BGE

SEE SHEET 2 FOR INDEX OF SHEETS

STATE OF TEXAS DEPARTMENT OF TRANSPORTATION

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FUNCTION DESIGN S EXCEED E TRAFFIC TRAFFIC TRAFFIC

PLANS OF PROPOSED STATE HIGHWAY IMPROVEMENT PROJECT NO. BR 2B20(103), ETC. CR

HOUSTON COUNTY

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| | | CSJ | | | PROPOSED NBI | ROAD FT | WAY MI | BRII FT | DGE MI | TOT/ FT | ALS MI |
| BGE, Inc. | | 0911-28-049 | | TRIBUTARY | 111140AA0220004 | 384.00 | 0.073 | 130.00 | 0.025 | 514.00 | 0.097 |
| BCC 10777 Westheimer, Suite 400, Houston, TX 77042 Tel; 281-558-8700 • www.bgeinc.com TBPE Registration No. F-1046 | | 0911-28-054 | HICKORY CREEK | | 111140AA0214002 | 437.05 | 0.083 | 160.00 | 0.030 | 597.05 | 0.113 |
| | | 0311 20 000 | TOTA | LS | 111140440551002 | 1141.05 | 0.216 | 395.00 | 0.075 | 1536.05 | 0.291 |
| tiere Registration No. P. 1049 | BEGIN INCIDENTAL CON CR 1050 AT HICKORY CREEK STA 110+32.05 END INCIDENTAL CONST STA 111+07.00 BEGIN PROJECT BR 2B2 CR 1050 AT HICKORY CREEK CSJ: 0911-28-054 STA 104+35.00 END PROJECT BR 2B20 (CR 1050 AT HICKORY CREEK CSJ: 0911-28-054 STA 110+32.05 LAT: 31.38798° LONG: -95.31593° BEGIN PROJECT BR 2B20 CR 3585 AT WRIGHT CREEK CSJ: 0911-28-060 STA 106+10.00 END PROJECT BR 2B20(1 | 0911-28-060 STRUCTION RUCTION 0 (115) 115) | WRIGHT CREEK TOTA LIMITS: CF FOR THE | 3585 AT AND CR CONSTRUCT OF REP CF CF CF CF CF CF | 111140AA0357002 WRIGHT CREEK, CR 1060 AT HICKORY ION OF BRIDGE RE LACE BRIDGE AND A | 320.00 1141.05 1050 AT CREEK TI PLACEMEI APPROACH CR 1050 187 1050 CR 1050 187 1050 187 1050 1 | 0.061 0.216 THICKC RIB NT CONS IES | 105.00 395.00 ORY CREE SISTING | 0.020 0.075 EK, | 425.00 1536.05 BEC CR HIC CS STA ENC CR HIC CS STA LAT | 0.080 |
| | CR 3585 AT WRIGHT CREEK | L | | R 3585 A | | | * | - { (| L | | |
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| | LONG: -95.55243° | | | | NO RAILROAD CRO | DSSINGS | | | | | uSigned by: |
| | | | | | NTS | | | | | Elec | sleth C |

SPECIFICATIONS ADOPTED BY THE TEXAS DEPARTMENT OF TRANSPORTATION, NOVEMBER 1, 2014 AND SPECIFICATION ITEMS LISTED AND DATED AS FOLLOWS, SHALL GOVERN ON THIS PROJECT: REQUIRED CONTRACT PROVISIONS FOR ALL FEDERAL-AID CONSTRUCTION CONTRACTS (FORM FHWA 1273, MAY 2012).

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FOR LETTING:_____



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| NAL CLASS: LOCAL ROAD SPEED = MEET OR STATE DISTRICT COUNTY | NO. |
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| EXISTING | |
| CR_{3585} ; ADT (2013) = 55 TEXAS LFK HOUSTON | |
| CR 1050: ADT (2013) = 55 CONTROL SECTION JOB HIGHWAY NO. | |
| CR 1060: ADT (2013) = 55 0911 28 049, ETC. CR | |

FINAL PLANS

| LETTING DATE: |
|-----------------------------|
| DATE CONTRACTOR BEGAN WORK: |
| DATE WORK WAS COMPLETED: |
| DATE WORK WAS ACCEPTED: |
| FINAL CONTRACT COST: \$ |
| CONTRACTOR: |

CONSTRUCTION WORK ON THIS PROJECT WAS PERFORMED IN ACCORDANCE WITH PLANS, CONTRACT AND APPROVED CHANGE ORDERS.

DATE ___

JECT BR 2B20(103)

REEK TRIBUTARY -28-049 CT BR 2B20(103)

REEK TRIBUTARY -28-049 27438°

BARRICADES AND WARNING SIGNS

PROVIDE AND ERECT BARRICADES AND WARNING SIGNS IN ACCORDANCE WITH THE BARRICADE & CONSTRUCTION STANDARDS, TCP STANDARDS, THE "TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" AND AS DIRECTED.

(C) 2021

Texas Department of Transportation $^{(\circ)}$

CONCURRENCE:

COUNTY JUDGE, HOUSTON COUNTY

APPROVED FOR LETTING:__

Ebaleth Ortezo, P.E. 3/26/2021

1B27AAE71511446... DISTRICT DESIGN ENGINEER

 DocuSigned by: kelly O. Morris, P. E/26/2021

-F044211639424B4 DISTRICT ENGINEER

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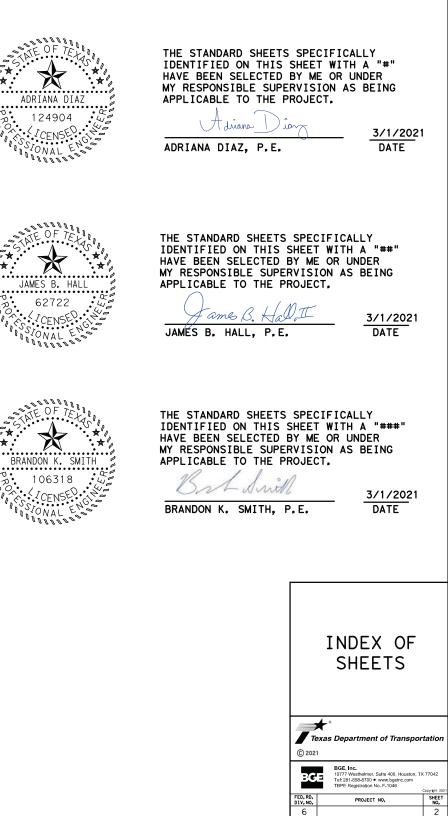
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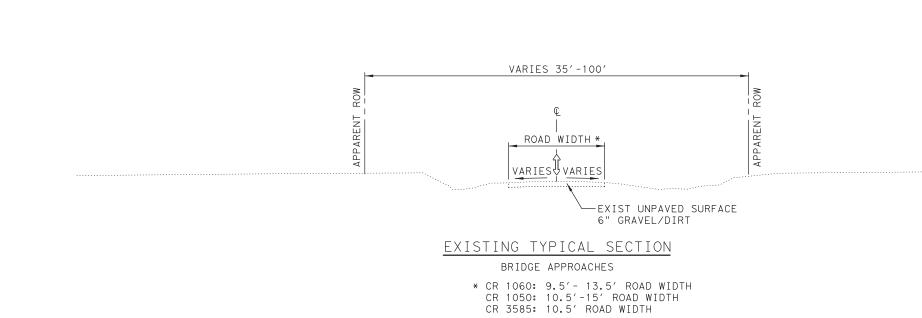
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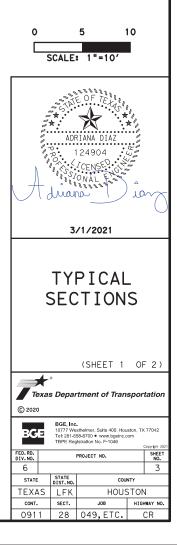
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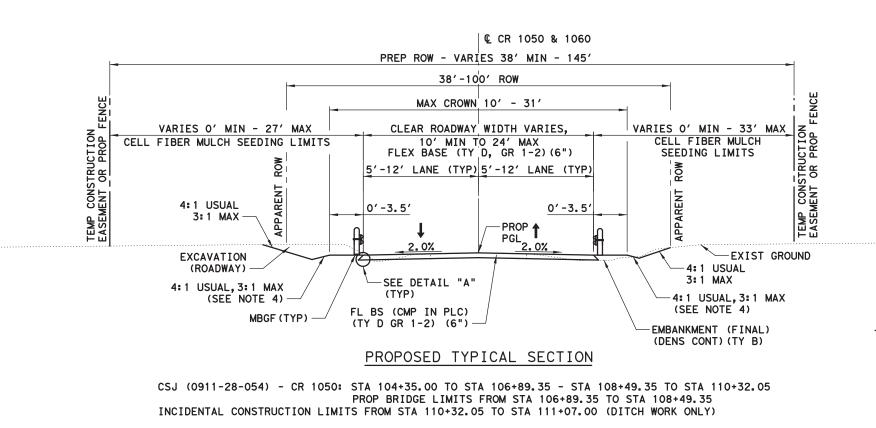
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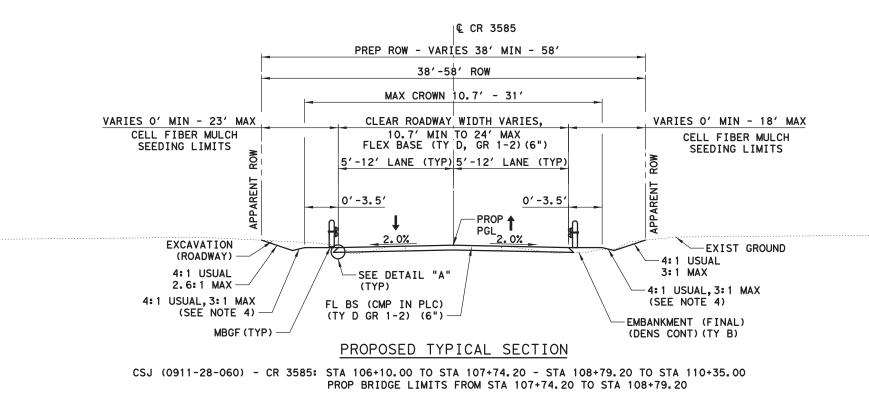


1. APPARENT RIGHT OF WAY SHOWN IS A PRESCRIBED WIDTH BASED ON VISIBLE FEATURES SUCH AS FENCE LINES, UTILITY MARKERS, AND THE MAINTAINED AREA WITH APPROXIMATE LIMITS AT TOP OF DITCH BACKSLOPE. A BOUNDARY SURVEY WAS NOT PERFORMED; NO CONVEYANCE NOR EASEMENT OF THE COUNTY ROAD COULD BE FOUND.

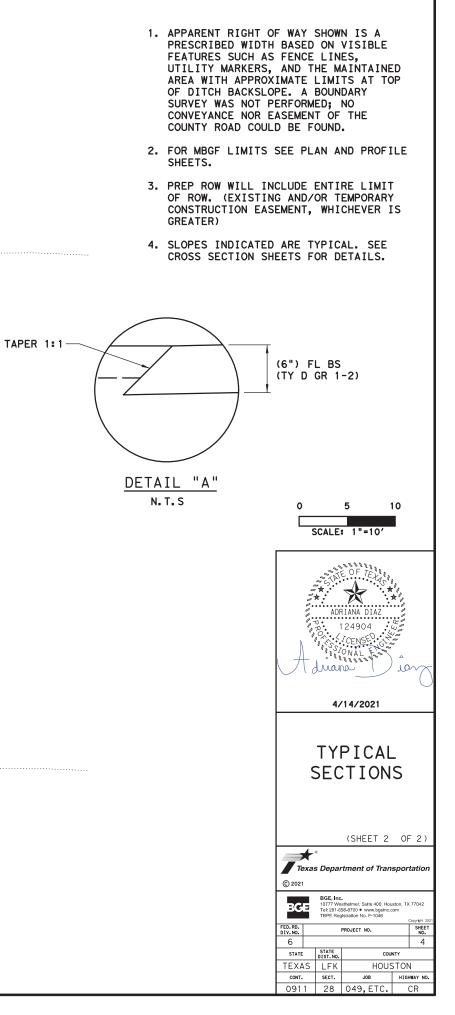




CSJ (0911-28-049) - CR 1060: STA 105+96.00 TO STA 107+65.89 - STA 108+95.89 TO STA 111+10.00 PROP BRIDGE LIMITS FROM STA 107+65.89 TO STA 108+95.89



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Highway: CR

Control: 0911-28-049, ETC

GENERAL NOTES:

Existing regulatory, warning and guide signs within project limits are to remain visible to the traveling public at all times. If a sign must be repositioned during construction operations, move and install the sign to an approved location. Use care when working near existing signs and repair or replace signs damaged by work operations. All work involved repositioning existing signs will be subsidiary to various bid items.

Furnish materials and make repairs to the existing roadway at any location damaged by construction operations. This work shall be done in an approved manner and will be subsidiary to various bid items.

Ensure drainage structures and outfall channels constructed on this project are free of silt and debris at the time of project acceptance. Final clean out work will be subsidiary to various bid items.

Maintain adequate surface drainage throughout the project limits during all phases of construction.

Roadway cross slopes shall conform approximately to the existing surface, unless otherwise directed.

Provide suitable access at all times to adjacent businesses, private property and side roads.

When construction work necessitates the moving of mailboxes, temporarily relocate them as necessary to keep them clear of construction operations and convenient for the mail carrier. Mounts for temporarily relocating mailboxes shall conform to the Department's "Compliant Work Zone Traffic Control Device List" or the mailbox standard. Temporary relocation of mailboxes will be subsidiary to various bid items.

Remove dirt, silt, rocks, debris and other foreign matter that accumulates in structures due to the Contractor's operations as directed. Keep stream channels open at all times. This work will not be paid for directly, but will be subsidiary to pertinent Items.

The bridge at CR 1050 at Hickory Creek has a posted weight limit of 15,000 lbs. per tandem axle. This weight limit shall not be exceeded during project construction.

The bridge at CR 1060 Hickory Creek Tributary has a posted weight limit of 15,000 lbs. per tandem axle. This weight limit shall not be exceeded during project construction.

The bridge at CR 3585 Wright Creek has a posted weight limit of 7,500 lbs. per tandem axle. This weight limit shall not be exceeded during project construction.

Contractor questions on this project are to be addressed to the following individual(s): Jesse.Sisco@txdot.gov Jesse Sisco Praveen Ramanathan Praveen.Ramanathan@txdot.gov

Highway: CR

Contractor questions will be accepted through email, phone, and in person by the above individuals.

All contractor questions will be reviewed by the Engineer. Once a response is developed, it will be posted to TxDOT's Public FTP at the following Address: https://ftp.dot.state.tx.us/pub/txdot-info/Pre-Letting%20Responses/ All questions submitted that generate a response will be posted through this site. The site is organized by District, Project Type (Construction or Maintenance), Letting Date, CCSJ/Project Name.

Project Mowing

Mow the highway right of way within the project limits a maximum of 3 cycles per year as directed. Mowing will not be measured or paid for directly, but will be subsidiary to various bid items.

The equipment used for mowing shall consist of approved mowing units capable of mowing on slopes without marring finished slope surfaces or injuring existing growth. The minimum cutting width shall not be less than 5 ft., unless otherwise approved.

Mow all areas of existing vegetation and vegetation placed during the project as directed. The mowing height shall be 5 in. unless otherwise directed. Repair portions of sod or grass that are injured during mowing operations as directed.

Mow as close as possible to all fixed objects, exercising extreme care not to damage trees, plants, shrubs, signs, delineators or other appurtenances which are part of the facility. Hand trim around such objects, unless otherwise specified.

Use safety chains or other manufacturer's safety device to prevent damage to people or property caused by flying debris propelled out from under rotary mowers. Chains shall be a minimum size of 5/16 in. and links spaced side by side around the mower's front, sides and rear. When mowing at the specified cutting height, the chains shall be long enough to drag the ground. If at any time, it is determined mowing or trimming equipment is defective to the point that it may affect the quality of work or create an unsafe condition, then that equipment shall be immediately repaired or replaced.

Litter Pickup

Remove litter from the right of way in the limits of this project a maximum of 3 cycles per year as directed. Litter pickup will not be measured or paid for directly, but will be subsidiary to various bid items.

The equipment used for litter pickup shall be approved.

County: Houston

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Sheet

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Collect and dispose of all litter deposited by construction operations or the traveling public including cans, bottles, paper, plastic items, metal scraps, lumber, etc. from within the project right of way or as directed. Properly dispose of all collected litter. Do not dump or stockpile collected litter on State property.

For removal of large dead animals, contact nearest TxDOT maintenance section for disposal instructions. Do not bury animal carcasses on State property.

Item 5: Control of the Work

In the event utility lines needing unforeseen adjustments are encountered during construction operations, alter operations and continue to prosecute the contract in such a manner that will allow utility adjustments to be made by others. An extension of working time may be granted for any delays caused by the utility adjustments if deemed necessary.

When a precast or cast-in-place concrete element is included in the plans, a precast concrete alternate may be submitted in accordance with "Standard Operating Procedure for Alternate Precast Proposal Submission" found online at https://www.txdot.gov/inside-txdot/formspublications/consultants-contractors/publications/bridge.html#design. Acceptance or denial of an alternate is at the sole discretion of the Engineer. Impacts to the project schedule and any additional costs resulting from the use of alternates are the sole responsibility of the Contractor.

Item 7: Legal Relations and Responsibilities

No significant traffic generator events identified.

This project consists of discrete construction projects separated a minimum 1/4 mile by undisturbed areas: therefore they are treated as separate plans of development. Area of disturbance and requirements for each are described as follows:

Hickory Creek Bridge at CR 1060 (CSJ 0911-28-049)

The Total Disturbed area for Hickory Creek Bridge is 0.648 acres. The disturbed area in this project and the contractor project specific locations (PSLs) within 1 mile of the project limits for the contractor will further establish the authorization requirements for storm water discharges. As the disturbed area including PSLs is less than 1 acre, the TPDES CGP does not apply, however, the contractor will adhere to the requirements of the SWP3 layouts. If the total area disturbed shown in the plans and PSLs within 1 mile of the project limits exceed 1 acre, the engineer will develop an SWP3 site plan and post a small construction site notice for the construction activities.

Hickory Creek Tributary Bridge at CR 1050 (CSJ 0911-28-054)

The Total Disturbed area for Hickory Creek Tributary Bridge is 0.757 acres. The disturbed area in this project and the contractor project specific locations (PSLs) within 1 mile of the project limits for the contractor will further establish the authorization requirements for storm water discharges. As the disturbed area including PSLs is less than 1 acre, the TPDES CGP does not

County: Houston

Highway: CR

apply, however, the contractor will adhere to the requirements of the SWP3 layouts. If the total area disturbed shown in the plans and PSLs within 1 mile of the project limits exceed 1 acre, the engineer will develop an SWP3 site plan and post a small construction site notice for the construction activities.

Wright Creek Bridge at CR 3585 (CSJ 0911-28-060)

The Total Disturbed area for Wright Creek Bridge is 0.420 acres. The disturbed area in this project and the contractor project specific locations (PSLs) within 1 mile of the project limits for the contractor will further establish the authorization requirements for storm water discharges. As the disturbed area including PSLs is less than 1 acre, the TPDES CGP does not apply, however, the contractor will adhere to the requirements of the SWP3 layouts. If the total area disturbed shown in the plans and PSLs within 1 mile of the project limits exceed 1 acre, the engineer will develop an SWP3 site plan and post a small construction site notice for the construction activities.

Dispose of all vegetative matter and any other materials removed from State Right of Way in accordance with applicable environmental laws, rules, regulations and requirements.

Burning locations must be approved by the Engineer prior to beginning. Burning activities must be conducted in compliance with Texas Commission on Environmental Quality (TCEQ) regulations. Notify the Engineer when burning activities will take place.

In order to maintain compliance with Chapter 64 of the Texas Parks and Wildlife Code and Migratory Bird Treaty Act (MBTA), construction activities that may affect nests (i.e. tree removal, tree limbing, bridge work) shall be conducted outside of the nesting season (March 15 to September 15). In the event birds or active nests (eggs and/or nestlings present) are encountered, contact the engineer prior to conducting work.

Item 8: Prosecution and Progress

For this project, working days will be computed and charged in accordance with Item 8, Section 3.1.4 "Standard Workweek".

Submit monthly progress schedules no later than the 20th calendar day of the month. Failure to comply with this deadline may result in the Engineer withholding progress (monthly) payments.

A 90-day delay has been included to allow contractors time for beam fabrication.

Provide a Critical Path Method (CPM) Construction Schedule unless otherwise approved.

Item 100: Preparing Right of Way

The equipment used to trim limbs shall be approved. A boom axe will not be allowed.

Review Item 7 for compliance with Chapter 64 of the Texas Parks and Wildlife Code and Migratory Bird Treaty Act (MBTA).

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General Notes

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Highway: CR

Sheet

Control: 0911-28-049, ETC

Item 110: Excavation Item 132: Embankment

Hauling materials with scrapers across or along existing roadways will not be permitted without written permission.

Drying of material deeper than 6 inches below subgrade elevations will not be permitted without written permission.

Grading required for shaping driveways and side road turnouts for pipe culverts at all access locations, will be subsidiary to various bid items.

All blading, rolling, and scraper work to construct and remove temporary slopes adjacent to pavement drop-offs, will be subsidiary to various bid items.

Compact embankment material used to reshape existing slopes to a density comparable with adjacent undisturbed material to the satisfaction of the Engineer.

Embankment(Type C) is to meet requirements for Type DS backfill per Item 423, "Retaining Walls"

Item 162: Sodding for Erosion Control

Provide Bermuda block sod unless St. Augustine is the prevailing grass cover at particular placement locations. Provide St. Augustine block sod at those locations.

Item 166: Fertilizer

Fertilize all seeded or sodded areas.

Item 168: Vegetative Watering

Equip water trucks with sprinkler systems capable of watering all of the entire seeded or sodded areas from the roadway.

Water all newly placed sodded or seeded areas at the time of installation. Thereafter, maintain the sodded or seeded areas in a well-watered condition, at no time allow the areas to dry to a condition where water stress is evident.

Item 169: Soil Retention Blankets

In areas designated for soil retention blankets (SRB) in the plans, furnish only spray-on products listed on the Approved Product List for Erosion Control Products based upon the Class and Type specified in the plans. Any substitution to spray -on products must be approved in writing, be listed on the Approved Product List for Erosion Control Products based upon Class and Type, and shall not contain UV degradable, photodegradable or polypropylene materials.

Highway: CR

Item 247: Flexible Base

Provide flexible base with a minimum plasticity index of 2.

Provide flexible base material with a minimum Bar Linear Shrinkage of 2% as determined by Test Method Tex-107-E, Part II.

Stockpiling of base material will not be required if testing has been performed and the material has been approved at the source. Deliver approved specified materials to the project.

Compaction requirements for flexible base are ordinary compaction.

Item 400: Excavation and Backfill for Structures

When cutting an existing roadway open to traffic, complete all operations including structural excavation, laying pipe and backfilling within daylight hours the day they are initiated.

Replace excavated material deemed unsuitable for backfilling with material approved by the Engineer, paid for under the pertinent bid items or as extra work. This provision does not apply to excavated materials that are too wet and are replaced for the Contractor's convenience to expedite the work.

When excavation does not generate enough material to complete the backfill, additional material must be approved prior to use. Additional material will be subsidiary to various bid items.

Item 421: Hydraulic Cement Concrete

The Engineer will provide curing facilities and strength testing equipment for acceptance testing at Lufkin Area Engineer Office, 1805 N. Timberland Dr., Lufkin, TX 75901.

Item 422: Concrete Superstructures

Saw-cut grooves are not required.

Item 427: Surface Finishes for Concrete

Provide a rub finish for Surface Area III.

