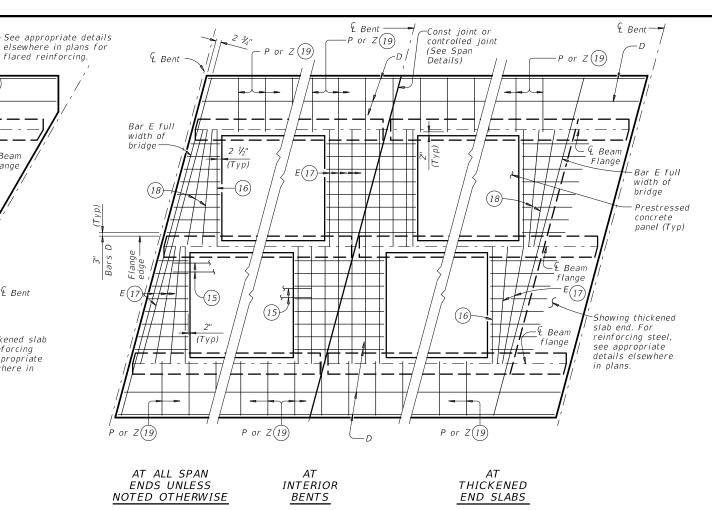
end. For reinforcina

details elsewhere in

steel, see appropriate

. Beam flanae

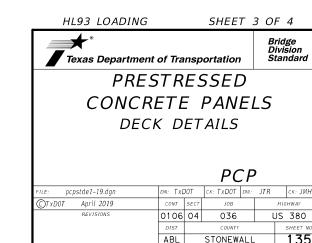
flange

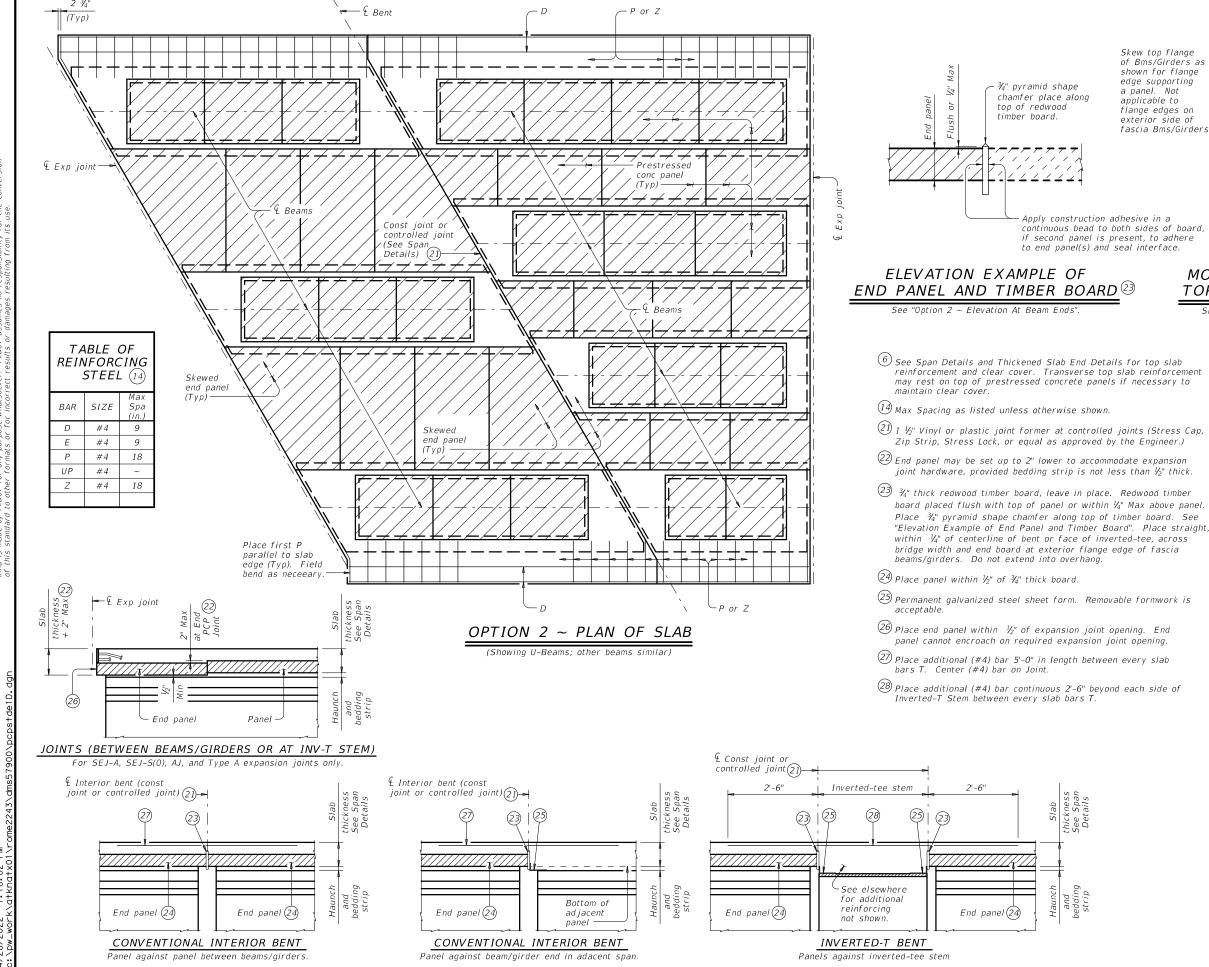


### OPTION 1 ~ PLAN OF SLABS WITH SKEWED REINFORCEMENT

- See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover.
- (9) Butt adjacent bedding strips together with adhesive. Cut v-notches, approx 1/4" deep, in the top of the bedding strips at 8' o.c.
- (14) Max Spacing as listed unless otherwise shown.
- 15) At connection with cast-in-place slab, extend longitudinal panel reinforcement. See PCP-FAB for details.
- ig(16ig) Maintain one Bar E(#4) parallel to panel ends (Typ).
- (17) Bars E(#4) not continuous over beam flanges must overlap beam flange 6" Min.
- (18) Add flared Bars E(#4) (Min Spa = 6", Max Spa = 12") as required at panel ends.
- (19) Where possible, Bars E(#4) may be extended into overhangs to replace Bars P(#4). Bars Z(#4) are required for sloped overhangs with U-Beams.
- (20) See appropriate thickened slab end details for reinforcing and limits of thickened slab end.







SPECIAL OPTION 2 CONSTRUCTION NOTES:

OPTION 2 ~ SHOWING MODIFICATION TO BEAM/GIRDER

TOP FLANGE FOR SKEWS OVER 5° Showing I-Bm/I-Girder, U-Bms and Steel Bms simila

Bottom Flange

Face of Web

ace of Web

¶ Interior Bent, Face

of Abut Bkwl or Face

of Inverted-T Stem

Skew top flange of Bms/Girders as shown for flange

edge supporting

flange edges on

exterior side of fascia Bms/Girders.

a panel. Not

applicable to

When Option 2 is chosen bottom mat of thickened end slab reinforcing is not required. Use the same top mat as shown on the Thickened Slab End Details sheet.

Placing panels adjacent to expansion joints and bent centerlines prior to completing interior panel placement is recommended. Saw cutting panels to fit is acceptable when approved by the Engineer. Minimum distance from a saw cut edge to a panel strand is 1 ½".

Do not extend the longitudinal panel reinforcement into the cast-in-place slab.

Top flanges of beams and girders on skewed bridges

must be modified as shown on this drawing. The Contractor is responsible for coordinating this modification with the beam fabricator prior to submitting shop drawings for approval.

Fabricator may optionally skew the whole end. When electing to skew whole end, girder end details and bearing type at conventional interior bent must be changed to use condition at abutment. Fabricator must coordinate change in bearing type, bearing centerline location, and dowel location with Engineer and Contractor. Show appropriate changes on girder and bearing shop drawings.

Bending of anchor studs of expansion joints shown on standards AJ, SEJ-A and SEJ-S(0) is permissible if necessary to clear top of end panels. The Contractor is responsible for coordinating modifications with the joint fabricator. Submit shop drawings for approval when modifications to expansion joint hardware are

Bedding strips under skewed end panels must conform to the requirements of Item 422 except their minimum compressive strength must be 60 psi. Provide Bars AA, G, K and OA from standard IGTS

HL93 LOADING

in the slab.

SHEET 4 OF 4

Bridge Division Standard



**PRESTRESSED** CONCRETE PANELS DECK DETAILS

PCP

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TxDOT April 2019	CONT	SECT	JOB			HIGHWAY
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	ABL		STONEW	ALL	.	136



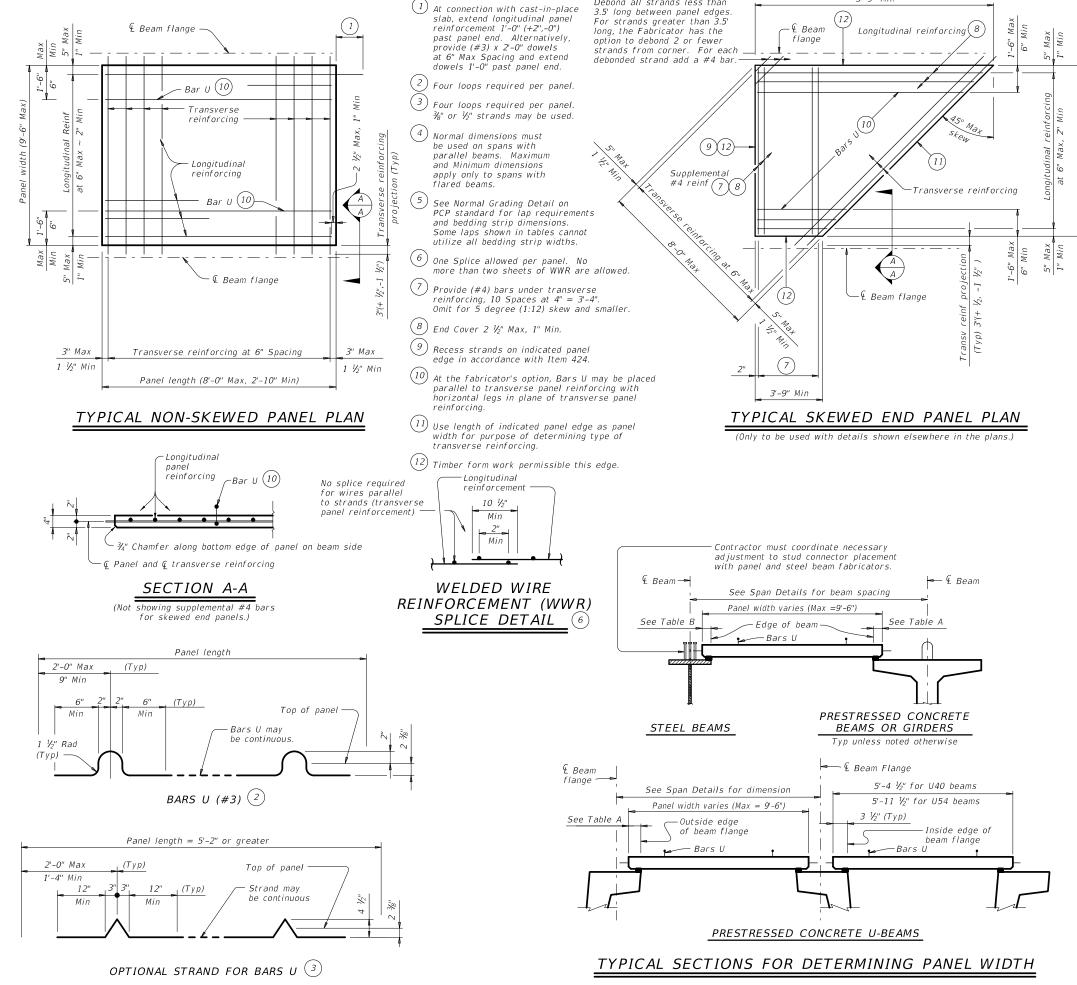


	TABLE	Ξ A (Δ	1)(5)	TA	BLE B	(4)(5	5)
Beam Type	Normal (In.)	Min (In.)	Max (In.)	Top Flange Width	Normal (In.)	Min (In.)	Max (In.)
Α	3	2 ½	3 ½	11" to 12"	2 ¾	2 ½	2 3/2
В	3	2 ½	3 ½	Over 12" to 15"	3 1/4	3	3 1/4
С	4	3	4 1/2	Over 15" to 18"	4	3	4 3/4
IV	6	4	7 ½	Over 18"	5	3 1/2	6 1/4
VI	6 ½	4 1/2"	8 ½	,			
U40 - 54	5 ½	5 ½	7				
Tx28-70	6	5	7 ½				
XB20 - 40	4	3	4 1/2				
XSB12 - 15	4	3	4 1/2				

#### GENERAL NOTES:

3'-9" Min

Debond all strands less than

Provide Class H concrete for panels. Release strength f'ci=3,500 psi. Minimum 28 day strength f'c=5,000 psi.

Provide 3/4" chamfer along bottom edge of panel on beam side.

Do not use epoxy-coated reinforcing steel bar or strand in panels. Remove laitance from top panel surface. Finish top of panel to a roughness between a No. 6 and No. 9 concrete

surface profile, inclusive, as specified by the International Concrete Repair Institute (ICRI).

Shop drawings for the fabrication of panels will not require the Engineer's approval if fabrication is in accordance with the details shown on this

A panel layout which identifies location of each panel must be developed by the Fabricator. Permanently mark each panel in accordance with the panel layout. A copy of the layout is to be provided to the Engineer.

#### TRANSVERSE PANEL REINFORCEMENT:

For panel widths over 5', use %" or  $\frac{1}{2}$ " Dia (270k) prestressing strands with a tension of 14.4 kips per strand.

For panel widths over 3'-6" up to and including 5', use  $\frac{3}{6}$ " or  $\frac{1}{2}$ " Dia (270k) prestressing strands with a tension of 14.4 kip per strand. Optionally, (#4) Grade 60 reinforcing bars may be used in lieu of prestressed strands. For panel widths up to 3'-6", use (#4) Grade 60 reinforcing bars (prestressed strands alone are not allowed)

Place transverse panel reinforcement at panel centroid and space at 6" Max.

### LONGITUDINAL PANEL REINFORCEMENT:

Any of the following options may be used for longitudinal panel reinforcement:

- 1. (#3) Grade 60 reinforcing steel at 6" Max Spacing. No splices allowed.
- 2. %" Dia prestressing strands at 4 ½" Max Spacing (unstressed). No splices allowed.
- 3.  $\frac{1}{2}$ " Dia prestressing strands at 6" Max Spacing (unstressed). No splices allowed.
- 4. Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) providing 0.22 sq in per foot of panel width. Wires larger than D11 not permitted Provide transverse wires to ensure proper handling of reinforcing. One

splice per panel is allowed. See WWR Splice Detail.

No combination of longitudinal reinforcement options in a panel is allowed. Place longitudinal panel reinforcement above or below transverse panel reinforcement. Must be placed above transverse panel reinforcement for skewed end panels with supplemental (#4) reinforcement.

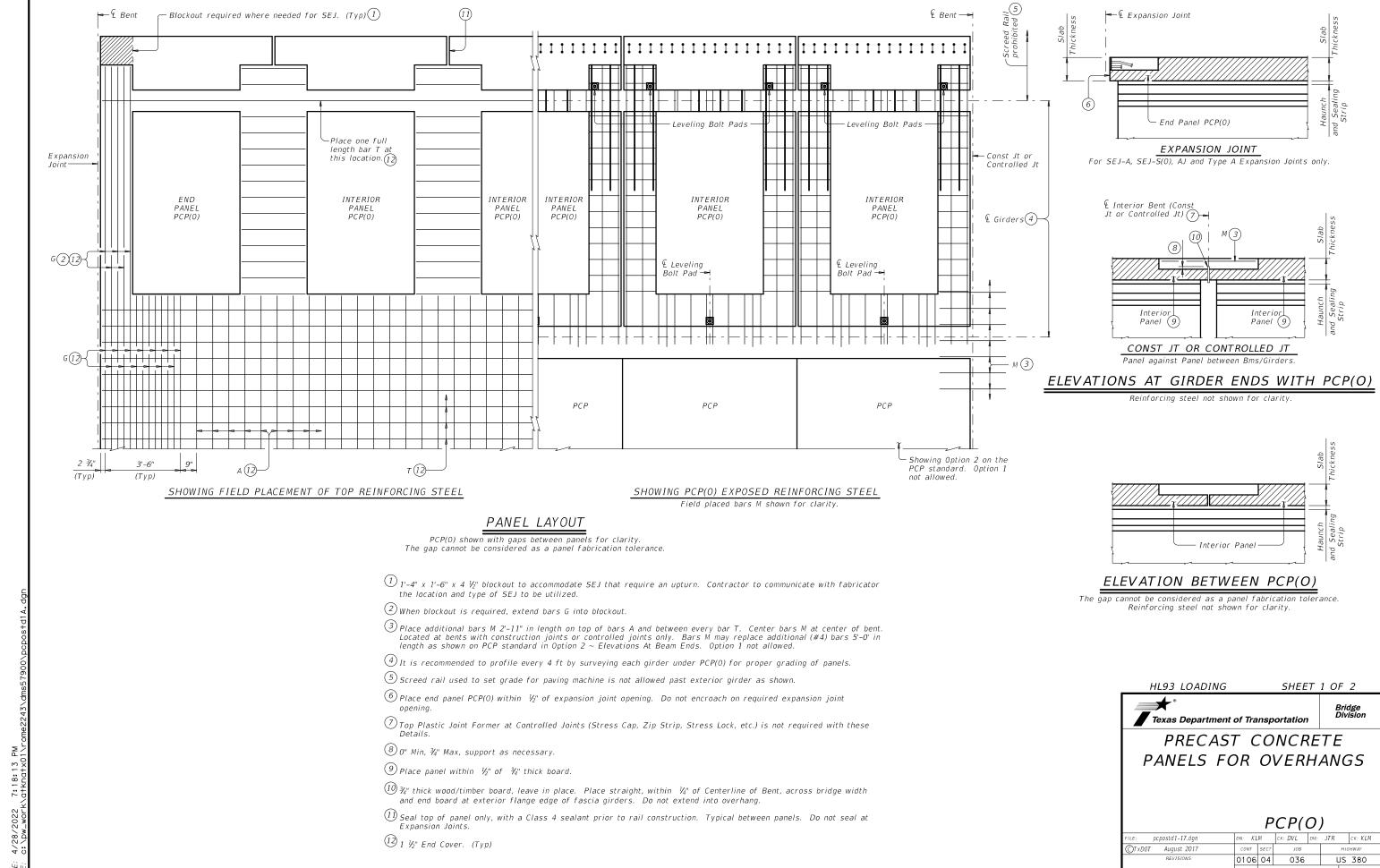
HL93 LOADING



PRESTRESSED CONCRETE PANEL FABRICATION **DETAILS** 

PCP-FAB

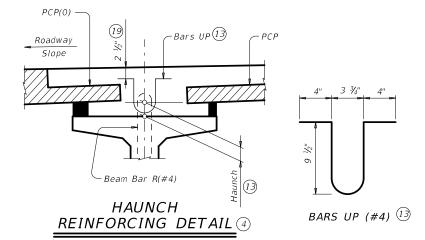
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©TxD0T April 2019	CONT	SECT	JOB			HIGHWAY
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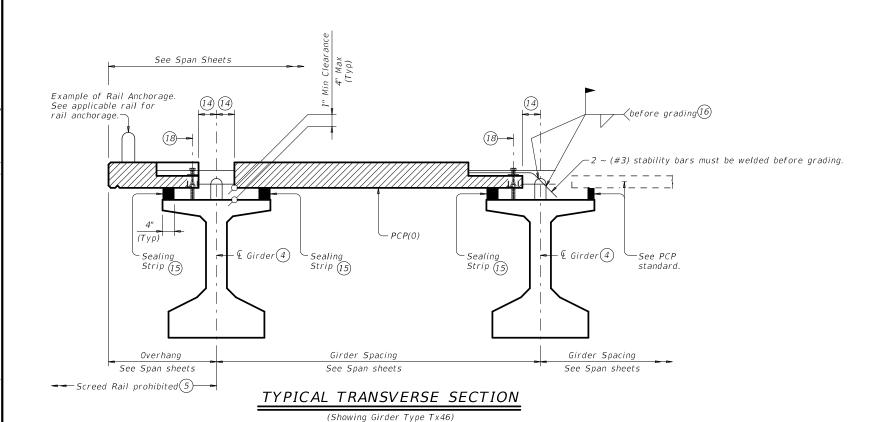


STONEWALL

BAR TABLE								
BAR	SIZE	MAX SPA (IN)						
A (12)(17)	#4	9"						
G (12)(17)	#4	3½"						
М	#4	9"						
т (12(17)	#4	9"						

- igaingle It is recommended to profile every 4 ft by surveying each girder under PCP(0) for proper grading of panels.
- $^{igotimes}$  Screed rail used to set grade for paving machine is not allowed past exterior girder as shown.
- (12) 1 ½" End Cover on bars. (Typ)
- $rac{\textcircled{1}}{3}$  Space bars UP(#4) with girder bars R(#4) in all areas where measured haunch exceeds 3  $rac{1}{2}$ " with Prestressed Concrete I-Girders. Epoxy coating for Bars UP is not required.
- (14) 6" plus or minus.
- Delace sealing strip at flange edge as shown. Butt adjacent sealing strips longitudinally together with adhesive. Use pencil vibrators with concrete placement over girder and between sealing strips to avoid rupturing sealing strips. Cut sealing strips 2" higher than anticipated haunch thickness and compress
- (16) (#3) Panel bars F must be field bent and welded to the R bars in girder. Two bars F per panel.
- $\widehat{\mathbb{U}}$  Field placed bars that are allowed to be lapped. Reinforcing steel that protrudes from panels are not considered bars to be lapped. See "Material Notes" for applicable bar laps
- $(18)^{\circ}$ Leveling Bolt Pad. 1" Dia Coil Rod or 1" Dia Coil Bolt shown, are furnished by the contractor. After grading each PCP(0) panel with the 1" Dia coil rods or coil bolts, secure each panel in its final resting position (plastic shims, welding, etc) and remove all 1" Dia coil rods or coil bolts for the cast-in-place concrete. Coil rods/bolts may be left in place at contractor's option. If coil rods/bolts are left in place, coil rods/bolts must have at least 2  $\frac{1}{2}$ " of cover to top of finish grade. Grading bolts are inadequate to carry all conceivable screed/construction loads. Panel support method must be calculated, location identified, and placed on shop drawings. Method chosen to support panels must be adequate for all construction loads. Panel support method must be placed/constructed after final grading and before screed rail placement.
- 19 Unless shown otherwise on Span Details.





#### CONSTRUCTION NOTES:

Placing panels adjacent to expansion joints and bent centerlines prior to completing interior panel placement is recommended.

Ensure proper cleaning of construction debris and consolidation of concrete mortar under the edges of the panels. Place sealing strips at girder flange edges so that adequate space is provided for the mortar to flow a minimum of 8" transversely under the panels as the slab concrete is placed.

Panel placement with Option 1 on the PCP standard is not allowed. It is recommended to profile every 4 ft by surveying each girder under PCP(0) for proper grading of panels.

To allow the proper amount of mortar to flow between girder and

panel, maintain a minimum vertical opening of 1". Roadway cross-slope reduces the opening available for entry of the mortar. Sealing strips vary in thickness along girder are therefore required.

Seal the top panel with a Class 4 sealant as shown in the Panel Lavout.

#### MATERIAL NOTES:

Provide Grade 60 reinforcing steel in cast-in-place slab. See Table of Reinforcing Steel for size and spacing of reinforcement.

If the reinforcing steel is shown on the Span Details to be epoxy coated, then epoxy coat bars A, G, M, & T.

Provide bar laps, where required, as follows:

Uncoated  $\sim #4 = 1'-7''$ Epoxy Coated  $\sim #4 = 2'-5''$ 

Provide sealing strips comprised of one layer low density polyurethane (1.0 Lbs density) foam sealing strips or equivalent. Oversize the height of sealing strips by 2". Bond sealing strips to the girder with 3M Scotch ® 4693 or equivalent adhesive compatible with sealing strips.

#### GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. These details can be used as an option to construct the deck overhang when noted on the Span details and in conjunction with the PCP(0)-FAB, PCP and applicable Standard sheets.

These details are only applicable for Prestr Conc I-Girders. Any additional reinforcement or concrete required on these details is subsidiary to the bid Item "Reinforced Concrete Slab".

