CIVIL

G

Armon

GROUP

TREVOR L. CASTILLA 85405 :5

CENSE

L. Castill.

CONSULTING

SEE SHEET 2 FOR INDEX OF SHEETS

TEXAS REGISTERED ENGINEERING FIRM NO. F-9356

02/04/2022

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF

CONTRACTS (FORM FHWA 1273, MAY 2012).

TRANSPORTATION, NOVEMBER 1, 2014 AND SPECIFICATION ITEMS

LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION

1575 HERITAGE DRIVE, STE. 308

P 972.569.9193 F 972.569.9197

MCKINNEY, TEXAS 75069

STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT

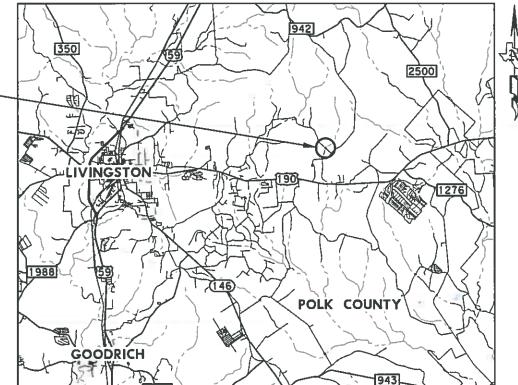
PROJECT NO. BR 2B20(098)

CR POLK COUNTY

CSJ	ROAI	DWAY	BRI	DGE	тот	ALS
	FT	MI	FT MI		FT	MI
0911-04-089	300.00	0.057	75.00	0.014	375.00	0.071

LIMITS: KELLY RD AT BLUFF CREEK

FOR THE CONSTRUCTION OF BRIDGE REPLACEMENT CONSISTING OF REPLACE BRIDGE AND APPROACHES



2665

SCALE:

CSJ: 0911-04-089 STA 12+45.00 LAT: 30.7220681* LONG: -94.7925471* END PROJECT STA 16+20.00 LAT: 30.7227788* LONG: -94.7934109*

BEGIN PROJECT KELLY RD AT BLUFF CREEK

NO RAILROAD CROSSINGS



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NO EXCEPTIONS NO EQUATIONS

FHWA	-	SHEET	NO.			
TEXAS DIVISION	BR	2B20 (09)B)	1		
STATE	DISTRICT	COUNTY				
TEXAS	LFK	POLK				
CONTROL	SECTION	JOB HIGHWAY NO.				
0911	04	089 CR				

FUNCTIONAL CLASS: LOCAL ROAD DESIGN SPEED = MEET OR EXCEED EXISTING CONDITIONS ADT (2015) = 62

FINAL PLANS

LETTING DATE:
DATE CONTRACTOR BEGAN WORK:
DATE WORK WAS COMPLETED:
DATE WORK WAS ACCEPTED:
FINAL CONTRACT COST: \$
CONTRACTOR:

CONSTRUCTION WORK ON THIS PROJECT WAS PERFORMED IN ACCORDANCE WITH PLANS, CONTRACT AND APPROVED CHANGE ORDERS.

DATE

SHEET NO. DESCRIPTION

GENERAL

1	TITLE SHEET
2	INDEX OF SHEETS
3	TYPICAL SECTIONS
4, 4A-4C	GENERAL NOTES
5	ESTIMATE & QUANTITY SHEET
6 - 7	SUMMARY OF QUANTITIES

TRAFFIC CONTROL PLAN

8	TRAFFIC CONTROL PLAN
9 - 20	BC(1)-21 THRU BC(12)-21

ROADWAY DETAILS

- HORIZONTAL & VERTICAL CONTROL INDEX 21
- 22 HORIZONTAL & VERTICAL CONTROL SHEET
- 23 HORIZONTAL ALIGNMENT DATA PLAN & PROFILE
- 24
- 25 GRADING LAYOUT
- GF(31)-19 26

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- GF (31) DAT-19 27
- 28 SGT (11S) 31-18
- 29 SGT (12S) 31-18
 - SGT (15) 31-20 30

DRAINAGE DETAILS

- 31 DRAINAGE AREA MAP
- 32 HYDRAULIC DATA SHEET
- 33 SCOUR PROFILE

BRIDGE DETAILS

- 34 BRIDGE LAYOUT
- 35 BORING LOGS
- ESTIMATED QUANTITIES AND BEARING SEAT ELEVATIONS 36
- 37 39 AIG-24
- 40 A.J
- 41 42 CSAB
- 43 44 FD

#

- 45 46 IGD ± 47 - 49
- IGEB 50 - 51 #
- IGMS 52 - 53 IGSD-24 #
- # 54 IGSK
- 55 IGTS #
- 56 57 MEBR(C) #
- 58 61 PCP #
- PCP-FAB 62
- 63 64 PMDF
- ± 65 - 66 SIG-24
- # 67 - 68 SRR #
- 69 70 TYPE T631LS



THE STANDARD SHEETS SPECIFICALLY IDENTIFIED ABOVE BY A * HAVE BEEN ISSUED BY ME, OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.

02/23/2022

DATE

astilla Arenop L.

TREVOR L. CASTILLA, P.E.

SHEET NO. DESCRIPTION

TRAFFIC DETAILS

71	D	&	OM(1)-20
72	D	&	OM(2)-20
73	D	&	OM(3)-20
74	D	&	OM(5)-20
75	D	&	OM(VIA)-20

ENVIRONMENTAL ISSUES

76	TXDOT SWP3 INDEX
77	SWP3 LAYOUT
78 - 79	EPIC
80	EC(1)-16
81	EC(2)-16

CROSS SECTIONS

82 - 86 CROSS SECTIONS



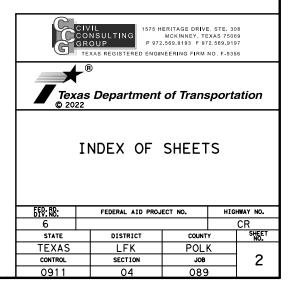
THE STANDARD SHEETS SPECIFICALLY IDENTIFIED ABOVE BY A # HAVE BEEN ISSUED BY ME, OR UNDER MY RESPONSIBLE SUPERVISION AS BEING APPLICABLE TO THIS PROJECT.

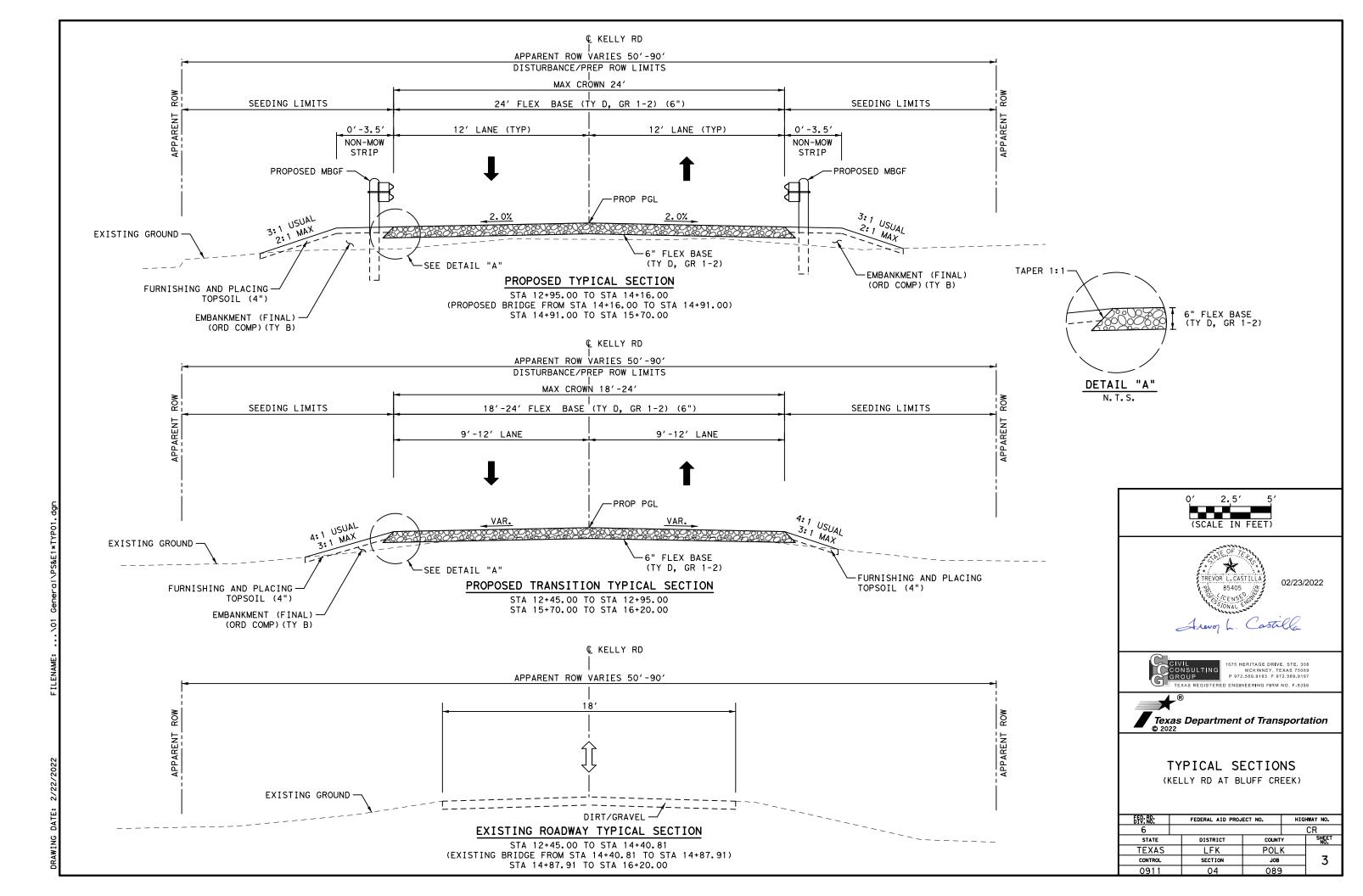
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02/22/2022

DATE

JAMES B. HALL II, P.E.





Highway: CR

Sheet

Control: 0911-04-089

GENERAL NOTES:

Existing regulatory, warning and guide signs within project limits are to remain visible to the traveling public at all times. If a sign must be repositioned during construction operations, move and install the sign to an approved location. Use care when working near existing signs and repair or replace signs damaged by work operations. All work involved repositioning existing signs will be subsidiary to various bid items.

Furnish materials and make repairs to the existing roadway at any location damaged by construction operations. This work shall be done in an approved manner and will be subsidiary to various bid items.

Ensure drainage structures and outfall channels constructed on this project are free of silt and debris at the time of project acceptance. Final clean out work will be subsidiary to various bid items.

Maintain adequate surface drainage throughout the project limits during all phases of construction.

Roadway cross slopes shall conform approximately to the existing surface, unless otherwise directed.

Provide suitable access at all times to adjacent businesses, private property and side roads.

Remove dirt, silt, rocks, debris and other foreign matter that accumulates in structures due to the Contractor's operations as directed. Keep stream channels open at all times. This work will not be paid for directly, but will be subsidiary to pertinent Items.

The bridge at Bluff Creek has a posted weight limit of 7500 lbs. per tandem axle. This weight limit shall not be exceeded during project construction.

Contractor questions on this project are to be addressed to the following individual(s): Shannon Ramos, Area Engineer Shannon.Ramos@txdot.gov Homar.Munoz@txdot.gov Homar Munoz, Asst. Area Engineer

Contractor questions will be accepted through email, phone, and in person by the above individuals.

All contractor questions will be reviewed by the Engineer. Once a response is developed, it will be posted to TxDOT's Public FTP at the following Address:

https://ftp.dot.state.tx.us/pub/txdot-info/Pre-Letting%20Responses/

All questions submitted that generate a response will be posted through this site. The site is organized by District, Project Type (Construction or Maintenance), Letting Date, CCSJ/Project Name.

County: Polk

Highway: CR

The contractor's attention is directed to the EPIC sheet(s) included in this plan set for additional information regarding environmental permits, issues, and commitments.

Project Mowing

Mow the right of way within the project limits a maximum of 3 cycles per year as directed. Mowing will not be measured or paid for directly, but will be subsidiary to various bid items.

The equipment used for mowing shall consist of approved mowing units capable of mowing on slopes without marring finished slope surfaces or injuring existing growth. The minimum cutting width shall not be less than 5 ft., unless otherwise approved.

Mow all areas of existing vegetation and vegetation placed during the project as directed. The mowing height shall be 5 in. unless otherwise directed. Repair portions of sod or grass that are injured during mowing operations as directed.

Mow as close as possible to all fixed objects, exercising extreme care not to damage trees, plants, shrubs, signs, delineators or other appurtenances which are part of the facility. Hand trim around such objects, unless otherwise specified.

Use safety chains or other manufacturer's safety device to prevent damage to people or property caused by flying debris propelled out from under rotary mowers. Chains shall be a minimum size of 5/16 in. and links spaced side by side around the mower's front, sides and rear. When mowing at the specified cutting height, the chains shall be long enough to drag the ground. If at any time, it is determined mowing or trimming equipment is defective to the point that it may affect the quality of work or create an unsafe condition, then that equipment shall be immediately repaired or replaced.

Litter Pickup

Remove litter from the right of way in the limits of this project a maximum of 3 cycles per year as directed. Litter pickup will not be measured or paid for directly, but will be subsidiary to various bid items.

The equipment used for litter pickup shall be approved.

Collect and dispose of all litter deposited by construction operations or the traveling public including cans, bottles, paper, plastic items, metal scraps, lumber, etc. from within the project right of way or as directed. Properly dispose of all collected litter. Do not dump or stockpile collected litter on State property.

Precast Alternate Proposals.

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/inside-txdot/formspublications/consultants-contractors/publications/bridge.html#design. Acceptance or denial of an

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General Notes

County:	Polk
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Highway: CR

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Sheet

alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

Item 5: Control of the Work

In the event utility lines needing unforeseen adjustments are encountered during construction operations, alter operations and continue to prosecute the contract in such a manner that will allow utility adjustments to be made by others. An extension of working time may be granted for any delays caused by the utility adjustments if deemed necessary.

Item 7: Legal Relations and Responsibilities

No significant traffic generator events identified.

The proposed work of this project is to replace the bridge and approaches at Kelly Road over Bluff Creek. The total disturbed area is 0.503 acres. The disturbed area in this project and the contractor project specific locations (PSLs) within 1 mile of the project limits for the contract will further establish the authorization requirements for storm water discharges. As the disturbed area including PSLs is less than 1 acre, the TPDES CGP does not apply; however, the contractor shall place BMPs as directed to adhere to water quality requirements associated with section 404/401 permits. If the total area disturbed shown in the plans and PSLs within 1 mile of the project limits exceed 1 acre, the engineer will develop a SWP3 site plan and post a small construction site notice for the construction activities.

Dispose of all vegetative matter and any other materials removed from State Right of Way in accordance with applicable environmental laws, rules, regulations and requirements.

Burning locations must be approved by the Engineer prior to beginning. Burning activities must be conducted in compliance with Texas Commission on Environmental Quality (TCEQ) regulations. Notify the Engineer when burning activities will take place.

In order to maintain compliance with Chapter 64 of the Texas Parks and Wildlife Code and Migratory Bird Treaty Act (MBTA), construction activities that may affect nests (i.e. tree removal, tree limbing, bridge work) shall be conducted outside of the nesting season (March 15 to September 15). In the event birds or active nests (eggs and/or nestlings present) are encountered, contact the engineer prior to conducting work.

Item 8: Prosecution and Progress

For this project, working days will be computed and charged in accordance with Item 8, Section 3.1.4 "Standard Workweek".

Submit monthly progress schedules no later than the 20th calendar day of the month. Failure to comply with this deadline may result in the Engineer withholding progress (monthly) payments.

A 90 day delay has been included to give contractors flexibility of when to start work due to the time needed for the fabrication of beams.

County: Polk

Highway: CR

Provide a Critical Path Method (CPM) Construction Schedule unless otherwise approved.

Item 100: Preparing Right of Way

The equipment used to trim limbs shall be approved. A boom axe will not be allowed.

Item 110: Excavation Item 132: Embankment

Hauling materials with scrapers across or along existing roadways will not be permitted without written permission.

Drying of material deeper than 6 inches below subgrade elevations will not be permitted without written permission.

Grading required for shaping driveways and side road turnouts for pipe culverts at all access locations, will be subsidiary to various bid items.

All blading, rolling, and scraper work to construct and remove temporary slopes adjacent to pavement drop-offs, will be subsidiary to various bid items.

Compact embankment material used to reshape existing slopes to a density comparable with adjacent undisturbed material to the satisfaction of the Engineer.

Item 162: Sodding for Erosion Control

Provide Bermuda block sod unless St. Augustine is the prevailing grass cover at particular placement locations. Provide St. Augustine block sod at those locations.

Item 166: Fertilizer

Fertilize all seeded or sodded areas.

Item 168: Vegetative Watering

Equip water trucks with sprinkler systems capable of watering all of the entire seeded or sodded areas from the roadway.

Water all newly placed sodded or seeded areas at the time of installation. Thereafter, maintain the sodded or seeded areas in a well-watered condition, at no time allow the areas to dry to a condition where water stress is evident.

Item 169: Soil Retention Blankets

In areas designated for soil retention blankets (SRB) in the plans, furnish only spray-on products listed on the Approved Product List for Erosion Control Products based upon the Class and Type specified in the plans. Any substitution to spray –on products must be approved in writing, be

General Notes

Sheet 4A

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listed on the Approved Product List for Erosion Control Products based upon Class and Type, and shall not contain UV degradable, photodegradable or polypropylene materials.

Item 247: Flexible Base

Provide flexible base with a minimum plasticity index of 2.

Provide flexible base material with a minimum Bar Linear Shrinkage of 2% as determined by Test Method Tex-107-E, Part II.

Stockpiling of base material will not be required if testing has been performed and the material has been approved at the source. Deliver approved specified materials to the project.

Compaction requirements for flexible base are ordinary compaction.

Remove and properly dispose of any piles of asphaltic concrete and all other debris left on the right of way daily.

Item 421: Hydraulic Cement Concrete

The Engineer will provide curing facilities and strength testing equipment for acceptance testing.

Livingston Area Office, 3161 US 59 N., Livingston TX 77351

Item 422: Concrete Superstructures

Saw-cut grooves are not required.

Item 427: Surface Finishes for Concrete

Provide a rub finish for Surface Area I.

Item 432: Riprap

Stone riprap will require the placement of filter fabric prior to placement of stones.

Welded wire fabric will not be allowed for reinforcing concrete riprap. Reinforcing shall consist of No. 3 or 4 bars meeting the requirements of grade 60 reinforcing steel. Place bars on 12 in. centers in each direction, supported on reinforcing chairs.

Item 496: Removing Structures

Lead was detected in paint less than 90 ppm on the Bluff Creek bridge at Kelly Rd. (0911-04-089). Requirements of Item 6 Special Provisions are not required. Contractor may request a copy of the Asbestos and Lead Paint Inspection Report from the Area Engineer. Any contracting personnel who may disturb the steel components should be made aware of the lead content in the materials so that they may use proper OSHA procedures.

County: Polk

Highway: CR

Item 502: Barricades, Signs, and Traffic Handling

Traffic Control Plan (TCP):

Ensure the Contractor's Responsible Person (CRP) or their alternate for Barricades, Signs and Traffic Handling is available at all times and able to receive instructions from the Engineer or authorized Department representative. The CRP shall be a person that is usually at the project site during normal working hours.

For protection of the traveling public, direct traffic through the work area using signs, flaggers and other devices. Required signs are shown in the plans on the Barricade and Construction Standards and Traffic Control Plan Sheets. The latest edition of the "Texas Manual on Uniform Traffic Control Devices" shall also be used as a guide for handling traffic on this project.

Provide adequate flaggers to protect the traveling public when working on or near a roadway carrying traffic. All flaggers shall wear hardhats and reflective vests.

Install "Be Prepared to Stop" (CW3-4) and "Flagger Ahead" (CW20-7aD) signs when flaggers are present. Position the signs where good visibility and traffic control can be maintained.

Provide one high-intensity yellow, rotating dome-light on all equipment such as distributors, spreader boxes, lay-down machines, dump trucks, rollers, backhoes, road graders, loaders, etc. within the work zone. Mount lights high enough to be visible from all directions and operating when the equipment is in the work zone. On all other equipment such as automobiles, trailers, etc. use emergency flashers while within the work zone.

All blading, rolling and scraper work to construct and remove temporary slopes adjacent to pavement drop-offs, will be considered subsidiary to various bid items.

Notify the Engineer prior to placing any materials or equipment on the right of way. Any equipment, stockpiles, or materials placed within 30 ft. of the driving lane must have adequate signs, barricades or other warning devices as approved. As a minimum place an 8 ft. wide TY III Barricade or barrels on the approach side of each site that is within 30 ft. of the driving lane. Use TY III Barricade or barrels for the site similarly on the departure side if the location is within 30 ft. of the opposing traffic lane.

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

Texas Transportation Code 547.105 authorizes the use of warning lights to promote safety and provides an effective means of gaining the travelling public's attention as they drive in areas

Sheet 4B

Control: 0911-04-089

where construction crews are present. In order to influence the public to move over when high risk construction activities are taking place, minimize the utilization of blue warning lights. These lights must be used only while performing work on or near the travel lanes or shoulder where the travelling public encounters construction crews that are not protected by a standard work zone set up such as a lane closure, shoulder closure, or one-way traffic control. Refrain from leaving the warning lights engaged while travelling from one work location to another or while parked on the right of way away from the pavement or a work zone.

All workers shall wear reflective clothing meeting ANSI Class II requirements during the day and ANSI Class III requirements during the night.

Item 506: Temporary Erosion, Sedimentation, and Environmental Controls

Locations and types of BMPs may require adjustments prior to or after placement as directed by the Engineer. Adjustments should be made to ensure BMPs are working effectively and maintain compliance with the Construction General Permit. Notify the Engineer prior to making adjustments.

Other erosion or sediment control measures deemed necessary by the Engineer will be paid for in accordance with article 4.4, "Changes in the Work".

Item 540: Metal Beam Guard Fence

Use round timber posts.

Use timber post on all metal beam guard fence installations except where steel posts are required.

At the close of work each day, protect the ends of metal beam guard fence in an approved manner, so that no blunt ends are exposed to approaching traffic. Plastic drums will be required at these locations.

Item 658: Delineator and Object Marker Assemblies

Install delineators on the departure side of the posts when mounting to metal beam guard fence and guardrail end treatments.

Install D-SW delineators on the departure side of steel bridge rail posts.



CONTROLLING PROJECT ID 0911-04-089

DISTRICT Lufkin HIGHWAY CR 1243 COUNTY Polk

Estimate & Quantity Sheet

		CONTROL SECTIO	ON JOB	0911-04-089			
	PROJECT			T ID A00061486			
		COUNT		Polk		TOTAL EST.	TOTAL
	HIGI		HWAY CR 1243			FINAL	
ALT	BID CODE	DESCRIPTION	UNIT	EST.	FINAL		
	100-6002	PREPARING ROW	STA	3.750		3.750	
	110-6001	EXCAVATION (ROADWAY)	CY	190.000		190.000	
	132-6003	EMBANKMENT (FINAL)(ORD COMP)(TY B)	CY	595.000		595.000	
	160-6003	FURNISHING AND PLACING TOPSOIL (4")	SY	502.000		502.000	
	162-6002	BLOCK SODDING	SY	150.000		150.000	
	164-6009	BROADCAST SEED (TEMP) (WARM)	SY	633.000		633.000	
	164-6011	BROADCAST SEED (TEMP) (COOL)	SY	633.000		633.000	
	164-6021	CELL FBR MLCH SEED(PERM)(RURAL)(SANDY)	SY	1,265.000		1,265.000	
	168-6001	VEGETATIVE WATERING	MG	54.000		54.000	
	169-6002	SOIL RETENTION BLANKETS (CL 1) (TY B)	SY	448.000		448.000	
	247-6073	FL BS (CMP IN PLC)(TY D GR 1-2) (6")	SY	775.000		775.000	
	400-6005	CEM STABIL BKFL	CY	81.000		81.000	
	416-6004	DRILL SHAFT (36 IN)	LF	300.000		300.000	
	420-6013	CL C CONC (ABUT)	CY	37.600		37.600	
	422-6001	REINF CONC SLAB	SF	1,950.000		1,950.000	
	422-6023	SHEAR KEY	CY	0.400		0.400	
	425-6035	PRESTR CONC GIRDER (TX28)	LF	298.000		298.000	
	432-6033	RIPRAP (STONE PROTECTION)(18 IN)	CY	304.000		304.000	
	450-6019	RAIL (TY T631LS)	LF	198.000		198.000	
	454-6003	ARMOR JOINT	LF	44.000		44.000	
	496-6009	REMOV STR (BRIDGE 0 - 99 FT LENGTH)	EA	1.000		1.000	
	500-6001	MOBILIZATION	LS	1.000		1.000	
	502-6001	BARRICADES, SIGNS AND TRAFFIC HANDLING	МО	6.000		6.000	
	506-6002	ROCK FILTER DAMS (INSTALL) (TY 2)	LF	50.000		50.000	
	506-6011	ROCK FILTER DAMS (REMOVE)	LF	50.000		50.000	
	506-6034	CONSTRUCTION PERIMETER FENCE	LF	117.000		117.000	
	506-6038	TEMP SEDMT CONT FENCE (INSTALL)	LF	823.000		823.000	
	506-6039	TEMP SEDMT CONT FENCE (REMOVE)	LF	823.000		823.000	
	540-6001	MTL W-BEAM GD FEN (TIM POST)	LF	27.000		27.000	
	540-6016	DOWNSTREAM ANCHOR TERMINAL SECTION	EA	2.000		2.000	
	544-6001	GUARDRAIL END TREATMENT (INSTALL)	EA	2.000		2.000	
	644-6076	REMOVE SM RD SN SUP&AM	EA	2.000		2.000	
	658-6016	INSTL DEL ASSM (D-SW)SZ (BRF)GF1 (BI)	EA	6.000		6.000	
	658-6053	INSTL OM ASSM (OM-3L)(TWT)GND	EA	2.000		2.000	
	18	EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART)	LS	1.000		1.000	
		SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING)	LS	1.000		1.000	

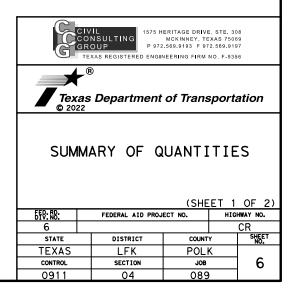


DISTRICT	COUNTY	CCSJ	SHEET
Lufkin	Polk	0911-04-089	5

		ROAD	WAY QUANTIT	IES			
	100	110	132	247	540	540	544
	6002	6001	6003	6073	6001	6016	6001
ITEM DESCRIPTION	PREPARING ROW	EXCAVATION (ROADWAY)	EMBANKMENT (FINAL) (ORD COMP) (TY B)	FL BS (CMP IN PLC) (TY D GR 1-2) (6")	MTL W-BEAM GD FEN (TIM POST)	DOWNSTREAM ANCHOR TERMINAL SECTION	GUARDRAIL END TREATMENT (INSTALL)
	STA	СҮ	CY	SY	LF	EA	EA
KELLY RD AT BLUFF CREEK							
STA 12+45.00 TO STA 14+16.00	1.71	7	330	447	13.5	1	1
STA 14+16.00 TO STA 14+91.00	0.75	176	182				
STA 14+91.00 TO STA 16+20.00	1.29	7	83	328	13.5	1	1
PROJECT TOTAL	3.75	190	595	775	27.0	2	2

REMOVAL QUANTITIES						
	496	644				
	6009	6076				
ITEM DESCRIPTION	REMOV STR (BRIDGE O - 99 FT LENGTH)	REMOVE SM RD SN SUP&AM				
	EA	EA				
KELLY RD AT BLUFF CREEK	1	2				
PROJECT TOTAL	1	2				

DELINEATOR AND OBJEC	T MARKER QU	ANTITIES	
	658	658	
	6016	6053	
ITEM DESCRIPTION	INSTL DEL ASSM (D-SW) SZ(BRF) GF1(BI)	INSTL OM ASSM (OM-3L) (TWT)GND	
	EA	EA	
KELLY RD AT BLUFF CREEK			
STA 12+45.00 TO STA 14+16.00	2	1	
STA 14+16.00 TO STA 14+91.00	2		
STA 14+91.00 TO STA 16+20.00	2	1	
PROJECT TOTAL	6	2	



	SWP3 QUANTITIES											
	160	162	164	164	164	168	169	506	506	506	506	506
	6003	6002	6009	6011	6021	6001	6002	6002	6011	6034	6038	6039
ITEM DESCRIPTION	FURNISHING AND PLACING TOPSOIL (4")	BLOCK SODDING *	BROADCAST SEED (TEMP) (WARM)	BROADCAST SEED (TEMP) (COOL)	CELL FBR MLCH SEED (PERM) (RURAL) (SANDY)	VEGETATIVE WATERING (10 GAL/SY)× (2 APPS)	SOIL RETENTION BLANKETS (CL 1) (TY B)	ROCK FILTER DAMS (INSTALL) (TY 2)	ROCK FILTER DAMS (REMOVE)	CONSTRUCTION PERIMETER FENCE	TEMP SEDMT CONT FENCE (INSTALL)	TEMP SEDMT CONT FENCE (REMOVE)
	SY	SY	SY	SY	SY	MG	SY	LF	LF	LF	LF	LF
KELLY RD AT BLUFF CREEK	502	150	633	633	1,265	54	448	50	50	117	823	823
PROJECT TOTAL	502	150	633	633	1,265	54	448	50	50	117	823	823

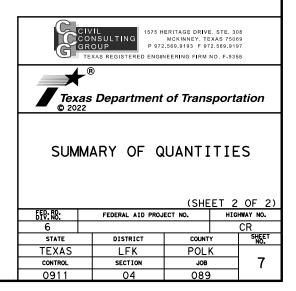
* BLOCK SODDING TO BE USED ONLY AS DIRECTED.

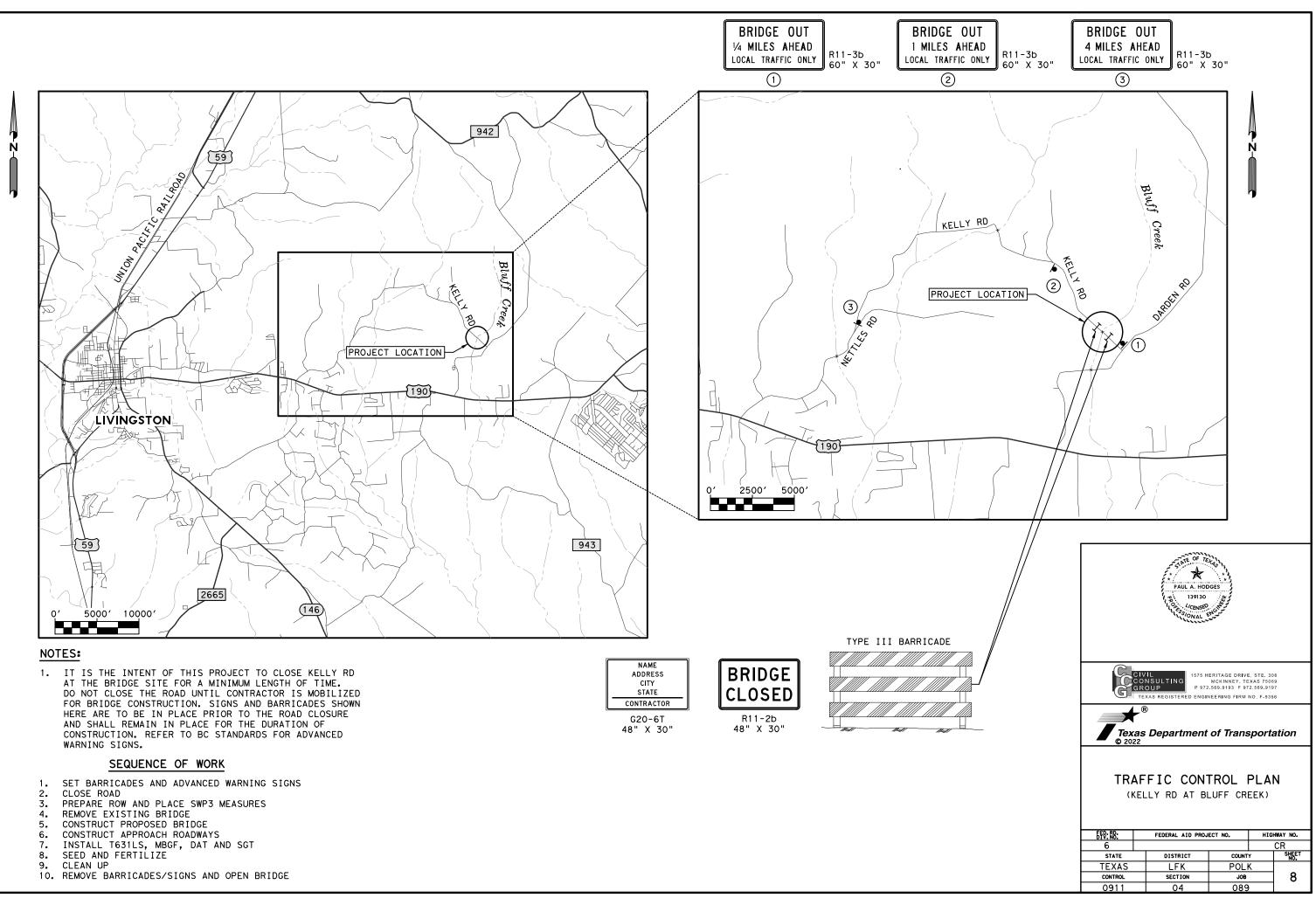
NOTES:

1. LOCATIONS AND TYPES OF BMPS MAY REQUIRE ADJUSTMENTS PRIOR TO OR AFTER PLACEMENT AS DIRECTED BY THE ENGINEER. ADJUSTMENTS SHOULD BE MADE TO ENSURE BMPS ARE WORKING EFFECTIVELY AND MAINTAIN COMPLIANCE WITH THE CONSTRUCTION GENERAL PERMIT AND WATER QUALITY REQUIREMENTS ASSOCIATED TO SECTION 404/401 PERMITS. NOTIFY THE ENGINEER PRIOR TO MAKING ADJUSTMENTS.

BRIDGE QUANTITIES									
	400	416	420	422	422	425	432	450	454
	6005	6004	6013	6001	6023	6035	6033	6019	6003
ITEM DESCRIPTION	CEM STABIL BKFL	DRILL SHAFT (36 IN)	CL C CONC (ABUT)	REINF CONC SLAB	SHEAR KEY	PRESTR CONC GIRDER (TX28)	RIPRAP (STONE PROTECTION) (18 IN)	RAIL (TY T631LS)	ARMOR JOINT
	CY	LF	CY	SF	CY	LF	CY	LF	LF
NBI: 11-187-0-AA03-29-007									
KELLY RD AT BLUFF CREEK	81.0	300	37.6	1,950	0.4	298	304	198	44
PROJECT TOTAL	81.0	300	37.6	1,950	0.4	298	304	198	44

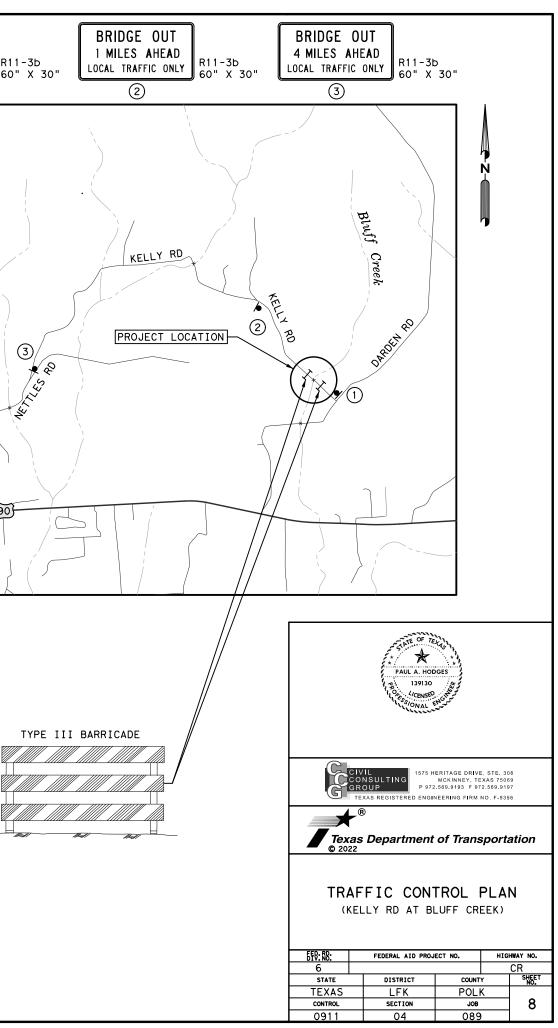






_					
	NAME	N			
ΙB	ADDRESS	ADI			
	CITY	C			
I ()	STATE	S			
	CONTRACTOR	CONTRACTOR			
	G20-6T	G2			
4	8" X 30"				





BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

- 1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- The development and design of the Traffic Control Plan (TCP) is the 2. responsibility of the Engineer.
- The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop. sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- The temporary traffic control devices shown in the illustrations of the 9. BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. Where highway construction or maintenance work is being undertaken, other than mobile operations as defined by the Texas Manual on Uniform Traffic Control Devices, CSJ limit signs are required. CSJ limit signs are shown on BC(2). The OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. The BEGIN ROAD WORK NEXT X MILES. CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits. For mobile operations, CSJ limit signs are not required.
- 11. Traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY NOTES:

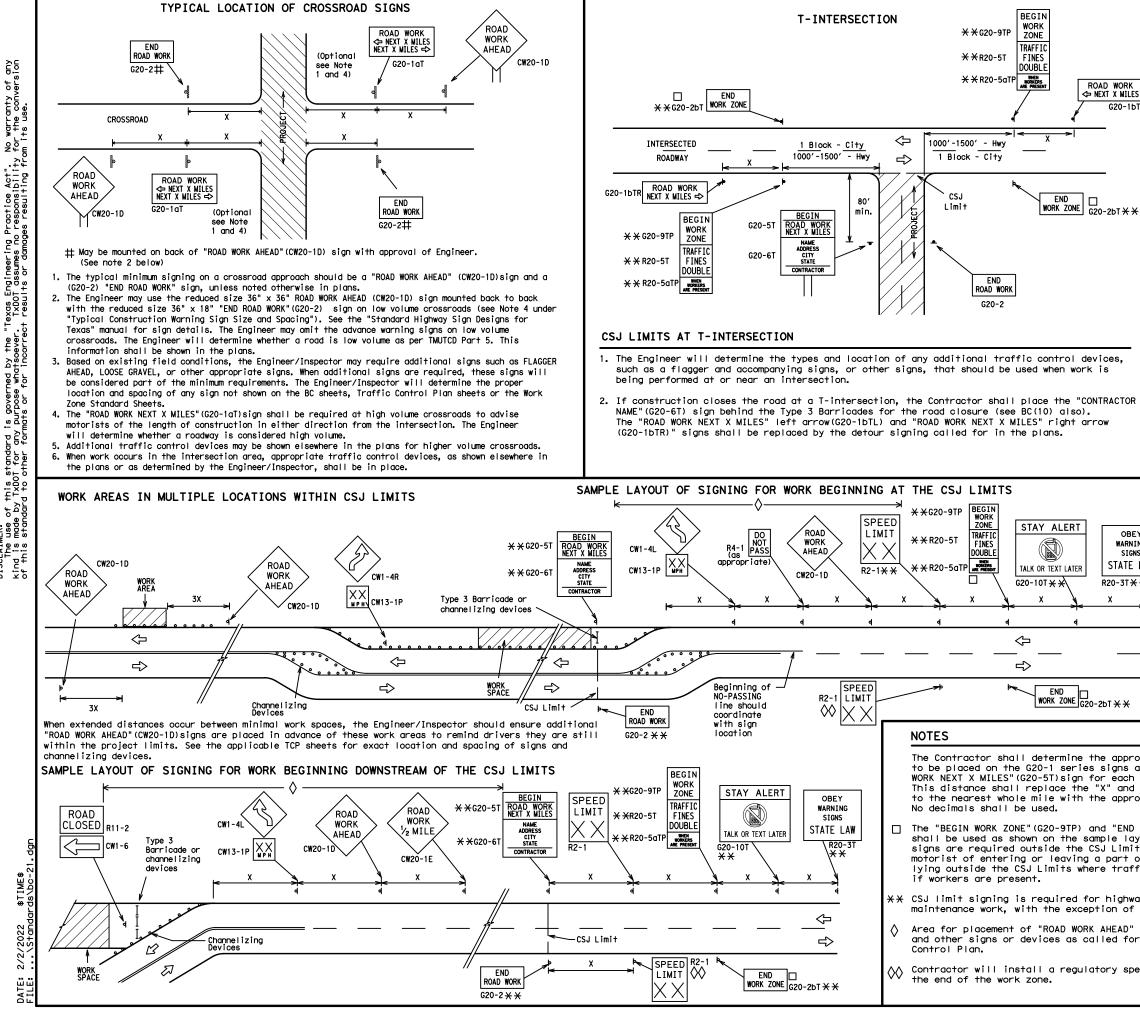
- 1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel." or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.
- 2. Except in emergency situations, flagger stations shall be illuminated when flagging is used at night.

COMPLIANT WORKZONE TRAFFIC CONTROL DEVICES

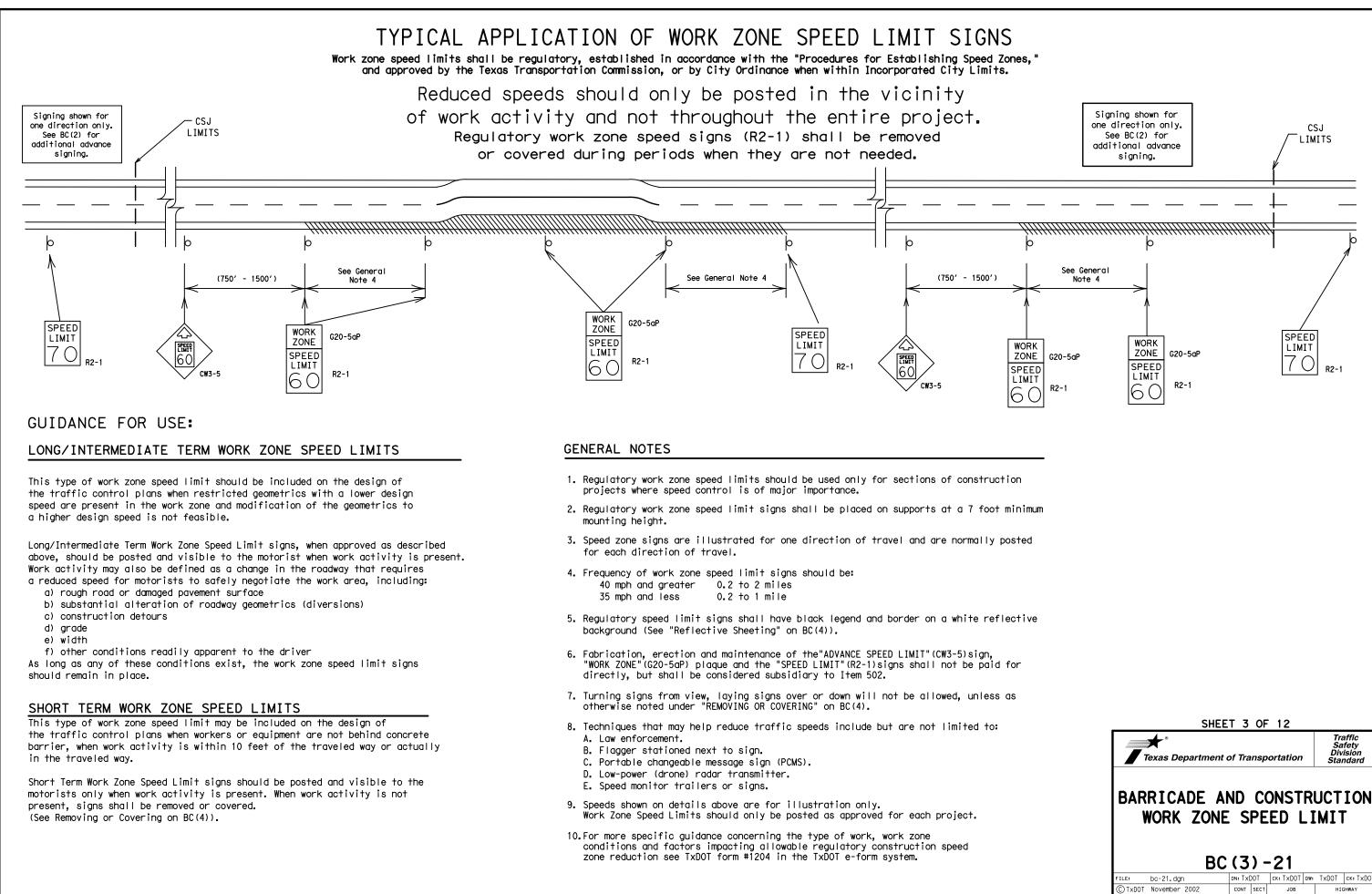
- 1. Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-gualified products and their sources.
- 2. Work zone traffic control devices shall be compliant with the Manual for Assessing safety Hardware (MASH).

THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov					
COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD)					
DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS)					
MATERIAL PRODUCER LIST (MPL)					
ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)"					
STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD)					
TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD)					
TRAFFIC ENGINEERING STANDARD SHEETS					

SHEE	SHEET 1 OF 12						
Traffic Safety Division Standard							
BARRICADE AND CONSTRUCTION GENERAL NOTES AND REQUIREMENTS BC(1)-21							
FILE: bc-21.dgn	DN: TXDC)T CK:TXDOT DW:	TxDOT CK: TXDOT				
© TxDOT November 2002	CONT SE	CT JOB	HIGHWAY				
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								15.6
	TYPICAL CON	STRUCTIO	ON WA	RNING SIGN	SIZ	E AND S	SPACIN	IG "°°°
		SIZ	E			SF	PACING	;
AD WORK IEXT X WILES	Sign Number or Series	Convent Roa	ional d	Expressway/ Freeway		Posted Speed	Sign Spaci "X"	
G20-1bTL	CW20 ⁴ CW21					MPH	Fee (Appr	
	CW22	48" ×	48"	48" × 48"		30	120	
	CW23					35	160	
-	CW25					40	240	
	CW1, CW2,					45	320	
0-2bT X X	CW7, CW8,	36" ×	36"	48" × 48"		50	400	
0 201 X X	CW9, CW11, CW14					55	500	-
	CW14					60 65	600 700	
	CW3, CW4,					65 70	800	
	CW5, CW6,	48" ×	48"	48" × 48"		70 75	900	
	CW8-3, CW10, CW12					80	1000	
								3
						*	*	-
ices, is TRACTOR	(TMUTCD) typica	the "Texas Il applicat Re from wor	Manual ion dia	on Uniform Traf agrams or TCP Sto	fic C andar e War	control De d Sheets. ming sign	vices"	
	GENERAL NOTES	5						
ow	1. Special or larg	per size si	gns may	/ be used as nec	essar	у.		
	2. Distance betwee advance warning		nould be	e increased as re	equir	ed to have	e 1500 f	feet
	3. Distance betwee or more advance		nould be	e increased as re	equir	ed to hav	e 1/2	mile
OBEY WARNING SIGNS STATE LAW R20-3T X X X	Note 2 under "ī 5. Only diamond sh 6. See sign size l	he discret Typical Loc haped warni isting in	ion of ation o ng sign "TMUTCI	the Engineer as of Crossroad Sig n sizes are indic	per ns". cateo k or	TMUTCD Pa 1. the "Stan	rt 5. Se dard Hiç	jhway
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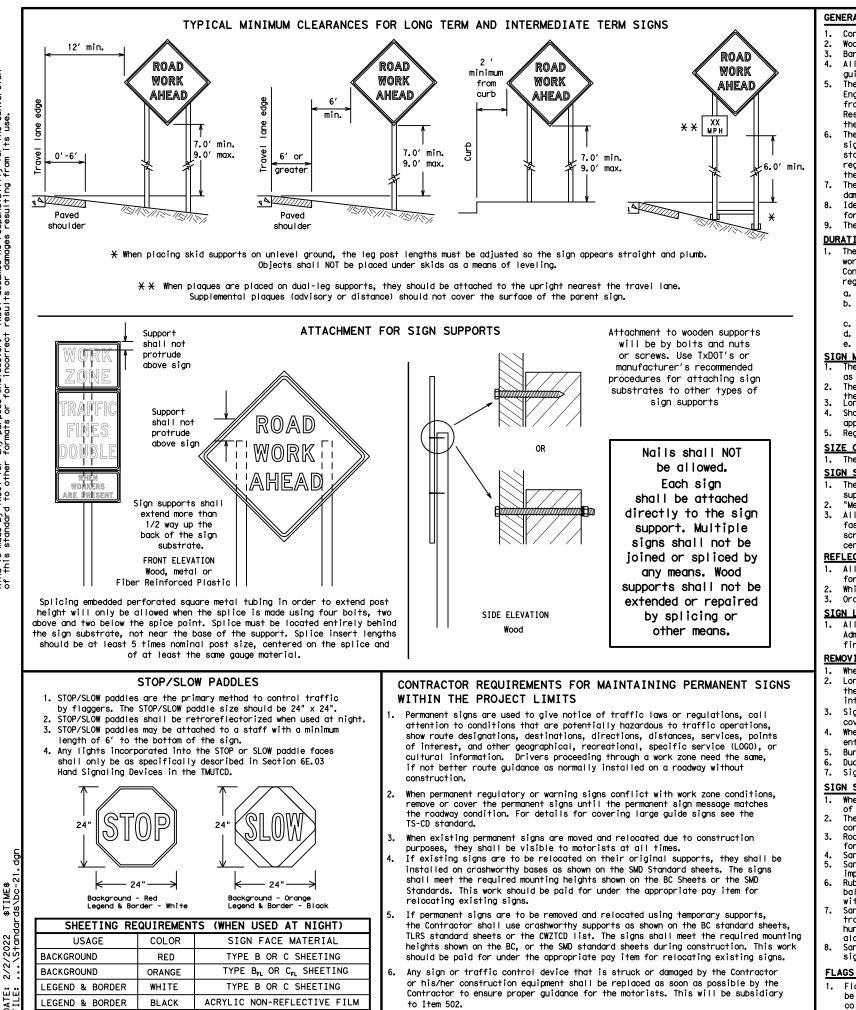
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GENERAL NOTES FOR WORK ZONE SIGNS

- Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.
- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports.
- guide the traveling public safely through the work zone.
- the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes. the Engineer can verify the correct procedures are being followed.
- damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- for identification shall be 1 inch.
- The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

<u>DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6)</u>

- 1. The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of regard to crashworthiness and duration of work requirements.
- a. Long-term stationary work that occupies a location more than 3 days.
- more than one hour. Short-term stationary - daytime work that occupies a location for more than 1 hour in a single daylight period.
- Short, duration work that occupies a location up to 1 hour.
- Mobile work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

SIGN MOUNTING HEIGHT

- as shown for supplemental plaques mounted below other signs.
- the ground. Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to
- appropriate Long-term/Intermediate sign height.