Item 432: Riprap

Stone riprap will require the placement of filter fabric prior to placement of stones.

Item 464: Reinforced Concrete Pipe

Lay each private entrance or side road pipe culvert to the line and grade as directed.

At locations where existing driveway pipes are to be removed and replaced, replace the top 6 in. of the existing driveway with material as shown on the plans.

Sheet 5B

Control: 0911-28-049, ETC

General Notes

| County: | Houston |
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Highway: CR

Sheet

Control: 0911-28-049, ETC

When excavation does not generate enough material to complete the backfill, additional material must be approved prior to use.

Item 467: Safety End Treatment

Use Type II precast concrete units of the same style and design.

Provide 12 in. deep toewalls on Type II precast safety end treatments.

To improve drainage, grade existing ditch within ten feet of proposed safety end treatment. This work shall be subsidiary to Item 467.

When excavation does not generate enough material to complete the backfill, additional material must be approved prior to use. Additional material will be subsidiary to various bid items.

Check each location where safety end treatments are to be installed to verify pipe lengths shown will produce the desired slope. Extra pipe will be paid for, but removing and replacing safety end treatment units previously installed under this Contract will not be paid for.

Place safety end treatments along the same slope as the pipe.

Item 496: Removing Structures

The structure(s) to be removed have surface coatings which may contain hazardous materials. Provide for the safety and health of employees and abide by all OSHA Standards and Regulations.

Suspect paint on the columns of bridge over Hickory Creek Tributary at CR 1060 contains lead at a concentration below reporting limit (BRL) or <10ppm, therefore abatement is not required. Contractor may request a copy of the Asbestos and Lead Paint Inspection Report from the Area Engineer.

Salvage existing bridge deck, beams, and railing and neatly stockpile at one end of each bridge project at Right of Way line, to be picked up by the county.

Place salvageable county road pipe at the Right of Way line, to be picked up by the county.

Item 502: Barricades, Signs, and Traffic Handling

Traffic Control Plan (TCP):

Ensure the Contractor's Responsible Person (CRP) or their alternate for Barricades, Signs and Traffic Handling is available at all times and able to receive instructions from the Engineer or authorized Department representative. The CRP shall be a person that is usually at the project site during normal working hours.

For protection of the traveling public, direct traffic through the work area using signs, flaggers and other devices. Required signs are shown in the plans on the Barricade and Construction

County: Houston

Highway: CR

Standards and Traffic Control Plan Sheets. The latest edition of the "Texas Manual on Uniform Traffic Control Devices" shall also be used as a guide for handling traffic on this project.

Provide one high-intensity yellow, rotating dome-light on all equipment such as distributors, spreader boxes, lay-down machines, rollers, backhoes, road graders, loaders, etc. Mount lights high enough to be visible from all directions and operating when the equipment is within 30 ft. of the travel way. On all other equipment such as trucks, trailers, automobiles, etc. use emergency flashers while within the work zone.

Notify the Engineer prior to placing any materials or equipment on the right of way. Locate equipment, stockpiles or other materials not in use as far as possible from the driving lanes and in no case closer than 30 ft. unless otherwise authorized. Any equipment, stockpiles, or materials placed within 30 ft. of the driving lane must have adequate signs, barricades or other warning devices as approved. As a minimum place an 8 ft. wide TY III Barricade or barrels on the approach side of each site that is within 30 ft. of the driving lane. Use TY III Barricade or barrels for the site similarly on the departure side if the location is within 30 ft. of the opposing traffic lane.

The Contractor Force Account "Safety Contingency" that has been established for this project is intended to be utilized for work zone enhancements, to improve the effectiveness of the Traffic Control Plan, that could not be foreseen in the project planning and design stage. These enhancements will be mutually agreed upon by the Engineer and the Contractor's Responsible Person based on weekly or more frequent traffic management reviews on the project. The Engineer may choose to use existing bid items if it does not slow the implementation of enhancement.

Texas Transportation Code 547.105 authorizes the use of warning lights to promote safety and provides an effective means of gaining the travelling public's attention as they drive in areas where construction crews are present. In order to influence the public to move over when high risk construction activities are taking place, minimize the utilization of blue warning lights. These lights must be used only while performing work on or near the travel lanes or shoulder where the travelling public encounters construction crews that are not protected by a standard work zone set up such as a lane closure, shoulder closure, or one-way traffic control. Refrain from leaving the warning lights engaged while travelling from one work location to another or while parked on the right of way away from the pavement or a work zone.

All workers on TxDOT right-of-way shall wear reflective clothing meeting ANSI Class II requirements during the day and ANSI Class III requirements during the night.

Item 506: Temporary Erosion, Sedimentation, and Environmental Controls

Locations and types of BMP's may require adjustments prior to or after placement as directed by the Engineer. Adjustments should be made to ensure BMP's are working effectively and efficiently. Notify the Engineer prior to making adjustments.

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Highway: CR

Control: 0911-28-049, ETC

Sheet

Other erosion or water pollution control measure deemed necessary by the Engineer will be paid for in accordance with article 4.4, "Changes in the Work".

Place temporary sediment control fence at locations as directed in addition to locations shown in the plans.

Item 540: Metal Beam Guard Fence

Use round timber posts.

Item 552: Wire Fence

Remove temporary fencing upon completion of permanent fencing unless otherwise directed. Removal of temporary fencing will be considered subsidiary to Item 552, "Wire Fence". All materials used in the temporary fence will remain the property of the Contractor. Existing fence should not be removed until temporary or proposed fence is installed. Temporary fence is subsidiary to Item 552, "Wire Fence" and is to be used during construction. Temporary fence shall be constructed as Wire Fence (Ty C).

Item 560: Mailbox Assemblies

Repair and, if necessary, replace mailboxes damaged by construction operations.

Use 1 size 3 reflector mounted as directed for single and double mailbox assemblies.

Item 644: Small Roadside Sign Assemblies

Existing supports shall not be reused, and shall become the property of the Contractor.

Item 658: Delineator and Object Marker Assemblies

Install delineators on the departure side of the posts when mounting to metal beam guard fence and guardrail end treatments.

5D



CONTROLLING PROJECT ID 0911-28-049

DISTRICT Lufkin HIGHWAY CR **COUNTY** Houston

QUANTITY SHEET

| | | CONTROL SECTIO | ON JOB | 0911-28 | -049 | 0911-28 | 8-054 | 0911-28 | -060 | | |
|-----|----------|--|--------|-----------|-------|-----------|-------|-----------|-------|------------|----------------|
| | | PROJ | ECT ID | A00062 | 647 | A00061 | L475 | A00061 | .484 | | |
| | | C | OUNTY | Houst | on | Houst | on | Houst | on | TOTAL EST. | TOTAL FINAL |
| | | HIG | HWAY | CR | | CR | | CR | | | TIMAL |
| ALT | BID CODE | DESCRIPTION | UNIT | EST. | FINAL | EST. | FINAL | EST. | FINAL | | |
| | 100-6002 | PREPARING ROW | STA | 5.140 | | 6.720 | | 4.250 | | 16.110 | |
| | 110-6001 | EXCAVATION (ROADWAY) | CY | 1,235.000 | | 1,121.000 | | 761.000 | | 3,117.000 | |
| | 132-6003 | EMBANKMENT (FINAL)(ORD COMP)(TY B) | CY | 49.000 | | 128.000 | | 211.000 | | 388.000 | |
| | 132-6005 | EMBANKMENT (FINAL)(ORD COMP)(TY C) | CY | | | 30.000 | | | | 30.000 | |
| | 164-6009 | BROADCAST SEED (TEMP) (WARM) | SY | 873.000 | | 1,061.000 | | 533.000 | | 2,467.000 | |
| | 164-6011 | BROADCAST SEED (TEMP) (COOL) | SY | 873.000 | | 1,061.000 | | 533.000 | | 2,467.000 | |
| | 164-6021 | CELL FBR MLCH SEED(PERM)(RURAL)(SANDY) | SY | 1,745.000 | | 2,122.000 | | 1,066.000 | | 4,933.000 | |
| | 168-6001 | VEGETATIVE WATERING | MG | 69.800 | | 85.000 | | 42.600 | | 197.400 | |
| | 169-6003 | SOIL RETENTION BLANKETS (CL 1) (TY C) | SY | 300.000 | | 380.000 | | 183.000 | | 863.000 | |
| | 247-6073 | FL BS (CMP IN PLC)(TY D GR 1-2) (6") | SY | 980.000 | | 1,195.000 | | 678.000 | | 2,853.000 | |
| | 400-6005 | CEM STABIL BKFL | CY | 50.000 | | 66.000 | | 50.000 | | 166.000 | |
| | 407-6001 | SHEET PILING (PZ - 27) | SF | | | 1,575.000 | | | | 1,575.000 | |
| | 416-6002 | DRILL SHAFT (24 IN) | LF | 420.000 | | | | 420.000 | | 840.000 | |
| | 416-6003 | DRILL SHAFT (30 IN) | LF | | | 360.000 | | | | 360.000 | |
| | 420-6013 | CL C CONC (ABUT) | CY | 20.400 | | 27.200 | | 20.400 | | 68.000 | |
| | 420-6029 | CL C CONC (CAP) | CY | 13.200 | | 17.800 | | 13.200 | | 44.200 | |
| | 420-6037 | CL C CONC (COLUMN) | CY | 1.400 | | | | 4.200 | | 5.600 | |
| | 422-6005 | REINF CONC SLAB (BOX BEAM) | SF | | | 4,186.000 | | | | 4,186.000 | |
| | 422-6007 | REINF CONC SLAB (SLAB BEAM) | SF | 3,380.000 | | | | 2,730.000 | | 6,110.000 | |
| | 422-6023 | SHEAR KEY | CY | | | 21.200 | | | | 21.200 | |
| | 425-6001 | PRESTR CONC BOX BEAM (4B20) | LF | | | 634.000 | | | | 634.000 | |
| | 425-6002 | PRESTR CONC BOX BEAM (5B20) | LF | | | 317.000 | | | | 317.000 | |
| | 425-6012 | PRESTR CONC SLAB BEAM (5SB15) | LF | 642.500 | | | | 517.500 | | 1,160.000 | |
| | 432-6002 | RIPRAP (CONC)(5 IN) | CY | | | 5.000 | | | | 5.000 | |
| | 432-6033 | RIPRAP (STONE PROTECTION)(18 IN) | CY | 348.000 | | 172.000 | | 255.000 | | 775.000 | |
| | 450-6019 | RAIL (TY T631LS) | LF | 284.000 | | 352.000 | | 234.000 | | 870.000 | |
| | 467-6363 | SET (TY II) (18 IN) (RCP) (6: 1) (P) | EA | | | 2.000 | | | | 2.000 | |
| | 496-6009 | REMOV STR (BRIDGE 0 - 99 FT LENGTH) | EA | 1.000 | | 1.000 | | 1.000 | | 3.000 | |
| | 500-6001 | MOBILIZATION | LS | 2.00% | | 2.00% | | 96.00% | | 100.00% | |
| | 502-6001 | BARRICADES, SIGNS AND TRAFFIC HANDLING | МО | 14.000 | | | | | | 14.000 | |
| | 506-6002 | ROCK FILTER DAMS (INSTALL) (TY 2) | LF | 70.000 | | 80.000 | | 80.000 | | 230.000 | |
| | 506-6011 | ROCK FILTER DAMS (REMOVE) | LF | 70.000 | | 80.000 | | 80.000 | | 230.000 | |
| | 506-6038 | TEMP SEDMT CONT FENCE (INSTALL) | LF | 332.000 | | 315.000 | | 239.000 | | 886.000 | |
| | 506-6039 | TEMP SEDMT CONT FENCE (REMOVE) | LF | 332.000 | | 315.000 | | 239.000 | | 886.000 | |
| | 540-6001 | MTL W-BEAM GD FEN (TIM POST) | LF | 191.000 | | 173.000 | | 153.500 | | 517.500 | |
| | 540-6016 | DOWNSTREAM ANCHOR TERMINAL SECTION | EA | 2.000 | | 2.000 | | 2.000 | | 6.000 | |
| | 544-6001 | GUARDRAIL END TREATMENT (INSTALL) | EA | 2.000 | | 2.000 | | 2.000 | | 6.000 | |



| DISTRICT | COUNTY | CCSJ | SHEET |
|----------|---------|-------------|-------|
| Lufkin | Houston | 0911-28-049 | 6 |



CONTROLLING PROJECT ID 0911-28-049

DISTRICT Lufkin HIGHWAY CR

COUNTY Houston

QUANTITY SHEET

| | | CONTROL SECTIO | N JOB | 0911-28 | -049 | 0911-28 | 8-054 | 0911-28 | 8-060 | | |
|-----|-----------|--|-------|---------|-------|---------|-------|------------|-------|------------|----------------|
| | | PROJI | CT ID | A00062 | 647 | A00061 | 475 | A00061 | 484 | | |
| | | co | DUNTY | Houston | | Houst | on | Houst | on | TOTAL EST. | TOTAL FINAL |
| | | HIG | HWAY | CR | CR | | CR | | | | |
| ALT | BID CODE | DESCRIPTION | UNIT | EST. | FINAL | EST. | FINAL | EST. FINAL | | | |
| | 552-6003 | WIRE FENCE (TY C) | LF | 262.000 | | 304.000 | | | | 566.000 | |
| | 552-6008 | WIRE FENCE (WATER GAP) | LF | | | 114.000 | | | | 114.000 | |
| | 560-6004 | MAILBOX INSTALL-S (TWG-POST) TY 2 | EA | | | 2.000 | | | | 2.000 | |
| | 644-6076 | REMOVE SM RD SN SUP&AM | EA | 2.000 | | 2.000 | | 2.000 | | 6.000 | |
| | 658-6016 | INSTL DEL ASSM (D-SW)SZ (BRF)GF1 (BI) | EA | 4.000 | | 4.000 | | 4.000 | | 12.000 | |
| | 658-6062 | INSTL DEL ASSM (D-SW)SZ 1(BRF)GF2(BI) | EA | 2.000 | | 2.000 | | 2.000 | | 6.000 | |
| | 18 | EROSION CONTROL MAINTENANCE: CONTRACTOR FORCE ACCOUNT WORK (PART) | LS | 1.000 | | | | | | 1.000 | |
| | | SAFETY CONTINGENCY: CONTRACTOR FORCE ACCOUNT WORK (PARTICIPATING) | LS | 1.000 | | | | | | 1.000 | |
| 1 | 464-6003 | RC PIPE (CL III)(18 IN) | LF | | | 42.000 | | | | 42.000 | |
| 1A | 4122-6014 | THERMOPLASTIC PIPE(18 IN)(PP)(TYPE III) | LF | | | 42.000 | | | | 42.000 | |



| DISTRICT | COUNTY | CCSJ | SHEET |
|----------|---------|-------------|-------|
| Lufkin | Houston | 0911-28-049 | 6A |

| | | | | | | SUMMAR | Y OF ROADWAY QUANT | TTIES | | | | | | | |
|---|----------------|------------------------|--------------|---------------------|------------------|--|-----------------------------|--|---------------------------------|---|--|----------------------|---------------------------|--|---|
| | | | | | ITEM 100 | ITEM 247 | ITEM 464 | ITEM 467 | ITEM | 540 | ITEM 544 | ITEN | 1 552 | ITEM 560 | ITEM 4122 |
| | | | | | | | BASE BID | | | | | | | | ALTERNATE B |
| ITEM DESCRIPTION | | | | | PREPARING ROW | FL BS (CMP IN PLC) (TY D GR 1-2) (6") ② | RC PIPE (CL III) (18 IN) | SET (TY II) (18 IN) (RCP) (6: 1) (P) | MTL W-BEAM GD FEN (TIM POST) | DOWNSTREAM ANCHOR TERMINAL SECTION | GUARDRAIL END TREATMENT (INSTALL) | WIRE FENCE (TY C) | WIRE FENCE (WATER GAP) | MAILBOX INSTALL-S (TWG-POST) TY 2 | THERMOPLASTI PIPE(18 IN)(PP)(TYP III)(7) |
| | | | | | STA | SY | LF | EA | LF | EA | EA | LF | LF | EA | LF |
| CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) | | | | DGE (AT CR 3585) | | | | | | | | | | | |
| STA | 106+10.00 | - | STA | 107+74,20 | 1.64 | 291 | | | 92.5 | 1 | 1 | | | | |
| STA | 107+74.20 | - | STA | 108+79.20 | 1.05 | | | | | | | | | | |
| STA | 108+79.20 | - | STA | 110+35.00 | 1.56 | 387 | | | 61 | 1 | 1 | | | | |
| CSJ 0911-28-060 TOTALS | | 0911-28-060 TOTALS | 4.25 | 678 | 0 | 0 | 153.5 | 2 | 2 | 0 | 0 | 0 | 0 | | |
| CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) | | | | | | | | | | | | | | | |
| STA | 104+35.00 | - | STA | 106+89.35 | 2.54 | 663 | | | 106 | 1 | 1 | 255 | | | |
| STA | 106+89.35 | - | STA | 108+49.35 | 1.60 | | | | | | | 49 | 114 | | |
| STA | 108+49.35 🌀 | - | STA | 110+32.05 6 | 1.83 | 532 | 42 | 2 | 67 | 1 | 1 | | | 2 | 42 |
| STA | 110+32.05 | - | STA | 111+07.00 | 0.75 | | | | | | | | | | |
| | | | CSJ (| 0911-28-054 TOTALS | 6.72 | 1,195 | 42 | 2 | 173 | 2 | 2 | 304 | 114 | 2 | 42 |
| C | SJ 0911-28-049 | HICKORY (| CREEK TRIB E | BRIDGE (AT CR 1060) | | | | | | | | | | | |
| STA | 105+96.00 | - | STA | 107+65.89 | 1.70 | 427 | | | 77 | 1 | 1 | 63 | | | |
| STA | 107+65.89 | - | STA | 108+95.89 | 1.30 | | | | | | | | | | |
| STA | 108+95.89 | - | STA | 111+10.00 | 2.14 | 553 | | | 114 | 1 | 1 | 199 | | | |
| | | CSJ 0911-28-049 TOTALS | | | 5.14 | 980 | 0 | 0 | 191 | 2 | 2 | 262 | 0 | 0 | 0 |
| - | | | | PROJECT TOTALS | 16.11 | 2,853 | 42 | 2 | 517.5 | 6 | 6 | 566 | 114 | 2 | 42 |

① REMOVAL OF EXISTING FENCE SHALL BE SUBSIDIARY TO ITEM 552.

② QTY INCLUDES DRIVEWAY FL BS.

6 INCIDENTAL CONSTRUCTION

⑦ POLYPROPYLENE (PP) PIPE CAN BE USED AS AN ALTERNATE TO RCP PIPE UNLESS OTHERWISE DIRECTED FOR USE IN PLANS.

| | SUMMARY OF BRIDGE QUANTITIES | | | | | | | | | | | | | | | | |
|------------------|---|-----------------------|---------------------------|---------------------------|------------------------|-----------------------|--------------------------|----------------------------------|-----------------------------------|--------------|-----------------------------------|-----------------------------------|-------------------------------------|----------------------------|---|---------------------|--|
| | | ITEM 400 | ITEM | 1 416 | | ITEM 42 | 0 | | ITEM 422 | | | ITEM 425 | | ITEM 432 | ITEM 432 | ITEM 450 | ITEM 496 |
| ITEM DESCRIPTION | | CEM STABIL BKFL | DRILL SHAFT (24 IN) | DRILL SHAFT (30 IN) | CL C CONC (ABUT) | CL C CONC (CAP) | CL C CONC (COLUMN) | REINF CONC SLAB (BOX BEAM) | REINF CONC SLAB (SLAB BEAM) | SHEAR KEY | PRESTR CONC BOX BEAM (4B20) | PRESTR CONC BOX BEAM (5B20) | PRESTR CONC SLAB BEAM (5SB15) | RIPRAP (CONC) (5 IN) | RIPRAP (STONE PROTECTION) (18 IN) | RAIL (TY T631LS) | REMOV STR (BRIDGE 0 - 99 FT LENGTH) |
| | | CY | LF | LF | CY | CY | CY | SF | SF | CY | LF | LF | LF | CY | CY | LF | EA |
| CSJ 0911-28-060 | WRIGHT CREEK BRIDGE (AT CR 3585) | 50 | 420 | | 20.4 | 13.2 | 4.2 | | 2,730 | | | | 517.5 | | 255 | 234 | 1 |
| CSJ 0911-28-054 | HICKORY CREEK BRIDGE (AT CR 1050) | 66 | | 360 | 27.2 | 17.8 | | 4,186 | | 21.2 | 634 | 317 | | 5.0 | 172 | 352 | 1 |
| CSJ 0911-28-049 | HICKORY CREEK TRIB BRIDGE (AT CR 1060) | 50 | 420 | | 20.4 | 13.2 | 1.4 | | 3,380 | | | | 642.5 | | 348 | 284 | 1 |
| PROJECT TOTALS | | 166 | 840 | 360 | 68.0 | 44.2 | 5.6 | 4,186 | 6,110 | 21.2 | 634 | 317 | 1,160.0 | 5.0 | 775 | 870 | 3 |