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar

HL93 LOADING

SHEET 2 OF 2

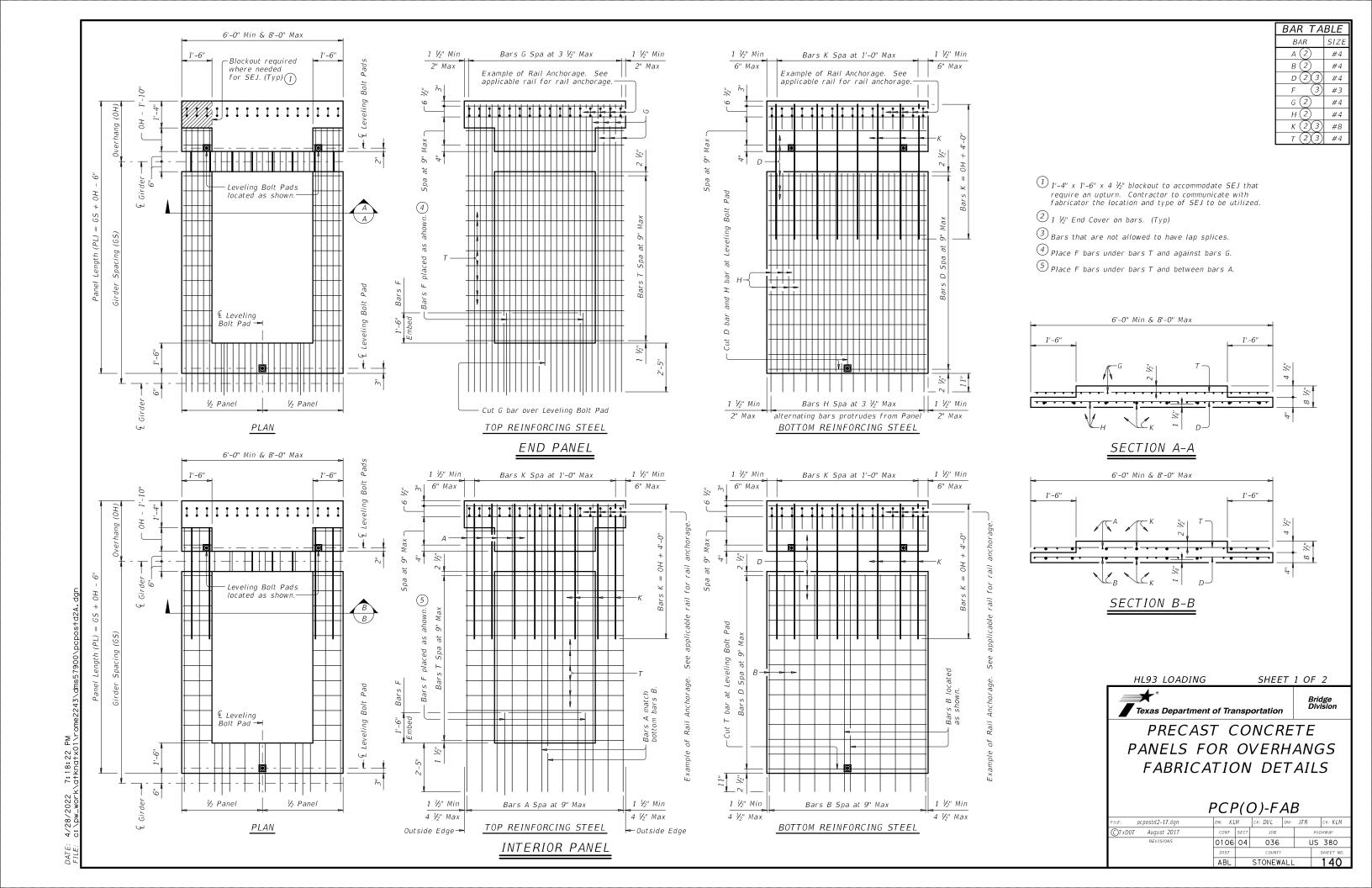
Bridge Division

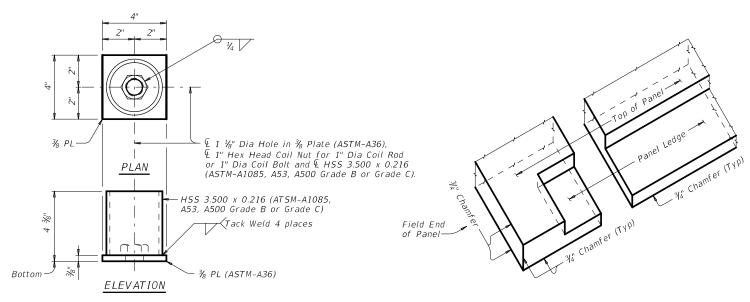


PRECAST CONCRETE PANELS FOR OVERHANGS

PCP(O)

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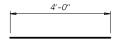


### LEVELING BOLT PAD DETAILS

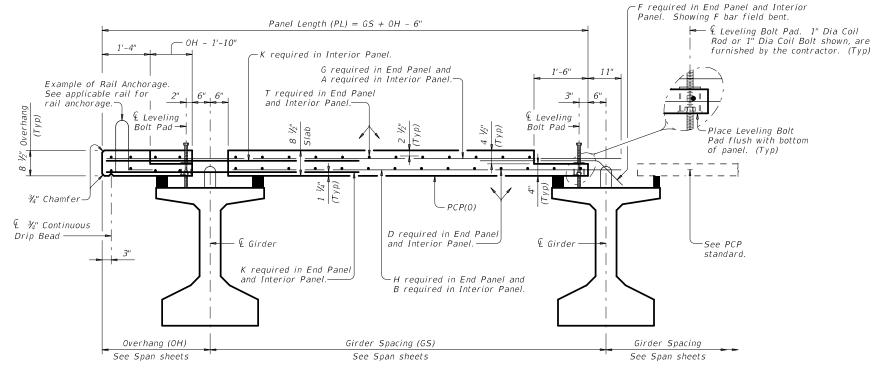
Galvanize if epoxy coated reinforcing steel is used in slab. Do not oil this assembly.

### ISOMETRIC VIEW AT CORNER OF PANEL

Showing Typical Chamfers on Panel. Drip Bead and reinforcing steel not shown for clarity.



BARS F



### TYPICAL TRANSVERSE SECTION

(Showing Girder Type Tx46)

#### CONSTRUCTION/FABRICATION NOTES:

Remove laitance from top panel surface. Finish top surface area of panel with a broom finish. Finish top ledge of panel to a roughness between a No. 6 and No. 9 concrete surface profile, inclusive, as specified by the International Concrete Repair Institute (ICRI).

Provide ¾" concrete chamfers as shown on these details.

Do not lap splice bars D, F, K & T. Bars A, B, G & H, may
be spliced with only one lap splice allowed on each bar.

Panels must be fabricated by a fabricator meeting the

Panels must be fabricated by a fabricator meeting the requirements of DMS 7300 for Multi-Project Nonstressed Member Fabrication Plant.

#### MATERIAL NOTES:

Provide Class H concrete (f'c=4000 psi) in panels. Provide Class H (HPC) concrete for panels if required elsewhere in plans. Maximum large aggregate size is 1".

Provide material as capture on this standard for the

Provide material as shown on this standard for the Leveling Bolt Pad.

Provide Grade 60 conventional reinforcing steel.
Provide epoxy coated reinforcement for bars A, B, D, G,
H, K & T if slab reinforcement is epoxy coated.

An equal area and spacing of deformed Welded Wire Reinforcement (WWR) ASTM-A1064 may be substituted for bars A, B, D, G, H & T, unless otherwise noted. Bars F and K can not be replaced with WWR.

K can not be replaced with WWR. Galvanize leveling bolt pad assembly if epoxy-coated reinforcing steel is used in slab.

#### GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. These details are only applicable for Prestr Conc I-Girders. Any additional reinforcement, lifting devices or epoxy coated reinforcement required on these details are subsidiary to the bid Item "Reinforced Concrete Slab".

See railing details for rail anchorage in panel overhang. A panel layout which identifies location of each panel must be developed by the fabricator. Permanently mark each panel in accordance with the panel layout. A copy of the layout is to be provided to the Engineer.

Submit stable lifting methods and devices to the Engineer for approval.

Shop drawings for the fabrication of panels will require the Engineer's approval.

Cover dimensions are clear dimensions, unless noted therwise.

Reinforcing bar dimensions shown are out-to-out of har

HL93 LOADING

SHEET 2 OF 2

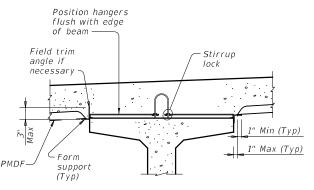


DIVISION

PRECAST CONCRETE
PANELS FOR OVERHANGS
FABRICATION DETAILS

PCP(O)-FAB

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	ABL		STONEW	ALL		141



# PRESTR CONC I-BEAMS AND I-GIRDERS WITH STIRRUP LOCKS

flush with edge

1" Max (Typ)

of beam

Stirrup lock -

– Form

support

U-BEAMS WITH STIRRUP LOCKS

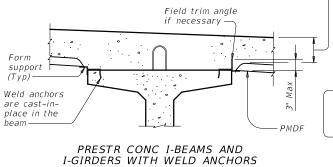
- Form supports -

Field trim angle

if necessary

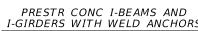
Intermittent

weld



Slab thickness.

See Span Details (1)



support

PMDF

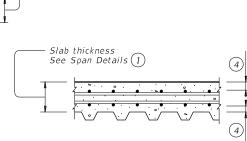
cast-in-place

Weld anchors are

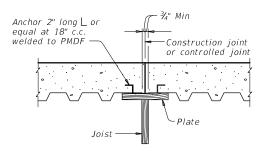
Slab thickness, See Span Details (1)

Field trim angle

if necessary —



### TYP LONGITUDINAL SLAB SECTION



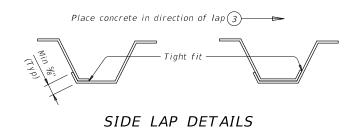
Note: In spans where PMD forms are used, timber forms must be used at construction joints. Adequate provision must be made to support edge of metal form and to provide anchorage of metal form to slab concrete where joined to wood forms.

### SECTION THRU CONSTRUCTION JOINT

#### FOR PRESTR CONC U-BEAM AND STEEL GIRDER BRIDGES:

Unless shown elsewhere in the plans, size, spacing, and orientation of bottom mat of slab reinforcement must match the top mat of reinforcing shown on the span details except all bottom mat bars are to be #5. Bottom mat reinforcement nd additional concrete is subsidiary to Item 422 "Concrete Superstructures." FOR PRESTR CONC TX-GIRDER BRIDGES:

See Miscellaneous Slab Details, Prestr Concrete I-Girders (IGMS) standard sheet for bottom mat reinforcing



- (1) Slab thickness minus ⅓" if corrugations match reinforcing bars.
- (2) Welding of form supports to tension flanges will not be permitted. Other methods of providing wind hold down resistance for PMDF in tension flange zones will be considered. At least one layer of sheet metal must be provided between the flange and the weld joint.
- (3) The direction of concrete placement will be such that the upper layer of the form overlap is loaded first.
- (4) See Span details for cover requirements.

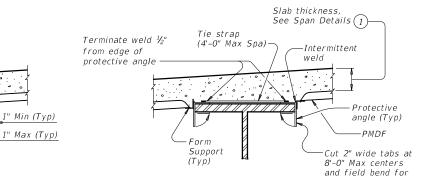
GENERAL NOTES: Steel for Permanent Metal Deck Forms (PMDF) and support angles shall conform to ASTM A653, structural steel (SS), with coating designation G165. Steel must have a minimum yield strength of 33 ksi. Minimum thickness of PMDF is 20 gage

and that of support angles and protective angles is 12 gage.
Submit two copies of forming plans for PMDF to the Engineer These plans must show all essential details of proposed form sheets, closures, fasteners, supports, connectors, special conditions and size and location of welds. These plans must clearly show areas of tension flanges for steel beams and provisions for protecting the tension flanges from welding notch effects by inclusion of separating sheet metal or other positive method. These plans must be designed, signed, and sealed by a licensed professional engineer. Department approval of these plans is not required, but the Department reserves the right to require modifications to the plans. The Contractor is responsible for the adequacy of these plans

The details and notes shown on this standard are to be used

as a guide in preparation of the forming plans.

All material, labor, tools and incidentals necessary to form a bridge deck with Permanent Metal Deck Forms is considered subsidiary to Item 422, "Concrete Superstructures".



STEEL BEAMS

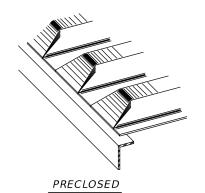
AT TENSION FLANGES (2)

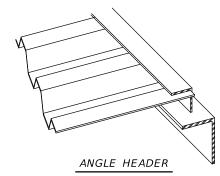
wind hold down

U-BEAMS WITH WELD ANCHORS

STEEL BEAMS AT COMPRESSION FLANGES

## TYPICAL TRANSVERSE SECTIONS





NOTE: This type is to be used for skewed ends only.

### TYPES OF END CLOSURES



### PERMANENT METAL DECK FORMS

### **PMDF**

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TXDOT April 2019	CONT	SECT	JOB		HIG	HWAY
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<ul> <li>20: Modified box note by adding steel beams/girders and subsidiary.</li> </ul>	DIST		COUNTY			SHEET NO.
	ABL	STONEWALL 14:			142	

## SHEET 1 OF 2

DESIGN NOTES:
As a minimum, PMDF and support angles must

construction loads. Flexural stresses due to these design loads must not exceed 75 percent

reinforcement and concrete or 120 psf, whichever

1/180 of the form design span, but not

1/240 of the form design span, but not

The form design span must not be less than

measured parallel to the form flutes, minus 2".

Form sheets must not be permitted to rest directly on the top of beam flanges. Form

All attachments must be made by permissible welds, screws, bolts, clips or other means

metal assembly screws must be installed with

torque-limiting devices to prevent stripping. Only welds or bolts must be used to support

Welding and welds must be in accordance with the provisions of Item 448, "Structural Field Welding", pertaining to fillet welds.

All welds must be made by a qualified welder

in accordance with Item 448.

All permanently exposed form metal, where the galvanized coating has been damaged, must

accordance with Item 445, "Galvanizing". Minor heat discoloration in areas of welds need

Flutes must line up uniformly across the entire width of the structure where main

unless shown on the plans. The location of and forming details for any construction joint used must be shown on the forming plans.

reinforcing steel is located in the flute. Construction joints will not be permitted

Forms below a construction joint must be removed after curing of the slab.
A sequence for uniform vibration of concrete

must be approved by the Engineer prior to

concrete placement. Attention must be given

to prevent damage to the forms, yet provide

in the flutes and at headers and/or

proper vibration to prevent voids or honeycomb

be thoroughly cleaned and repaired in

shown on the the forming plans. All sheet

sheets must be securely fastened to form supports and must have a minimum bearing length of one inch at each end. Form supports must be placed in direct contact with beam

the clear distance between beam flanges,

CONSTRUCTION NOTES:

more than 0.75", for design spans greater

more than 0.50", for design spans of 10'

of the yield strength of the steel. Allowable stress for weld metal must be 12,400 psi.
Maximum deflection under the weight of forms

is greater, shall not exceed the following:

than 10'.

vertical loads.

not be touched up.

construction joints.

be designed for the dead load of the form,

reinforcement and concrete plus 50 psf for

Permanent

or removable forms -Top of slab to top of beam at & brg ~ See Span Details

AT SLAB OVER ABUT BKWL OR INV TEE STEM FOR CONC BEAMS WITHOUT THICKENED SLAB END

Slab thickness (1)

See Span Details

∽End diaphragm

AT CONC END DIAPHRAGM

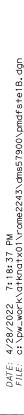
FOR PRESTRESSED I-BEAMS

AND STEEL BEAMS

-Top of slab to top of beam at ⊈ bearing ~ See Span Details

Slab thickness (1)

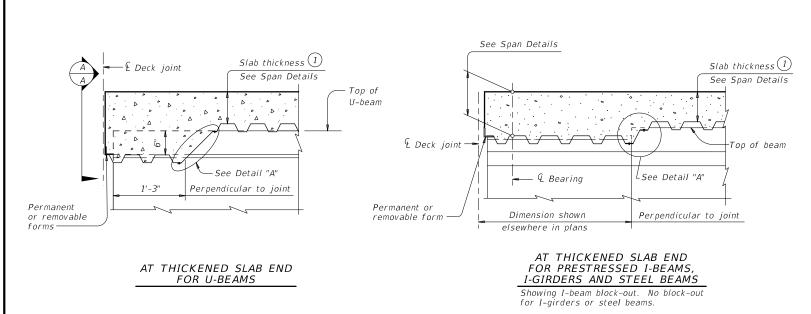
See Span Details

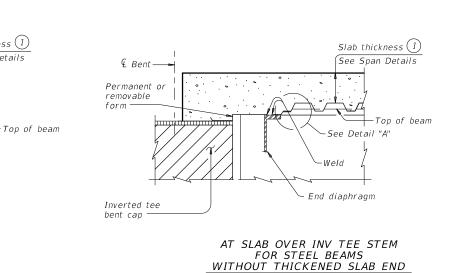


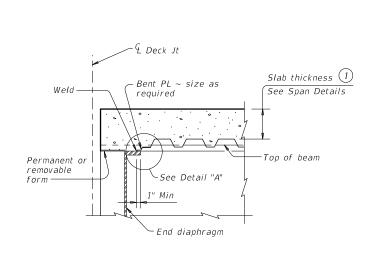
Permanent or removable

€ Deck joint

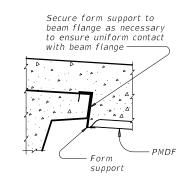
& Bearing .



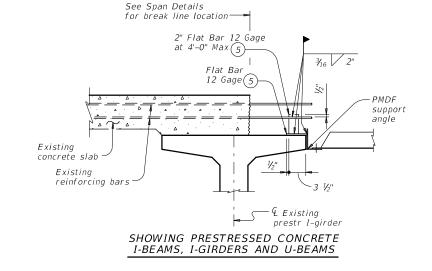


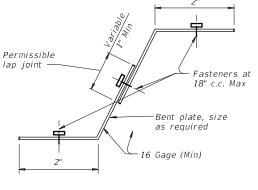


AT END DIAPHRAGM FOR STEEL BEAMS WITHOUT THICKENED SLAB END



# SECTION A-A





Bent PL or L ~ size as required

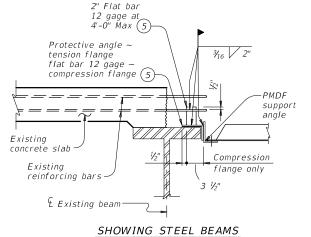
Fasteners at

- PMD Form, end closure required where form is cut on skew

18" c.c. Max

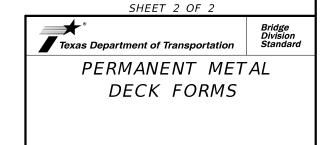


DETAIL "B"



## WIDENING DETAILS





	FINDI							
FILE: pmdfste1-20.dgn	DN: TXE	DOT	ск: ТхD0Т	DW:	TxD0T	ck: TxD0T		
©TxDOT April 2019	CONT	SECT	JOB		Н	IGHWAY		
REVISIONS	0106	04	036		US 380			
02-20: Modified box note by adding steel beams/girders and subsidiary.	DIST		COUNTY			SHEET NO.		
	A R I		STONEW	ΛII		1/17		

DMDF

1) Slab thickness minus %" if corrugations match reinforcing bars

Anchors cast in diaphragm

(5) Minimum yield stress of 12 gage bars shall be 40 ksi

DETAILS AT ENDS OF BEAMS

-Top of beam



7:18:42 F

Median barrier

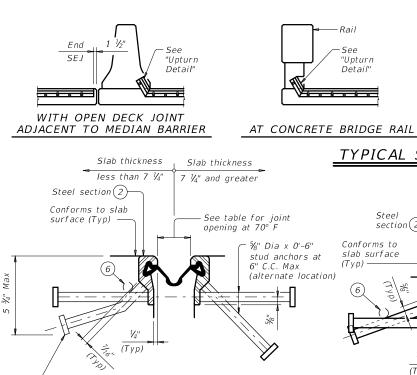
not anchored to slab

End SEJ

at toe of

WITH OPEN DECK JOINT BELOW MEDIAN BARRIER

barrier -



Bend studs as shown when depth of CIP concrete

SECTION THRU WATSON BOWMAN

ACME (SE-400 OR SE-500) JOINTS

is less than 7 1/4" at joint location

Median barrier

anchored to slab

"Upturn

- SEJ continuous

Cast or install barrier

after joint system installation

AT MEDIAN BARRIER

"Upturr

AT SIDEWALK

- See table for joint opening at 70°F

TYPICAL SECTIONS (5)

Steel

Conforms to

slab surface

(Typ)

section(2)

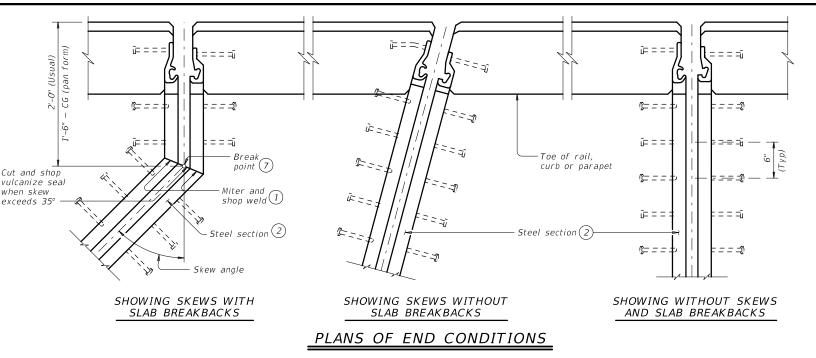
(Typ)

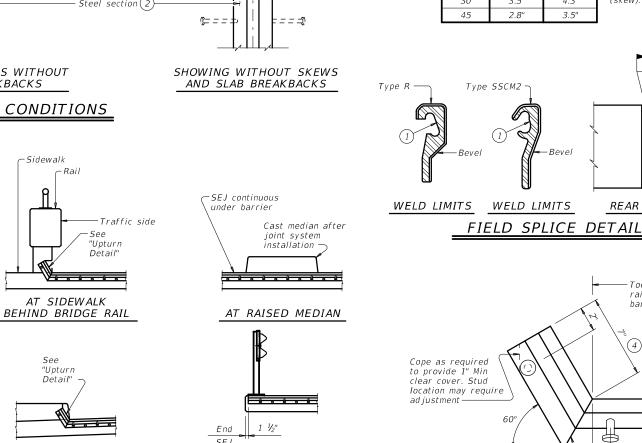
SECTION THRU D.S. BROWN

(A2R-400 OR A2R-XTRA) JOINTS

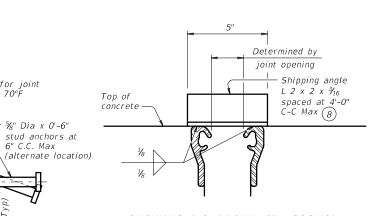
Detail

under barrier





AT STEEL POST BRIDGE RAIL



### SHOWING D.S. BROWN (Ty SSCM2)

(All joints are similar.) (Studs are not shown for clarity.)

Erection bolts are not allowed.

### TABLE OF SEALED EXPANSION JOINT INFORMATION 4" JOINT 5" JOINT STEEL SECTION (2) MANUFACTURER Seal Joint Joint Opening (3 Type Opening (. Type D.S. Brown Type SSCM2 A2R-400 A2R-XTRA Watson Bowman Acme Type R SF-400 SE-500

### REDUCED LONGITUDINAL MOVEMENT RANGE SKEW JOINT SIZE (deg) 4.0" 5.0" 15 4.0" 5.0" 30 3.5" 4.3" 3.5"

### **DESIGN NOTES:**

REAR VIEW

-Toe of sidewalk,

rail or median

barrier

UPTURN DETAIL

Joints installed on a skew have reduced ability to accommodate longitudinal movement. Use table values to determine the correct joint size for skewed installations

For other skews over 25 degrees, calculate reduced movement range by multiplying joint size by cosine

Weld top

and back.

Grind top

smooth

- (1) Remove all burrs which will be in contact with seal prior to making splice.
- $^{ig(2)}$  Shape of steel section shown is typical. Variations in sections must be approved by the Engineer.
- $\stackrel{\textstyle \bigcirc}{}$  These openings are also the recommended minimum installation openings.
- $\stackrel{ ext{$(4)$}}{}$  Reduce for sidewalk or parapet heights less than 6". (5) Other conditions affecting the joint profile should
- (6) Move transverse bars that are in conflict with SEJ studs, in either the bridge slab or approach slab, to rest at the junction of the studs.
- See Span details for location of break point.

be noted elsewhere.

8 Align shipping angle perpendicular to joint.

### FABRICATION NOTES:

Temporarily shop assemble corresponding sections of sealed expansion joints (SEJ), check for fit, and match mark for shipment Secure corresponding sections together for shipment with shipping

angle. Do not use erection bolts.

The seal must be continuous and included in the price bid for sealed expansion joint.

Ship steel sections in convenient lengths of 10'-0" Min and 24'-0" Max unless necessary for staged construction or widenings. One shop splice is permitted in each shipping length provided no piece is less than 2'-0" long and sufficient studs are added to limit the stud to shop splice distance to 2" Min and 4" Max.

Weld studs in accordance with AWS D1.1.

Butt weld all shop and field splices and grind smooth areas in contact with seal. Make all necessary field splice joint preparations in the shop.

Paint the entire steel section with System II or IV primer in

accordance with Item 446, "Feild Cleaning and Painting Steel", unless required to galvanize when shown in the plans. Provide galvanizing in accordance with Item 445, "Galvanizing". Provide paints in accordance with Item 446.2. Prepare steel and apply paint in accordance with Item 446.4.7.3 and 446.4.7.4.

Shop drawings for the fabrication of sealed expansion joints will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.

### CONSTRUCTION NOTES:

Secure the sealed expansion joint in position and place to the proper grade and alignment by welding braces to adjacent reinforcing steel, to prestressed beam stirrups, or to anchors cast in concrete diaphragms. Include cost of temporary bracing in the price bid for sealed expansion ioint.

Remove shipping angle immediately after each joint half is secured in place. Grind smooth, and touch up with organic zinc-rich paint. Clean and prepare seal cavity for seal installation as per the Manufacturer's installation procedures.

### **GENERAL NOTES:**

Provide sealed expansion joints in the size and at locations shown

Minimum slab and overhang thickness required for the use of SEJ-M is 6 1/2".



SEALED EXPANSION JOINT TYPE M WITHOUT OVERLAY

SEJ-M

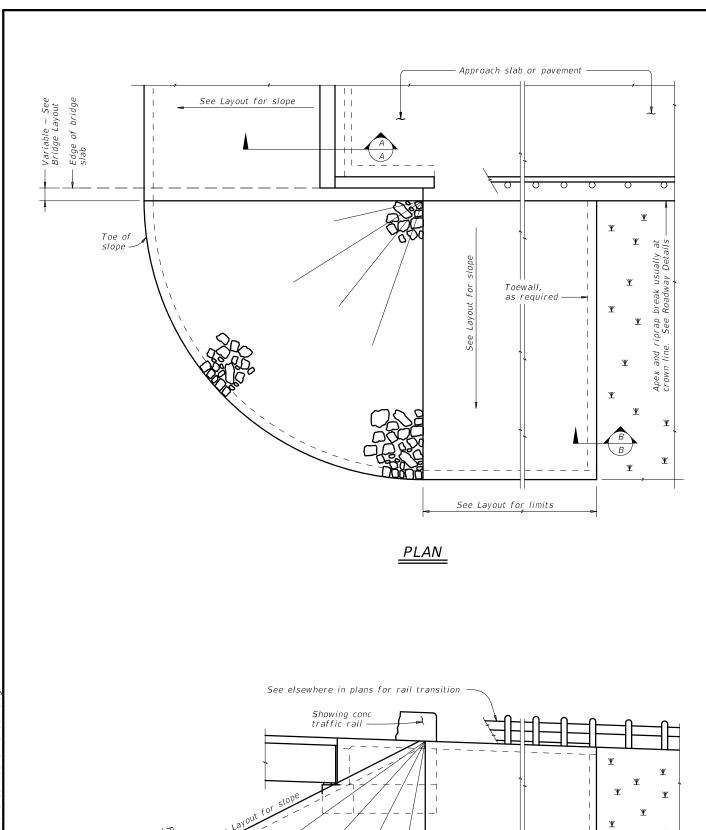
Bridge Division Standard

sejmste1-19.dgn	DN: TXDOT		ск: ТхD0Т	DW:	JTR	ск: ЈМН
xDOT April 2019	CONT	SECT	JOB		HIGHWAY	
REVISIONS	0106	04 036			US	380
	DIST	COUNTY				SHEET NO.
	ABL	STONEWALL 144				

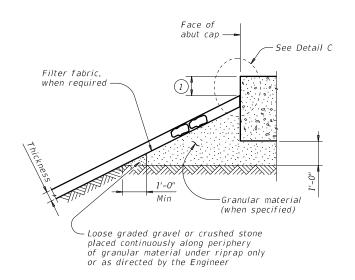
# SHIPPING ANGLE

An alternate method of securing joint sections may be used if approved by the Bridge Division.