SIZE OF SIGNS

1. The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

SIGN SUBSTRATES

- "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave. centers. The Engineer may approve other methods of splicing the sign face.

REFLECTIVE SHEETING

- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).

SIGN LETTERS

first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- When sign messages may be confusing or do not apply, the signs shall be removed or completely covered.
- intersections where the sign may be seen from approaching traffic. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely
- covered when not required.
- entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.
- Burlap shall NOT be used to cover signs. Duct tape or other adhesive material shall NOT be affixed to a sign face.
- Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbags will be tied shut to keep the sand from spilling and to maintain a
- constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular
- impact. Rubber (such as tire inner tubes) shall NOT be used. Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list. Sandbags shall only be placed along or laid over the base supports of the
- traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

1. Flags may be used to draw attention to warning signs. When used, the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

No warranty of any for the conversion m its use. Texas Engineering Practice Act". TxDOT assumes no responsibility it results or damages resulting fro DISCLAIMER: The use of this standard is governed by the "T Kind is made by TXDD1 for any purpose whatseever. of this standard to other formats or for incorrect

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All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and

The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in

The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD) for small roadside signs. Supports for temporary large roadside signs shall meet the requirements detailed on the Temporary Large Roadside Signs (TLRS) standard sheets. The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a auestion regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so

The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or

Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used

work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in

Intermediate-term stationary - work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting

The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except

The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above

Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports. All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6"

1. All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300 White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL}, shall be used for rigid signs with orange backgrounds.

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of

Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any

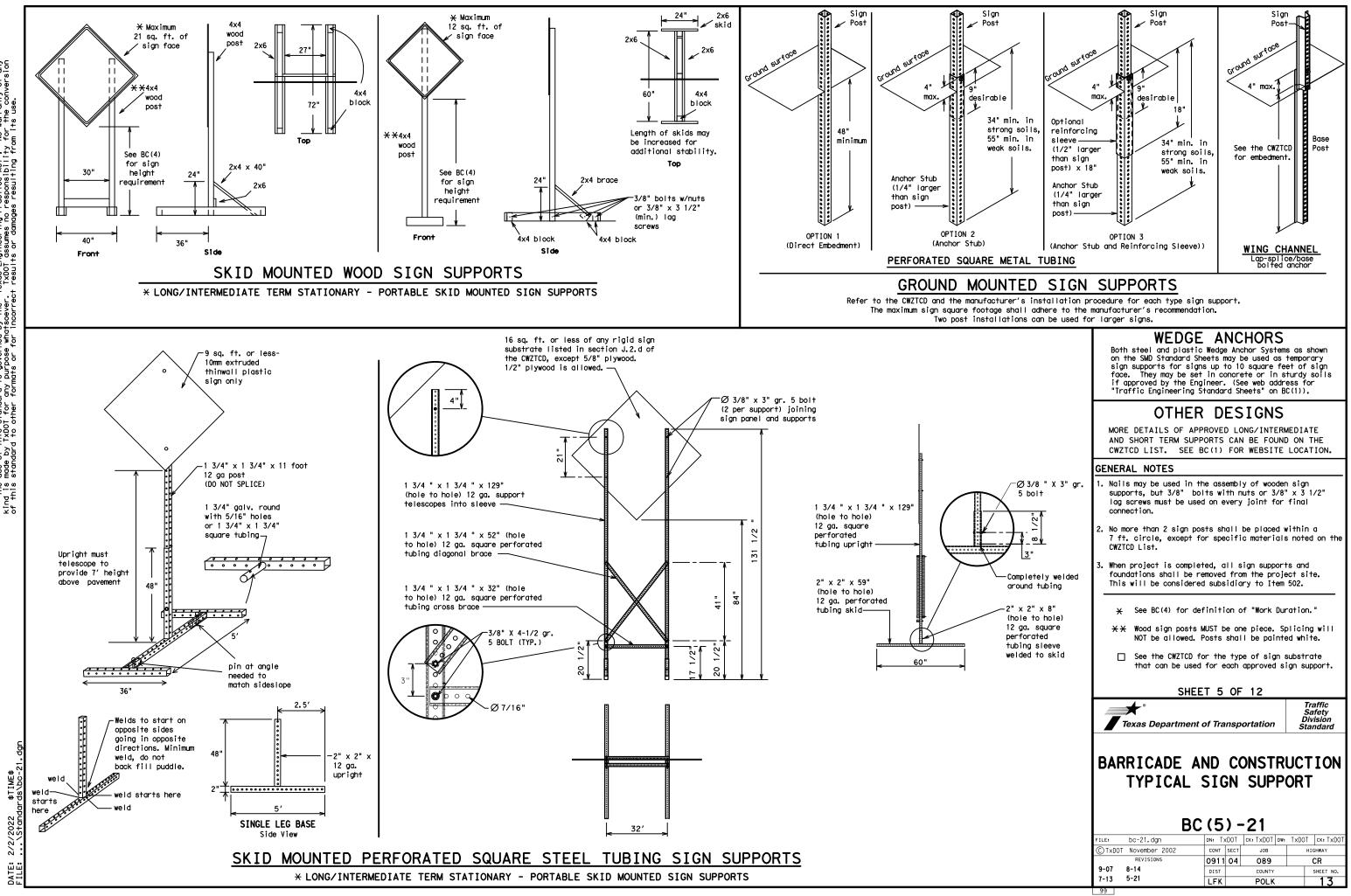
When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the

SHEET 4 OF 12

• • Texas Department of Transportation Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

BC (4) -21								
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PORTABLE CHANGEABLE MESSAGE SIGNS

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- Messages on PCMS should contain no more than 8 words (about four to 2. eight characters per word), not including simple words such as "TO." "FOR." "AT." etc.
- Messages should consist of a single phase, or two phases that 3. alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., 4. "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use, the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to 7. start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line.
- Do not use the word "Danger" in message.
 Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- 15. PCMS character height should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

WORD OR PHRASE	ABBREVIATION	WORD OR PHRASE	ABBREVIATION
Access Road	ACCS RD	Major	MAJ
Alternate	ALT	Miles	MI
Avenue	AVE	Miles Per Hour	MPH
Best Route	BEST RTE	Minor	MNR
Boulevard	BLVD	Monday	MON
Bridge	BRDG	Normal	NORM
Cannot	CANT	North	N
Center	CTR	Northbound	(route) N
Construction Ahead	CONST AHD	Parking	PKING RD
CROSSING	XING	Road	
Detour Route	DETOUR RTE	Right Lane	RT LN SAT
Do Not	DONT	Saturday	
East	F	Service Road	SERV RD
Eastbound	(route) E	Shoulder	SHLDR
Emergency	EMER	Slippery	SLIP
Emergency Vehicle	EMER VEH	South	S (resulta) S
Entrance, Enter	ENT	Southbound	(route) S
Express Lane	EXP LN	Speed	SPD ST
Expressway	EXPWY	Street	
XXXX Feet	XXXX FT	Sunday	SUN
Fog Ahead	FOG AHD	Telephone	PHONE
Freeway	FRWY. FWY	Temporary	TEMP
Freeway Blocked	FWY BLKD	Thursday	THURS
Friday	FRI	To Downtown	TO DWNTN
Hazardous Driving		Traffic	TRAF
Hazardous Material		Travelers	TRVLRS
High-Occupancy	HOV	Tuesday	TUES
Vehicle		Time Minutes	TIME MIN
Highway	HWY	Upper Level	UPR LEVEL
Hour(s)	HR, HRS	Vehicles (s)	VEH, VEHS
Information	INFO	Warning	WARN
It Is	ITS	Wednesday	WED
Junction	JCT	Weight Limit	WT LIMIT
Left	LFT	West	W
Left Lane		Westbound	(route) W
Lane Closed	LPT LN LN CLOSED	Wet Pavement	WET PVMT
Lower Level	LWR LEVEL	Will Not	WONT
Maintenance Roadway	MAINT		

RECOMMENDED PHASES AND FORMATS FOR PCMS MESSAGES DURING ROADWORK ACTIVITIES

(The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

Road/Lane/Ramp Closure List

	mΡ			0111
FREEWAY CLOSED X MILE		FRONTAGE ROAD CLOSED		ROADW XXX
ROAD CLOSED AT SH XXX		SHOULDER CLOSED XXX FT		FLAGO XXXX
ROAD CLSD AT FM XXXX		RIGHT LN CLOSED XXX FT		RIGHT NARRO XXXX
RIGHT X LANES CLOSED		RIGHT X LANES OPEN		MERGI TRAFF XXXX
CENTER LANE CLOSED		DAYTIME LANE CLOSURES		LOOS GRAV XXXX
NIGHT LANE CLOSURES		I-XX SOUTH EXIT CLOSED		DETO X MI
VARIOUS LANES CLOSED		EXIT XXX CLOSED X MILE		ROADW PAS SH XX
EXIT CLOSED		RIGHT LN TO BE CLOSED		BUM XXXX
MALL DRIVEWAY CLOSED		X LANES CLOSED TUE - FRI		TRAFF SIGN XXXX
XXXXXXXX BLVD CLOSED	*	LANES SHIFT in	Phase	1 must be

Other Co	ndition List
ROADWORK XXX FT	ROAD REPAIRS XXXX FT
FLAGGER XXXX FT	LANE NARROWS XXXX FT
RIGHT LN NARROWS XXXX FT	TWO-WAY TRAFFIC XX MILE
MERGING TRAFFIC XXXX FT	CONST TRAFFIC XXX FT
LOOSE GRAVEL XXXX FT	UNEVEN LANES XXXX FT
DETOUR X MILE	ROUGH ROAD XXXX FT
ROADWORK PAST SH XXXX	ROADWORK NEXT FRI-SUN
BUMP XXXX FT	US XXX EXIT X MILES
TRAFFIC SIGNAL XXXX FT	LANES SHIFT

	e/Effect on Travel List
MERGE RIGHT	FORM X LINES RIGHT
DETOUR NEXT X EXITS	USE XXXXX RD EXIT
USE EXIT XXX	USE EXIT I-XX NORTH
STAY ON US XXX SOUTH	USE I-XX E TO I-XX N
TRUCKS USE US XXX N	WATCH FOR TRUCKS
WATCH FOR TRUCKS	EXPECT DELAYS
EXPECT DELAYS	PREPARE TO STOP
REDUCE SPEED XXX FT	END SHOULDER USE
USE OTHER ROUTES	WATCH FOR WORKERS
STAY IN LANE	×

APPLICATION GUIDELINES

- 1. Only 1 or 2 phases are to be used on a PCMS.
- 2. The 1st phase (or both) should be selected from the "Road/Lane/Ramp Closure List" and the "Other Condition List".
- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- 4. A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- 5. If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

- 1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.
- appropriate.
- be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary.
- 7. FT and MI. MILE and MILES interchanged as appropriate. 8. AT. BEFORE and PAST interchanged as needed.
- 9. Distances or AHEAD can be eliminated from the message if a
- location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

used with STAY IN LANE in Phase 2.

FULL MATRIX PCMS SIGNS

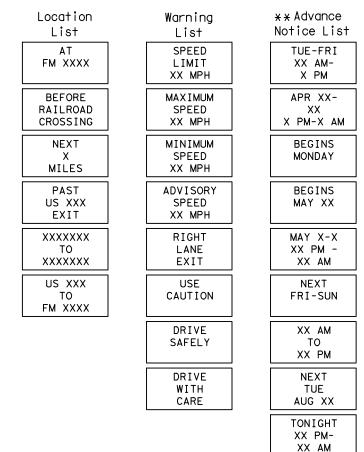
- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 un CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of shall maintain the legibility/visibility requirement listed above.
- When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and 3. for, or replace that sign.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC same size arrow.

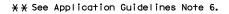
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Roadway

designation # IH-number, US-number, SH-number, FM-number

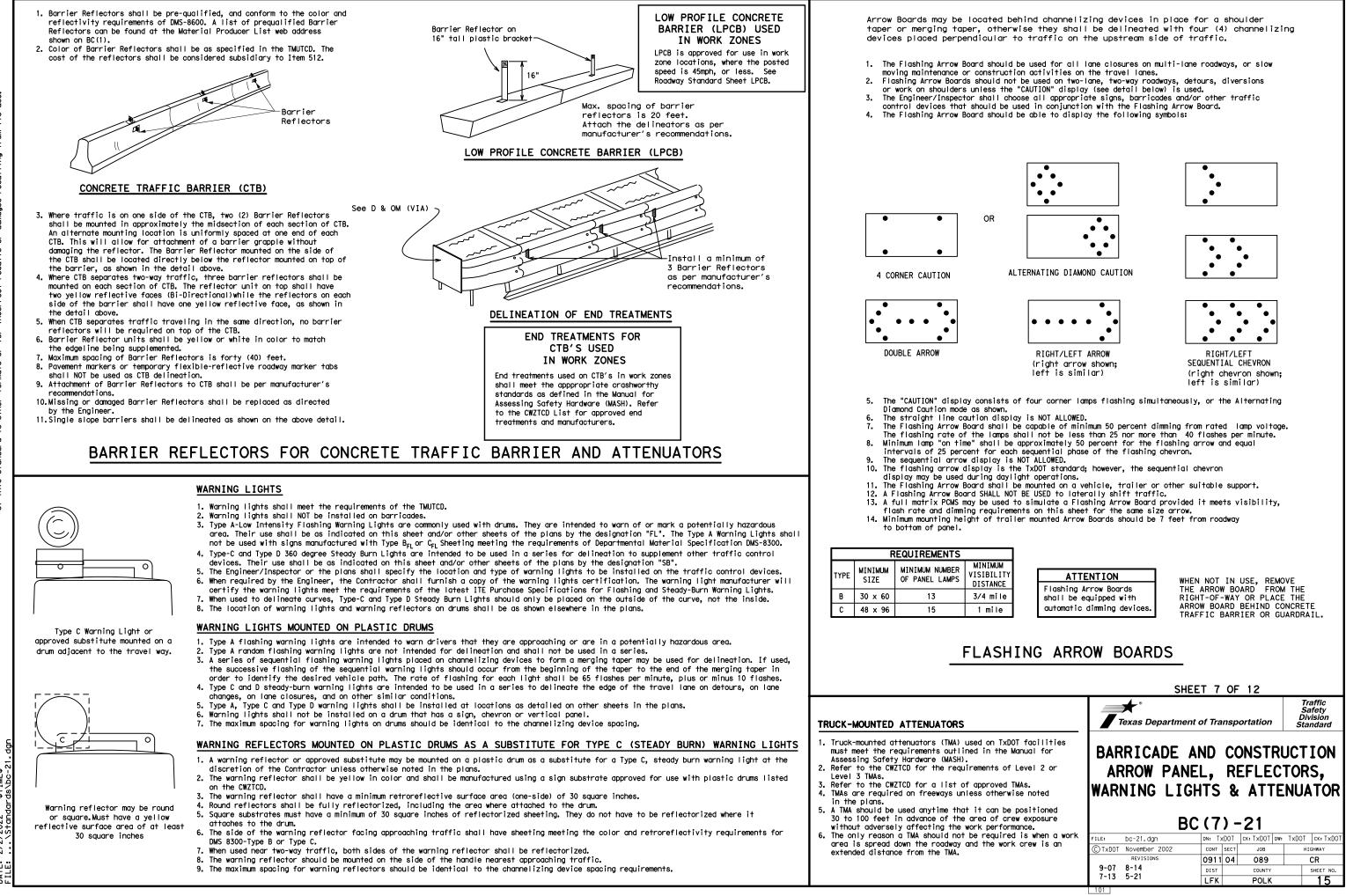
Phase 2: Possible Component Lists



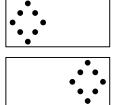


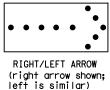
2. Roadway designations IH, US, SH, FM and LP can be interchanged as EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can

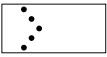
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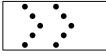


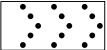
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GENERAL NOTES

- 1. For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections, one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 5. Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

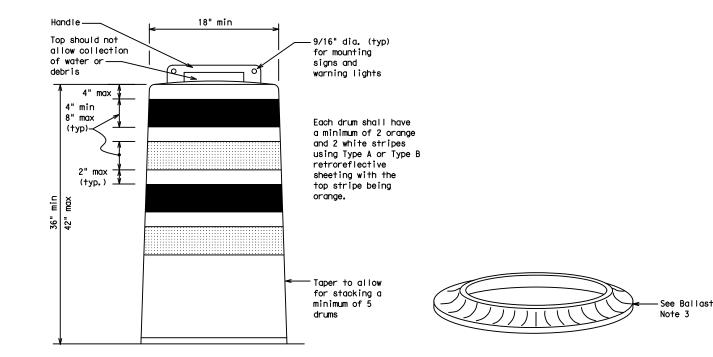
- Pre-qualified plastic drums shall meet the following requirements:
- 1. Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10. Drum and base shall be marked with manufacturer's name and model number.

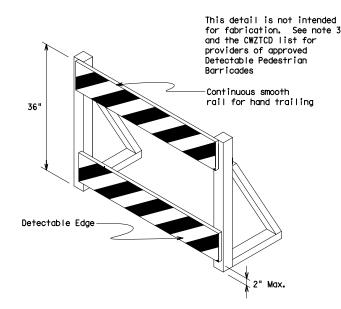
RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A or Type B reflective sheeting shall be supplied unless otherwise specified in the plans.
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

BALLAST

- Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- 3. Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- 5. When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.

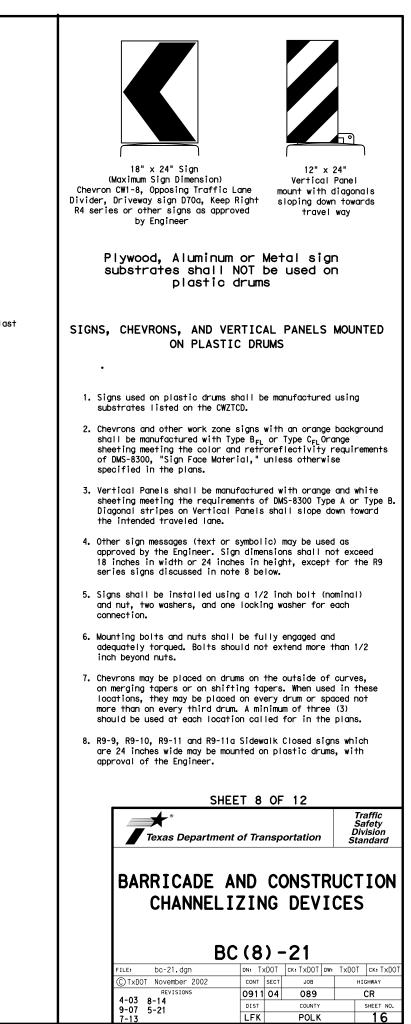


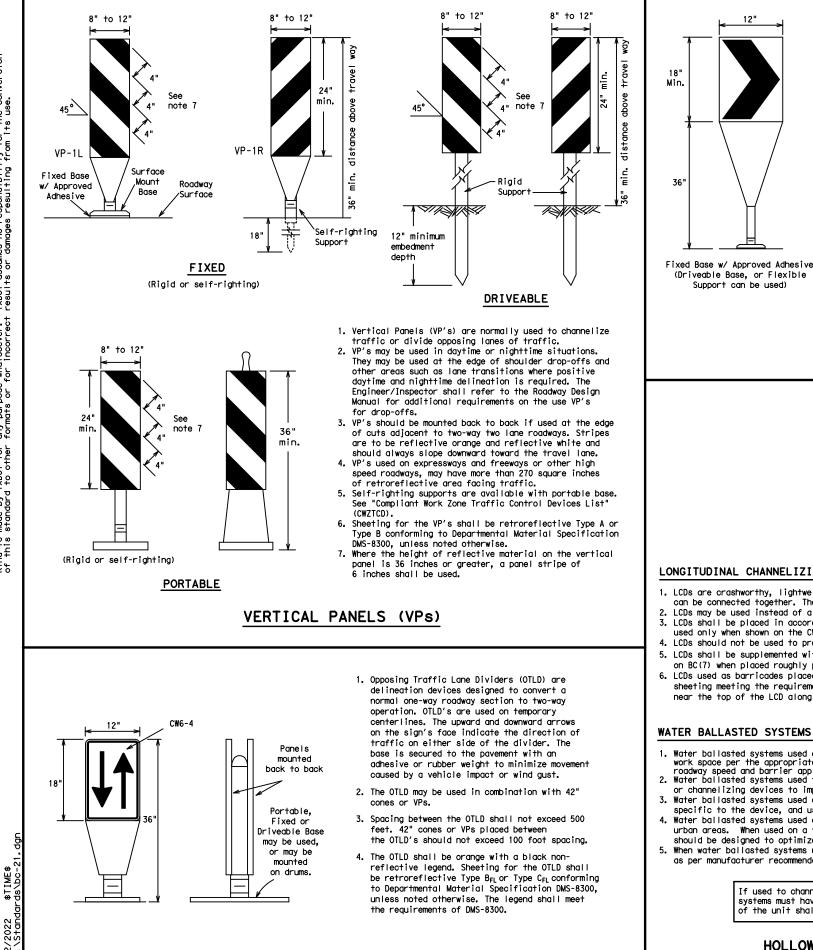


DETECTABLE PEDESTRIAN BARRICADES

- When existing pedestrian facilities are disrupted, closed, or relocated in a TTC zone, the temporary facilities shall be detectable and include accessibility features consistent with the features present in the existing pedestrian facility. Refer to WZ(BTS-2) for Pedestrian Control requirements for Sidewalk Diversions, Sidewalk Detours and Crosswalk Closures.
 Where pedestrians with visual disabilities normally use the
- Where pedestrians with visual disabilities normally use the closed sidewalk, a Detectable Pedestrian Barricade shall be placed across the full width of the closed sidewalk instead of a Type 3 Barricade.
- Detectable pedestrian barricades similar to the one pictured above, longitudinal channelizing devices, some concrete barriers, and wood or chain link fencing with a continuous detectable edging can satisfactorily delineate a pedestrian path.
- 4. Tape, rope, or plastic chain strung between devices are not detectable, do not comply with the design standards in the "Americans with Disabilities Act Accessibility Guidelines (ADAAG)" and should not be used as a control for pedestrian movements.
- Warning lights shall not be attached to detectable pedestrian barricades.
- Detectable pedestrian barricades should use 8" nominal barricade rails as shown on BC(10) provided that the top rail provides a smooth continuous rail suitable for hand trailing with no splinters, burrs, or sharp edges.

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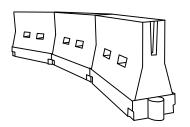




OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

- 1. The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the outside of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible for at least 500 feet.
- 5. Chevrons shall be orange with a black nonreflective legend. Sheeting for the chevron shall be retroreflective Type BFL or Type CFL conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the requirements of DMS-8300.
- 6. For Long Term Stationary use on tapers or transitions on freeways and divided highways, self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

CHEVRONS



LONGITUDINAL CHANNELIZING DEVICES (LCD)

12"

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact. 2. LCDs may be used instead of a line of cones or drums.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and used only when shown on the CWZTCD list.
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers on BC(7) when placed roughly parallel to the travel lanes.
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10). Place reflective sheeting near the top of the LCD along the full length of the device.

WATER BALLASTED SYSTEMS USED AS BARRIERS

- Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the work space per the appropriate Manual for Assessing Safety Hardware (MASH) crashworthiness requirements based on roadway speed and barrier application.
- Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation
- or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings. 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements
- specific to the device, and used only when shown on the CWZTCD list. Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH) urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length
- should be designed to optimize road user operations considering the available geometric conditions. When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated as per manufacturer recommendations or flared to a point outside the clear zone.

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top of the unit shall not be less than 32 inches in height.

HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

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2/2/2022

GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.

Posted Speed	Formula	Minimum Desirable Taper Lengths XX			Suggested Maximu Spacing of Channelizing Devices		
		10' Offset	11' Offset	12' Offset	On a Taper	0n a Tangent	
30	2	150′	165′	180′	30′	60′	
35	$L = \frac{WS^2}{60}$	205′	225′	245′	35′	70'	
40	60	265′	295′	320′	40′	80'	
45		450′	495′	540′	45′	90'	
50		500'	550′	600′	50′	100′	
55	L=WS	550′	605′	660′	55′	110'	
60	L-#5	600′	660′	720'	60′	120'	
65		650′	715′	780′	65′	130′	
70		700′	770′	840′	70′	140′	
75		750′	825′	900′	75′	150′	
80		800'	880′	960′	80′	160′	

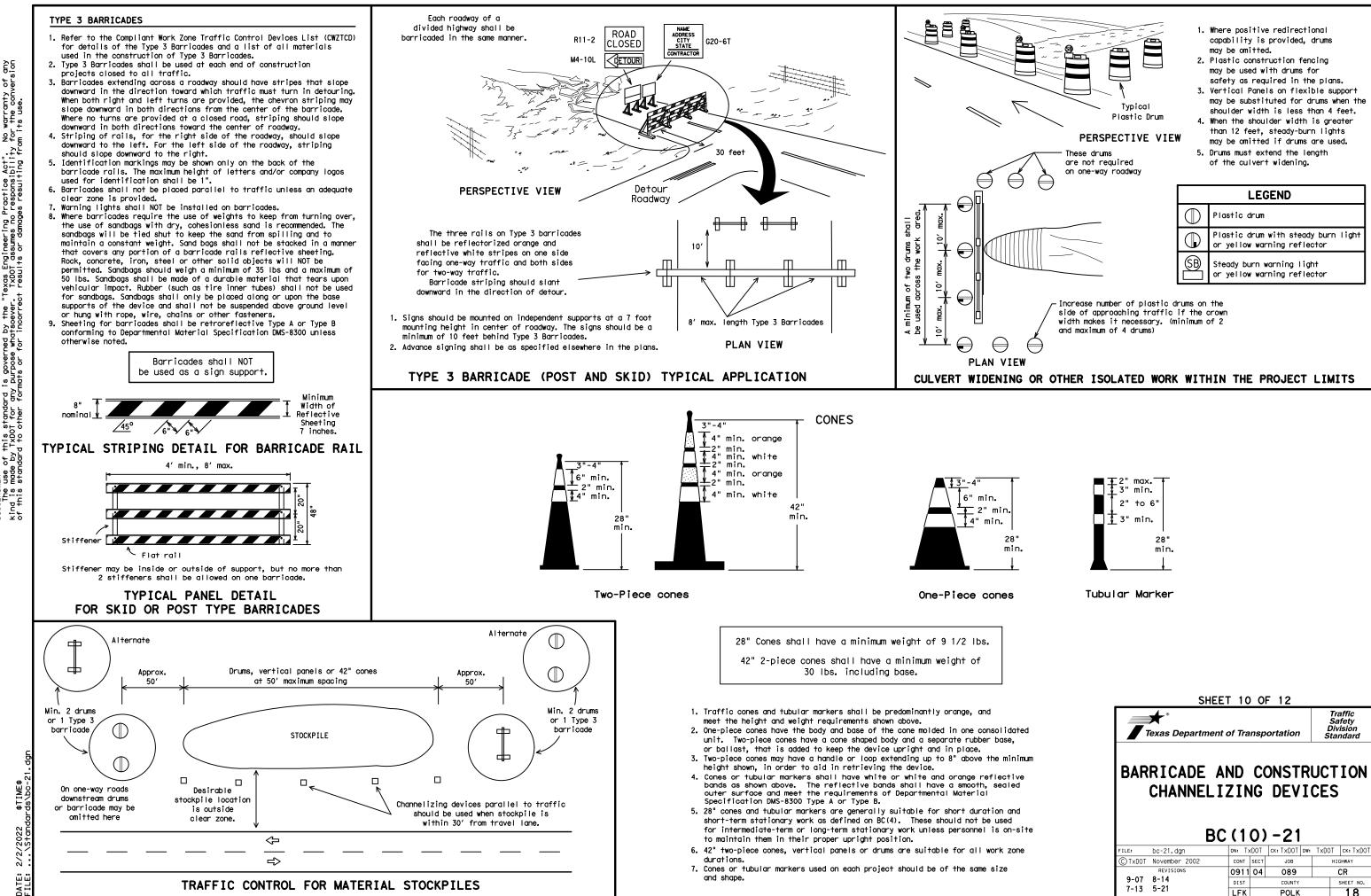
L=Length of Taper (FT.) W=Width of Offset (FT.) S=Posted Speed (MPH) SUGGESTED MAXIMUM SPACING OF

CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

SHEET 9 OF 12	
Texas Department of Transportation	Traffic Safety Division Standard

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

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WORK ZONE PAVEMENT MARKINGS

GENERAL

- The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- 2. Color, patterns and dimensions shall be in conformance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 3. Additional supplemental pavement marking details may be found in the plans or specifications.
- Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- 5. When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings."

RAISED PAVEMENT MARKERS

- 1. Raised pavement markers are to be placed according to the patterns on BC(12).
- All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- 1. Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

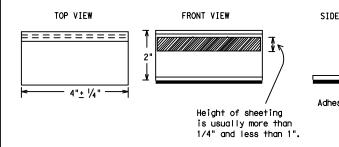
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markings and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- 8. Removal of raised pavement markers shall be as directed by the Engineer.
- Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS," unless otherwise stated in the plans.
- 10.Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECU TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARK TABS TO THE PAVEMENT SURFACE

- Temporary flexible-reflective roadway marker tabs used as guiden shall meet the requirements of DMS-8242.
- Tabs detailed on this sheet are to be inspected and accepted by Engineer or designated representative. Sampling and testing is normally required, however at the option of the Engineer, either or "B" below may be imposed to assure quality before placement or roadway.
 - A. Select five (5) or more tabs at random from each lot or sl and submit to the Construction Division, Materials and Par Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix (5) tabs at 24 inch intervals on an asphaltic pavement in straight line. Using a medium size passenger vehicle or pi run over the markers with the front and rear tires at a sp of 35 to 40 miles per hour, four (4) times in each directi more than one (1) out of the five (5) reflective surfaces be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. Standard Sheet TCP(7-1) for tab placement on seal coat work.

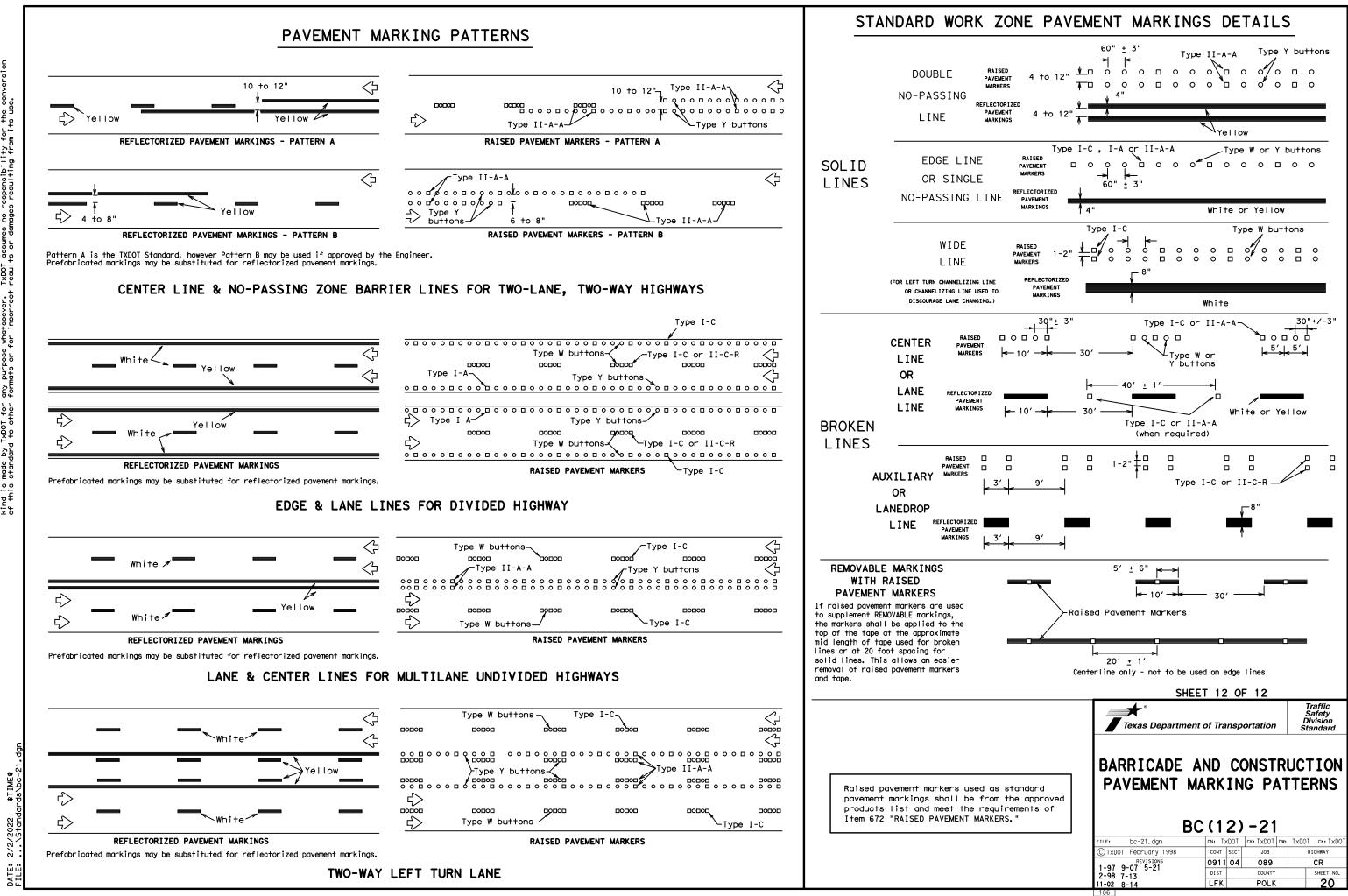
RAISED PAVEMENT MARKERS USED AS GUIDEMARK

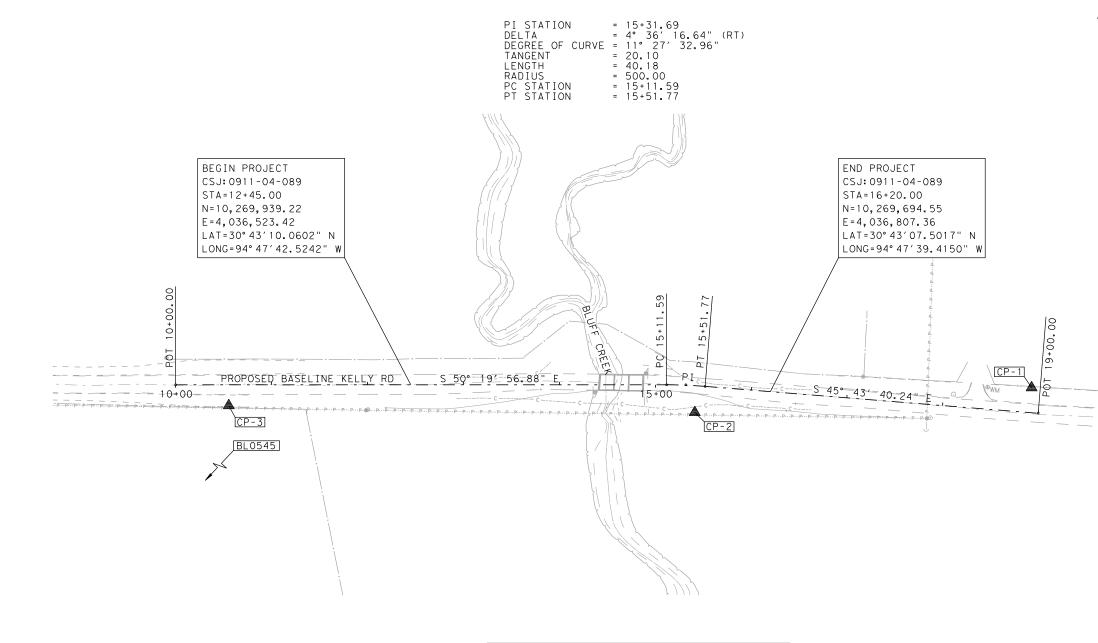
- Raised pavement markers used as guidemarks shall be from the ap product list, and meet the requirements of DMS-4200.
- All temporary construction raised pavement markers provided on project shall be of the same manufacturer.
- Adhesive for guidemarks shall be bituminous material hot applie butyl rubber pad for all surfaces, or thermoplastic for concresurfaces.

Guidemarks shall be designated as:

YELLOW - (two amber reflective surfaces with yellow body). WHITE - (one silver reflective surface with white body).

	DEPARTMENTAL MATERIAL SPECIFICATION	ONS
	PAVEMENT MARKERS (REFLECTORIZED)	DMS-4200
	TRAFFIC BUTTONS	DMS-4300
EW	EPOXY AND ADHESIVES	DMS-6100
57	BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS	DMS-6130
	PERMANENT PREFABRICATED PAVEMENT MARKINGS TEMPORARY REMOVABLE, PREFABRICATED	DMS-8240
	PAVEMENT MARKINGS	DMS-8241
N	TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS	DMS-8242
7	A list of prequalified reflective raised pavement non-reflective traffic buttons, roadway marker tab pavement markings can be found at the Material Pro web address shown on BC(1).	s and othe
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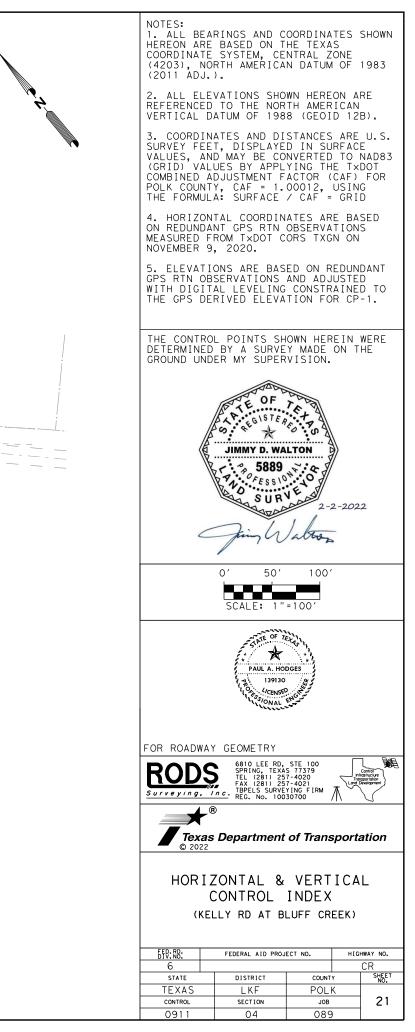


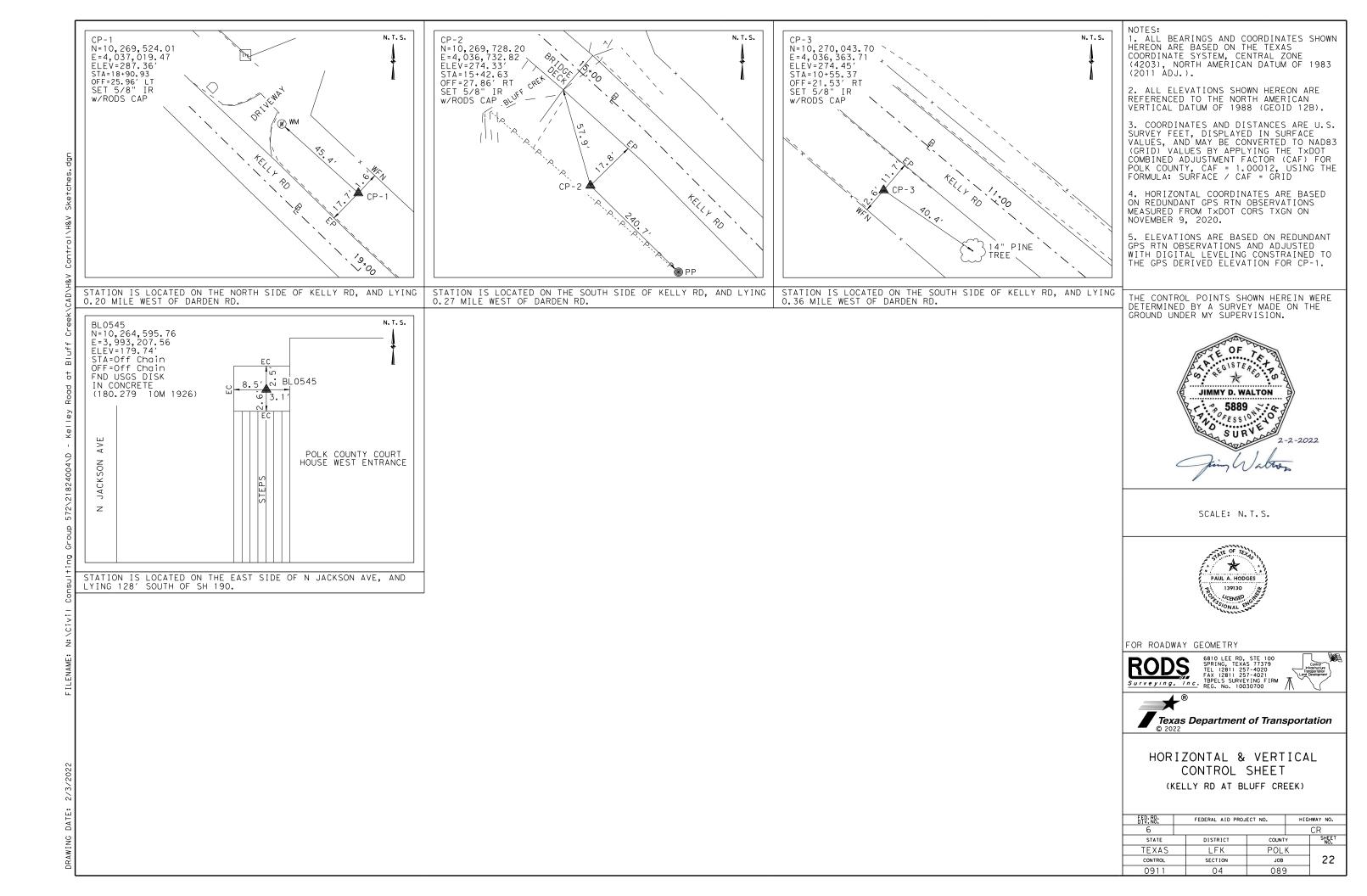
From	То	D	ired	ctio	n		Distance
CP-3	CP-2	S	49°	28′	39.16"	E	485.57′
CP-2	CP-1	S	54°	32′	11.43"	E	351.94′

Point	North	East	Elevation	Station	Offset	Description
CP-3	10,270,043.70	4,036,363.71	274.45′	10+55.37	21.53′ RT	SET 5/8" IR W/RODS CAP
CP-2	10,269,728.20	4,036,732.82	274.33′	15+42.63	27.86′ RT	SET 5/8" IR W/RODS CAP
CP-1	10,269,524.01	4,037,019.47	287.36′	18+90.93	25.96′ LT	SET 5/8" IR W/RODS CAP
BL0545	10,264,595.76	3,993,207.56	179.74′	Off Chain	Off Chain	FND USGS DISK IN CONC(180.279 10M 1926)
TXGN	10,277,330.62	3,996,340.07	184.84′	Off Chain	Off Chain	TXGN

Control Name		ed: NAD83 (vario nate Informatic	'	Measur Coordi	Deferent (Published - Measured)				
	N. Coord.	E. Coord.	Elev.	N. Coord.	E. Coord.	Elev.	North	East	Elev.
BL0545	10,264,599	3,993,196	180.2	10,264,595.76	3,993,207.56	179.74	2.8	-11.7	0.46

Notes: Measured values were determined with redundant GPS measurements, conventional surveying methods and digital leveling; BL0545 is of Second Vertical Order, Class Zero; published values are based on NAD83 (1986 Adj), NAVD88 (VERTCON); coordinates were determined with hand-held GPS.