| | | | SL | JMMARY OF SWP3 QUANT | TITIES (5) | | | | | |
|---------------------------------|--|---|---|--|--|---|--|--|---|---|
| | | | ITEM 1 | 64 | ITEM 168 | ITEM 169 | | ITE | M 506 | |
| 1 | TEM DESCRIPTION | BROADCAST SEED (TEMP) (WARM) | BROADCAST SEED (TEMP) (COOL) | CELL FBR MLCH SEED(PERM) (RURAL)(SANDY) | VEGETATIVE WATERING ③ | SOIL RETENTION BLANKETS (CL 1) (TY C) | ROCK FILTER DAMS (INSTALL) (TY 2) | ROCK FILTER DAMS (REMOVE) | TEMP SEDMT CONT FENCE (INSTALL) | TEMP SEDM CONT FENC (REMOVE) |
| | | SY | SY | SY | MG | SY | LF | LF | LF | LF |
| CSJ 0911-28-060 | WRIGHT CREEK BRIDGE (AT CR 3585) | 533 | 533 | 1,066 | 42.6 | 183 | 80 | 80 | 239 | 239 |
| CSJ 0911-28-054 | HICKORY CREEK BRIDGE (AT CR 1050) | 982 | 982 | 1,964 | 78.6 | 356 | 80 | 80 | 315 | 315 |
| CSJ 0911-28-054 (INCIDENTAL) | HICKORY CREEK BRIDGE (AT CR 1050) | 79 | 79 | 158 | 6.4 | 24 | | | | |
| CSJ 0911-28-049 | HICKORY CREEK TRIB BRIDGE (AT CR 1060) | 873 | 873 | 1,745 | 69.8 | 300 | 70 | 70 | 332 | 332 |
| P | ROJECT TOTALS | 2,467 | 2,467 | 4,933 | 197.4 | 863 | 230 | 230 | 886 | 886 |
| | CSJ 0911-28-060 CSJ 0911-28-054 CSJ 0911-28-054 (INCIDENTAL) CSJ 0911-28-049 | CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) HICKORY CREEK TRIB BRIDGE | ITEM DESCRIPTIONSEED (TEMP) (WARM)CSJ 0911-28-060WRIGHT CREEK BRIDGE (AT CR 3585)533CSJ 0911-28-054HICKORY CREEK BRIDGE (AT CR 1050)982CSJ 0911-28-054HICKORY CREEK BRIDGE (AT CR 1050)79CSJ 0911-28-049HICKORY CREEK TRIB BRIDGE (AT CR 1060)873 | ITEM DESCRIPTIONBROADCAST SED (TEMP)BROADCAST SEED (TEMP)CSJ 0911-28-060WRIGHT CREEK BRIDGE (AT CR 3585)533533CSJ 0911-28-054HICKORY CREEK BRIDGE (AT CR 1050)982982CSJ 0911-28-054HICKORY CREEK BRIDGE (AT CR 1050)7979CSJ 0911-28-049HICKORY CREEK TRIB BRIDGE873873 | ITEM DESCRIPTION BROADCAST SED (TEMP) (WARM) BROADCAST SED (TEMP) (COOL) CELL FBR MLCH SEED (TEMP) (RURAL) (SANDY) SY SY SY CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) 533 533 1,066 CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) 982 982 1,964 CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) 79 79 158 CSJ 0911-28-049 HICKORY CREEK TRIB BRIDGE (AT CR 1060) 873 873 1,745 | ITEM DESCRIPTION BROADCAST SED (TEMP) (WARM) BROADCAST SED (TEMP) (COOL) CELL FBR MLCH SED (PERM) (RURAL) (SANDY) VEGETATIVE WATERING (3) CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) 533 533 1,066 42.6 CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) 982 982 1,964 78.6 CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) 79 79 158 6.4 CSJ 0911-28-049 HICKORY CREEK TRIB BRIDGE (AT CR 1060) 873 873 1,745 69.8 | ITEM DESCRIPTION BROADCAST SED (TEMP) BROADCAST SED (TEMP) BROADCAST SED (TEMP) CELL FBR MLCH SED (TEMP) (RURAL) (SANDY) VEGETATIVE WATERING (SANDY) SOIL RETENTION BLANKETS (CL 1) (TY C) CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) 533 533 1,066 42.6 183 CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) 982 982 1,964 78.6 356 CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) 79 79 158 6.4 24 CSJ 0911-28-049 HICKORY CREEK TRIB BRIDGE (AT CR 1060) 873 873 1,745 69.8 300 | ITEM DESCRIPTION BROADCAST SEED (TEMP) (WARM) BROADCAST SEED (TEMP) (COOL) CELL FBR MLCH SEED (PERM) (RURAL) (SANDY) VEGETATIVE WATERING (3) SOIL RETENTION BLANKETS (CL1) (TY C) ROCK FILTER DAMS (INSTALL) (TY C) CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) 533 533 1,066 42.6 183 80 CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) 982 982 1,964 78.6 356 80 CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) 79 79 158 6.4 24 1 CSJ 0911-28-049 HICKORY CREEK TRIB BRIDGE 873 873 1,745 69.8 300 70 | ITEM DESCRIPTION BROADCAST SED (TEMP) (WARM) BROADCAST (COOL) BROADCAST SED (TEMP) (COOL) CELL FBR MLCH SEED (TEMP) (RURAL) (SANDY) VEGETATIVE WATERING (3) SOIL RETENTION SUNKETS (CL 1) (TY C) ROCK FILTER DAMS (INSTALL) (TY 2) ROCK FILTER DAMS (REMOVE) SY SY SY MG SY LF LF CSJ 0911-28-060 WRIGHT CREEK BRIDGE (AT CR 3585) 533 533 1,066 42.6 183 80 80 CSJ 0911-28-054 HICKORY CREEK BRIDGE (AT CR 1050) 982 982 1,964 78.6 356 80 80 CSJ 0911-28-054 (INCIDENTAL) HICKORY CREEK BRIDGE (AT CR 1050) 79 79 158 6.4 24 | Item DESCRIPTIONBROADCAST SED (TEMP) (WARM)CELL FBR MLCH SED (TEMP) (RURAL) (SANDY)VEGETATIVE WATERING (3)SOIL RETENTION DLANKETS (CL1) (TY 2)ROCK FILTER DAMS (INSTALL) (TY 2)ROCK FILTER DAMS (REMOVE)TEMP SEDMT CONT FENCE (INSTALL) (INSTALL)CSJ 0911-28-060 (SJ 0911-28-054 (INCIDENTAL)WRIGHT CREEK BRIDGE (AT CR 3585)5335331,06642.61838080239CSJ 0911-28-054 (INCIDENTAL)HICKORY CREEK BRIDGE (AT CR 1050)9829821,96478.63568080315CSJ 0911-28-054 (INCIDENTAL)HICKORY CREEK BRIDGE (AT CR 1050)79791586.424111CSJ 0911-28-049HICKORY CREEK TRIB BRIDGE (AT CR 1060)8738731,74569.83007070332 |

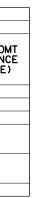
③ ESTIMATED AT 10 GAL/SY FOR 2 APPLICATIONS

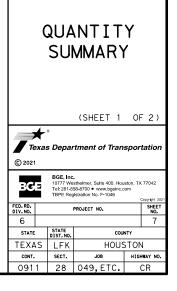
(4) SEE GENERAL NOTES FOR ADDITIONAL INFO

(5) LOCATIONS AND TYPES OF BMP'S MAY REQUIRE ADJUSTMENTS PRIOR TO OR AFTER PLACEMENT AS DIRECTED BY THE ENGINEER. ADJUSTMENTS SHOULD BE MADE TO ENSURE BMP'S ARE WORKING EFFECTIVELY AND EFFICIENTLY. NOTIFY THE ENGINEER PRIOR TO MAKING ADJUSTMENTS.

T:\PCSETUP\Tables\TXD0T.TBL .PLOTDRV\T×DOT. TABLE: DRIVER: PLOT

PEN





| | SUMMARY OF DELINEATOR AND OBJECT MARKER QUANTITIES | | | | | | |
|------------------------|--|-------|---|--|------|-------|--|
| | | | | | ITEM | 1 658 | |
| | | ITEM | INSTL DEL ASSM (D-SW)SZ (BRF)GF1 (BI) | INSTL DEL ASSM (D-SW)SZ 1 (BRF)GF2(BI) | | | |
| | | | | | EA | EA | |
| CSJ | 0911-28-060 | WR | IGHT CREE | K BRIDGE (AT CR 3585) | | | |
| STA | 106+10.00 | - | STA | 107+74.20 | 1 | 1 | |
| STA | 107+74.20 | - | STA | 108+79.20 | 2 | | |
| STA | 108+79.20 | - | STA | 110+35.00 | 1 | 1 | |
| CSJ 0911-28-060 TOTALS | | | | | 4 | 2 | |
| CSJ | 0911-28-054 | HIC | KORY CRE | EK BRIDGE (AT CR 1050) | | | |
| STA | 104+35.00 | - | STA | 106+89.35 | 1 | 1 | |
| STA | 106+89.35 | - | STA | 108+49.35 | 3 | | |
| STA | 108+49.35 | - | STA | 110+32.05 | | 1 | |
| | | | С | SJ 0911-28-054 TOTALS | 4 | 2 | |
| CSJ | 0911-28-049 | HICKO | RY CREEK | TRIB BRIDGE (AT CR 1060) | | | |
| STA | 105+96.00 | - | STA | 107+65.89 | | 1 | |
| STA | 107+65.89 | - | STA | 108+95.89 | 3 | | |
| STA | 108+95.89 | - | STA | 111+10.00 | 1 | 1 | |
| | | | C | SJ 0911-28-049 TOTALS | 4 | 2 | |
| | | | | PROJECT TOTALS | 12 | 6 | |

| SUM | MARY OF REMOVAL QUANTITIES | |
|-----------------|---|---------------------------|
| | | ITEM 64 |
| IT | EM DESCRIPTION | REMOVE RD SN SUP&AN |
| | | EA |
| CSJ 0911-28-060 | WRIGHT CREEK BRIDGE (AT CR 3585) | 2 |
| CSJ 0911-28-054 | HICKORY CREEK BRIDGE (AT CR 1050) | 2 |
| CSJ 0911-28-049 | HICKORY CREEK TRIB BRIDGE (AT CR 1060) | 2 |
| PR | OJECT TOTALS | 6 |

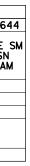
| SUMMARY OF SHEET PILING QUANTITIES | | | | | |
|------------------------------------|--------------------------------------|--|---------------------------|--|--|
| | | ITEM 132 🛈 | ITEM 407 | | |
| | ESCRIPTION | EMBANKMENT (FINAL)(ORD COMP)(TY_C) | SHEET PILING (PZ - 27) | | |
| | | CY | SF | | |
| CSJ 0911-28-054 | HICKORY CREEK BRIDGE (AT CR 1050) | 30 | 1,575 | | |

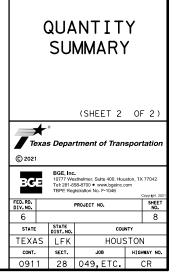
① TYPE DS BACKFILL TO BE PAID FOR UNDER ITEM 132. TYPE DS BACKFILL SHALL BE IN ACCORDANCE WITH ITEM 423, "RETAINING WALLS".

| | ITEM | | 110 | 132 |
|-----------------|-----------|-------------------|-------------------------|--|
| STA | то | STA | EXCAVATION (ROADWAY) | EMBANKMENT (FINAL) (ORE COMP) (TY B) |
| | | | CY | CY |
| CSJ 0911-28-060 | WRIC | GHT CREEK BRIDGE | | |
| 106+10.00 | - | 106+50.00 | 6 | 3 |
| 106+50.00 | - | 107+00.00 | 4 | 15 |
| 107+00.00 | - | 107+50.00 | 4 | 50 |
| 107+50.00 | - | 107+74.20 | 73 | 19 |
| 107+74.20 | - | 108+79.20 | 585 | |
| 108+79.20 | - | 109+00.00 | 65 | 20 |
| 109+00.00 | - | 109+50.00 | 5 | 73 |
| 109+50.00 | - | 110+00.00 | 11 | 28 |
| 110+00.00 | - | 110+35.00 | 8 | 3 |
| CSJ 09 | 11-28-06 | 0 TOTALS | 761 | 211 |
| | | | | |
| CSJ 0911-28-054 | HICK | ORY CREEK BRIDGE | | |
| 104+35.00 | - | 104+50.00 | 4 | 2 |
| 104+50.00 | - | 105+00.00 | 23 | 8 |
| 105+00.00 | - | 105+50,00 | 32 | 6 |
| 105+50.00 | - | 106+00.00 | 35 | 4 |
| 106+00.00 | - | 106+50,00 | 80 | 10 |
| 106+50.00 | - | 106+89.35 | 105 | 6 |
| 106+89.35 | - | 108+49.35 | 577 | |
| 108+49.35 | - | 108+50.00 | 4 | |
| 108+50.00 | - | 109+00.00 | 158 | 34 |
| 109+00.00 | - | 109+50.00 | 16 | 44 |
| 109+50.00 | - | 110+00.00 | 28 | 13 |
| 110+00.00 | - | 110+32.05 | 27 | 1 |
| CSJ 091 | 1-28-054 | SUBTOTAL | 1,089 | 128 |
| 110+32.05 | - | 111+07.00 | 32 | |
| INCIDENTAL CS | 5J 0911-2 | 8-054 SUBTOTAL | 32 | 0 |
| CSJ 09 | 11-28-05 | 4 TOTALS | 1,121 | 128 |
| | | | | |
| CSJ 0911-28-049 | HICKORY | CREEK TRIB BRIDGE | | |
| 105+96.00 | - | 106+00.00 | 1 | |
| 106+00.00 | - | 106+50.00 | 19 | 5 |
| 106+50.00 | - | 107+00.00 | 27 | 8 |
| 107+00.00 | - | 107+50.00 | 35 | 10 |
| 107+50.00 | - | 107+65.89 | 70 | 2 |
| 107+65.89 | - | 108+95.89 | 708 | |
| 108+95.89 | - | 109+00.00 | 29 | 1 |
| 109+00.00 | - | 109+50.00 | 162 | 12 |
| 109+50.00 | - | 110+00.00 | 80 | 6 |
| 110+00.00 | | 110+50.00 | 59 | 3 |
| 110+50.00 | - | 111+00.00 | 42 | 2 |
| 111+00.00 | _ | 111+10.00 | 3 | <u> </u> |
| | 11-28-04 | 9 TOTALS | 1,235 | 49 |
| 634.04 | | | | |

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PHASE 1-3 - CSJ 0911-28-049, -054, & -060

-PREP ROW AND PLACE PROPOSED FENCE AS SHOWN ON THE PLANS. REVIEW ITEM 7 OF THE GENERAL NOTES FOR COMPLIANCE WITH CHAPTER 64 OF THE TEXAS PARKS AND WILDLIFE CODE AND MIGRATORY BIRD TREATY ACT (MBTA).

PHASE 1 - CSJ 0911-28-060 (CR 3585 AT WRIGHT CREEK)

-PLACE ADVANCED WARNING SIGNS IN ACCORDANCE WITH TXDOT STANDARDS AND THE LATEST EDITION OF THE TEXAS MUTCD. -INSTALL STORM WATER POLLUTION PREVENTION DEVICES IN ACCORDANCE WITH SWP3 PLANS. -PLACE TRAFFIC CONTROL DEVICES IN ACCORDANCE WITH DETOUR LAYOUTS. CLOSE BRIDGE. -REMOVE ALL CONFLICTING SIGNS. THIS SHALL BE SUBSIDIARY TO THE PERTINENT BID ITEMS. -BEGIN CONSTRUCTION OF BRIDGE. -CONSTRUCT BRIDGE APPROACHES AND MBGF. -OPEN BRIDGE. -CONSTRUCT BACKFILL SLOPE AND PLACE VEGETATION. -PERFORM FINAL PROJECT CLEAN UP AND REMOVE ALL PROJECT BARRICADES, TEMPORARY SIGNS, AND

PHASE 2 - CSJ 0911-28-054 (CR 1050 AT HICKORY CREEK)

-PLACE ADVANCED WARNING SIGNS IN ACCORDANCE WITH TXDOT STANDARDS AND THE LATEST EDITION OF THE TEXAS MUTCD. -INSTALL STORM WATER POLLUTION PREVENTION DEVICES IN ACCORDANCE WITH SWP3 PLANS.

- -PLACE TRAFFIC CONTROL DEVICES IN ACCORDANCE WITH DETOUR LAYOUTS. CLOSE BRIDGE.
- -REMOVE ALL CONFLICTING SIGNS. THIS SHALL BE SUBSIDIARY TO THE PERTINENT BID ITEMS.
- -INSTALL SHEET PILING.
- -BEGIN CONSTRUCTION OF BRIDGE.
- -CONSTRUCT BRIDGE APPROACHES AND MBGF.
- -OPEN BRIDGE.

SWP3 DEVICES.

- -CONSTRUCT BACKFILL SLOPE AND PLACE VEGETATION.
- -PERFORM FINAL PROJECT CLEAN UP AND REMOVE ALL PROJECT BARRICADES, TEMPORARY SIGNS, AND SWP3 DEVICES.

PHASE 3 - CSJ 0911-28-049 (CR 1060 AT HICKORY CREEK TRIB)

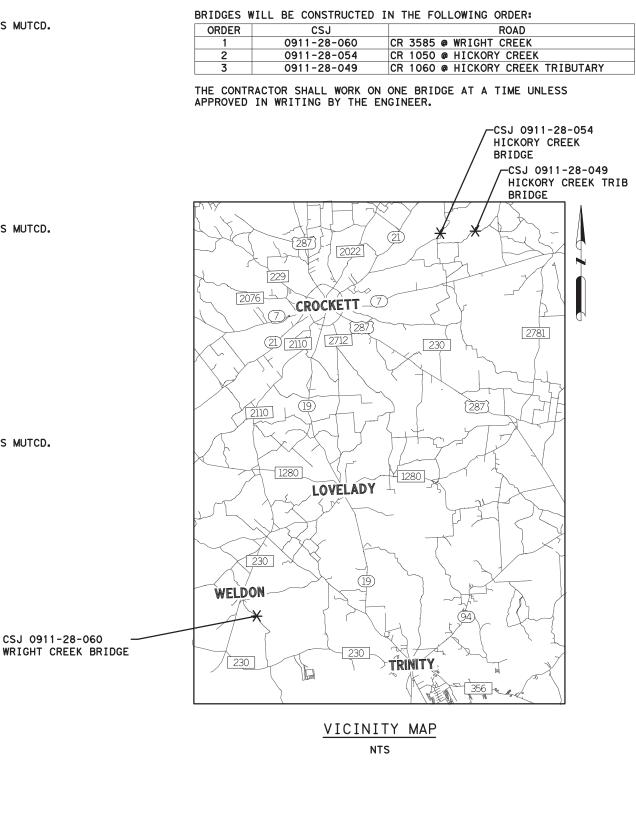
-PLACE ADVANCED WARNING SIGNS IN ACCORDANCE WITH TXDOT STANDARDS AND THE LATEST EDITION OF THE TEXAS MUTCD. -INSTALL STORM WATER POLLUTION PREVENTION DEVICES IN ACCORDANCE WITH SWP3 PLANS. -PLACE TRAFFIC CONTROL DEVICES IN ACCORDANCE WITH DETOUR LAYOUTS. CLOSE BRIDGE. -REMOVE ALL CONFLICTING SIGNS. THIS SHALL BE SUBSIDIARY TO THE PERTINENT BID ITEMS. -BEGIN CONSTRUCTION OF BRIDGE. -CONSTRUCT BRIDGE APPROACHES AND MBGF. -OPEN BRIDGE. -CONSTRUCT BACKFILL SLOPE AND PLACE VEGETATION. -PERFORM FINAL PROJECT CLEAN UP AND REMOVE ALL PROJECT BARRICADES, TEMPORARY SIGNS, AND SWP3 DEVICES.

3. MAINTAIN ACCESS TO PRIVATE PROPERTY AT ALL TIMES.

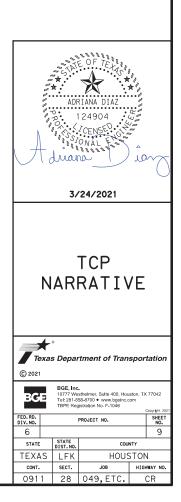
CSJ 0911-28-060

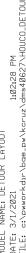
GENERAL NOTES:

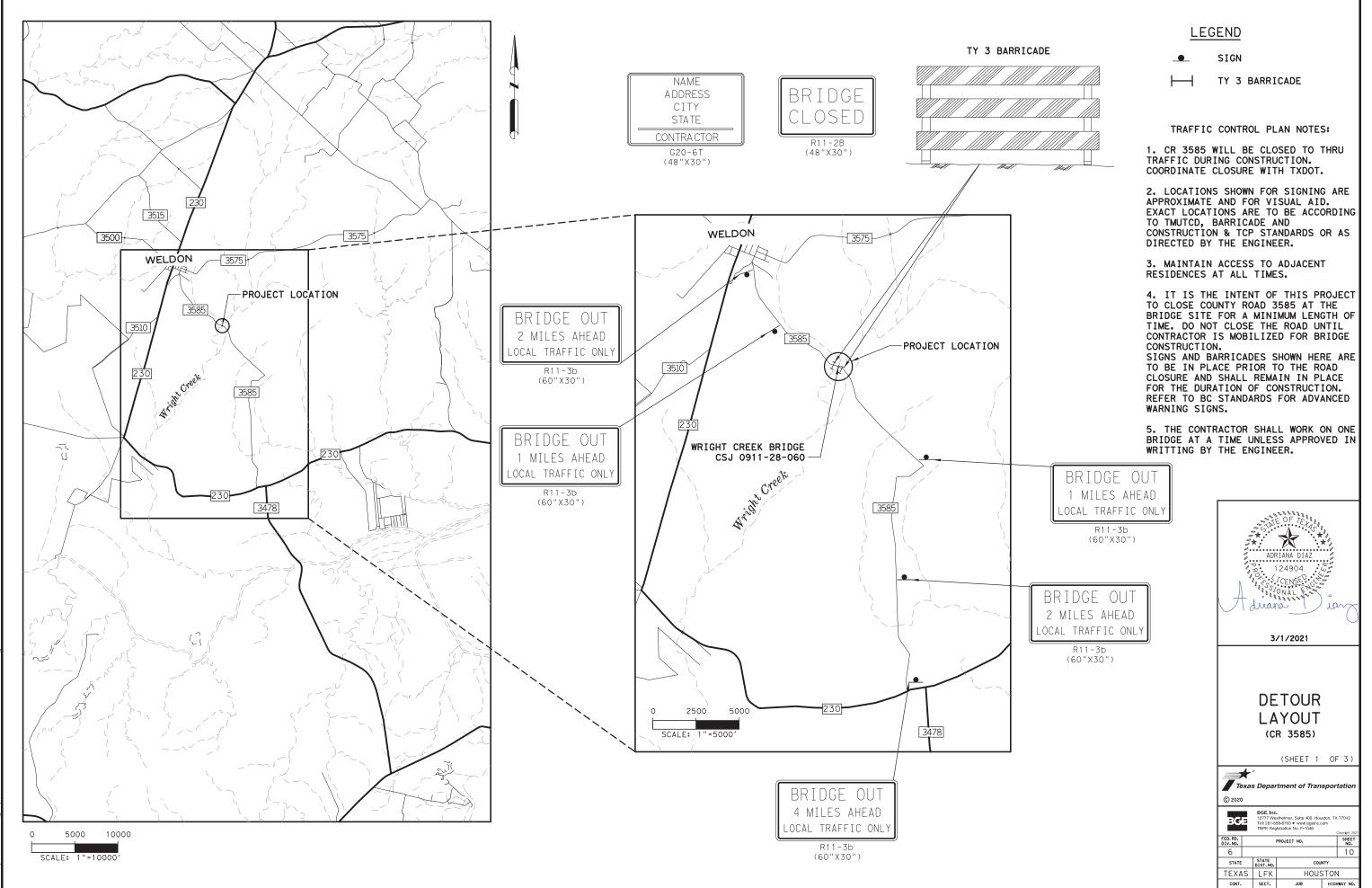
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| 2 | 0911-28-054 | CR | 1 |
| 3 | 0911-28-049 | CR | 1 |



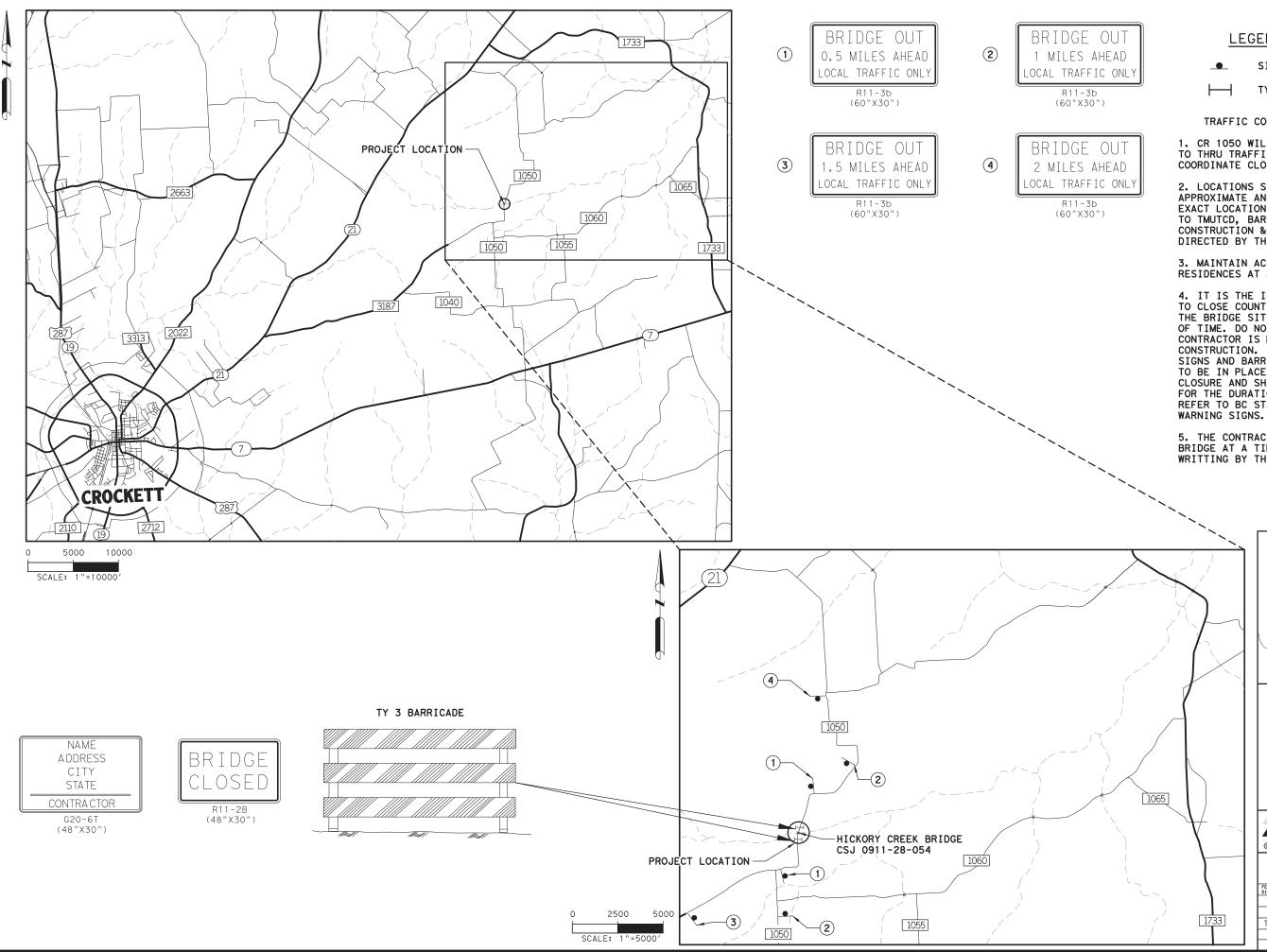
1. SIGNS TO BE RELOCATED ON TEMP MOUNTS DURING EACH PHASE SHALL BE SUBSIDIARY TO ITEM 502. 2. POSITIVE DRAINAGE SHALL BE PROVIDED AT ALL TIMES.