ELEVATION

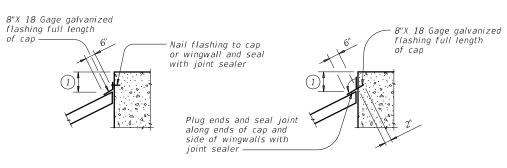


# Type R, Type F, Common 1'-0" Protection Thickness

### SECTION B-B

Provide toewall when shoulder drain is located adjacent to limits of stone riprap. Omit toewall when thickness of protection riprap is greater than 18".

## SECTION A-A AT CAP



### CAP OPTION A

### CAP OPTION B

## <u>DETAIL</u> C

### GENERAL NOTES:

Refer to Item 432, "Riprap" for stone size and gradation, and construction details. See Layout for limits and thickness of riprap specified.
See elsewhere in plans for locations and details of

shoulder drains.

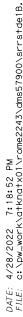
1) Top of cap to top of riprap dimension varies as directed by the Engineer. Provide 9" Min for beam/slab type bridges and 1'-6" for slab span, box beam, or slab beam bridges.

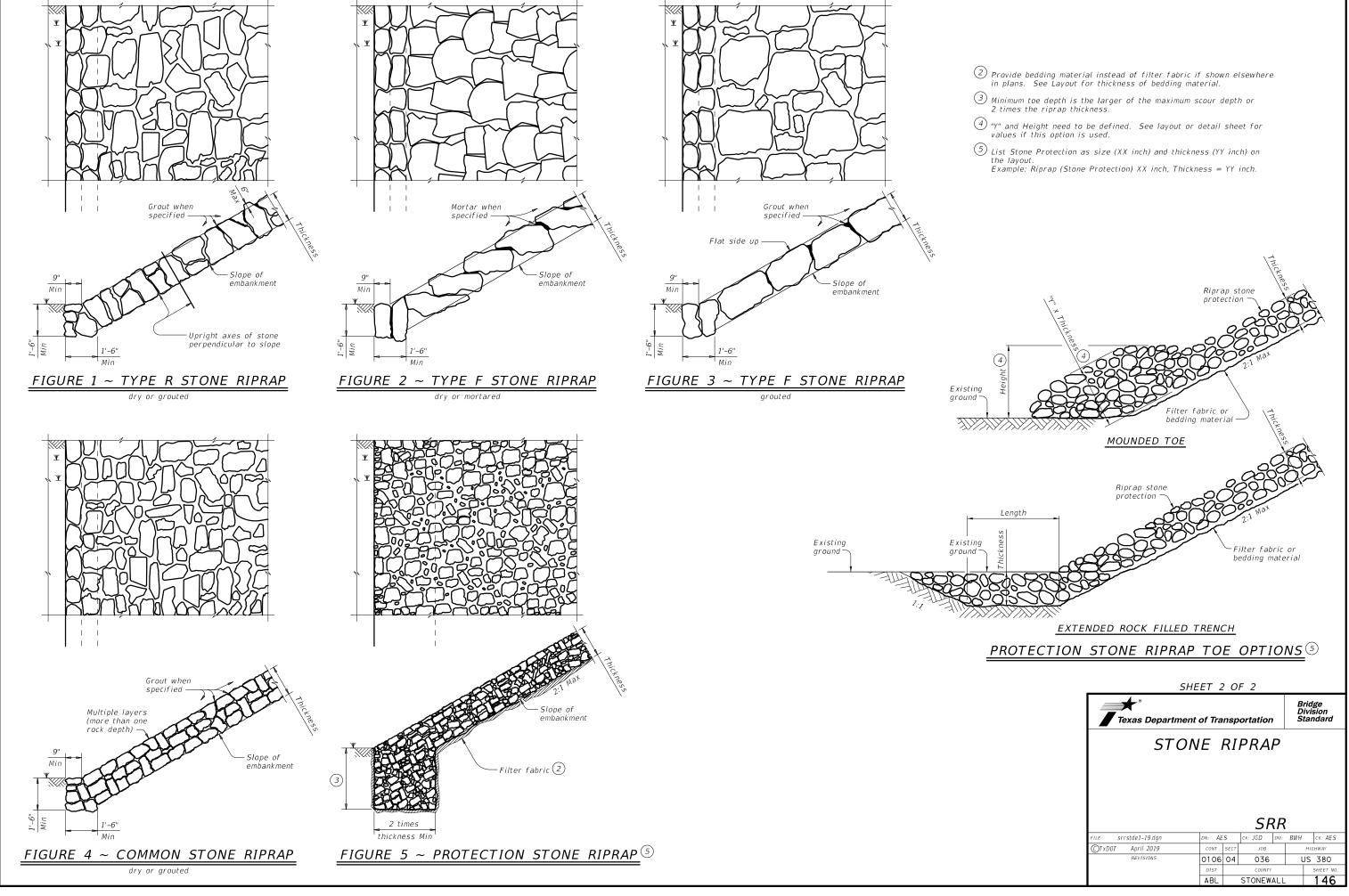




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FILE: Srrstde1-19.dgn	DN: AES		ck: JGD	DW:	BWH	CK: AES	
©TxD0T April 2019	CONT	SECT	JOB		HIC	HIGHWAY	
REVISIONS	0106	04	036		US 380		
	DIST	COUNTY				SHEET NO.	
	ΛRI	STONEWALL				1 4 5	





Parapet End =

Wingwall Length

(Variable) 5'-0" Min 5'-0"

3'-0"

End of Bridge Rail

for payment

— 4'-0" Min & 9'-0" Max ~ End Post

6'-0" Opening

# ROADWAY ELEVATION OF RAIL ON BOX CULVERTS

Permissible

Joint -

Construction

4'-0" Min & 9'-0" Max ~ End Post

4'-0"

Interior Post

Interior Post | Opening

4'-0"

Concrete Panel Length

6'-0" Opening

4'-0"

Interior Post

Showing 0° skew culvert. Skewed culverts similar. See RAC standard for details not shown. Vertical joints in concrete rail are not required, unless shown elsewhere.

1) Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.

Wingwall Length minus 5'-0" (Varies)

SHEET 1 OF 3

Bridge Division Standard Texas Department of Transportation

-4'-0" Min & 9'-0" Max ~ End Post

See "Post Joint

Detail" (Typ)

See "Post

Joint Detail"

Concrete Panel Length

Parapet End

(Variable) 5'-0" Min

2'-0"

Limits of Culvert

AT PARALLEL WINGS

Parallel Wing

5'-0"

3'-0"

4'-0"

Interior Post Opening

6'-0"

Controlled Joint or

Construction Joint

End of Bridge Rail

- 4 Thrie-Beam

Connector (1)

for payment

¼" Min

¾" Max

AT BENTS WITHOUT SLAB EXP JOINTS

6'-0" Opening

Same as Slab

Jt Opening

AT BENTS WITH SLAB EXP JOINTS

6'-0" <u>Opening</u>

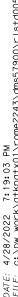
1. 1

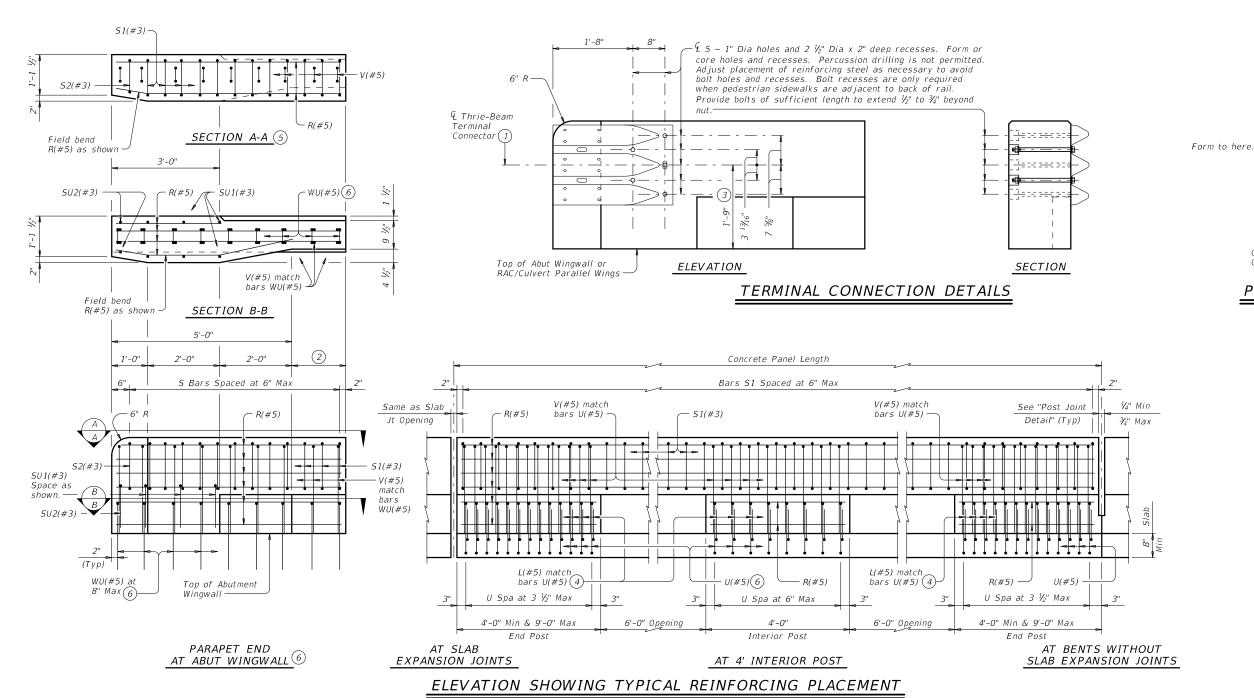
Variable

TRAFFIC RAIL

TYPE T223

E: rIstd005-19.dgn	DN: TXE	DOT CK: TXDOT DW:		JTR	CK: AES	ı	
TxDOT September 2019	CONT	SECT	JOB		HIGHWAY		ı
REVISIONS	0106	04	036		U	S 380	ı
	DIST		COUNTY			SHEET NO.	ı
	ABL		STONEW	ALL		147	ı





Showing rail on slab. Rail on box culvert similar.

- 1 Terminal Connectors and associated hardware are to be paid for under the Item "Metal Beam Guard Fence". Attach Metal Beam Guard Fence Transitions to the bridge rail and extend along the embankment unless otherwise shown in the plans.
- ② Wingwall Length minus 5'-0" (Varies)
- 3 Increase 2" for structures with overlay.
- 4 Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.
- Bars SU1(#3), SU2(#3) and WU(#5) not shown for clarity.
- 6 Substitute Bars U(#5) for Bars WU(#5) when parapet end is located on achorage curb over culvert top slab. Use Bars WU(#5) in culvert parallel wings.

SHEET 2 OF 3

0pening

Controlled Joint or

Construction Joint

POST JOINT DETAIL

Provide at all interior bents without slab expansion joints.

1/4" Min

¾" Max

V groove



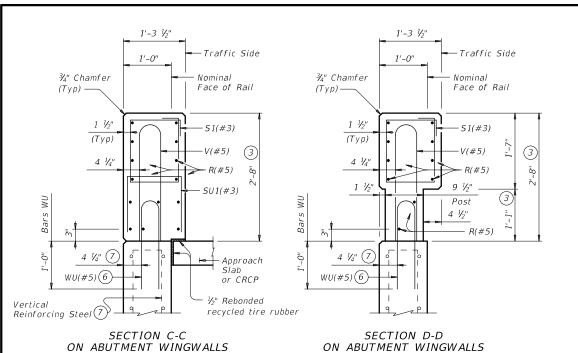
TRAFFIC RAIL

TYPE T223

.E: rlstd005-19.dgn	DN: TXDOT		ck: TxD0T	DW:	JTR	CK: AES	
TxDOT September 2019	CONT	SECT	JOB		Н	HIGHWAY	
REVISIONS	0106	04 036			US	US 380	
	DIST	COUNTY				SHEET NO.	
	ΛRI	STONEWALL				1 // 2	

OR CIP RETAINING WALLS





1'-3 1/2" 1'-3 1/2" 1'-0" 1'-0" ¾" Chamfer Nominal Nominal ¾" Chamfer Face of Rail Face of Rail (Typ) -(Typ)-51(#3) 51(#3) Const Jt (3) (Typ) (Typ) Top of 4 1/4" Post 1 1/2" 4 1/3" Slab Bars L, U and V Posi ۷<u>[</u>3] L(#5) (4) ypical Water Barrier (if used) U(#5)(6) AT POST AT OPENING

SECTIONS THRU RAIL

Sections on box culverts similar

- (2) Wingwall Length minus 5'-0" (Varies)
- 3 Increase 2" for structures with overlay.
- Bars L(#5) are part of rail reinforcing and are included in unit price bid for railing. Space with Bars U. Bars L match slab bar cover. Bars L may be bundled with top slab reinforcing if spacing is equivalent.

OR CIP RETAINING WALLS

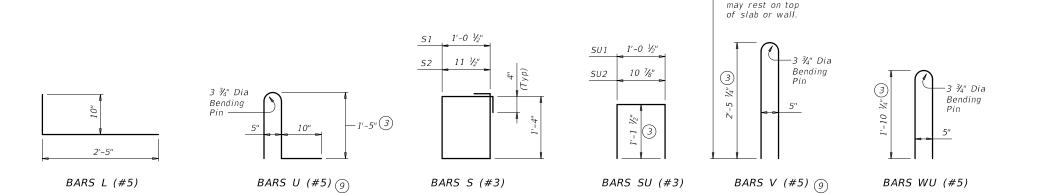
- 6 Substitute Bars U(#5) for Bars WU(#5) when parapet end is located on anchorage curb over culvert top slab. Use Bars WU(#5) in culvert parallel wings.
- (7) When vertical reinforcing has closer clear cover over horizontal reinforcing in abutment wingwalls on traffic side of wall, move the horizontal wingwall/retaining wall reinforcing to the inside of Bars WU where bars conflict.
- $\fbox{8}$  Top longitudinal slab bar may be adjusted laterally 3" plus or minus to tie reinforcing.
- (9) At the Contractor's option, Bars V may be replaced by extending Bars U to 2'-5 1/4" above the roadway surface without overlay.

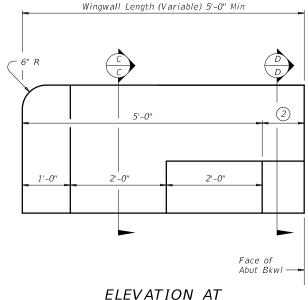
### € Concrete Rail Footprint Outside Edge Outside Edge of Slab. Abut Wingwall ← € Concrete Rail Expansion Joint. Location of Rail Expansion € Slab Joint must be at the intersection of & Slab Expansion Joint, Expansion 4 Rail Footprint and perpendicular to slab outside edge. Joint Cross-hatched area must have " Preformed Bitumuminous Fiber Material under concrete rail, as shown -Traffic Side of Rail

## PLAN OF RAIL AT EXPANSION JOINTS

Installed bar

ON BRIDGE SLAB





ABUTMENT WINGWALL

CONSTRUCTION NOTES:
Face of rail and parapet must be vertical transversely unless otherwise shown in the plans or approved by the Engineer.

Provide water barriers at openings draining onto undercrossing roadways and sidewalks. They may be cast-in-place or precast in convenient lengths and bonded to the bridge deck with an approved

Chamfer all exposed corners.

### MATERIAL NOTES:

ON BRIDGE SLAB

Provide Class "C" concrete. Provide Class "C" (HPC) if required elsewhere.

Provide Grade 60 reinforcing steel.

Epoxy coat or galvanize all reinforcing steel if slab bars are epoxy coated or galvanized.

Deformed Welded Wire Reinforcing (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars U, V, and WU unless noted otherwise. Provide the same laps as required for reinforcing

Provide bar laps, where required, as follows:

Uncoated or galvanized ~ #5 = 2'-0" Epoxy coated  $\sim #5 = 3'-0''$ 

### **GENERAL NOTES:**

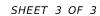
This rail has been evaluated by full-scale crash test to meet MASH TL-3 criteria. This rail can be used for speeds of 50 mph and greater when a TL-3 rated guard fence transition is used. When a TL-2 rated guard fence transition is used, this rail can only be used for speeds of 45 mph and less.

Do not use this railing on bridges with expansion joints providing more than 5" movement.

Rail anchorage details shown on this standard may require modification for select structure types. See appropriate details elsewhere in plans for these modifications. Shop drawings are not required for this rail

Average weight of railing with no overlay is 358 plf

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.





TRAFFIC RAIL

Bridge Division Standard

*TYPE T223* 

•	• •	_		_			
FILE: rlstd005-19.dgn	DN: TXE	DOT	ck: TxD0T	DW:	JTR	CK: AES	
©TxD0T September 2019	CONT	SECT	JOB			HIGHWAY	
REVISIONS	0106	04	036		Į	US 380	
	DIST		COUNTY			SHEET NO.	
	ABL		STONEW	ALL		149	

			SUMMARY	OF S	M A	\ L	LSIG	NS				
81.411					(TYPE A)	(TYPE G)					XX (X-XXXX)	BRIDGE MOUNT CLEARANCE
PLAN SHEET	SIGN	SIGN			1		POST TYPE	POSTS	ANCHOR TYPE		ITING DESIGNATION	SIGNS
NO.	NO.	NOMENCLATURE	SIGN	DIMENSIONS	ALUMINUM	UMINU	FRP = Fiberglass TWT = Thin-Wall	1 or 2	UB=Universal Bolt		1EXT or 2EXT = # of Ext BM = Extruded Wind Beam WC = 1.12 #/ft Wing	(See Note 2)
					- 1		10BWG = 10 BWG	1 01 2	SB=Slipbase-Bolt	T = "T"	Channe I	TY = TYPE
					FLAT	X	S80 = Sch 80		WS=Wedge Steel WP=Wedge Plastic	U = "U"	EXAL= Extruded Alum Sign Panels	TY N TY S
1	1	I-3	Salt Fork of Brazos River	144 X 36	X		1 OBWG	1	SA SA	T	Tuliers	11.3
1	2	I-3		144 X 36		_	1 OBWG	1	SA	т		
	2	1-5	Salt Fork of Brazos River	144 7 36			100#6	'	, SA			

ALUMINUM SIGN B	ANKS THICKNESS
Square Feet	Minimum Thickness
Less than 7.5	0.080"
7.5 to 15	0.100"
Greater than 15	0.125"

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

http://www.txdot.gov/

### NOTE:

- 1. Sign supports shall be located as shown on the plans, except that the Engineer may shift the sign supports, within design guidelines, where necessary to secure a more desirable location or to avoid conflict with utilities. Unless otherwise shown on the plans, the Contractor shall stake and the Engineer will verify all sign support locations.
- For installation of bridge mount clearance signs, see Bridge Mounted Clearance Sign Assembly (BMCS)Standard Sheet.
- For Sign Support Descriptive Codes, see Sign Mounting Details Small Roadside Signs General Notes & Details SMD(GEN).



Traffic Operations Division Standard

# SUMMARY OF SMALL SIGNS

# SOSS

	•		_					
.E:	sums16.dgn	DN: TXDOT		ck: TxDOT	DW:	T×D0	T CK: T	TXDOT
T×DOT	May 1987	CONT	SECT	JOB			HIGHWAY	
4.5	REVISIONS 0106 04 036			US 380		0		
·16 ·16		DIST	DIST COUNTY SHEET NO.					NO.
. 0		ABL		STONEWALL				0

NOTES: SUBMITTAL 1. ALL SIGNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH DETAILS FOUND IN THE TXDOT "SIGN CREW FIELDBOOK", "TMUTCD" & "STANDARD HIGHWAY SIGN DETAILS FOR TEXAS", LATEST EDITIONS. -STA 743+00.00 OFF 12.00' LT MATCH EXISTING BEGIN (A)(D) - © US 380 -STA 743+00.00 OFF 0.00' MATCH EXISTING BEGIN ( AND (B) x2 100% <u>EXIST ROW</u> **LEGEND** EXIST ROW (A) RE PM W/ RET REQ TY I (W) (4") (SLD) (100 MIL) RE PM W/ RET REQ TY I (Y)(4")(SLD)(100 MIL) (C) REFL PAV MRKR TY II A-A (D) CENTERLINE/EDGELINE RUMBLE STRIP EXIST ROW X PROPOSED SMALL SIGN ID EXIST ROW ■ PROPOSED SMALL SIGN PROPOSED OBJECT MARKER **XD**€ PROPOSED DELINEATOR Salt Fork of Brazos River → TRAFFIC DIRECTION -STA 746+40.00 END (D) -STA 743+00.00 OFF 12.00' RT MATCH EXISTING BEGIN (A)(D) Salt Fork of Brazos River CENSTONAL ENTER -**B** ×2 —€ US 380 4/28/2022 00 00 0 **ATKINS** EXIST ROW Texas Department of Transportation Abilene District US 380 SIGNING AND PAVEMENT MARKING LAYOUT SHEET 1 OF : PROJECT No. SIGNED: SG TEXAS SEE TITLE SHEET US 380 CHECKED: JD

RAWN: SG STATE DISTRICT

COUNTY

STONEWALL 0106

JOB No.

-STA 763+00.00 OFF 12.00' LT MATCH EXISTING END (A)(D)

-STA 763+00.00 OFF 0.00' MATCH EXISTING END © MAND B ×2

STA 763+00.00 OFF 12.00' RT MATCH EXISTING END (A)(D)

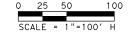
-Ç US 380

NOTES:

1. ALL SIGNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH DETAILS FOUND IN THE TXDOT "SIGN CREW FIELDBOOK", "TMUTCD" & "STANDARD HIGHWAY SIGN DETAILS FOR TEXAS", LATEST EDITIONS.

# LEGEND

- A RE PM W/ RET REQ TY I (W) (4") (SLD) (100 MIL)
- B RE PM W/ RET REQ TY I (Y) (4") (SLD) (100 MIL)
- © REFL PAV MRKR TY II A-A
- D CENTERLINE/EDGELINE RUMBLE STRIP
- $\overline{\langle X \rangle}$  PROPOSED SMALL SIGN ID
- PROPOSED SMALL SIGN
- PROPOSED OBJECT MARKER
- ₩ PROPOSED DELINEATOR
- → TRAFFIC DIRECTION





# **ATKINS**TBPE REC. # F-



US 380

SIGNING AND PAVEMENT MARKING LAYOUT

SHEET 2 OF

						SH	EEI	2 OF 2
GNED	:SG	FED. RD DIV. No.	STATE	PROJE	ECT No.	HIGHWAY No.		
KED:	JD	6	TEXAS	SEE TIT	LE SHE	U	S 380	
vn:	SG	STATE DISTRICT	COUNTY	CONTROL No.			OB o.	SHEET No.
KED:	JD	ABL	STONEWALL	0106	04	0	36	152

2.3" Radius, 0.8" Border, White on Green; [Salt Fork] ClearviewHwy-5-W-R; [of] ClearviewHwy-5-W-R; [Brazos] ClearviewHwy-5-W-R; [River] ClearviewHwy-5-W-R;