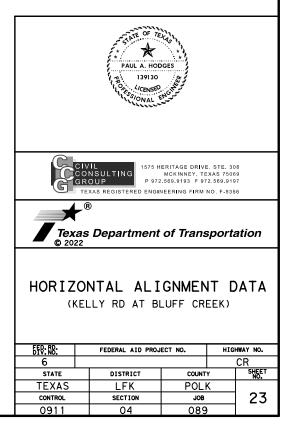


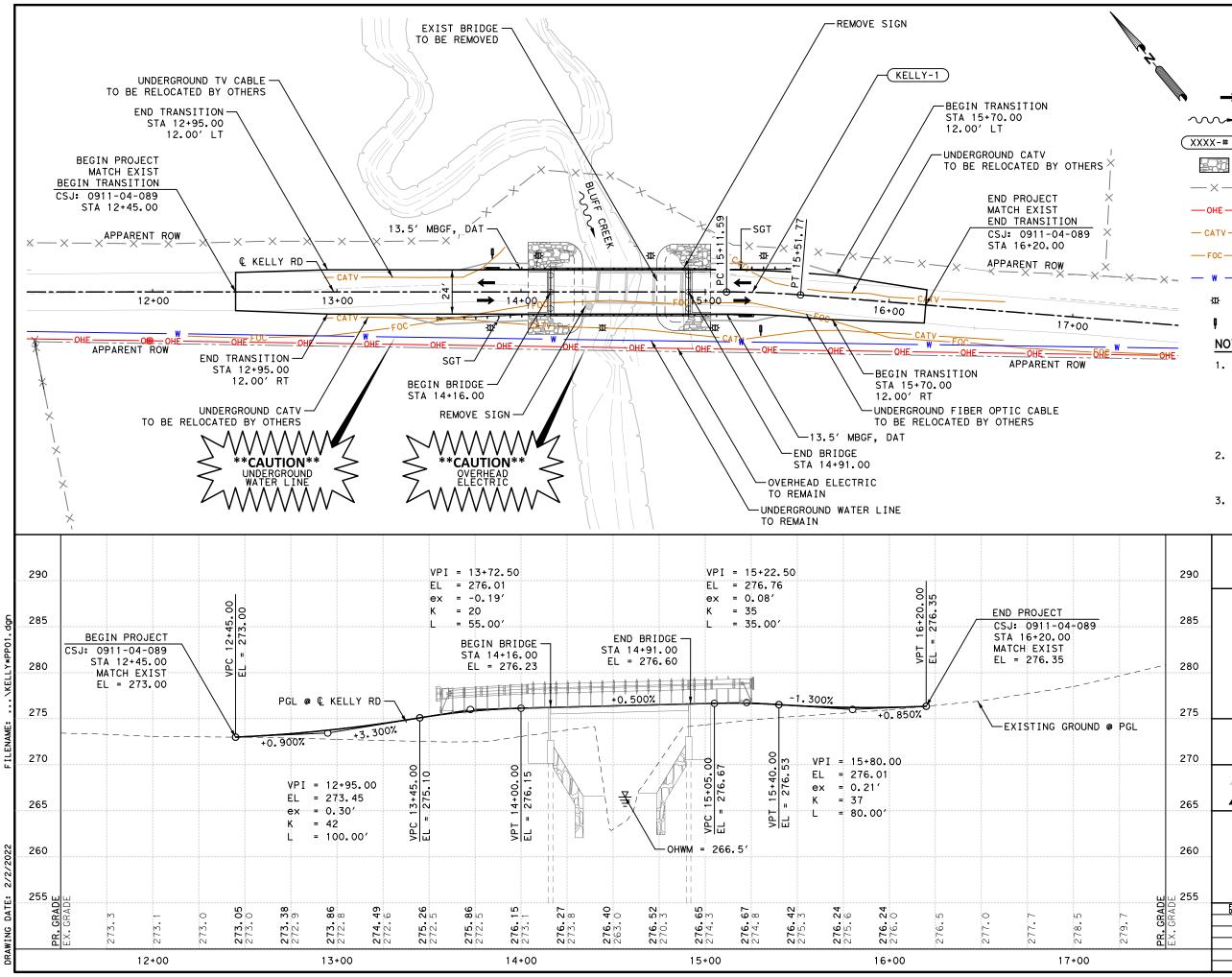


Chain KELLY contains: KELLY01 CUR KELLY-1 KELLY02

Beginning chain KELLY description

Point KELLY01	N 10,27	0,095.61 E	4,036,334.83 Sta	10+00.00
Course from KELLY01	1 to PC KELLY-	1 S 50° 19	′ 56.88" E Dis† 511.59	
		Curve D		
		*	*	
Curve KELLY-1 P.I. Station Delta = Degree =	15+31.6 4° 36′ 16.64 11° 27′ 32.96	(RT)	10,269,756.22 E	4,036,744.11
rangent = _ength =	20.1 40.1	0		
Radius = External =	500.0 0.4	10		
_ong Chord = Mid. Ord. =	40.1 0.4			
P.C. Station	15+11.5		10,269,769.05 E	4,036,728.63
P.T. Station	15+51.7		10,269,742.18 E	4,036,758.50
C.C. Back = S 50	0° 19′ 56.88"	N F	10,269,384.17 E	4,036,409.47
	5° 43′ 40.24"			
Chord Bear = S 48	3° 01′ 48.56"	E		
Course from PT KELL	_Y-1 to KELLYC)2 S 45° 43	′ 40.24" E Dis† 348.23	
Point KELLY02	N 10,26	59,499.10 E	4,037,007.84 Sta	19+00.00

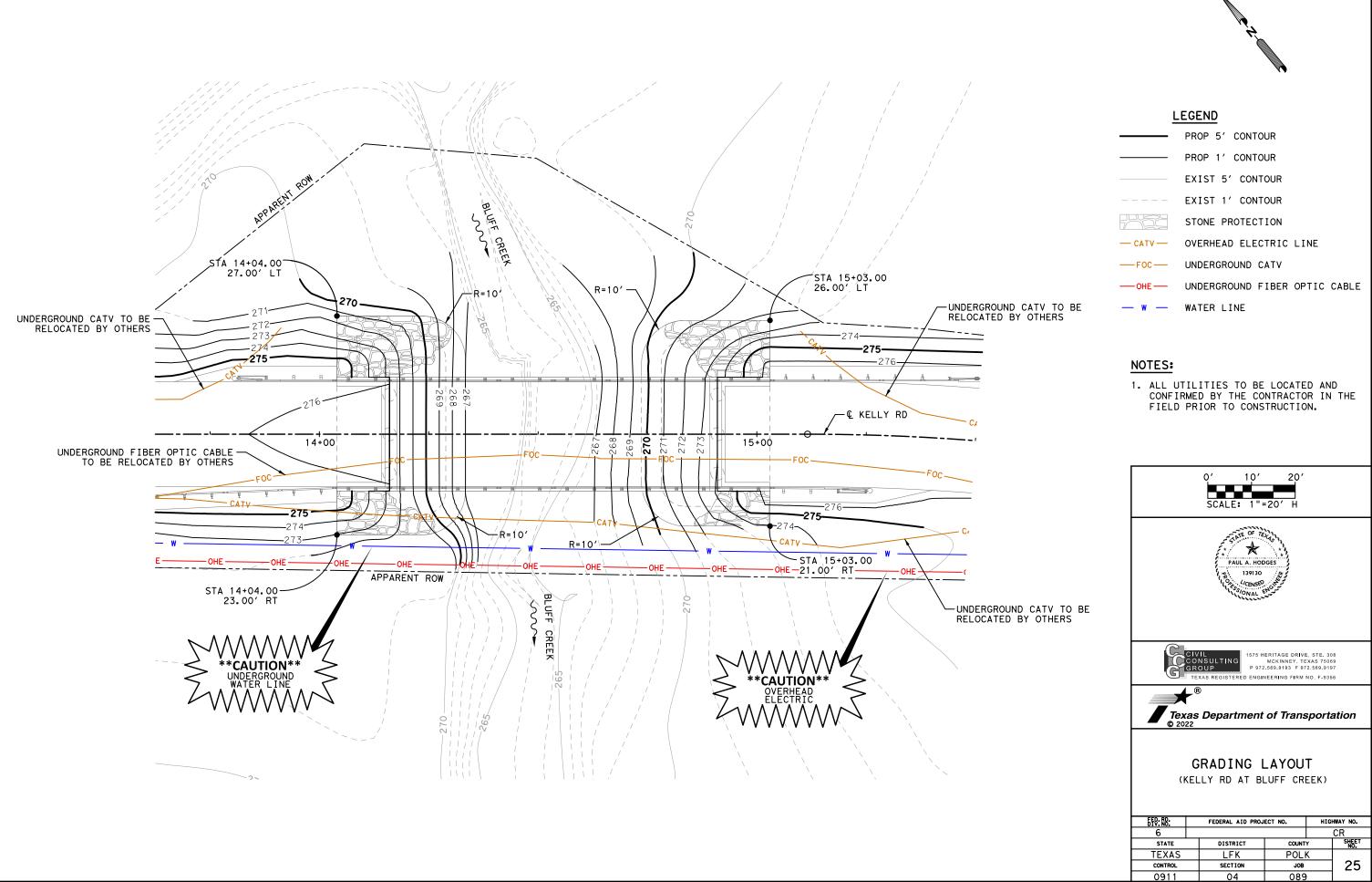




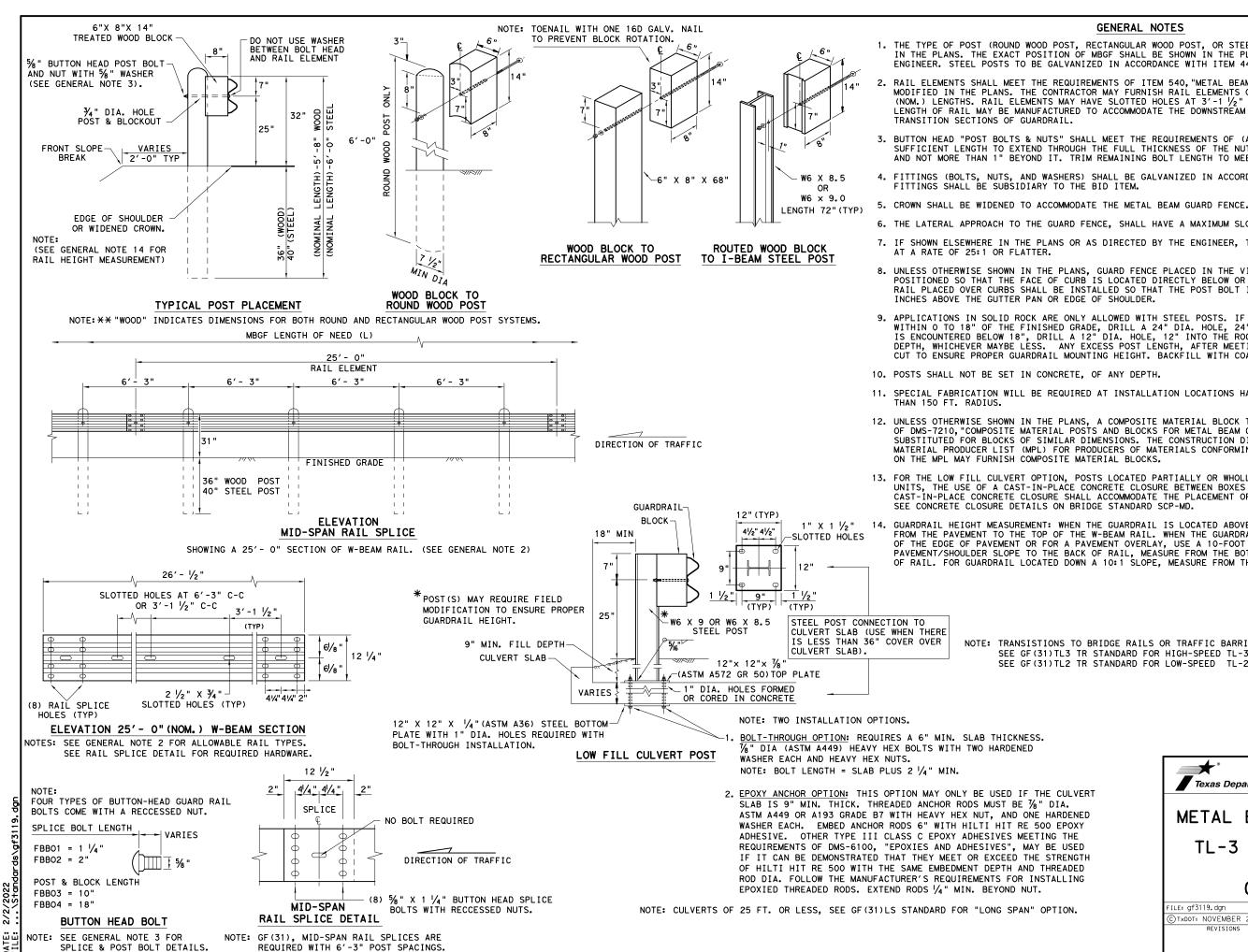
	LEGEND
\rightarrow	PROPOSED TRAFFIC DIRECTION
~~ ~	CHANNEL FLOW DIRECTION
XXXX-#	HORIZONTAL CURVE #
5-00	STONE PROTECTION
—×—	EXISTING FENCE
— оне —	OVERHEAD ELECTRIC LINE
— CATV —	UNDERGROUND CATV
— FOC —	UNDERGROUND FIBER OPTIC CABLE
— w —	WATER LINE
ф	DEL ASSM (D-SW)SZ(BRF)(GF1)(BI)
ę	INSTL OM ASSM (OM-3L)(TWT)GND
NOTE	<u>:S:</u>
	APPARENT RIGHT OF WAY SHOWN IS A

- IS A PRESCRIBED WIDTH BASED ON EXISTING FENCE LINES AND UTILITY MARKERS. A BOUNDARY SURVEY WAS NOT PERFORMED, NO CONVEYANCE OR EASEMENT OF THE PUBLIC ROAD TO THE COUNTY COULD BE FOUND.
- ALL UTILITIES TO BE LOCATED AND CONFIRMED BY THE CONTRACTOR IN THE FIELD PRIOR TO CONSTRUCTION.
- WHEN BRIDGE IS REMOVED, REMOVE EXISTING LOAD ZONE SIGNS APPROACHING THE BRIDGE.

		290		0' 25' SCALE: 1"	50' 50' H 10' V		
-089		285		PAUL A, HOE	tas MGES		
		280		1 9 139130 1 0 4/CENSE 1 0 5/ONAL	AL DE STATE		
		275					
ROUND @	PGL	270		NSULTING	ERITAGE DRIVE MCKINNEY, TE 569.9193 F 97 NEERING FIRM I	XAS 7506 2.569.919	9 7
		265		Department	of Trans	sporta	ation
		260	F	PLAN & PF	ROFIL	E	
	щ	255 СКАDE	(KEI	LY RD AT BI	_UFF CRI	EEK)	
	AD	AD	EED: RD:	FEDERAL AID PROJ	ECT NO.	HIG	HWAY NO.
с. 2	7. SR	3R	6				CR
78.	- CT		STATE	DISTRICT	COUNT		SHEET NO.
^N	27 PR	ΕX	TEXAS	LFK	POLI	<	
+00			CONTROL	SECTION	JOB		24
.00			0911	04	089		
			5311		000		



	PROP 5' CONTOUR
	PROP 1' CONTOUR
	EXIST 5' CONTOUR
	EXIST 1' CONTOUR
	STONE PROTECTION
— CATV —	OVERHEAD ELECTRIC LINE
— FOC —	UNDERGROUND CATV
OHE	UNDERGROUND FIBER OPTIC CABLE
— w —	WATER LINE



GENERAL NOTES

1. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST, OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. THE EXACT POSITION OF MBGF SHALL BE SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER. STEEL POSTS TO BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING."

RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED IN THE PLANS. THE CONTRACTOR MAY FURNISH RAIL ELEMENTS OF 25'- 0", OR 12'- 6" (NOM.) LENGTHS. RAIL ELEMENTS MAY HAVE SLOTTED HOLES AT $3'-1 \frac{1}{2}$ " C-C OR 6'-3" C-C. A SPECIAL LENGTH OF RAIL MAY BE MANUFACTURED TO ACCOMMODATE THE DOWNSTREAM ANCHOR TERMINAL (DAT) AND THE

3. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 5/8" WASHER (FWC16g) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING BOLT LENGTH TO MEET REQUIRED LENGTH.

4. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING. FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.

6. THE LATERAL APPROACH TO THE GUARD FENCE, SHALL HAVE A MAXIMUM SLOPE OF 1V:10H.

7. IF SHOWN ELSEWHERE IN THE PLANS OR AS DIRECTED BY THE ENGINEER, THE GUARD FENCE MAY BE FLARED

8. UNLESS OTHERWISE SHOWN IN THE PLANS. GUARD FENCE PLACED IN THE VICINITY OF CURBS SHALL BE POSITIONED SO THAT THE FACE OF CURB IS LOCATED DIRECTLY BELOW OR BEHIND THE FACE OF THE RAIL. RAIL PLACED OVER CURBS SHALL BE INSTALLED SO THAT THE POST BOLT IS LOCATED APPROXIMATELY 25

9. APPLICATIONS IN SOLID ROCK ARE ONLY ALLOWED WITH STEEL POSTS. IF SOLID ROCK IS ENCOUNTERED WITHIN 0 TO 18" OF THE FINISHED GRADE, DRILL A 24" DIA. HOLE, 24" INTO THE ROCK. IF SOLID ROCK IS ENCOUNTERED BELOW 18", DRILL A 12" DIA. HOLE, 12" INTO THE ROCK OR TO THE STANDARD EMBEDMENT DEPTH, WHICHEVER MAYBE LESS. ANY EXCESS POST LENGTH, AFTER MEETING THESE DEPTHS, MAY BE FIELD CUT TO ENSURE PROPER GUARDRAIL MOUNTING HEIGHT. BACKFILL WITH COARSE AGGREGATE MATERIAL.

11. SPECIAL FABRICATION WILL BE REQUIRED AT INSTALLATION LOCATIONS HAVING A CURVATURE OF LESS

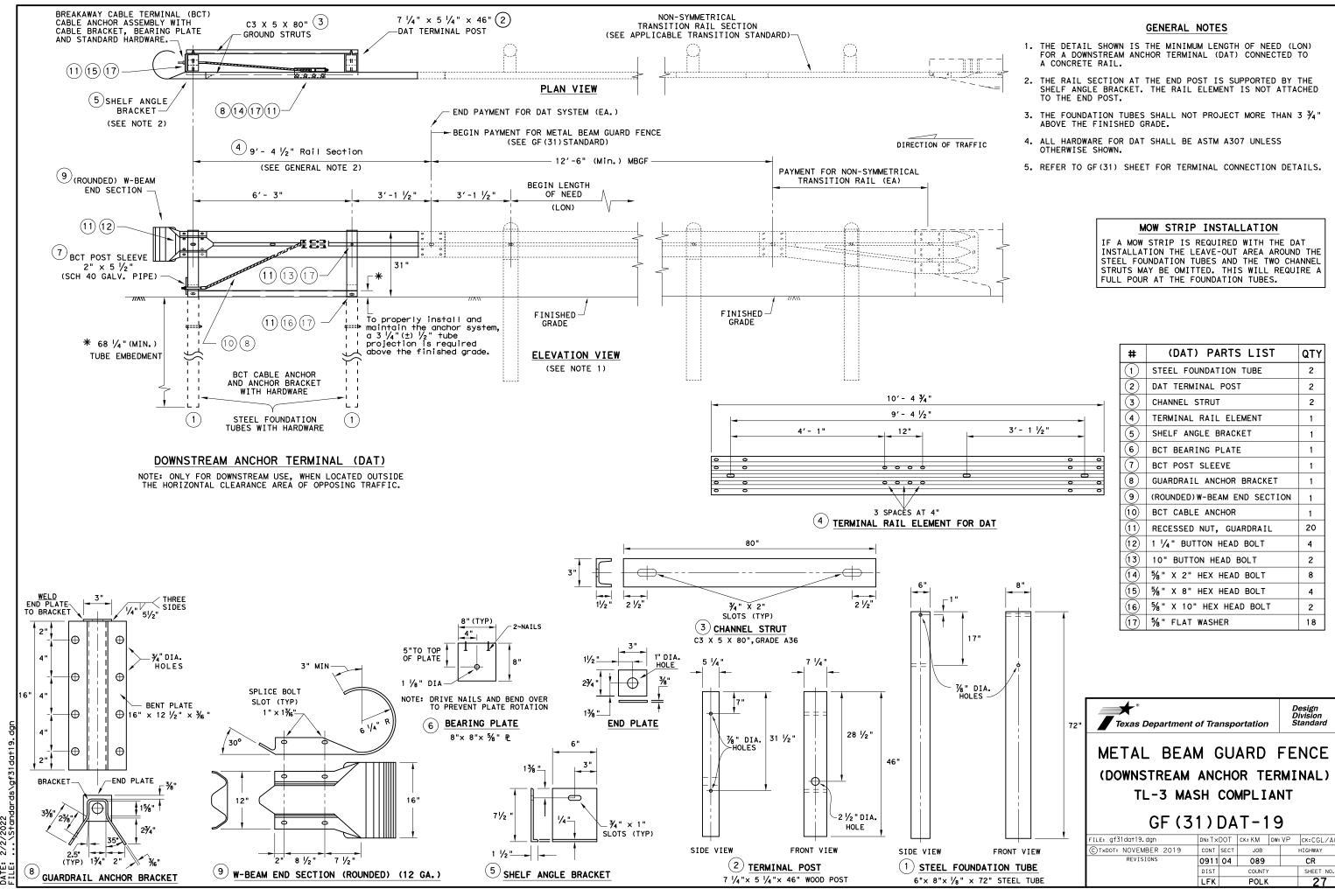
12. UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. THE CONSTRUCTION DIVISION, TXDOT MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210 ONLY PRODUCERS

13. FOR THE LOW FILL CULVERT OPTION, POSTS LOCATED PARTIALLY OR WHOLLY BETWEEN PRECAST BOX CULVERT UNITS, THE USE OF A CAST-IN-PLACE CONCRETE CLOSURE BETWEEN BOXES IS REQUIRED. THE LENGTH OF THE CAST-IN-PLACE CONCRETE CLOSURE SHALL ACCOMMODATE THE PLACEMENT OF THE LOW FILL CULVERT OPTION.

14. GUARDRAIL HEIGHT MEASUREMENT: WHEN THE GUARDRAIL IS LOCATED ABOVE PAVEMENT, MEASURE THE HEIGHT S FROM THE PAVEMENT TO THE TOP OF THE W-BEAM RAIL. WHEN THE GUARDRAIL IS LOCATED UP TO 2 FT. OFF OF THE EDGE OF PAVEMENT OR FOR A PAVEMENT OVERLAY, USE A 10-FOOT STRAIGHTEDGE TO EXTEND THE PAVEMENT/SHOULDER SLOPE TO THE BACK OF RAIL, MEASURE FROM THE BOTTOM OF STRAIGHTEDGE TO THE TOP OF RAIL. FOR GUARDRAIL LOCATED DOWN A 10:1 SLOPE, MEASURE FROM THE NOMINAL TERRAIN.

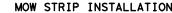
> NOTE: TRANSISTIONS TO BRIDGE RAILS OR TRAFFIC BARRIERS. SEE GF (31) TL3 TR STANDARD FOR HIGH-SPEED TL-3 TRANSITIONS. SEE GF (31) TL2 TR STANDARD FOR LOW-SPEED TL-2 TRANSITIONS.



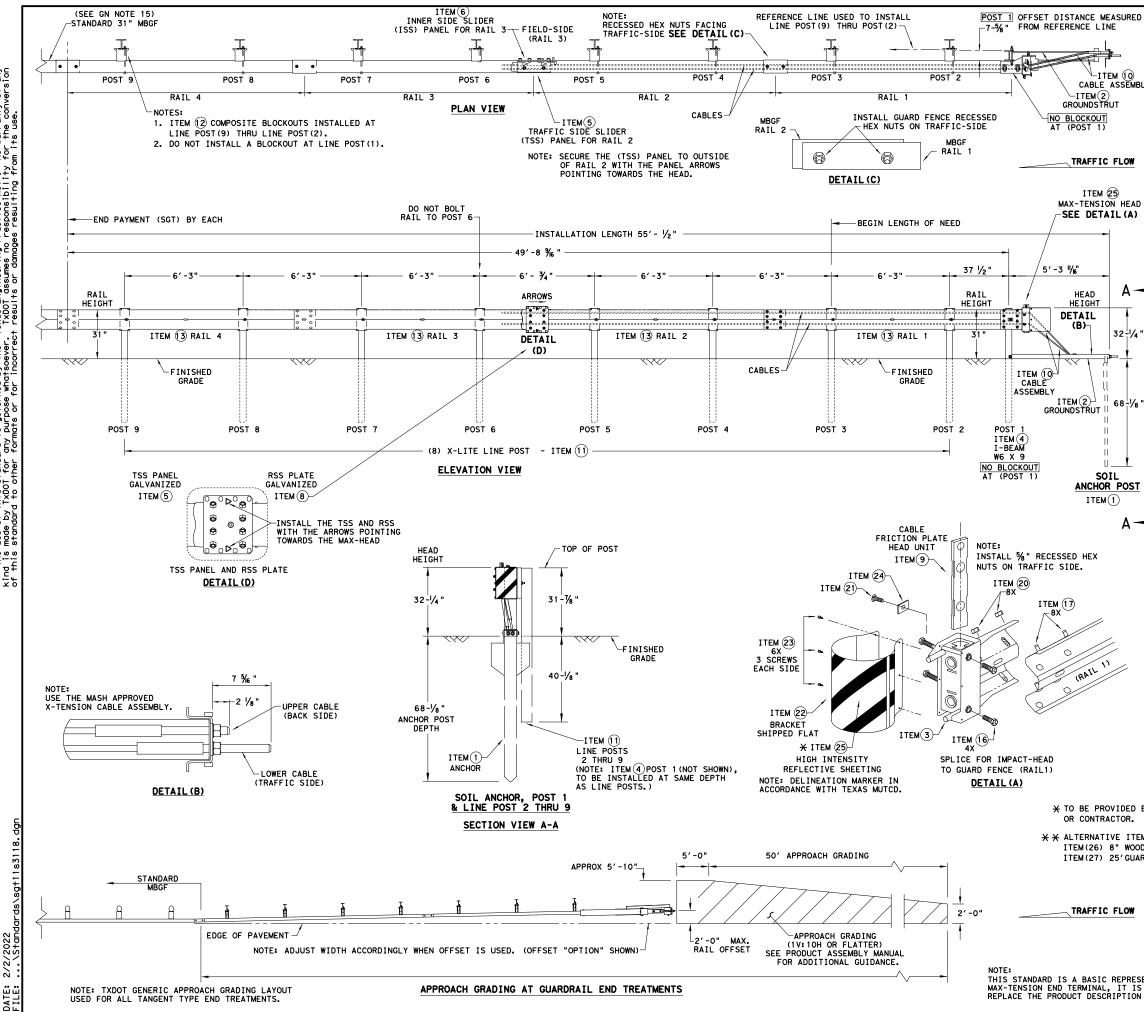


2/2/2022 DATE:

- 3. THE FOUNDATION TUBES SHALL NOT PROJECT MORE THAN 3 $\frac{3}{4}$ " Above the finished grade.
- 5. REFER TO GF(31) SHEET FOR TERMINAL CONNECTION DETAILS.



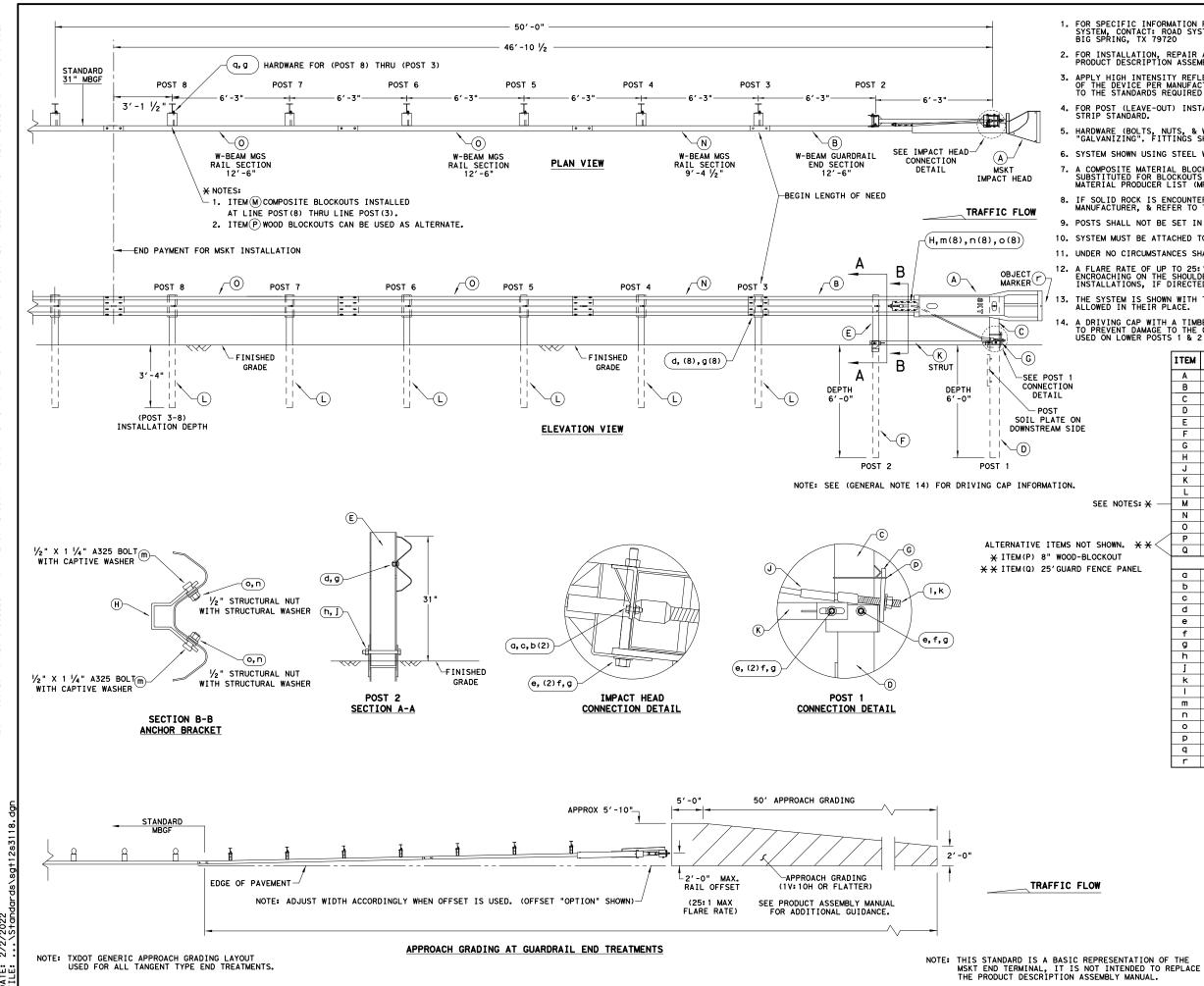
IF A MOW STRIP IS REQUIRED WITH THE DAT INSTALLATION THE LEAVE-OUT AREA AROUND THE STEEL FOUNDATION TUBES AND THE TWO CHANNEL STRUTS MAY BE OMITTED. THIS WILL REQUIRE A



DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TxDDT for any purpose whatsoever. TxDDT assumes no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use.

2/2/2022

URED					GENERAL NOTES	
	1.	FOR SPEC GUIDANCE (LTS) - B	IFIC IN OF TH ARRIER	NFORMATION E SYSTEM, SYSTEMS,	N REGARDING INSTALLATION AND TECHN CONTACT: LINDSAY TRANSPORTATION S INC. AT (707) 374-6800	ICAL OLUTIONS
10	2.	FOR INST INSTALLA	ALLATION I	ON, REPAII NSTRUCTIO	R, & MAINTENANCE REFER TO THE; MAX N MANUAL. P/N MANMAX REV D (ECN 35	-TENSION 16).
SEMBLY	3.	FRONT FA	CE OF	THE DEVIC	FLECTIVE SHEETING, "OBJECT MARKER" E PER MANUFACTURE'S RECOMMENDATION THE STANDARDS REQUIRED IN TEXAS M	S. OBJECT
	4.			E-OUT) IN RIP STAND	STALLATION AND GUIDANCE SEE TXDOT': ARD.	S LATEST
~	5.			ONENTS ARI SE STATED	E GALVANIZED PER ASTM A123 OR EQUI'	VALENT
LOW	6.				L WIDE FLANGE POST WITH COMPOSITE	BLOCKOUTS.
HEAD . (A)	7.	MAY BE S	UBSTIT	UTED FOR	KOUT THAT MEETS THE REQUIREMENTS O BLOCKOUTS SIMILAR DIMENSIONS. SEE CER LIST(MPL)FOR CERTIFIED PRODUCE	CONSTRUCTION
					ANUAL FOR SPECIFIC PANEL LAPPING G TERED SEE THE MANUFACTURER'S INSTA	
		MANUAL F	OR INS	TALLATION	GUIDANCE.	
					IN CONCRETE. IMBER OR PLASTIC INSERT SHALL BE US	SED WHEN
Α-		DRIVING	POST	TO PREVEN	T DAMAGE TO THE GALVANIZING ON TOP LL NEVER BE INSTALLED WITHIN A CUR ¹	OF THE POST.
		OF GUAR	DRAIL.			
2 - 1/4 "	13.	IF A DE WITH TE			R IS REQUIRED, MARKER SHALL BE IN A	ACCORDANCE
+	14.	THE SYS			TH 12'-6" MBGF PANELS, 25'-0" MBGF	PANELS
8-1⁄8"	15.			12'-6" OF NSION SYS	12GA. MBGF IS REQUIRED IMMEDIATEL' TEM.	Y DOWNSTREAM
		ITEM#	PART	NUMBER	DESCRIPTION	QTY
		1	BSI-16	610060-00	SOIL ANCHOR - GALVANIZED	1
		2	-	610061-00	GROUND STRUT - GALVANIZED	1
-		3		610062-00 610063-00	MAX-TENSION IMPACT HEAD W6x9 I-BEAM POST 6FTGALVANIZED	1
POST		5	-	610064-00	TSS PANEL - TRAFFIC SIDE SLIDER	1
		6	BSI-16	610065-00	ISS PANEL - INNER SIDE SLIDER	1
۸ <u> </u>		7	BSI-16	610066-00	TOOTH - GEOMET	1
A -		8		610067-00	RSS PLATE - REAR SIDE SLIDER	1
		9	B06105	58 610069-00	CABLE FRICTION PLATE - HEAD UNIT CABLE ASSEMBLY - MASH X-TENSION	2
		11		012078-00	X-LITE LINE POST-GALVANIZED	8
		12	B09053		8" W-BEAM COMPOSITE-BLOCKOUT XT110	8
		13		004386	12'-6" W-BEAM GUARD FENCE PANELS 1	2GA. 4
		14		102027-00	X-LITE SQUARE WASHER	
		15	-	001886 001885	% " X 7" THREAD BOLT HH (GR.5)GEOM 3/4" X 3" ALL-THREAD BOLT HH (GR.5)	
		10	400111		5/8" X 1 1/4" GUARD FENCE BOLTS (GR. 2	
		18	200184		5% X 10" GUARD FENCE BOLTS MGAL	8
/		19	200163	36	% " WASHER F436 STRUCTURAL MGAL	2
		20	400111		5% " RECESSED GUARD FENCE NUT (GR. 2	
		21	BSI-20		% " X 2" ALL THREAD BOLT (GR.5)GEO DELINEATION MOUNTING (BRACKET)	VET 1
		22	BSI-10	701063-00	1/4" X 3/4" SCREW SD HH 410SS	7
		24	400205		GUARDRAIL WASHER RECT AASHTO FWR03	1
	×	-		DTE BELOW	HIGH INTENSITY REFLECTIVE SHEETING	
×	÷×	26	400233 BSI-40		8" W-BEAM TIMBER-BLOCKOUT, PDB01B 25' W-BEAM GUARDRAIL PANEL,8-SPACE	8 12GA. 2
		28		(Rev-(D)	MAX-TENSION INSTALLATION INSTRUCTI	
					4	
	DI	STRIBUTOR	8		F [®]	Design Division
OR.				Тез	xas Department of Transportation	Standard
		T SHOWN. CKOUTS				
		NCE PANEL	.s		-TENSTAN END TED	
					-TENSION END TER	
					MASH - TL-3	
LOW						
					SGT (11S) 31-18	
				-		TXDOT CK: CL
		ION OF TH			FEBRUARY 2018 CONT SECT JOB REVISIONS 0911 04 089	HIGHWAY CR
		INTENDED WBLY MANU			DIST COUNTY	SHEET NO.
					LFK POLK	28



ATSOEV USE. ITS FOR ANY PURPOSE RESULTING FROM MADE BY TXDOT TS OR DAMAGES OF ANY KIND IS INCORRECT RESUL - NO WARRANTY FORMATS OR FOR THE "TEXAS ENGINEERING PRACTICE ACT" CONVERSIONOF THIS STANDARD TO OTHER ₽Ħ DISCLAIMER: THE USE OF THIS STANDARD IS GOVERNED TXDOT ASSUMES NO RESPONSIBILITY FOR 1

> /2022 2/2/ DATE:

1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: ROAD SYSTEMS, INC. (432)263-2435. 3616 OLD HOWARD COUNTY AIRPORT, BIG SPRING, TX 79720

FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE: MSKT END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL (PUBLICATION~062717).

3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.

4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

5. HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POSTS WITH COMPOSITE BLOCKOUTS.

7. A COMPOSITE MATERIAL BLOCKOUTS THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

8. IF SOLID ROCK IS ENCOUNTERED IN THE AREA OF (POST 1) AND / OR (POST 2) CONTACT THE MANUFACTURER, & REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE 9. POSTS SHALL NOT BE SET IN CONCRETE.

10. SYSTEM MUST BE ATTACHED TO STANDARD 31" MBGF.

11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE MSKT SYSTEM BE CURVED.

12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.

13. THE SYSTEM IS SHOWN WITH TWO 12'-6" MBGF PANELS, ONE 25'-0" MBGF PANEL IS ALSO ALLOWED IN THEIR PLACE.

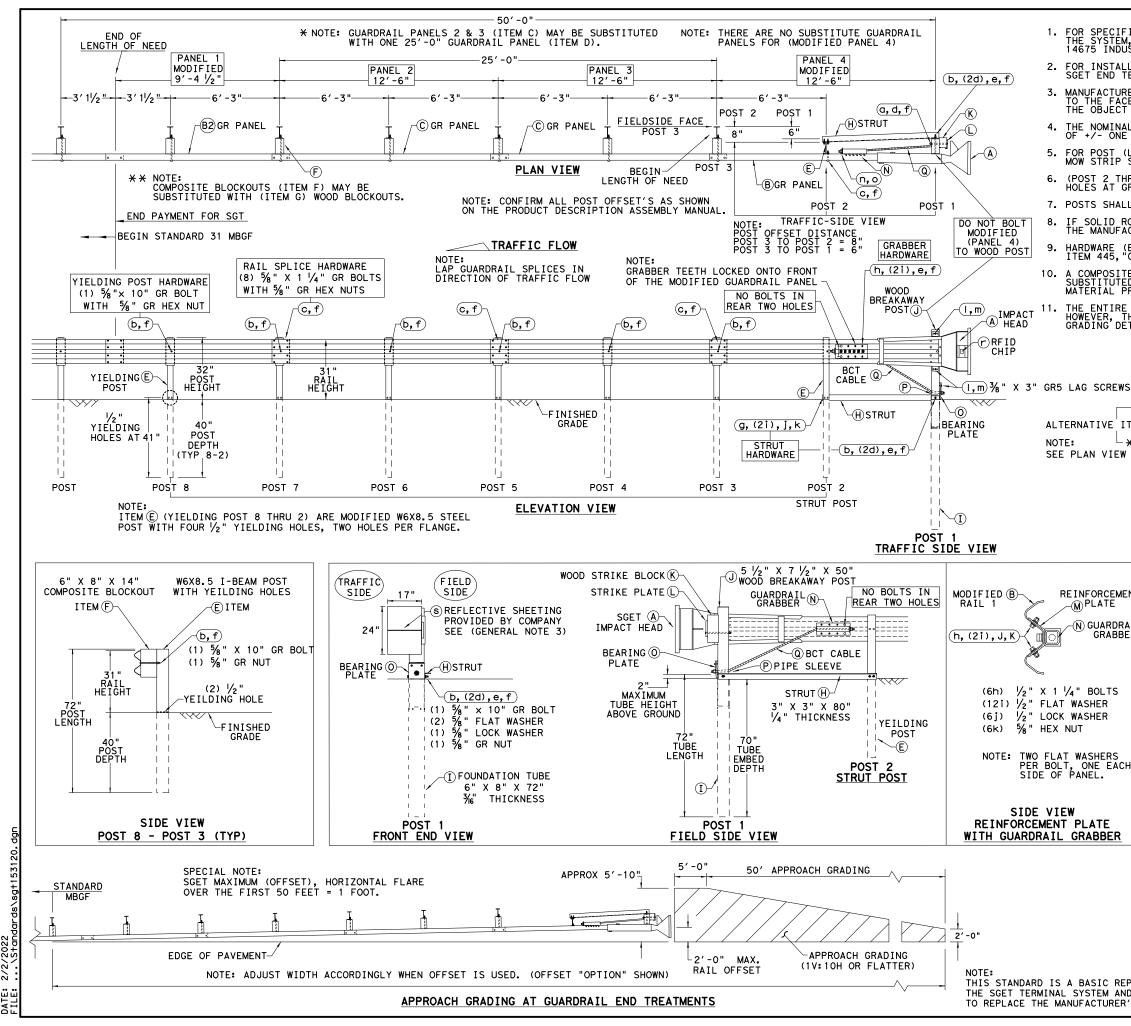
A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POSTS 3-8 TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST. SPECIAL DRIVING CAP TO BE USED ON LOWER POSTS 1 & 2 TO PREVENT DAMAGE TO THE WELDED PLATES.

-	Α	1		
F		1	MSKT IMPACT HEAD	MS3000
	в	1	W-BEAM GUARDRAIL END SECTION, 12 Ga.	SF1303
	С	1	POST 1 - TOP (6" X 6" X 1/8" TUBE)	MTPHP1A
	D	1	POST 1 - BOTTOM (6' W6X15)	MTPHP1B
	Е	1	POST 2 - ASSEMBLY TOP	UHP2A
	F	1	POST 2 - ASSEMBLY BOTTOM (6' W6X9)	HP2B
	G	1	BEARING PLATE	E750
	н	1	CABLE ANCHOR BOX	S760
	J	1	BCT CABLE ANCHOR ASSEMBLY	E770
Γ	к	1	GROUND STRUT	MS785
	L	6	W6x9 OR W6x8.5 STEEL POST	P621
NOTES: 🗙 —	М	6	COMPOSITE BLOCKOUTS	CBSP-14
	N	1	W-BEAM MGS RAIL SECTION (9'-4 1/2")	G12025
	0	2	W-BEAM MGS RAIL SECTION (12'-6")	G1203A
<u>, yy</u>	P	6	WOOD BLOCKOUT 6" X 8" X 14"	P675
₩N. **<	Q	1	W-BEAM MGS RAIL SECTION (25'-0")	G1209
JT PANEL			SMALL HARDWARE	
FANEL	a	2	‰ " × 1 " HEX BOLT (GRD 5)	B5160104A
	b	4	% "WASHER	W0516
	С	2	% "HEX NUT	N0516
Γ	d	25	5% " Dia. x 1 ¼ " SPLICE BOLT (POST 2)	B580122
Γ	е	2	5%∥ Dia. × 9″ HEX BOLT (GRD A449)	B580904A
	f	3	% " WASHER	W050
	g	33	‰" Dia. H.G.R NUT	N050
	h	1	¾" Dia. × 8 ½" HEX BOLT (GRD A449)	B340854A
	J	1	¾" Dia. HEX NUT	N030
E E E	k	2	1 ANCHOR CABLE HEX NUT	N100
F	1	2	1 ANCHOR CABLE WASHER	W100
F	m	8	1/2" × 1 1/4" A325 BOLT WITH CAPTIVE WASHER	SB12A
F	n	8	1/2" STRUCTURAL NUTS	N012A
F	0	8	1 1/16 " O.D. × 1/16 " I.D. STRUCTURAL WASHERS	W012A
ľ	р	1	BEARING PLATE RETAINER TIE	CT-100ST
F	q	6	5% " × 10" H.G.R. BOLT	B581002
F	r	1	OBJECT MARKER 18" X 18"	E3151
-				
		Γ	Texas Department of Transportation	Design Division Standard

SINGLE GUARDRAIL TERMINAL MSKT-MASH-TL-3

SGT (12S) 31-18

	LFK		POLK			29
	DIST		COUNTY	,	1	SHEET NO.
REVISIONS	0911	04	089		CR	
C TxDOT: APRIL 2018	CONT	SECT	JOB		H	IGHWAY
FILE: sg+12s3118.dgn	DN:T×	DOT	ск:км	DW:	VP	CK:CL



2/2/2022 DATE: FIIF:

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1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202

2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL. PRODUCT DESCRIPTION ASSEMBLY MANUAL.

MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER' TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD. 4. THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.

5. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

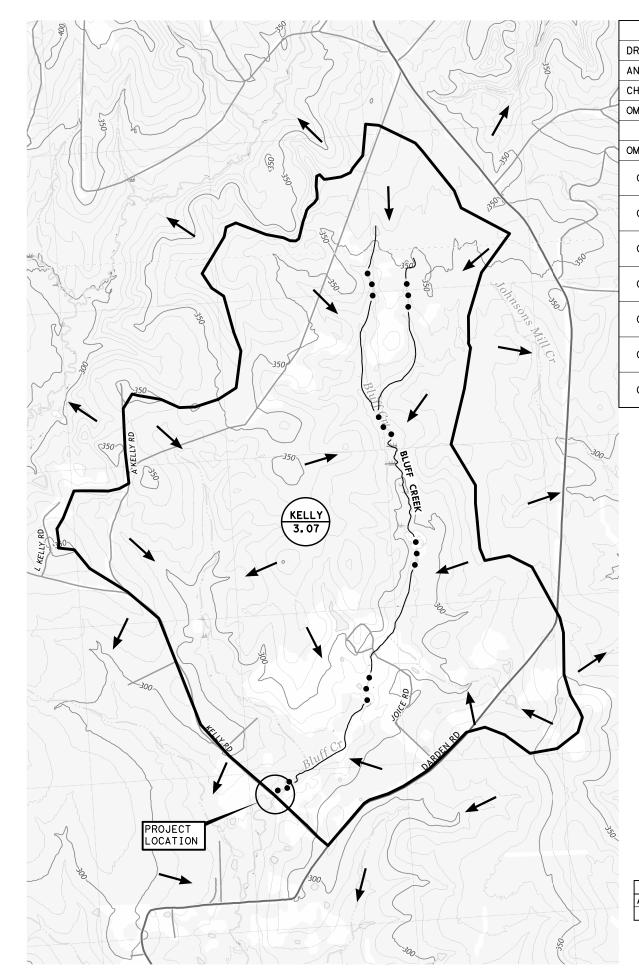
6. (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS. 7. POSTS SHALL NOT BE SET IN CONCRETE.

IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.

HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.

	ITEM	QTY	MAIN SYSTEM COMPONENTS	ITEM #
	Α	1	SGET IMPACT HEAD	SIH1A
	В	1	MODIFIED GUARDRAIL PANEL 12'-6" 12GA	126SPZGP
s	B2	1	MODIFIED GUARDRAIL PANEL 9'-4 1/2" 12GA	GP94
	С	2	STANDARD GUARDRAIL PANEL 12'-6" 12GA	GP126
- X -	D	1	STANDARD GUARDRAIL PANEL 25'-0" 12GA	GP25
тгис	E	7	MODIFIED YIELDING I-BEAM POST W6×8.5	YP6MOD
TEMS	F	6	COMPOSITE BLOCKOUT 6" X 8" X 14"	CB08
* * -	G	6	WOOD BLOCKOUT 6" X 8" X 14"	WBO8
	Н	1	STRUT 3" X 3" X 80" × 1/4" A36 ANGLE	STR80
	I	1	FOUNDATION TUBE 6" X 8" X 72" × 3/6"	FNDT6
	J	1	WOOD BREAKAWAY POST 5 $\frac{1}{2}$ " x 7 $\frac{1}{2}$ " x 50"	WBRK50
	K	1	WOOD STRIKE BLOCK	WSBLK14
	L	1	STRIKE PLATE 1/4" A36 BENT PLATE	SPLT8
	м	1	REINFORCEMENT PLATE 12 GA. GR55	REPLT17
	N	1	GUARDRATI GRABBER 2 1/2" X 2 1/2" X 16 1/2"	GGR17
	0	1	BEARING PLATE 8" X 8 $\frac{5}{8}$ " X $\frac{5}{8}$ " A36 PIPE SLEEVE 4 $\frac{1}{4}$ " X 2 $\frac{3}{8}$ " O.D. (2 $\frac{1}{8}$ " I.D.)	BPLT8
	P	1	PIPE SLEEVE 4 $\frac{1}{4}$ × 2 $\frac{3}{8}$ 0 D (2 $\frac{1}{6}$ L D)	PSLV4
	Q	1	BCT CABLE 3/4" X 81" LENGTH	CBL81
	u.		SMALL HARDWARE	CDLOT
	-	1	% X 12" GUARDRAIL BOLT 307A HDG	4.0000LT
NT	a 5			12GRBLT
	Ь	7	5/8" X 10" GUARDRAIL BOLT 307A HDG	10GRBLT
	c	33	5/8" X 1 1/4" GR SPLICE BOLTS 307A HDG	1GRBLT
<u>IL</u>	d	3	5% " FLAT WASHER F436 A325 HDG	58FW436
R	е	1	% LOCK WASHER HDG	58LW
	f	39	% " GUARDRAIL HEX NUT HDG	58HN563
	g	2	1/2" X 2" STRUT BOLT A325 HDG	2BLT
	h	6	1/2" X 1 1/4" PLATE BOLT A325 HDG	125BLT
	i	16	$\frac{1}{2}$ " FLAT WASHER F436 A325 HDG	12FWF436
	j	8	$\frac{1}{2}$ " LOCK WASHER HDG	12LW
	k	8	1/2" HEX NUT A563 HDG	12HN563
		4	⅔" X 3" HEX LAG SCREW GR5 HDG	38LS
	m	4	⅔" FLAT WASHER F436 A325 HDG	38FW844
	n	2	1" FLAT WASHER F436 A325 HDG	1FWF436
	0	2	1" HEX NUT A563DH HDG	1 HN563
H	р	1	18" TO 24" LONG ZIP TIE RATED 175-200LB	ZPT18
	q	1	1 1⁄2" X 4" SCH-40 PVC PIPE	PSPCR4
	r	1	RFID CHIP RATED MIL-STD-810F	RFID810F
	s	1	IMPACT HEAD REFLECTIVE SHEETING	RS30M
			Texas Department of Transportation	Design Division Standard
			SPIG INDUSTRY, LI SINGLE GUARDRAIL TER	
			SGET - TL-3 - MAS	SH
			SGT (15) 31-20	
			FILE: sg+153120. dgn DN: TxDOT CK: KM DW: V	
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	SSEMBL	Y MA	NUAL.	30



OMEGA EM REGRESSION EQUATION PARAMETERS (DES	IGN METHOD)
DRAINAGE AREA (A)	3.07 SQ MI
ANNUAL PRECIPITATION (P)	52.00 INCHES
CHANNEL SLOPE (S)	0.0063 FT/FT
DMEGA EM (Ω)	-0.253
DMEGA EM REGRESSION EQUATIONS	PEAK DISCHARGE (CFS)
$Q_2 = P^{1.398}S^{0.270} \times 10^{0.776}\Omega + 50.98 - 50.30A^{-0.0058}$	411.4
$Q_5 = P^{1.308}S^{0.372} \times 10^{0.885}\Omega + 16.62 - 15.32A^{-0.0215}$	735.6
$Q_{10} = P^{1.203}S^{0.403} \times 10^{0.918}\Omega + 13.62 - 11.97A^{-0.0289}$	947.6
$Q_{25} = P^{1.140}S^{0.446} \times 10^{0.945}\Omega + 11.79 - 9.819A^{-0.0374}$	1287.2
$Q_{50} = P^{1.105}S^{0.476} \times 10^{0.961}\Omega + 11.17 - 8.997A^{-0.0424}$	1570.3
$Q_{100} = P^{1.071}S^{0.507} \times 10^{0.969}\Omega + 10.82 - 8.448A^{-0.0467}$	1903.9
Q ₅₀₀ = P ^{0.988} S ^{0.569} × 10 ^{0.976} Ω + 10.40 - 7.605A ^{-0.0554}	2812.2

HYDROLOGIC COMPUTATIONS (CHECK METHOD)

HYDROLOGIC CHECK METHOD: SCS UNIT HYDROGRAPH DRAINAGE AREA: 3.07 SQ MI

DESIGN FREQUENCY: MEET OR EXCEED EXISTING, 100-YR CHECK SUMMARY OF SOIL CONDITIONS AND LAND USE:

CONDITIONS OF THE WATERSHED CONSISTED OF RURAL UNDEVELOPED WOODLANDS

AND MEADOWS. THE SOIL CONDITIONS WITHIN THE WATERSHED CONSISTED OF PREDOMINATELY SOIL TYPE B. SOILS DATA WAS OBTAINED FROM THE NRCS WEB SOIL SURVEY.

SUMMARY OF INPUT PARAMETERS:

DRAINAGE AREA -- LAND USE REMAINED CONSISTENT THROUGHOUT THE BASIN. THE 3.07 SQ MI WATERSHED WAS ANALYZED AS A SINGLE BASIN WITH A COMPOSITE RCN VALUE OF 58. THE TIME OF CONCENTRATION WAS CALCULATED USING THE KIRBY-KIRPICH METHOD. THE CALCULATED TIME OF CONCENTRATION AT THE STREAM CROSSING IS 2.21 HOURS.

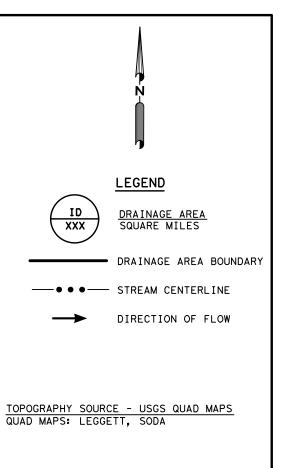
AN RCN ADJUSTMENT WAS NOT NECESSARY BASED ON THE TXDOT HYDRAULIC DESIGN MANUAL, FIGURE 4-20.