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SIGN

TY 3 BARRICADE

TRAFFIC CONTROL PLAN NOTES:

1. CR 1050 WILL BE CLOSED TO THRU TRAFFIC DURING CONSTRUCTION. COORDINATE CLOSURE WITH TXDOT.

2. LOCATIONS SHOWN FOR SIGNING ARE APPROXIMATE AND FOR VISUAL AID. EXACT LOCATIONS ARE TO BE ACCORDING TO TMUTCD, BARRICADE AND CONSTRUCTION & TCP STANDARDS OR AS DIRECTED BY THE ENGINEER.

3. MAINTAIN ACCESS TO ADJACENT RESIDENCES AT ALL TIMES.

4. IT IS THE INTENT OF THIS PROJECT TO CLOSE COUNTY ROAD 1050 AT THE BRIDGE SITE FOR A MINIMUM LENGTH OF TIME. DO NOT CLOSE THE ROAD UNTIL CONTRACTOR IS MOBILIZED FOR BRIDGE CONSTRUCTION. SIGNS AND BARRICADES SHOWN HERE ARE TO BE IN PLACE PRIOR TO THE ROAD CLOSURE AND SHALL REMAIN IN PLACE FOR THE DURATION OF CONSTRUCTION. REFER TO BC STANDARDS FOR ADVANCED

5. THE CONTRACTOR SHALL WORK ON ONE BRIDGE AT A TIME UNLESS APPROVED IN WRITTING BY THE ENGINEER.

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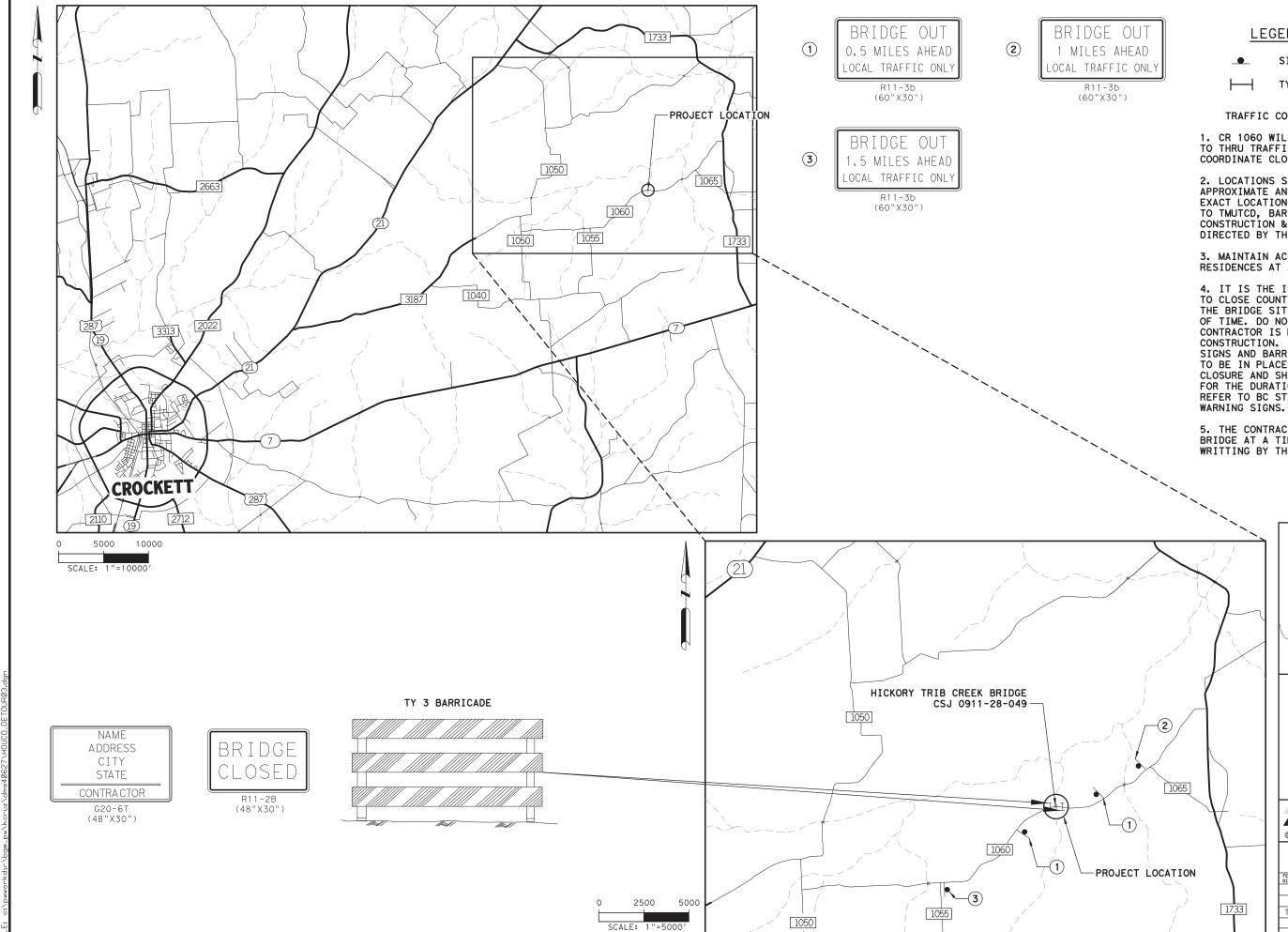
ADRIANA DIAZ 124904

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Texas Department of Transportation @ 2020

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LEGEND

SIGN

TY 3 BARRICADE

TRAFFIC CONTROL PLAN NOTES:

1. CR 1060 WILL BE CLOSED TO THRU TRAFFIC DURING CONSTRUCTION. COORDINATE CLOSURE WITH TXDOT.

2. LOCATIONS SHOWN FOR SIGNING ARE APPROXIMATE AND FOR VISUAL AID. EXACT LOCATIONS ARE TO BE ACCORDING TO TMUTCD, BARRICADE AND CONSTRUCTION & TCP STANDARDS OR AS DIRECTED BY THE ENGINEER.

3. MAINTAIN ACCESS TO ADJACENT RESIDENCES AT ALL TIMES.

4. IT IS THE INTENT OF THIS PROJECT TO CLOSE COUNTY ROAD 1060 AT THE BRIDGE SITE FOR A MINIMUM LENGTH OF TIME. DO NOT CLOSE THE ROAD UNTIL CONTRACTOR IS MOBILIZED FOR BRIDGE CONSTRUCTION. SIGNS AND BARRICADES SHOWN HERE ARE TO BE IN PLACE PRIOR TO THE ROAD CLOSURE AND SHALL REMAIN IN PLACE FOR THE DURATION OF CONSTRUCTION. REFER TO BC STANDARDS FOR ADVANCED

5. THE CONTRACTOR SHALL WORK ON ONE BRIDGE AT A TIME UNLESS APPROVED IN WRITTING BY THE ENGINEER.



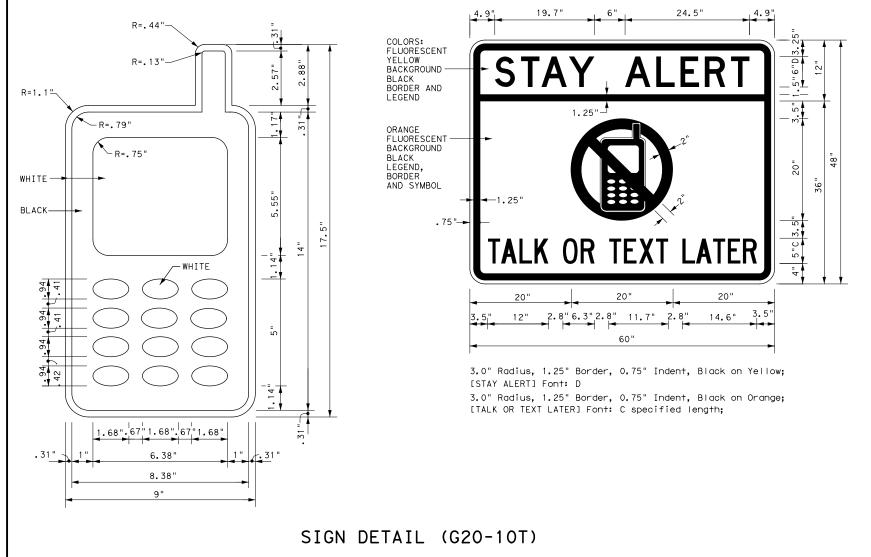
| Tel: 281-558-8700 • www.bgeinc.com TBPE Registration No. F-1046 | | | | | | | | |
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BARRICADE AND CONSTRUCTION (BC) STANDARD SHEETS GENERAL NOTES:

- 1. The Barricade and Construction Standard Sheets (BC sheets) are intended to show typical examples for placement of temporary traffic control devices, construction pavement markings, and typical work zone signs. The information contained in these sheets meet or exceed the requirements shown in the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- The development and design of the Traffic Control Plan (TCP) is the 2. responsibility of the Engineer.
- The Contractor may propose changes to the TCP that are signed and sealed by a licensed professional engineer for approval. The Engineer may develop. sign and seal Contractor proposed changes.
- 4. The Contractor is responsible for installing and maintaining the traffic control devices as shown in the plans. The Contractor may not move or change the approximate location of any device without the approval of the Engineer.
- 5. Geometric design of lane shifts and detours should, when possible, meet the applicable design criteria contained in manuals such as the American Association of State Highway and Transportation Officials (AASHTO), "A Policy on Geometric Design of Highways and Streets," the TxDOT "Roadway Design Manual" or engineering judgment.
- When projects abut, the Engineer(s) may omit the END ROAD WORK, TRAFFIC FINES DOUBLE, and other advance warning signs if the signing would be redundant and the work areas appear continuous to the motorists. If the adjacent project is completed first, the Contractor shall erect the necessary warning signs as shown on these sheets, the TCP sheets or as directed by the Engineer. The BEGIN ROAD WORK NEXT X MILES sign shall be revised to show appropriate work zone distance.
- The Engineer may require duplicate warning signs on the median side of divided highways where median width will permit and traffic volumes justify the signing.
- 8. All signs shall be constructed in accordance with the details found in the "Standard Highway Sign Designs for Texas," latest edition. Sign details not shown in this manual shall be shown in the plans or the Engineer shall provide a detail to the Contractor before the sign is manufactured.
- The temporary traffic control devices shown in the illustrations of the 9. BC sheets are examples. As necessary, the Engineer will determine the most appropriate traffic control devices to be used.
- 10. As shown on BC(2), the OBEY WARNING SIGNS STATE LAW sign, STAY ALERT TALK OR TEXT LATER (see Sign Detail G20-10T) and the WORK ZONE TRAFFIC FINES DOUBLE sign with plaque shall be erected in advance of the CSJ limits. However, the TRAFFIC FINES DOUBLE sign will not be required on projects consisting solely of mobile operation work, such as striping or milling edgeline rumble strips. The BEGIN ROAD WORK NEXT X MILES, CONTRACTOR and END ROAD WORK signs shall be erected at or near the CSJ limits.
- 11. Except for devices required by Note 10, traffic control devices should be in place only while work is actually in progress or a definite need exists.
- 12. The Engineer has the final decision on the location of all traffic control devices.
- 13. Inactive equipment and work vehicles, including workers' private vehicles must be parked away from travel lanes. They should be as close to the right-of-way line as possible, or located behind a barrier or guardrail, or as approved by the Engineer.

WORKER SAFETY APPAREL NOTES:

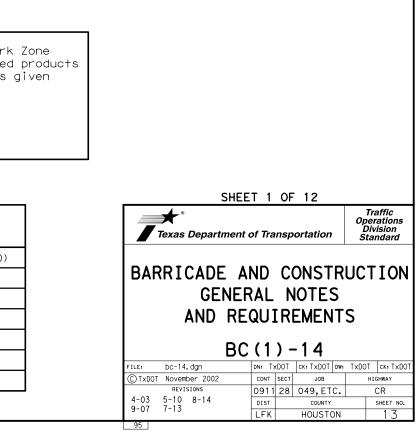
1. Workers on foot who are exposed to traffic or to construction equipment within the right-of-way shall wear high-visibility safety apparel meeting the requirements of ISEA "American National Standard for High-Visibility Apparel," or equivalent revisions, and labeled as ANSI 107-2004 standard performance for Class 2 or 3 risk exposure. Class 3 garments should be considered for high traffic volume work areas or night time work.



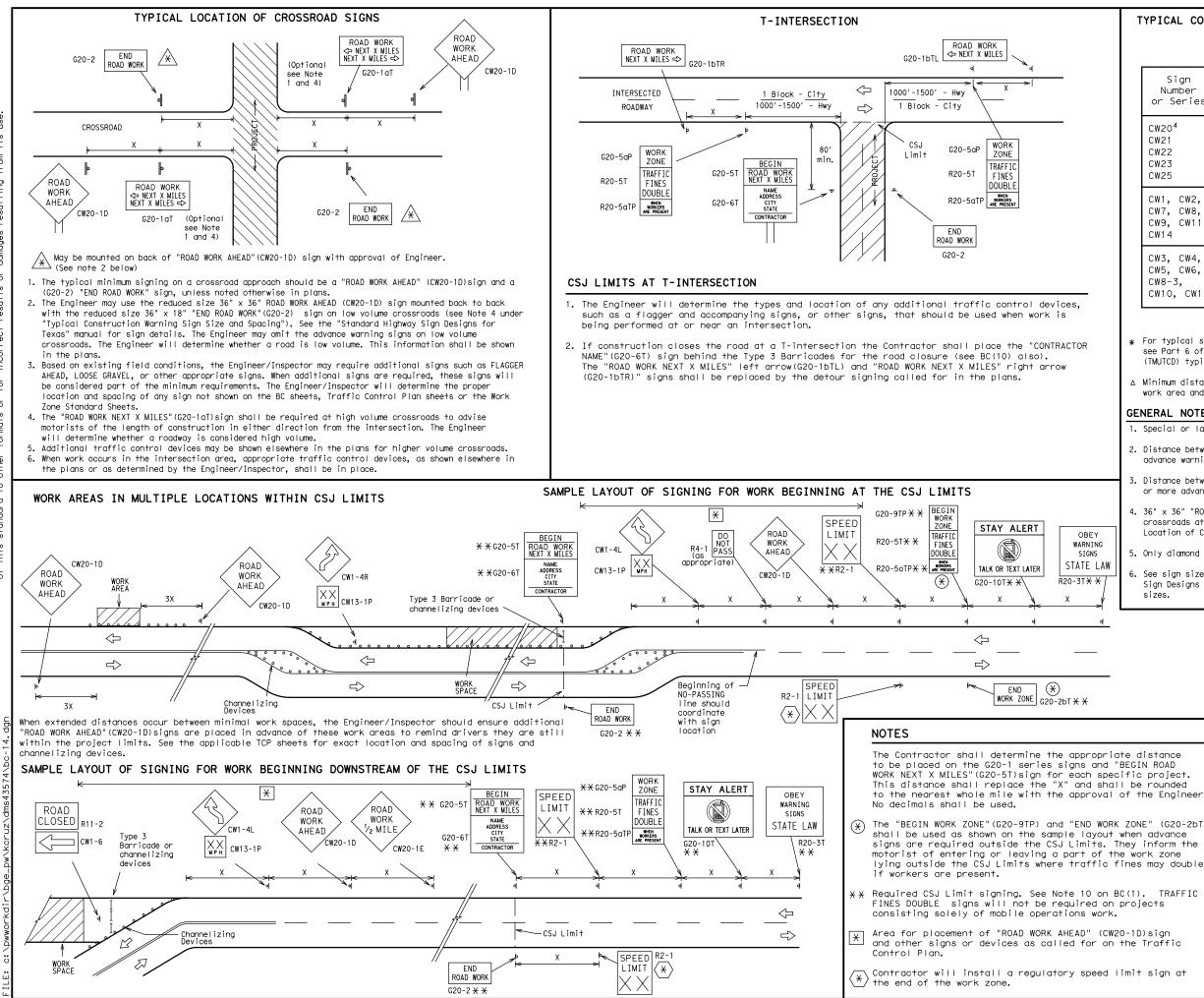
Only pre-qualified products shall be used. The "Compliant Work Zone Traffic Control Devices List" (CWZTCD) describes pre-qualified products and their sources and may be found on-line at the web address given below or by contacting:

Texas Department of Transportation Traffic Operations Division - TE Phone (512) 416-3118

| THE DOCUMENTS BELOW CAN BE FOUND ON-LINE AT http://www.txdot.gov |
|---|
| COMPLIANT WORK ZONE TRAFFIC CONTROL DEVICES LIST (CWZTCD) |
| DEPARTMENTAL MATERIAL SPECIFICATIONS (DMS) |
| MATERIAL PRODUCER LIST (MPL) |
| ROADWAY DESIGN MANUAL - SEE "MANUALS (ONLINE MANUALS)" |
| STANDARD HIGHWAY SIGN DESIGNS FOR TEXAS (SHSD) |
| TEXAS MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (TMUTCD) |
| TRAFFIC ENGINEERING STANDARD SHEETS |







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TYPICAL CONSTRUCTION WARNING SIGN SIZE AND SPACING $^{\rm l,5,6}$

SIZE

| Sign Number or Series | Conventional Road | Expressway/ Freeway | | |
|---|----------------------|------------------------|--|--|
| CW20 ⁴ CW21 CW22 CW23 CW25 | 48" × 48" | 48" × 48" | | |
| CW1, CW2, CW7, CW8, CW9, CW11, CW14 | 36" × 36" | 48" × 48" | | |
| CW3, CW4, CW5, CW6, CW8-3, CW10, CW12 | 48" × 48" | 48" × 48" | | |

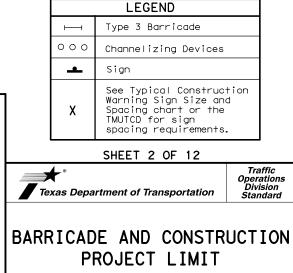
| Posted Speed | Sign Spacing "X" | | | | | |
|-----------------|------------------------|--|--|--|--|--|
| MPH | Feet (Apprx.) | | | | | |
| 30 | 120 | | | | | |
| 35 | 160 | | | | | |
| 40 | 240 | | | | | |
| 45 | 320 | | | | | |
| 50 | 400 | | | | | |
| 55 | 500 ² | | | | | |
| 60 | 600 ² | | | | | |
| 65 | 700 ² | | | | | |
| 70 | 800 ² | | | | | |
| 75 | 900 ² | | | | | |
| 80 | 1000 ² | | | | | |
| * | * 3 | | | | | |

SPACING

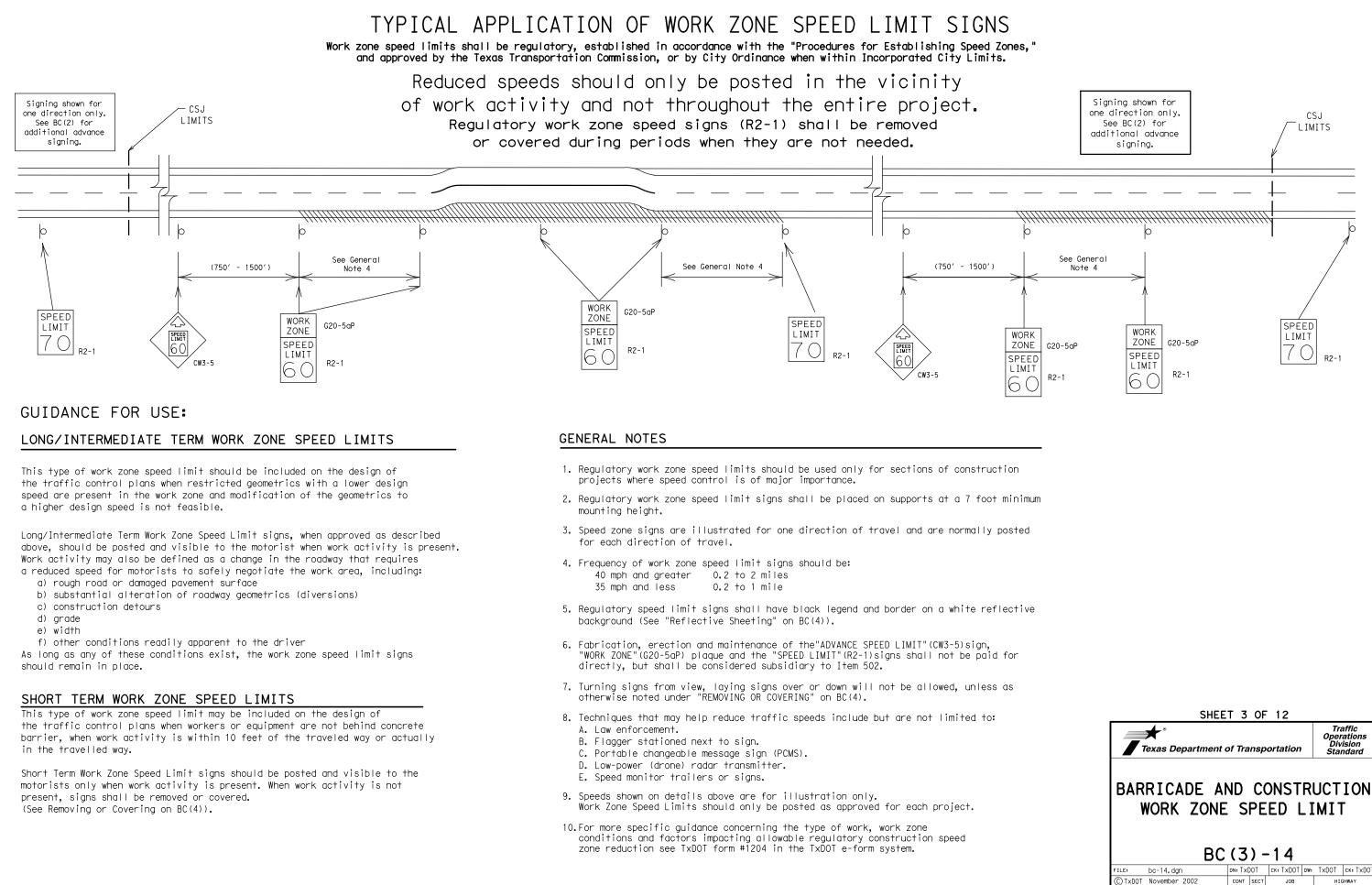
- * For typical sign spacings on divided highways, expressways and freeways, see Part 6 of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) typical application diagrams or TCP Standard Sheets.
- △ Minimum distance from work area to first Advance Warning sign nearest the work area and/or distance between each additional sign.