Texas Department of Transportation

\*\*Department of Transportation\*\*

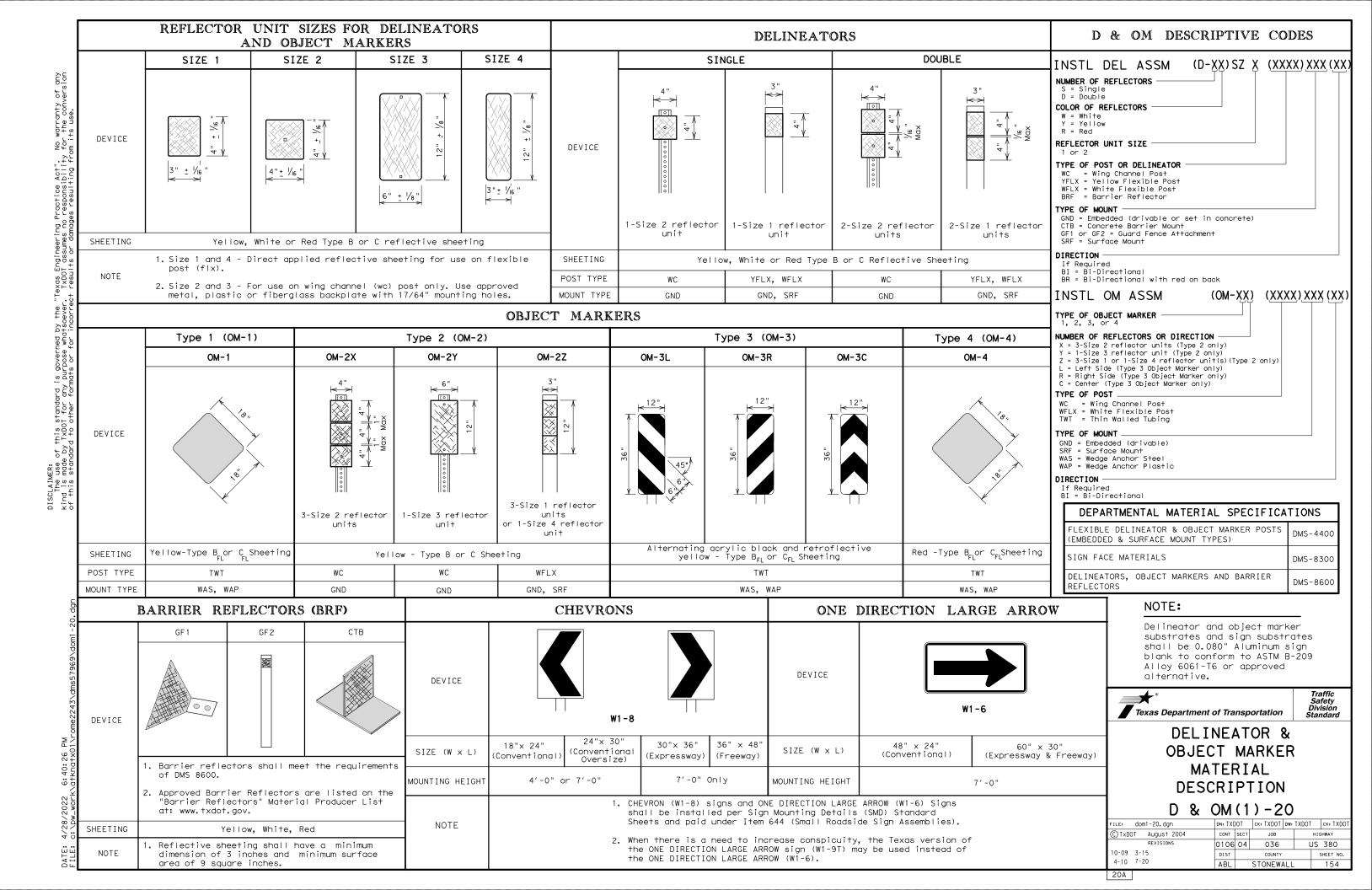
\*\*Abilene District\*\*

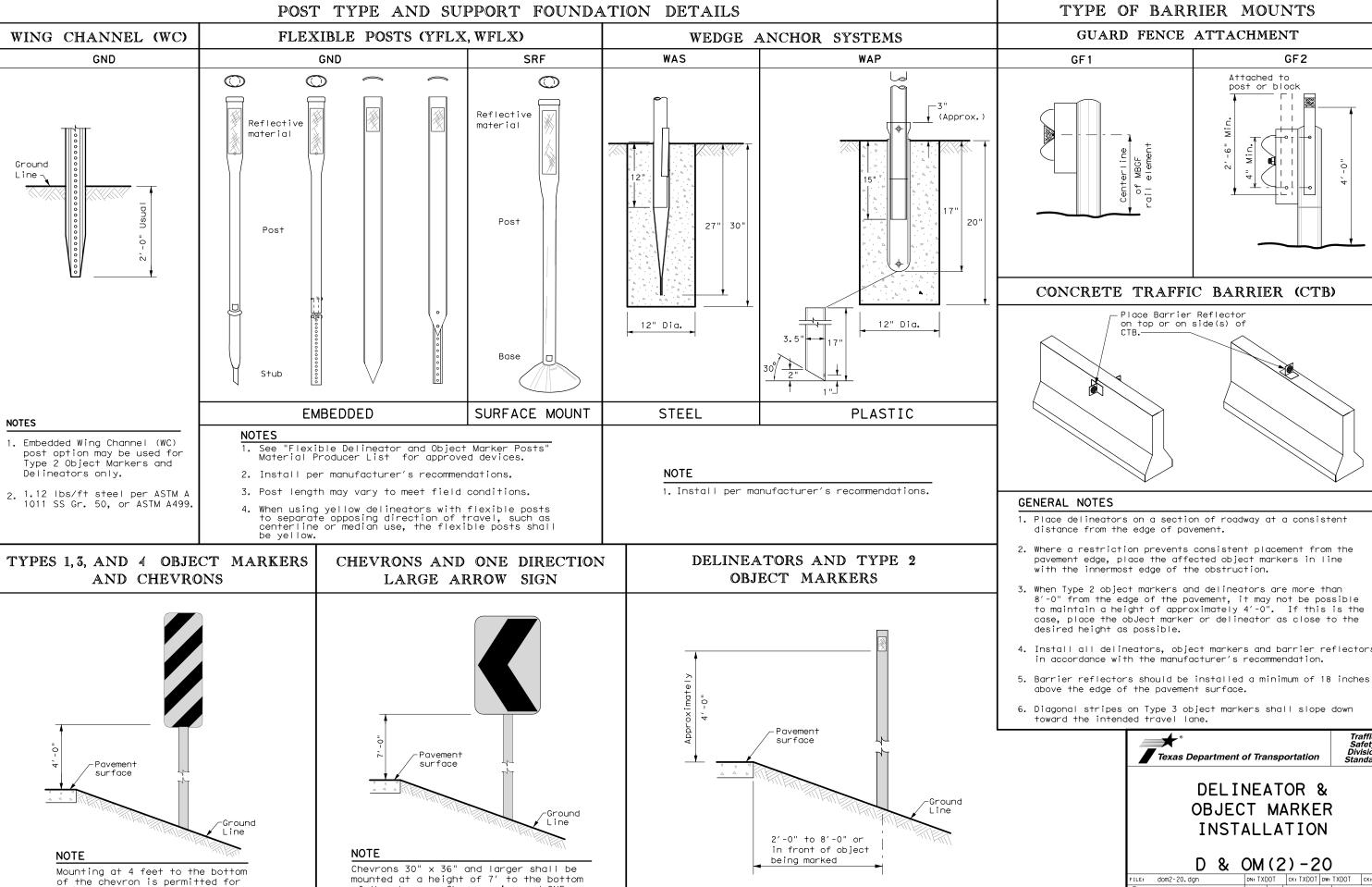
US 380

SIGN DETAILS

SHEET 1 OF 1

SIGNED	:SG	FED. RD DIV. No.	STATE	PROJE	CT No.		HIGHWAY No.		
ECKED:	JNR	6	TEXAS	SEE TIT	LE SHE	US 380			
AWN:	SG	STATE DISTRICT	COUNTY	CONTROL No.			OB 0.	SHEET No.	
ECKED:	JNR	ABL	STONEWALL	0106	04	0	36	153	





of the chevron. Chevron sign and ONE

paid under item 644.

DIRECTION LARGE ARROW sign (W1-9T) shall

be installed per SMD standard sheets and

No warranty of any for the conversion

governed by the "Texas Engineering Practice Act". rpose whatsoever. IXDOI assumes no responsibility s or for incorrect results or damanas resultion for

chevrons that will not exceed

a height of 6'-6" to the top of

the chevron (sizes  $24" \times 30"$  and

ABL

4-10 7-20

20B

0106 04 036 US 380 STONEWALL

GF2

- 1. Place delineators on a section of roadway at a consistent
- pavement edge, place the affected object markers in line
- to maintain a height of approximately 4'-0". If this is the case, place the object marker or delineator as close to the
- in accordance with the manufacturer's recommendation.

**OBJECT MARKER** 

DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO

10-09 3-15

CONT SECT

INSTALLATION

See general notes 1, 2 and 3.

4. Install all delineators, object markers and barrier reflectors

JOB

Traffic Safety Division Standard

**DELINEATOR &** 

C)TxDOT August 2004 HIGHWAY

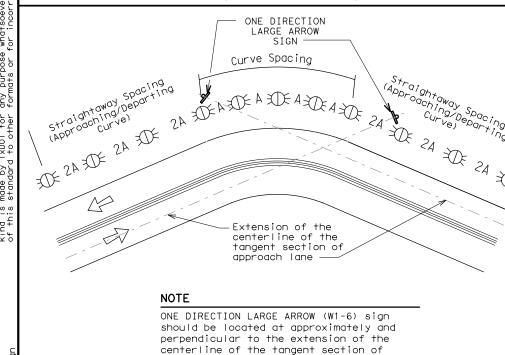
D & OM(2) - 20

# MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

Amount by which Advisory Speed	Curve Advisory Speed		
is less than Posted Speed	Turn (30 MPH or less)	Curve (35 MPH or more)	
5 MPH & 10 MPH	• RPMs	• RPMs	
15 MPH & 20 MPH	• RPMs and One Direction Large Arrow sign	<ul> <li>RPMs and Chevrons; or</li> <li>RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons.</li> </ul>	
25 MPH & more	RPMs and Chevrons; or      RPMs and One Direction     Large Arrow sign where     geometric conditions or     roadside obstacles prevent     the installation of	• RPMs and Chevrons	

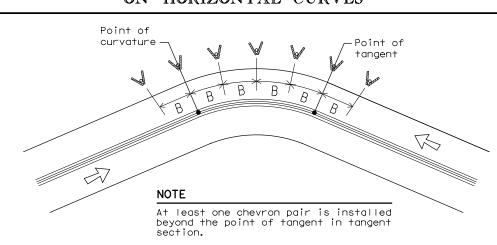
# SUGGESTED SPACING FOR DELINEATORS ON HORIZONTAL CURVES

chevrons



### SUGGESTED SPACING FOR CHEVRONS ON HORIZONTAL CURVES

approach lane.



# DELINEATOR AND CHEVRON SPACING

WHEN DEGREE OF CURVE OR RADIUS IS KNOWN

Degree of Curve of Curve         Radius of Curve         Spacing in Curve         Spacing in Straightaway         Chevron Spacing in Curve           1         5730         225         450         ————————————————————————————————————		FEET				
1         5730         225         450         ————————————————————————————————————	of	of	in	in	Spacing in	
2       2865       160       320         3       1910       130       260       200         4       1433       110       220       160         5       1146       100       200       160         6       955       90       180       160         7       819       85       170       160         8       716       75       150       120         9       637       75       150       120         10       573       70       140       120         11       521       65       130       120         12       478       60       120       120         13       441       60       120       120         14       409       55       110       80         15       382       55       110       80         16       358       55       110       80         19       302       50       100       80         23       249       40       80       80         29       198       35       70       40         38       151			А	2A	В	
3         1910         130         260         200           4         1433         110         220         160           5         1146         100         200         160           6         955         90         180         160           7         819         85         170         160           8         716         75         150         120           9         637         75         150         120           10         573         70         140         120           11         521         65         130         120           12         478         60         120         120           13         441         60         120         120           14         409         55         110         80           15         382         55         110         80           16         358         55         110         80           19         302         50         100         80           23         249         40         80         80           29         198         35         70         <	1	5730	225	450		
4     1433     110     220     160       5     1146     100     200     160       6     955     90     180     160       7     819     85     170     160       8     716     75     150     120       9     637     75     150     120       10     573     70     140     120       11     521     65     130     120       12     478     60     120     120       13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	2	2865	160	320		
5         1146         100         200         160           6         955         90         180         160           7         819         85         170         160           8         716         75         150         160           9         637         75         150         120           10         573         70         140         120           11         521         65         130         120           12         478         60         120         120           13         441         60         120         120           14         409         55         110         80           15         382         55         110         80           16         358         55         110         80           19         302         50         100         80           23         249         40         80         80           29         198         35         70         40           38         151         30         60         40	3	1910	130	260	200	
6     955     90     180     160       7     819     85     170     160       8     716     75     150     160       9     637     75     150     120       10     573     70     140     120       11     521     65     130     120       12     478     60     120     120       13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	4	1433	110	220	160	
7         819         85         170         160           8         716         75         150         160           9         637         75         150         120           10         573         70         140         120           11         521         65         130         120           12         478         60         120         120           13         441         60         120         120           14         409         55         110         80           15         382         55         110         80           16         358         55         110         80           19         302         50         100         80           23         249         40         80         80           29         198         35         70         40           38         151         30         60         40	5	1146	100	200	160	
8     716     75     150     160       9     637     75     150     120       10     573     70     140     120       11     521     65     130     120       12     478     60     120     120       13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	6	955	90	180	160	
9         637         75         150         120           10         573         70         140         120           11         521         65         130         120           12         478         60         120         120           13         441         60         120         120           14         409         55         110         80           15         382         55         110         80           16         358         55         110         80           19         302         50         100         80           23         249         40         80         80           29         198         35         70         40           38         151         30         60         40	7	819	85	170	160	
10         573         70         140         120           11         521         65         130         120           12         478         60         120         120           13         441         60         120         120           14         409         55         110         80           15         382         55         110         80           16         358         55         110         80           19         302         50         100         80           23         249         40         80         80           29         198         35         70         40           38         151         30         60         40	8	716	75	150	160	
11     521     65     130     120       12     478     60     120     120       13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	9	637	75	150	120	
12     478     60     120     120       13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	10	573	70	140	120	
13     441     60     120     120       14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	11	521	65	130	120	
14     409     55     110     80       15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	12	478	60	120	120	
15     382     55     110     80       16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	13	441	60	120	120	
16     358     55     110     80       19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	14	409	55	110	80	
19     302     50     100     80       23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	15	382	55	110	80	
23     249     40     80     80       29     198     35     70     40       38     151     30     60     40	16	358	55	110	80	
29     198     35     70     40       38     151     30     60     40	19	302	50	100	80	
38 151 30 60 40	23	249	40	80	80	
	29	198	35	70	40	
57 101 20 40 40	38	151	30	60	40	
	57	101	20	40	40	

Curve delineator approach and departure spacing should include 3 delineators spaced at 2A. This spacing should be used during design preparation or when the degree of curve is known.

# DELINEATOR AND CHEVRON SPACING

WHEN DEGREE OF CURVE OR RADIUS IS NOT KNOWN

Advisory Speed (MPH)	Spacing in Curve	Spacing in Straightaway	Chevron Spacing in Curve
	А	2×A	В
65	130	260	200
60	110	220	160
55	100	200	160
50	85	170	160
45	75	150	120
40	70	140	120
35	60	120	120
30	55	110	80
25	50	100	80
20	40	80	80
15	35	70	40

If the degree of curve is not known, delineator spacing may be determined based on the Advisory Speed of the curve. Use the delineator curve spacing for each Advisory Speed (MPH).

# DELINEATOR AND OBJECT MARKER APPLICATION AND SPACING

CONDITION	REQUIRED TREATMENT	MINIMUM SPACING
Frwy./Exp. Tangent	RPMs	See PM-series and FPM-series standard sheets
Frwy./Exp. Curve	Single delineators on right side	See delineator spacing table
Frwy/Exp.Ramp	Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4))	100 feet on ramp tangents  Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves)
Acceleration/Deceleration Lane	Double delineators (see Detail 3 on D&OM(4))	100 feet (See Detail 3 on D & OM (4))
Truck Escape Ramp	Single red delineators on both sides	50 feet
Bridge Rail (steel or concrete)and Metal Beam Guard Fence	Bi-Directional Delineators when undivided with one lane each direction  Single Delineators when multiple lanes each direction	Equal spacing (100'max) but not less than 3 delineators
Concrete Traffic Barrier (CTB) or Steel Traffic Barrier	Barrier reflectors matching the color of the edge line	Equal spacing 100' max
Cable Barrier	Reflectors matching the color of the edge line	Every 5th cable barrier post (up to 100'max)
Guard Rail Terminus/Impact Head	Divided highway - Object marker on approach end Undivided 2-lane highways - Object marker on approach and departure end	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6)
Bridges with no Approach Rail	Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail	See D & OM(5)
Reduced Width Approaches to Bridge Rail	Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end
Culverts without MBGF	Type 2 Object Markers	See D & OM (5)  See Detail 2 on D & OM(4)
Crossovers	Double yellow delineators and RPMs	See Detail 1 on D & OM (4)
Pavement Narrowing (lane merge) on Freeways/Expressway	Single delineators adjacent to affected lane for full length of transition	100 feet
NOTES		

- Unless indicated otherwise, the delineator or barrier reflector color shall conform to the color of the pavement edge line on the side of the road where the delineators or barrier reflectors are placed.
- 2. Barrier reflectors may be used to replace required delineators.
- 3. Single red delineators may be mounted on the back side of delineator posts for wrong way driver applications

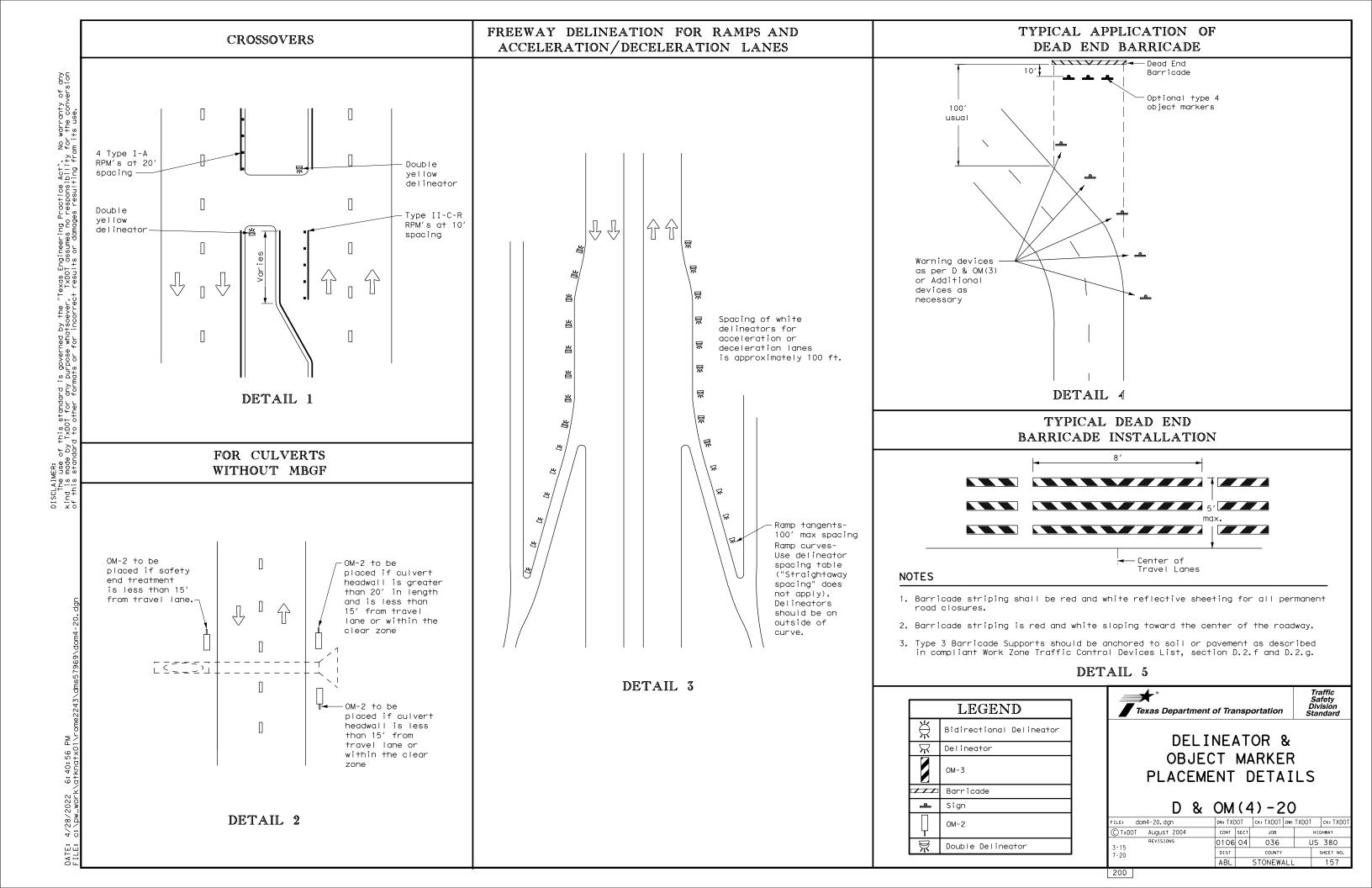
LEGEND		
$\mathbb{A}$	Bi-directional Delineator	
$\mathbb{R}$	Delineator	
<b>♣</b> Sign		



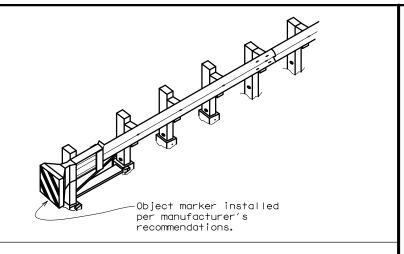
DELINEATOR & OBJECT MARKER PLACEMENT DETAILS

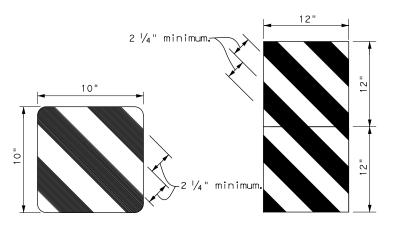
D & OM(3) - 20

				_	
ILE: dom3-20.dgn	DN: TX[	OT.	ck: TXDOT	DW: TXDOT	ck: TXDOT
C)TxDOT August 2004	CONT	SECT	JOB		HIGHWAY
	0106	04	036	- L	JS 380
3-15 8-15	DIST		COUNTY		SHEET NO.
3-15 7-20	ABL		STONEWA	ALL	156



6: 41: 05

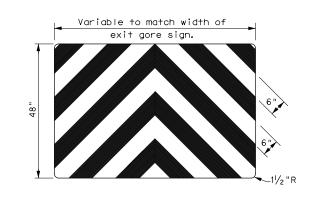




OBJECT MARKERS SMALLER THAN 3 FT<sup>2</sup>

EXIT 444

BACK PANEL (OPTIONAL)



### NOTES

- Object Markers shall conform to the Texas MUTCD and meet the color and reflectivity requirement of Department Material Specification DMS 8300. Background shall be yellow reflective sheeting (Type B or C) and Chevron shall be black.
- 2. Object Markers may be fabricated from adhesive backed reflective sheeting applied directly to guardrail end treatment, or applied directly to an "end cap" as per the manufacturer's recommendation. Direct applied sheeting shall provide a smooth surface and have no wrinkles, air bubbles, cuts or tears. A radius at the corners is not required for direct applied sheeting.
- 3. Object Marker size may be reduced to fit smaller devices. Width of alternating black and yellow stripes are typically 6". Object Markers smaller than 3ft may have reduced width stripes of a minimum of 2  $\frac{1}{4}$ ".
- 4. Pop rivets, screws, or nuts and bolts may be used to attach object markers and reflectors. Holes, slots or other openings may be cut or drilled through object markers to allow cable or other attachments.
- 5. Object Marker at nose of attenuator is subsidiary to the attenuator.
- 6. See D & OM (1-4) for required barrier reflectors.



Traffic Safety Division Standard

DELINEATOR &
OBJECT MARKER
FOR VEHICLE IMPACT
ATTENUATORS

D & OM(VIA)-20

LE: domvia20.dgn	DN: TX[	OT	ck: TXDOT	DW:	TXDOT	ck: TXDOT
TxDOT December 1989	CONT	SECT	JOB		н	GHWAY
REVISIONS	0106	04	036		US	380
-92 8-04 -95 3-15	DIST		COUNTY			SHEET NO.
-98 7-20	ABL		STONEWA	٩LL		159

20G

# REQUIREMENTS FOR INDEPENDENT MOUNTED ROUTE SIGNS

SHEETING REQUIREMENTS			
USAGE COLOR SIGN FACE MATERIAL			
BACKGROUND	WHITE	TYPE A SHEETING	
BACKGROUND	ALL OTHERS	TYPE B OR C SHEETING	
LEGEND & BORDERS	WHITE	TYPE A SHEETING	
LEGEND & BORDERS	BLACK	ACRYLIC NON-REFLECTIVE FILM	
LEGEND & BORDERS	ALL OTHERS	TYPE B or C SHEETING	



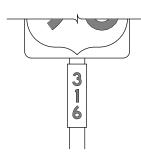




TYPICAL EXAMPLES

# REQUIREMENTS FOR BLUE, BROWN & GREEN D AND I SERIES GUIDE SIGNS

SHEETING REQUIREMENTS				
USAGE	COLOR	SIGN FACE MATERIAL		
BACKGROUND	ALL	TYPE B OR C SHEETING		
LEGEND & BORDERS	WHITE	TYPE D SHEETING		
LEGEND, SYMBOLS & BORDERS	ALL OTHERS	TYPE B OR C SHEETING		













TYPICAL EXAMPLES

### GENERAL NOTES

- Signs to be furnished shall be as detailed elsewhere in the plans and/or as shown on sign tabulation sheet. Standard sign designs and arrow dimensions can be found in the "Standard Highway Sign Designs for Texas" (SHSD).
- 2. White legend shall use the Clearview Alphabet. The following Clearview fonts shall be used to replace the existing white Federal Highway Administration (FHWA) Standard Highway Alphabets, when not specified in the SHSD, or in the plans.

В	CV-1W
С	CV-2W
D	CV-3W
E	CV-4W
Emod	CV-5WR
F	CV-6W

- 3. Route sign legend (ie. IH, US, SH and FM shields) shall use the Federal Highway Administration (FHWA) Standard Highway Alphabets B, C, D, E, Emod or F).
- 4. Lateral spacing between letters and numerals shall conform with the SHSD, and any approved changes thereto. Lateral spacing of legend shall provide a balanced appearance when spacing is not shown.
- 5. Independent mounted route sign with white or colored legend and borders shall be applied by screening process with transparent color ink, transparent colored overlay film to white background sheeting or cut-out white sheeting to colored background sheeting, or combination thereof. White legend, symbols and borders on all other signs shall be cut-out white sheeting applied to colored background sheeting.
- 6. Information regarding borders and radii for signs is found in the "Standard Highway Sign Designs for Texas". Dimensions shown and described for borders and corner radii on parent sign are nominal. Borders may vary in width as much as 1/2 inch. Corner radii above 3 inches may vary in width as much as 1 inch. Borders and corner radii within a parent sign must be of matching widths. The sign area outside the corner radius should be trimmed or rounded.
- 7. Sign substrate shall be any material that meets the Departmental Material Specification requirements of DMS-7110 or approved alternative.
- 8. Mounting details of roadside signs are shown in the "SMD series" Standard Plan Sheets.

l	DEPARTMENTAL MATERIAL SPEC	IFICATIONS
	ALUMINUM SIGN BLANKS	DMS-7110
	SIGN FACE MATERIALS	DMS-8300

ALUMINUM SIGN	BLANKS THICKNESS
Square Feet	Minimum Thickness
Less than 7.5	0.080
7.5 to 15	0.100
Greater than 15	0.125

The Standard Highway Sign Designs for Texas (SHSD) can be found at the following website.

http://www.txdot.gov/

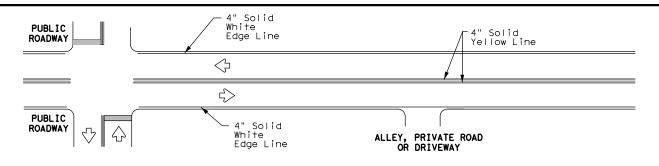


Traffic Operations Division Standard

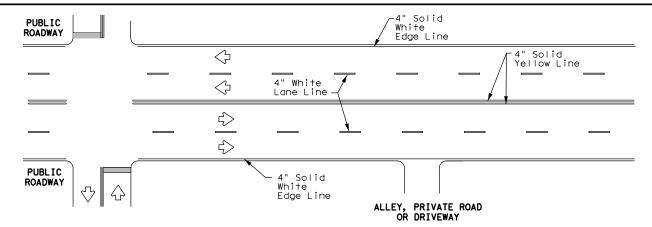
TYPICAL SIGN REQUIREMENTS

TSR(3)-13

FILE:	tsr3-13.dgn	DN: T	OOT	ck: TxDOT	DW:	TxD0	T CK: TXDOT
© TxD0T	October 2003	CONT	SECT	JOB			HIGHWAY
	REVISIONS	0106	04	036		L	IS 380
12-03 7-1	3	DIST		COUNTY			SHEET NO.
9-08		ABL	STONEWALL				160

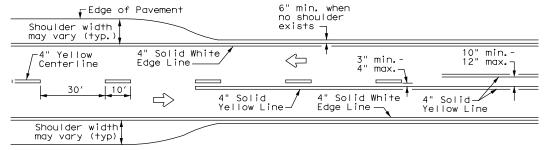


### TYPICAL TWO-LANE, TWO-WAY PAVEMENT MARKINGS THROUGH INTERSECTIONS



## TYPICAL MULTI-LANE, TWO-WAY PAVEMENT MARKINGS THROUGH INTERSECTIONS

being marked equal to or less than 40 MPH.