PRECIPITATION -- THE STORM FREQUENCIES ANALYZED INCLUDE THE 2, 5, 10, 25, 50, AND 100-YEAR RETURN PERIOD STORM EVENTS. FREQUENCY STORM RAINFALL DEPTHS ARE INPUT FOR EACH OF THESE RETURN PERIODS FOR A 24 HOUR EVENT. THE RAINFALL DEPTH-DURATION FREQUENCY DATA WAS TAKEN FROM NOAA'S PRECIPITATION FREQUENCY DATA SERVER (ATLAS 14) FOR 5 MIN, 15 MIN, 1 HR, 2 HR, 3 HR, 6 HR, 12 HR, AND 24 HR DURATION. THE STORM DISTRIBUTION USED WAS A BALANCED STORM.

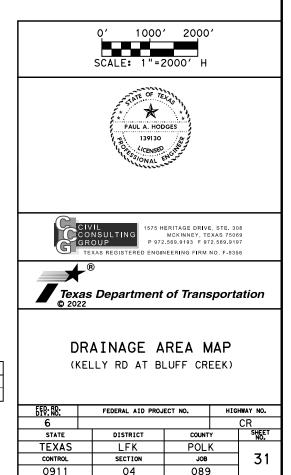
DACIN	AREA	Тс	LAG TIME	BASE	IMP
BASIN	(SQ MI)	(HR)	(MIN)	RCN	
KELLY	3.07	2.21	79.5	58	

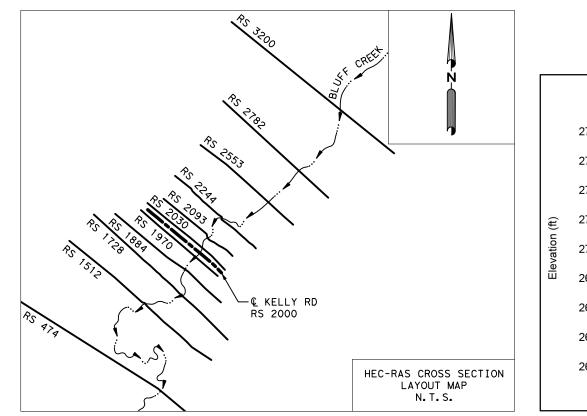
RUNOFF DISCHARGES AT THE CROSSING WERE CALCULATED FROM THE INPUT PARAMETERS USING HEC-HMS (VER. 4.3).

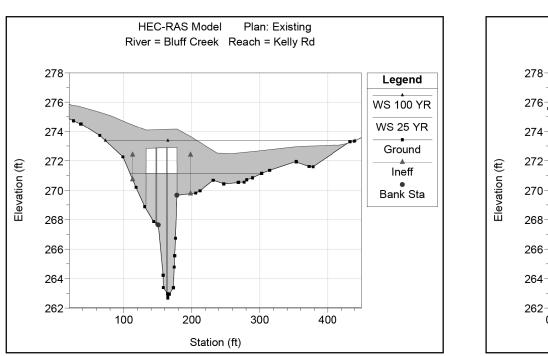
SUMMARY OF DISCHARGES						
ANNUAL RECURRENCE INTERVAL (ARI)	2-YR	5-YR	10-YR	25-YR	50-YR	100-YR
Q (CFS)	271.7	645.4	1005.3	1569.3	2059.0	2607.0



PERVIOUS (%) 1%







NOTES:

- 1. PROPOSED BRIDGE IS LOCATED AT HEC-RAS STA 2000. UPSTREAM CROSS SECTION IS AT HEC-RAS STA 2030 & DOWNSTREAM CROSS SECTION IS AT HEC-RAS STA 1970.
- 2. PROPOSED OVERALL BRIDGE WIDTH IS 26 FEET.
- 3. PROPOSED BRIDGE LENGTH IS 75 FEET.
- EXISTING BRIDGE STRUCTURE CONVEYS THE CALCULATED 50-YR ARI STORM FREQUENCY.

HYDRAULIC METHOD:

WATER SURFACE ELEVATIONS COMPUTED USING A HEC-RAS (V.5.0.7) MODEL CREATED FOR BLUFF CREEK, FILE: KELLYHECRAS.PRJ. THE HEC-RAS MODEL WAS DEVELOPED USING FIELD SURVEY, TNRIS LIDAR AND PROPOSED BRIDGE GEOMETRY.

EXISTING CONDITION WATER SURFACE ELEVATIONS COMPUTED FROM HEC-RAS MODEL PLAN "EXISTING".

PROPOSED CONDITION WATER SURFACE ELEVATIONS COMPUTED FROM HEC-RAS MODEL PLAN "PROPOSED".

BOUNDARY CONDITIONS BASED ON NORMAL DEPTH WITH A SLOPE OF 0.0063 FT/FT.

FLOOD HAZARD AREA:

BLUFF CREEK IS IDENTIFIED ON FEMA FIRM PANEL 48373C0500C, DATED SEPTEMBER 3, 2010. KELLY RD CROSSES A FLOOD HAZARD AREA WITH ZONE "X" DESIGNATION.

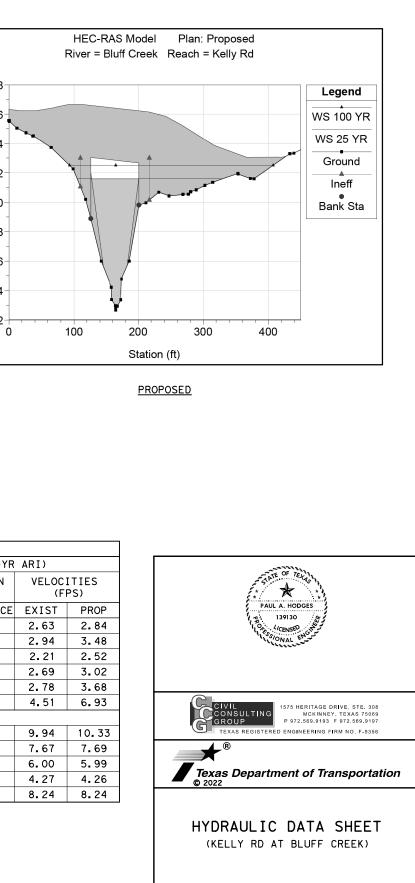
HOLLIE OLIVER THE LOCAL FLOODPLAIN ADMINISTRATOR, _

WAS NOTIFIED OF THE PROJECT ON _______.FINAL H&H REPORT WILL

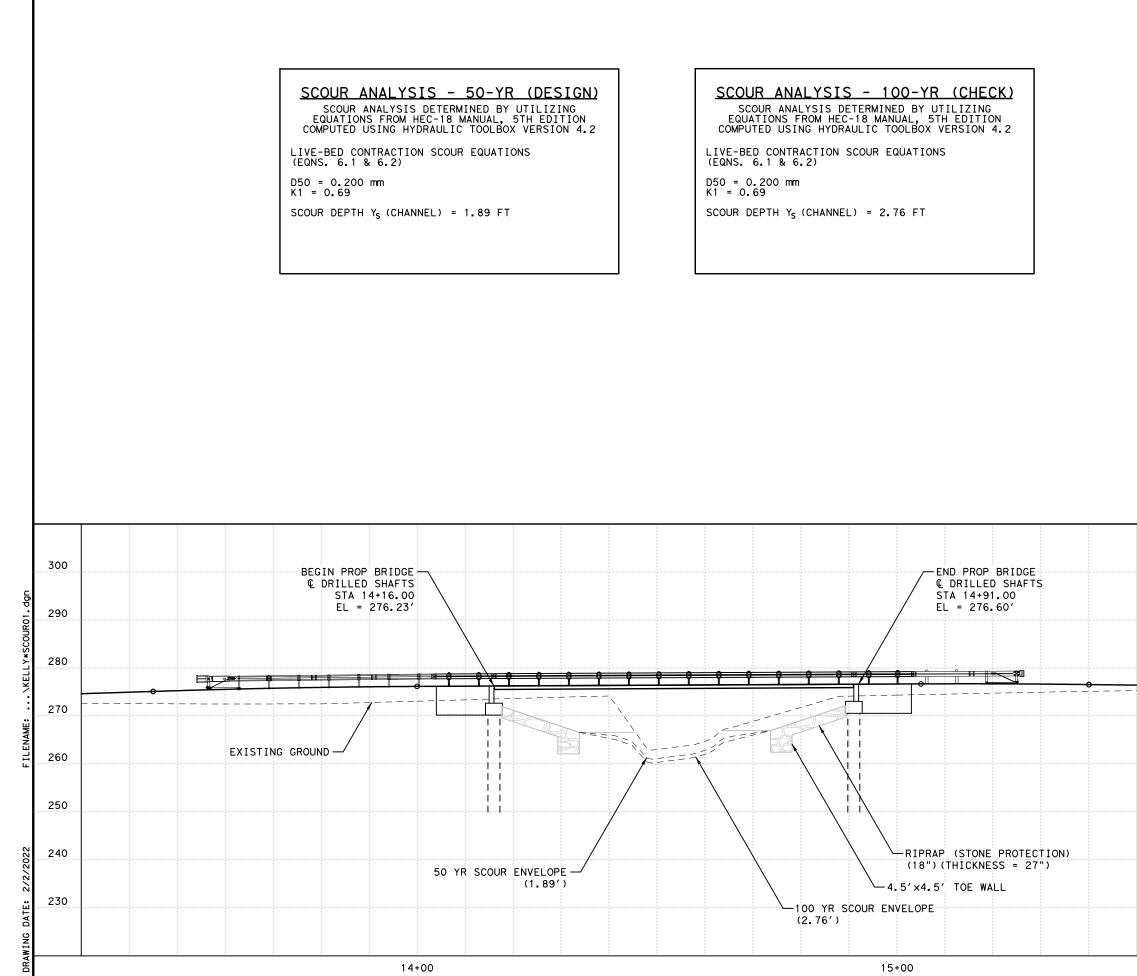
BE SUBMITTED TO FLOOD PLAIN ADMINISTRATOR WHEN DESIGN IS COMPLETED.

HEC-RAS SUMMARY TABLE DESIGN FREQUENCY (25-YR ARI) CHECK FREQUENCY (100-YR ARI) WATER SURFACE ELEVATIONS VELOCITIES WATER SURFACE ELEVATION STA FLOWS FLOWS (FT) (FPS) (FT) (CFS) (CFS) PROP DIFFERENCE EXIST PROP DIFFERENCE EXIST PROP EXIST 3200 1287.20 273.72 273.72 0.00 2.76 2.76 1903.90 274.34 274.21 -0.13 2782 1287.20 272.58 4.29 1903.90 273.84 272.56 -0.02 4.37 273.51 -0.33 2553 1287.20 272.35 272.32 -0.03 2.70 1903.90 273.75 273.37 -0.38 2.66 1287.20 272.17 272.13 -0.04 1903.90 273.65 273.23 -0.42 2244 2.92 3.01 1287.20 272.03 271.99 3.58 -0.49 2093 -0.04 3.56 1903.90 273.59 273.10 1287.20 271.48 271.47 1903.90 273.37 272.37 2030 -0.01 5.80 5.69 -1.00 KELLY RD @ BLUFF CREEK (HEC-RAS STA 2000) 1970 1287.20 270.94 270.94 0.00 7.44 7.68 1903.90 271.33 271.32 -0.01 1287.20 270.73 270.73 1903.90 271.09 271.08 1884 0.00 6.05 6.05 -0.01 1728 1287.20 270.54 270.54 4.77 4.78 1903.90 270.81 270.81 0.00 0.00 1512 1287.20 269.78 269.78 0.00 3.73 3.73 1903.90 270.15 270.15 0.00 1287.20 267.74 267.74 1903.90 268.14 268.14 474 0.00 7.70 7.70 0.00

EXISTING



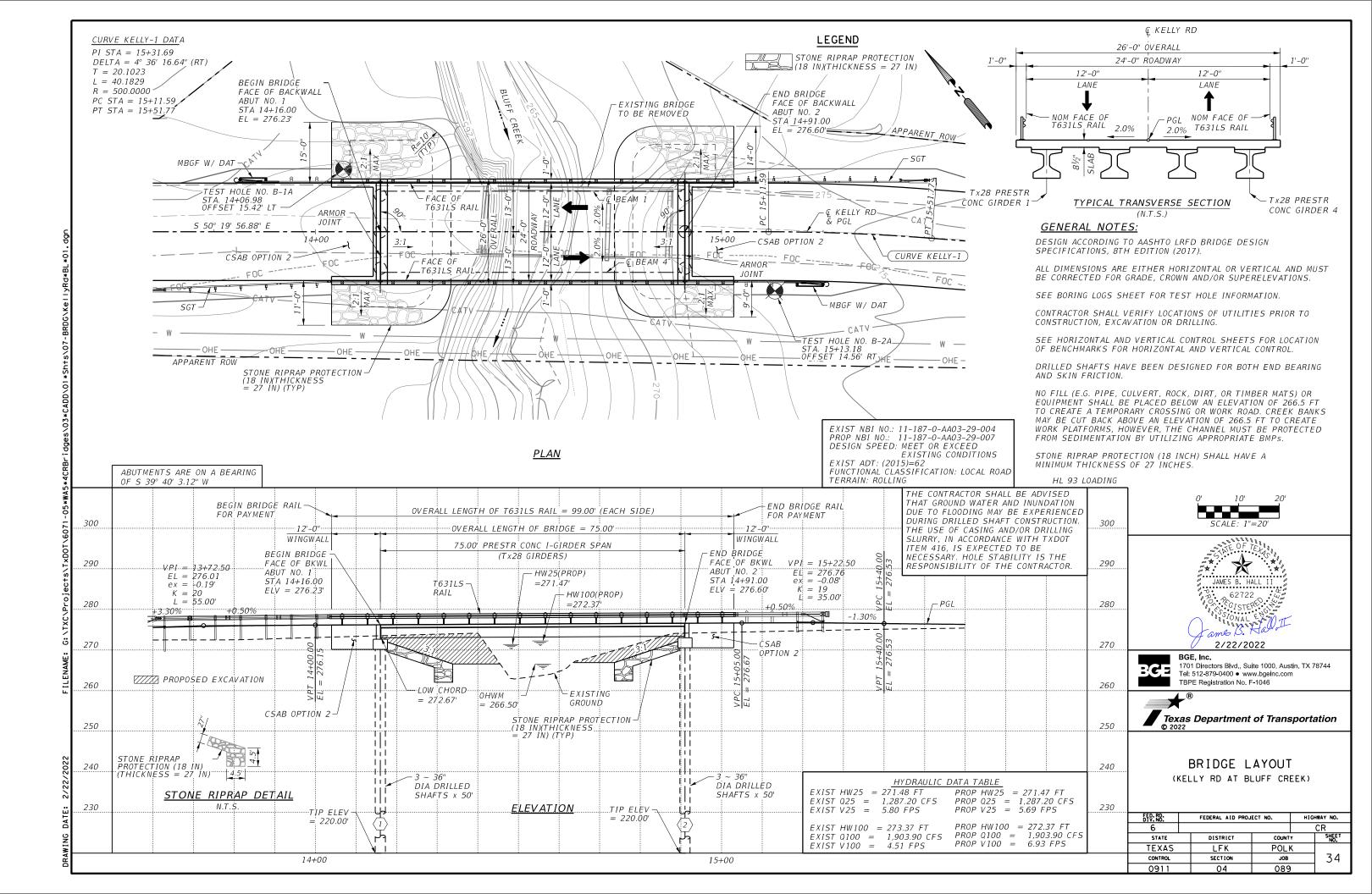
FED.RD. FEDERAL AID PROJECT NO			ECT NO.	HIG	WAY NO.
6					CR
STATE		DISTRICT	COUNTY		SHEET NO.
TEXAS		LFK	POLK		
CONTROL		SECTION	JOB		32
0911		04	089		

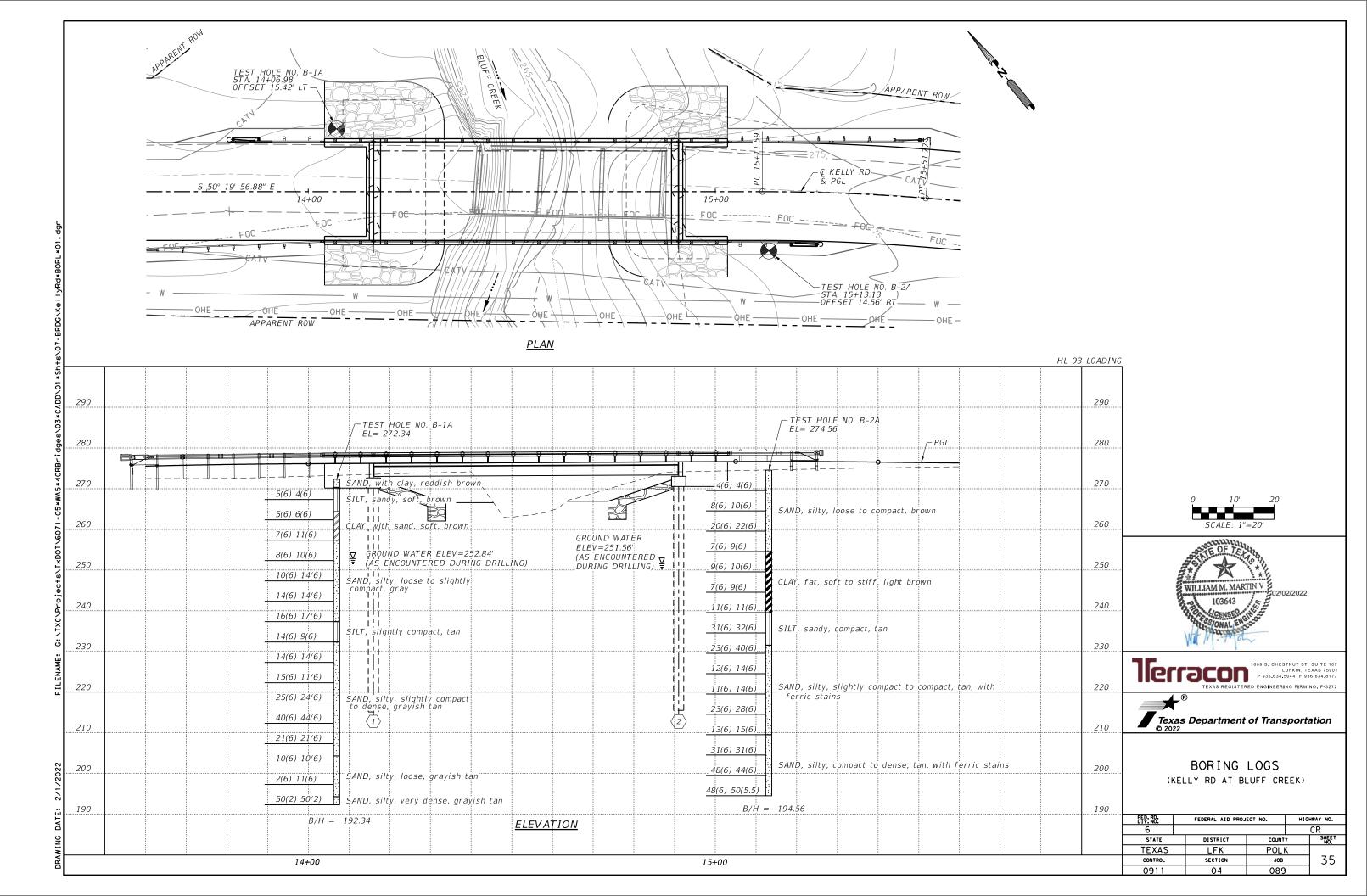


NOTES:

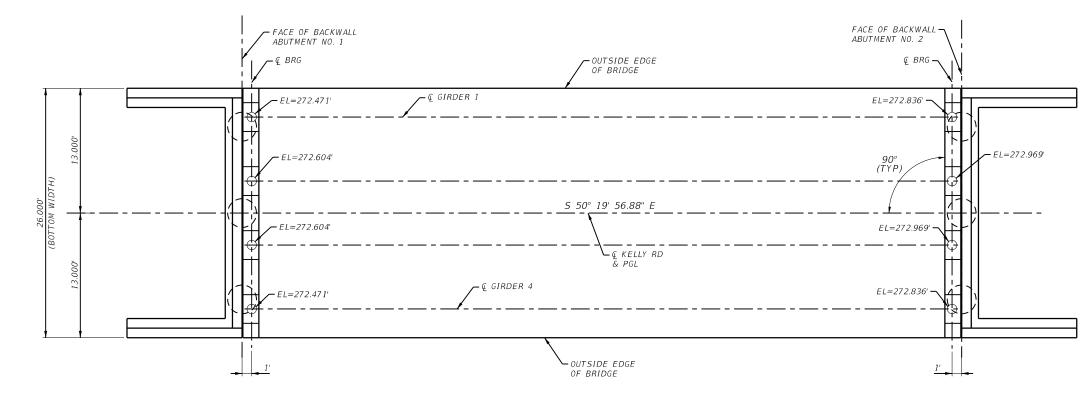
- 1. THERE IS NO EVIDENCE OF SCOUR FOR THE EXISTING BRIDGE.
- 2. ABUTMENTS TO BE PROTECTED WITH RIPRAP (STONE PROTECTION). ABUTMENT SCOUR IS NOT REQUIRED PER TXDOT GEOTECHNICAL MANUAL.
- 3. POLK COUNTY SHOULD REGULARLY INSPECT THE STONE RIPRAP PROTECTION TO ENSURE SLOPE STABILITY.

	300	0' 10' 20' SCALE: 1"=20' H "=20' H PAUL A. HODGES 139130 NAL PLOY					
	290						
	280						
	270						
	260	ISTS HERITAGE DRIVE, STE. 308 MCKINNEY, TEXAS 78069 972.569.9193 F 972.569.9197 TEXAS REGISTERED ENGINEERING FIRM NO. F.9356 Texas Department of Transportation © 2022 SCOUR PROFILE (KELLY RD AT BLUFF CREEK)					
	250						
	240						
	230						
		FED: RD:	FEDERA	FEDERAL AID PROJECT NO.			HWAY NO.
		6					CR
		state TEXAS		FK			SHEET NO.
		CONTROL			JOB	POLK Job	
			0911 04 089			33	
		0011		/ 1	003		

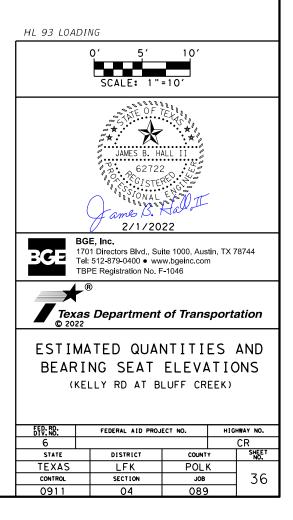


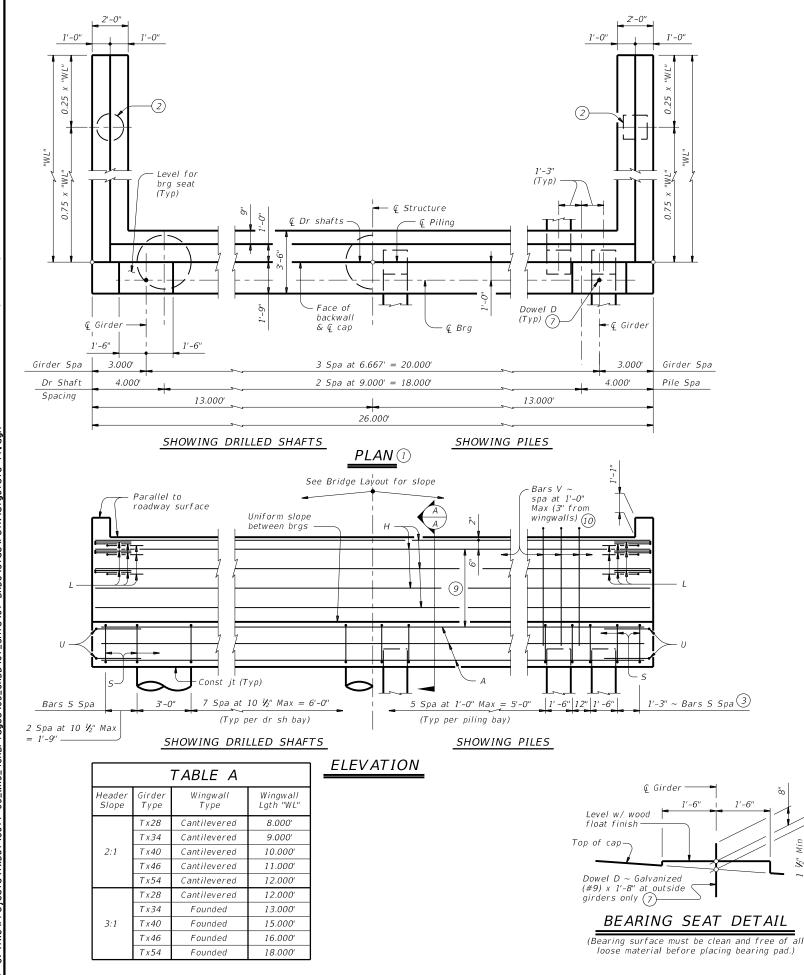


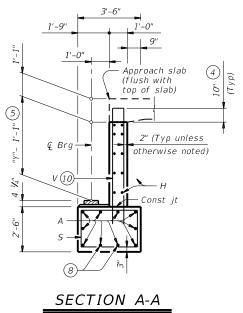
			SUMMARY	OF BRIDGE ITE	MS					
	400	416	420	422	422	425	432	450	454	496
	6005	6004	6013	6001	6023	6035	6033	6019	6003	6009
KELLY RD AT BLUFF CREEK NBI: 11-187-0-AA03-29-007	CEM STABIL BKFL	DRILL SHAFT (36 IN)	CL C CONC (ABUT)	REINF CONC SLAB	SHEAR KEY	PRESTR CONC GIRDER (TX28)	RIPRAP (STONE PROTECTION) (18 IN)	RAIL (TY T631LS)	ARMOR JOINT	REMOV STR (BRIDGE 0 - 99 FT LENGTH)
	СҮ	LF	СҮ	SF	СҮ	LF	СҮ	LF	LF	EA
2 ~ ABUTMENTS	81	300	37.6					48.0		
1 ~ 75' PRESTR CONC TX GIRDERS				1,950	0.4	298.00	304	150.0	44	
PROJECT TOTALS	81	300	37.6	1,950	0.4	298.00	304	198.0	44	1



BEARING SEAT ELEVATIONS







(With approach slab) (6)

See Table A for variable dimensions based on header slope and girder type.

- 2 See Table A to determine if wingwall foundations are required.
- ③ For piling larger than 16" adjust Bars S spacing as required to avoid piling.
- 4 Increase as required to maintain 3" from finished grade.
- 5 See Span details for "Y" value.
- 6 See Bridge Layout to determine if approach slab is present.

⑦ Omit Dowels D at end of multi-span unit. Adjust reinforcing steel total accordingly.

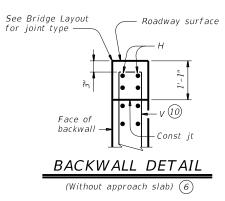
(8) With pile foundations, move Bars A shown to clear piles.

9 Spacing based on girder type: Tx28 ~ 3 spaces at 1'-0" Max $Tx34 \sim 3$ spaces at 1'-0" Max $Tx40 \sim 4$ spaces at 1'-0" Max $Tx46 \sim 4$ spaces at 1'-0" Max Tx54 ~ 5 spaces at 1'-0" Max

(10) Field bend as needed to clear piles.

TABLE OF FOUNDATION LOADS

Span Length	All Girde	er Types
Ft	Tons/Shaft	Tons/Pile
40	64	54
45	69	56
50	73	59
55	77	61
60	81	63
65	85	65
70	88	67
75	92	69
80	96	71
85	100	73
90	104	75
95	108	77
100	111	79
105	115	80
110	119	82
115	123	84
120	126	86
125	130	88



GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. See Bridge Layout for header slope and foundation

type, size and length. See Common Foundation Details (FD) standard sheet

for all foundation details and notes. See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment details, if applicable.

See applicable rail details for rail anchorage in wingwalls. These abutment details may be used with standard

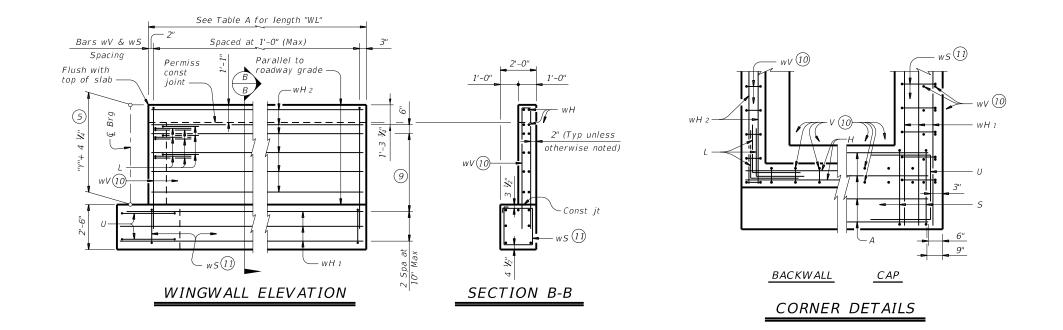
SIG-24 only.

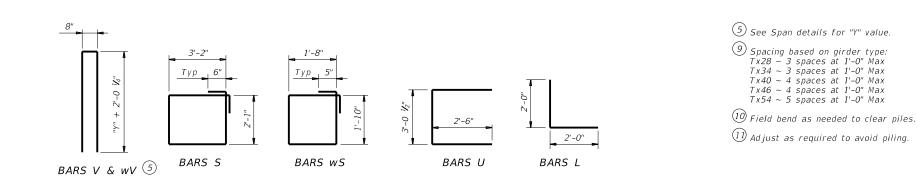
Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out f bar.

MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere in the plans. Provide Grade 60 reinforcing steel. Galvanize dowel bars D.

HL93 LOADING			SF	1EE	ET 1	OF 3
Texas Department	of Tra	nsp	ortation	,	D	ridge ivision tandard
ABU	ΤM	ΙE	NTS			
TYPE TX.	28	ΤН	RU T	⁻ X	54	
PRESTR C	ΟΝ	5 .	I-GIR	DI	ERS	5
24'	RO/	D	NAY			
		A	IG-2	24		
FILE: aig01sts-17.dgn	DN: TA	R	ск: КСМ	DW:	JTR	ск: TAR
©TxDOT August 2017	CONT	SECT	JOB			HIGHWAY
REVISIONS	0911	04	089			CR
	DIST		COUNTY			SHEET NO.
	LFK		POLK	:		37





HL93 LOADING			SHEI	ET 2	OF 3
Texas Department	of Tra	nsp	ortation	Di	idge vision andard
ABU	TM	IE I	NTS		
TYPE TX	28	ΤН	RU TX	54	
PRESTR C	ΟΝ	<u> </u>	I-GIRD	ERS	
24'	ROA	D	NAY		
		Д	IG-24	1	
FILE: aig01sts-17.dgn	DN: TA	R	CK: KCM DW.	JTR	ςκ: TAR
©TxDOT August 2017	CONT	SECT	JOB		HIGHWAY
REVISIONS	0911	04	089		CR
	DIST		COUNTY		SHEET NO.
	LFK		POLK		38

TABLES OF ESTIMATED QUANTITIES WITH 2:1 HEADER SLOPE 12

TYPE Tx28 Girders Bar No. Size Length Weight A 10 #11 25'-0" 1,328 D(7) 2 #9 1'-8" 11 H 8 #6 25'-8" 308 L 18 #6 4'-0" 108 S 22 #5 11'-6" 264 U 4 #6 8'-1" 49 V 25 #5 11'-4" 266 WH1 14 #6 9'-5" 198 WH2 20 #6 7'-8" 230 wF3 18 #4 7'-10" 94	A D(
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	A D(
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	D(
H 8 #6 25'-8" 308 L 18 #6 4'-0" 108 S 22 #5 11'-6" 264 U 4 #6 8'-1" 49 U 4 #6 8'-1" 49 V 25 #5 11'-4" 296 V 25 #5 11'-4" 296 WH1 14 #6 9'-5" 198 WH1 14 #6 10'-5" 219 WH2 20 #6 7'-8" 230 #H 8 #6 25'-8" 308 H 10 #6 25'-8" U 4 #6 4'-0" 108 L 18 #6 4'-0" V 25 #5 11'-6" 264 8'-1" 49 U 4 #6 8'-1" WH1 14 #6 10'-5" 219 WH1 14 #6 11'-5" WH2 20 #6 8'-8" 260 WH2 24 #6 9'-8" </td <td></td>	
L 18 #6 4'-0" 108 S 22 #5 11'-6" 264 V 4 #6 8'-1" 49 V 4 #6 8'-1" 49 V 25 #5 11'-4" 296 V 25 #5 11'-4" 296 V 25 #5 11'-4" 296 WH1 14 #6 9'-5" 198 WH1 14 #6 10'-5" 219 WH1 14 #6 9'-8" WH2 20 #6 7'-8" 230 #H2 20 #6 8'-8" 260 WH2 24 #6 9'-8"	11
S 22 #5 11'-6" 264 S 22 #5 11'-6" 264 U 4 #6 8'-1" 49 U 4 #6 8'-1" 49 V 25 #5 11'-4" 296 V 25 #5 11'-4" 322 wH1 14 #6 9'-5" 198 wH1 14 #6 10'-5" 219 wH1 14 #6 11'-5" wH2 20 #6 7'-8" 230 #H2 20 #6 8'-8" 260 wH2 24 #6 9'-8"	п
U 4 #6 8'-1" 49 V 25 #5 11'-4" 296 V 25 #5 11'-4" 296 WH1 14 #6 9'-5" 198 WH1 14 #6 10'-5" 219 WH1 14 #6 11'-5" WH2 20 #6 7'-8" 230 WH2 20 #6 8'-8" 260 WH2 24 #6 9'-8"	L
V 25 #5 11'-4" 296 V 25 #5 12'-4" 322 V 25 #5 13'-4" wH1 14 #6 9'-5" 198 wH1 14 #6 10'-5" 219 wH1 14 #6 11'-5" wH2 20 #6 7'-8" 230 wH2 20 #6 8'-8" 260 wH2 24 #6 9'-8"	5
wH1 14 #6 9'-5" 198 wH1 14 #6 10'-5" 219 wH1 14 #6 11'-5" wH2 20 #6 7'-8" 230 wH2 20 #6 8'-8" 260 wH1 14 #6 11'-5"	U
wH2 20 #6 7'-8" 230 wH2 20 #6 8'-8" 260 wH2 24 #6 9'-8"	V
	vH1
w5 18 #4 7'-10" 94 w5 20 #4 7'-10" 105 w5 22 #4 7'-10"	vH2
	wS
wV 18 #5 11'-4" 213 wV 20 #5 12'-4" 257 wV 22 #5 13'-4"	wV
Reinforcing Steel Lb 3,099 Reinforcing Steel Lb 3,231 Reinforcing Steel Lb	Reiı
Class "C" Concrete CY 15.2 Class "C" Concrete CY 16.6 Class "C" Concrete CY	Clas

TABLES OF ESTIMATED QUANTITIES WITH 3:1 HEADER SLOPE 12

Bar No. A 10 D(7) 2 H 8 L 18 S 22 U 4	Size #11 #9 #6 #6	Length 25'-0" 1'-8" 25'-8" 4'-0"	Weight 1,328 11 308	Bar A D(7)	No. 10 2	Size #11	Leng 25'-		Weight	Bar	No.	Size	Len	gth	Weight	Bar
D(7) 2 H 8 L 18 S 22	#9 #6 #6	1'-8" 25'-8"	11 308	D(7)			25'-	0"	4 3 3 6							
H 8 L 18 S 22	#6	25'-8"	308		2	".0		0	1,328	Α	10	#11	25'-	-0''	1,328	А
L 18 5 22	#6					#9	1'-8	3"	11	D(7)	2	#9	1'-	8"	11	D(7)
5 22		4'-0"		Н	8	#6	25'-	8"	308	H	10	#6	25'-	-8''	386	Н
			108	L	18	#6	4'-0)"	108	L	18	#6	4'-	0"	108	L
11 4	#5	11'-6"	264	5	22	#5	11'-	6"	264	5	22	#5	11'-	-6"	264	S
	#6	8'-1"	49	U	4	#6	8'-1	1"	49	U	4	#6	8'-	1"	49	U
V 25	#5	11'-4"	296	V	25	#5	12'-	4"	322	V	25	#5	13'-	-4"	348	V
wH1 14	#6	13'-5"	282	wH1	14	#6	14'	5"	303	wH1	14	#6	16'-	-5"	345	wH1
wH2 20	#6	11'-8"	350	wH2	20	#6	12'-	8"	381	wH2	24	#6	14'-	-8''	529	wH2
wS 26	#4	7'-10"	136	wS	28	#4	7'-1	0"	147	wS	32	#4	7'-1	0"	167	wS
wV 26	#5	11'-4"	307	wV	28	#5	12'-	4"	360	wV	32	#5	13'-	-4"	445	wV
Reinforcing :	Steel	Lb	3,439	Reinfo	orcing St	eel		Lb	3,581	Reinfo	rcing St	eel		Lb	3,980	Reinf
Class "C" Cor	ncrete	СҮ	17.8	Class	"C" Conc	rete		СҮ	19.3	Class	"C" Conc	rete		СҮ	21.7	Class

Omit Dowels D at end of multi-span unit. Adjust reinforcing steel total accordingly.

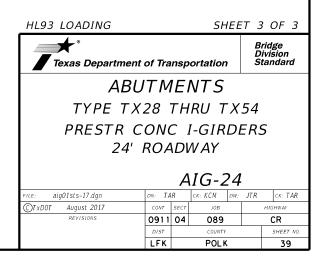
Quantities shown are for one abutment only (with approach slab). With no approach slab, add 1.0 CY Class "C" concrete and 154 lbs reinforcing steel for 4 additional Bars H.

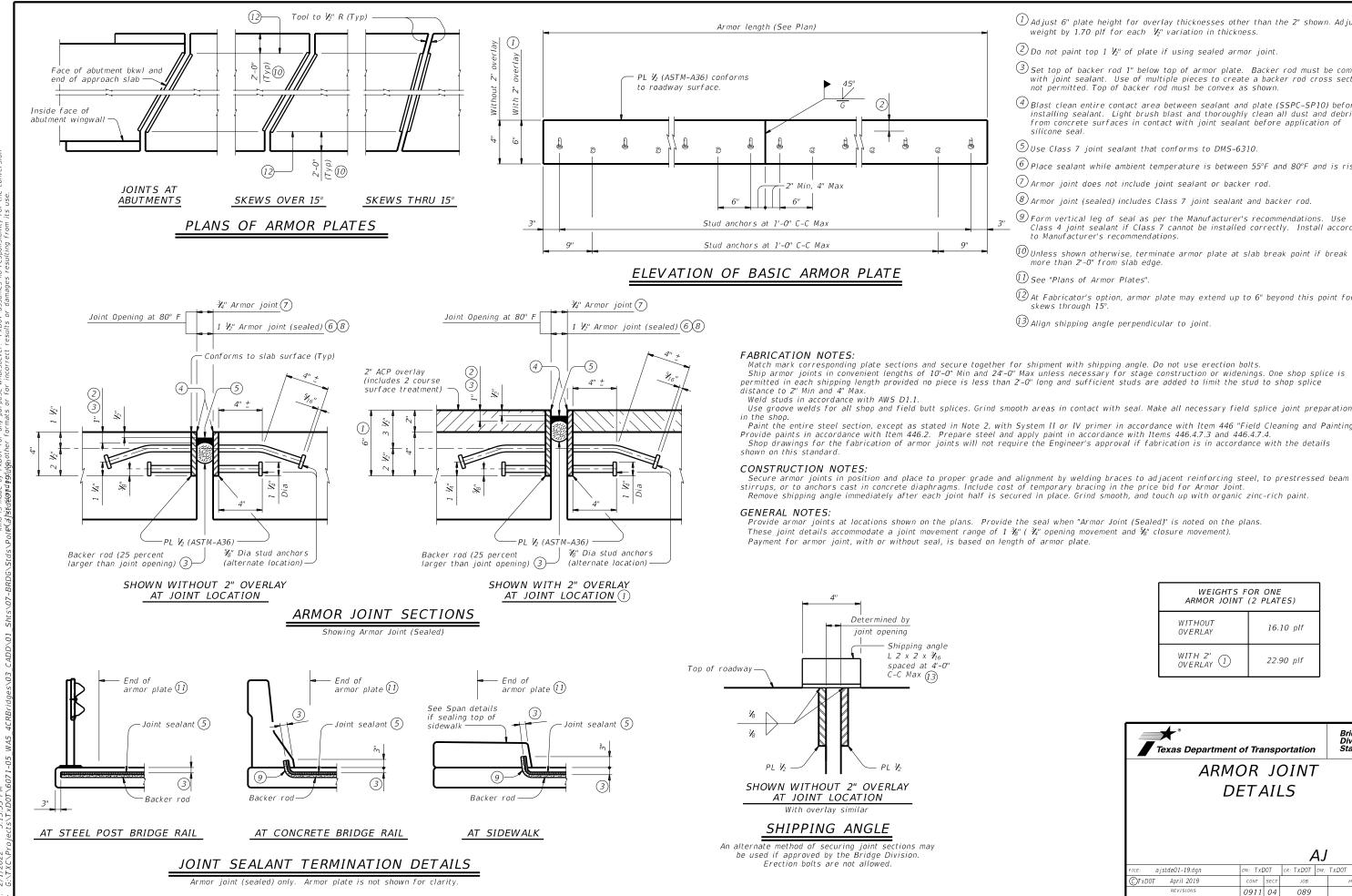
ΡE	Tx4	6 Gir	ders	
э.	Size	Ler	igth	Weight
0	#11	25'	-0"	1,328
?	#9	1'-	-8"	11
0	#6	25'	-8"	386
8	#6	4'-	-0"	108
2	#5	11'	-6"	264
t.	#6	8'-	-1"	49
5	#5	14'	-4"	374
4	#6	12'	-5"	261
4	#6	10'	-8"	385
4	#4	7'-	10"	126
4	#5	14'	-4"	359
g St	eel		Lb	3,651
Conc	rete		СҮ	19.7

	ΤΥΡΕ	Tx5	4 Gir	ders	
Bar	No.	Size	Len	ngth	Weight
А	10	#11	25'	-0"	1,328
D(7)	2	#9	1'-	-8"	11
Н	12	#6	25'	-8"	463
L	18	#6	4'-	-0"	108
S	22	#5	11'	-6"	264
U	4	#6	8'-	-1"	49
V	25	#5	15'	-8"	409
wH1	14	#6	13'	-5"	282
wH2	28	#6	11'	-8"	491
wS	26	#4	7'-	10"	136
wV	26	#5	15'	-8"	425
Reinfo	orcing St	eel		Lb	3,966
Class	"C" Conc	rete		СҮ	21.6

	ΤΥΡΕ	Tx4	6 Gir	ders	
ar	No.	Size	Ler	ngth	Weight
A	10	#11	25'	-0"	1,328
D(7)	2	#9	1'-	-8"	11
Н	10	#6	25'	-8"	386
L	18	#6	4'-	-0"	108
S	22	#5	11'	-6"	264
U	4	#6	8'-	-1"	49
V	25	#5	14'	-4"	374
Н1	14	#6	17'	-5"	366
Н2	24	#6	15'	-8"	565
vS	34	#4	7'-	10"	178
vV	34	#5	14'	-4"	508
einfo	orcing St	eel		Lb	4,137
lass	"C" Conc	rete		СҮ	23.4

	ΤΥΡΕ	Tx5	4 Gir	ders	
Bar	No.	Size	Ler	ngth	Weight
А	10	#11	25'	-0"	1,328
D(7)	2	#9	1'-	-8"	11
Н	12	#6	25'	-8"	463
L	18	#6	4'-	-0"	108
S	22	#5	11'	-6"	264
U	4	#6	8'-	-1"	49
V	25	#5	15'	-8"	409
wH1	14	#6	19'	-5"	408
wH2	28	#6	17'	-8"	743
wS	38	#4	7'-	10"	199
wV	38	#5	15'	-8"	621
Reinfo	orcing St	eel		Lb	4,603
Class	"C" Conc	rete		СҮ	26.4

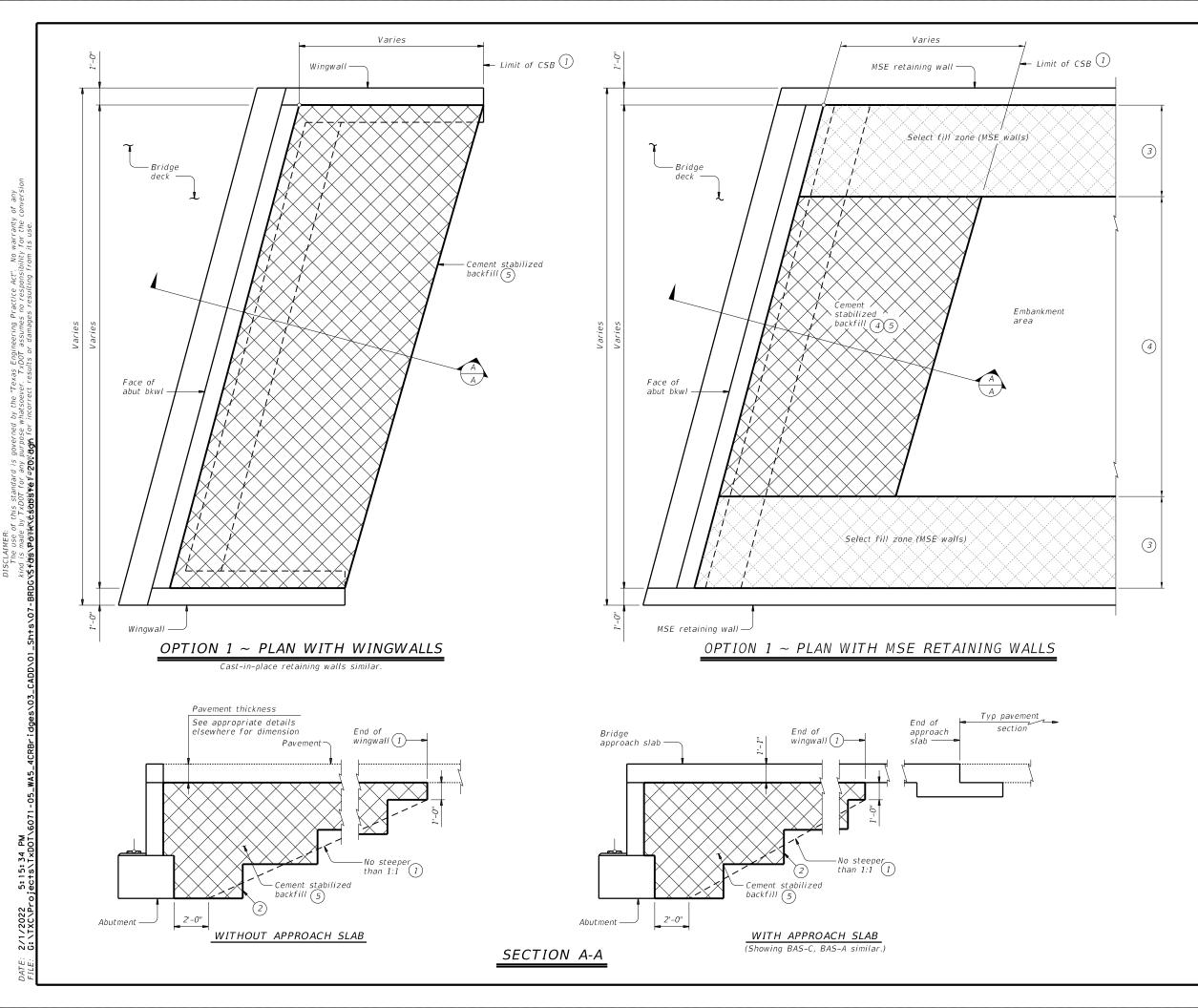




- (1) Adjust 6" plate height for overlay thicknesses other than the 2" shown. Adjust weight by 1.70 plf for each $\frac{1}{2}$ variation in thickness.
- 2 Do not paint top 1 \rlap{k} " of plate if using sealed armor joint.
- 3 Set top of backer rod 1" below top of armor plate. Backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- (4) Blast clean entire contact area between sealant and plate (SSPC-SP10) before installing sealant. Light brush blast and thoroughly clean all dust and debris from concrete surfaces in contact with joint sealant before application of silicone seal
- (5) Use Class 7 joint sealant that conforms to DMS-6310.
- igoplus Place sealant while ambient temperature is between 55°F and 80°F and is rising.
- (7) Armor joint does not include joint sealant or backer rod.
- 8 Armor joint (sealed) includes Class 7 joint sealant and backer rod.
- (9) Form vertical leg of seal as per the Manufacturer's recommendations. Use Class 4 joint sealant if Class 7 cannot be installed correctly. Install according to Manufacturer's recommendations.
- (0) Unless shown otherwise, terminate armor plate at slab break point if break is more than 2'-0" from slab edge.
- (1) See "Plans of Armor Plates".
- 12 At Fabricator's option, armor plate may extend up to 6" beyond this point for skews through 15°.
- (13) Align shipping angle perpendicular to joint.
- Ship armor joints in convenient lengths of 10'-0" Min and 24'-0" Max unless necessary for stage construction or widenings. One shop splice is permitted in each shipping length provided no piece is less than 2'-0" long and sufficient studs are added to limit the stud to shop splice
- Use groove welds for all shop and field butt splices. Grind smooth areas in contact with seal. Make all necessary field splice joint preparations
- Paint the entire steel section, except as stated in Note 2, with System II or IV primer in accordance with Item 446 "Field Cleaning and Painting Steel." Provide paints in accordance with Item 446.2. Prepare steel and apply paint in accordance with Items 446.4.7.3 and 446.4.7.4. Shop drawings for the fabrication of armor joints will not require the Engineer's approval if fabrication is in accordance with the details
- Remove shipping angle immediately after each joint half is secured in place. Grind smooth, and touch up with organic zinc-rich paint.