GENERAL NOTES

- 1. Special or larger size signs may be used as necessary.
- 2. Distance between signs should be increased as required to have 1500 feet advance warning.
- 3. Distance between signs should be increased as required to have 1/2 mile or more advance warning.
- 4. 36" x 36" "ROAD WORK AHEAD" (CW20-1D)signs may be used on low volume crossroads at the discretion of the Engineer. See Note 2 under "Typical Location of Crossroad Signs".
- 5. Only diamond shaped warning sign sizes are indicated.
- 6. See sign size listing in "TMUTCD", Sign Appendix or the "Standard Highway Sign Designs for Texas" manual for complete list of available sign design sizes.



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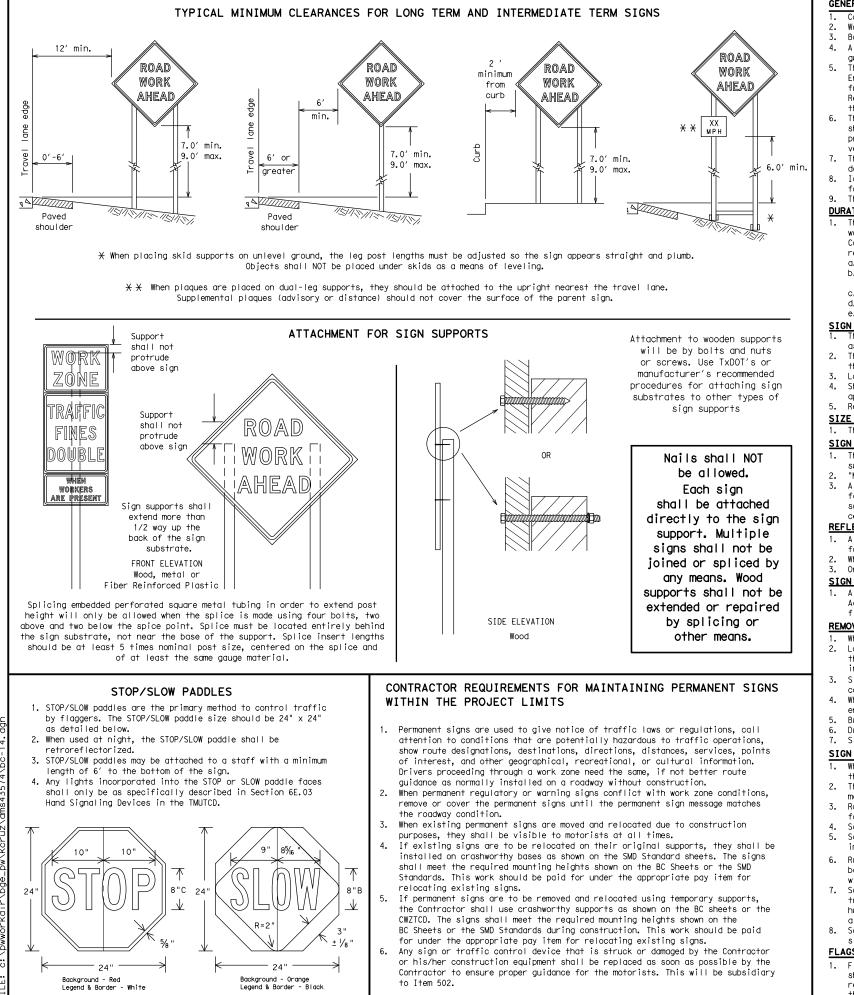
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GENERAL NOTES FOR WORK ZONE SIGNS

- Wooden sign posts shall be painted white.
- Barricades shall NOT be used as sign supports.
- auide the traveling public safely through the work zone.
- the Inspector's TxDOT diary and having both the Inspector and Contractor initial and date the agreed upon changes.
- verify the correct procedures are being followed.
- damaged or marred reflective sheeting as directed by the Engineer/Inspector.
- for identification shall be 1 inch.

The Contractor shall replace damaged wood posts. New or damaged wood sign posts shall not be spliced.

- DURATION OF WORK (as defined by the "Texas Manual on Uniform Traffic Control Devices" Part 6) regard to crashworthiness and duration of work requirements.
- a. Long-term stationary work that occupies a location more than 3 days. b. more than one hour.
- Short-term stationary daytime work that occupies a location for more than 1 hour in a single daylight period.
- Short, duration work that occupies a location up to 1 hour. d.

SIGN MOUNTING HEIGHT

- as shown for supplemental plaques mounted below other signs.
- the around. Long-term/Intermediate-term Signs may be used in lieu of Short-term/Short Duration signing.
- appropriate Lona-term/Intermediate sign height.
- SIZE OF SIGNS

SIGN SUBSTRATES

- All wooden individual sign panels fabricated from 2 or more pieces shall have one or more plywood cleat, 1/2" thick by 6" wide, centers. The Engineer may approve other methods of splicing the sign face. REFLECTIVE SHEETING

- for rigid signs or DMS-8310 for roll-up signs. The web address for DMS specifications is shown on BC(1).

SIGN LETTERS

first class workmanship in accordance with Department Standards and Specifications.

REMOVING OR COVERING

- intersections where the sign may be seen from approaching traffic. Signs installed on wooden skids shall not be turned at 90 degree angles to the roadway. These signs should be removed or completely covered when not required.
- Burlap shall NOT be used to cover signs.
- Duct tape or other adhesive material shall NOT be affixed to a sign face.

SIGN SUPPORT WEIGHTS

- 1. Where sign supports require the use of weights to keep from turning over, the use of sandbags with dry, cohesionless sand should be used. The sandbaas will be tied shut to keep the sand from spilling and to
- maintain a constant weight.
- Rock, concrete, iron, steel or other solid objects shall not be permitted for use as sign support weights. Sandbags should weigh a minimum of 35 lbs and a maximum of 50 lbs.
- Sandbags shall be made of a durable material that tears upon vehicular impact. Rubber (such as tire inner tubes) shall NOT be used.
- Rubber ballasts designed for channelizing devices should not be used for ballast on portable sign supports. Sign supports designed and manufactured with rubber bases may be used when shown on the CWZTCD list.
- Sandbags shall only be placed along or laid over the base supports of the traffic control device and shall not be suspended above ground level or hung with rope, wire, chains or other fasteners. Sandbags shall be placed along the length of the skids to weigh down the sign support.
- Sandbags shall NOT be placed under the skid and shall not be used to level sign supports placed on slopes.

FLAGS ON SIGNS

Flags may be used to draw attention to warning signs. When used the flag shall be 16 inches square or larger and shall be orange or fluorescent red-orange in color. Flags shall not be allowed to cover any portion of the sign face.

No warranty of any for the conversion om its use. Texas Engineering Practice Act". TXDOT assumes no responsibility t results or damages resulting fro is governed by the "Te purpose whatsoever. nats or for incorrect DISCLAIMER: The use of this standard i kind is made by TXDOT for any of this standard to other form

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Contractor shall install and maintain signs in a straight and plumb condition and/or as directed by the Engineer.

All signs shall be installed in accordance with the plans or as directed by the Engineer. Signs shall be used to regulate, warn, and

The Contractor may furnish either the sign design shown in the plans or in the "Standard Highway Sign Designs for Texas" (SHSD). The Engineer/Inspector may require the Contractor to furnish other work zone signs that are shown in the TMUTCD but may have been omitted from the plans. Any variation in the plans shall be documented by written agreement between the Engineer and the Contractor's Responsible Person. All changes must be documented in writing before being implemented. This can include documenting the changes in

The Contractor shall furnish sign supports listed in the "Compliant Work Zone Traffic Control Device List" (CWZTCD). The Contractor shall install the sign support in accordance with the manufacturer's recommendations. If there is a question regarding installation procedures, the Contractor shall furnish the Engineer a copy of the manufacturer's installation recommendations so the Engineer can

The Contractor is responsible for installing signs on approved supports and replacing signs with damaged or cracked substrates and/or

Identification markings may be shown only on the back of the sign substrate. The maximum height of letters and/or company logos used

The types of sign supports, sign mounting height, the size of signs, and the type of sign substrates can vary based on the type of work being performed. The Engineer is responsible for selecting the appropriate size sign for the type of work being performed. The Contractor is responsible for ensuring the sign support, sign mounting height and substrate meets manufacturer's recommendations in

Intermediate-term stationary - work that occupies a location more than one daylight period up to 3 days, or nighttime work lasting

Mobile - work that moves continuously or intermittently (stopping for up to approximately 15 minutes.)

The bottom of Long-term/Intermediate-term signs shall be at least 7 feet, but not more than 9 feet, above the paved surface, except

2. The bottom of Short-term/Short Duration signs shall be a minimum of 1 foot above the pavement surface but no more than 2 feet above

Short-term/Short Duration signs shall be used only during daylight and shall be removed at the end of the workday or raised to

Regulatory signs shall be mounted at least 7 feet, but not more than 9 feet, above the paved surface regardless of work duration.

1. The Contractor shall furnish the sign sizes shown on BC (2) unless otherwise shown in the plans or as directed by the Engineer.

1. The Contractor shall ensure the sign substrate is installed in accordance with the manufacturer's recommendations for the type of sign support that is being used. The CWZTCD lists each substrate that can be used on the different types and models of sign supports. "Mesh" type materials are NOT an approved sign substrate, regardless of the tightness of the weave.

fastened to the back of the sign and extending fully across the sign. The cleat shall be attached to the back of the sign using wood screws that do not penetrate the face of the sign panel. The screws shall be placed on both sides of the splice and spaced at 6"

All signs shall be retroreflective and constructed of sheeting meeting the color and retro-reflectivity requirements of DMS-8300 White sheeting, meeting the requirements of DMS-8300 Type A, shall be used for signs with a white background. 3. Orange sheeting, meeting the requirements of DMS-8300 Type B_{FL} or Type C_{FL}, shall be used for rigid signs with orange backgrounds.

1. All sign letters and numbers shall be clear, and open rounded type uppercase alphabet letters as approved by the Federal Highway Administration (FHWA) and as published in the "Standard Highway Sign Design for Texas" manual. Signs, letters and numbers shall be of

When sign messages may be confusing or do not apply, the signs shall be removed or completely covered. Long-term stationary or intermediate stationary signs installed on square metal tubing may be turned away from traffic 90 degrees when the sign message is not applicable. This technique may not be used for signs installed in the median of divided highways or near any

When signs are covered, the material used shall be opaque, such as heavy mil black plastic, or other materials which will cover the entire sign face and maintain their opaque properties under automobile headlights at night, without damaging the sign sheeting.

Signs and anchor stubs shall be removed and holes backfilled upon completion of work.

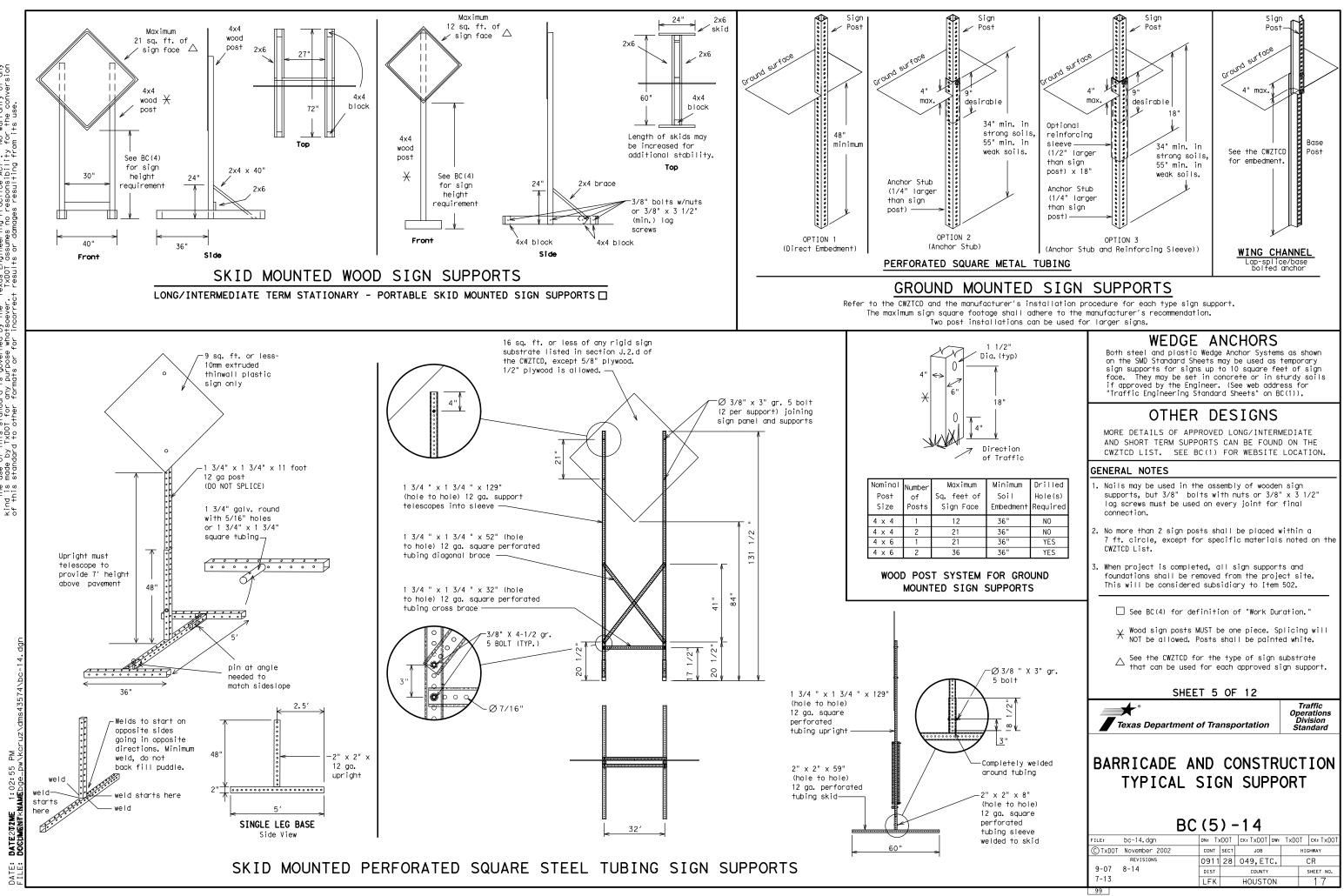
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Texas Department of Transportation

Traffic Operation Division Standard

BARRICADE AND CONSTRUCTION TEMPORARY SIGN NOTES

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| 98 | | | | | | | | |



WHEN NOT IN USE. REMOVE THE PCMS FROM THE RIGHT-OF-WAY OR PLACE THE PCMS BEHIND BARRIER OR GUARDRAIL WITH SIGN PANEL TURNED PARALLEL TO TRAFFIC

PORTABLE CHANGEABLE MESSAGE SIGNS

- 1. The Engineer/Inspector shall approve all messages used on portable changeable message signs (PCMS).
- 2. Messages on PCMS should contain no more than 8 words (about four to eight characters per word), not including simple words such as "TO, "FOR." "AT." etc.
- 3. Messages should consist of a single phase, or two phases that alternate. Three-phase messages are not allowed. Each phase of the message should convey a single thought, and must be understood by itself.
- Use the word "EXIT" to refer to an exit ramp on a freeway; i.e., 4. "EXIT CLOSED." Do not use the term "RAMP."
- Always use the route or interstate designation (IH, US, SH, FM) along with the number when referring to a roadway.
- When in use the bottom of a stationary PCMS message panel should be a minimum 7 feet above the roadway, where possible.
- The message term "WEEKEND" should be used only if the work is to 7. start on Saturday morning and end by Sunday evening at midnight. Actual days and hours of work should be displayed on the PCMS if work is to begin on Friday evening and/or continue into Monday morning.
- The Engineer/Inspector may select one of two options which are available for displaying a two-phase message on a PCMS. Each phase may be displayed for either four seconds each or for three seconds each.
- Do not "flash" messages or words included in a message. The message should be steady burn or continuous while displayed.
- 10. Do not present redundant information on a two-phase message; i.e., keeping two lines of the message the same and changing the third line. 11. Do not use the word "Danger" in message.
- 12. Do not display the message "LANES SHIFT LEFT" or "LANES SHIFT RIGHT" on a PCMS. Drivers do not understand the message.
- 13. Do not display messages that scroll horizontally or vertically across the face of the sign.
- 14. The following table lists abbreviated words and two-word phrases that are acceptable for use on a PCMS. Both words in a phrase must be displayed together. Words or phrases not on this list should not be abbreviated, unless shown in the TMUTCD.
- 15. PCMS character beight should be at least 18 inches for trailer mounted units. They should be visible from at least 1/2 (.5) mile and the text should be legible from at least 600 feet at night and 800 feet in daylight. Truck mounted units must have a character height of 10 inches and must be legible from at least 400 feet.
- 16. Each line of text should be centered on the message board rather than left or right justified.
- 17. If disabled, the PCMS should default to an illegible display that will not alarm motorists and will only be used to alert workers that the PCMS has malfunctioned. A pattern such as a series of horizontal solid bars is appropriate.

| WORD OR PHRASE | ABBREVIATION | WORD OR PHRASE | ABBREVIATION |
|-----------------------|--------------|--------------------------|--------------|
| Access Road | ACCS RD | Major | MAJ |
| Alternate | ALT | Miles | MI |
| Avenue | AVE | Miles Per Hour | MPH |
| Best Route | BEST RTE | Minor | MNR |
| Boulevard | BLVD | Monday | MON |
| Bridge | BRDG | Normal | NORM |
| Cannot | CANT | North | N |
| Center | CTR | Northbound | (route) N |
| Construction Ahead | CONST AHD | Parking | PKING |
| CROSSING | XING | Road | RD |
| Detour Route | DETOUR RTE | Right Lane | RT LN SAT |
| Do Not | DONT | Saturday Service Road | SERV RD |
| East | E | Shoulder | SHLDR |
| Eastbound | (route) E | Slippery | SHLDR |
| Emergency | EMER | South | S |
| Emergency Vehicle | EMER VEH | Southbound | (route) S |
| Entrance, Enter | ENT | Speed | SPD |
| Express Lane | EXP LN | Street | ST |
| Expressway | EXPWY | Sunday | SUN |
| XXXX Feet | XXXX FT | Telephone | PHONE |
| Fog Ahead | FOG AHD | Temporary | TEMP |
| Freeway | FRWY, FWY | Thursday | THURS |
| Freeway Blocked | FWY BLKD | To Downtown | TO DWNTN |
| Friday | FRI | Traffic | TRAF |
| Hazardous Driving | | Travelers | TRVLRS |
| Hazardous Material | HAZMAT | Tuesday | TUES |
| High-Occupancy | HOV | Time Minutes | TIME MIN |
| Vehicle | HWY | Upper Level | UPR LEVEL |
| Highway | | Vehicles (s) | VEH, VEHS |
| Hour(s) | HR, HRS | Warning | WARN |
| Information | INFO | Wednesday | WED |
| It Is | ITS | Weight Limit | WT LIMIT |
| Junction | JCT | West | W |
| Left | LFT | Westbound | (route) W |
| Left Lane | LFT LN | Wet Pavement | WET PVMT |
| Lane Closed | LN CLOSED | Will Not | WONT |
| Lower Level | LWR LEVEL | | 1.0000 |
| Maintenance | MAINT | | |

designation # IH-number, US-number, SH-number, FM-number

| RECOMMENDED | PHASES | AND | FORMATS | FOR | PCMS | MESSAGES | DUR I |
|-------------|--------|-----|---------|-----|------|----------|-------|
| | | | | | | | |

(The Engineer may approve other messages not specifically covered here.)

Phase 1: Condition Lists

Road/Lane/Ramp Closure List

| FREEWAY CLOSED X MILE | FRONTAGE ROAD CLOSED | | RC X |
|-----------------------------|--------------------------------|----------|-----------------|
| ROAD CLOSED AT SH XXX | SHOULDER CLOSED XXX FT | | FI XX |
| ROAD CLSD AT FM XXXX | RIGHT LN CLOSED XXX FT | | R I N/ X) |
| RIGHT X LANES CLOSED | RIGHT X LANES OPEN | | MI TI X: |
| CENTER LANE CLOSED | DAYTIME LANE CLOSURES | | l G X: |
| NIGHT LANE CLOSURES | I-XX SOUTH EXIT CLOSED | | D X |
| VARIOUS LANES CLOSED | EXIT XXX CLOSED X MILE | | RC SI |
| EXIT CLOSED | RIGHT LN TO BE CLOSED | | X |
| MALL DRIVEWAY CLOSED | X LANES CLOSED TUE - FRI | | TI S XX |
| XXXXXXXX BLVD CLOSED | ¥ LANES SHIFT I | in Phase | 1 must |

| Other Co | ndition List |
|--------------------------------|-------------------------------|
| ROADWORK XXX FT | ROAD REPAIRS XXXX FT |
| FLAGGER XXXX FT | LANE NARROWS XXXX FT |
| RIGHT LN NARROWS XXXX FT | TWO-WAY TRAFFIC XX MILE |
| MERGING TRAFFIC XXXX FT | CONST TRAFFIC XXX FT |
| LOOSE GRAVEL XXXX FT | UNEVEN LANES XXXX FT |
| DETOUR X MILE | ROUGH ROAD XXXX FT |
| ROADWORK PAST SH XXXX | ROADWORK NEXT FRI-SUN |
| BUMP XXXX FT | US XXX EXIT X MILES |
| TRAFFIC SIGNAL XXXX FT | LANES SHIFT |

st be used with STAY IN LANE in Phase 2.

APPLICATION GUIDELINES

- Only 1 or 2 phases are to be used on a PCMS. 2. The 1st phase (or both) should be selected from the
- "Road/Lane/Ramp Closure List" and the "Other Condition List".
- 3. A 2nd phase can be selected from the "Action to Take/Effect on Travel, Location, General Warning, or Advance Notice Phase Lists".
- 4. A Location Phase is necessary only if a distance or location is not included in the first phase selected.
- 5. If two PCMS are used in sequence, they must be separated by a minimum of 1000 ft. Each PCMS shall be limited to two phases, and should be understandable by themselves.
- 6. For advance notice, when the current date is within seven days of the actual work date, calendar days should be replaced with days of the week. Advance notification should typically be for no more than one week prior to the work.

WORDING ALTERNATIVES

1. The words RIGHT, LEFT and ALL can be interchanged as appropriate.

¥

Action to Take/Effect on Travel

List

FORM

X LINES

RIGHT

USE

XXXXX

RD EXIT

USE EXIT

I-XX

NORTH

USE

I-XX F

TO I-XX N

WATCH

FOR

TRUCKS

EXPECT

DELAYS

PREPARE

ТΟ

STOP

END

SHOULDER

USE

WATCH

FOR

WORKERS

MERGE

RIGHT

DETOUR

NEXT

X EXITS

USE

EXIT XXX

STAY ON

US XXX

SOUTH

TRUCKS

USE

US XXX N

WATCH

FOR

TRUCKS

EXPECT

DELAYS

REDUCE

SPEED

XXX FT

USE

OTHER

ROUTES

STAY

ĪΝ

ΙΔNF

¥

- appropriate.
- EAST, WEST, NORTH and SOUTH (or abbreviations E, W, N and S) can be interchanged as appropriate.
- 4. Highway names and numbers replaced as appropriate.
- 5. ROAD, HIGHWAY and FREEWAY can be interchanged as needed.
- 6. AHEAD may be used instead of distances if necessary. 7. FT and MI. MILE and MILES interchanged as appropriate.
- 8. AT. BEFORE and PAST interchanged as needed.
- 9. Distances or AHEAD can be eliminated from the message if a location phase is used.