TWO LANE TWO-WAY ROADWAY

WITH OR WITHOUT SHOULDERS

-6" min.

⊢6" min.

3" min.-4" usual-(12" max. for

traveled way

greater than 48' only)

 $\Rightarrow$ 

 $\Rightarrow$ 

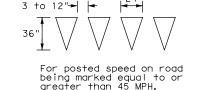
-Edge of Pavement

-Edge of Pavement

4" Solid Yellow Line-

4" Solid White





### YIELD LINES

### Pavement Edge -4" Solid White 4" White Lane Line\_ $\langle \neg$ Edge Line 4" Solid Yellow 10′ -4" Solid Yellow Line Edge Line -See Note 25 See \_ Note 1-10" min. Taper max. Optional 8" Solid White Line Dotted 8" White ΔΔΔΔΔ Extension See note 3 ine ♣48" min. from edge Triangles line to 4" Solid Yellowstop/yield Storage Edge Line Deceleration 4" Solid White $\Rightarrow$ White Lane Line Edge Line-

FOUR LANE DIVIDED ROADWAY CROSSOVERS

### **NOTES**

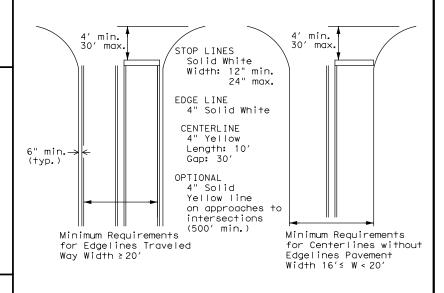
- 1. Where divided highways are separated by median widths at the median opening itself of 30 feet or more, median openings shall be signed as two separate intersections. Each median opening has two width measurements, with one measurement for each approach. The narrow median width will be the controlling width to determine if signs are required. Yield signs are the typical intersection control. Stop signs are optional as determined by the Engineer.
- 2. Install median striping (double yellow centerlines and stop bars/yield triangles) when a 50' or greater median centerline can be placed. Stop bars shall only be used with stop signs. Yield traingles shall only be used with yield signs.
- 3. Length of turn bays, including taper, deceleration, and storage lengths shall be as shown on the plans or as directed by the Engineer.

### **GENERAL NOTES**

- 1. Edgeline striping shall be as shown in the plans or as directed by the Engineer. The edgeline should not be placed less less than 6 inches from the edge of pavement. This distance may vary due to pavement raveling or other conditions. Edgelines are not required in curb and gutter sections of roadways.
- 2. The traveled way includes only that portion of the roadway used for vehicular travel. It does not include the parking lanes, sidewalks, berms and shoulders. The traveled ways shall be measured from the inside of edgeline to the inside of edgeline of a two lane roadway.

MATERIAL SPECIFICATIONS	
PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
EPOXY AND ADHESIVES	DMS-6100
BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
TRAFFIC PAINT	DMS-8200
HOT APPLIED THERMOPLASTIC	DMS-8220
PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240

All pavement marking materials shall meet the required Departmental Material Specifications as specified by the plans.



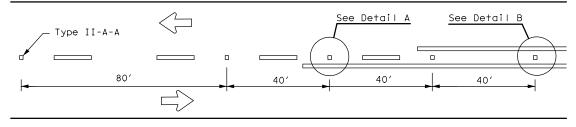
### GUIDE FOR PLACEMENT OF STOP LINES, EDGE LINE & CENTERLINE

Based on Traveled Way and Pavement Widths for Undivided Highways

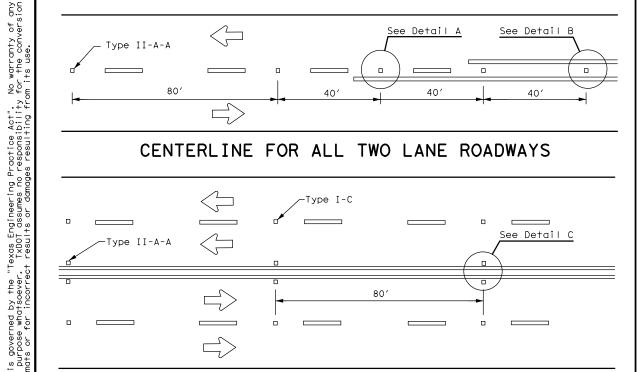


PM(1) - 20

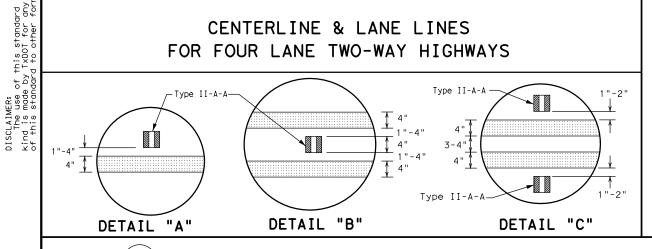
FILE: pm1-20.dgn	DN:		CK:	DW:	CK:
© TxDOT November 1978	CONT	SECT	JOB		HIGHWAY
8-95 3-03 REVISIONS	0106	04	036	ı	JS 380
5-00 2-12	DIST		COUNTY		SHEET NO.
8-00 6-20	ABL		STONEW	ALL	161



### CENTERLINE FOR ALL TWO LANE ROADWAYS



## CENTERLINE & LANE LINES FOR FOUR LANE TWO-WAY HIGHWAYS



OPTIONAL 6" EDGE

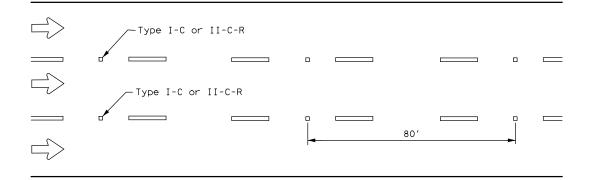
OR LÂNE LINE

LINE, CENTER LINE

NOTE

# Centerline < Symmetrical around centerline Continuous two-way left turn lane Type II-A-A 80′ Type I-C

### CENTERLINE AND LANE LINES FOR TWO-WAY LEFT TURN LANE



### LANE LINES FOR ONE-WAY ROADWAY (NON-FREEWAY FACILITIES)

Raised pavement markers Type II-C-R shall have clear face toward normal traffic and red face toward wrong-way traffic.

### CENTER OR EDGE LINE <del>|</del> 12"± 1" 30′ BROKEN LANE LINE REFLECTORIZED PROFILE PATTERN DETAIL USING REFLECTIVE PROFILE PAVEMENT MARKINGS 18"± 1" -300 to 500 mil , in height 12"<u>+</u> 1" $5\frac{1}{2}$ " ± $\frac{1}{2}$ " 31/4 "± 3/4 "\$ A quick field check for the thickness 2 to 3"--2 to 3"-of base line and profile marking is approximately equal to a stack of 5 quarters to a maximum height of 7 quarters.

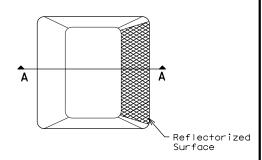
Profile markings shall not be placed on roadways with a posted speed limit of 45 MPH or less.

### GENERAL NOTES

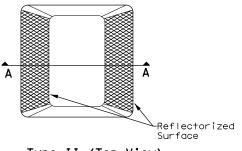
- 1. All raised pavement markers placed in broken lines shall be placed in line with and midway between the stripes.
- 2. On concrete pavements the raised pavement markers should be placed to one side of the longitudinal

	MATERIAL SPECIFICATIONS	
	PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
_	EPOXY AND ADHESIVES	DMS-6100
	BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
	TRAFFIC PAINT	DMS-8200
	HOT APPLIED THERMOPLASTIC	DMS-8220
	PERMANENT PREFABRICATED PAVEMENT MARKINGS	DMS-8240
	·	

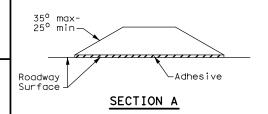
All pavement marking materials shall meet the required Departmental Material Specifications as specified by the plans.



Type I (Top View)



Type II (Top View)



RAISED PAVEMENT MARKERS



POSITION GUIDANCE USING RAISED MARKERS RELECTORIZED PROFILE

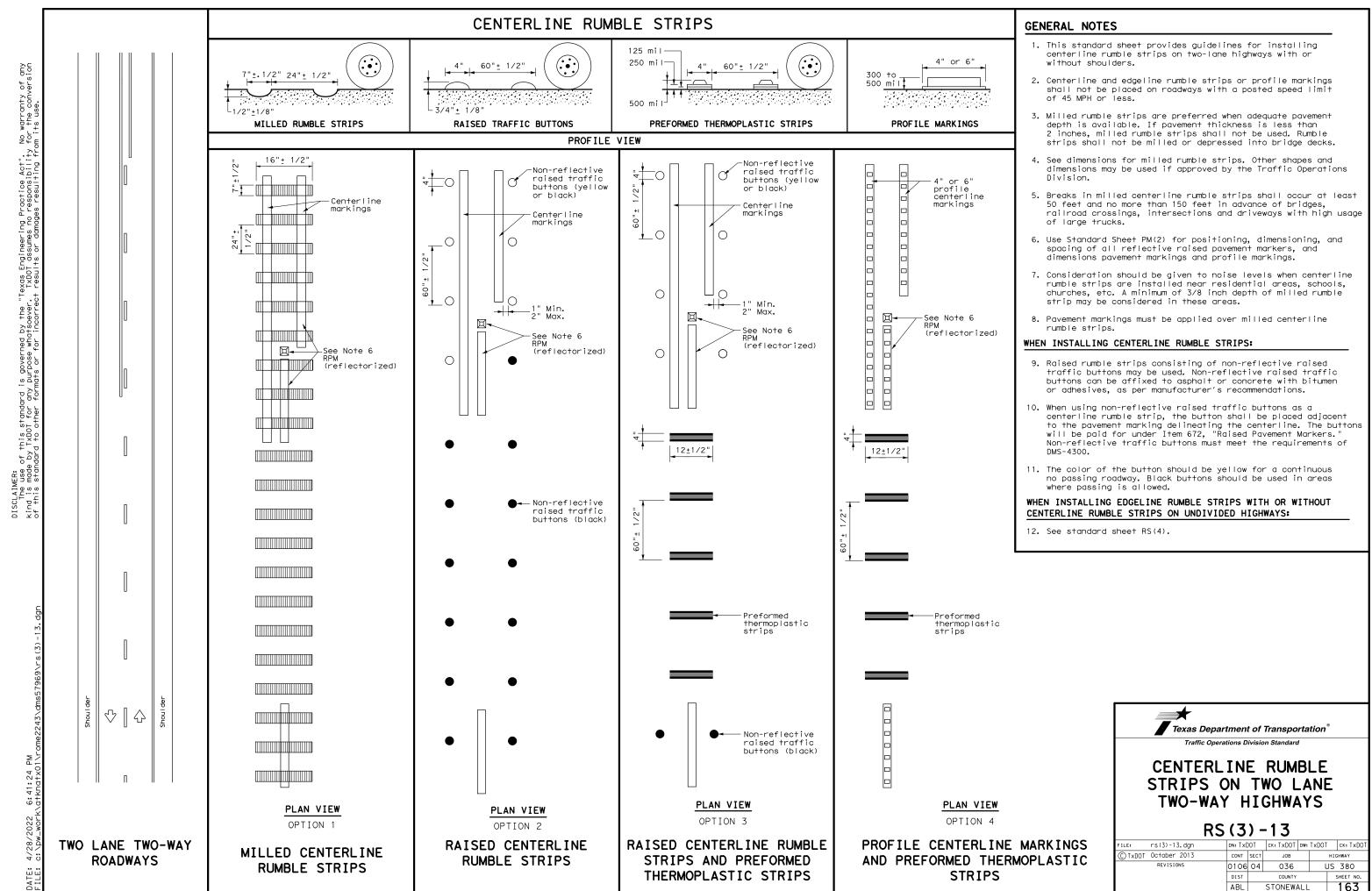
Traffic Safety Division Standard

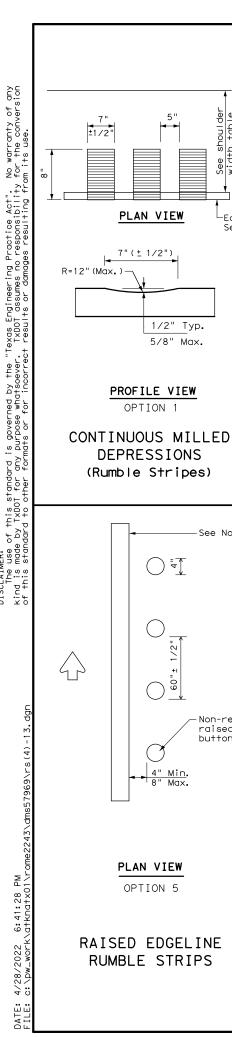
**MARKINGS** PM(2) - 20

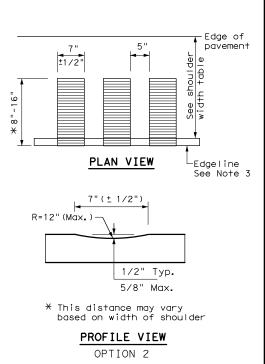
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4" EDGE LINE,

CENTER LINE OR LANE LINE







CONTINUOUS MILLED

**DEPRESSIONS** 

(Rumble Stripes)

4" or 6'

profile

edgeline markina

See Note 3

PLAN VIEW

OPTION 6

PROFILE EDGELINE

**MARKINGS** 

Edge of

pavement

-Edgeline

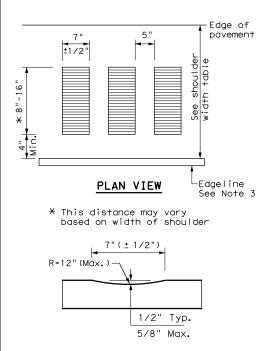
-See Note 3

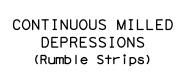
Non-reflective raised traffic

1/2" Typ.

5/8" Max.

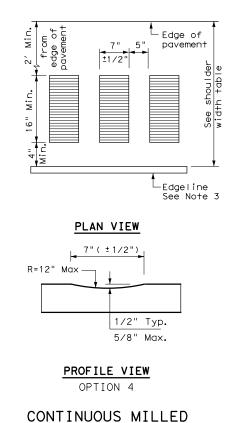
Seé Note 3



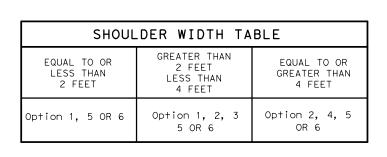


PROFILE VIEW

OPTION 3



**DEPRESSIONS** (Rumble Strips)



### **GENERAL NOTES**

- 1. Rumble strips and profile markings shall not be placed on roadways with a posted speed limit of 45 MPH or less.
- 2. Milled rumble strips are preferred when adequate pavement depth is available. If pavement thickness is less than 2 inches, milled rumble strips shall not be used. Rumble strips shall not be milled or depressed into bridge decks.
- 3. Use Standard Sheet PM(2) for positioning, dimensioning, and spacing of all reflective raised pavement markers, pavement markings, and profile markings.
- 4. See the table below for determining what options may be used for edgeline rumble strips.

### WHEN INSTALLING MILLED DEPRESSION EDGELINE RUMBLE STRIPS:

- 5. See dimensions for milled rumble strips. Other shapes and dimensions may be used if approved by the Traffic Operations
- 6. Pavement markings can be applied over milled shoulder rumble strips to create an edgeline rumble stripe.
- 7. Breaks in edgeline rumble strips shall occur at least 50 feet and no more than 150 feet in advance of bridges, railroad crossings, intersections and driveways with high usage of large trucks when installed on conventional highways.
- 8. Rumble strips shall not be placed across exit or entrance ramps, acceleration and deceleration lanes, crossovers, gore areas or intersections with other roadways.
- 9. Consideration should be given to noise levels when edgeline rumble strips are installed near residential areas, schools, churches, etc. A minimum of 3/8 inches depth of milled rumble strip may be considered in these areas.
- 10. On roadways with high bicycle activity, consideration should be given before the installation of edgeline rumble strips. Things to consider include size of rumble strips, rumble strip material and location of rumble strips on the shoulder If the designer determines that gaps are needed in the rumble strips due to bicycle use of the road, then follow the requirement shown in FHWA Technical Advisory T5040.39, or latest version. A detail of the spacing shall be included in the plans.

### WHEN INSTALLING RAISED OR PROFILE EDGELINE RUMBLE STRIPS:

- 11. Raised rumble strips consisting of non-reflective raised traffic buttons may be used. Non-reflective raised traffic buttons can be affixed to asphalt or concrete with bitumen or adhesives, as per the manufacturer's recommendations.
- 12. Non-reflective traffic buttons shall be placed adjacent to the pavement marking delineating the edgeline when used as a rumble strip. The color of the button should match the color of the adjacent edgeline marking (white or yellow). The buttons will be paid for under Item 672, "Raised Pavement Markers." Non-reflective traffic buttons must meet the requirements of DMS-4300.
- 13. Non-reflective traffic buttons shall not be placed across exit or entrance ramps, acceleration and deceleration lanes, crossovers, gore areas or intersections with other roadways.
- 14. Breaks in edgeline rumble strips using raised traffic buttons shall occur at least 50 feet and no more than 150 feet in advance of bridges, railroad crossing, intersections and driveways with high usage of large trucks when installed on
- 15. The minimum distance between the edgeline and the buttons should be used if the shoulder is less than 8 feet in width.
- 16. Raised profile thermoplastic markings used as edgelines may substitute for buttons.



ON UNDIVIDED OR TWO LANE HIGHWAYS RS(4) - 13

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C) TxDOT	October 2013	CONT	SECT	JOB		н	GHWAY
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		DIST		COUNTY			SHEET NO.
		ABL		STONEW	ALL		164

93

SIGN SUPPORT DESCRIPTIVE CODES (Descriptive Codes correspond to project estimate and quantities sheets)

### SM RD SGN ASSM TY XXXXX(X)XX(X-XXXX)

### Post Type

FRP = Fiberglass Reinforced Plastic Pipe (see SMD(FRP)) TWT = Thin-Walled Tubing (see SMD(TWT))

10BWG = 10 BWG Tubing (see SMD(SLIP-1) to (SLIP-3)) S80 = Schedule 80 Pipe (see SMD(SLIP-1) to (SLIP-3))

### Number of Posts (1 or 2) -

### Anchor Type

UA = Universal Anchor - Concreted (see SMD(FRP) and (TWT)) UB = Universal Anchor - Bolted down (see SMD(FRP) and (TWT))

WS = Wedge Anchor Steel - (see SMD(TWT))

WP = Wedge Anchor Plastic (see SMD(TWT))

SA = Slipbase - Concreted (see SMD(SLIP-1) to (SLIP-3))

SB = Slipbase - Bolted Down (see SMD(SLIP-1) to (SLIP-3))

### Sign Mounting Designation

P = Prefab. "Plain" (see SMD(SLIP-1) to (SLIP-3), (TWT), (FRP))

T = Prefab. "T" (see SMD(SLIP-1) to (SLIP-3), (TWT)) U = Prefab. "U" (see SMD(SLIP-1) to (SLIP-3))

No more than 2 sign

posts should be located

within a 7 ft. circle.

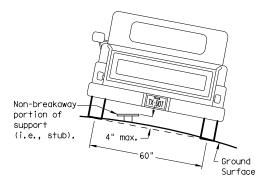
1EXT or 2EXT = Number of Extensions (see SMD(SLIP-1) to (SLIP-3), (TWT))| BM = Extruded Wind Beam (see SMD(SLIP-1) to (SLIP-3))

WC = 1.12 #/ft Wing Channel (see SMD(SLIP-1) to (SLIP-3))

diameter

EXAL = Extruded Aluminum Sign Panels (see SMD(SLIP-3))

## REQUIRED CLEARANCE FOR BREAKAWAY SUPPORT



To avoid vehicle undercarriage snagging, any substantial remains of a breakaway support. when it is broken away, should not project more than 4 inches above a 60-inch chord (i.e., typical space between wheel paths).

# SIGN LOCATION

### PAVED SHOULDERS

BEHIND BARRIER

\*\*Sign clearance based on distance required for proper guard rail or concrete barrier performance.

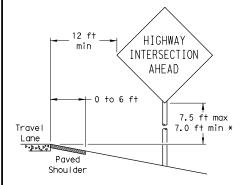
2 ft min\*\*

Travel

D. 2 . 4 . 10 4

Paved

Shoul der



### LESS THAN 6 FT. WIDE

HIGHWAY

INTERSECTION

AHEAD

7.5 ft max

7.0 ft min \*

Guard

BEHIND GUARDRAIL

When the shoulder is 6 ft. or less in width. the sign must be placed at least 12 ft. from the edge of the travel lane.

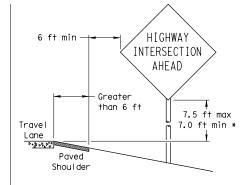
5 ft min\*\*

Travel

D. 21 . 4. 10°4

Paved

Shoul der



### GREATER THAN 6 FT. WIDE

When the shoulder is greater than 6 ft in width. the sign must be placed at least 6 ft. from the edge of the shoulder.

HIGHWAY

INTERSECTION

AHEAD

7.5 ft max

7.0 ft min >

### When this sign is needed at the end of a two-lane, two way roadway, the right edge of the sign should be in line with the centerline of the roadway. Place as close to ROW as practical.

Paved

Shoulder

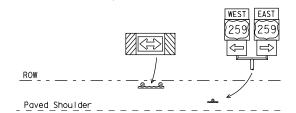
T-INTERSECTION

· 12 ft min

← 6 ft min

7.5 ft max

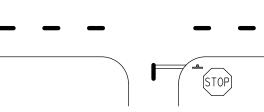
7.0 ft min \*



Edge of Travel Lane

Travel

Lane



- \* Signs shall be mounted using the following condition that results in the greatest sign elevation:
- (1) a minimum of 7 to a maximum of 7.5 feet above the edge of the travel lane or (2) a minimum of 7 to a maximum of 7.5 feet above the
- grade at the base of the support when sign is installed on the backslope.

The maximum values may be increased when directed by

See the Traffic Operations Division website for detailed drawings of sign clamps, Triangular Slipbase System components and Wedge Anchor System components.

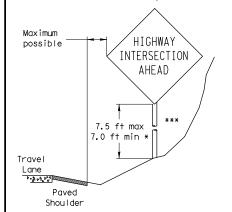
The website address is: http://www.txdot.gov/publications/traffic.htm

### RESTRICTED RIGHT-OF-WAY (When 6 ft min. is not possible.)

Concrete

BEHIND CONCRETE BARRIER

Barrier



Right-of-way restrictions may be created by rocks, water, vegetation, forest, buildings, a narrow island, or other factors.

In situations where a lateral restriction prevents the minimum horizontal clearance from the edge of the travel lane, signs should be placed as far from the travel lane as practical.

\*\*\* Post may be shorter if protected by guardrail or if Engineer determines the post could not be hit due to extreme

### TYPICAL SIGN ATTACHMENT DETAIL SIGNS WITH PLAQUES

-Nut. Lock

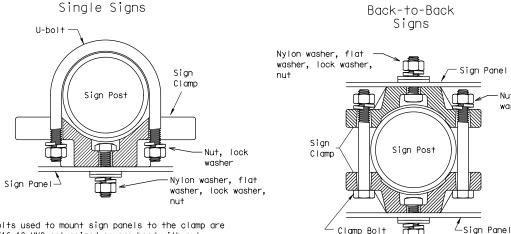
Not Acceptable

7 ft.

diameter

circle

Not Acceptable



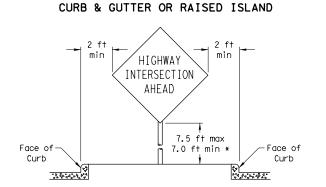
washer, lock washer,

7 ft.

diameter

circle

### EAST 7.5 ft max-7.0 ft min \* When a supplemental plaque Travel or secondary sign is used, the 7 ft sign height is 4,000,000 measured to the bottom of the supplemental plaque Payed or secondary sign. Shou I der





Texas Department of Transportation Traffic Operations Division

# SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS GENERAL NOTES & DETAILS

SMD (GEN) -08

© TxDOT July 2002	DN: TXD	OT	CK: TXDOT	DW:	TXDOT	CK: TXDOT	
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# Bolts used to mount sign panels to the clamp are Clamp Bolt 5/16-18 UNC galvanized square head with nut, nylon washer, flat washer and lock washer. The Nylon washer, flat

Not Acceptable

Approximate Bolt Length Pipe Diameter Specific Clamp Universal Clamp 3 or 3 1/2" 2" nominal 2 1/2" nominal 3 or 3 1/2" 3 1/2 or 4" 3" nominal 3 1/2 or 4" 4 1/2"

- Sian Bolt

Acceptable

diameter

circle

# Sign clamps may be either the specific size clamp

When two sign clamps are used to mount signs

back-to-back, use a 5/16-18 UNC galvanized hex

right. The bolt length may need to be adjusted

head per ASTM A307 with nut and helical-spring lock

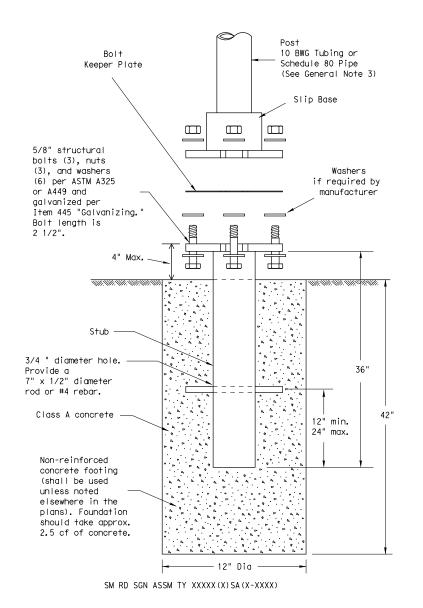
washer. The approximate bolt lengths for various post

sizes and sign clamp types are given in the table at

bolt length is 1 inch for aluminum.

depending upon field conditions.

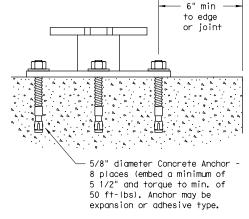
### TRIANGULAR SLIPBASE INSTALLATION GENERAL REQUIREMENTS



### NOTE

There are various devices approved for the Triangular Slipbase System. Please reference the Material Producer List for approved slip base systems. http://www.txdot.gov/business/producer list.htm The devices shall be installed per manufacturers' recommendations. Installation procedures shall be provided to the Engineer by Contractor.