WEIGHTS F ARMOR JOINT	
WITHOUT OVERLAY	16.10 plf
WITH 2" OVERLAY 1	22.90 plf

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- (1) Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.
- 2 Bench backfill as shown with 12" (approximate) bench depths.
- (3) Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.
- (4) When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.
- (5) If shown in the plans flowable backfill can be used as a substitute for cement stabilized backfill with the following

constraints: a). If flowable backfill is to be placed over MSE backfill then a filter fabric will be placed over the MSE backfill prior to placement of the flowable fill; and b). Place flowable fill in lifts not exceeding 2 feet in height, place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowability).

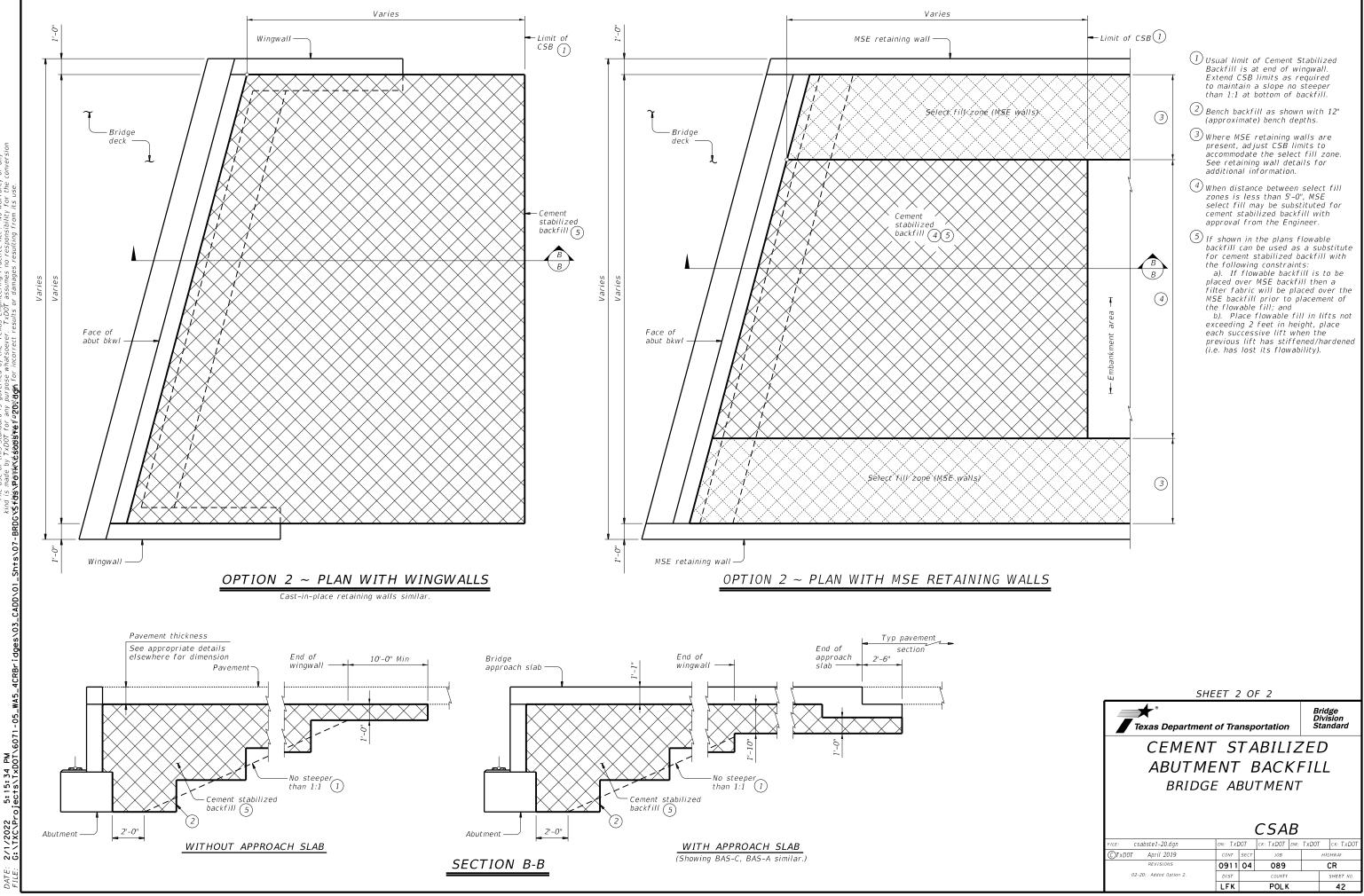
GENERAL NOTES:

See the Bridge Layout for selected Option. Option 2 is intended for new construction requiring high plasticity embankment fill with a plasticity index (PI) greater than 30 or pavement built in poor native soil. Poor soils are defined as high plasticity clays or expansive clays. Option 1 is intended for construction only requiring PI controlled embankment fill or excavation in competent soils/rocks in order to construct the abutment. *Provide Cement Stabilized Backfill (CSB) meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments.*

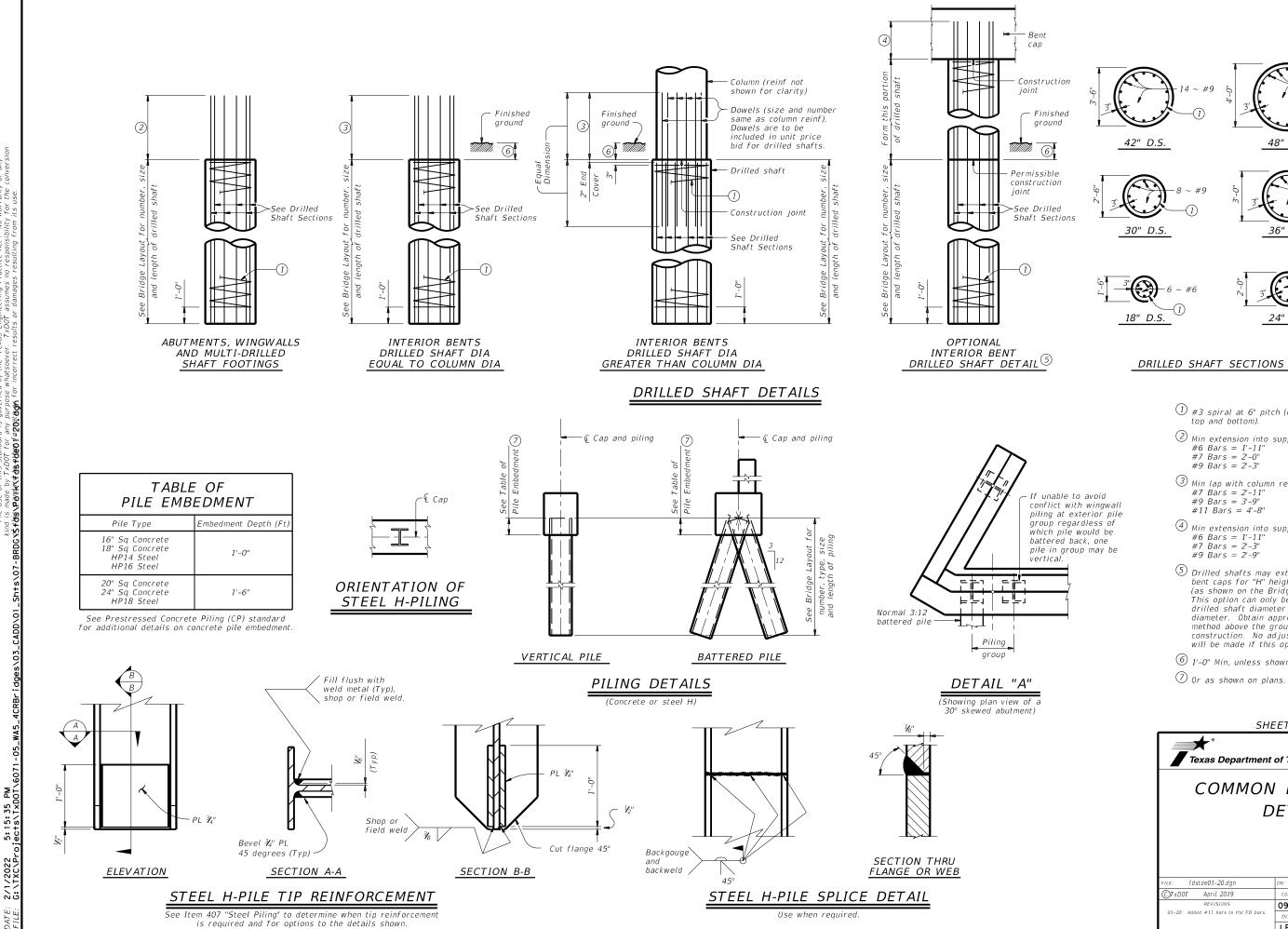
If required elsewhere in the plans, provide Flowable Backfill meeting the requirements of Item 401, "Flowable Backfill", to the limits shown at bridge abutments. Details are drawn showing left forward skew. See Bridge Layout for actual skew direction.

These details do not apply when Concrete Block retaining walls are used in lieu of wingwalls.

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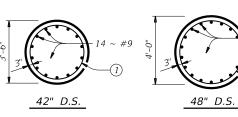


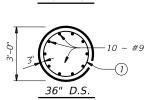
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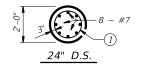
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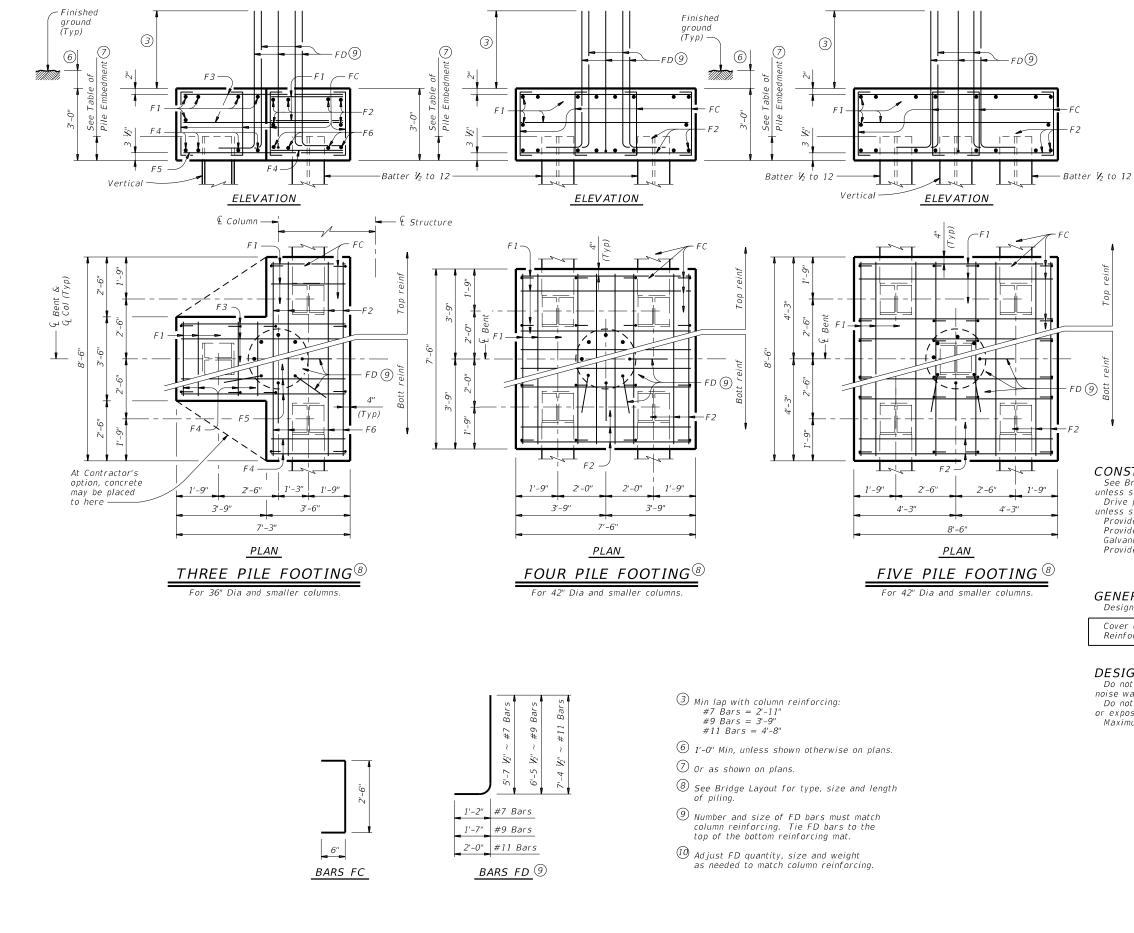


18 ~ #9



- 1 #3 spiral at 6" pitch (one and a half flat turns top and bottom).
- ② Min extension into supported element: #6 Bars = 1'-11" #7 Bars = 2'-0" #9 Bars = 2'-3"
- ③ Min lap with column reinf: #7 Bars = 2'-11" #9 Bars = 3'-9" #11 Bars = 4'-8"
- (4) Min extension into supported element: #6 Bars = 1'-11" #7 Bars = 2'-3"
- #9 Bars = 2'-9''
- 5 Drilled shafts may extend to the bottom of bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the drilled shaft diameter equals the column diameter. Obtain approval of the forming method above the ground line prior to construction. No adjustments in payment will be made if this option is used.
- 6 1'-0" Min, unless shown otherwise on plans.
- ⑦ Or as shown on plans.

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			OF FO		
			COLUN		
			PILE FOOT		
Bar	No.	Size	Lengti		Weight
F 1	11	#4	3'- 2	"	23
F2	6	#4	8'- 2	"	33
F3	6	#4	6'- 11	"	28
F 4	8	#9	3'- 2	"	86
F5	4	#9	6'- 11	!"	94
F6	4	#9	8'- 2	"	111
FC	12	#4	3'- 6	"	28
FD 1 Ø	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	623
Class	"С" Со	ncrete		СҮ	4.8
		ONE 4	PILE FOOT	ING	
Bar	No.	Size	Lengti	h	Weight
F 1	20	#4	7'- 2	"	96
F2	16	#8	7'- 2	"	306
FC	16	#4	3'- 6	"	37
FD 🚺	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	659
Class	"C" Co	ncrete		СҮ	6.3
		ONE 5	PILE FOOT	<i>'ING</i>	
Bar	No.	Size	Lengti	h	Weight
F 1	20	#4	8'- 2	"	109
F2	16	#9	8'- 2	"	444
FC	24	#4	3'- 6		56
FD 🚺	8	#9	8'- 1	"	220
Reinf	orcing	Steel		Lb	829
Class	"C" Co	ncrete		СҮ	8.0

CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details Unless shown otherwise. Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile

unless shown otherwise.

Provide Class C Concrete (f'c = 3,600 psi), unless shown otherwise. Provide Grade 60 reinforcing steel. Galvanize reinforcing if shown elsewhere in the plans.

Provide bar laps for drilled shaft reinforcing, where required, as follows: Uncoated or galvanized (#6) ~ 2'-6" Uncoated or galvanized (#7) ~ 2'-11" Uncoated or galvanized (#9) ~ 3'-9"

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.

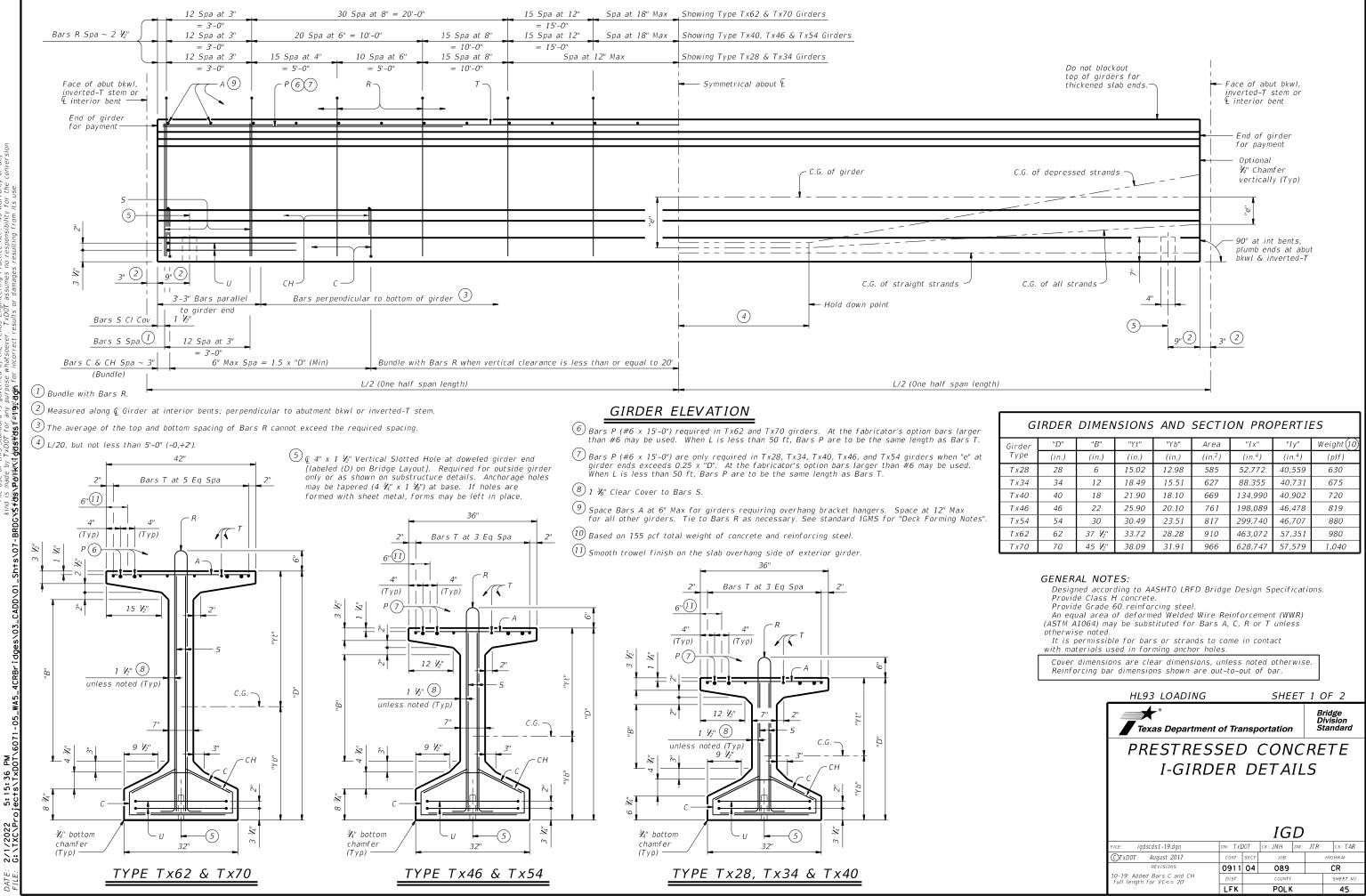
Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

DESIGNER NOTES: Do not use the drilled shaft details shown on this standard for retaining wall,

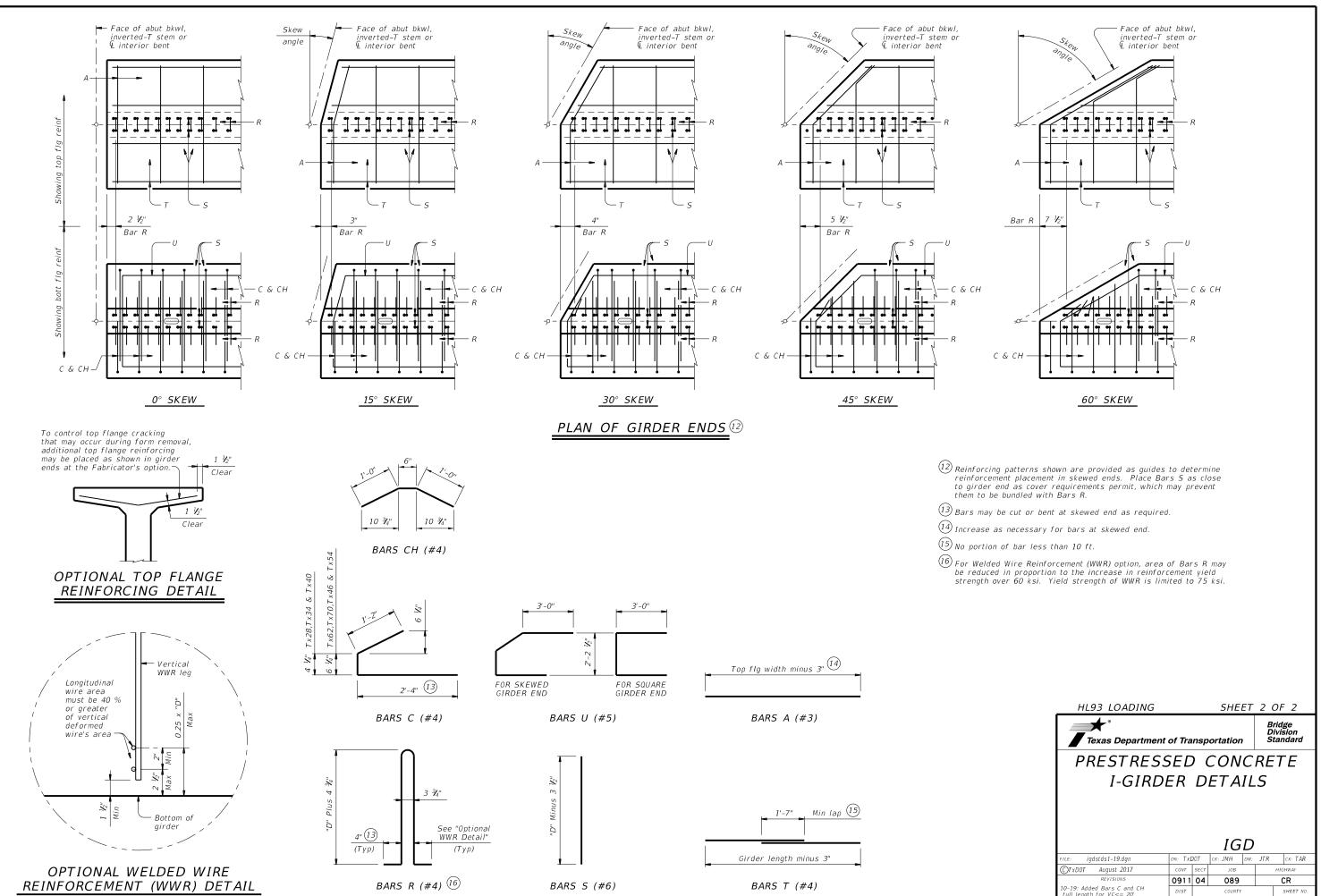
Do not use the formed shart details shown on this standard for recaming wan, noise wall, barrier, or sign foundations without structural evaluation. Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray. Maximum allowable pile loads for the footings shown are:

SHOWH are.				
72 Tons/Pile	with	24"	Dia	Columns
80 Tons/Pile	with	30"	Dia	Columns
100 Tons/Pile	with	36"	Dia	Columns
120 Tons/Pile	with	42"	Dia	Columns

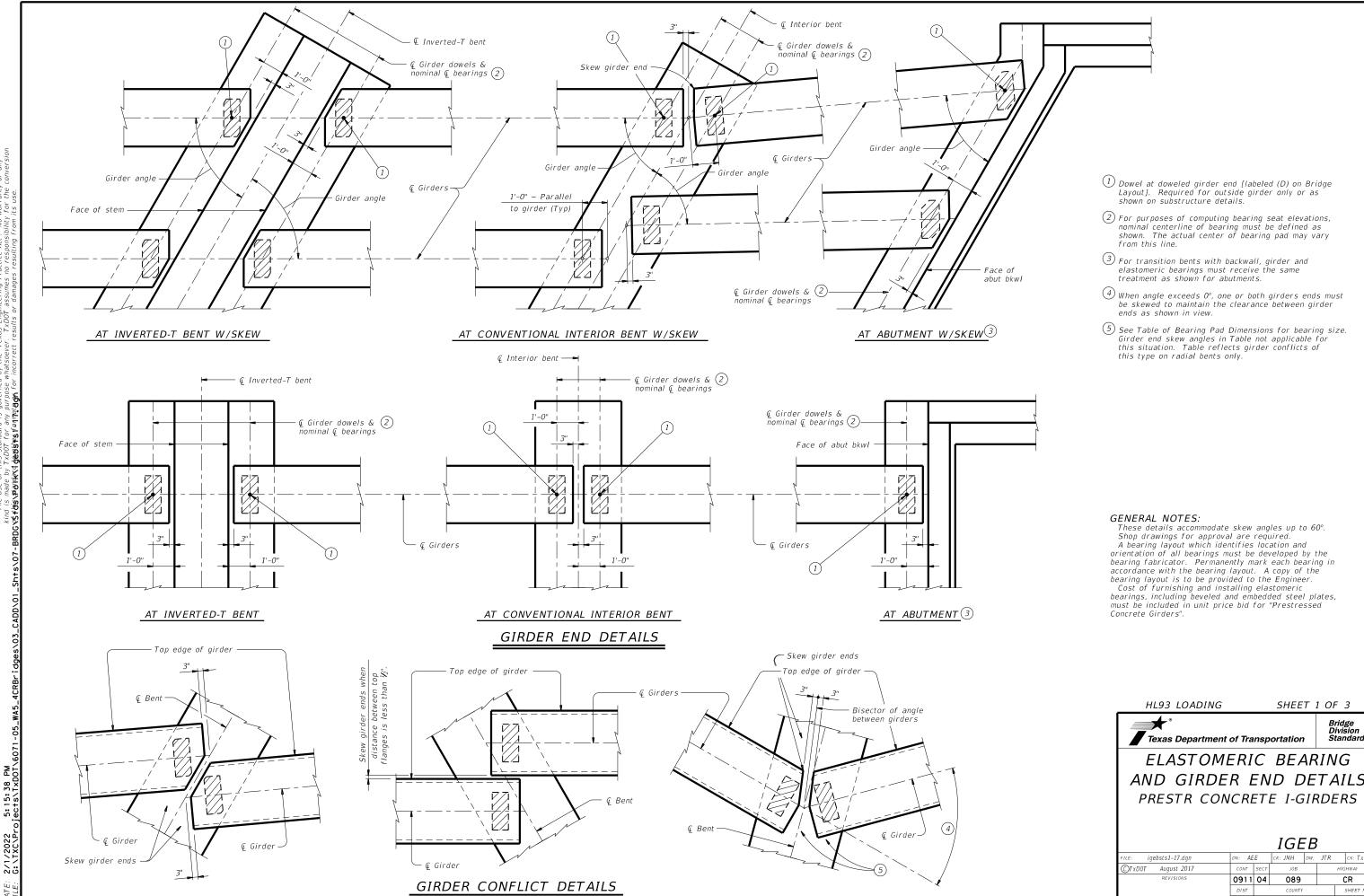
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Girder	"D"	"B"	"Yt"	"Y b"	Area	"Ix"	"Iy"	Weight (10)
Туре	(in.)	(in.)	(in.)	(in.)	(in.²)	(in.4)	(in.4)	(plf)
T x 28	28	6	15.02	12.98	585	52,772	40,559	630
Tx34	34	12	18.49	15.51	627	88,355	40,731	675
Tx40	40	18	21.90	18.10	669	134,990	40,902	720
Тх46	46	22	25.90	20.10	761	198,089	46,478	819
T x 54	54	30	30.49	23.51	817	299,740	46,707	880
Tx62	62	37 ½"	33.72	28.28	910	463,072	57,351	980
T x 7 0	70	45 ½"	38.09	31.91	966	628,747	57,579	1,040

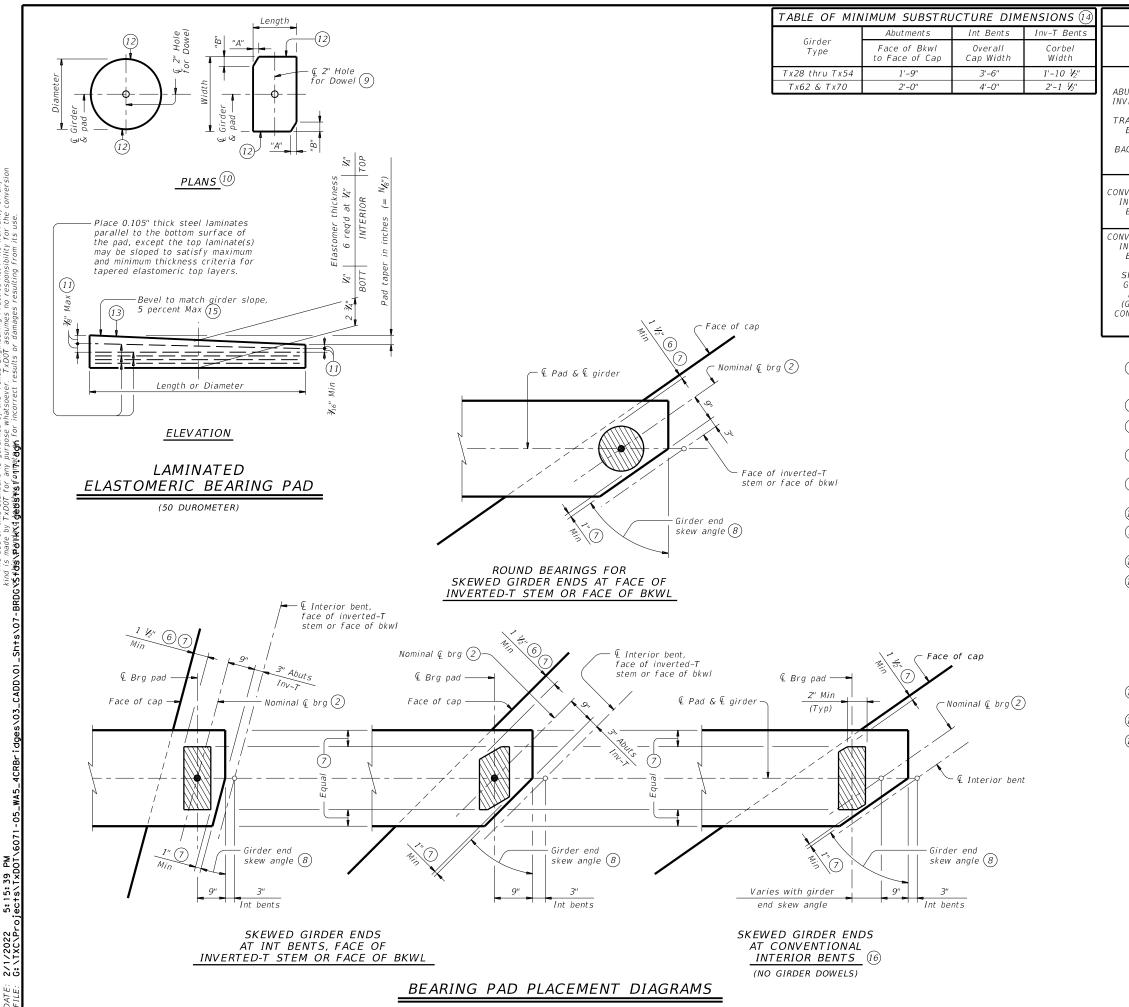


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	TADLL	OF BEARI	ING PAD DIMEN	ISIONS		
Bent Type	Girder Type	Bearing Type	Girder End Skew Angle Range	Pad Size Lgth x Wdth	Pad Dimer	
, ype	, ype	(13)	Range	Lgen x haen	"A"	"B"
		G-1-"N"	0° thru 21°	8" x 21"		
BUTMENTS.	T x 28,T x 34, T x 40,T x 46	G-2-"N"	21°+ thru 30°	8" x 21"	1 ½"	2 ½"
VERTED-Ť	& Tx54	G-3-"N"	30°+ thru 45°	9" x 21"	4 ½"	4 ½"
AND RANSITION		G-4-"N"	45°+ thru 60°	15" Dia		
BENTS		G-5-"N"	0° thru 21°	9" x 21"		
WITH	T x62 &	G-6-"N"	21°+ thru 30°	9" x 21"	1 ¹ / ₂ "	2 ¹ ⁄2"
ACKWALLS	тх70	G-7-"N"	30°+ thru 45°	10" x 21"	4 ½"	4 ½"
		G-8-"N"	45°+ thru 60°	10" x 21"	7 ½″	4 ¹ ⁄ ₄ "
	Tx28,Tx34,					
IVENTIONAL INTERIOR	Tx40,Tx46					
BENTS	& Tx54	G-1-"N"	0° thru 60°	8" x 21"		
	Tx62 & Tx70	G-5-"N"	0° thru 60°	9" x 21"		
VENTIONAL		G-1-"N"	0° thru 18°	8" x 21"		
NTERIOR BENTS	Т x 28,Т x 34, Т x 40.Т x 46	G-2-"N"	18°+ thru 30°	8" x 21"	1 ½"	2 ½"
WITH	& Tx54	G-9-"N"	30°+ thru 45°	8" x 21"	3"	3"
SKEWED		G-10-"N"	45°+ thru 60°	9" x 21"	6"	3 ½"
GIRDER ENDS		G-5-"N"	0° thru 18°	9" x 21"		
(GIRDER	Т x62 &	G-5-"N"	18°+ thru 30°	9" x 21"		
ONFLICTS)	Tx70	G-11-"N"	30°+ thru 45°	9" x 21"	1 1/2"	1 1/2"
(16)		G-12-"N"	45°+ thru 60°	9" x 21"	3"	1 3⁄4"

2 For purposes of computing bearing seat elevations, nominal centerline of bearing must be defined as shown. The actual center of bearing pad may vary from this line.

6 3" for inverted-T.

7 Place centerline pad as near nominal centerline bearing as possible between limits shown.

(8) Girder end skew angle is equal to 90° minus the girder angle except at some conflicting girders.

(9) Provide 2" dia hole only at locations required. See Substructure details for location.

(10) See Table of Bearing Pad Dimensions for dimensions.

(11) Maximum and minimum layer thicknesses shown are for elastomer only, on tapered layers.

(12) Locate Permanent Mark here.

 $\fbox{13}$ Indicate BEARING TYPE on all pads. For tapered pads, locate BEARING TYPE on the high side. The Fabricator must include the value of "N" (amount of taper in $\frac{1}{18}$ " increments) in this mark. Examples: N=0, (for O" taper)

N=1, (for $\frac{1}{8}$ " taper)

N=2, (for V_4 " taper) (etc.)

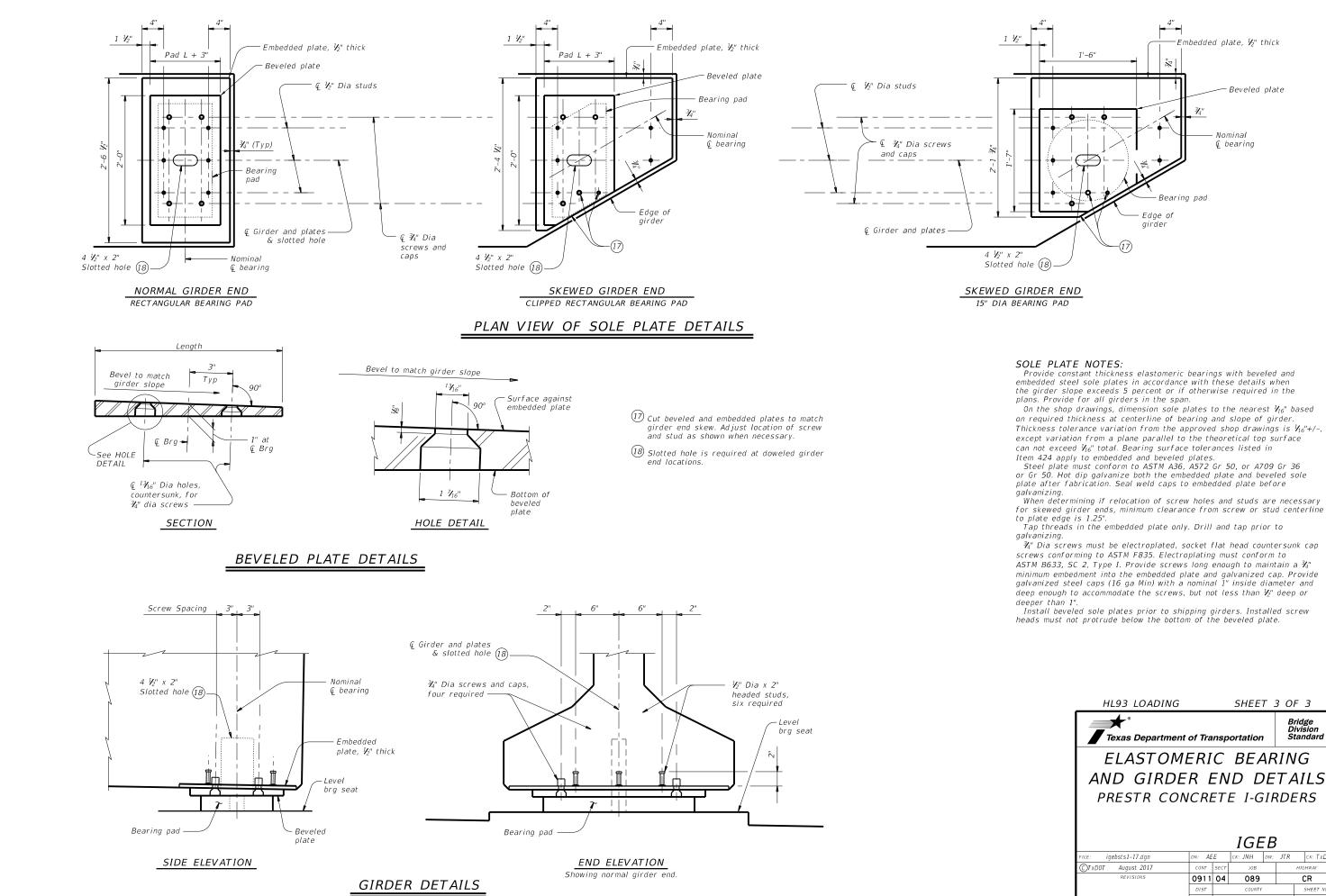
Fabricated pad top surface slope must not vary from plan girder slope by more than $\left(\begin{array}{c} 0.0625''\\ Length \text{ or Dia} \end{array}\right)$ IN/IN.

 $\stackrel{(14)}{\longrightarrow}$ Substructure dimensions must satisfy the minimums provided to accommodate the elastomeric bearings shown on this standard.

(15) See sheet 3 of 3 for beveled plate use when slopes exceed 5 percent.

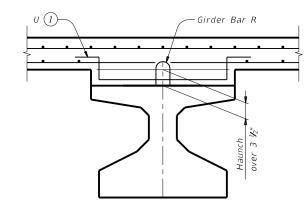
(16) If girder end is skewed for a girder conflict at an interior bent and a beveled sole plate is required, use bearing type for abutments at this location. Location of bearing centerline is to be set as for abutments in this case.

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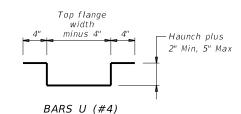


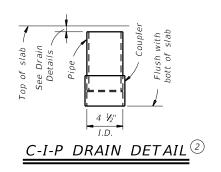
deep enough to accommodate the screws, but not less than V_2 " deep or

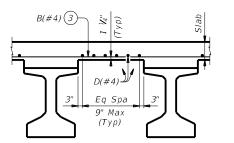
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HAUNCH REINFORCING DETAIL

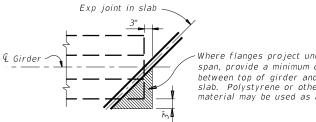






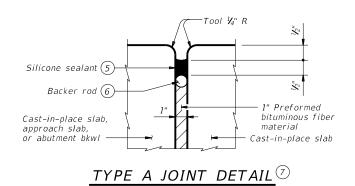
TYPICAL PART TRANSVERSE SLAB SECTION WITHOUT PCP

Top reinforcing steel not shown for clarity.

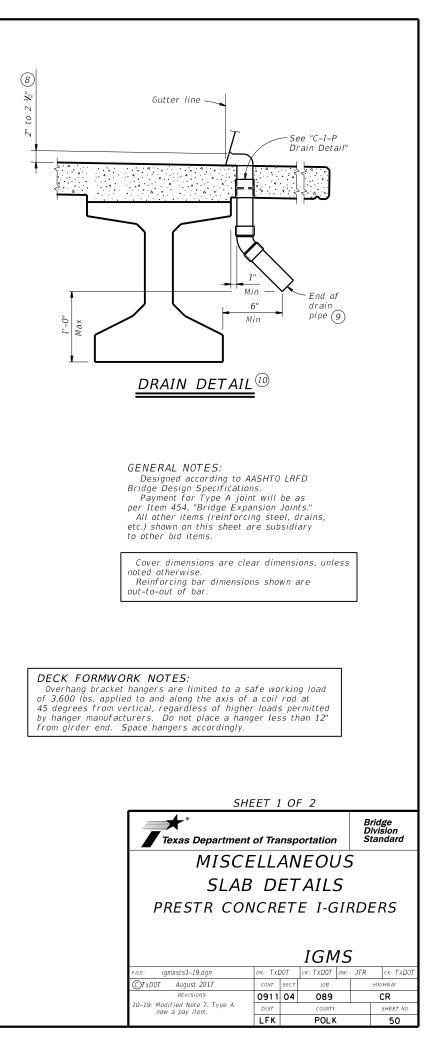


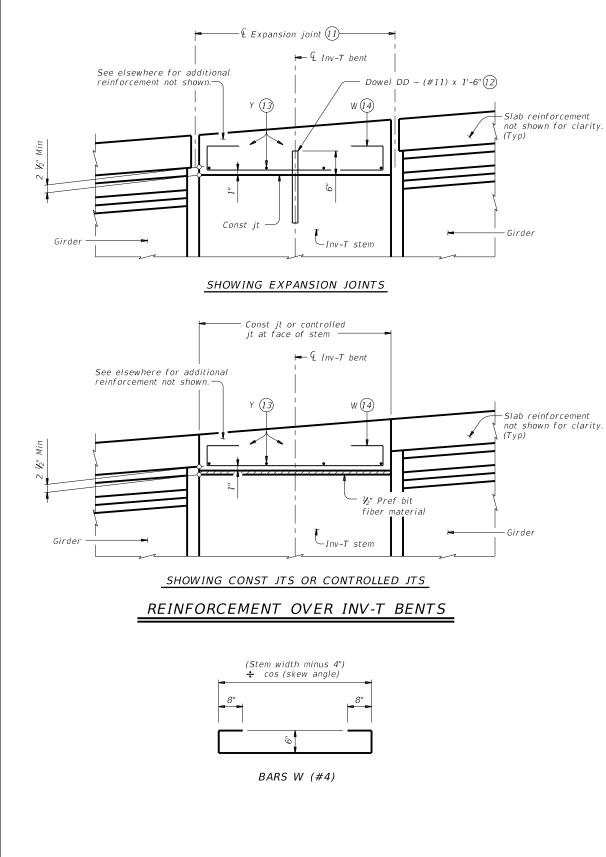
Where flanges project under slab of adjacent span, provide a minimum of $\frac{1}{2}$ " clearance between top of girder and bottom of adjacent slab. Polystyrene or other suitable compressible material may be used as a filler.

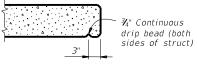
TREATMENT AT GIRDER END FOR SKEWED SPANS



- (1) Space Bars U with girder Bars R in all areas where measured haunch exceeds 3 ${\cal V}_{2}$ ".
- 2 Roughen outside of PVC with coarse rasp or equal to ensure bond with cast-in-place concrete.
- 3 Bars B(#4) spaced at 9" Max with 2" end cover. Overhang option, Contractor's may end alternating bars B(#4) at centerline outside girder.
- 4 Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated ~ #4 = 1'-7" Epoxy coated ~ #4 = 2'-5"
- 5 Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- (6) 1 Y_4'' backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- \oslash The maximum distance between Type A expansion joints is 100'. See Bridge Layout for location of joints.
- (8) Drain entrance formed in rail or sidewalk.
- 9 Water may not be discharged onto girders.
- (1) All drain pipe and fittings to be 4" diameter (Sch 40) PVC. See Item 481 "Pipe for Drains" for pipe, connections and solvent welding. Bend reinforcing steel to clear PVC 1". Drain length and location is as directed by the Engineer. Drains are not permitted over roadways or railways, or within 10'-0" of bent caps. Degrease outside of exposed PVC, apply acrylic water base primer, then coat with same surface finishing material as used for outside girder face. Variations of the above designs, as required for the type of rail used and its location on the structure, may be installed with the approval and direction of the Engineer.









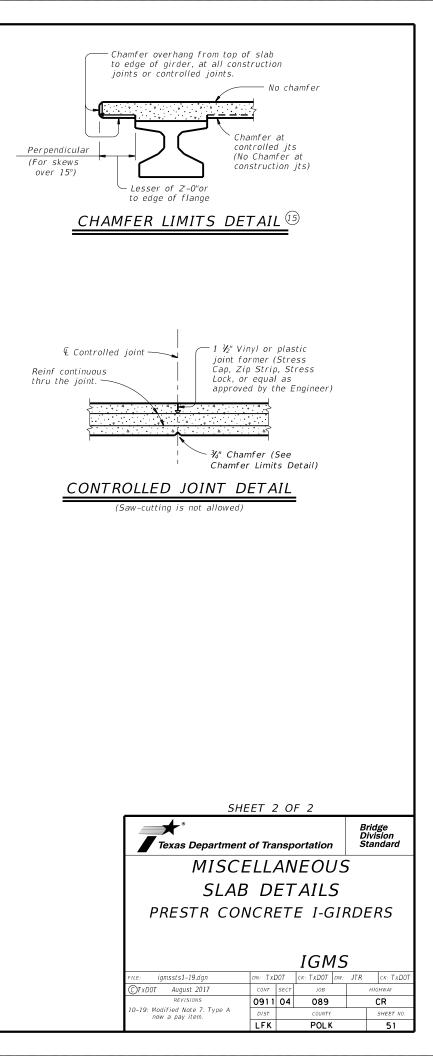
1) See Layout for joint type.

12 Dowels DD (#11) spaced at 5 Ft Max. See Inv-T bents for quantity and location.

- 13 Space Bars Y (#4) at 12" Max. Use 2" end cover. Number of Bars Y must satisfy spacing limit. Place parallel to bent.
- (14) Space Bars W at 12" Max (3" from end of cap). Tilt if necessary to maintain cover requirements. Place parallel to longitudinal slab reinforcement.