PCMS SIGNS WITHIN THE R.O.W. SHALL BE BEHIND GUARDRAIL OR CONCRETE BARRIER OR SHALL HAVE A MINIMUM OF FOUR (4) PLASTIC DRUMS PLACED PERPENDICULAR TO TRAFFIC ON THE UPSTREAM SIDE OF THE PCMS, WHEN EXPOSED TO ONE DIRECTION OF TRAFFIC. WHEN EXPOSED TO TWO WAY TRAFFIC. THE FOUR DRUMS SHOULD BE PLACED WITH ONE DRUM AT EACH OF THE FOUR CORNERS OF THE UNIT.

FULL MATRIX PCMS SIGNS

- 1. When Full Matrix PCMS signs are used, the character height and legibility/visibility requirements shall be maintained as listed in Note 15 un CHANGEABLE MESSAGE SIGNS" above.
- 2. When symbol signs, such as the "Flagger Symbol" (CW20-7) are represented graphically on the Full Matrix PCMS sign and, with the approval of t shall maintain the legibility/visibility requirement listed above.
- 3. When symbol signs are represented graphically on the Full Matrix PCMS, they shall only supplement the use of the static sign represented, and for, or replace that sian.
- 4. A full matrix PCMS may be used to simulate a flashing arrow board provided it meets the visibility, flash rate and dimming requirements on BC same size arrow

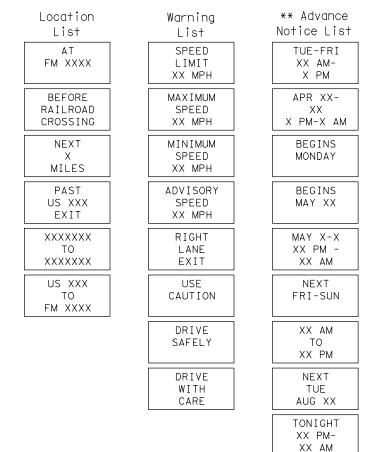
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Roadway

ING ROADWORK ACTIVITIES

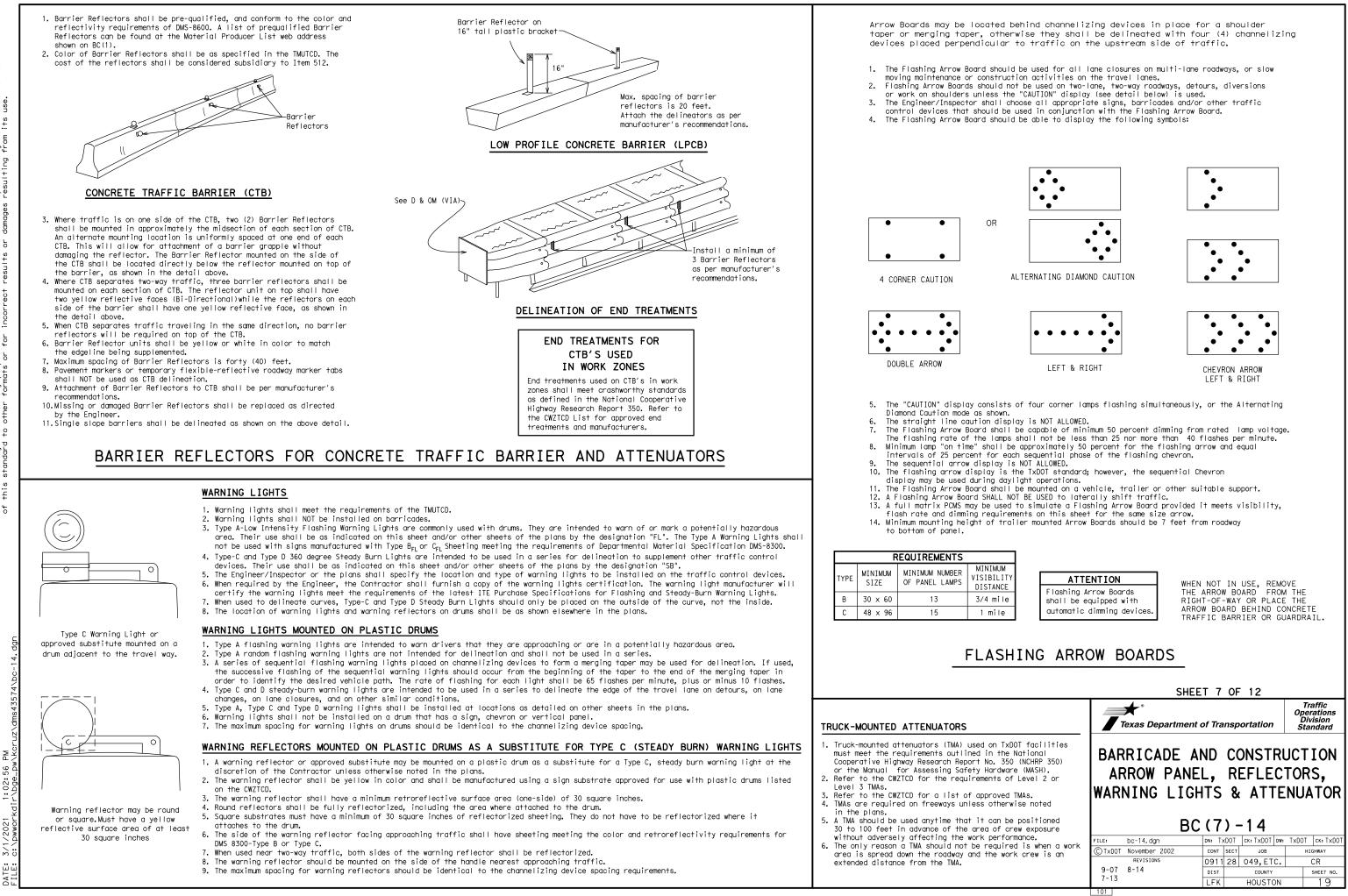
Phase 2: Possible Component Lists



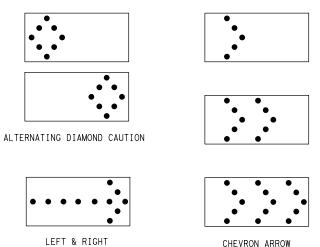
X X See Application Guidelines Note 6.

2. Roadway designations IH, US, SH, FM and LP can be interchanged as

| | SHEE | т 6 0 | F 12 | | |
|------------------------|-------------------------------------|-----------|---------------|---------------|-----------------------------------|
| | Texas Department of | of Trans | portation | Opera Divi | affic ations ision ndard |
| | BARRICADE AN PORTABLE MESSAGE | CHA | ANGEAB | LE | ON |
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| d shall not substitute | © TxDOT November 2002 | CONT SEC | т јов | HIG | HWAY |
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GENERAL NOTES

- 1. For long term stationary work zones on freeways, drums shall be used as the primary channelizing device.
- 2. For intermediate term stationary work zones on freeways, drums should be used as the primary channelizing device but may be replaced in tangent sections by vertical panels, or 42" two-piece cones. In tangent sections one-piece cones may be used with the approval of the Engineer but only if personnel are present on the project at all times to maintain the cones in proper position and location.
- 3. For short term stationary work zones on freeways, drums are the preferred channelizing device but may be replaced in tapers, transitions and tangent sections by vertical panels, two-piece cones or one-piece cones as approved by the Engineer.
- 4. Drums and all related items shall comply with the requirements of the current version of the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD) and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 5. Drums, bases, and related materials shall exhibit good workmanship and shall be free from objectionable marks or defects that would adversely affect their appearance or serviceability.
- The Contractor shall have a maximum of 24 hours to replace any plastic drums identified for replacement by the Engineer/Inspector. The replacement device must be an approved device.

GENERAL DESIGN REQUIREMENTS

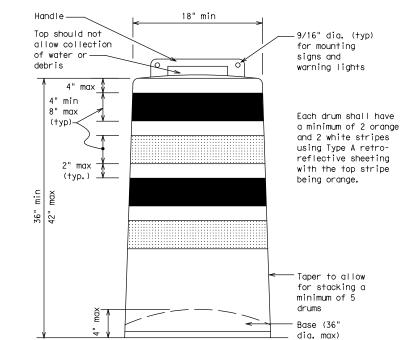
- Pre-qualified plastic drums shall meet the following requirements:
- 1. Plastic drums shall be a two-piece design; the "body" of the drum shall be the top portion and the "base" shall be the bottom.
- 2. The body and base shall lock together in such a manner that the body separates from the base when impacted by a vehicle traveling at a speed of 20 MPH or greater but prevents accidental separation due to normal handling and/or air turbulence created by passing vehicles.
- Plastic drums shall be constructed of lightweight flexible, and deformable materials. The Contractor shall NOT use metal drums or single piece plastic drums as channelization devices or sign supports.
- 4. Drums shall present a profile that is a minimum of 18 inches in width at the 36 inch height when viewed from any direction. The height of drum unit (body installed on base) shall be a minimum of 36 inches and a maximum of 42 inches.
- 5. The top of the drum shall have a built-in handle for easy pickup and shall be designed to drain water and not collect debris. The handle shall have a minimum of two widely spaced 9/16 inch diameter holes to allow attachment of a warning light, warning reflector unit or approved compliant sign.
- 6. The exterior of the drum body shall have a minimum of four alternating orange and white retroreflective circumferential stripes not less than 4 inches nor greater than 8 inches in width. Any non-reflectorized space between any two adjacent stripes shall not exceed 2 inches in width.
- 7. Bases shall have a maximum width of 36 inches, a maximum height of 4 inches, and a minimum of two footholds of sufficient size to allow base to be held down while separating the drum body from the base.
- 8. Plastic drums shall be constructed of ultra-violet stabilized, orange, high-density polyethylene (HDPE) or other approved material.
- 9. Drum body shall have a maximum unballasted weight of 11 lbs.
- 10.Drum and base shall be marked with manufacturer's name and model number.

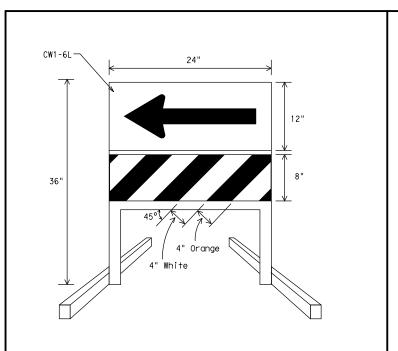
RETROREFLECTIVE SHEETING

- The stripes used on drums shall be constructed of sheeting meeting the color and retroreflectivity requirements of Departmental Materials Specification DMS-8300, "Sign Face Materials." Type A reflective sheeting shall be supplied unless otherwise specified in the plans.
- 2. The sheeting shall be suitable for use on and shall adhere to the drum surface such that, upon vehicular impact, the sheeting shall remain adhered in-place and exhibit no delaminating, cracking, or loss of retroreflectivity other than that loss due to abrasion of the sheeting surface.

BALLAST

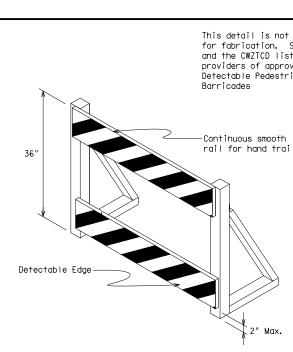
- Unballasted bases shall be large enough to hold up to 50 lbs. of sand. This base, when filled with the ballast material, should weigh between 35 lbs (minimum) and 50 lbs (maximum). The ballast may be sand in one to three sandbags separate from the base, sand in a sand-filled plastic base, or other ballasting devices as approved by the Engineer. Stacking of sandbags will be allowed, however height of sandbags above pavement surface may not exceed 12 inches.
- Bases with built-in ballast shall weigh between 40 lbs. and 50 lbs. Built-in ballast can be constructed of an integral crumb rubber base or a solid rubber base.
- 3. Recycled truck tire sidewalls may be used for ballast on drums approved for this type of ballast on the CWZTCD list.
- 4. The ballast shall not be heavy objects, water, or any material that would become hazardous to motorists, pedestrians, or workers when the drum is struck by a vehicle.
- 5. When used in regions susceptible to freezing, drums shall have drainage holes in the bottoms so that water will not collect and freeze becoming a hazard when struck by a vehicle.
- 6. Ballast shall not be placed on top of drums.
- 7. Adhesives may be used to secure base of drums to pavement.





DIRECTION INDICATOR BARRICADE

- The Direction Indicator Barricade may be used in tapers, transitions, and other areas where specific directional auidance to drivers is pecessary.
- guidance to drivers is necessary.If used, the Direction Indicator Barricade should be used in series to direct the driver through the transition and into the intended travel lane.
- 3. The Direction Indicator Barricade shall consist of One-Direction Large Arrow (CW1-6) sign in the size shown with a black arrow on a background of Type B_{FL}or Type C_{FL}Orange retroreflective sheeting above a rail with Type A retroreflective sheeting in alternating 4" white and orange stripes sloping downward at an angle of 45 degrees in the direction road users are to pass. Sheeting types shall be as per DMS 8300.
- 4. Double arrows on the Direction Indicator Barricade will not be allowed.
- 5. Approved manufacturers are shown on the CWZICD List. Ballast shall be as approved by the manufacturers instructions.

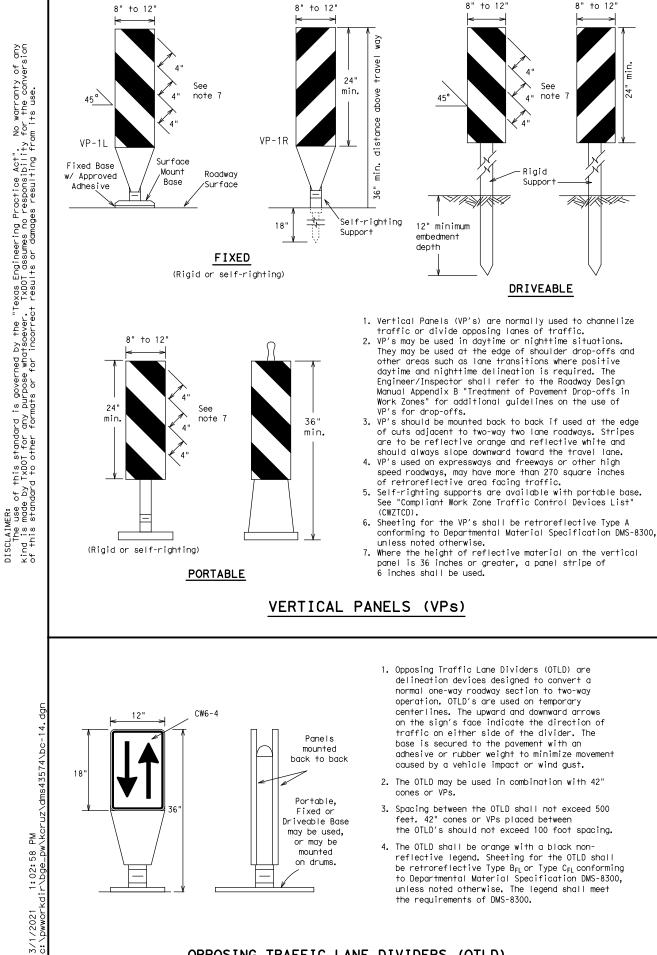


DETECTABLE PEDESTRIAN BARRICADES

- When existing pedestrian facilities are disrupted, cl relocated in a TIC zone, the temporary facilities sha detectable and include accessibility features consist the features present in the existing pedestrian facil
- 2. Where pedestrians with visual disabilities normally a closed sidewalk, a device that is detectable by a per with a visual disability traveling with the aid of a shall be placed across the full width of the closed
- Detectable pedestrian barricades similar to the one above, longitudinal channelizing devices, some concr barriers, and wood or chain link fencing with a cont detectable edging can satisfactorily delineate a ped path.
- 4. Tape, rope, or plastic chain strung between devices detectable, do not comply with the design standards "Americans with Disabilities Act Accessibility Guide for Buildings and Facilities (ADAAG)" and should not as a control for pedestrian movements.
- 5. Warning lights shall not be attached to detectable p barricades.
- Detectable pedestrian barricades may use 8" nominal barricade rails as shown on BC(10) provided that the rail provides a smooth continuous rail suitable for t trailing with no splinters, burrs, or sharp edges.

ion

| | 18" x 24" Sign (Maximum Sign Dimension) Chevron CWI-8, Opposing Traffic Lane Divider, Driveway sign D70a, Keep Right R4 series or other signs as approved by Engineer12" x 24" Vertical Panel mount with diagonals sloping down towards travel way |
|--|--|
| | Plywood, Aluminum or Metal sign substrates shall NOT be used on plastic drums |
| | SIGNS, CHEVRONS, AND VERTICAL PANELS MOUNTED ON PLASTIC DRUMS |
| t intended See note 3 | Signs used on plastic drums shall be manufactured using substrates listed on the CWZTCD. |
| st for oved rian | Chevrons and other work zone signs with an orange background shall be manufactured with Type B_{FL} or Type C_{FL}Orange sheeting meeting the color and retroreflectivity requirements of DMS-8300, "Sign Face Material," unless otherwise specified in the plans. |
| n ailing | Vertical Panels shall be manufactured with orange and white sheeting meeting the requirements of DMS-8300 Type A Diagonal stripes on Vertical Panels shall slope down toward the intended traveled lane. |
| | 4. Other sign messages (text or symbolic) may be used as approved by the Engineer. Sign dimensions shall not exceed 18 inches in width or 24 inches in height, except for the R9 series signs discussed in note 8 below. |
| | Signs shall be installed using a 1/2 inch bolt (nominal) and nut, two washers, and one locking washer for each connection. |
| | Mounting bolts and nuts shall be fully engaged and adequately torqued. Bolts should not extend more than 1/2 inch beyond nuts. |
| | 7. Chevrons may be placed on drums on the outside of curves, on merging tapers or on shifting tapers. When used in these locations they may be placed on every drum or spaced not more than on every third drum. A minimum of three (3) should be used at each location called for in the plans. |
| closed, or | R9-9, R9-10, R9-11 and R9-11a Sidewalk Closed signs which are 24 inches wide may be mounted on plastic drums, with approval of the Engineer. |
| nall be stent with ility. | SHEET 8 OF 12 |
| use the erson i long cane sidewalk. pictured | Traffic Operations Division Standard |
| rete Finuous destrian are not in the elines the used | BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES |
| pedestrian | BC (8) -14 |
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36' for at least 500 feet. requirements of DMS-8300. 6. For Long Term Stationary use on tapers or Fixed Base w/ Approved Adhesive (Driveable Base, or Flexible Support can be used) **CHEVRONS** ' 9 9 LONGITUDINAL CHANNELIZING DEVICES (LCD) 2. LCDs may be used instead of a line of cones or drums. used only when shown on the CWZTCD list. on BC(7) when placed roughly parallel to the travel lanes. LCD along the full length of the device. WATER BALLASTED SYSTEMS USED AS BARRIERS specific to the device, and used only when shown on the CWZTCD list. as per manufacturer recommendations or flared to a point outside the clear zone. of the unit shall not be less than 32 inches in height. HOLLOW OR WATER BALLASTED SYSTEMS USED AS LONGITUDINAL CHANNELIZING DEVICES OR BARRIERS

12"

18'

Min.

- 1. The chevron shall be a vertical rectangle with a minimum size of 12 by 18 inches.
- 2. Chevrons are intended to give notice of a sharp change of alignment with the direction of travel and provide additional emphasis and guidance for vehicle operators with regard to changes in horizontal alignment of the roadway.
- 3. Chevrons, when used, shall be erected on the outside of a sharp curve or turn, or on the far side of an intersection. They shall be in line with and at right angles to approaching traffic. Spacing should be such that the motorist always has three in view, until the change in alignment eliminates its need.
- 4. To be effective, the chevron should be visible
- 5. Chevrons shall be orange with a black nonreflective leaend. Sheeting for the chevron shall be retroreflective Type Br or Type Cr conforming to Departmental Material Specification DMS-8300, unless noted otherwise. The legend shall meet the
- transitions on freeways and divided highways self-righting chevrons may be used to supplement plastic drums but not to replace plastic drums.