### CONCRETE ANCHOR



SM RD SGN ASSM TY XXXXX(X)SB(X-XXXX)

Concrete anchor consists of 5/8" diameter stud bolt with UNC series bolt threads on the upper end. Heavy hex nut per ASTM A563, and hardened washer per ASTM F436. The stud bolt shall have a minimum yield and ultimate tensile strength of 50 and 75 KSI, respectively. Nuts, bolts and washers shall be galvanized per Item 445, "Galvanizing." Adhesive type anchors shall have stud bolts installed with Type III epoxy per DMS-6100, "Epoxies and Adhesives." Adhesive anchors may be loaded after adequate epoxy cure time per the manufacturer's recommendations. Top of bolt shall extend at least flush with top of the nut when installed. The anchor, when installed in 4000 psi normalweight concrete with a 5 1/2" minimum embedment, shall have a minimum allowable tension and shear of 3900 and 3100 psi, respectively.

### GENERAL NOTES:

- 1. Slip base shall be permanently marked to indicate manufacturer. Method, design, and location of marking are subject to approval of the TxDOT Traffic Standards Engineer.
- 2. Material used as post with this system shall conform to the following specifications:

10 BWG Tubing (2.875" outside diameter)

0.134" nominal wall thickness

Seamless or electric-resistance welded steel tubing or pipe Steel shall be HSLAS Gr 55 per ASTM A1011 or ASTM A1008

Other steels may be used if they meet the following:

55,000 PSI minimum yield strength 70,000 PSI minimum tensile strength

20% minimum elongation in 2"

Wall thickness (uncoated) shall be within the range of 0.122" to 0.138"

Outside diameter (uncoated) shall be within the range of 2.867" to 2.883"

Galvanization per ASTM A123 or ASTM A653 G210. For precoated steel tubing (ASTM A653), recoat

tube outside diameter weld seam by metallizing with zinc wire per ASTM B833.

Schedule 80 Pipe (2.875" outside diameter)

0.276" nominal wall thickness

Steel tubing per ASTM A500 Gr C

Other seamless or electric-resistance welded steel tubing or pipe with equivalent

outside diameter and wall thickness may be used if they meet the following:

46,000 PSI minimum yield strength

62,000 PSI minimum tensile strength

21% minimum elongation in 2"

Wall thickness (uncoated) shall be within the range of 0.248" to 0.304" Outside diameter (uncoated) shall be within the range of 2.855" to 2.895"

Galvanization per ASTM A123

3. See the Traffic Operations Division website for detailed drawings of sign clamps and Texas Universal Triangular Slipbase System components. The website address is:

http://www.txdot.gov/publications/traffic.htm

4. Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.

### ASSEMBLY PROCEDURE

### Foundation

- 1. Prepare 12-inch diameter by 42-inch deep hole. If solid rock is encountered, the depth of the foundation may be reduced such that it is embedded a minimum of 18 inches into the solid rock.
- 2. The Engineer may permit batches of concrete less than 2 cubic yards to be mixed with a portable. motor-driven concrete mixer. For small placements less than 0.5 cubic yards, hand mixing in a suitable container may be allowed by Engineer. Concrete shall be Class A.
- 3. Push the pipe end of the slip base stub into the center of the concrete. Rotate the stub back and forth while pushing it down into the concrete to assure good contact between the concrete and stub. Continue to work the stub into the concrete until it is between 2 to 4 inches above the ground.
- 4. Plumb the stub. Allow a minimum of 4 days to set, unless otherwise directed by the Engineer.
- 5. The triangular slipbase system is multidirectional and is designed to release when struck from any

- 1. Cut support so that the bottom of the sign will be 7 to 7.5 feet above the edge of the travelway (i.e., edge of the closest lane) when slip plate is below the edge of pavement or 7 to 7.5 feet above slip plate when the slip plate is above the edge of the travelway. The cut shall be plumb and
- 2. Attach sign to support using connections shown. When multiple signs are installed on the same support, ensure the minimum clearance between each sign is maintained. See SMD(SLIP-2) for clearances based on sign types.



# SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

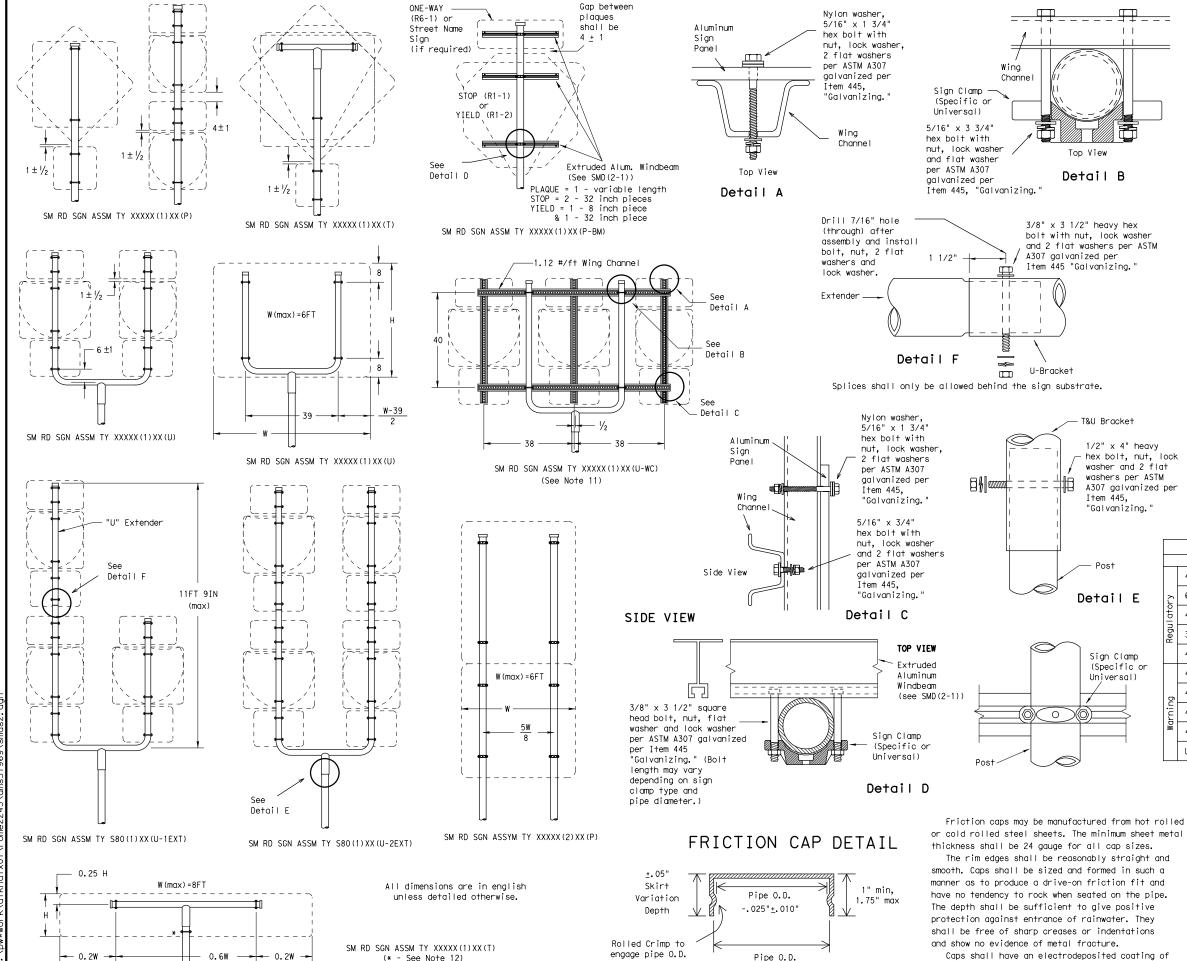
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6:41:43



### GENERAL NOTES:

Top View

Detail B

T&U Bracket

Item 445,

Detail E

Sign Clamp

Universal)

0

zinc in accordance with the requirements of ASTM

B633 Class FE/ZN 8.

+.025"<u>+</u>.010"

(Specific or

"Galvanizing.

1/2" x 4" heavy

hex bolt, nut, lock

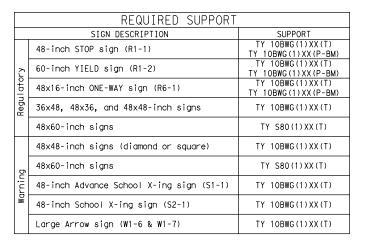
A307 galvanized per

washer and 2 flat

washers per ASTM

1.	SIGN SUPPORT	# OF POSTS	MAX. SIGN AREA
	10 BWG	1	16 SF
	10 BWG	2	32 SF
	Sch 80	1	32 SF
	Sch 80	2	64 SF

- 2. The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.
- 3. Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.
- 4. Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.
- 5. Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.
- 6. For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of areater height.
- 7. When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly' connected to each other except through the sign panel. This will allow each support to act independently when impacted by an errant vehicle.
- Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.
- 9. Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing.
- 10. Additional route markers may be added vertically, provided the total sign area does not exceed the maximum allowable amount per Note 1.
- 11. Additional sign clamp required on the "T-bracket" post for 24 inch height signs. Place the clamp 3 inches above bottom of sign when possible.
- 12. Post open ends shall be fitted with Friction Caps.
- 13. Sign blanks shall be the sizes and shapes shown on the plans.



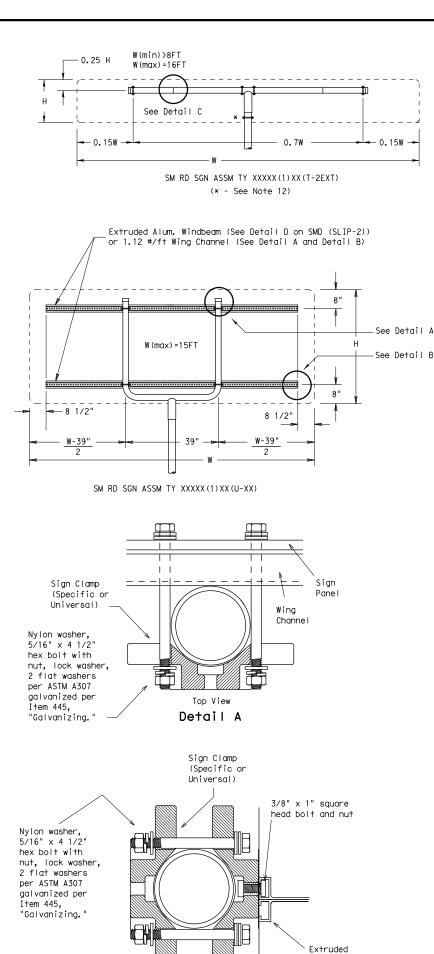
Texas Department of Transportation Traffic Operations Division

# SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

SMD (SLIP-2) -08

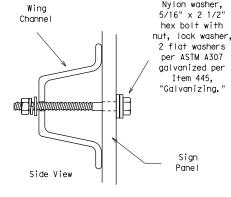
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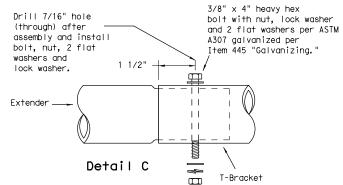


EXTRUDED ALUMINUM SIGN WITH T BRACKET

Aluminum Panel



Detail B



Splices shall only be allowed behind the sign substrate.

Sign

Clamps

(Specific or

Universal)

3/8" x 4 1/2

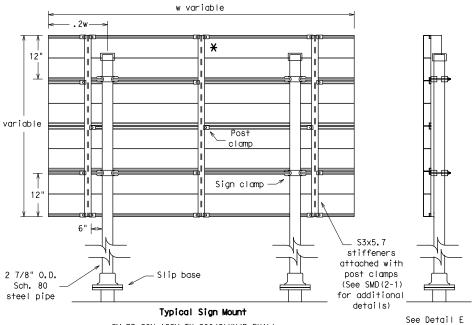
square head bolt, nut, flat washer and lock washer per

ASTM A307 galvanized

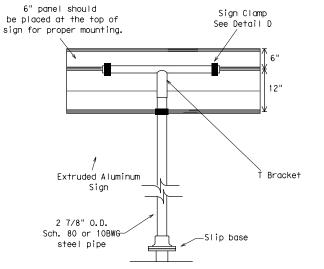
per Item 445.

"Galvanizing.

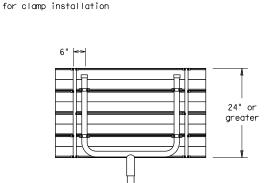
Detail E



SM RD SGN ASSM TY S80(2)XX(P-EXAL) \* Additional stiffener placed at approximate center of signs when sign width is greater than 10'.



Extruded Aluminum Sign With T Bracket



Use Extruded Alum. Windbeam as stiffeners See SMD (2-1) for additional details See Detail E

for clamp installation

### GENERAL NOTES:

1.	SIGN SUPPORT	# OF POSTS	MAX. SIGN AREA
	10 BWG	1	16 SF
	10 BWG	2	32 SF
	Sch 80	1	32 SF
	Sch 80	2	64 SF

- 2. The Engineer may require that a Schedule 80 post be used in place of a 10 BWG where a sign height is abnormally high due to a fill slope.
- 3. Sign supports shall not be spliced except where shown. Sign support posts shall not be spliced.
- 4. Aluminum sign blanks shall conform to Departmental Material Specifications DMS-7110 and shall have the following minimum thicknesses: 0.080 for signs less than 7.5 sq. ft., 0.100 for signs 7.5 to 15 sq. ft., and 0.125 for signs greater than 15 sq. ft.
- 5. Signs that require specific supports due to reasons in addition to windloading are indicated on the "REQUIRED SUPPORT" table on this sheet.
- 6. For horizontal rectangular signs fabricated from flat aluminum, T-brackets are used for signs 24 inches or less in height. U-brackets are used for signs of areater height.
- 7. When two triangular slipbase supports are used to support a single sign, they shall not be "rigidly' connected to each other except through the sign panel. This will allow each support to act independently when impacted by an errant vehicle.
- Wing channel shall meet ASTM A 1011 SS Gr 50 and be galvanized per ASTM A 123.
- 9. Excess pipe, wing channel, or windbeam shall be cut off so that it does not extend beyond the sign panel (i.e., excess support shall not be visible when the sign is viewed from the front.) Repair galvanized coating at cut support ends per Item 445, "Galvanizing."
- 10. Sign blanks shall be the sizes and shapes shown on
- 11. Additional sign clamp required on the "T-bracket" post for 24 inch high signs. Place the clamp 3 inches above bottom of sign when possible.
- 12. Post open ends shall be fitted with Friction Caps.

REQUIRED SUPPORT						
	SIGN DESCRIPTION	SUPPORT				
	48-inch STOP sign (R1-1)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
_	60-inch YIELD sign (R1-2)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
Regulatory	48x16-inch ONE-WAY sign (R6-1)	TY 10BWG(1)XX(T) TY 10BWG(1)XX(P-BM)				
Regu	36x48, 48x36, and 48x48-inch signs	TY 10BWG(1)XX(T)				
	48x60-inch signs	TY S80(1)XX(T)				
	48x48-inch signs (diamond or square)	TY 10BWG(1)XX(T)				
ğ	48x60-inch signs	TY S80(1)XX(T)				
Warning	48-inch Advance School X-ing sign (S1-1)	TY 10BWG(1)XX(T)				
×	48-inch School X-ing sign (S2-1)	TY 10BWG(1)XX(T)				
	Large Arrow sign (W1-6 & W1-7)	TY 10BWG(1)XX(T)				



# SIGN MOUNTING DETAILS SMALL ROADSIDE SIGNS TRIANGULAR SLIPBASE SYSTEM

SMD (SLIP-3) -08

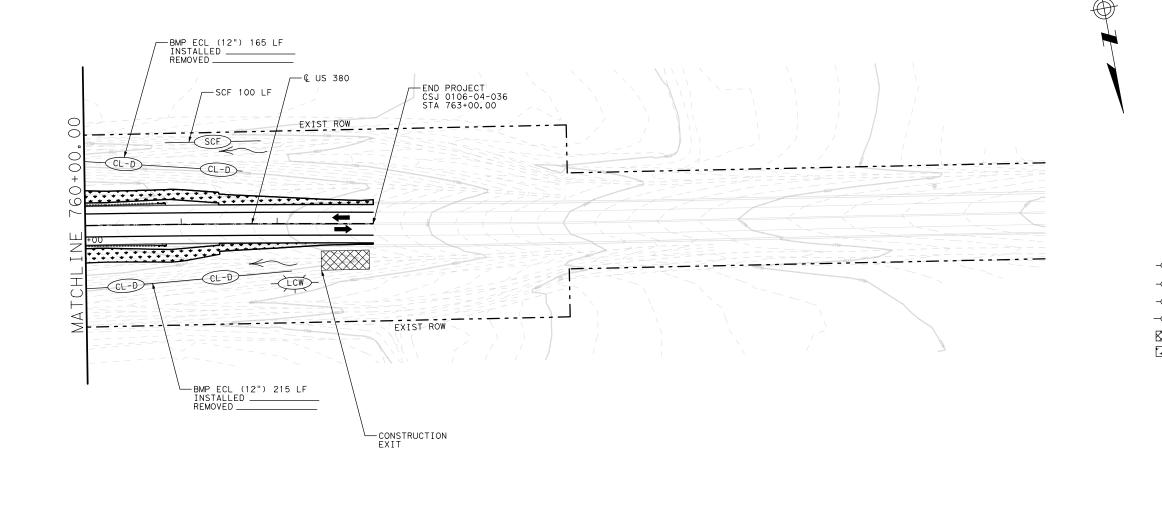
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	DIST		COUNTY SHEET NO.			SHEET NO.	
	ABL		STONEWA	\LL		168	

NOTES: 1. SEDIMENT CONTROLS SHALL BE IN PLACE PRIOR TO COMMENCING ANY SOIL DISTURBING ACTIVITIES. -BMP ECL (12") 300 LF INSTALLED ... 2. ALL CONTROLS TO BE PLACED AS SHOWN ON STANDARDS EC(1), EC(2), EC(3), AND EC(9). SUBMITTA -BMP RFD (TY 3) 20 LF INSTALLED \_\_\_\_\_ REMOVED \_\_\_\_ 3. ALL PERIMETER SEDIMENT CONTROLS TO REMAIN IN PLACE FOR THE DURATION OF THE PROJECT OR UNTIL DISTURBED AREAS ARE STABILIZED. -BEGIN PROJECT CSJ 0106-04-036 STA 743+00.00 —SCF 100 LF -CONSTRUCTION EXIT -LIMITS OF CRITICAL HABITAT 4. EROSION CONTROL QUANTITIES ARE APPROXIMATE AND MAY BE ADJUSTED TO MEET FIELD CONDITIONS. 00 00. 5. EXACT LOCATION OF ROCK FILTER DAM (RDF) AND TEMPORARY SEDIMENT CONTROL FENCE TO BE DETERMINED BY THE ENGINEER. CL-D 00% 6. FINAL LOCATIONS OF THE LINED CONCRETE WASHOUTS TO BE DETERMINED BY THE ENGINEER. SUBSIDIARY TO ITEM 506 "TEMPORARY EROSION, SEDIMENTATION, AND ENVIRONMENTAL CONTROLS" **LEGEND** -CL-D- EROSION CONTROL LOGS EXIST ROW ROCK FILTER DAM (TY #) SEDIMENT CONTROL FENCE LINED CONCRETE WASHOUT CONSTRUCTION EXIT SEEDING -BMP RFD (TY 3) 20 LF INSTALLED \_\_\_\_\_ REMOVED \_\_\_\_\_ TRAFFIC DIRECTION - @ US 380 FLOW DIRECTION **ENVIRONMENTALLY** -BMP ECL (12") 200 LF INSTALLED \_\_\_\_ SCALE = 1"=100' SENSITIVE AREA REMOVED \_ BMP ECL (12") 85 LF — INSTALLED \_\_\_\_\_ REMOVED \_\_\_\_\_ **ENVIRONMENTALLY** SENSITIVE AREA BMP RFD (TY 3) 20 LF-INSTALLED \_\_\_\_ REMOVED \_\_\_\_ JENELLE N. ROMERO 97940 CONSTRUCTION OF THE STATE -€ US 380 Ō  $\circ$ 0  $\circ$ **ATKINS** EXIST ROW Texas Department of Transportation Abilene District US 380 LIMITS OF CRITICAL HABITAT BMP RFD (TY 3) 20 LF INSTALLED SW3P SITE PLAN REMOVED \_\_ BMP ECL (12") 85 LF - INSTALLED \_\_\_\_\_ SIGNED: SG CHECKED: JD TEXAS

SHEET 1 OF

4/28/2022

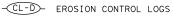
PROJECT No. HIGHWAY No. SEE TITLE SHEET US 380 RAWN: SG STATE CONTROL SECTION JOB No. COUNTY STONEWALL 0106 04

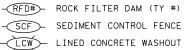


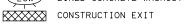
## NOTES:

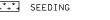
- 1. SEDIMENT CONTROLS SHALL BE IN PLACE PRIOR TO COMMENCING ANY SOIL DISTURBING ACTIVITIES.
- 2. ALL CONTROLS TO BE PLACED AS SHOWN ON STANDARDS EC(1), EC(2), EC(3), AND EC(9).
- 3. ALL PERIMETER SEDIMENT CONTROLS TO REMAIN IN PLACE FOR THE DURATION OF THE PROJECT OR UNTIL DISTURBED AREAS ARE STABILIZED.
- 4. EROSION CONTROL QUANTITIES ARE APPROXIMATE AND MAY BE ADJUSTED TO MEET FIELD CONDITIONS.
- 5. EXACT LOCATION OF ROCK FILTER DAM (RDF) AND TEMPORARY SEDIMENT CONTROL FENCE TO BE DETERMINED BY THE ENGINEER.
- 6. FINAL LOCATIONS OF THE LINED CONCRETE
  WASHOUTS TO BE DETERMINED BY THE ENGINEER,
  SUBSIDIARY TO ITEM 506 "TEMPORARY EROSION,
  SEDIMENTATION, AND ENVIRONMENTAL CONTROLS"

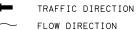
# **LEGEND**















# **ATKINS**



US 380

SW3P SITE PLAN

						SH	EET	2 OF 2
SIGNED	:SG	FED. RD DIV. No.	STATE	PROJE	ECT No.		ніс	SHWAY No.
HECKED:	JD	6	TEXAS	SEE TIT	LE SHE	ΞT	U	S 380
RAWN:	SG	STATE DISTRICT	COUNTY	CONTROL No.	SECTION No.		OB o.	SHEET No.
HECKED:	JD	ABL	STONEWALL	0106	04	0	36	170

# SITE DESCRIPTION PROJECT LIMITS: THE PROJECT LIMITS SHOWN ON THE TITLE SHEET AND LIMITS OF TXDOT RIGHT OF WAY SHALL ALSO BE THE LIMITS OF COVERAGE OF THE SW3P. PROJECT LOCATION MAPS: TITLE SHEET DRAINAGE PATTERNS: DRAINAGE AREA MAPS APPROX. SLOPES ANTICIPATED AFTER MAJOR GRADING AND AREAS OF SOIL DISTURBANCE: TYPICAL MAJOR CONTROLS AND LOCATIONS OF STABILIZATION PRACTICES: SW3P SITE PLAN PROJECT SPECIFIC LOCATIONS: TO BE SPECIFIED BY PROJECT FIELD OFFICE AND LOCATED IN THE PROJECT SW3P FILE. SURFACE WATERS AND DISCHARGE LOCATIONS: DRAINAGE SHEETS TYPICAL AREAS WHICH WILL NOT BE DISTURBED: SW3P SITE PLAN ENDANGERED SPECIES, DESIGNATED CRITICAL HABITAT AND HISTORIC PROPERTY: EPIC SHEET ESTIMATED START DATES AND DURATION OF ACTIVITIES IN THE INTENDED SCHEDULE/SEQUENCE OF EARTH-DISTURBING ACTIVITIES: CONTRACT TIME ESTIMATE NATURE OF ACTIVITY: FOR THE CONSTRUCTION OF BRIDGE REPLACEMENT CONSISTING OF: REPLACING BRIDGE AND BRIDGE APPROACHES. MAJOR SOIL DISTURBING ACTIVITIES: SOIL DISTURBING ACTIVITIES WILL INCLUDE BRIDGE AND BRIDGE APPROACH REPLACEMENT. TOTAL PROJECT AREA: 6.65 ACRES TOTAL AREA TO BE DISTURBED (AT EACH SITE): 2.23 ACRES WEIGHTED RUNOFF COEFFICIENT BEFORE CONSTRUCTION: WEIGHTED RUNOFF COEFFICIENT AFTER CONSTRUCTION: EXISTING CONDITION OF SOIL & VEGETATIVE COVER: GOOD % OF EXISTING VEGETATIVE COVER: 69%

NAME OF RECEIVING WATERS:

RIVER BASIN

STREAM SEGMENT 1238 OF THE BRAZOS

### EROSION AND SEDIMENT CONTROLS

USE "T" OR "P" IN THE BLANKS BELOW IF APPLICABLE (T= TEMPORARY, P= PERMANENT)

### SOIL STABILIZATION PRACTICES:

Ρ	BUFFER ZONES	Р	PERMANENT PLANTING, SODDING, OR SEEDING
	MULCHING		PRESERVATION OF NATURAL RESOURCES
	TEMPORARY SEEDING		SOIL RETENTION BLANKET
	OTHER		OTHER

### OTHER:

DISTURBED AREAS ON WHICH CONSTRUCTION ACTIVITY HAS CEASED (TEMPORARILY OR PERMANENTLY) SHALL BE STABILIZED WITHIN 14 DAYS UNLESS ACTIVITIES ARE SCHEDULED TO RESUME WITHIN 14 DAYS.

FOR CONSTRUCTION PROJECTS, THIS DISTRICT OF THE TEXAS DEPARTMENT OF TRANSPORTATION USES SITEMANAGER, A COMPUTER BASED CONSTRUCTION RECORD-KEEPING SYSTEM, AS PART OF RECORD FOR PROJECT WORK INCLUDING ENVIRONMENTAL RELATED ACTIVITIES. DOCUMENTATION DESCRIBING MAJOR GRADING ACTIVITES, TEMPORARY OR PERMANENT CESSATION OF CONSTRUCTION AND STABILIZATION MEASURE IS PART OF THIS SYSTEM AND IS INCORPORATED BY REFERENCE INTO THIS SW3P.