15 See Span details for type of joint and joint locations.

DATE:



DTRUCTURE pe Tx28 Girders 24' Roadway 8.5" Slab	SPAN NO. 40	GIRDER NO.	GIRDER TYPE	NON-	PRES	TRESSI	NG STRA	NDS		I STE	RAND	1	CRETE	DESIGN	DESIGN	REQUIRED	LIVE	LOAD			
24' Roadway				STD	TOTAL	517F	STRGTH	"e"	"e"	_	TERN	RELEASE STRGTH	MINIMUM 28 DAY	LOAD COMP	LOAD TENSILE	MINIMUM ULTIMATE	DISTR	IBUTION TOR	STREN		
24' Roadway				STRAND PATTERN	NO.	(in)	fpu (ksi)	€ (in)	END (in)	NO.	TO END (in)	1 f'ci (ksi)	COMP STRGTH f'c (ksi)	STRESS (TOP @) (SERVICE I) fct(ksi)	STRESS (BOTT @) (SERVICE III) fcb(ksi)	MOMENT CAPACITY (STRENGTH I) (kip-ft)		2)		0pr	SERVICE III
24' Roadway	45	ALL	T x 28		10	0.6	270	10.48	10.48		(11)	4.000	5.000	1.055	-1.423	1382	Moment 0.670	Shear 0.850	Inv 1.56	2.02	Inv 1.98
24' Roadway	45	ALL	Tx28		12	0.6	270	10.48	10.48			4.500	5.000	1.332	-1.744	1525	0.650	0.850	1.58	2.05	1.79
	50 55	ALL ALL	Т x 28 Т x 28		12 14	0.6 0.6	270 270	10.48 10.48	10.48 9.62	2	8.5	4.200 4.000	5.000 5.000	1.645 1.969	-2.113 -2.490	1657 1919	0.630 0.610	0.860 0.860	1.25 1.27	1.62 1.64	1.25 1.11
	60	ALL	T x 28		18	0.6	270	10.04	7.81	4	14.5	4.000	5.600	2.320	-2.901	2206	0.600	0.870	1.43	1.86	1.14
	65	ALL	T x 28		22	0.6	270	9.75	6.12	4	24.5	4.300	5.900	2.716	-3.337	2486	0.580	0.870	1.55	2.00	1.14
	70 75	ALL ALL	Т x 28 Т x 28		26 28	0.6 0.6	270 270	9.56 9.48	6.48 6.62	4 4	24.5 24.5	5.200 5.600	6.300 7.800	3.131 3.572	-3.802 -4.291	2793 3110	0.570 0.560	0.870 0.880	1.26 1.38	1.89 1.81	1.01 1.08
	40	ALL	Tx34		10	0.6	270	13.01	13.01			4.000	5.000	0.835	-1.089	1605	0.690	0.830	1.85	2.40	2.60
	45	ALL	Tx34		10	0.6	270	13.01	13.01			4.500	5.500	1.050	-1.332	1750	0.670	0.840	1.90	2.46	2.42
	50 55	ALL ALL	T x 34 T x 34		12 12	0.6 0.6	270 270	13.01 13.01	13.01 13.01			4.000 4.000	5.000 5.000	1.294 1.553	-1.612 -1.904	1868 1981	0.650 0.630	0.840 0.840	1.53 1.24	1.98 1.61	1.81 1.33
pe Tx34 Girders 24' Roadway	60	ALL	Tx34		14	0.6	270	13.01	12.44	2	6.5	4.000	5.000	1.845	-2.231	2287	0.620	0.850	1.27	1.64	1.22
8.5" Slab	65 70	ALL ALL	T x 34 T x 34		16 20	0.6 0.6	270 270	12.76 12.41	11.76 9.61	4 4	8.5 18.5	4.000 4.000	5.000 5.100	2.161 2.461	-2.579 -2.902	2605 2888	0.610 0.590	0.850 0.850	1.25 1.46	1.62 1.89	1.06 1.13
	75	ALL	T x 34		20	0.6	270	12.41	9.01 7.84	4	30.5	4.300	5.400	2.401	-3.283	3223	0.580	0.850	1.40	2.04	1.15
	80	ALL	Tx34		26	0.6	270	12.09	8.09	4	30.5	4.700	5.700	3.168	-3.660	3554	0.570	0.860	1.39	1.96	1.04
	85	ALL	Tx34		30	0.6	270	11.81	7.81	6	26.5	5.400	6.100	3.567	-4.078	3909	0.560	0.860	1.46	2.00	1.04
	40 45	ALL ALL	T x 40 T x 40		10 10	0.6 0.6	270 270	15.60 15.60	15.60 15.60			4.000 4.000	5.000 5.000	0.697 0.873	-0.889 -1.080	1671 1972	0.720 0.690	0.820 0.820	2.10 1.74	2.73 2.26	3.15 2.50
	50	ALL	Tx40		12	0.6	270	15.60	15.60			4.000	5.000	1.065	-1.299	2276	0.670	0.830	1.78	2.31	2.33
	55 60	ALL ALL	Т x 40 Т x 40		12 14	0.6 0.6	270 270	15.60 15.60	15.60 15.60			4.000 4.200	5.000 5.000	1.283 1.522	-1.538 -1.801	2237 2434	0.650 0.640	0.830 0.830	1.46 1.49	1.90 1.93	1.80 1.66
pe Tx40 Girders	65	ALL	T x 40		14	0.6	270	15.60 15.60	15.60			4.200	5.000	1.780	-2.081	2688	0.630	0.830	1.49	1.60	1.25
24' Roadway 8.5" Slab	70	ALL	Tx40		16	0.6	270	15.35	14.85	4	6.5	4.000	5.000	2.035	-2.349	2989	0.610	0.840	1.28	1.65	1.17
0.5 5100	75 80	ALL ALL	T x 40 T x 40		18 22	0.6 0.6	270 270	15.16 14.87	14.27 11.24	4	8.5 24.5	4.000 4.000	5.000 5.000	2.328 2.616	-2.657 -2.961	3337 3681	0.600 0.590	0.840 0.850	1.28 1.47	1.66 1.90	1.05 1.11
	85	ALL	Tx40		26	0.6	270	14.68	9.76	4	36.5	4.400	5.100	2.930	-3.287	4041	0.580	0.850	1.60	2.08	1.22
	90 05	ALL	T x 40		28	0.6	270	14.60	10.03	4	36.5	4.800	5.500	3.259	-3.626	4410	0.570	0.850	1.55	2.01	1.07
	95 100	ALL ALL	Т x 40 Т x 40		32 36	0.6 0.6	270 270	14.23 13.93	8.60 8.93	6 6	36.5 36.5	5.100 5.800	5.800 6.600	3.620 4.006	-3.991 -4.393	4799 5245	0.560 0.560	0.850 0.850	1.62 1.47	2.10 1.94	1.06 1.06
	40	ALL	Tx46		10	0.6	270	17.60	17.60			4.000	5.000	0.613	-0.708	1732	0.740	0.810	2.35	3.05	3.78
	45 50	ALL ALL	Т x 46 Т x 46		10 12	0.6 0.6	270 270	17.60 17.60	17.60 17.60			4.000 4.000	5.000 5.000	0.768 0.937	-0.865 -1.042	2066 2452	0.720 0.700	0.810 0.820	1.93 1.97	2.50 2.55	3.01 2.81
	55	ALL	T x 46		12	0.6	270	17.60	17.60			4.000	5.000	1.127	-1.235	2726	0.680	0.820	1.63	2.11	2.22
	60	ALL	T x 46		14	0.6	270	17.60	17.60			4.000	5.000	1.332	-1.438	2951	0.660	0.820	1.68	2.18	2.10
	65 70	ALL ALL	T x 46 T x 46		14 14	0.6 0.6	270 270	17.60 17.60	17.60 17.60			4.000 4.000	5.000 5.000	1.557 1.798	-1.662 -1.898	2905 3157	0.650 0.640	0.820 0.830	1.41 1.18	1.82 1.52	1.64 1.25
pe Tx46 Girders 24' Roadway	75	ALL	Tx46		16	0.6	270	17.35	16.85	4	6.5	4.000	5.000	2.050	-2.137	3495	0.620	0.830	1.23	1.59	1.17
8.5" Slab	80 85	ALL ALL	Т x 46 Т x 46		18 22	0.6 0.6	270 270	17.16 16.88	16.27 15.06	4	8.5 14.5	4.000 4.000	5.000 5.000	2.304 2.591	-2.384 -2.656	3859 4249	0.610 0.600	0.830 0.830	1.25 1.46	1.63 1.89	1.09 1.30
	90	ALL	T x 46		24	0.6	270	16.77	14.10	4	20.5	4.000	5.000	2.870	-2.923	4631	0.590	0.840	1.45	1.88	1.06
	95	ALL	T x 46		28	0.6	270	16.60	11.46	4	40.5	4.200	5.000	3.192	-3.234	5087	0.590	0.840	1.57	2.03	1.08
	100 105	ALL ALL	Т x 46 Т x 46		32 36	0.6 0.6	270 270	16.23 15.94	9.48 9.94	6 6	42.5 42.5	4.400 5.000	5.000 5.800	3.524 3.856	-3.542 -3.851	5513 5937	0.580 0.570	0.840 0.840	1.65 1.72	2.14 2.23	1.07 1.17
	110 115	ALL ALL	Т x 46 Т x 46		38 42	0.6 0.6	270 270	15.81 15.60	10.45 10.75	6 6	40.5 40.5	5.400 6.000	6.300 7.000	4.200 4.584	-4.169 -4.532	6370 6886	0.560 0.560	0.840 0.840	1.67 1.46	2.16 1.96	1.04 1.05
24.5 22.5 20.5				3 ½. (Typ)	2"	30.5 — 28.5 — 24.5 — 24.5 — 18.5 — 18.5 — 18.5 — 18.5 — 10.5 — 10.5 — 6.5 —		 }	$\dot{\phi}$		3 ½" (Typ)	Spa at 2"	36.5 32.5 30.5 28.5 26.5 24.5 18.5 16.5 14.5 10.5 6.5 4.5			3 ½"	sna at 2"	26.5 - 24.5 -			

DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TXDDT or any purpose whatsoever. TXDDT assumes no responsibility for the convers.

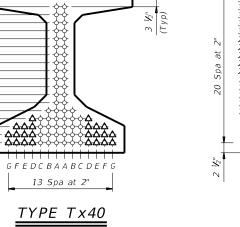
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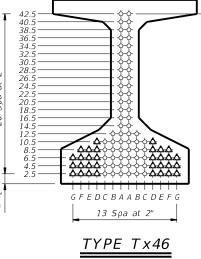
2.5

12

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XXXX44444**XXXX** 2.5 2.5 2 Ż GEEDCBAABCDEEG GEEDCBAABCDEEG \sim \sim 13 Spa at 2" 13 Spa at 2" *TYPE Tx34* TYPE Tx28





NON-STANDARD STRAND PATTERNS

STRAND ARRANGEMENT AT ∉ OF GIRDER PATTERN

1) Based on the following allowable stresses (ksi):

Compression = 0.65 f'ci

Tension = $0.24\sqrt{f'ci}$

Optional designs must likewise conform.

(2) Portion of full HL93.

DESIGN NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to AASTHO Manual for Bridge Evaluation.

Optional designs for girders 120 feet or longer must have a calculated residual camber equal to or greater than that of the designed girder.

Prestress losses for the designed girders have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform.

FABRICATION NOTES:

Provide Class H concrete. Provide Grade 60 reinforcing steel bars.

Use low relaxation strands, each pretensioned to 75 percent of f pu.

Strand debonding must comply with Item 424.4.2.2.2.4. Full-length debonded strands are only permitted in positions marked Δ . Double wrap full-length debonded strands in outer most position of each when shown on this sheet, the Fabricator has the option of

furnishing either the designed girder or an approved optional design. All optional design submittals must be signed, sealed and

dated by a Professional Engineer registered in the State of Texas. Seal cracks in girder ends exceeding 0.005" in width as directed by the Engineer. The fabricator is permitted to decrease the spacing of Bars R and S by providing additional bars to help limit crack width provided the decreased spacing results in no less than 1" clear between bars. The fabricator must take an approved corrective action if cracks greater than 0.005" form on a repetitive basis.

DEPRESSED STRAND DESIGNS:

Locate strands for the designed girder as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc., beginning each row in the "A" position and working outward until the required number of strands is reached. All strands in the "A" position must be depressed, maintaining the 2" spacing so that, at the girder ends, the upper two strands are in the position shown in the table.

HL93 LOADING			SHE	ET :	1 OF 2
Texas Department of	of Tra	nsp	ortation	D	ridge ivision tandard
PRESTRESS I-GIRDEF DE 24'	R S SI	GT. GI	ANDA VS		
	1	G	SD-24	l	
FILE: ig01stds-21.dgn	DN: EF	С	CK: AJF DW:	EFC	ск: TAR
CTxDOT August 2017	CONT	SECT	JOB		HIGHWAY
REVISIONS 10-19: Redesigned girders.	0911	04	089		CR
1–21: Added load rating.	DIST		COUNTY		SHEET NO.
	LFK		POLK		52

			DES	SIGNED	GIRDE	RS					ESSED	CONC	RETE		OPTION	AL DESIGN			LC	DAD R	ATING
STRUCTURE	SPAN NO.	GIRDER NO.	GIRDER TYPE	NON- STD STRAND PATTERN	PRES TOTAL NO.	SIZE	NG STRA STRGTH fpu	"e" ⊈	"e" END		RAND TERN	RELEASE STRGTH	MINIMUM 28 DAY COMP STRGTH f'c	DESIGN LOAD COMP STRESS (TOP Q) (SERVICE I)	DESIGN LOAD TENSILE STRESS (BOTT Ç) (SERVICE III)	REQUIRED MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I)	DISTRI FAC	LOAD IBUTION CTOR	STREM	_	SERVICE III
Type Tx54 Girders 24' Roadway 8.5" Slab	40 45 50 55 60 65 70 75 80 85 90 95 100	ALL ALL ALL ALL ALL ALL ALL ALL ALL ALL	T x 54 T x 54		8 10 12 12 14 14 16 16 16 18 20 22 26 20	(in) 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6	(ksi) 270 270 270 270 270 270 270 270	(in) 21.01 21.01 21.01 21.01 21.01 21.01 20.76 20.76 20.76 20.56 20.41 20.28 20.08	(in) 21.01 21.01 21.01 21.01 21.01 21.01 21.01 20.26 20.76 19.67 19.21 18.46 16.39 12.21	4 4 4 4 6	(in) 6.5 8.5 10.5 14.5 28.5 24.5	(ksi) 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000	(ksi) 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000	fct(ksi) 0.511 0.636 0.781 0.938 1.108 1.285 1.482 1.689 1.912 2.148 2.379 2.639 2.896 2.190	fcb(ksi) -0.578 -0.703 -0.850 -1.007 -1.173 -1.348 -1.540 -1.733 -1.944 -2.166 -2.384 -2.624 -2.871 2.120	(kip-ft) 1798 2126 2533 2951 3271 3547 3502 3745 4001 4406 4806 5234 5699 6152	Moment 0.770 0.740 0.720 0.700 0.680 0.670 0.660 0.640 0.630 0.620 0.610 0.600 0.600 0.600	Shear 0.800 0.800 0.810 0.810 0.810 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820	Inv 2.05 2.24 1.81 1.90 1.60 1.66 1.41 1.47 1.26 1.07 1.33 1.35 1.55 1.55	0pr 2.66 2.90 2.35 2.46 2.07 2.16 1.82 1.91 1.63 1.39 1.73 1.75 1.97	Inv 3.76 3.69 2.91 2.79 2.25 2.16 1.73 1.66 1.30 1.00 1.16 1.07 1.14 1.02
es resulting fro	105 110 115 120 125	ALL ALL ALL ALL ALL	T x 54 T x 54 T x 54 T x 54 T x 54 T x 54		30 32 36 38 42	0.6 0.6 0.6 0.6 0.6	270 270 270 270 270 270	19.81 19.63 19.34 19.22 19.01	12.21 11.38 12.01 13.22 12.72	6 6 6 6	44.5 50.5 50.5 44.5 50.5	4.000 4.100 4.700 5.200 5.600	5.000 5.000 5.500 6.100 6.600	3.180 3.477 3.786 4.116 4.415	-3.130 -3.400 -3.679 -3.985 -4.257	6153 6619 7096 7646 8113	0.590 0.580 0.570 0.570 0.560	0.830 0.830 0.830 0.830 0.830 0.830	1.51 1.63 1.60 1.65 1.71	1.96 2.12 2.07 2.14 2.24	1.02 1.03 1.00 1.01 1.09
estemp to stylinse type Tx62 Girders 24' Roadway 8.5" Slab	60 65 70 75 80 85 90 95 100 105 110 115 120 125 130	ALL ALL ALL ALL ALL ALL ALL ALL ALL ALL	T x62 T x62		12 12 14 16 16 20 22 24 26 30 34 36 40	0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6 0.6	270 270 270 270 270 270 270 270 270 270	25.78 25.78 25.78 25.53 25.53 25.53 25.53 25.18 25.05 24.94 24.85 24.25 24.25 24.11 23.88	25.78 25.78 25.78 25.53 25.53 25.53 24.78 23.96 23.28 22.70 17.78 15.07 17.11 16.68	4 4 4 6 6 6 6 6	6.5 10.5 14.5 18.5 40.5 58.5 54.5 54.5 58.5	4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.000 4.200 4.200 5.100	5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000 5.000	0.878 1.016 1.171 1.332 1.506 1.691 1.885 2.081 2.295 2.514 2.723 2.963 3.213 3.480 3.733	-0.986 -1.133 -1.293 -1.455 -1.633 -1.819 -2.013 -2.209 -2.420 -2.642 -2.850 -3.083 -3.325 -3.591 -3.836	3525 3847 4173 4132 4429 4610 5051 5493 5959 6475 6936 7440 7957 8551 9072	0.700 0.690 0.680 0.650 0.640 0.630 0.620 0.610 0.610 0.610 0.590 0.580 0.580 0.570	0.800 0.800 0.810 0.810 0.810 0.810 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820 0.820	1.81 1.89 1.61 1.68 1.45 1.24 1.29 1.11 1.16 1.37 1.39 1.56 1.55 1.64 1.52	2.35 2.45 2.08 2.18 1.88 1.61 1.68 1.44 1.50 1.78 1.80 2.02 2.01 2.13 2.09	2.73 2.64 2.16 2.10 1.72 1.37 1.31 1.02 1.01 1.10 1.03 1.09 1.00 1.04 1.02

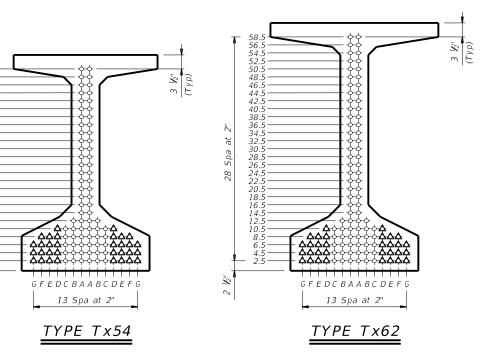
50.5 48.5 46.5 42.5 38.5 36.5 32.5 32.5 22.5 22.5 22.5 18.5 12.5 12.5 12.5 5 6.5

4.5 2.5

24 Spa at 2"

2 15"





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NON	I-STANDARD STRAND PATTERNS
PATTERN	STRAND ARRANGEMENT AT € OF GIRDER

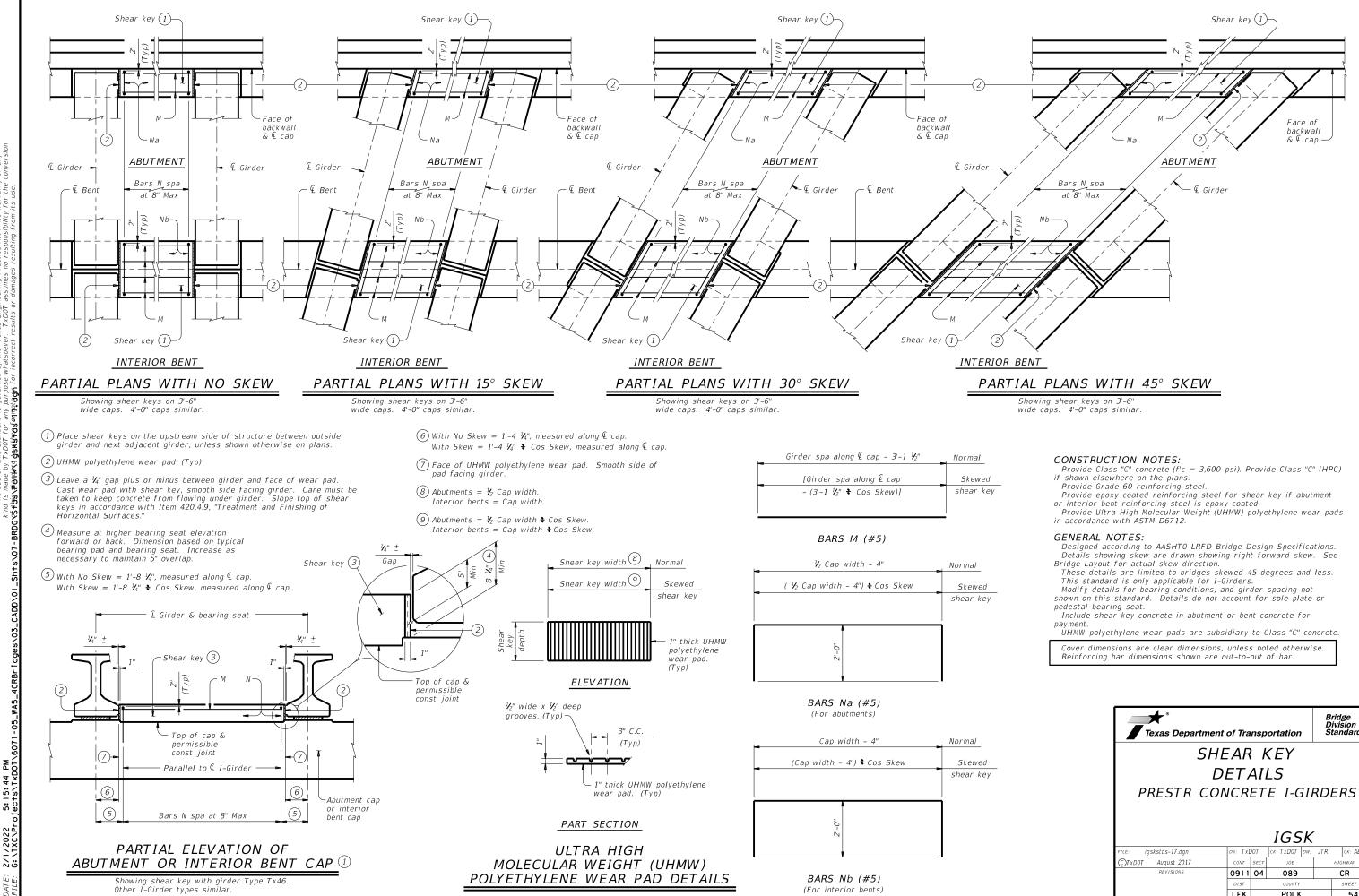
(1) Based on the following allowable stresses (ksi): Compression = 0.65 f'ci

Tension = $0.24\sqrt{f'ci}$

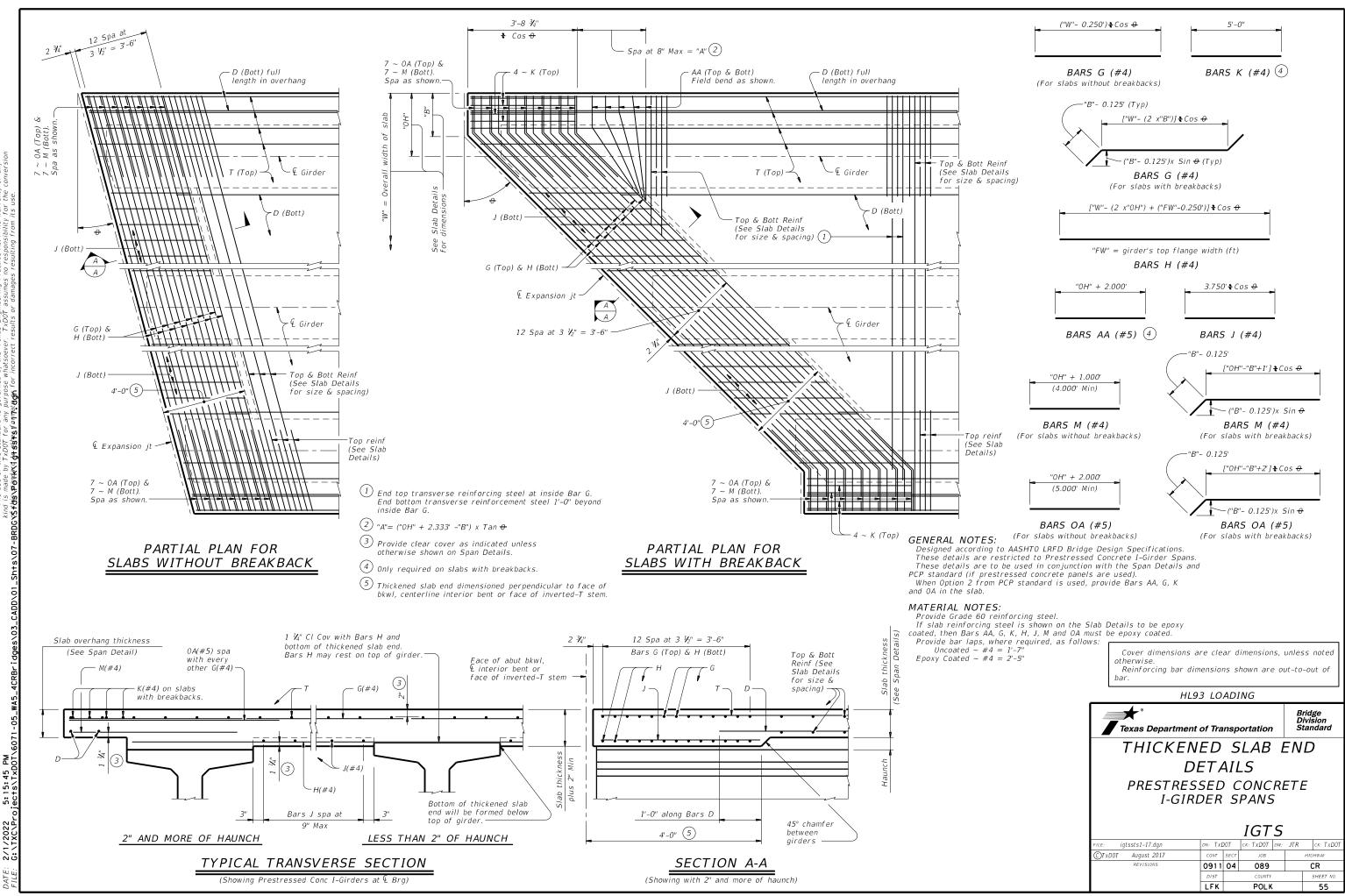
Optional designs must likewise conform.

2 Portion of full HL93.

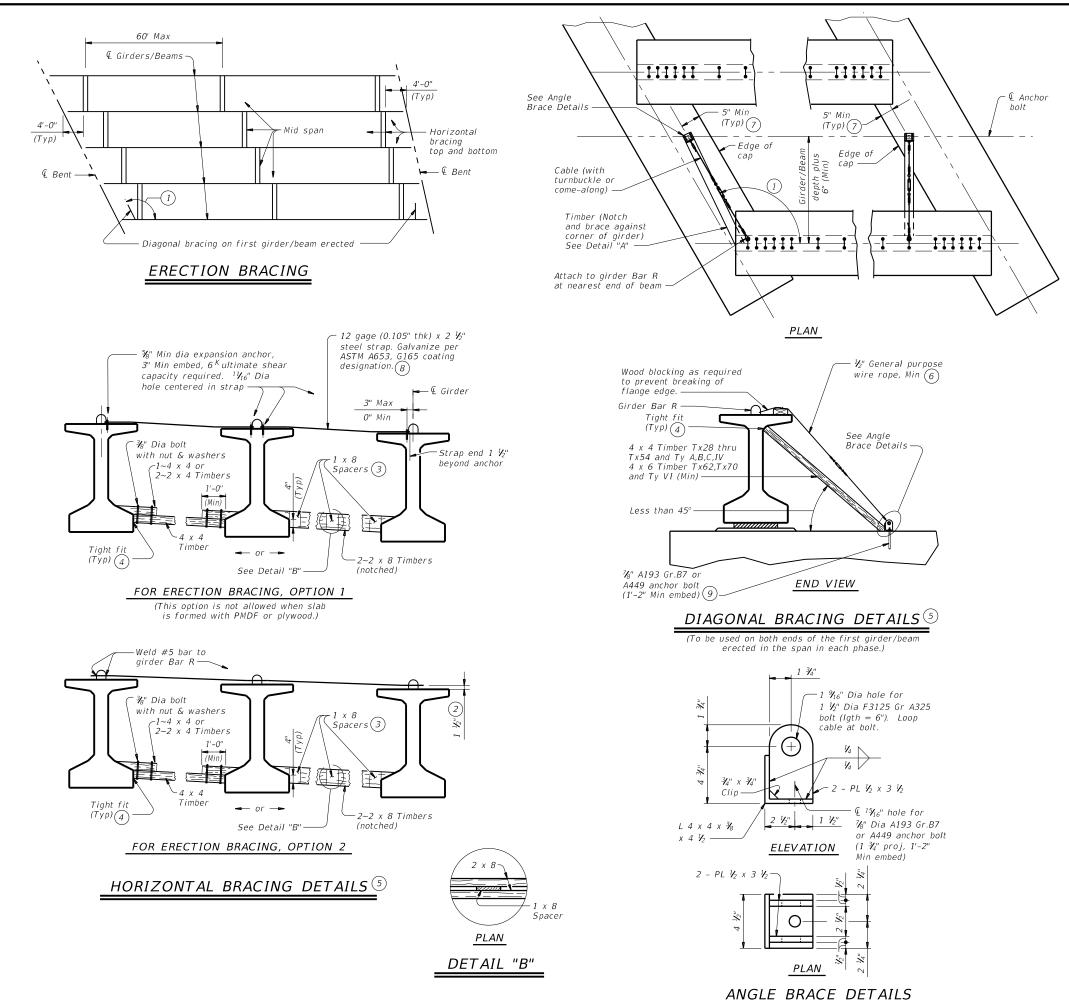
HL93 LOADING			SHEL	ET 2	2 OF 2
Texas Department	of Tra	nsp	ortation	D	ridge ivision tandard
	R S SI ROA	GT GT AD	ANDA	RD	
FILE: iq01stds-21.dan	DN: EF	_	CK: AJF DW:	EFC	ск: TAR
©TxDOT August 2017	CONT	SECT	JOB		HIGHWAY
REVISIONS 10-19: Redesigned girders.	0911	04	089		CR
1–21: Added Íoad rating.	DIST		COUNTY		SHEET NO.
	LFK		POLK		53



Texas Departm	ent of Transp	ortation	Bridge Division Standard
S	HEAR I	< <i>EY</i>	
	DETAI	LS	
PRESTR C	ONCRET	E I-GI	RDERS
PRESTR C	ONCRET	IGSK	
PRESTR C			
		IGSK	
ғи.е: igskstds-17.dgn	DN: TXDOT	IGSK	JTR CK: AES
ғиле: igskstds-17.dgn ©TxD0T August 2017	DN: TXDOT CONT SECT	IGSK CK: TXDOT DW: JOB	JTR ск: AES HIGHWAY



Μċ 5: 15: 45 | ects/TxD0



HAULING & ERECTION:

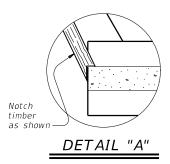
The Contractor's attention is directed to the possible lateral instability of prestressed concrete girders and beams over 130' long, especially during hauling and erection. The use of the following methods to improve stability is encouraged: Locate lifting devices at the maximum practical distance from girder ends; use external lateral stiffening devices during hauling and erection; lift with vertical lines using two machines; and take care in handling to minimize inertial and impact forces.

ERECTION BRACING: Erection bracing details shown are considered the minimum for fulfilling the bracing requirements of Item 425.

Required erection bracing must be placed immediately after erection of each girder and remain in place until additional bracing as required for slab placement is in place. This standard is needed in all cases to meet requirements for Slab Placement Bracing.

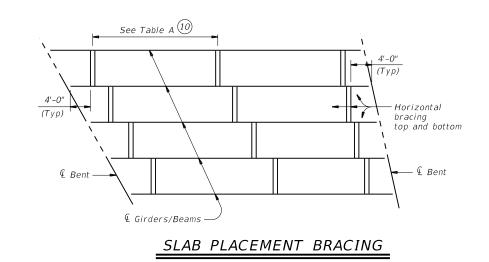
PHASED CONSTRUCTION:

Place erection and slab placement bracing for all girders in a phase as shown in these details. For phases after first, also place erection and slab placement bracing between outer girder of completed phase and adjacent girder of current phase. When the phase construction joint is between girders, top bracing can be omitted

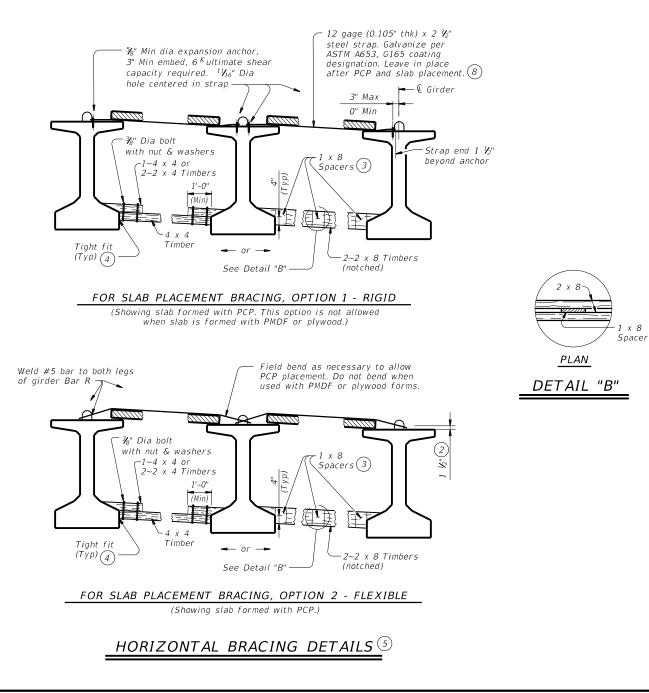


- (1) If angle shown exceeds 120 degrees, move diagonal brace to other side of girder/beam and place square to girder/beam. This may prevent exterior girder from being erected first.
- (2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R (See Sheet 2 of 2).
- 3 Clear distance between spacers must not exceed 3'. Nail together with 16d nails.
- (4) Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- 6 All hardware used with cable must be able to develop a minimum 25 kips breaking strength. Use thimbles at all loops in cable. Install cable clamps with saddles bearing against the live end and U-bolts bearing aginst the dead end.
- (7) It is acceptable to tie anchor bolts to cap reinforcement.
- 8 Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- (9) Anchor bolt may be drilled and epoxied in place. Provide 25k minimum pullout. Core drill hole.

SH	IEET 1	0	= 2	
Texas Department	t of Tra	nsp	ortation	Bridge Division Standard
MINIMUM	ERI	ΞC	TION	AND
BRACING	REC	QU	IREM	ENTS
PRESTRES I-GIRDER				
	٨	1E	BR(C)	
FILE: mebcsts1–17.dgn	DN: TX	DOT	CK: TXDOT DW:	ТхDOT ск: ТхDOT
CTxDOT August 2017	CONT	SECT	JOB	HIGHWAY
REVISIONS	0911	04	089	CR
	DIST		COUNTY	SHEET NO.
	LFK		POLK	56



		TAB	LE A		
OPTION 1-RI	GID BRACING (ST	EEL STRAP)	OPTION 2-FLEXI	BLE BRACING (NO	D. 5 OVER PCP)
	Maximum Bra	ncing Spacing		Maximum Bra	acing Spacing
Girder or Beam Type	Slab Overhang less than 4'-0" (11)	Slab Overhang 4'-0" and greater (11)	Girder or Beam Type	Slab Overhang less than 4'-0" (11)	Slab Overhang 4'-0" and greater (11)
T x 28	V_4 points	\mathcal{V}_4 points	Tx28	¼ points	¥₀ points
Tx34	V_4 points	\mathcal{V}_4 points	Tx34	¼ points	¥₀ points
T x 40	V_4 points	¥₀ points	Tx40	¼ points	¥₀ points
Tx46	V_4 points	¥₀ points	Tx46	V_4 points	V ₈ points
T×54	V_4 points	¥₀ points	Tx54	V_4 points	V ₈ points
Tx62	V_4 points	$\mathcal{V}_{\!\!\mathcal{B}}$ points	Tx62	\mathcal{V}_4 points	V ₈ points
Tx70	V₄ points	½ points	T x70	\mathcal{V}_4 points	¥₀ points
А	∛ ₈ points	V ₈ points	A	2.0 ft	1.5 ft
В	∛a points	$\mathcal{V}_{\!\!\mathcal{B}}$ points	В	3.0 ft	2.0 ft
С	$\mathcal{V}_{\!\!8}$ points	¥₀ points	С	4.5 ft	2.0 ft
IV	V_4 points	$V_{\!8}$ points	IV	\mathcal{V}_4 points	4.0 ft
VI	V_4 points	$V_{\!\!8}$ points	VI	V_4 points	4.0 ft





(2) Place and weld #5 bars as shown during erection. If forming deck with prestressed panels, bars can be temporarily removed, one at a time, during panel erection. Re-install bar prior to additional panel erection. Bars can rest on panels and be bent down and welded to girder Bars R.

(3) Clear distance between spacers must not exceed 3'. Nail together with 16d nails.

- 4 Use wedges as necessary to obtain tight fit. Nail wedges to timbers.
- (5) Pressure treated landscape timbers can not be used.
- (8) Prior to installing, field bend strap to lay flush on both girders' top flange and slope between flange tips.
- (10) Bracing spacing (V_4 and V_8 points) measured between first and last typical brace location.

(1) Measure slab overhang from centerline of girder or beam. When overhang varies in span, determine bracing spacing based on largest overhang.

SLAB PLACEMENT BRACING:

The details for slab placement bracing are considered minimum for fulfilling the requirements of Specification Items 422 and 425. Required slab placement bracing must remain in place until slab concrete has attained a compressive strength of 3000 psi.

GENERAL NOTES:

Bracing details for spans longer than 150' are not provided. The Contractor must submit proposed bracing details for such conditions to the Engineer for approval prior to erection.

Systems equal to or better than those shown may be used provided details of such systems are submitted to and approved by the Engineer prior to erection.

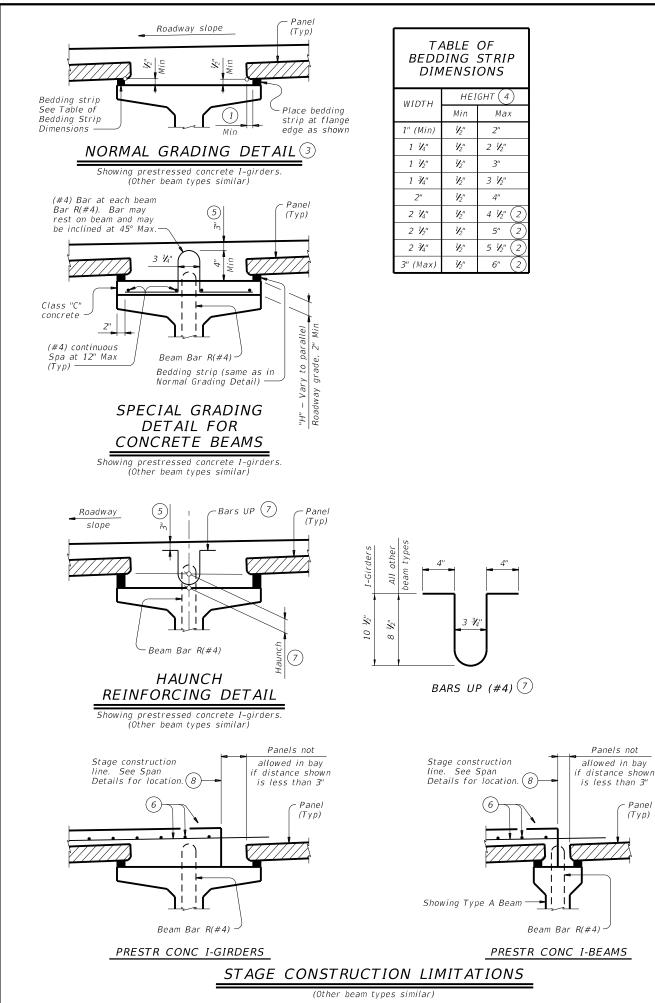
Use of these systems or details does not relieve the Contractor of the responsibility for the adequacy of the bracing and the safety of the structure.

Removal of bracing for short periods of time to align girders and beams is permissible.

All turn-buckles, come-alongs, anchors and other connections must be capable of developing the full strength of the cable shown.

Furnish anchor bolts and nuts in accordance with Item 449, "Anchor Bolts".

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ig(1) 2" Min for I-giders, 1 u_2 " Min for all other beam types.

 $\left(^{2}
ight)$ Allowed for I-girders, not allowed on other beam types

(3) To reduce the quantity of cast-in-place concrete, bedding strip thickness may be increased in $\mathcal{V}_4^{\prime\prime}$ increments. Bedding strips must be comprised of one layer. Bond bedding strips to the beams with an adhesive compatible with bedding strips. Bedding strips over 2.5" high may need to be bonded to panels. The same thickness strip must be used under any one panel edge and the maximum change in thickness between adjacent panels is $\mathcal{V}_4^{\prime\prime}$. Alternatively, bedding strips may be cut to grade. Panels may be supported by an alternate method, using a commercial product, if approved by the Engineer of Bridge Design, Bridge Division. If bedding strips exceed 6" high for I-Girders, 4" high for all other beam types, use Special Grading Detail for Concrete Beams or submit an alternate method to the Bridge Division for approval.

(4) Height must not exceed twice the width.

Panel

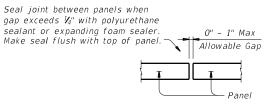
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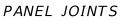
(5) Provide clear cover as indicated unless otherwise shown on Span Details.

- $\binom{6}{6}$ See Span Details and Thickened Slab End Details for top slab reinforcement and clear cover. Transverse top slab reinforcement may rest on top of prestressed concrete panels if necessary to maintain clear cover
- (7) Space Bars UP(#4) with Beam Bars R(#4) in all areas where measured haunch exceeds 3 \mathcal{V}_2 " with I-girders, and 3" for all other beam types. Epoxy coating for Bars UP is not required.

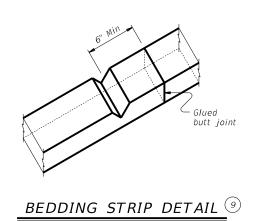
(8) Do not locate construction joints on top of a panel.

deep, in the top of the bedding strips at 8' o.c..





(Panel reinforcing not shown for clarity. The gap cannot be considered as a panel fabrication tolerance. Adjust panel placement to minimize joint openings.)



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CONSTRUCTION NOTES:

Erected panels must bear uniformly on bedding strips of extruded polystyrene placed along top flange edges.

Placing panels to minimize joint openings is recommended. If additional blocking is needed, special grading details for supporting the panels and extra reinforcing between beam and slab will be considered subsidiary to deck construction.

Bars U, shown on PCP-FAB, may be bent over or cut off if necessary. Care must be taken to ensure proper cleaning of

construction debris and consolidation of concrete material under the edges of the panels. Bedding strips must be placed at beam flange edges so that adequate space is provided for the mortar to flow a minimum of 1 $\frac{1}{2}$ under the panels as the slab concrete is placed. To allow the proper amount of mortar to flow between

beam and panel, the minimum vertical opening must be at least V_2 ". Roadway cross-slope reduces the opening available for entry of the mortar. Bedding strips varying in thickness across the beam are therefore required.

For clear span between U-beams less than or equal to 18", see Permissible Slab Forming Detail on Miscellaneous Slab Detail sheets, UBMS.

MATERIAL NOTES:

Provide Grade 60 reinforcing steel in the cast-in-place slab. See Table of Reinforcing Steel for size and spacing of reinforcement.

If the top and bottom layer of reinforcing steel is shown on the Span Details to be epoxy coated, then the D, E, P, & Z bars must be epoxy coated

Provide bar Laps, where required, as follows: Uncoated ~ #4 = 1'-7"

Epoxy Coated $\sim #4 = 2'-5''$

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

Panel placement may follow either Option 1 or Option 2 except Option 1 must be used if the skew exceeds 45 degrees.

Use of Prestressed Concrete Panels is not permitted for horizontally curved steel plate or tub girders. See Span Details for other possible restrictions on their use.

These details are to be used in conjunction with the Span Details, PCP-FAB and other applicable standard drawings.

When panel support (bedding strips) deviates from what is shown herein, provide details signed and sealed by a professional Engineer.

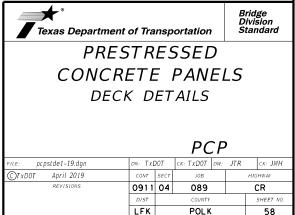
Any additional reinforcement or concrete required on this standard is considered subsidiary to the bid Item "Reinforced Concrete Slab".

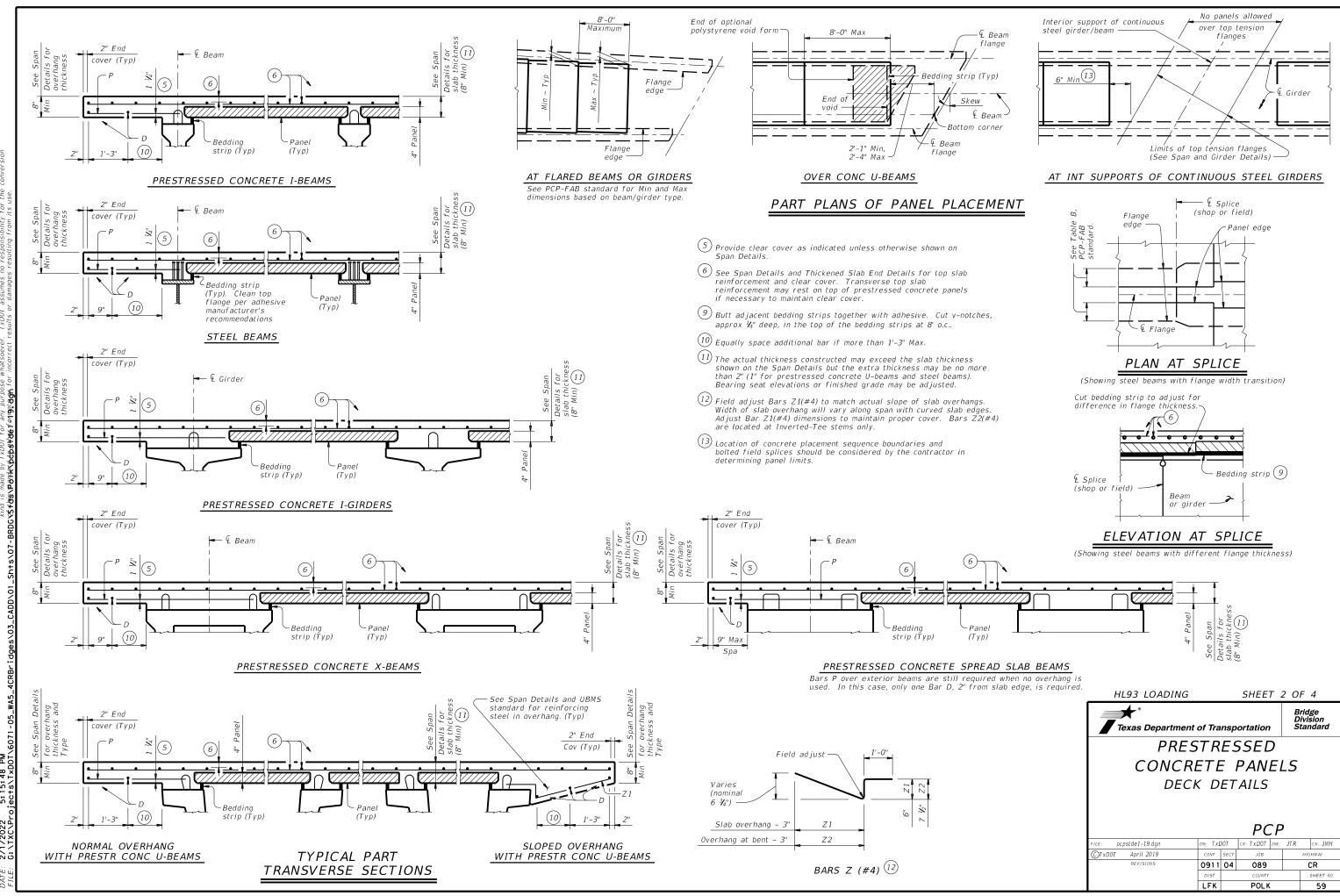
Cover dimensions are clear dimensions, unless noted otherwise

Reinforcing bar dimensions shown are out-to-out of bar

HL93 LOADING

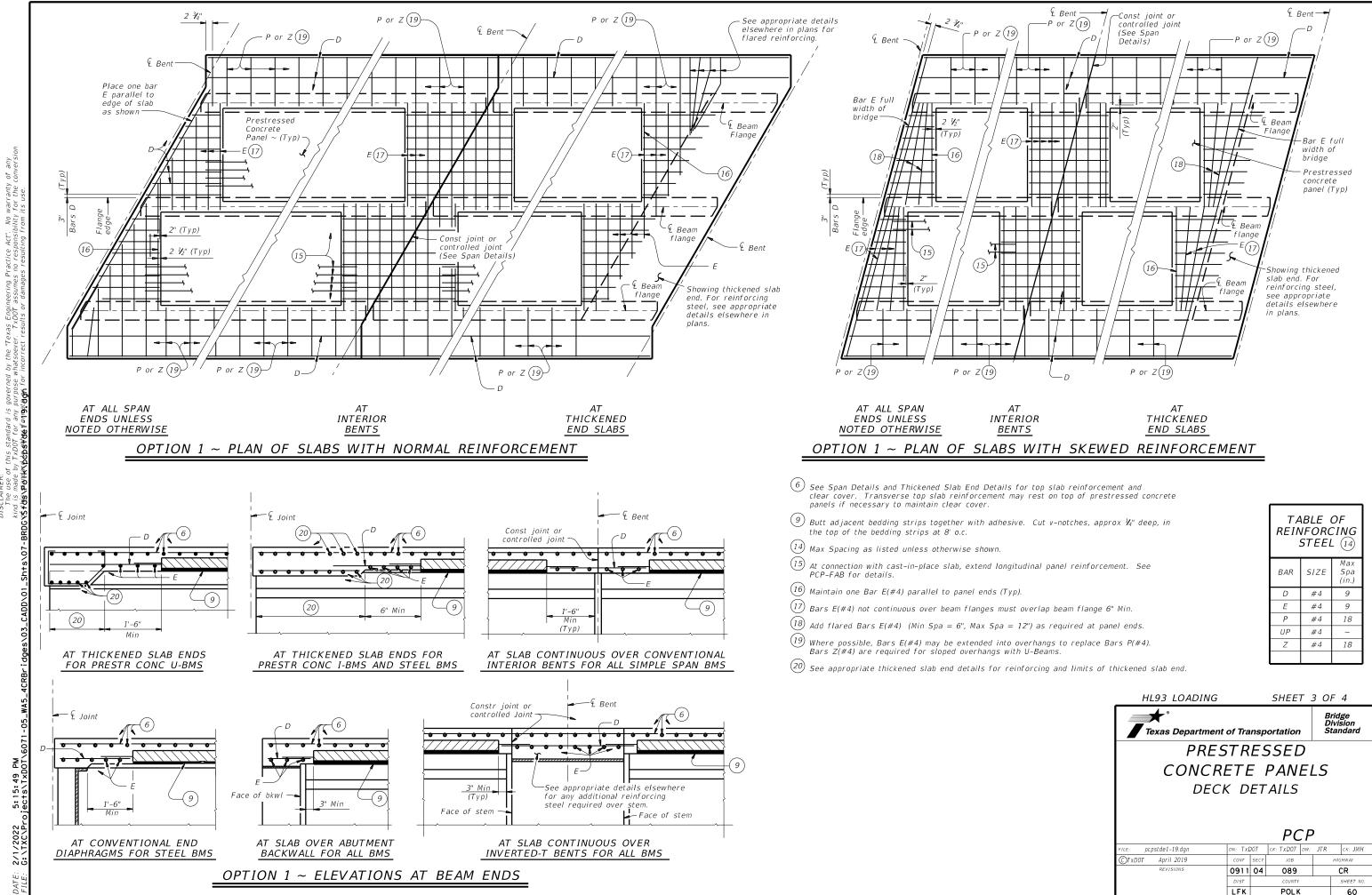
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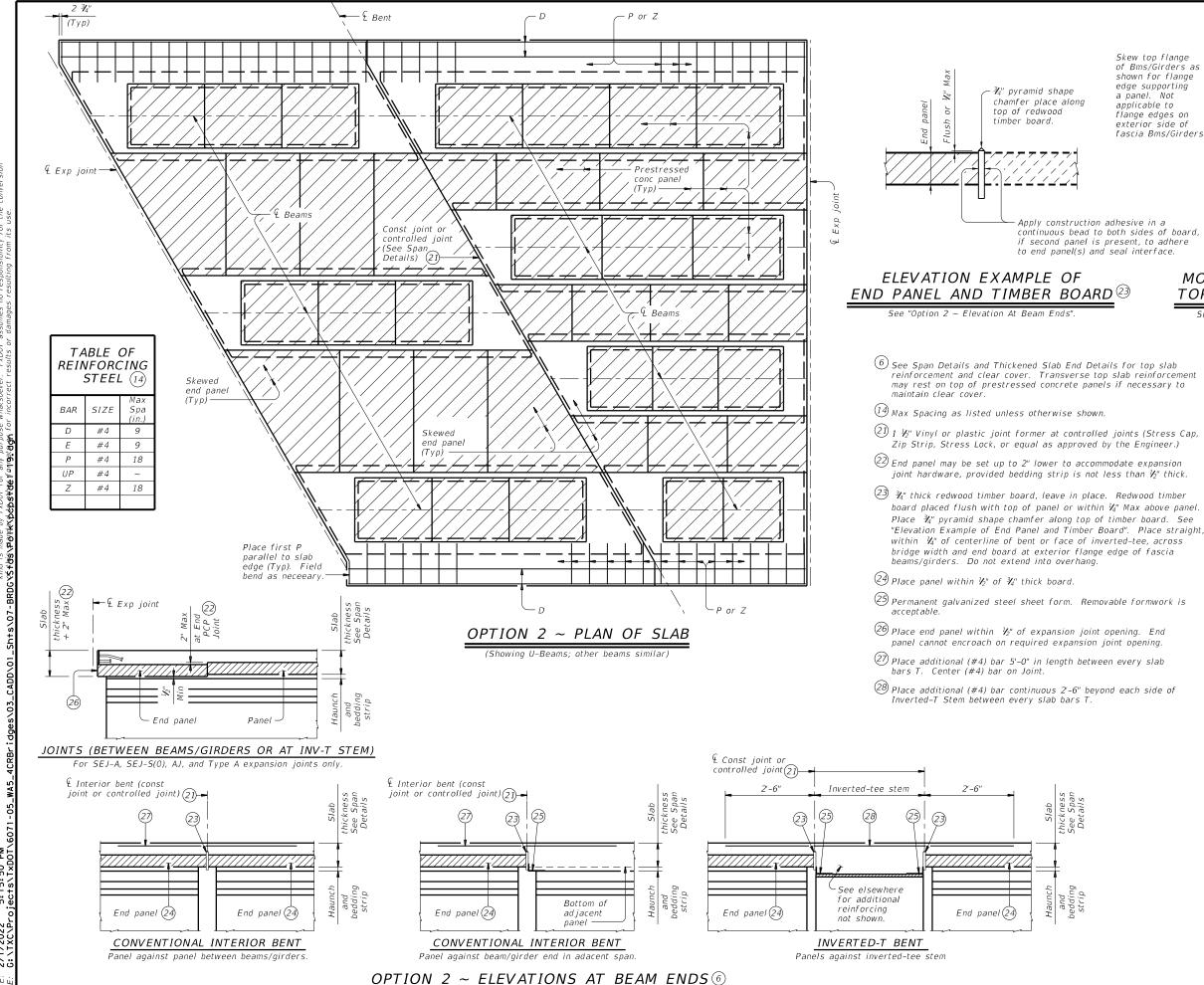