- 1. LCDs are crashworthy, lightweight, deformable devices that are highly visible, have good target value and can be connected together. They are not designed to contain or redirect a vehicle on impact.
- 3. LCDs shall be placed in accordance to application and installation requirements specific to the device, and
- 4. LCDs should not be used to provide positive protection for obstacles, pedestrians or workers.
- 5. LCDs shall be supplemented with retroreflective delineation as required for temporary barriers
- 6. LCDs used as barricades placed perpendicular to traffic should have at least one row of reflective sheeting meeting the requirements for barricade rails as shown on BC(10) placed near the top of the
- 1. Water ballasted systems used as barriers shall not be used solely to channelize road users, but also to protect the
- work space per the appropriate NCHRP 350 crashworthiness requirements based on roadway speed and barrier application. 2. Water ballasted systems used to channelize vehicular traffic shall be supplemented with retroreflective delineation
- or channelizing devices to improve daytime/nighttime visibility. They may also be supplemented with pavement markings. 3. Water ballasted systems used as barriers shall be placed in accordance to application and installation requirements
- 4. Water ballasted systems used as barriers should not be used for a merging taper except in low speed (less than 45 MPH
- urban areas. When used on a taper in a low speed urban area, the taper shall be delineated and the taper length should be designed to optimize road user operations considering the available geometric conditions.
- When water ballasted systems used as barriers have blunt ends exposed to traffic, they should be attenuated

If used to channelize pedestrians, longitudinal channelizing devices or water ballasted systems must have a continuous detectable bottom for users of long canes and the top

OPPOSING TRAFFIC LANE DIVIDERS (OTLD)

DATE:

GENERAL NOTES

- 1. Work Zone channelizing devices illustrated on this sheet may be installed in close proximity to traffic and are suitable for use on high or low speed roadways. The Engineer/Inspector shall ensure that spacing and placement is uniform and in accordance with the "Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 2. Channelizing devices shown on this sheet may have a driveable, fixed or portable base. The requirement for self-righting channelizing devices must be specified in the General Notes or other plan sheets.
- 3. Channelizing devices on self-righting supports should be used in work zone areas where channelizing devices are frequently impacted by errant vehicles or vehicle related wind gusts making alignment of the channelizing devices difficult to maintain. Locations of these devices shall be detailed elsewhere in the plans. These devices shall conform to the TMUTCD and the "Compliant Work Zone Traffic Control Devices List" (CWZTCD).
- 4. The Contractor shall maintain devices in a clean condition and replace damaged, nonreflective, faded, or broken devices and bases as required by the Engineer/Inspector. The Contractor shall be required to maintain proper device spacing and alignment.
- 5. Portable bases shall be fabricated from virgin and/or recycled rubber. The portable bases shall weigh a minimum of 30 lbs.
- 6. Pavement surfaces shall be prepared in a manner that ensures proper bonding between the adhesives, the fixed mount bases and the pavement surface. Adhesives shall be prepared and applied according to the manufacturer's recommendations.
- 7. The installation and removal of channelizing devices shall not cause detrimental effects to the final pavement surfaces, including pavement surface discoloration or surface integrity. Driveable bases shall not be permitted on final pavement surfaces. The Engineer/Inspector shall approve all application and removal procedures of fixed bases.

| Posted Speed | Minimum Suggested Maxi Desirable Spacing of Formula Taper Lengths Channelizing X X Devices | | | ng of lizing | | | |
|-----------------|--|---------------|---------------|-----------------|---------------|-----------------|--|
| * | | 10' Offset | 11' Offset | 12' Offset | On a Taper | On a Tangent | |
| 30 | | 150′ | 165′ | 180′ | 30′ | 60′ | |
| 35 | $L = \frac{WS^2}{60}$ | 205′ | 225′ | 245′ | 35′ | 70′ | |
| 40 | 60 | 265′ | 295′ | 320′ | 40′ | 80′ | |
| 45 | | 450′ | 495′ | 540′ | 45′ | 90′ | |
| 50 | | 500′ | 550′ | 600′ | 50′ | 100′ | |
| 55 | L=WS | 550′ | 605′ | 660′ | 55′ | 110′ | |
| 60 | L 115 | 600′ | 660′ | 720′ | 60′ | 120′ | |
| 65 | | 650′ | 715′ | 780′ | 65′ | 130′ | |
| 70 | | 700′ | 770′ | 840′ | 70′ | 140′ | |
| 75 | | 750′ | 825′ | 900′ | 75′ | 150′ | |
| 80 | | 800′ | 880′ | 960′ | 80′ | 160′ | |

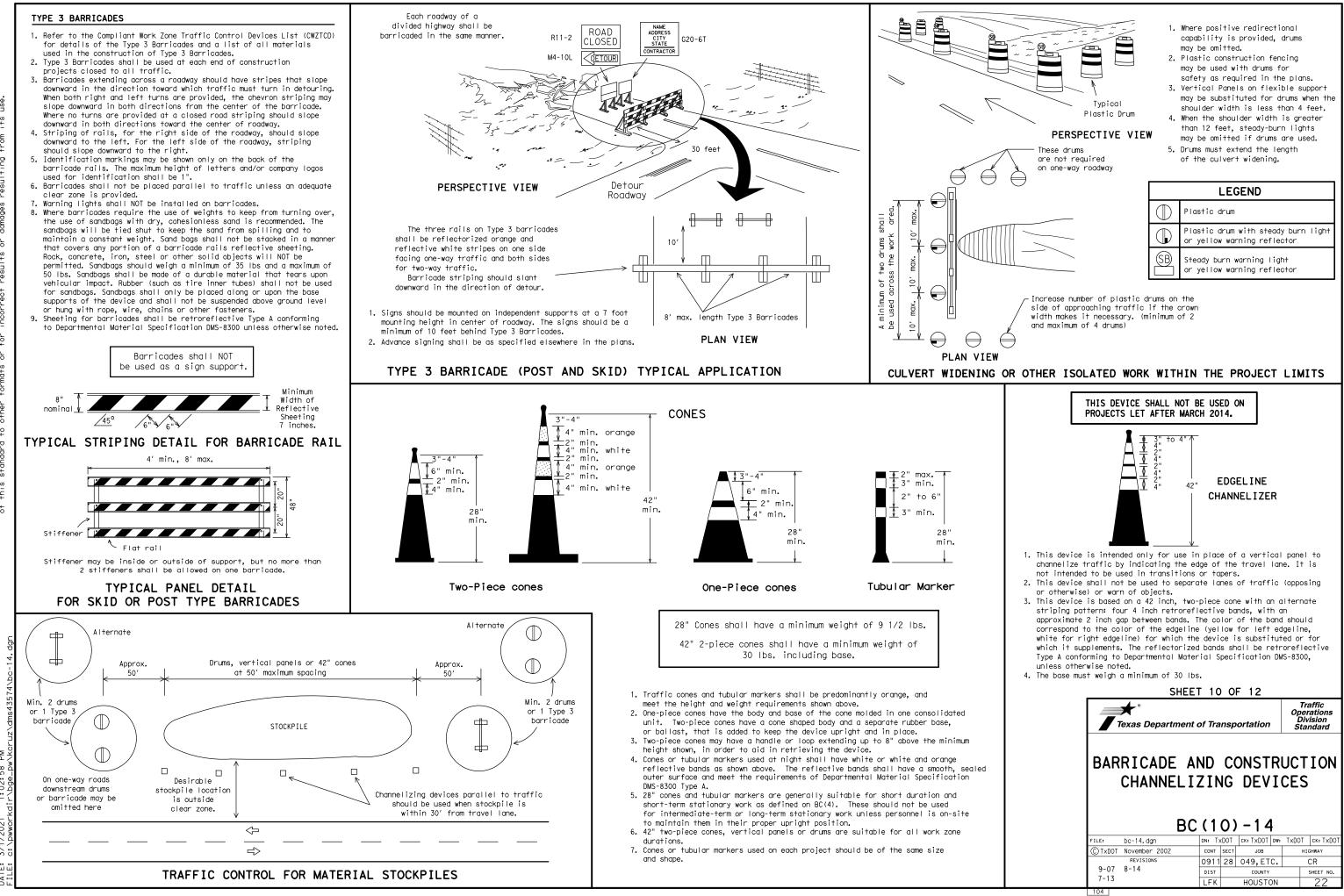
 \times Taper lengths have been rounded off. L=Length of Taper (FT.) W=Width of Offset (FT.) S=Posted Speed (MPH)

SUGGESTED MAXIMUM SPACING OF CHANNELIZING DEVICES AND MINIMUM DESIRABLE TAPER LENGTHS

| SHEET 9 OF 12 | |
|------------------------------------|---|
| Texas Department of Transportation | Traffic Operations Division Standard |
| | |

BARRICADE AND CONSTRUCTION CHANNELIZING DEVICES

| BC (9) – 14 | | | | | | | | |
|-------------|---------------|--|----------------|------|-----------|-----|-------|-----------|
| ILE: | bc-14.dgn | | dn: T> | DOT | ск: TxDOT | DW: | TxDOT | ск: ТхDОТ |
| C) TxDOT | November 2002 | | CONT | SECT | JOB | | ні | GHWAY |
| | REVISIONS | | 0911 | 28 | 049,ET | с. | | CR |
| 9-07 | 8-14 | | DIST | | COUNTY | | | SHEET NO. |
| 7-13 | | | LFK HOUSTON 21 | | | 21 | | |
| 103 | | | | | | | | |



i or

M 58 1:02: DATE:

WORK ZONE PAVEMENT MARKINGS

GENERAL

- 1. The Contractor shall be responsible for maintaining work zone and existing pavement markings, in accordance with the standard specifications and special provisions, on all roadways open to traffic within the CSJ limits unless otherwise stated in the plans.
- 2. Color, patterns and dimensions shall be in conformance with the Texas Manual on Uniform Traffic Control Devices" (TMUTCD).
- 3. Additional supplemental pavement marking details may be found in the plans or specifications.
- 4. Pavement markings shall be installed in accordance with the TMUTCD and as shown on the plans.
- 5. When short term markings are required on the plans, short term markings shall conform with the TMUTCD, the plans and details as shown on the Standard Plan Sheet WZ(STPM).
- 6. When standard pavement markings are not in place and the roadway is opened to traffic, DO NOT PASS signs shall be erected to mark the beginning of the sections where passing is prohibited and PASS WITH CARE signs at the beginning of sections where passing is permitted.
- 7. All work zone pavement markings shall be installed in accordance with Item 662, "Work Zone Pavement Markings.

RAISED PAVEMENT MARKERS

- 1. Raised pavement markers are to be placed according to the patterns on BC(12).
- 2. All raised pavement markers used for work zone markings shall meet the requirements of Item 672, "RAISED PAVEMENT MARKERS" and Departmental Material Specification DMS-4200 or DMS-4300.

PREFABRICATED PAVEMENT MARKINGS

- 1. Removable prefabricated pavement markings shall meet the requirements of DMS-8241.
- 2. Non-removable prefabricated pavement markings (foil back) shall meet the requirements of DMS-8240.

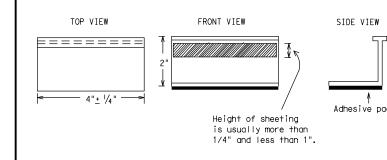
MAINTAINING WORK ZONE PAVEMENT MARKINGS

- 1. The Contractor will be responsible for maintaining work zone pavement markings within the work limits.
- 2. Work zone pavement markings shall be inspected in accordance with the frequency and reporting requirements of work zone traffic control device inspections as required by Form 599.
- 3. The markings should provide a visible reference for a minimum distance of 300 feet during normal daylight hours and 160 feet when illuminated by automobile low-beam headlights at night, unless sight distance is restricted by roadway geometrics.
- 4. Markings failing to meet this criteria within the first 30 days after placement shall be replaced at the expense of the Contractor as per Specification Item 662.

REMOVAL OF PAVEMENT MARKINGS

- 1. Pavement markings that are no longer applicable, could create confusion or direct a motorist toward or into the closed portion of the roadway shall be removed or obliterated before the roadway is opened to traffic.
- 2. The above shall not apply to detours in place for less than three days, where flaggers and/or sufficient channelizing devices are used in lieu of markings to outline the detour route.
- 3. Pavement markings shall be removed to the fullest extent possible, so as not to leave a discernable marking. This shall be by any method approved by TxDOT Specification Item 677 for "Eliminating Existing Pavement Markinas and Markers".
- 4. The removal of pavement markings may require resurfacing or seal coating portions of the roadway as described in Item 677.
- 5. Subject to the approval of the Engineer, any method that proves to be successful on a particular type pavement may be used.
- 6. Blast cleaning may be used but will not be required unless specifically shown in the plans.
- 7. Over-painting of the markings SHALL NOT BE permitted.
- 8. Removal of raised pavement markers shall be as directed by the Engineer.
- 9. Removal of existing pavement markings and markers will be paid for directly in accordance with Item 677, "ELIMINATING EXISTING PAVEMENT MARKINGS AND MARKERS, " unless otherwise stated in the plans.
- 10.Black-out marking tape may be used to cover conflicting existing markings for periods less than two weeks when approved by the Engineer.

Temporary Flexible-Reflective Roadway Marker Tabs



STAPLES OR NAILS SHALL NOT BE USED TO SECURE TEMPORARY FLEXIBLE-REFLECTIVE ROADWAY MARKER TABS TO THE PAVEMENT SURFACE

- 1. Temporary flexible-reflective roadway marker tabs used as guidemarks shall meet the requirements of DMS-8242.
- 2. Tabs detailed on this sheet are to be inspected and accepted by the Engineer or designated representative. Sampling and testing is not normally required, however at the option of the Engineer, either "A" or "B" below may be imposed to assure quality before placement on the roadway.
 - A. Select five (5) or more tabs at random from each lot or shipment and submit to the Construction Division, Materials and Pavement Section to determine specification compliance.
 - B. Select five (5) tabs and perform the following test. Affix five (5) tabs at 24 inch intervals on an asphaltic pavement in a straight line. Using a medium size passenger vehicle or pickup, run over the markers with the front and rear tires at a speed of 35 to 40 miles per hour, four (4) times in each direction. No more than one (1) out of the five (5) reflective surfaces shall be lost or displaced as a result of this test.
- 3. Small design variances may be noted between tab manufacturers.
- 4. See Standard Sheet WZ(STPM) for tab placement on new pavements. See Standard Sheet TCP(7-1) for tab placement on seal coat work.

RAISED PAVEMENT MARKERS USED AS GUIDEMARKS

- 1. Raised pavement markers used as guidemarks shall be from the approved product list, and meet the requirements of DMS-4200.
- 2. All temporary construction raised pavement markers provided on a project shall be of the same manufacturer.
- 3. Adhesive for auidemarks shall be bituminous material hot applied or butyl rubber pad for all surfaces, or thermoplastic for concrete surfaces.

Guidemarks shall be designated as:

YELLOW - (two amber reflective surfaces with yellow body). WHITE - (one silver reflective surface with white body).

M 59 1:02: r\bge.

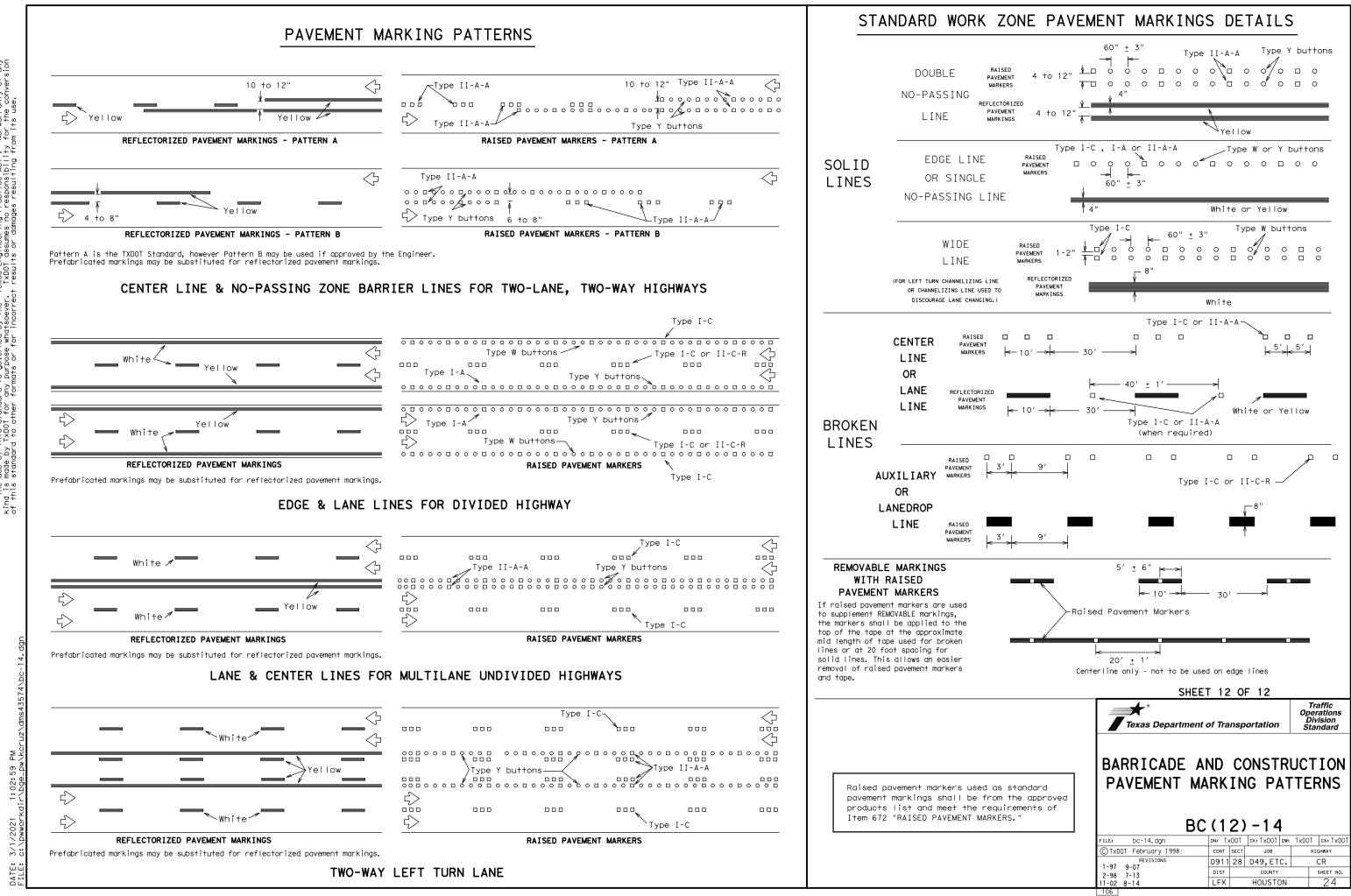
3/1/2021

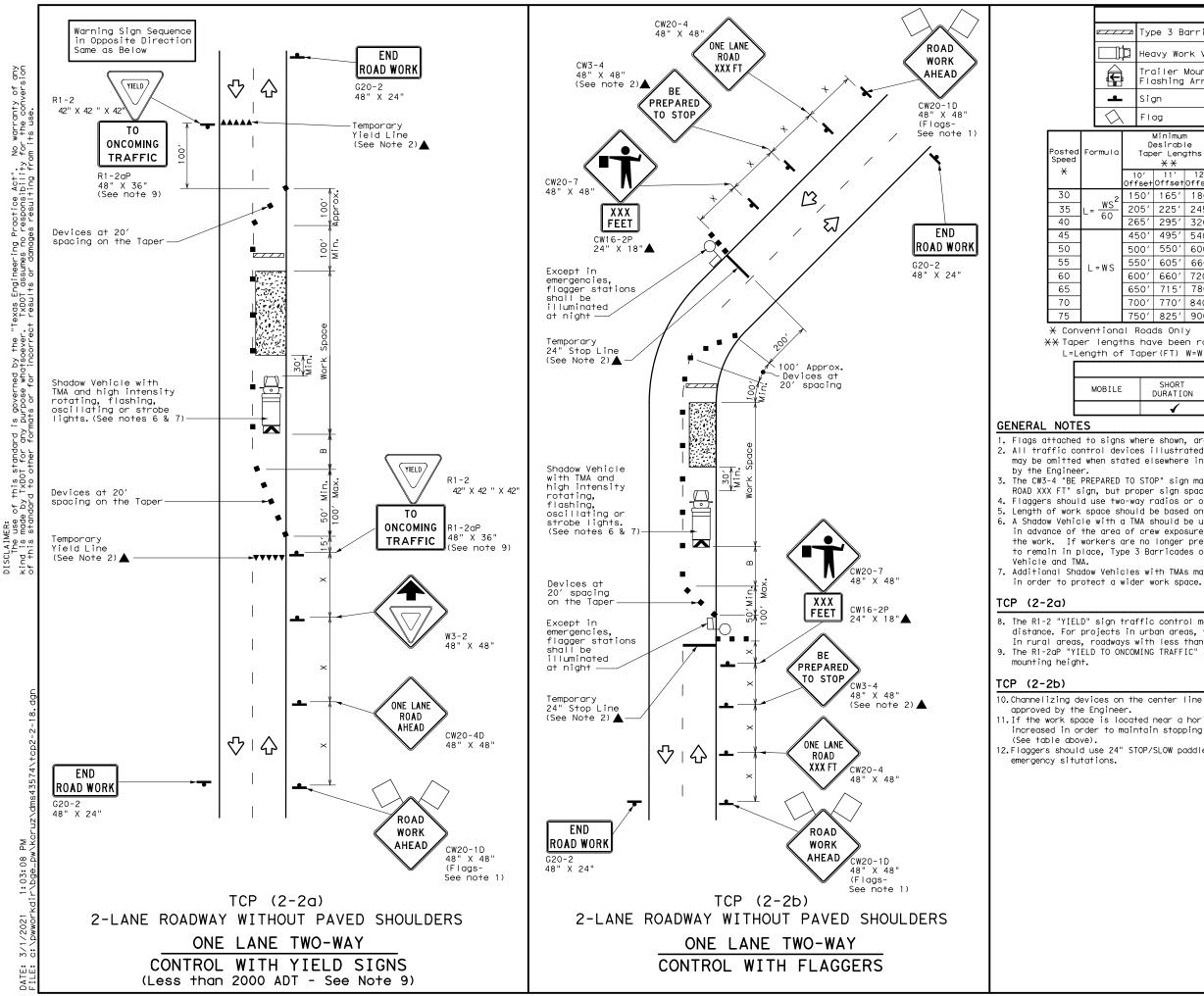
DATE:

| DEPARTMENTAL MATERIAL SPECIFICATIONS | | | | | |
|---|----------|--|--|--|--|
| PAVEMENT MARKERS (REFLECTORIZED) | DMS-4200 | | | | |
| TRAFFIC BUTTONS | DMS-4300 | | | | |
| EPOXY AND ADHESIVES | DMS-6100 | | | | |
| BITUMINOUS ADHESIVE FOR PAVEMENT MARKERS | DMS-6130 | | | | |
| PERMANENT PREFABRICATED PAVEMENT MARKINGS | DMS-8240 | | | | |
| TEMPORARY REMOVABLE, PREFABRICATED PAVEMENT MARKINGS | DMS-8241 | | | | |
| TEMPORARY FLEXIBLE, REFLECTIVE ROADWAY MARKER TABS | DMS-8242 | | | | |

A list of predualified reflective raised pavement markers. non-reflective traffic buttons, roadway marker tabs and other pavement markings can be found at the Material Producer List web address shown on BC(1).

| SHEET 11 OF 12 | | | | | | | |
|--|--------|------|----------|------|--|--|--|
| | | | | | Traffic erations Division tandard | | |
| BARRICADE AND CONSTRUCTION PAVEMENT MARKINGS BC(11)-14 | | | | | | | |
| FILE: bc-14. dgn | DN: TX | | | TxDO | T CK: TXDOT | | |
| (C) TxDOT February 1998 | - | SECT | JOB | 1 | HIGHWAY | | |
| REVISIONS | | 28 | 049,ETC. | | CR | | |
| 2-98 9-07 1-02 7-13 | DIST | | COUNTY | - 1 | SHEET NO. | | |
| | | | | | | | |





| LEGEND | | | | | | | | | |
|--------|---|--|---------------|---------------|-----------------------------------|-----------------------------|-------------------------------|------|------|
| | | ₫ Type 3 Barricade | | | | С | hanneliz | | |
|][| Heavy Work Vehicle | | | | | ruck Mour ttenuator | | | |
| | Trailer Mounted Flashing Arrow Board | | | M | | Portable Message S | | | |
| Sign | | | \langle | Т | raffic F | 1 | | | |
| λ | F | lag | | | ПO | F | lagger | | |
| a | To | Minimum Suggested Maximum Desirable Spacing of Taper Lengths Channelizing X X Devices | | m | Minimum Sign Spacing "X" | Suggested S Longitudinal | Stopping Sight Distance | | |
| | 10′ Offse | 11' e+Offset | 12' Offset | On a Taper | On a Tangen | + | Distance | "B" | |
| 2 | 150 | 1651 | 180′ | 30′ | 60′ | | 120′ | 90′ | 200′ |
| - | 205 | ' 225' | 245′ | 35′ | 70′ | | 160′ | 120′ | 250′ |
| | 265 | ′ 295′ | 320′ | 40′ | 80′ | | 240' | 155′ | 305′ |
| | 450 | ' 495' | 540′ | 45′ | 90′ | | 320′ | 195′ | 360′ |
| | 500 | ' 550' | 600′ | 50′ | 100′ | | 400′ | 240′ | 425′ |
| | 550 | ′ 605′ | 660′ | 55′ | 110′ | | 500′ | 295′ | 495′ |
| | 600 | ′ 660′ | 720′ | 60′ | 120′ | | 600′ | 350′ | 570′ |
| | 650 | ′ 715′ | 780′ | 65′ | 130′ | | 700′ | 410′ | 645′ |
| | 700 | ' 770' | 840′ | 70′ | 140′ | | 800′ | 475′ | 730′ |
| | 750 | ' 825' | 900′ | 75′ | 150′ | | 900′ | 540′ | 820′ |

 $\ensuremath{\text{X}}\xspace$ Taper lengths have been rounded off.

L=Length of Taper(FT) W=Width of Offset(FT) S=Posted Speed(MPH)

| | TYPICAL USAGE | | | | | | |
|-----------------------|-------------------|--------------------------|---------------------------------|-------------------------|--|--|--|
| E | SHORT DURATION | SHORT TERM STATIONARY | INTERMEDIATE TERM STATIONARY | LONG TERM STATIONARY | | | |
| ✓ | | 1 | 1 | | | | |

1. Flags attached to signs where shown, are REQUIRED. 2. All traffic control devices illustrated are REQUIRED, except those denoted with the triangle symbol may be omitted when stated elsewhere in the plans, or for routine maintenance work, when approved

3. The CW3-4 "BE PREPARED TO STOP" sign may be installed after the CW20-4 "ONE LANE ROAD XXX FT" sign, but proper sign spacing shall be maintained. 4. Flaggers should use two-way radios or other methods of communication to control traffic. 5. Length of work space should be based on the ability of flaggers to communicate. 6. A Shadow Vehicle with a TMA should be used anytime it can be positioned 30 to 100 feet in advance of the area of crew exposure without adversely affecting the performance or quality of the work. If workers are no longer present but road or work conditions require the traffic control to remain in place, Type 3 Barricades or other channelizing devices may be substituted for the Shadow

7. Additional Shadow Vehicles with TMAs may be positioned off the paved surface, next to those shown

8. The R1-2 "YIELD" sign traffic control may be used on projects with approaches that have adequate sight distance. For projects in urban areas, work space should be no longer than one half city block. In rural areas, roadways with less than 2000 ADT, work space should be no longer than 400 feet. 9. The R1-2aP "YIELD TO ONCOMING TRAFFIC" sign shall be placed on a support at a 7 foot minimum

10. Channelizing devices on the center line may be omitted when a pilot car is leading traffic and

11. If the work space is located near a horizontal or vertical curve, the buffer distances should be increased in order to maintain stopping sight distance to the flagger and a queue of stopped vehicles.