### STRUCTURAL PRACTICES.

STRUCTURAL PRACTIC	F2:
 STORM SEWERS SEDIMENT BASINS SEDIMENT TRAPS SILT FENCES ROCK FILTER DAMS	DIVERSION DIKE AND SWALE COMBINATIONS DIVERSION, INTERCEPTOR, OR PERIMETER DIKES DIVERSION, INTERCEPTOR, OR PERIMETER SWALE: ROCK BEDDING AT CONSTRUCTION EXIT STONE OUTLET STRUCTURES STORM INLET SEDIMENT TRAP TEMPORARY EROSION CONTROL LOGS (BIOLOGS) TIMBER MATTING AT CONSTRUCTION EXIT VEGETATIVE FILTER STRIPS VELOCITY CONTROL DEVICES LINED CONCRETE WASHOUT
OFFSITE VEHICLE TR	ACKING CONTROLS:
 HAUL ROADS DAMPENED FOR DU EXCESS DIRT ON ROAD REMOVE LOADED HAUL TRUCKS TO BE C	D DAILY

### NARRATIVE - SEQUENCE OF CONSTRUCTION (STORM WATER MANAGEMENT) ACTIVITIES:

THE ORDER OF ACTIVITIES WILL BE AS FOLLOWS: SEE THE SEQUENCE OF WORK SHEET

STABILIZED CONSTRUCTION ENTRANCE

### STORM WATER MANAGEMENT:

OTHER

INSTALL EROSION CONTROL LOGS (ECL), ROCK FILTER DAMS (RFD), AND SEDIMENT CONTROL FENCE (SCF) PRIOR TO CONSTRUCTION.



### OTHER EROSION AND SEDIMENT CONTROLS:

### MAINTENANCE:

ALL EROSION AND SEDIMENT CONTROLS WILL BE MAINTAINED IN GOOD WORKING ORDER. IF A REPAIR IS NECESSARY, IT WILL BE DONE AT THE EARLIEST DATE POSSIBLE, BUT NO LATER THAN 7 CALENDAR DAYS AFTER THE SURROUNDING EXPOSED GROUND HAS DRIED SUFFICIENTLY TO PREVENT FURTHER DAMAGE FROM HEAVY EQUIPMENT. THE AREAS ADJACENT TO CREEKS AND DRAINAGE WAYS SHALL HAVE PRIORITY FOLLOWED BY DEVICES PROTECTING STORM SEWER INLETS.

### INSPECTION:

AN INSPECTION WILL BE PERFORMED BY A TXDOT INSPECTOR EVERY 7 DAYS. AN INSPECTION AND MAINTENANCE REPORT WILL BE MADE PER EACH INSPECTION. BASED ON THE INSPECTION RESULTS, THE CONTROLS SHALL BE REVISED PER THE INSPECTION REPORT.

### WASTE MATERIALS:

ALL WASTE MATERIALS WILL BE COLLECTED AND STORED IN A SECURELY LIDDED METAL DUMPSTER. THE DUMPSTER WILL MEET ALL STATE AND LOCAL CITY SOLID WASTE MANAGEMENT REGULATIONS. ALL TRASH AND CONSTRUCTION DEBRIS FROM THE SITE WILL BE DEPOSITED IN THE DUMPSTER. THE DUMPSTER WILL BE EMPTIED AS NECESSARY OR AS REQUIRED BY LOCAL REGULATION AND THE TRASH WILL BE HAULED TO A PERMITTED LANDFILL. NO CONSTRUCTION WASTE MATERIAL WILL BE BURIED ON SITE. CONSTRUCTION DEBRIS AND LITTER SHOULD BE PICKED UP ON A DAILY BASIS UNLESS OTHERWISE DIRECTED BY THE ENGINEER. WASTE AND DIRT PILES SHOULD BE REMOVED ON A WEEKLY BASIS.

### HAZARDOUS WASTE (INCLUDING SPILL REPORTING):

NO LONG TERM WATER QUALITY IMPACTS ARE EXPECTED AS A RESULT OF THE PROPOSED PROJECT. SEE THE NEXT PLAN SHEET FOR A LIST OF POTENTIAL POLLUTANTS. IN THE EVENT OF A MAJOR SPILL. NOTIFY THE TXDOT ENGINEER IMMEDIATELY. ALL PERSONNEL WILL BE INSTRUCTED IN THE PROCEDURES FOR SPILL HANDLING AND DISPOSING OF ANY HAZARDOUS MATERIALS THEY WILL BE USING. ALL SPILLS, INCLUDING THOSE OF LESS THAN 25 GALLONS SHALL BE CLEANED IMMEDIATELY AND ANY CONTAMINATED SOIL SHALL BE IMMEDIATELY REMOVED FROM THE SITE AND BE DISPOSED OF PROPERLY. DESIGNATED AREAS SHALL BE DETERMINED BY THE AREA ENGINEER FOR SPOILS DISPOSAL AND MATERIAL STORAGE. THESE AREAS SHALL BE PROTECTED FROM RUN-ON AND RUN-OFF. MATERIALS RESULTING FROM THE DESTRUCTION OF EXISTING ROADS AND BEING REMOVED AND/OR DISPOSED OF BY THE CONTRACTOR WILL BE DONE SO IN ACCORDANCE WITH ALL FEDERAL, STATE, AND LOCAL LAWS, ORDINANCES AND REGULATIONS AND WITH THE APPROVAL OF THE PROJECT ENGINEER. ANY CHANGES TO AMBIENT WATER QUALITY DURING CONSTRUCTION OF THE PROPOSED PROJECT SHALL BE PROHIBITED AND MAY RESULT IN ADDITIONAL WATER QUALITY CONTROL MEASURES, WHICH SHALL BE MITIGATED AS SOON AS POSSIBLE AND SHALL BE REPORTED TO THE TEXAS COMMISSION ON ENVIRONMENTAL QUALITY (TCEQ) WITHIN 24 HOURS OF BECOMING AWARE OF IMPACTS.

### SANITARY WASTE:

ALL SANITARY WASTE WILL BE COLLECTED FROM THE PORTABLE UNITS AS NECESSARY OR AS REQUIRED BY LOCAL REGULATION BY A LICENSED SANITARY WASTE MANAGEMENT CONTRACTOR.

### REMARKS:

CONSTRUCTION STAGING AREAS AND VEHICLE MAINTENANCE AREAS SHALL BE CONSTRUCTED BY THE CONTRACTOR IN A MANNER TO MINIMIZE THE RUNOFF OF POLLUTANTS. ALL WATERWAYS SHALL BE CLEARED AS SOON AS PRACTICABLE OF TEMPORARY EMBANKMENT. TEMPORARY BRIDGES, MATTING, FALSEWORK PILING, DEBRIS OR OTHER OBSTRUCTIONS PLACED DURING CONSTRUCTION OPERATIONS THAT ARE NOT PART OF THE FINISHED WORK. DISPOSAL AREAS, STOCKPILES, AND HAUL ROADS SHALL BE CONSTRUCTED IN A MANNER THAT WILL MINIMIZE AND CONTROL THE AMOUNT OF SEDIMENT THAT MAY ENTER RECEIVING WATERS. DISPOSAL AREAS SHALL NOT BE LOCATED IN ANY WETLAND. WATER BODY OR STREAMBED.



TXDOT STORM WATER POLLUTION PREVENTION PLAN (SW3P)

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	FHWA DIVISION	PF	PROJECT NO.						,	
ı	6	SEE	SEE TITLE SHEET					380		
	STATE		COUNTY					SHEET NO.		
	TEXAS		STONE	EW/	ALL					
	DISTRICT	CONTROL	SECTIO	NC	JO	В		171		
	ABL	0106	04		03	6				

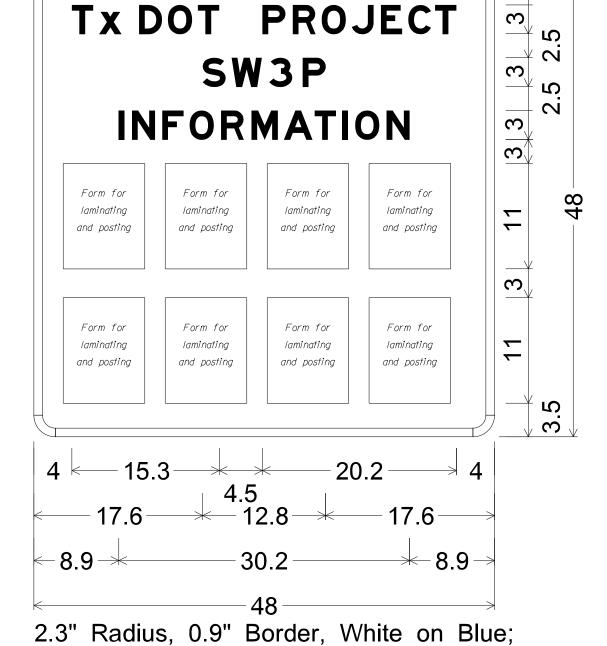
LIST OF POTENTIAL POLLUTANTS								
POTENTIAL POLLUTANT	RELATED SOURCE	CONTROLS						
CEMENTATEOUS MATERIAL AND CEMENTATEOUS AGGREGATES (BROKEN CONCRETE)	REMOVAL OF CONCRETE RIPRAP, CULVERT COMPONENTS, BRIDGE COMPONENTS, ETC.	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
MILLED ASPHALTIC CEMENT PAVEMENT (MILLINGS)	OBLITERATION OF ABANDONED ROAD AND PLANING OF ASPHALT	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
VIRGIN ASPHALTIC MATERIAL INCLUSIVE OF PRIME OILS, PRECOAT AGGREGATES, AND HOT MIX BITUMINOUS MIXTURES	APPLICATIONS OF PRIME COATS, SEAL COAT, AND PAVING OPERATIONS	THIS MATERIAL SHALL BE APPLIED AT APPROPRIATE RATES FOR CONSTRUCTION PURPOSES WHICH WILL PRECLUDE THESE MATERIALS FROM ENTERING RUNOFF. IN THE EVENT OF ANY UNINTENDED DISCHARGE, CONTROLS TO CONTAIN RUNOFF WILL BE IMMEDIATELY PLACED AND TORON WILL BE IMMEDIATELY NOTIFIED.						
CONCRETE, REBAR, WIRE, WIRE FABRIC LUMBER, NAILS, STYROFOAM BLOCK, FIBERBOARD, CURING COMPOUND AND LINSEED OIL	CONSTRUCTION OF CONCRETE BRIDGE COMPONENTS SUCH AS DRILLED SHAFTS, CULVERTS, ABUTMENTS, BENTS, REINFORCED CONCRETE SLABS, RAIL, INLET, CONCRETE TRAFFIC BARRIERS, CURB AND GUTTER, RIPRAP AND SIGN FOUNDATIONS	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF. ANY TEMPORARY FILLS MUST BE REMOVED IN THEIR ENTIRETY AND THE AFFECTED AREAS RETURNED TO THEIR PREEXISTING CONDITION/ELEVATION.						
MASONRY CONCRETE BLOCK, GEOGRID FABRIC, CARDBOARD, AND PLASTIC RAP	CONSTRUCTION OF MODULAR RETAINING WALL SYSTEMS	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
WOOD POSTS, STEEL POSTS, BARRELS, CONES, SIGN BOARDS (ALUMINUM AND PLYBOARD), FASTENERS, NUTS, BOLTS, AND WASHERS	PLACEMENT AND/OR REMOVAL OF BARRICADES, SIGNS AND TRAFFIC CONTROL DEVICES	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
WOOD POST, STEEL POST, STEEL FASTENERS, NUTS, BOLTS, AND WASHERS	CONSTRUCTION OF METAL BEAM GUARD FENCE	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
STRUCTURAL STEEL I-BEAM, SIGN BOARDS, AND CONCRETE FOUNDATIONS	REMOVAL OF ROADSIDE SIGN ASSEMBLIES LARGE AND SMALL	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
THERMOPLASTIC PAINT, GLASS BEADS, REFLECTIVE TABS, AND RAISED REFLECTIVE PAVEMENT MARKERS	APPLICATION OF PAVEMENT MARKINGS/MARKERS	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
PETROLEUM PRODUCTS (SMALL QUANTITIES INTRODUCED BY CONTRACTOR)	EQUIPMENT FAILURE, MAINTENANCE AND REPAIR	ALL EQUIPMENT AND VEHICLE MAINTENANCE SHALL BE PERFORMED IN A DESIGNATED AREA WITH APPROPRIATE MEASURES FOR CONTAINMENT AND PROPER DISPOSAL OF ALL WASTE MATERIALS INCLUDING HYDRAULIC OIL AND OTHER LIQUIDS IN ACCORDANCE STATE AND LOCAL WASTE MANAGEMENT REGULATIONS. ALL MATERIAL STORED PRIOR TO DISPOSAL SHALL BE CONTAINED IN A CONTAINER WITH A SECURE COVER MEETING ALL STATE AND LOCAL WASTE MANAGEMENT REGULATIONS.						
ELIGIBLE NON-STORM WATER DISCHARGES INCLUDING BUT NOT LIMITED TO NON-POTABLE WATER AND NON-STORM WATER DISCHARGE	MOISTURE APPLICATIONS FOR DUST CONTROL, DENSITY, VEGETATION WATERING, NON-DETERGENT VEHICLE WASHING, AND AIR CONDITIONING CONDENSATE	THIS MATERIAL SHALL BE APPLIED AT APPROPRIATE RATES FOR CONSTRUCTION PURPOSES WHICH WILL PRECLUDE THESE MATERIALS FROM ENTERING RUNOFF. IN THE EVENT OF ANY UNINTENDED DISCHARGE, CONTROLS TO CONTAIN RUNOFF WILL BE IMMEDIATELY PLACED AND THE NON-POTABLE WATER WILL BE RECOVERED AND PROPERLY STORED FOR REUSE.						
SURVEY STAKE, FLAGGING TAPE AND PAINT	SURVEY STAKING, ALIGNMENT ESTABLISHMENT	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
WASTEWATER	WASHOUT AND CLEANOUT OF STUCCO, PAINT, FORM RELEASE OILS, CURING COMPOUNDS AND OTHER CONSTRUCTION MATERIALS	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
SOAPS AND SOLVENTS	VEHICLE AND EQUIPMENT WASHING	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						
UNSUITABLE FILL MATERIAL	EXCAVATION - ROADWAY, SPECIAL AND EROSION CONTROL	THIS CONSTRUCTION WASTE SHALL BE PROPERLY DISPOSED OF IN ACCORDANCE WITH STATE AND LOCAL REGULATIONS. WHEN STORED ON SITE PRIOR TO DISPOSAL, IT SHALL BE CONTAINED SO AS TO ENSURE THAT IT CANNOT ENTER SURFACE RUNOFF.						



# TxDOT STORM WATER POLLUTION PREVENTION PLAN (SW3P)

© 2022 ® Texas Department of Transportation NO SCALE SHEET 2 OF 2 FHWA DIVISION PROJECT NO. HIGHWAY NO. SEE TITLE SHEET US 380 6 SHEET NO. STATE COUNTY TEXAS STONEWALL 172 DISTRICT CONTROL SECTION JOB 036 ABL 0106 04

REV. DATE: 02/27/2014



[TxDOT PROJECT] E Mod;

[INFORMATION] E Mod;

[SW3P] E Mod;

3

The Forms needed for laminating and posting to the SW3P Notification Board will be provided by the Engineer. The total number of forms may vary. Notification Boards are to be constructed from Plywood,  $\frac{1}{2}$  or  $\frac{5}{8}$ -inch thick, in accordance with TxDOT Departmental Material Specification (DMS)-7100. The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The sign will be placed at a location within the right-of-way but outside the clear zone as directed by the Engineer. This work will not be paid for directly, but will be considered subsidiary to other items.



### SW3P NOTIFICATION BOARD DETAIL



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NO SCAL	HEET	1	OF	1				
FHWA DIVISION	PF	ROJECT NO	нІ	GHWA	Y NO.			
6	SEE TITLE SHEET (				JS 3	80		
STATE	COUNTY					SHEET NO.		
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I. STORM WATER POLLUTION PREVENTION-CLEAN WATER ACT SECTION 402 III. CULTURAL RESOURCES VI. HAZARDOUS MATERIALS OR CONTAMINATION ISSUES General (applies to all projects): TPDES TXR 150000: Storm water Discharge Permit or Construction General Permit Refer to TxDOT Standard Specifications in the event historical issues or required for projects with 1 or more acres disturbed soil. Projects with any Comply with the Hazard Communication Act (the Act) for personnel who will be working with archeological artifacts are found during construction. Upon discovery of disturbed soil must protect for erosion and sedimentation in accordance with hazardous materials by conducting safety meetings prior to beginning construction and archeological artifacts (bones, burnt rock, flint, pottery, etc.) cease making workers aware of potential hazards in the workplace. Ensure that all workers are work in the immediate area and contact the Engineer immediately. provided with personal protective equipment appropriate for any hazardous materials used. List MS4 Operator(s) that may receive discharges from this project. Obtain and keep on-site Material Safety Data Sheets (MSDS) for all hazardous products They may need to be notified prior to construction activities. Required Action No Action Required used on the project, which may include, but are not limited to the following categories: Paints, acids, solvents, asphalt products, chemical additives, fuels and concrete curing Action No. compounds or additives. Provide protected storage, off bare ground and covered, for No Action Required Required Action products which may be hazardous. Maintain product labelling as required by the Act. 1. Maintain an adequate supply of on-site spill response materials, as indicated in the MSDS In the event of a spill, take actions to mitigate the spill as indicated in the MSDS, 1. The project disturbs more than one acre but less than five acres of surface in accordance with safe work practices, and contact the District Spill Coordinator area. The contractor is responsible for the PSL as defined in the <u>Standard</u> immediately. The Contractor shall be responsible for the proper containment and cleanup Specifications for Construction and Maintenance of Highways, Streets, and of all product spills. Bridges (2014 Edition, Section 7.6., Page 44). The total disturbed acreage is the combined acreage to be disturbed on the project and the contractors PSL. Contact the Engineer if any of the following are detected: \* Dead or distressed vegetation (not identified as normal) 2. Prevent storm water pollution by controlling erosion and sedimentation in Trash piles, drums, canister, barrels, etc. accordance with TPDES Permit TXR 150000 \* Undesirable smells or odors \* Evidence of leaching or seepage of substances IV. VEGETATION RESOURCES 3. Comply with the SW3P and revise when necessary to control pollution or Does the project involve any bridge class structure rehabilitation or Preserve native vegetation to the extent practical. required by the Engineer. replacements (bridge class structures not including box culverts)? Contractor must adhere to Construction Specification Requirements Specs ☐ No Yes 4. Post Construction Site Notice (CSN) with SW3P information on or near 162, 164, 192, 193, 506, 730, 751, 752 in order to comply with the site, accessible to the public and TCEQ, EPA or other inspectors. requirements for invasive species, beneficial landscaping, and tree/brush If "No", then no further action is required. removal commitments. If "Yes", then TxDOT is responsible for completing asbestos assessment/inspection. 5. When Contractor project specific locations (PSL's) increase disturbed soil Are the results of the asbestos inspection positive (is asbestos present)? area to 5 acres or more, submit NOI to TCEQ and the Engineer. ☐ No Action Required Required Action II. WORK IN OR NEAR STREAMS, WATER BODIES AND WETLANDS CLEAN WATER Action No. If "Yes", then TxDOT must retain a DSHS licensed asbestos consultant to assist with ACT SECTIONS 401 AND 404 the notification, develop abatement/mitigation procedures, and perform management Minimize impacts to vegetation by removing/altering only what is necessary to construct the project. activities as necessary. The notification form to DSHS must be postmarked at least USACE Permit required for filling, dredging, excavating or other work in any 15 working days prior to scheduled demolition. water bodies, rivers, creeks, streams, wetlands or wet areas. 2. Re-seeding per District standard seed mix. If "No", then TxDOT is still required to notify DSHS 15 working days prior to any The Contractor must adhere to all of the terms and conditions associated with scheduled demolition. the following permit(s): In either case, the Contractor is responsible for providing the date(s) for abatement activities and/or demolition with careful coordination between the Engineer and No Permit Required asbestos consultant in order to minimize construction delays and subsequent claims. Nationwide Permit 14 - PCN not Required (less than 1/10th acre waters or Any other evidence indicating possible hazardous materials or contamination discovered wetlands affected) on site. Hazardous Materials or Contamination Issues Specific to this Project: V. FEDERAL LISTED, PROPOSED THREATENED, ENDANGERED SPECIES, Nationwide Permit 14 - PCN Required (1/10 to <1/2 acre, 1/3 in tidal waters) Required Action CRITICAL HABITAT, STATE LISTED SPECIES, CANDIDATE SPECIES ☐ No Action Required ☐ Individual 404 Permit Required AND MIGRATORY BIRDS. Action No. Other Nationwide Permit Required: NWP# If any of the listed species are observed, cease work in the immediate area, do not disturb species or habitat and contact the Engineer Required Actions: List waters of the US permit applies to, location in project and check Best Management Practices planned to control erosion, sedimentation immediately. The work may not remove active nests from bridges and other structures during nesting season of the birds associated with the nests. and post-project TSS. If caves or sinkholes are discovered, cease work in the immediate area, and contact the Engineer immediately. VII. OTHER ENVIRONMENTAL ISSUES (includes regional issues such as Edwards Aquifer District, etc.) ☐ No Action Required Required Action Required Action ☐ No Action Required The elevation of the ordinary high water marks of any areas requiring work Action No. to be performed in the waters of the US requiring the use of a nationwide Action No. 1. Migratory birds are present at the project site, including on US 380 permit can be found on the Bridge Layouts. 2. State and Federally protected aquatic species are present in the water ENVIRONMENTAL PERMITS. Best Management Practices: at the project site. Limitations to project activities are prescribed by the Biological Opinion and Texas Parks and Wildlife coordination. ISSUES AND COMMITMENTS Erosion Sedimentation Post-Construction TSS See General Notes for limitations. 3. Designated Critical Habitat is present in the project area. Work in **EPIC** Temporary Vegetation Silt Fence Vegetative Filter Strips Critical Habitat will be minimized. 4. State listed protected terrestrial species may occur in the project Rock Berm Retention/Irrigation Systems Blankets/Matting area. Best Management Practices for protected species will be ☐ Triangular Filter Dike Sedimentation Basin Mulch implemented. Texas Department of Transportation 5. Refer to General Notes for details. Sand Bag Berm Constructed Wetlands Sodding LIST OF ABBREVIATIONS ☐ Wet Basin ☐ Interceptor Swale Straw & Hay Bale Dike NO SCALE SHEET 1 OF Erosion Control Compost & Mulch BMP: Best Management Practice CGP: Construction General Permit Spill Prevention Control and Countermeasure ☐ Brush Berms Diversion Dike Storm Water Pollution Prevention Plan PROJECT NO. HIGHWAY NO. JENELLE N. ROMERO 4/29/2022 ☐ Erosion Control Compost Erosion Control Compost Compost Filter Berm and Socks DSHS: Texas Department of State Health Services PCN: Pre-Construction Notification FHWA: Federal Highway Administration Project Specific Location 6 SEE TITLE SHEET US 380 Compost Filter Berm and Socks Compost Filter Berm and Socks Sand Filter Systems MOA: Memorandum of Agreement TCFQ: Texas Carmission on Environmental Quality STATE SHEET NO COUNTY TPDES: Texas Pollutant Discharge Elimination System MOU: Memorandum of Understanding ☑ Temporary Erosion Control Logs☑ Temporary Erosion Control Logs☑ Temporary Erosion Control Logs Texas Parks and Wildlife Department Municipal Separate Storm water Sewer SystemTPWD: TEXAS STONEWALL (BIOLOGS) (BIOLOGS) (BIOLOGS) MBTA: Migratory Bird Treaty Act TxDOT: Texas Department of Transportation Preservation of Natural Sediment Traps Permanent Vegetation NOT: Notice of Termination Threatened and Endangered Species DISTRICT CONTROL JOB 174 SECTION Resources (Planting, Sodding, or Seeding) NWP: Nationwide Permit USACE: U.S. Army Corps of Engineers Sediment Basins

USFWS: U.S. Fish and Wildlife Service

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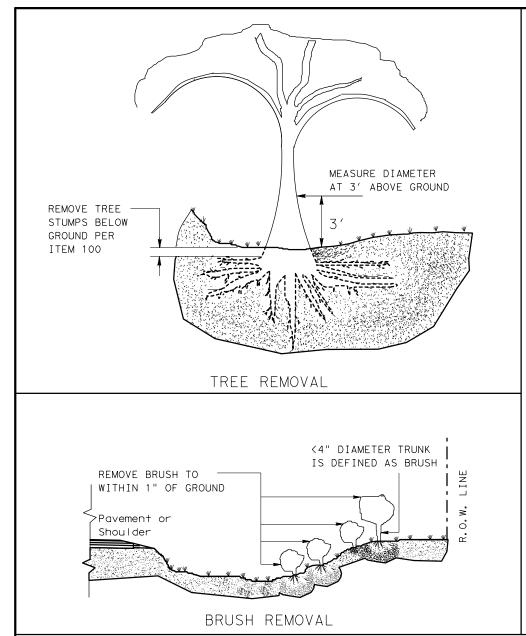
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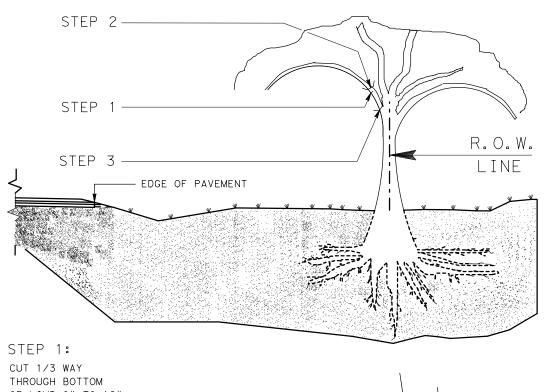
REV. DATE: 02/2015

Construction Exits

☐ Grassy Swales

NOI: Notice of Intent





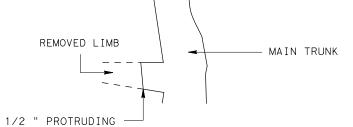
CUT 1/3 WAY
THROUGH BOTTOM
OF LIMB 8" TO 12"
ABOVE MAIN STEM
(OR TRUNK).