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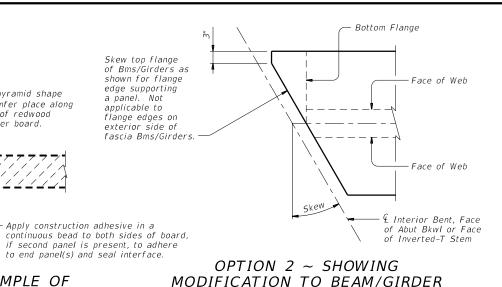
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TOP FLANGE FOR SKEWS OVER 5° Showing I-Bm/I-Girder, U-Bms and Steel Bms similar

SPECIAL OPTION 2 CONSTRUCTION NOTES:

When Option 2 is chosen bottom mat of thickened end slab reinforcing is not required. Use the same top mat as shown on the Thickened Slab End Details sheet. Placing panels adjacent to expansion joints and bent centerlines prior to completing interior panel placement is recommended. Saw cutting panels to fit is acceptable when approved by the Engineer. Minimum distance from a saw cut edge to a panel strand is 1 $\frac{1}{2}$ ". Do not extend the longitudinal panel reinforcement

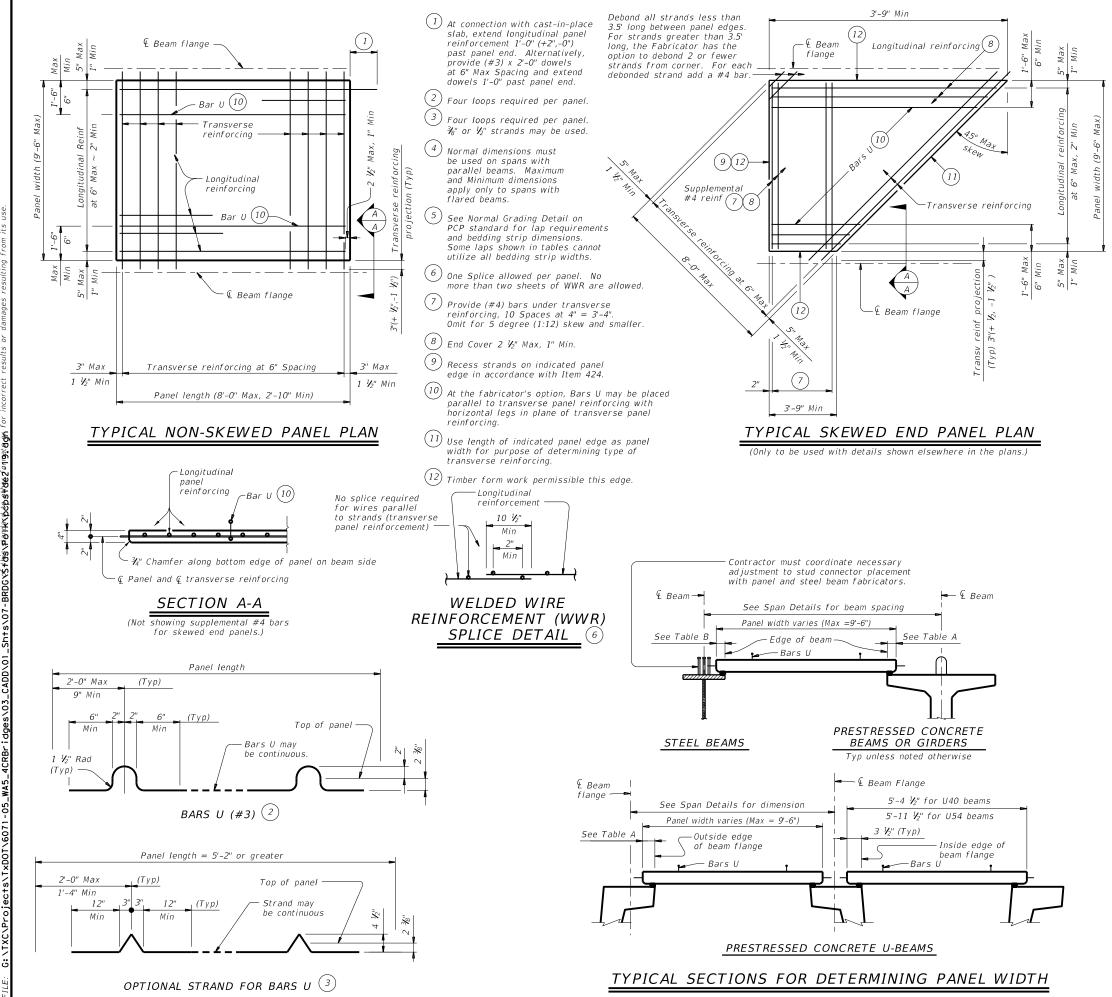
into the cast-in-place slab. Top flanges of beams and girders on skewed bridges must be modified as shown on this drawing. The Contractor is responsible for coordinating this modification with the beam fabricator prior to submitting shop drawings for approval.

Fabricator may optionally skew the whole end. When electing to skew whole end, girder end details and bearing type at conventional interior bent must be changed to use condition at abutment. Fabricator must coordinate change in bearing type, bearing centerline location, and dowel location with Engineer and Contractor. Show appropriate changes on girder and bearing shop drawings.

Bending of an of study of expansion joints shown on standards AJ, SEJ-A and SEJ-S(0) is permissible if necessary to clear top of end panels. The Contractor is responsible for coordinating modifications with the joint fabricator. Submit shop drawings for approval when modifications to expansion joint hardware are made.

Bedding strips under skewed end panels must conform to the requirements of Item 422 except their minimum compressive strength must be 60 psi. Provide Bars AA, G, K and OA from standard IGTS in the slab.

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	TABLE	E A (4	4)(5)
Beam Type	Normal (In.)	Min (In.)	Max (In.)
А	3	2 ½	3 ½
В	3	2 ½	3 ½
С	4	3	4 V ₂
IV	6	4	7 ½
VI	6 ¹ / ₂	4 ¹ ⁄2"	8 ¼
U40 - 54	5 ½	5 ½	7
Tx28-70	6	5	7 ½
XB20 - 40	4	3	4 ½
XSB12 - 15	4	3	4 ¹ / ₂

BLE B	4 (5	5)
Normal (In.)	Min (In.)	Max (In.)
2 ¥4	2 ¹ / ₂	2 ¥4
3 ¼	3	3 ¼
4	3	4 ¥4
5	3 ½	6 ¼
	Normal (In.) 2 ⅔	$ \begin{array}{c c} Normal & Min \\ (In.) & (In.) \\ 2 & & 2 & \\ 3 & & & \\ 4 & & & \\ \end{array} $

GENERAL NOTES:

Provide Class H concrete for panels. Release strength f'ci=3,500 psi. Minimum 28 day strength f'c=5,000 psi.

Provide $rac{3}{4}$ " chamfer along bottom edge of panel on beam side.

Do not use epoxy-coated reinforcing steel bar or strand in panels.

Remove laitance from top panel surface.

Finish top of panel to a roughness between a No. 6 and No. 9 concrete surface profile, inclusive, as specified by the International Concrete Repair Institute (ICRI).

Shop drawings for the fabrication of panels will not require the Engineer's approval if fabrication is in accordance with the details shown on this standard.

A panel layout which identifies location of each panel must be developed by the Fabricator. Permanently mark each panel in accordance with the panel layout. A copy of the layout is to be provided to the Engineer.

TRANSVERSE PANEL REINFORCEMENT:

For panel widths over 5', use $cegar{8}$ " or $ar{2}$ " Dia (270k) prestressing strands with a tension of 14.4 kips per strand.

For panel widths over 3'-6" up to and including 5', use $rac{3}{8}$ " or $rac{1}{2}$ " Dia

(270k) prestressing strands with a tension of 14.4 kip per strand. Optionally,
 (#4) Grade 60 reinforcing bars may be used in lieu of prestressed strands.
 For panel widths up to 3'-6", use (#4) Grade 60 reinforcing bars (prestressed

strands alone are not allowed). Place transverse panel reinforcement at panel centroid and space at 6" Max.

LONGITUDINAL PANEL REINFORCEMENT:

Any of the following options may be used for longitudinal panel reinforcement

1. (#3) Grade 60 reinforcing steel at 6" Max Spacing. No splices allowed. 2. ⅔" Dia prestressing strands at 4 ½" Max Spacing

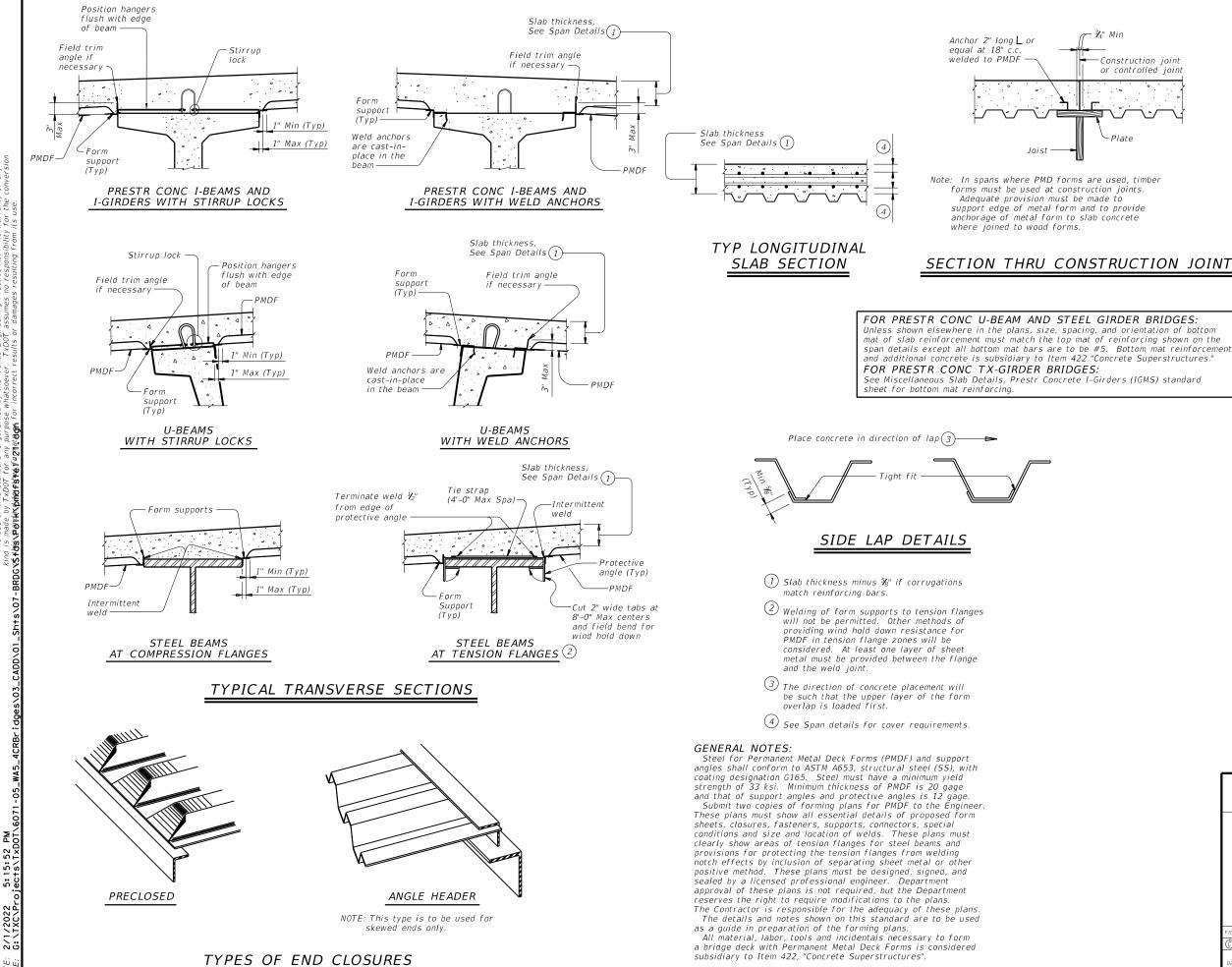
(unstressed). No splices allowed.

3. V_2 " Dia prestressing strands at 6" Max Spacing (unstressed). No splices allowed.

4. Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) providing 0.22 sq in per foot of panel width. Wires larger than D11 not permitted Provide transverse wires to ensure proper handling of reinforcing. One splice per panel is allowed. See WWR Splice Detail

No combination of longitudinal reinforcement options in a panel is allowed. Place longitudinal panel reinforcement above or below transverse panel reinforcement. Must be placed above transverse panel reinforcement for skewed end panels with supplemental (#4) reinforcement.

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-Construction joint or controlled joint



Plate

DESIGN NOTES:

As a minimum, PMDF and support angles must be designed for the dead load of the form, reinforcement and concrete plus 50 psf for construction loads. Flexural stresses due to these design loads must not exceed 75 percent of the yield strength of the steel. Allowable stress for weld metal must be 12,400 psi. Maximum deflection under the weight of forms reinforcement and concrete or 120 psf, whichever is greater, shall not exceed the following:

> 1/180 of the form design span, but not more than 0.50", for design spans of 10' or less.

1/240 of the form design span, but not more than 0.75", for design spans greater than 10'.

1/240 of the form design span, but not more than 0.75", for all design spans of railroad overpass bridge spans fully or partially over railroad right-of-way, and for all bridge spans of railroad underpass structures.

The form design span must not be less than the clear distance between beam flanges, measured parallel to the form flutes, minus 2".

CONSTRUCTION NOTES:

Form sheets must not be permitted to rest directly on the top of beam flanges. Form sheets must be securely fastened to form supports and must have a minimum bearing length of one inch at each end. Form supports must be placed in direct contact with beam flanges

All attachments must be made by permissible welds, screws, bolts, clips or other means shown on the the forming plans. All sheet metal assembly screws must be installed with torque-limiting devices to prevent stripping. Only welds or bolts must be used to support vertical loads.

Welding and welds must be in accordance with the provisions of Item 448, "Structural Field Welding", pertaining to fillet welds. All welds must be made by a qualified welder

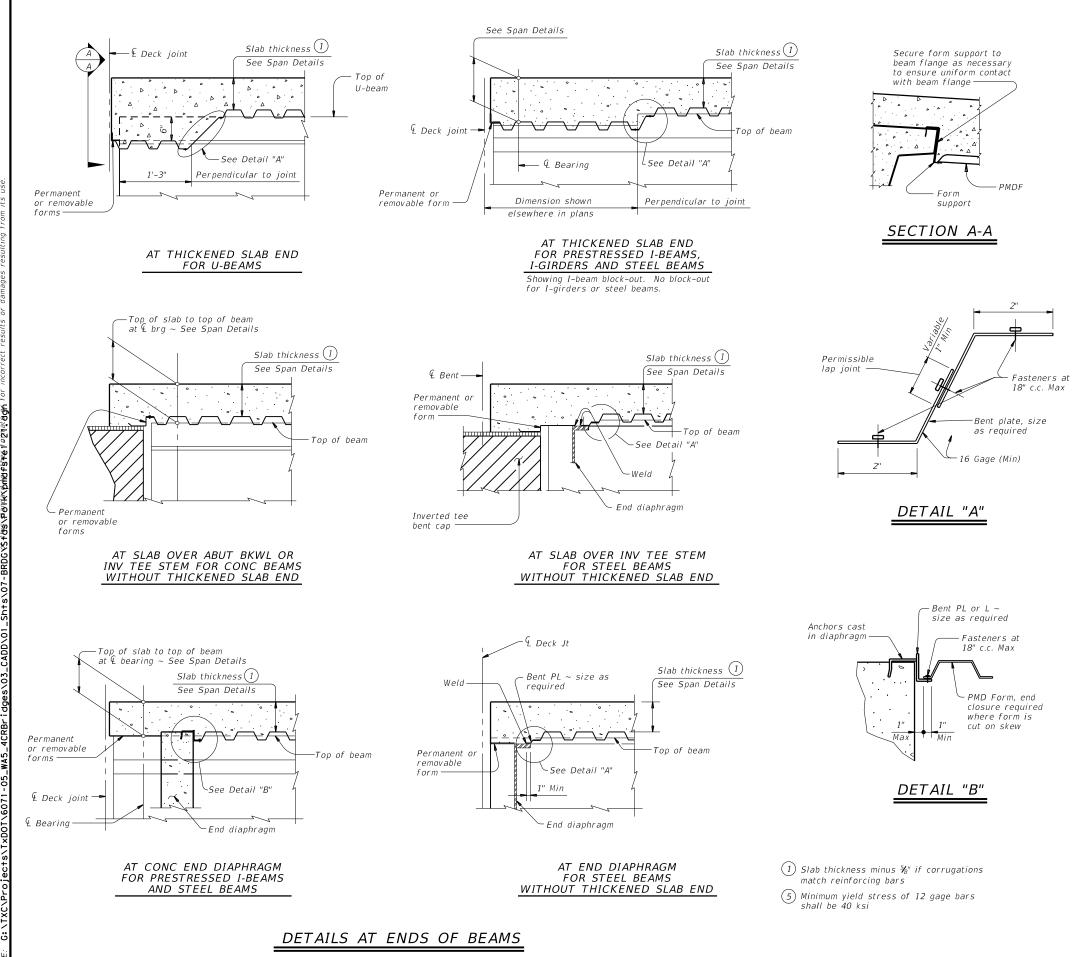
in accordance with Item 448. All permanently exposed form metal, where the galvanized coating has been damaged, must be thoroughly cleaned and repaired in accordance with Item 445, "Galvanizing". Minor heat discoloration in areas of welds need not be touched up.

Flutes must line up uniformly across the entire width of the structure where main reinforcing steel is located in the flute.

Construction joints will not be permitted unless shown on the plans. The location of and forming details for any construction joint used must be shown on the forming plans. Forms below a construction joint must be removed after curing of the slab. A sequence for uniform vibration of concrete

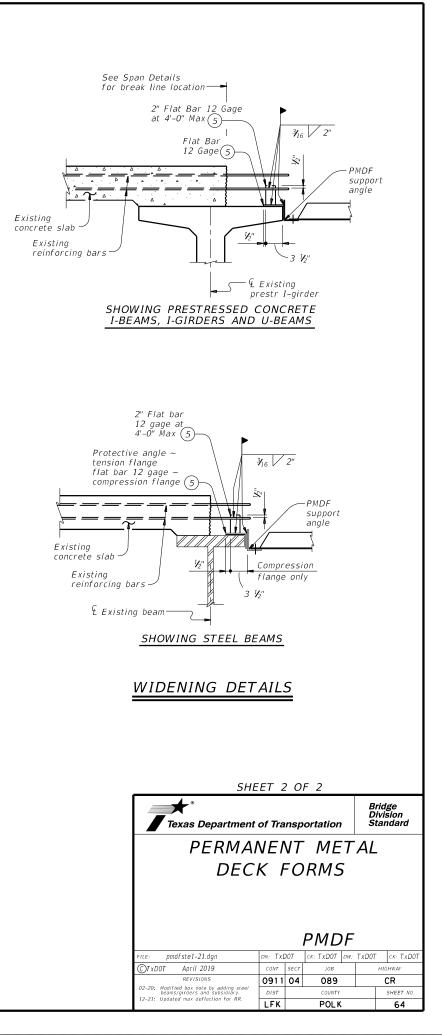
must be approved by the Engineer prior to concrete placement. Attention must be given to prevent damage to the forms, yet provide proper vibration to prevent voids or honeycomb in the flutes and at headers and/or construction joints.

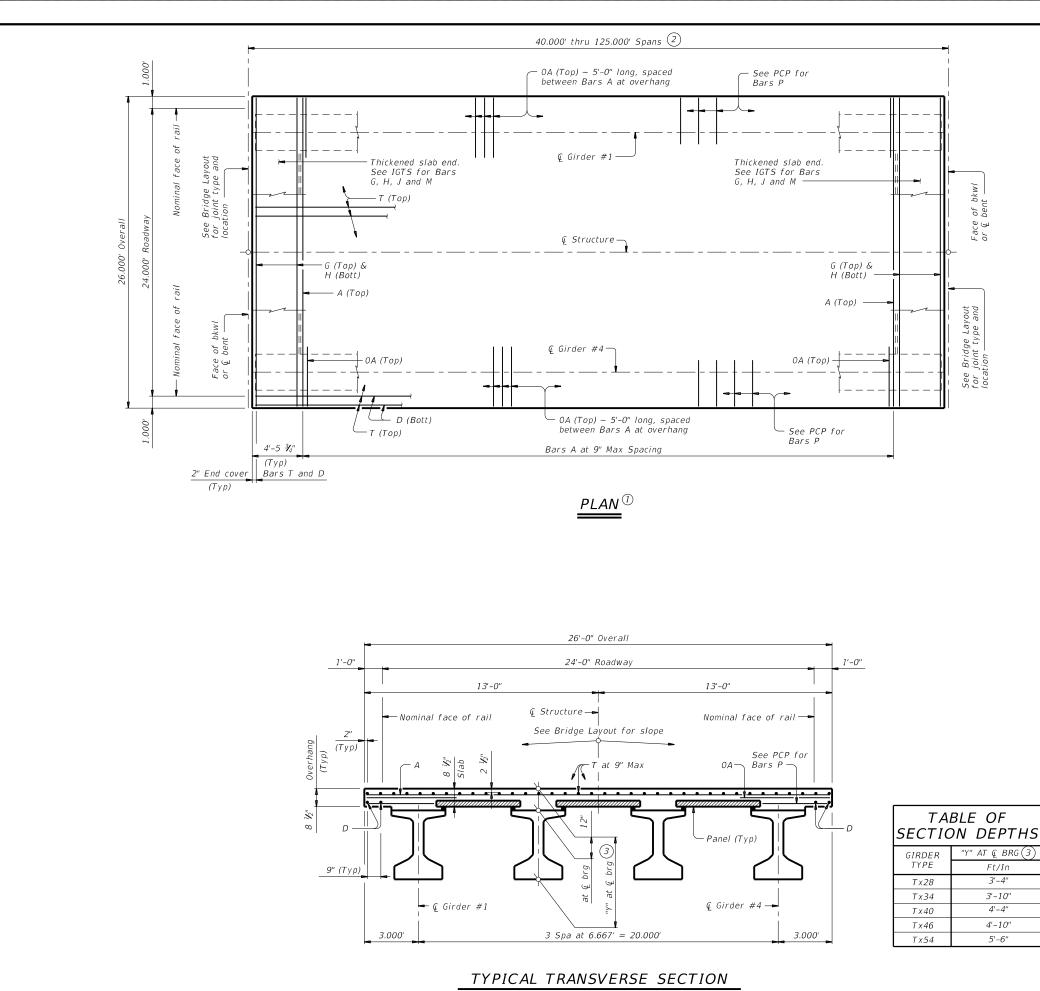
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			FMD				
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(Showing girder type Tx46)

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BAR	SIZE
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D	#4
G	#4
Н	#4
J	#4
М	#4
0A	#5
Р	#4
Т	#4

 If multi-span units (with slab continuous over interior bents) are indicated on the Bridge Layout, see standard IGCS for adjustment to slab reinforcement and quantities.

Span lengths for Prestressed Concrete I-Girder type: Type Tx28 for spans lengths 40.000' thru 75.000'. Type Tx34 for spans lengths 40.000' thru 85.000'. Type Tx40 for spans lengths 40.000' thru 100.000'. Type Tx46 for spans lengths 40.000' thru 115.000'. Type Tx54 for spans lengths 40.000' thru 125.000'.

(3) "Y" value shown is based on theoretical girder camber, dead load deflection from an 8 ½" concrete slab, a constant roadway grade, and using precast panels (PCP). The Contractor will adjust this value as necessary for any roadway vertical curve and/or if precast overhang panel (PCP(0)) option is use.

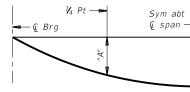
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TABLE OF DEAD LOAD DEFLECTIONS

TYPE	Tx28 GII	RDERS	TYPE	Tx34 GII	RDERS
Span Length	"A"	"B"	Span Length	"A"	"B"
Ft	Ft	Ft	Ft	Ft	Ft
40	0.007	0.010	40	0.004	0.006
45	0.012	0.017	45	0.007	0.010
50	0.019	0.027	50	0.011	0.016
55	0.028	0.040	55	0.017	0.024
60	0.041	0.057	60	0.024	0.034
65	0.056	0.079	65	0.033	0.047
70	0.077	0.108	70	0.046	0.064
75	0.102	0.143	75	0.061	0.085
			80	0.079	0.111
			85	0.102	0.143

ΓΥΡΕ	Tx40 GII	RDERS	
Span Length	"A"	"B"	
Ft	Ft	Ft	
40	0.003	0.004	
45	0.005	0.007	
50	0.007	0.010	
55	0.011	0.016	
60	0.016	0.022	
65	0.022	0.031	
70	0.030	0.042	
75	0.040	0.056	
80	0.052	0.073	
85	0.066	0.093	
90	0.084	0.118	
95	0.105	0.147	
100	0.130	0.182	

TIONS	IONS						TAB	LE OF	ESTIMA	ATED G	UANTI	TIES			
	Tx46 GI	RDERS	TYPE Tx54 GIRDERS			TYPE Tx54 GIRDER							sed Concrete		
Span Length	"A"	"B"	Span Length	"A"	"B"	1	SPAN LENGTH	REINF CONCRETE SLAB	ABUT T0 (4)	INT BT	ABUT TO (4)	TOTAL ⁵ REINF STEEL			
Ft	Ft	Ft	Ft	Ft	Ft				INT BT	INT BT	ABUT 🔍				
40	0.002	0.003	40	0.001	0.002		Ft	SF	LF	LF	LF	Lb			
45	0.004	0.005	45	0.002	0.003		40	1,040	158.00	158.00	158.00	2,392			
50	0.005	0.007	50	0.004	0.005		45	1,170	178.00	178.00	178.00	2,691			
55	0.008	0.011	55	0.005	0.007		50	1,300	198.00	198.00	198.00	2,990			
60	0.011	0.015	60	0.007	0.010		55	1,430	218.00	218.00	218.00	3,289			
65	0.015	0.021	65	0.010	0.014		60	1,560	238.00	238.00	238.00	3,588			
70	0.021	0.029	70	0.014	0.019		65	1,690	258.00	258.00	258.00	3,887			
75	0.027	0.038	75	0.018	0.025		70	1,820	278.00	278.00	278.00	4,186			
80	0.036	0.050	80	0.024	0.033		75	1,950	298.00	298.00	298.00	4,485			
85	0.046	0.064	85	0.030	0.042		80	2,080	318.00	318.00	318.00	4,784			
90	0.057	0.080	90	0.038	0.053		85	2,210	338.00	338.00	338.00	5,083			
95	0.071	0.100	95	0.047	0.066		90	2,340	358.00	358.00	358.00	5,382			
100	0.088	0.124	100	0.058	0.082		95	2,470	378.00	378.00	378.00	5,681			
105	0.108	0.151	105	0.071	0.100		100	2,600	398.00	398.00	398.00	5,980			
110	0.130	0.182	110	0.086	0.121		105	2,730	418.00	418.00	418.00	6,279			
115	0.156	0.219	115	0.103	0.144		110	2,860	438.00	438.00	438.00	6,578			
			120	0.123	0.172	1	115	2,990	458.00	458.00	458.00	6,877			
			125	0.145	0.203		120	3,120	478.00	478.00	478.00	7,176			
			-			-	125	3,250	498.00	498.00	498.00	7,475			



DEAD LOAD DEFLECTION DIAGRAM

Calculated deflections shown are due to the concrete slab on interior girders only (Ec = 5000 ksi). Adjust values as required for exterior girders and if optional slab forming is used. These values may require field verification.



(4) Fabricator will adjust lengths for girder slopes as required.

⁵ Reinforcing steel weight is calculated using an approximate factor of 2.3 lbs/SF.

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications.

[']Multi-span units, with slab continuous over interior bents, may be formed with the details shown on this sheet and standard IGCS.

See IGTS standard for Thickened Slab End details and

guantity adjustments. See PCP and PCP-FAB for panel details not shown. See PCP(0) and PCP(0)-FAB for precast overhang panel details if this option is used. See IGMS standard for miscellaneous details.

See applicable rail details for rail anchorage in slab. See PMDF standard for details and quantity adjustments if this option is used.

This standard does not support the use of transition hents

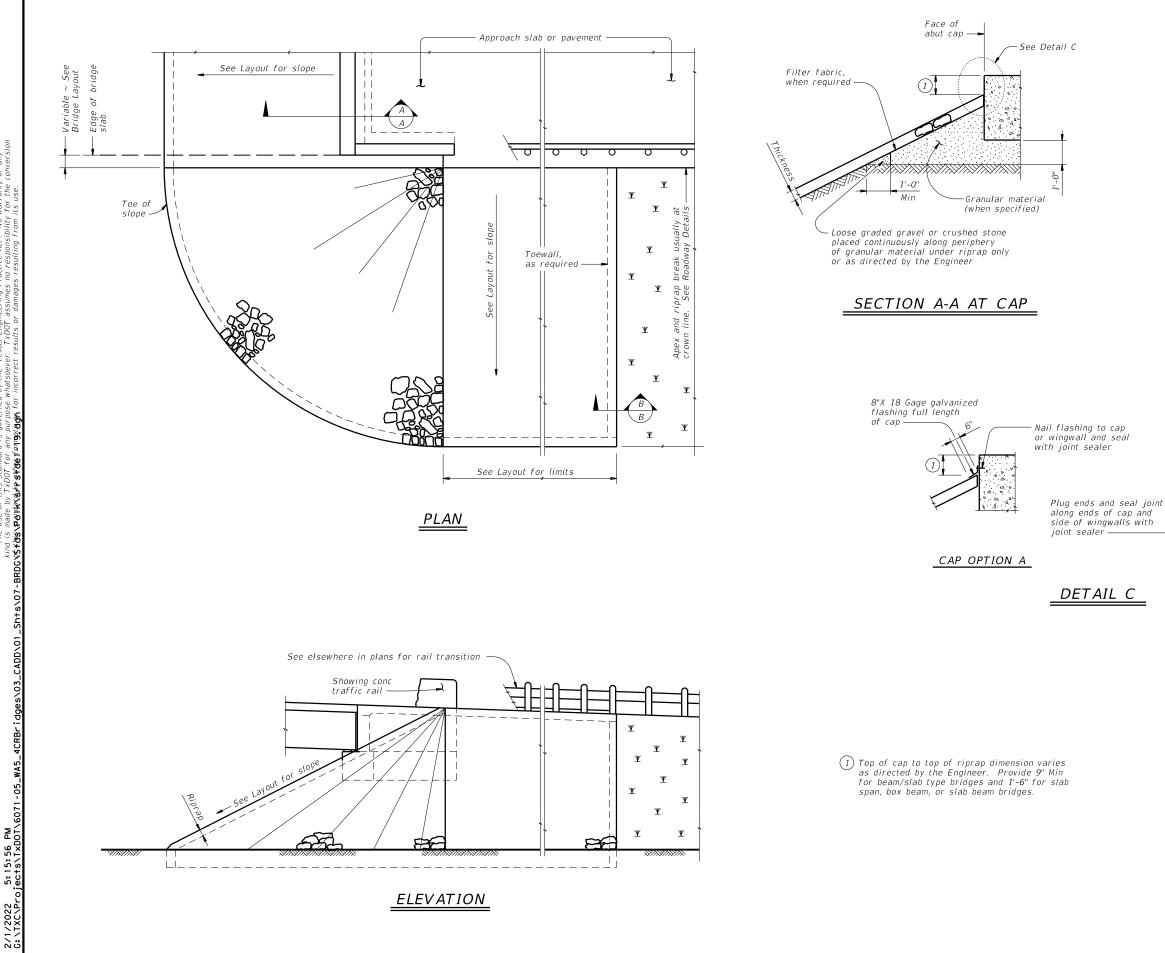
Cover dimensions are clear dimensions, unless noted otherwise.

MATERIAL NOTES: Provide Class S concrete (f'c = 4,000 psi). Provide Class S (HPC) concrete if shown elsewhere in the plans.

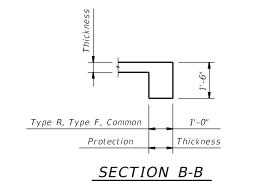
Provide Grade 60 reinforcing steel.

Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated $\sim #4 = 1'-7''$ Epoxy coated $\sim #4 = 2'-5''$ Deformed Welded Wire Reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A, D, OA, P or T unless noted otherwise.

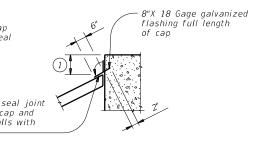
HL93 LOADING	2 OF 2							
Texas Department of Transportation Standard								
PRESTRESSED CONCRETE I-GIRDER SPANS (TYPE Tx28 THRU Tx54) 24' ROADWAY								
FILE: sig01sts-19.dqn	DN: JM		5IG-24	JTR CK: TAR				
CTXDOT August 2017	CONT	SECT	JOB	HIGHWAY				
REVISIONS	0911	04	089	CR				
10-19: Increased "X" and "Y" Values	DIST COUNTY			SHEET NO.				
	LFK POLK 66							



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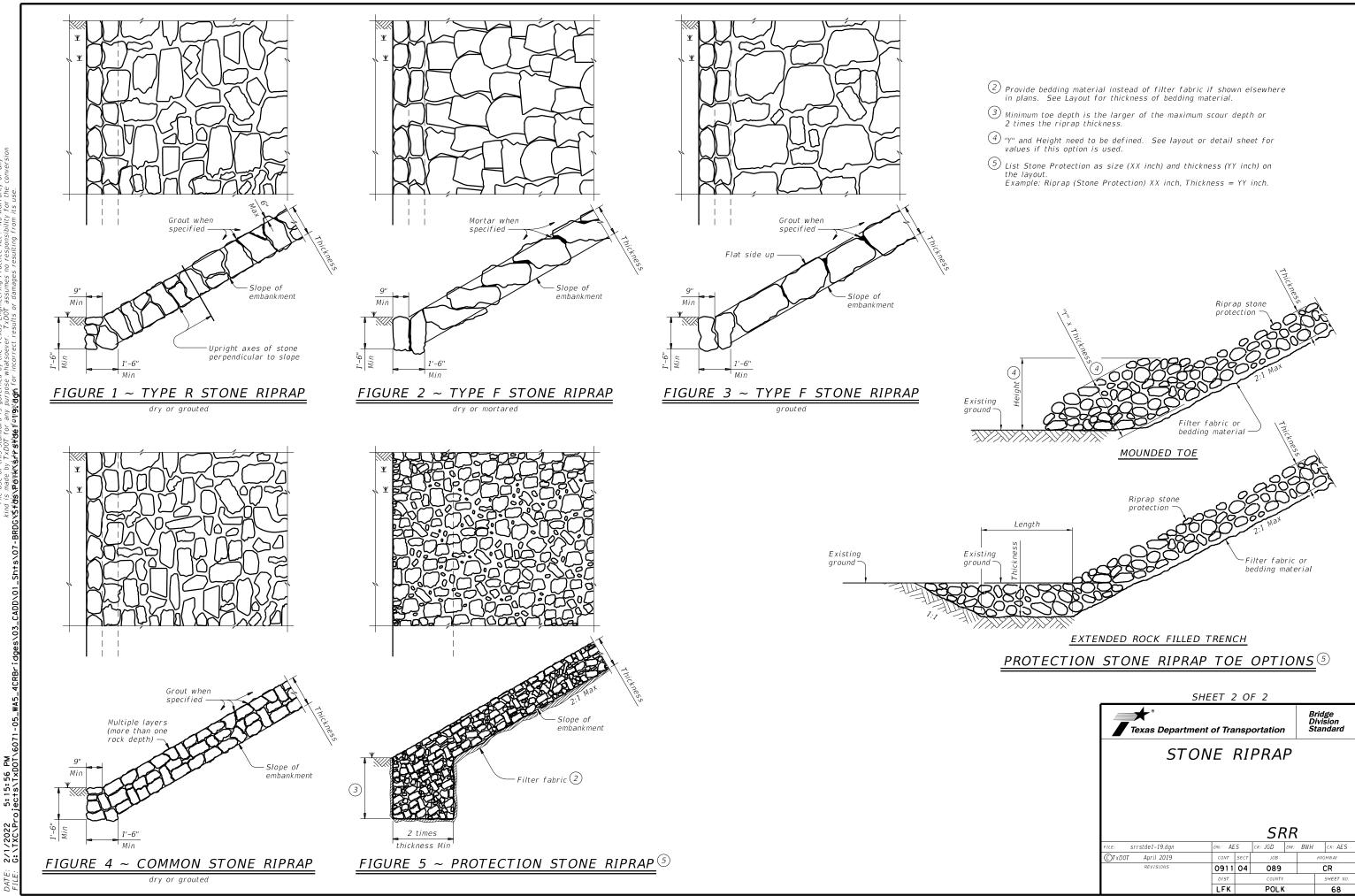
Provide toewall when shoulder drain is located adjacent to limits of stone riprap. Omit toewall when thickness of protection riprap is greater than 18".





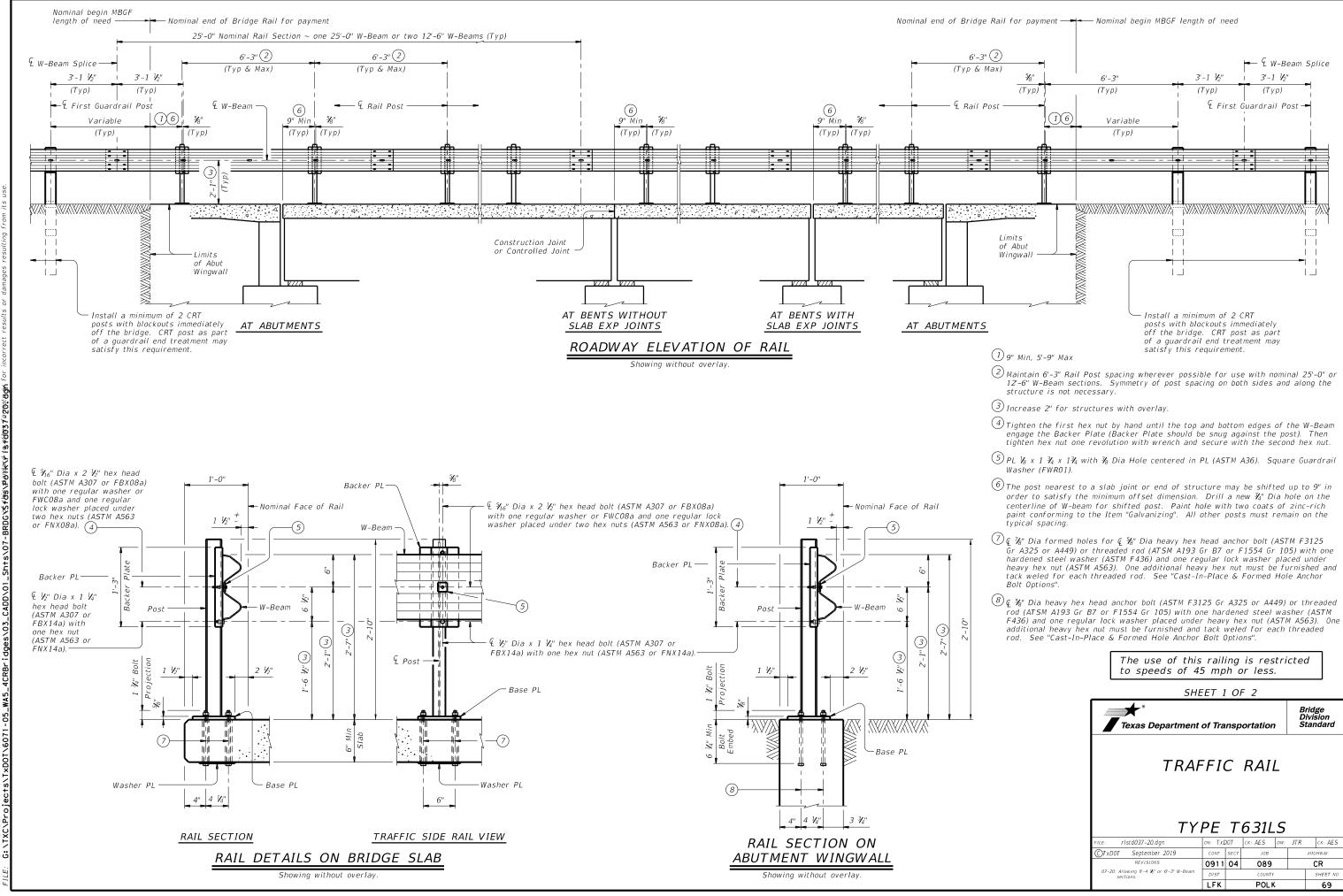
GENERAL NOTES: Refer to Item 432, "Riprap" for stone size and gradation, and construction details. See Layout for limits and thickness of riprap specified. See elsewhere in plans for locations and details of shoulder drains.

SHEET 1 OF 2							
Texas Department	n	Bridge Division Standard					
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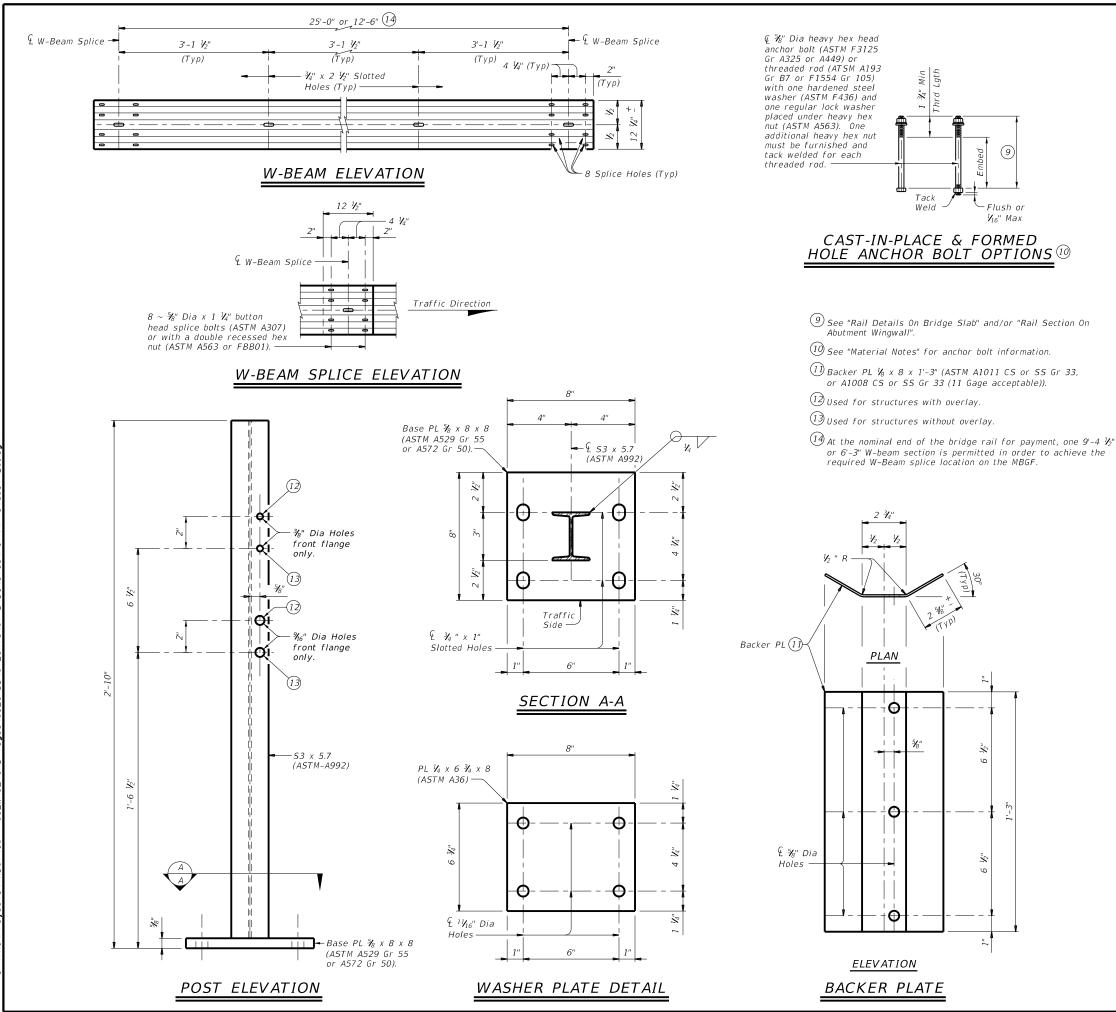
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	The use of this railing is restricted to speeds of 45 mph or less.							
	SHEET 1 OF 2							
	Texas Department	of Tra	nsp	ortation	Div	dge ision ndard		
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07-2	20: Allowing 9'-4 №" or 6'-3" W-Beam sections.	DIST		COUNTY		SHEET NO.		
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MBGF AND END TREATMENT NOTES:

This traffic railing must be anchored by metal beam guard fence (MBGF) and/or guard fence end treatments. Determine MBGF length of need in accordance with the Roadway Design Manual, unless otherwise specified. The minimum MBGF length of need required for anchoring the railing is: SGT; or DAT plus 12.5' of MBGF, as applicable. Provide CRT posts as shown in "Roadway Elevation of Rail."

CONSTRUCTION NOTES:

Face of rail post must be plumb unless otherwise approved by the Engineer. Post must be perpendicular to adjacent roadway grade. Use epoxy mortar under post base plates if gaps larger than \mathcal{V}_{16} " exist.

Fully anchored guardrail must be attached to each end of rail. A metal beam guard fence transition is not used with this rail. At the Contractor's option anchor bolts may be an adhesive

anchor system. See "Material Notes". Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

It is recommended to show a Rail Layout with rail posts and W-beam splices. Fabricator must submit erection drawings to the Engineer for approval.

Round or chamfer exposed edges of rail post and backer plate to approximately $~{\rm V_{16}''}$ by grinding.

Shop drawings are not required for this rail.

MATERIAL NOTES:

Galvanize all steel components.

Anchor bolts for base plate must be $\frac{9}{6}$ " Dia ASTM F3125 Gr A325 or A449 bolts (or ASTM A193 Gr B7 or F1554 Gr 105 threaded rods with one tack welded heavy hex nut each) with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements.

Optional adhesive anchorage system must be $\frac{1}{6}$ " Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 $\frac{1}{6}$ ". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing."

W-beam must meet the requirements of Item 540, "Metal Beam Guard Fence" except as modified in the plans. The Contractor may furnish rail elements of 25'-0" or 12'-6" (Nominal) lengths and a single rail element of $9'-4 \frac{14}{2}$ " or 6'-3" (Nominal) length.

W-Beam must have slotted holes at 3'-1 $\frac{1}{2}$ ".

Some part numbers from the "Task Force 13" Guide to Standardized Highway Barrier Hardware have been furnished for quick reference.