### STEP 2:

REMOVE LIMB 4" TO 6" BEYOND THE FIRST CUT

### STEP 3:

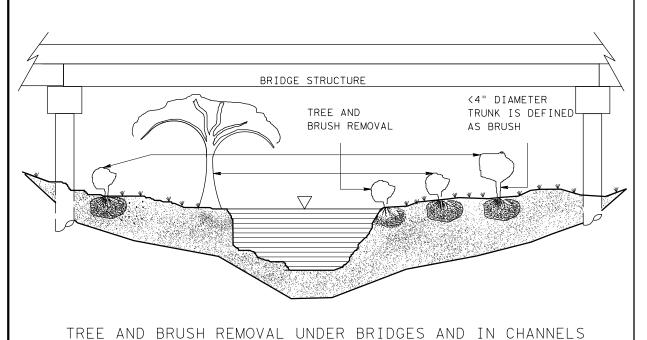
REMOVE STUB WITH A
SMOOTH CUT SO THAT
TRACE COLLAR OF THE
REMOVED LIMB PROTRUDES
APPROXIMATELY 1/2 "
FROM THE MAIN STEM



NOTE: SUCKERS ARE SMALL BRANCHES, LESS THAN 2" IN DIAMETER, THAT OCCUR BENEATH MAIN BRANCHES. REMOVE SUCKERS TO THE HEIGHT OF THE LOWEST MAIN BRANCH. STEPS 1,2 AND 3 APPLY TO MAIN BRANCHES (2" IN DIAMETER OR LARGER).

SUCKERS

TREE TRIMMING ON THE RIGHT OF WAY LINE



### GENERAL NOTES:

TREE AND BRUSH REMOVAL AND TREE TRIMMING

COLLAR

- 1. FOR TREES ON THE R.O.W. LINE, TRIM AND REMOVE ALL LIMBS ON THE PAVEMENT SIDE OF THE TRUNK 18' ABOVE THE PAVEMENT UNLESS OTHERWISE SHOWN ON THE PLANS.
- 2. TREES WITH TRUNKS FULLY CONTAINED WITHIN THE R.O.W. SHALL BE REMOVED UNLESS OTHERWISE SHOWN IN THE PLANS.
- 3. THE DIAMETER SHALL BE DETERMINED BY MEASUREMENT OF THE TRUNK CIRCUMFERENCE 3'
  ABOVE THE GROUND. TREES WITH TRUNKS OF LESS THAN 4" DIAMETER ARE CONSIDERED TO
  BE BRUSH. TREES WITH MULTIPLE TRUNKS AT THE POINT OF MEASUREMENT ARE MEASURED
  SEPARATELY.
- 4. PREP ROW (ITEM 100) BY THE STATION INCLUDES ALL TREE AND BRUSH REMOVAL AND TREE TRIMMING IN THE RIGHT OF WAY ON BOTH SIDES OF THE HIGHWAY. FOR DIVIDED HIGHWAYS THE MEDIAN IS INCLUDED. FOR HIGHWAYS WITH FRONTAGE ROADS, THE AREAS BETWEEN THE FRONTAGE ROADS AND MAIN LANES, AND THE AREAS OUTSIDE OF THE FRONTAGE ROADS ARE INCLUDED.
- 5. TREE AND BRUSH REMOVAL AND TRIMMING UNDER BRIDGES, IN AND ALONG CHANNELS, AND WITHIN TXDOT EASEMENTS SPECIFIED IN THE PLANS SHALL BE INCLUDED FOR PAYMENT UNDER ITEM 100 BY THE STATION. PRIOR TO PERFORMING THIS WORK, THE ENGINEER'S APPROVAL SHALL BE REQUIRED FOR METHOD AND EXTENT OF THIS REMOVAL. SOME LOCATIONS, SUCH AS SPECIAL AQUATIC SITES, SPRINGS, WETLANDS, AND OTHER LOCATIONS SCPECIFIED ELSEWHERE IN THE PLANS, MAY BE EXCLUDED.



# ABILENE DISTRICT TREE AND BRUSH REMOVAL



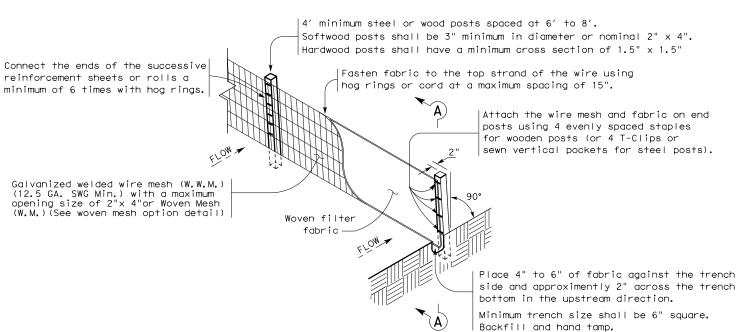
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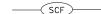
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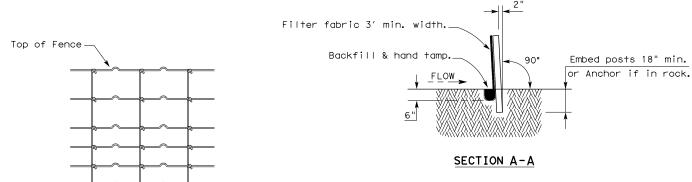
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### TEMPORARY SEDIMENT CONTROL FENCE





### HINGE JOINT KNOT WOVEN MESH (OPTION) DETAIL

Galvanized hinge joint knot woven mesh (12.5 GA.SWG Min.) requires a minimum of five horizontal wires spaced at a maximum of 12 inches apart and all vertical wires spaced at a maximum of 12 inches apart.

### SEDIMENT CONTROL FENCE USAGE GUIDELINES

A sediment control fence may be constructed near the downstream perimeter of a disturbed area along a contour to intercept sediment from overland runoff. A 2 year storm frequency may be used to calculate the flow rate to be filtered.

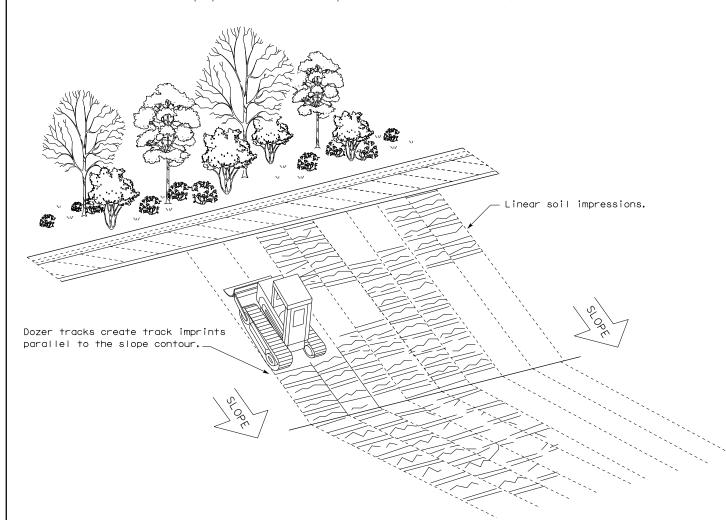
Sediment control fence should be sized to filter a maximum flow through rate of 100 GPM/FT<sup>2</sup>. Sediment control fence is not recommended to control erosion from a drainage area larger than 2 acres.

### LEGEND

Sediment Control Fence -(SCF)-

### **GENERAL NOTES**

- 1. Vertical tracking is required on projects where soil distributing activities have occurred unless otherwise approved.
- 2. Perform vertical tracking on slopes to temporarily stabilize soil.
- 3. Provide equipment with a track undercarriage capable of producing linear soil impressions measuring a minimum of 12" in length by 2" to 4" in width by 1/2" to 2" in depth.
- 4. Do not exceed 12" between track impressions.
- 5. Install continous linear track impressions where the minimum 12" length impressions are perpendicular to the slope or direction of water flow.



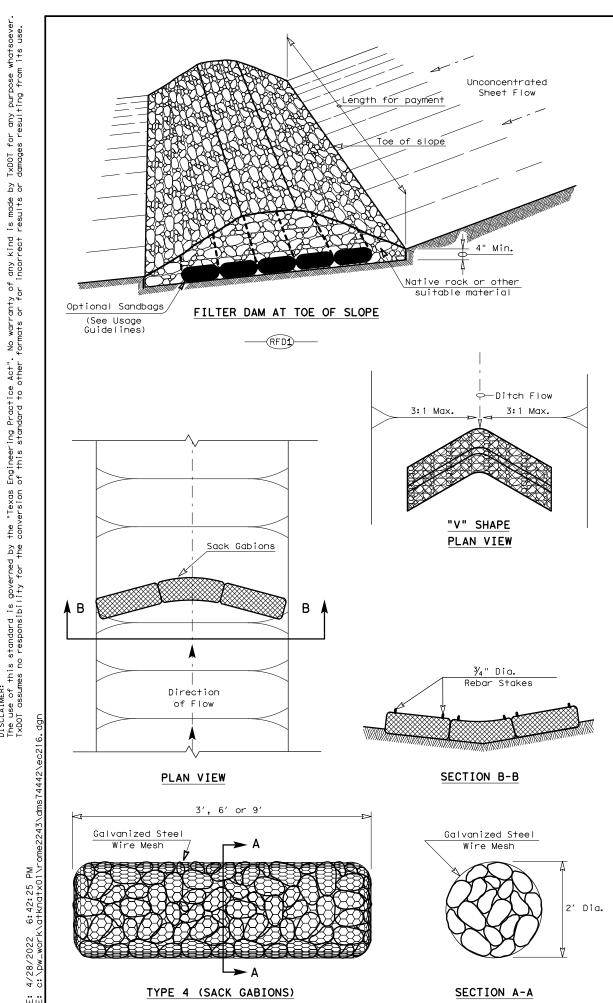
**VERTICAL TRACKING** 



TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES FENCE & VERTICAL TRACKING

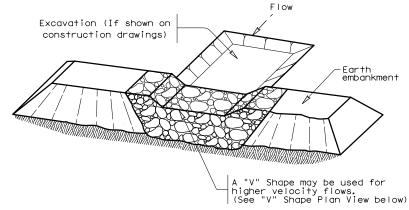
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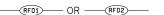


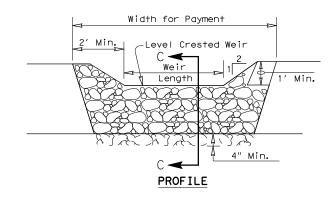
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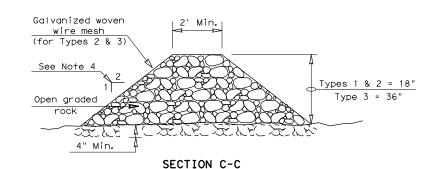
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### FILTER DAM AT SEDIMENT TRAP







### ROCK FILTER DAM USAGE GUIDELINES

Rock Filter Dams should be constructed downstream from disturbed areas to intercept sediment from overland runoff and/or concentrated flow. The dams should be sized to filter a maximum flow through rate of 60  ${\sf GPM/FT^2}$  of cross sectional area. A 2 year storm frequency may be used to calculate the flow rate.

Type 1 (18" high with no wire mesh) (3" to 6" aggregate): Type 1 may be used at the toe of slopes, around inlets, in small ditches, and at dike or swale outlets. This type of dam is recommended to control erosion from a drainage area of 5 acres or less. Type 1 may not be used in concentrated high velocity flows (approximently 8 Ft/Sec or more) in which aggregate wash out may occur. Sandbags may be used at the embedded foundation (4" deep min.) for better filtering efficiency of low flows if called for on the plans or directed by the Engineer.

Type 2 (18" high with wire mesh) (3" to 6" aggregate): Type 2 may be used in ditches and at dike or swale outlets.

Type 3 (36" high with wire mesh) (4" to 8" aggregate): Type 3 may be used in stream flow and should be secured to the stream bed.

Type 4 (Sack gabions) (3" to 6" aggregate): Type 4 May be used in ditches and smaller channels to form an erosion control dam.

Type 5: Provide rock filter dams as shown on plans.

# Galvanized Woven Wire Mesh (for Types 2 & 3) Width for payment SEE NOTE 6

### FILTER DAM AT CHANNEL SECTIONS

### 

### **GENERAL NOTES**

- 1. If shown on the plans or directed by the Engineer, filter dams should be placed near the toe of slopes where erosion is anticipated, upstream and/or downstream at drainage structures, and in roadway ditches and channels to collect sediment.
- 2. Materials (aggregate, wire mesh, sandbags, etc.) shall be as indicated by the specification for "Rock Filter Dams for Erosion and Sedimentation
- 3. The rock filter dam dimensions shall be as indicated on the SW3P plans.
- Side slopes should be 2:1 or flatter. Dams within the safety zone shall have sideslopes of 6:1 or flatter.
- 5. Maintain a minimum of 1' between top of rock filter dam weir and top of embankment for filter dams at sediment traps.
- 6. Filter dams should be embedded a minimum of 4" into existing ground.
- 7. The sediment trap for ponding of sediment laden runoff shall be of the dimensions shown on the plans.
- 8. Rock filter dam types 2 & 3 shall be secured with 20 gauge galvanized woven wire mesh with 1" diameter hexagonal openings. The aggregate shall be placed on the mesh to the height & slopes specified. The mesh shall be folded at the upstream side over the aggregate and tightly secured to itself on the downstream side using wire ties or hog rings. For in stream use, the mesh should be secured or staked to the stream bed prior to aggregate placement.
- 9. Sack Gabions should be staked down with  $\frac{3}{4}$ " dia. rebar stakes, and have a double-twisted hexagonal weave with a nominal mesh opening of 2  $\frac{1}{2}$ " x 3  $\frac{1}{4}$ "
- 10. Flow outlet should be onto a stabilized area (vegetation, rock, etc.).
- 11. The guidelines shown hereon are suggestions only and may be modified by

### PLAN SHEET LEGEND

Type 1 Rock Filter Dam Type 2 Rock Filter Dam Type 3 Rock Filter Dam

Type 4 Rock Filter Dam —

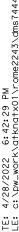


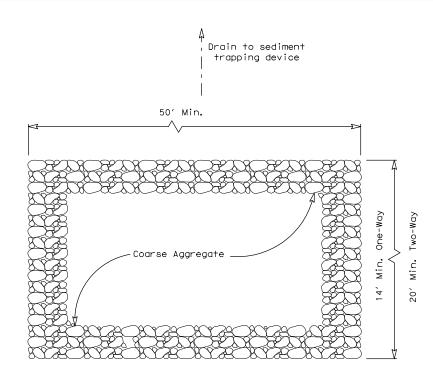
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

ROCK FILTER DAMS

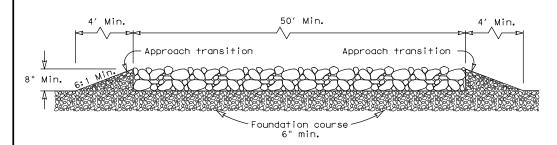
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### PLAN VIEW



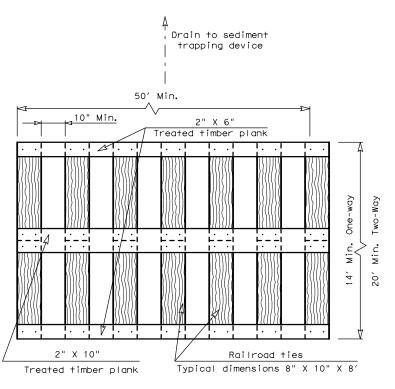
### **ELEVATION VIEW**

### CONSTRUCTION EXIT (TYPE 1)

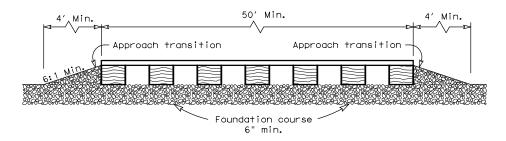
### ROCK CONSTRUCTION (LONG TERM)

### GENERAL NOTES (TYPE 1)

- 1. The length of the type 1 construction exit shall be as indicated on the plans, but not less than  $50^{\prime}$ .
- 2. The coarse aggregate should be open graded with a size of 4" to 8".
- The approach transitions should be no steeper than 6:1 and constructed as directed by the Engineer.
- 4. The construction exit foundation course shall be flexible base, bituminous concrete, portland cement concrete or other materials approved by the Engineer.
- 5. The construction exit shall be graded to allow drainage to a sediment trapping device.
- 6. The guidelines shown hereon are suggestions only and may be modified by the Engineer.
- 7. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.



### PLAN VIEW



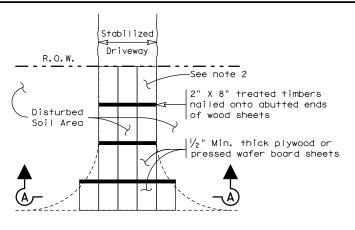
### **ELEVATION VIEW**

### CONSTRUCTION EXIT (TYPE 2)

### TIMBER CONSTRUCTION (LONG TERM)

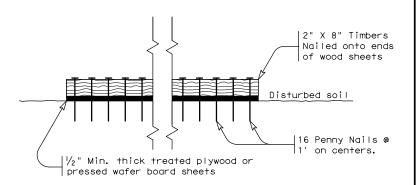
### GENERAL NOTES (TYPE 2)

- The length of the type 2 construction exit shall be as indicated on the plans, but not less than 50'.
- 2. The treated timber planks shall be attached to the railroad ties with  $\frac{1}{2}$  "x 6" min. lag bolts. Other fasteners may be used as approved by the Engineer.
- The treated timber planks shall be #2 grade min., and should be free from large and loose knots.
- 4. The approach transitions shall be no steeper than 6:1 and constructed as directed by the Engineer.
- The construction exit foundation course shall be flexible base, bituminous concrete, portland cement concrete or other material as approved by the Engineer.
- 6. The construction exit should be graded to allow drainage to a sediment trapping device.
- The guidelines shown hereon are suggestions only and may be modified by the Engineer.
- 8. Construct exits with a width of at least 14 ft. for one-way and 20 ft. for two-way traffic for the full width of the exit, or as directed by the engineer.



### Paved Roadway

### PLAN VIEW



# SECTION A-A

# CONSTRUCTION EXIT (TYPE 3) SHORT TERM

### GENERAL NOTES (TYPE 3)

- 1. The length of the type 3 construction exit shall be as shown on the plans, or as directed by the Engineer.
- The type 3 construction exit may be constructed from open graded crushed stone with a size of two to four inches spread a min. of 4" thick to the limits shown on the plans.
- The treated timber planks shall be #2 grade min., and should be free from large and loose knots.
- 4. The guidelines shown hereon are suggestions only and may be modified by the Engineer.



Design Division Standard

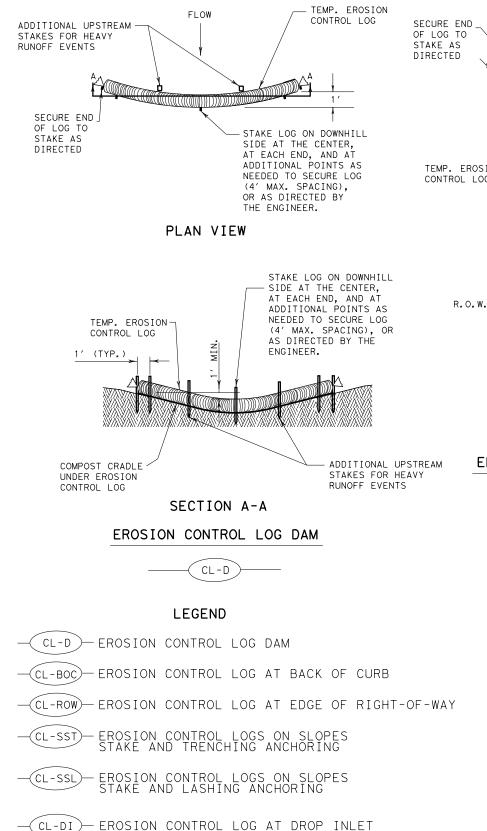
TEMPORARY EROSION,
SEDIMENT AND WATER
POLLUTION CONTROL MEASURES
CONSTRUCTION EXITS
EC (3) -16

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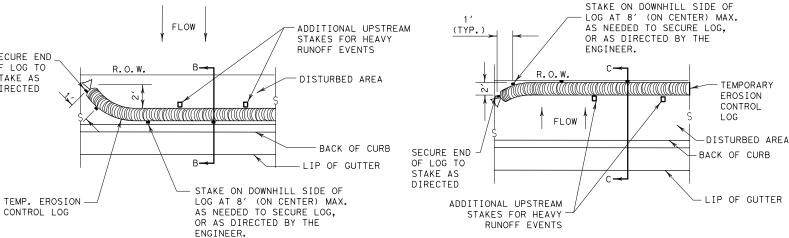
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EROSION CONTROL LOG AT CURB INLET

(cl-gi)— erosion control log at curb & grate inlet



TEMP. EROSION

COMPOST CRADLE

UNDER EROSION

CONTROL LOG

#3 BAR

CONTROL LOG

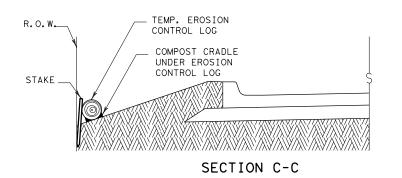
### PLAN VIEW

SECTION B-B

EROSION CONTROL LOG AT BACK OF CURB

CL-BOC

REBAR STAKE DETAIL



PLAN VIEW

# EROSION CONTROL LOG AT EDGE OF RIGHT-OF-WAY



An erosion control log sediment trap may be used to filter sediment out of runoff draining from an unstabilized area.

5 acres. The trap capacity should be 1800 CF/Acre (0.5" over

Control logs should be placed in the following locations:

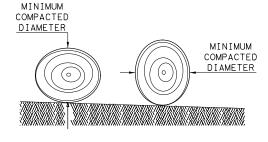
- 5. Just before the drainage leaves the construction limits where drainage flows away from the project.

The logs should be cleaned when the sediment has accumulated to a depth of 1/2 the log diameter.

will not be paid for separately.

### **GENERAL NOTES:**

- 1. EROSION CONTROL LOGS SHALL BE INSTALLED IN ACCORDANCE WITH MANFACTURER'S RECOMMENDATIONS, OR AS DIRECTED BY THE ENGINEER.
- 2. LENGTHS OF EROSION CONTROL LOGS SHALL BE IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS AND AS REQUIRED FOR THE PURPOSE INTENDED.
- 3. UNLESS OTHERWISE DIRECTED, USE BIODEGRADABLE OR PHOTODEGRADABLE CONTAINMENT MESH ONLY WHERE LOG WILL REMAIN IN PLACE AS PART OF A VEGETATIVE SYSTEM. FOR TEMPORARY INSTALLATIONS. USE RECYCLABLE CONTAINMENT MESH.
- FILL LOGS WITH SUFFICIENT FILTER MATERIAL TO ACHIEVE THE MINIMUM COMPACTED DIAMETER SPECIFIED IN THE PLANS WITHOUT EXCESSIVE DEFORMATION.
- STAKES SHALL BE 2" X 2" WOOD OR #3 REBAR, 2'-4' LONG, EMBEDDED SUCH THAT 2" PROTRUDES ABOVE LOG, OR AS DIRECTED BY THE ENGINEER.
- 6. DO NOT PLACE STAKES THROUGH CONTAINMENT MESH.
- 7. COMPOST CRADLE MATERIAL IS INCIDENTAL & WILL NOT BE PAID FOR SEPARATELY.
- SANDBAGS USED AS ANCHORS SHALL BE PLACED ON TOP OF LOGS & SHALL BE OF SUFFICIENT SIZE TO HOLD LOGS IN PLACE.
- TURN THE ENDS OF EACH ROW OF LOGS UPSLOPE TO PREVENT RUNOFF FROM FLOWING AROUND THE
- 10. FOR HEAVY RUNOFF EVENTS, ADDITIONAL UPSTREAM STAKES MAY BE NECESSARY TO KEEP LOG FROM FOLDING IN ON ITSELF.



DIAMETER MEASUREMENTS OF EROSION CONTROL LOGS SPECIFIED IN PLANS

SHEET 1 OF 3



Design Division Standard

TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

**EROSION CONTROL LOG** 

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### SEDIMENT BASIN & TRAP USAGE GUIDELINES

The drainage area for a sediment trap should not exceed Log Traps: the drainage area).

- 1. Within drainage ditches spaced as needed or min. 500' on center
- 2. Immediately preceding ditch inlets or drain inlets
- 3. Just before the drainage enters a water course
- 4. Just before the drainage leaves the right of way

Cleaning and removal of accumulated sediment deposits is incidental and

SECURE END OF LOG TO STAKE AS

DIRECTED

TEMP. EROSION

FLOW

CONTROL LOG



SANDBAG

# EROSION CONTROL LOG AT CURB & GRADE INLET

OVERLAP ENDS TIGHTLY 24" MINIMUM

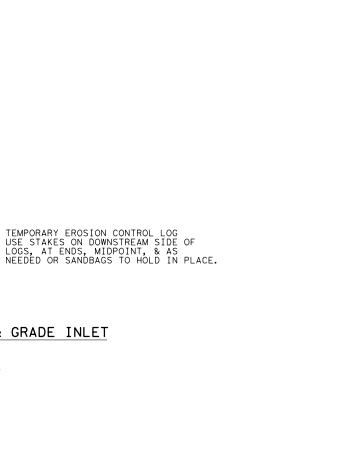
- FLOW

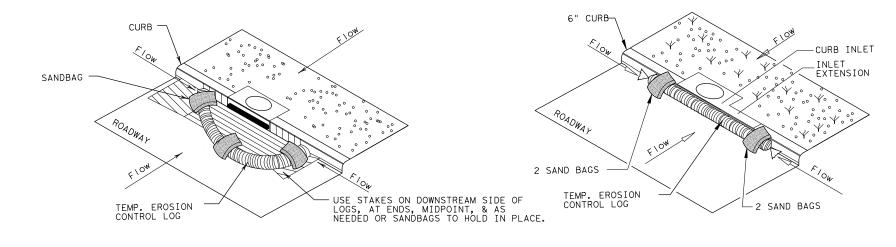
EROSION CONTROL LOG AT DROP INLET

CURB AND GRATE INLET

-STAKE OR USE SANDBAGS ON DOWNHILL SIDE OF LOG AS NEEDED TO HOLD IN PLACE (TYPICAL)

COMPLETELY SURROUND
DRAINAGE ACCESS TO
AREA DRAIN INLETS WITH
EROSION CONTROL LOG

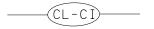




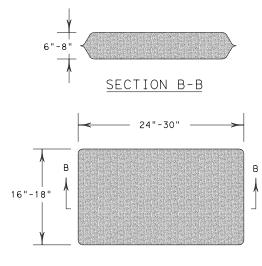
### EROSION CONTROL LOG AT CURB INLET

## EROSION CONTROL LOG AT CURB INLET





NOTE: EROSION CONTROL LOGS USED AT CURB INLETS SHOULD ONLY BE USED IF THEY WILL NOT IMPEDE TRAFFIC OR FLOOD THE ROADWAY OR WHEN THE STORM SEWER SYSTEM IS NOT FULLY FUNCTIONAL.



SANDBAG DETAIL



TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES

SHEET 3 OF 3

**EROSION CONTROL LOG** 

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