GENERAL NOTES:

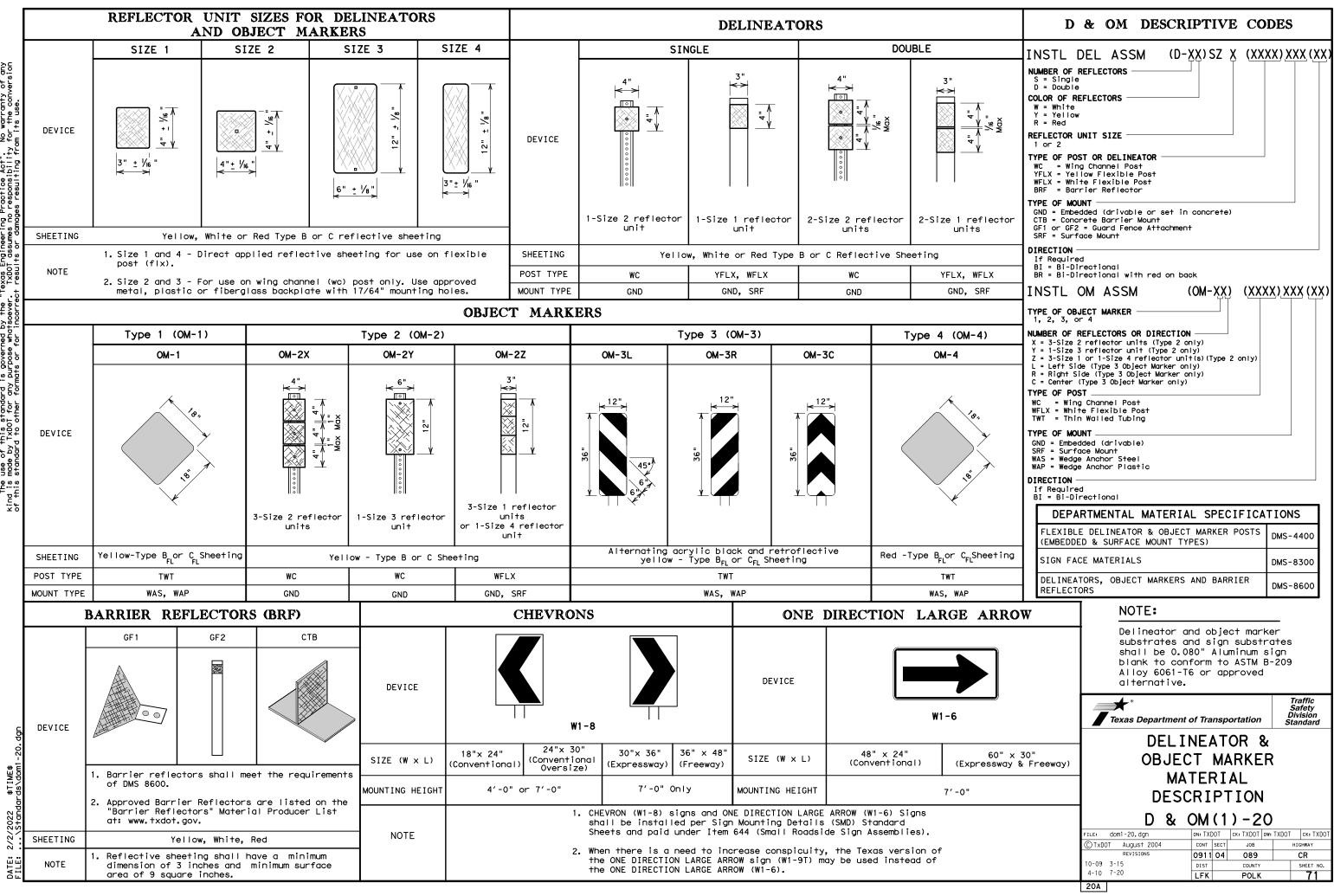
This railing has been successfully evaluated by full-scale crash test to meet MASH TL-2 criteria. This railing can be used for speeds of 45 mph and less.

This rail is designed to deflect approximately 2' to 2'-6" as it contains and redirects the errant vehicle. This rail may not be installed on top of or behind curbs that project above finished grade, on bridges with expansion joints providing more than 5" movement, on retaining walls, or on grade separations and interchanges.

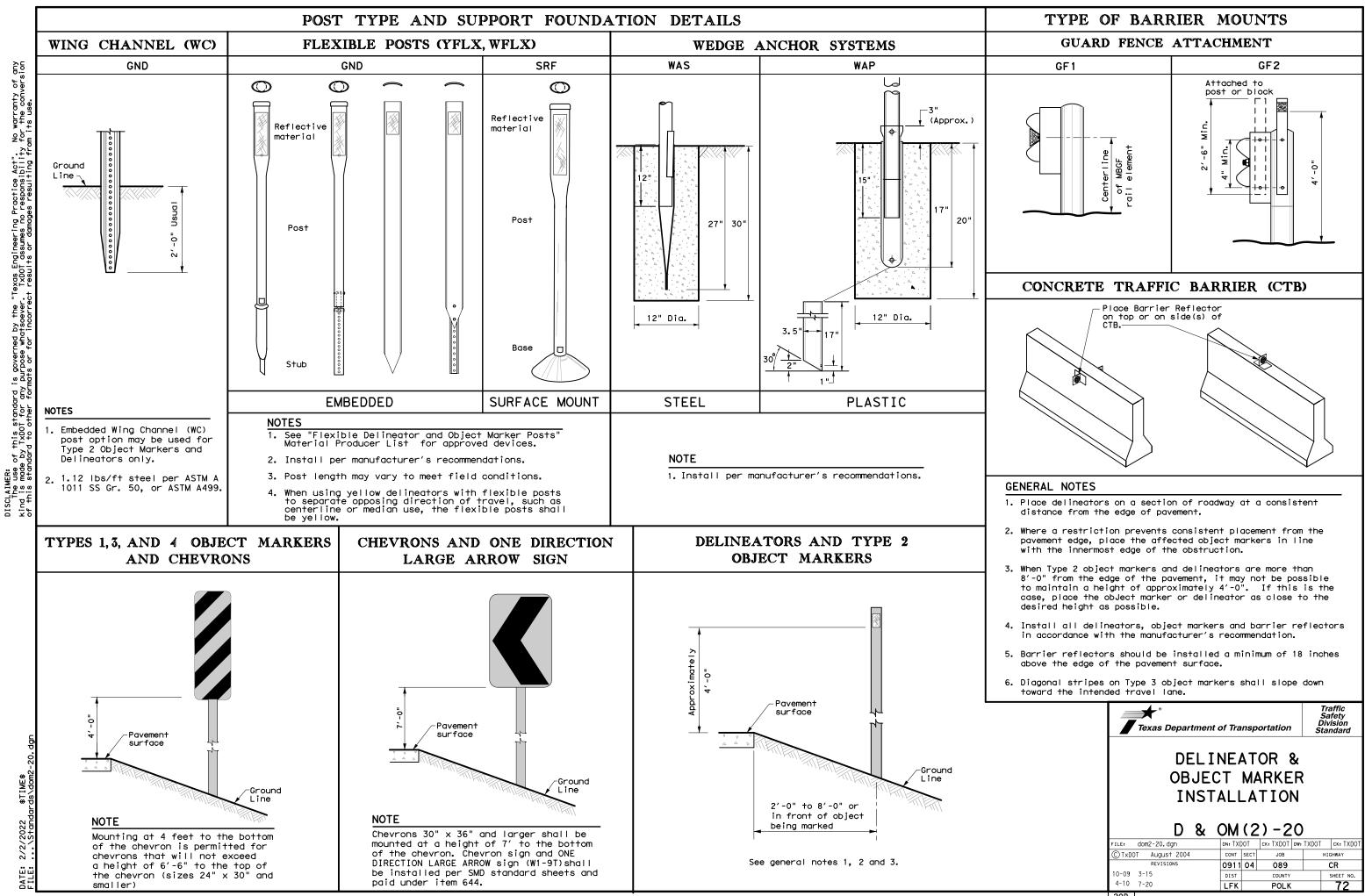
Repairs to impact-damaged post and base plate unit are not permitted. Replace all impact-damaged posts with a new post and base plate unit.

Average weight of railing with no overlay: 13 plf total.

SHEET 2 OF 2								
Texas Department of Transportation								
TRAFFIC RAIL								
FILE: rIstd037-20.dgn DN: TxD0T CK: AES DW: JTR CK: AES								
CTxDOT September 2019 CONT SECT JOB HIGHWAY								
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07-20: Allowing 9'-4 🖉 or 6'-3" W-Beam sections.	DIST	DIST COUNTY			SHEET NO.			
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"Texas Engineering Practice Act". No warranty of any . TxDDT assumes no responsibility for the conversion ct results or damages resulting from its use. SCLAIMER: The use of this standard is governed by the nd is made by TXDOT for any purpose whatsoever this standard to other formar's or for incorre



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MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

	WITH ADVISORY SPEEDS	
Amount by whi Advisory Spee		
is less than Posted Speed	Turn Curve (30 MPH or less) (35 MPH or more)	
5 MPH & 10 MPH	RPMs PMs	
15 MPH & 20 MPH	 RPMs and One Direction Large Arrow sign RPMs and Chevrons; or RPMs and One Direction La Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons. 	
25 MPH & more	 RPMs and Chevrons; or RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons RPMs and Chevrons 	
SUGO	ESTED SPACING FOR DELINEATORS ON HORIZONTAL CURVES	
	ONE DIRECTION LARGE ARROW SIGN	
	Curve Spacing	
straightawas (Approacting EDE 2A EDE	Extension of the centerline of the tangent section of approach lane	
	NOTE ONE DIRECTION LARGE ARROW (W1-6) sign should be located at approximately and perpendicular to the extension of the centerline of the tangent section of approach lane.	
SU	GGESTED SPACING FOR CHEVRONS ON HORIZONTAL CURVES	
	Point of curvature B B B B B B B B B B B B B B B B B B B	
	NOTE	

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of	Radius of	Spacing		Spacing	
urve	Curve	in Curve	in Straightawa	v in	
				^y Curve	Frwy/Exp.Ramp
		A	2A	В	
1	5730	225	450		Acceleration/Deceleration
2	2865	160	320		
3	1910	130	260	200	
4	1433	110	220	160	Truck Escape Ramp
5	1146	100	200	160	
6	955	90	180	160	
7	819	85	170	160	Bridge Rail (steel or
8	716	75	150	160	Concrete)and Metal Beam Guard Fence
9	637	75	150	120	
0	573	70	1 40	120]
1	521	65	130	120	Concrete Traffic Barrier (CTB)
2	478	60	120	120	or Steel Traffic Barrier
3	441	60	120	120	1
4	409	55	110	80	Cable Barrier
5	382	55	110	80	1
6	358	55	110	80	11
9	302	50	100	80	Guard Rail Terminus/Impact
23	249	40	80	80	Head
29	198	35	70	40	11
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for each Advisory Speed (MPH).

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OR AND OBJECT MARKER APPLICATION AND SPACING

LEGEND

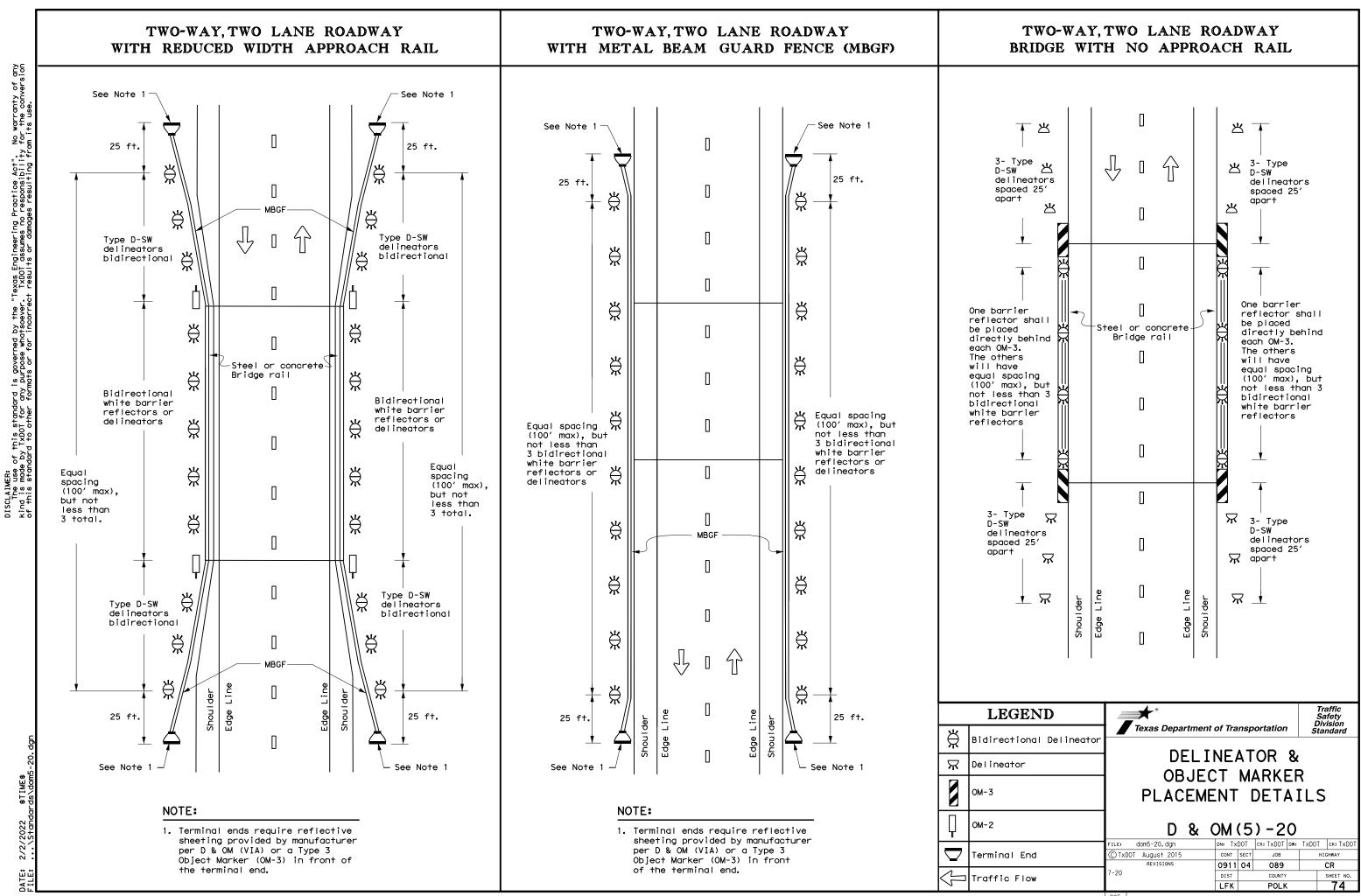
	-
REQUIRED TREATMENT	MINIMUM SPACING
RPMs	See PM-series and FPM-series standard sheets
Single delineators on right side	See delineator spacing table
Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4))	100 feet on ramp tangents Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves)
Double delineators (see Detail 3 on D&OM(4))	100 feet (See Detail 3 on D & OM (4))
Single red delineators on both sides	50 feet
Bi-Directional Delineators when undivided with one lane each direction Single Delineators when multiple lanes each direction	Equal spacing (100'max) but not less than 3 delineators
Barrier reflectors matching the color of the edge line	Equal spacing 100′ max
Reflectors matching the color of the edge line	Every 5th cable barrier post (up to 100'max)
Divided highway - Object marker on approach end Undivided 2-lane highways - Object marker on approach and departure end	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6)
Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail	See D & OM(5)
Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge	Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5)
Type 2 Object Markers	See Detail 2 on D & OM(4)
Double yellow delineators and RPMs	See Detail 1 on D & OM (4)
Single delineators adjacent to affected lane for full length of transition	100 feet

s indicated otherwise, the delineator or barrier reflector color shall conform e color of the pavement edge line on the side of the road where the delineators rrier reflectors are placed.

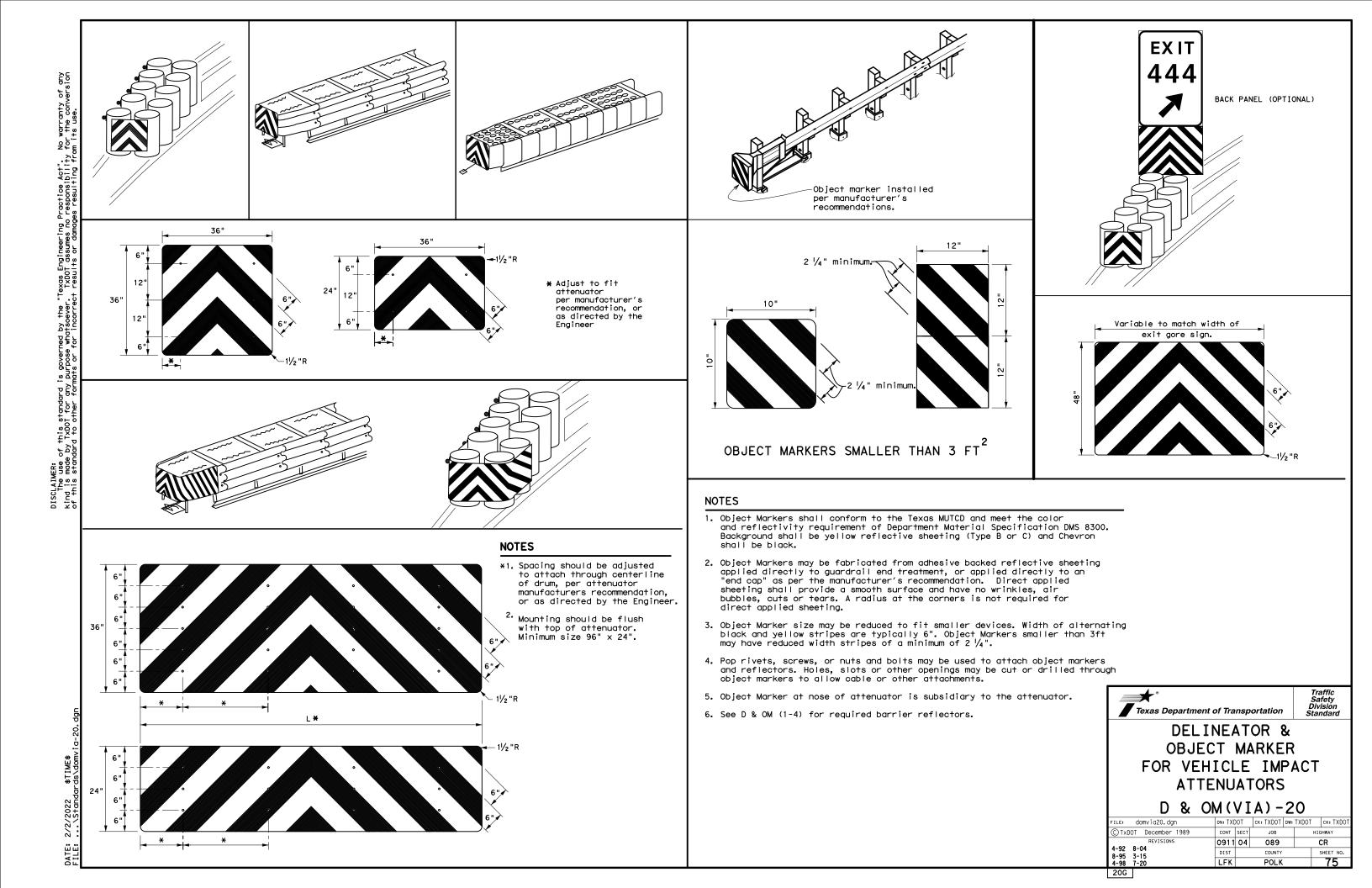
er reflectors may be used to replace required delineators.

le red delineators may be mounted on the back side of delineator posts for wrong driver applications

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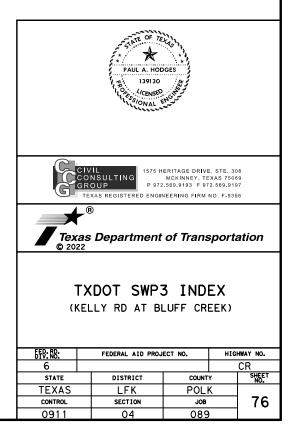


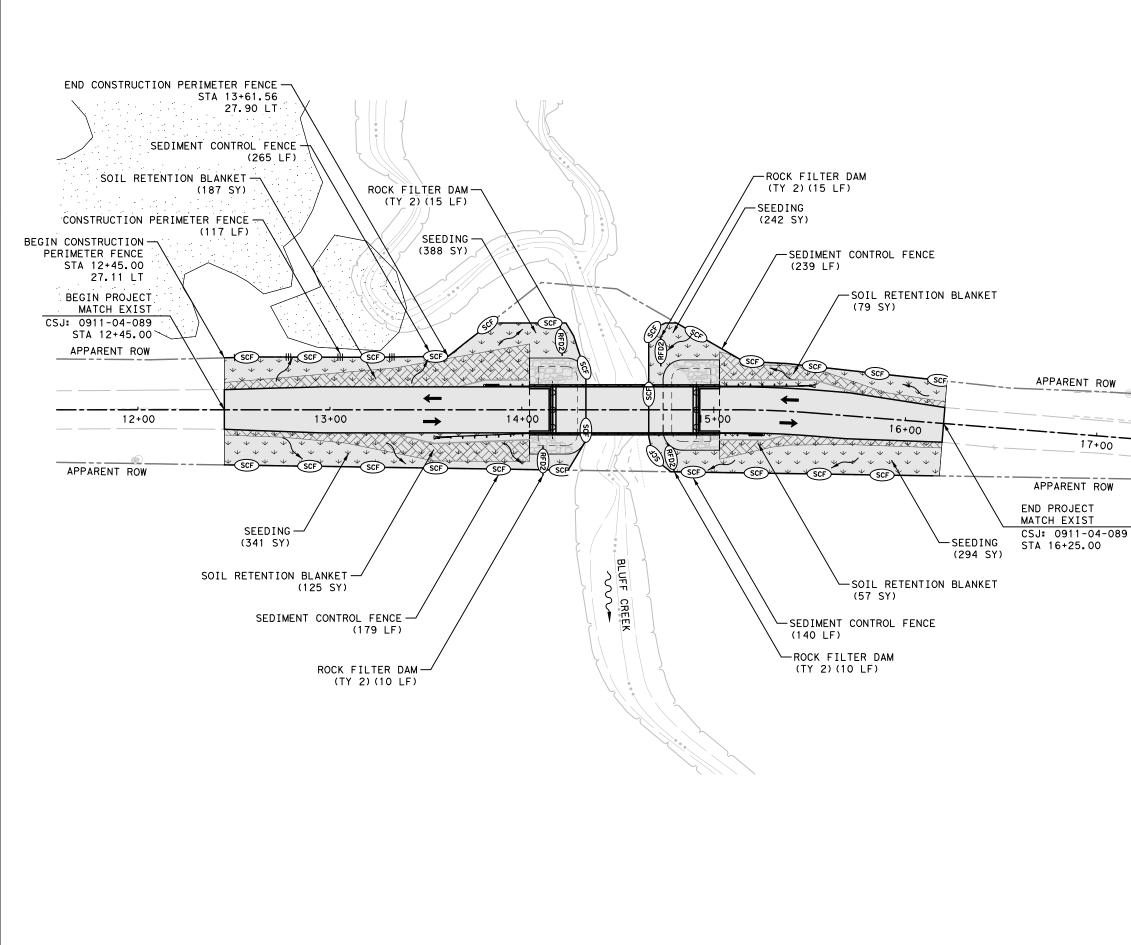
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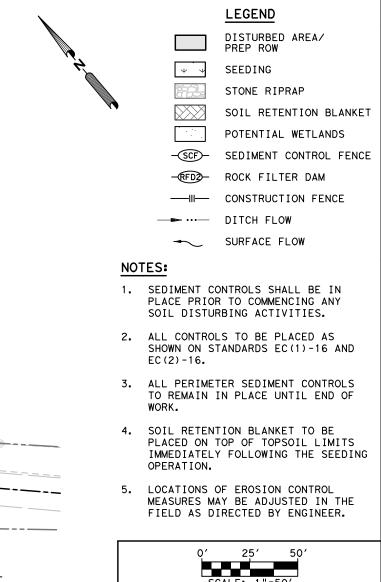


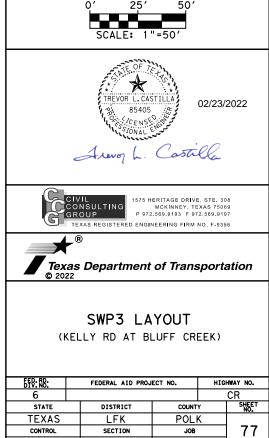
KELLY RD AT BLUFF CREEK (CSJ: 0911-04-089)

THE PROPOSED WORK OF THIS PROJECT IS TO REPLACE THE BRIDGE AND APPROACHES AT KELLY ROAD OVER BLUFF CREEK. THE TOTAL DISTURBED AREA IS 0.503 ACRES. THE DISTURBED AREA IN THIS PROJECT AND THE CONTRACTOR PROJECT SPECIFIC LOCATIONS (PSLS) WITHIN 1 MILE OF THE PROJECT LIMITS FOR THE CONTRACT WILL FURTHER ESTABLISH THE AUTHORIZATION REQUIREMENTS FOR STORM WATER DISCHARGES. AS THE DISTURBED AREA INCLUDING PSLS IS LESS THAN 1 ACRE, THE TPDES CGP DOES NOT APPLY; HOWEVER, THE CONTRACTOR SHALL PLACE BMPS AS DIRECTED TO ADHERE TO WATER QUALITY REQUIREMENTS ASSOCIATED WITH SECTION 404/401 PERMITS. IF THE TOTAL AREA DISTURBED SHOWN IN THE PLANS AND PSLS WITHIN 1 MILE OF THE PROJECT LIMITS EXCEED 1 ACRE, THE ENGINEER WILL DEVELOP A SWP3 SITE PLAN AND POST A SMALL CONSTRUCTION SITE NOTICE FOR THE CONSTRUCTION ACTIVITIES.









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Mulch Filter Berm and Socks Mulch Filter Berm and Socks Compost Filter Berm and Socks Vegetation Lined Ditches MOA: Memorandum of Agreement TCEQ: Texas Commission on Environmental Quality Moa: Memorandum of Agreement TCEQ: Texas Pollutant Discharge Elimination System Moa: Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System Moa: Moa: Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System Moa: Moa: Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System Moa: Moa: Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System Moa: Moa: Memorandum of Understanding TPDES: Texas Pollutant Discharge Elimination System Moa: Moa: Ministri Ministri Method Texas Pollutant Discharge Elimination System Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: Moa: <t< td=""><td>-</td><td></td><td></td><td></td><td>DSHS: Texas Department of State Health Services PCN: Pre-Construction Notification</td><td></td></t<>	-				DSHS: Texas Department of State Health Services PCN: Pre-Construction Notification	
MBTA: Migratory Bird Treaty Act TxDOT: Texas Department of Transportation Stone Outlet Sediment Traps Sand Filter Systems NOT: Notice of Termination T&E: Threatened and Endangered Species Sediment Region Grassy Swales	PIC				MOA: Memorandum of Agreement TCEQ: Texas Commission on Environmental Quality	
MBTA: Migratory Bird Treaty Act TxDOT: Texas Department of Transportation Stone Outlet Sediment Traps Sand Filter Systems NOT: Notice of Termination T&E: Threatened and Endangered Species Sediment Region Grassy Swales	39*E				MOU: Memorandum of Understanding IPDES: Texas Pollutant Discharge Elimination System MS4: Municipal Separate Stormwater Sewer System TPWD: Texas Parks and Wildlife Department	
By Sodiment Region Grassy Swales NWP: Nationwide Permit USACE: U.S. Army Corps of Engineers	ĕ				NOT: Notice of Termination T&E: Threatened and Endangered Species	
	E		Sediment Basins	🗌 Grassy Swales	NWP: Nationwide Permit USACE: U.S. Army Corps of Engineers	
	E E C					

\$TIME\$ DATE2/4/2022

MATERIALS OR CONTAMINATION ISSUES

ies to all projects):

azard Communication Act (the Act) for personnel who will be working with s by conducting safety meetings prior to beginning construction and are of potential hazards in the workplace. Ensure that all workers are conal protective equipment appropriate for any hazardous materials used. n-site Material Safety Data Sheets (MSDS) for all hazardous products et, which may include, but are not limited to the following categories: vents, asphalt products, chemical additives, fuels and concrete curing tives. Provide protected storage, off bare ground and covered, for be hazardous. Maintain product labelling as required by the Act.

ate supply of on-site spill response materials, as indicated in the MSDS. spill, take actions to mitigate the spill as indicated in the MSDS, safe work practices, and contact the District Spill Coordinator Contractor shall be responsible for the proper containment and cleanup ills.

eer if any of the following are detected: ressed vegetation (not identified as normal) drums, canister, barrels, etc. smells or odors

leaching or seepage of substances

ct involve any bridge class structure rehabilitation or bridge class structures not including box culverts)?

No No

no further action is required. TxDOT is responsible for completing asbestos assessment/inspection.

of the asbestos inspection positive (is asbestos present)?

No No

TxDOT must retain a DSHS licensed asbestos consultant to assist with on, develop abatement/mitigation procedures, and perform management necessary. The notification form to DSHS must be postmarked at least prior to scheduled demolition.

TxDOT is still required to notify DSHS 15 working days prior to any lition.

the Contractor is responsible for providing the date(s) for abatement /or demolition with careful coordination between the Engineer and Itant in order to minimize construction delays and subsequent claims.

nce indicating possible hazardous materials or contamination discovered dous Materials or Contamination Issues Specific to this Project:

Required Action on Required

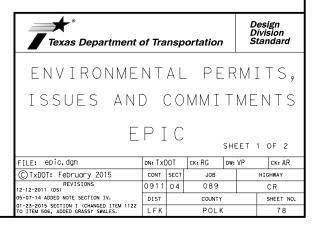
detected in paint less than 90ppm on the Bluff Creek bridge Requirements of Item 6 Special Provisions are not required. ay request a copy of the Asbestos and Lead Paint Inspection ne Area Engineer. Any contracting personnel who may disturb mponents should be made away of the lead content in the that they may use proper OSHA procedures.

RONMENTAL ISSUES

jion issues, such as Edwards Aquifer Districts, etc.)

Required

Required Action



NWP GENERAL CONDITIONS

AS APPLICABLE TO THIS PROJECT

2. AQUATIC LIFE MOVEMENTS. NO ACTIVITY MAY SUBSTANTIALLY DISRUPT THE NECESSARY LIFE CYCLE MOVEMENTS OF THOSE SPECIES OF AQUATIC LIFE INDIGENOUS TO THE WATERBODY. INCLUDING THOSE SPECIES THAT NORMALLY MIGRATE THROUGH THE AREA. UNLESS THE ACTIVITY'S PRIMARY PURPOSE IS TO IMPOUND WATER.

3. SPAWNING AREAS. ACTIVITIES IN SPAWNING AREAS DURING SPAWNING SEASONS MUST BE AVOIDED TO THE MAXIMUM EXTENT PRACTICABLE. ACTIVITIES THAT RESULT IN THE PHYSICAL DESTRUCTION (E.G., THROUGH EXCAVATION, FILL, OR DOWNSTREAM SMOTHERING BY SUBSTANTIAL TURBIDITY) OF AN IMPORTANT SPAWNING AREA ARE NOT AUTHORIZED.

6. SUITABLE MATERIAL. NO ACTIVITY MAY USE UNSUITABLE MATERIAL (E.G., TRASH. DEBRIS, CAR BODIES, ASPHALT, ETC.). MATERIAL USED FOR CONSTRUCTION OR DISCHARGED MUST BE FREE FROM TOXIC POLLUTANTS IN TOXIC AMOUNTS (SEE SECTION 307 OF THE CLEAN WATER ACT).

8. ADVERSE EFFECTS FROM IMPOUNDMENTS. IF THE ACTIVITY CREATES AN IMPOUNDMENT OF WATER, ADVERSE EFFECTS TO THE AQUATIC SYSTEM DUE TO ACCELERATING THE PASSAGE OF WATER, AND/OR RESTRICTING ITS FLOW MUST BE MINIMIZED TO THE MAXIMUM EXTENT PRACTICABLE.

9. MANAGEMENT OF WATER FLOWS. TO THE MAXIMUM EXTENT PRACTICABLE, THE PRE-CONSTRUCTION COURSE, CONDITION, CAPACITY, AND LOCATION OF OPEN WATERS MUST BE MAINTAINED FOR EACH ACTIVITY, INCLUDING STREAM CHANNELIZATION AND STORM WATER MANAGEMENT ACTIVITIES, EXCEPT AS PROVIDED BELOW. THE ACTIVITY MUST BE CONSTRUCTED TO WITHSTAND EXPECTED HIGH FLOWS. THE ACTIVITY MUST NOT RESTRICT OR IMPEDE THE PASSAGE OF NORMAL OR HIGH FLOWS, UNLESS THE PRIMARY PURPOSE OF THE ACTIVITY IS TO IMPOUND WATER OR MANAGE HIGH FLOWS. THE ACTIVITY MAY ALTER THE PRE-CONSTRUCTION COURSE, CONDITION, CAPACITY, AND LOCATION OF OPEN WATERS IF IT BENEFITS THE AQUATIC ENVIRONMENT (E.G., STREAM RESTORATION OR RELOCATION ACTIVITIES).

11. EQUIPMENT. HEAVY EQUIPMENT WORKING IN WETLANDS OR MUD FLATS MUST BE PLACED ON MATS. OR OTHER MEASURES MUST BE TAKEN TO MINIMIZE SOIL DISTURBANCE.

12. SOIL EROSION AND SEDIMENT CONTROLS. APPROPRIATE SOIL EROSION AND SEDIMENT CONTROLS MUST BE USED AND MAINTAINED IN EFFECTIVE OPERATING CONDITION DURING CONSTRUCTION, AND ALL EXPOSED SOIL AND OTHER FILLS, AS WELL AS ANY WORK BELOW THE ORDINARY HIGH WATER MARK OR HIGH TIDE LINE, MUST BE PERMANENTLY STABILIZED AT THE EARLIEST PRACTICABLE DATE. PERMITTEES ARE ENCOURAGED TO PERFORM WORK WITHIN WATERS OF THE UNITED STATES DURING PERIODS OF LOW-FLOW OR NO-FLOW.

13. REMOVAL OF TEMPORARY FILLS. TEMPORARY FILLS MUST BE REMOVED IN THEIR ENTIRETY AND THE AFFECTED AREAS RETURNED TO PRE-CONSTRUCTION ELEVATIONS. THE AFFECTED AREAS MUST BE REVEGETATED, AS APPROPRIATE.

14. PROPER MAINTENANCE. ANY AUTHORIZED STRUCTURE OR FILL SHALL BE PROPERLY MAINTAINED, INCLUDING MAINTENANCE TO ENSURE PUBLIC SAFETY AND COMPLIANCE WITH APPLICABLE NWP GENERAL CONDITIONS, AS WELL AS ANY ACTIVITY-SPECIFIC CONDITIONS ADDED BY THE DISTRICT ENGINEER TO AN NWP AUTHORIZATION.

23. MITIGATION. THE DISTRICT ENGINEER WILL CONSIDER SEVERAL FACTORS WHEN DETERMINING APPROPRIATE AND PRACTICABLE MITIGATION NECESSARY TO ENSURE THAT ADVERSE EFFECTS ON THE AQUATIC ENVIRONMENT ARE MINIMAL.

25. WATER QUALITY. WHERE STATES AND AUTHORIZED TRIBES, OR EPA WHERE APPLICABLE, HAVE NOT PREVIOUSLY CERTIFIED COMPLIANCE OF AN NWP WITH CWA SECTION 401, INDIVIDUAL 401 WATER QUALITY CERTIFICATION MUST BE OBTAINED OR WAIVED (SEE 33 CFR 330.4(C)). THE DISTRICT ENGINEER OR STATE OR TRIBE MAY REQUIRE ADDITIONAL WATER QUALITY MANAGEMENT MEASURES TO ENSURE THAT THE AUTHORIZED ACTIVITY DOES NOT RESULT IN MORE THAN MINIMAL DEGRADATION OR WATER QUALITY.

27. REGIONAL AND CASE-BY-CASE CONDITIONS. THE ACTIVITY MUST COMPLY WITH ANY REGIONAL CONDITIONS THAT MAY HAVE BEEN ADDED BY THE DIVISION ENGINEER (SEE 33 CFR 330.4(E)) AND WITH ANY CASE SPECIFIC CONDITIONS ADDED BY THE CORPS OR BY THE STATE, INDIAN TRIBE, OR U.S. EPA IN ITS SECTION 401 WATER QUALITY CERTIFICATION, OR BY THE STATE IN ITS COASTAL ZONE MANAGEMENT ACT CONSISTENCY DETERMINATION.

FOR A COMPLETE LIST OF GENERAL CONDITIONS GO TO:

http://www.swf.usace.army.mil/Missions/Regulatory/Permitting/NationwideGeneralPermits.aspx

ACTIVITIES REQUIRED FOR CROSSINGS OF WATERS OF THE UNITED STATES ASSOCIATED WITH THE CONSTRUCTION, EXPANSION, MODIFICATION, OR IMPROVEMENT OF LINEAR TRANSPORTATION PROJECT (E.G., ROADS, HIGHWAYS, RAILWAYS, TRAILS, AIRPORT RUNWAYS, AND TAXIWAYS) IN WATERS OF THE U.S. FOR LINEAR TRANSPORTATION PROJECTS IN NON-TIDAL WATERS, THE DISCHARGE CANNOT CAUSE THE LOSS OF GREATER THAN 1/2-ACRE OF WATERS OF THE U.S. ANY STREAM CHANNEL MODIFICATION, INCLUDING BANK STABILIZATION, IS LIMITED TO THE MINIMUM NECESSARY TO CONSTRUCT OR PROTECT THE LINEAR TRANSPORTATION PROJECT; SUCH MODIFICATIONS MUST BE IN THE IMMEDIATE VICINITY OF THE PROJECT.

THIS NWP ALSO AUTHORIZES TEMPORARY STRUCTURES, FILLS, AND WORK NECESSARY TO CONSTRUCT THE LINEAR TRANSPORTATION PROJECT. APPROPRIATE MEASURES MUST BE TAKEN TO MAINTAIN DOWNSTREAM FLOWS AND MINIMIZE FLOODING TO THE MAXIMUM EXTENT PRACTICABLE, WHEN TEMPORARY STRUCTURES, WORK, AND DISCHARGES, INCLUDING COFFERDAMS, ARE NECESSARY FOR CONSTRUCTION ACTIVITIES, ACCESS FILLS, OR DEWATERING OF CONSTRUCTION SITES. TEMPORARY FILLS MUST CONSIST OF MATERIALS, AND BE PLACED IN A MANNER THAT WILL NOT BE ERODED BY EXPECTED HIGH FLOWS. TEMPORARY FILLS MUST BE REMOVED IN THEIR ENTIRETY AND THE AFFECTED AREAS RETURNED TO PRE-CONSTRUCTION ELEVATIONS. THE AREAS AFFECTED BY TEMPORARY FILLS MUST BE REVEGETATED, AS APPROPRIATE.

THIS NWP CANNOT BE USED TO AUTHORIZE NON-LINEAR FEATURES COMMONLY ASSOCIATED WITH TRANSPORTATION PROJECTS, SUCH AS VEHICLE MAINTENANCE OR STORAGE BUILDINGS, PARKING LOTS, TRAIN STATIONS, OR AIRCRAFT HANGARS.

NOTIFICATION: THE PERMITTEE MUST SUBMIT A PRE-CONSTRUCTION NOTIFICATION (PCN) TO THE DISTRICT ENGINEER PRIOR TO COMMENCING THE ACTIVITY IF: (1) THE LOSS OF WATERS OF THE U.S. EXCEEDS 1/10-ACRE; OR (2) THERE IS A DISCHARGE IN A SPECIAL AQUATIC SITE, INCLUDING WETLANDS.

NOTE:

THE PROJECT CROSSES JURISDICTIONAL WATERS OF THE U.S. AND A NWP #14 WITH NO PCN HAS BEEN UTILIZED. THIS PERMIT AUTHORIZES THE ACTIVITIES WHICH WILL IMPACT WATERS OF THE U.S. THE NWP GENERAL CONDITIONS AND THE NWP #14 LIMITS MUST BE FOLLOWED IN ORDER TO MAINTAIN COMPLIANCE WITH THE NWP. NO COORDINATION HAS TAKEN PLACE WITH THE USACE BECAUSE IMPACTS WILL NOT EXCEED THE ABOVE CRITERIA. IF COORDINATION MAY BE NEEDED, CONTACT THE TXDOT LUFKIN DISTRICT ENVIRONMENTAL SECTION AT 1-800-687-8087.

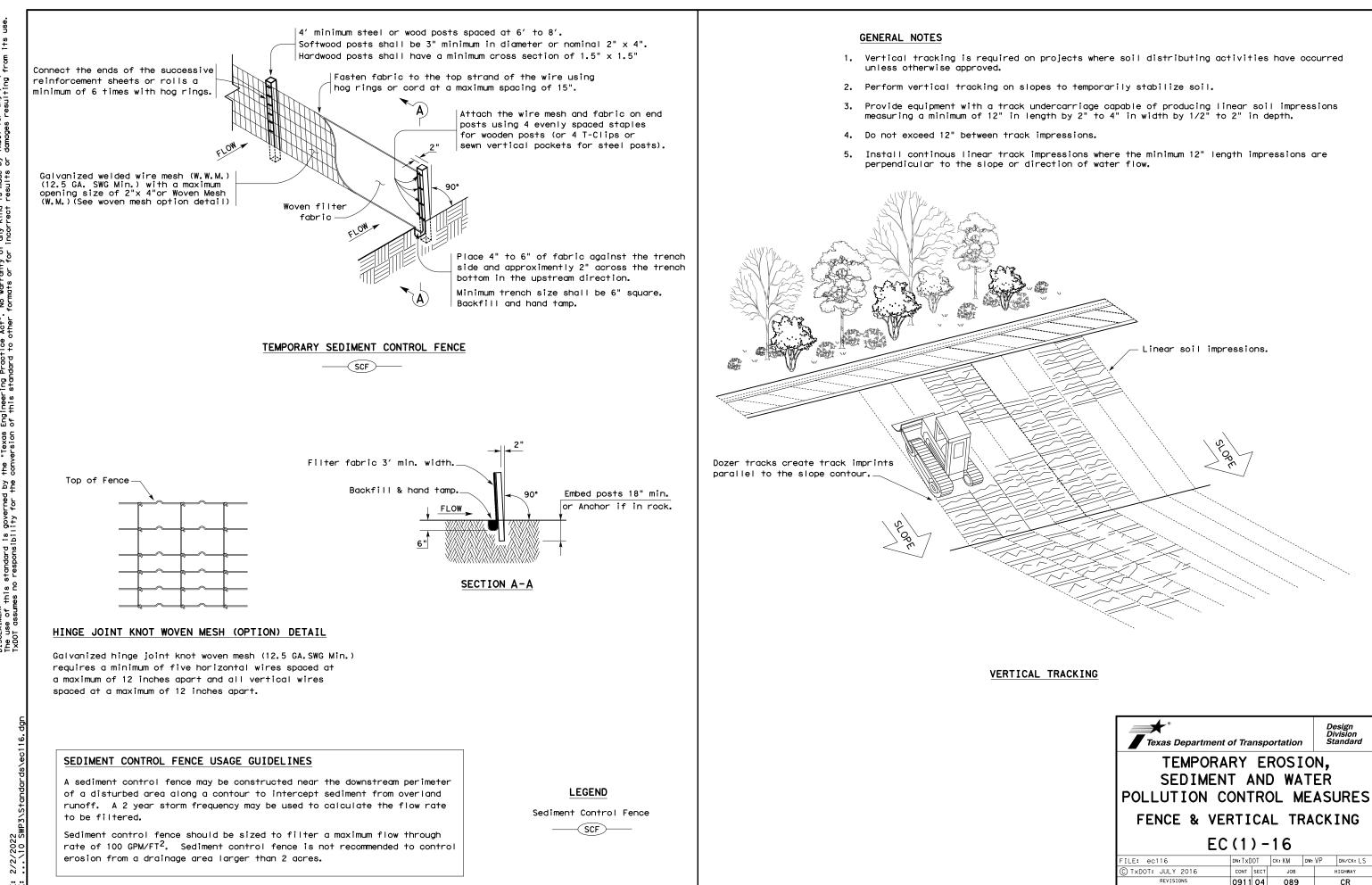
ENVIRONMENTAL PERMITS, (EPIC) ISSUES AND COMMITMENTS



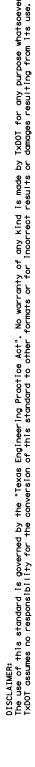
USACE - PERMIT #14

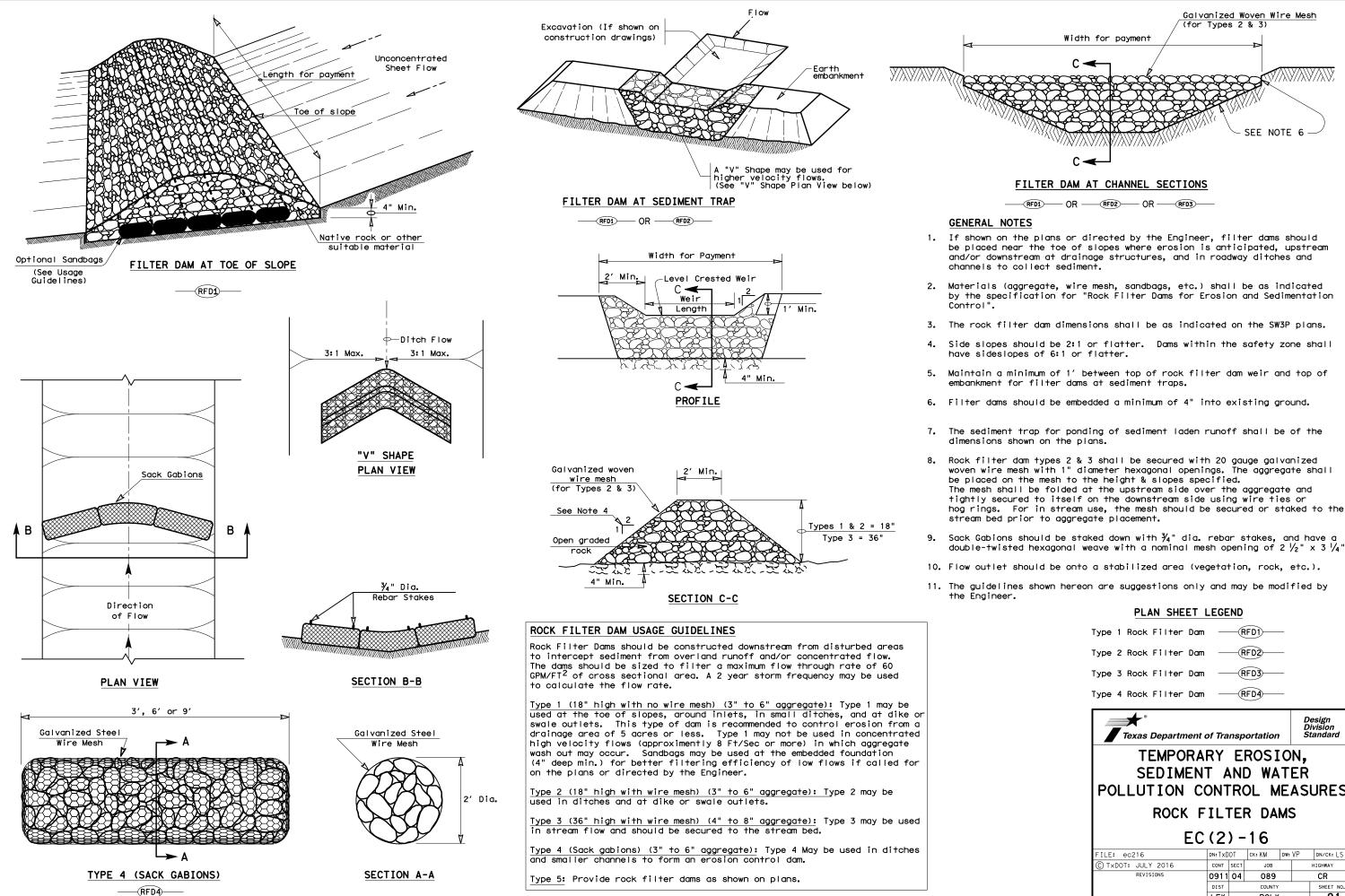
AS APPLICABLE TO THIS PROJECT

Texas Department of Transportation								
EPIC								
(ENVIRONMENTAL PERMITS,								
ISSUES AN	ISSUES AND COMMITMENTS)							
			SH	E E T	20)F 2		
FILE: epic.dgn	dn: Tx[)0T	ск: RG	DW:	VP	CK: AR		
C TxDOT: February 2015	C) TXDOT: February 2015 CONT SECT JOB HIGHWAY							
REVISIONS 0911 04 089 CR						CR		
05-07-14 ADDED NOTE SECTION IV.	DIST	r COUNTY				SHEET NO.		
01-23-2015 SECTION I (CHANGED ITEM 1122 TO ITEM 506, ADDED GRASSY SWALES.					79			



Texas Department of Transportation								
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES								
FENCE & VE	RTIC	AL TRA	CK	ING				
EC	(1)	-16						
FILE: ec116	DN: TXDOT	CK:KM DW	:VP	DN/CK: LS				
C TXDOT: JULY 2016 CONT SECT JOB HIGHWAY								
REVISIONS	0911 04 089 CR							
	DIST COUNTY SE			SHEET NO.				
	LFK	POLK		80				





Type 1 Rock Filter Dam	(RFD1						
Type 2 Rock Filter Dam								
Type 3 Rock Filter Dam								
Type 4 Rock Filter Dam	(RFD4						
Texas Department of	Texas Department of Transportation							
TEMPORARY EROSION, SEDIMENT AND WATER POLLUTION CONTROL MEASURES ROCK FILTER DAMS EC(2)-16								
FILE: ec216	DN: TXDOT	CK: KM DW:	VP DN/CK: LS					
C TXDOT: JULY 2016	CONT SECT	ЈОВ	HIGHWAY					
REVISIONS	0911 04	089	CR					
	DIST	COUNTY	SHEET NO.					
	LFK	POLK	81					