12.Flaggers should use 24" STOP/SLOW paddles to control traffic. Flags should be limited to

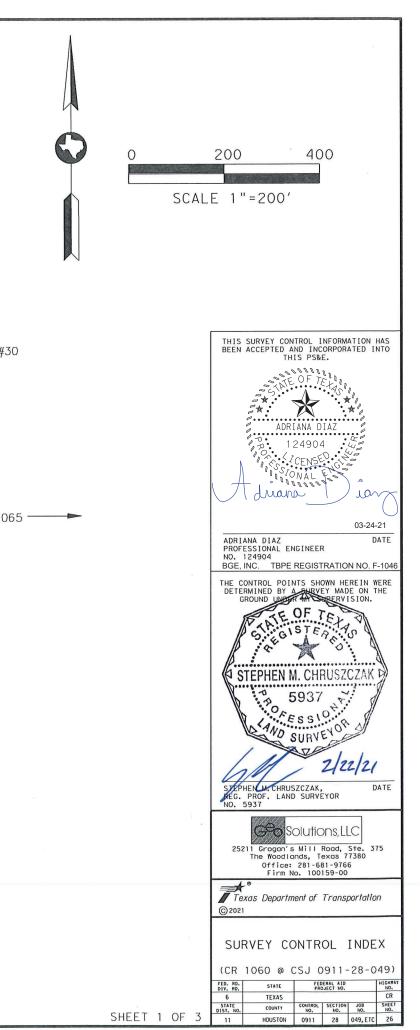
| Texas Department | of Trans | sportation | Op L | Traffic perations Division tandard |
|--|----------------|------------|---------|---|
| TRAFFIC ONE-LA TRAFF | NE | TWO-W | AY | N |
| TCP | (2-3 | 2)-1 | 8 | |
| FILE: tcp2-2-18. dgn | (2-) DN: | | 8 | CK: |
| | DN: | | - | CK: HIGHWAY |
| FILE: tcp2-2-18, dgn © TxDOT December 1985 REVISIONS | DN: CONT SE | СК: | DW: | |
| FILE: tcp2-2-18.dgn © TxDOT December 1985 | DN: CONT SE | CK: | DW: | HIGHWAY |

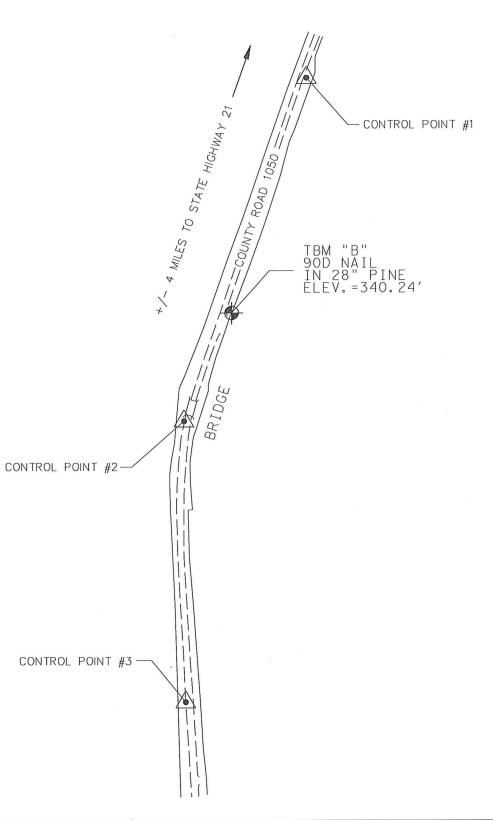
---+/- 1.4 MILES TO COUNTY ROAD 1055 BRIDGE - CONTROL POINT #30 4 COUNTY ROAD 1060 =. 0 CONTROL POINT #10-70J. └ CONTROL POINT #20 TBM "C" 90D NAIL – IN 30" PINE ELEV.=315.22' +/- 1.0 MILES TO COUNTY ROAD 1065

| CONTROL POINT NUMBER | NORTHING | EASTING | ELEVATION | MONUMENT DESCRIPTION |
|-------------------------|---------------|--------------|-----------|---|
| 10 | 10,504,735.40 | 3,874,014.02 | 320.75' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 20 | 10,504,896.22 | 3,874,508.83 | 311.14' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 30 | 10,504,814.09 | 3,875,242.82 | 320.24' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| твм "с" | 10,504,842.31 | 3,874,436.83 | 315.22' | SET 90D NAIL IN 30" PINE TREE |

NOTES:

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.

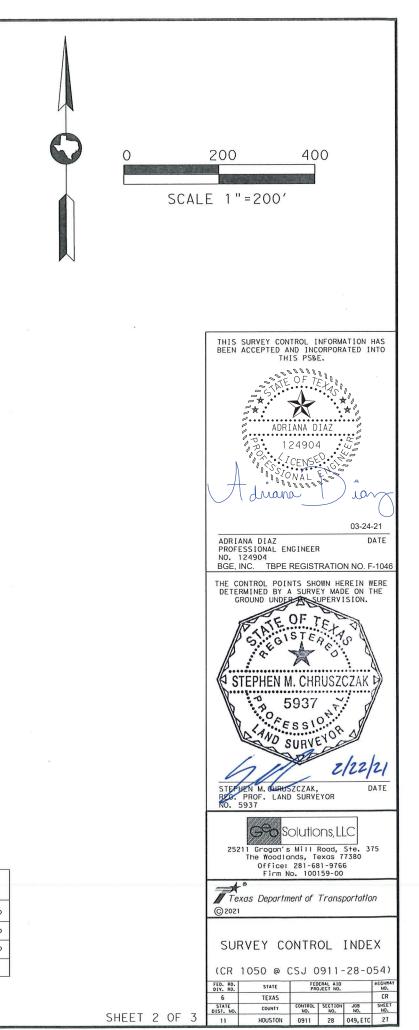


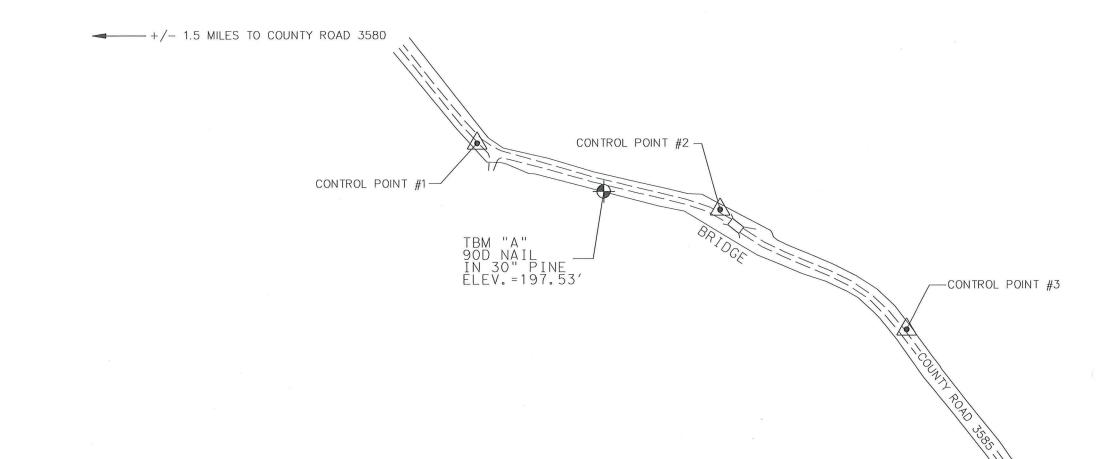


| NOT | FS: |
|-------|-----|
| I VVI | LJO |

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.

| CONTROL POINT NUMBER | NORTHING | EASTING | ELEVATION | MONUMENT DESCRIPTION |
|-------------------------|---------------|--------------|-----------|---|
| 1 | 10,504,577.37 | 3,861,871.15 | 349.38' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 2 | 10,503,862.99 | 3,861,615.95 | 334.81' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 3 | 10,503,276.40 | 3,861,619.93 | 338.33' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| твм "в" | 10,504,088.17 | 3,861,715.06 | 340.24' | SET 90D NAIL IN 28" PINE TREE |



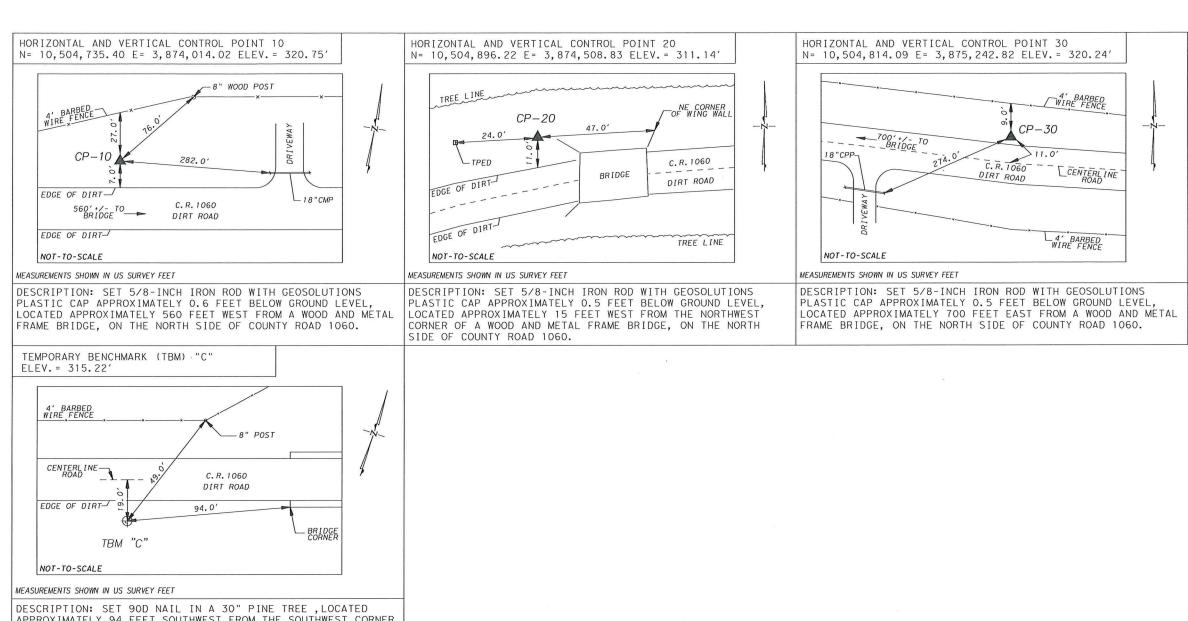


| CONTROL POINT NUMBER | NORTHING | EASTING | ELEVATION | MONUMENT DESCRIPTION |
|-------------------------|---------------|--------------|-----------|---|
| 1. | 10,361,849.59 | 3,793,330.93 | 198.19' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 2 | 10,361,711.26 | 3,793,839.35 | 194.06' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| 3 | 10,361,460.47 | 3,794,230.14 | 190.72' | SET 5/8-INCH IRON ROD WITH "GEOSOLUTIONS" CAP |
| TBM "A" | 10,361,749.19 | 3,793,595.77 | 197.53' | SET 90D NAIL IN 30" PINE TREE |

NOTES:

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.

| CONTROL INCOMPATION INCOMPATIO | |
|--|---|
| BEEN ACCEPTED AND INCORPORATED INTO ADRIANA DIAZ ADRIANA DIAZ 124904 ADRIANA DIAZ 03-24-21 ADRIANA DIAZ ADRIANA DIAZ ADRIANA DIAZ ADRIANA DIAZ ADRIANA DIAZ ADRIANA DIAZ MAL MAL 03-24-21 ADRIANA DIAZ DATE PROPOSITION NO. F-1046 THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION. STEPHEN M. CHRUSZCZAK 5937 5937 5937 5937 CES 100 SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY SURVEY ADRIANA DIAZ DATE NO. 5337 CES 10 Grogort's Mill Road, Stee, 375 OFTIGE 281-681-9316 FITM ROAL STEE, 375 FEAS DEpartment of Transportation (2221) | |
| NO. 124904 BGE, INC. TEPE REGISTRATION NO. F-1046 THE CONTROL POINTS SHOWN HEREIN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION. FOR TEAM OF TEAM STEPHEN M. CHRUSZCZAK F STEPHEN M. CHRUSZCZAK, DATE STEPHEN M. CHRUSZCZAK, CONSULT STEPHEN M. CHRUSZCZAK, DATE STEPHEN M. CHRUSZCZAK, CONSULT STEPHEN M. CHRU | BEEN ACCEPTED AND INCORPORATED INTO THIS PS&E. |
| 25211 Grogon's Mill Road, Ste. 375 The Woodlands, Texas 77380 Office: 281-681-9766 Firm No. 100159-00 Texas Department of Transportation © 2021 | NO. 124904 BGE, INC. TBPE REGISTRATION NO. F-1046 THE CONTROL POINTS SHOWN HERE IN WERE DETERMINED BY A SURVEY MADE ON THE GROUND UNDER MY SUPERVISION. FOF TEACHER STEPHEN M. CHRUSZCZAK, STEPHEN M. CHRUSZCZAK, THO SURVEYOR DATE |
| Image: Constraint of the state FEB. RD. STATE FEB. RD. FEB. STATE FEB. STATE FEB. RD. FEB. STATE | 25211 Grogan's Mill Road, Ste. 375 The Woodlands, Texas 77380 Office: 281-681-9766 Firm No. 100159-00 Texas Department of Transportation © 2021 SURVEY CONTROL INDEX (CR 3585 @ CSJ 0911-28-060) FD. RD. DIV. RD. 6 TEXAS |



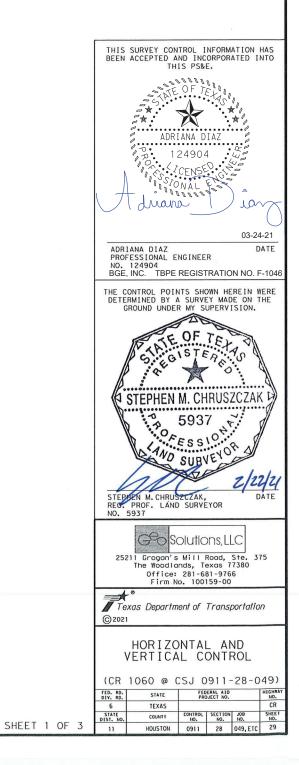
APPROXIMATELY 94 FEET SOUTHWEST FROM THE SOUTHWEST CORNER OF A WOOD AND METAL FRAME BRIDGE, ON THE SOUTH SIDE OF COUNTY ROAD 1060.

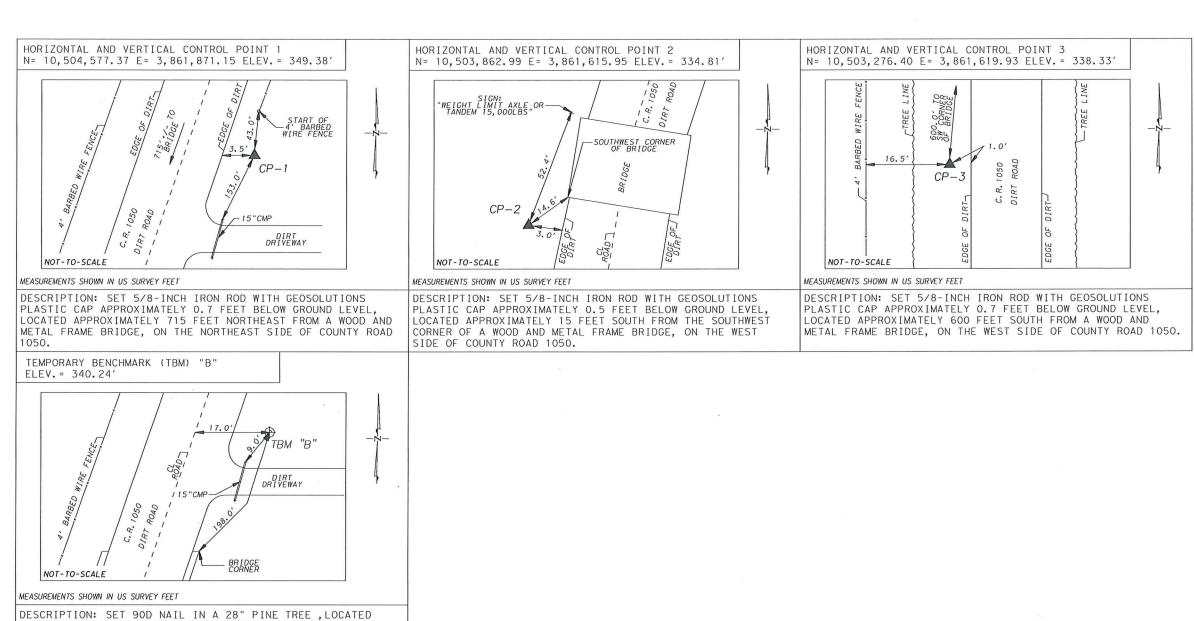
LEGEND:

▲ See noted on Sketch. (Control Point)

NOTES:

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.



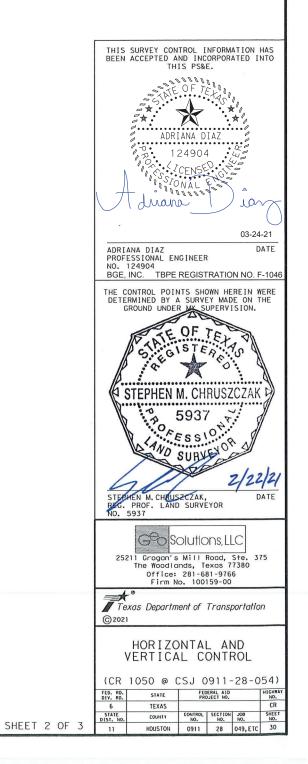


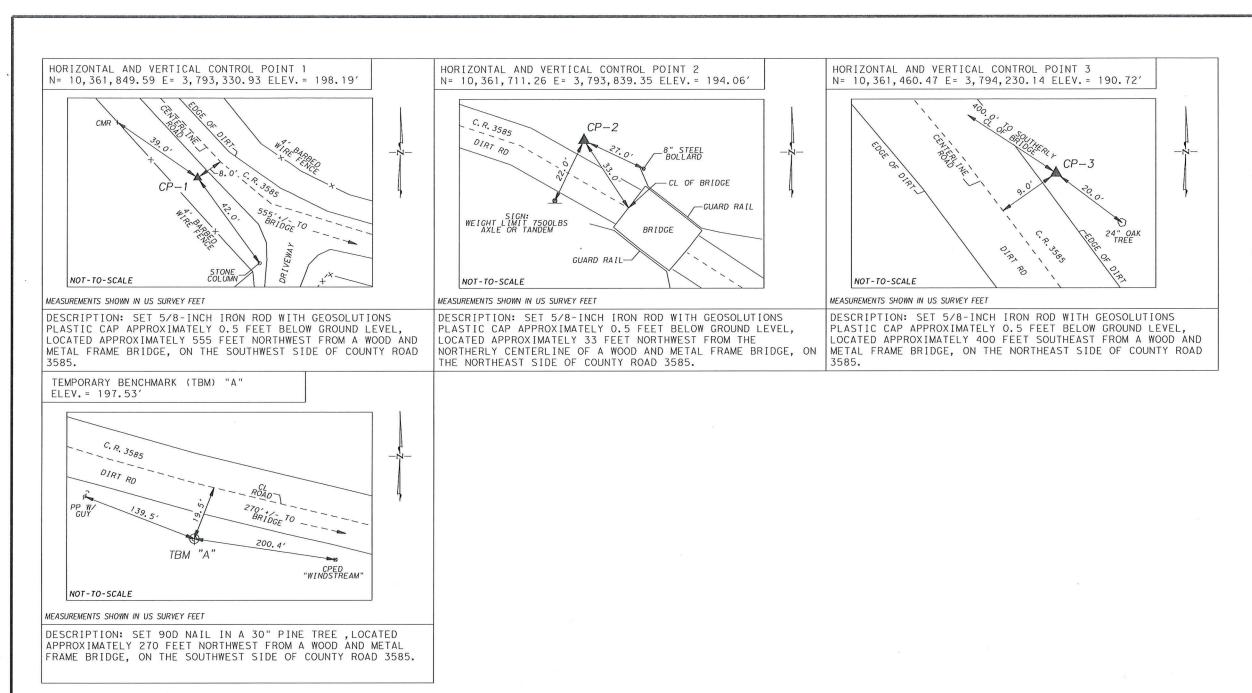
APPROXIMATELY 198 FEET NORTHEAST FROM THE NORTHEAST CORNER OF A WOOD AND METAL FRAME BRIDGE, ON THE EAST SIDE OF COUNTY ROAD 1060. LEGEND:

▲ See noted on Sketch. (Control Point)

NOTES:

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.





LEGEND:

▲ See noted on Sketch. (Control Point)

NOTES:

1.HORIZONTAL CONTROL IS BASED ON RTK GPS OBSERVATIONS HOLDING TXDOT (VRS) NETWORK. ALL COORDINATES ARE RELATED TO TEXAS COORDINATE SYSTEM NAD 83 CENTRAL ZONE, US SURVEY FEET. DISTANCES ARE IN SURFACE AND CAN BE CONVERTED TO GRID BY DIVIDING A COMBINED SCALE FACTOR OF 1.00012.



WRIGHT CREEK BRIDGE (CSJ 0911-28-060)

| Beginning chain CR3585 description | | | | | | | |
|--------------------------------------|--------------|------------------------|-----------------|--|--|--|--|
| | Curve D * | ata * | | | | | |
| Curve CR35851 | | | | | | | |
| P.I. Station 100+50.16 | N | 10,362,011.08 E | 3,793,203.00 | | | | |
| Delta = 1° 33′ 12.90" | | 10,002,011100 2 | 0,100,200,00 | | | | |
| Degree = 1° 32′ 09.65" | | | | | | | |
| Tangent = 50.57 | | | | | | | |
| Length = 101.14 | | | | | | | |
| Radius = 3,730.16 | | | | | | | |
| External = 0.34 | | | | | | | |
| Long Chord = 101.14 | | | | | | | |
| Mid. Ord. = 0.34 | | | | | | | |
| | NI. | | 7 707 171 00 | | | | |
| | | 10,362,050.95 E | 3,793,171.89 | | | | |
| P.T. Station 101+00.73 | N | 10, 361, 972.06 E | 3,793,235.18 | | | | |
| | Ν | 10,364,345.49 E | 3,796,112.85 | | | | |
| Back = $S 37^{\circ} 57' 41.08" E$ | | | | | | | |
| Ahead = $S 39^{\circ} 30' 53.98'' E$ | | | | | | | |
| Chord Bear = S 38° 44' 17.53" E | | | | | | | |
| Course from PT CR35851 to PC CR358 | 52 S 39° | 30' 53.98" E Dist 110. | 98 | | | | |
| | Curve D | lata | | | | | |
| | ¥ | * | | | | | |
| Curve CR35852 | n | <u>^</u> | | | | | |
| P. I. Station 102+37.96 | N | 10 361 966 20 E | 3 703 322 50 | | | | |
| $Delta = 11^{\circ} 12' 37.50''$ | | 10,361,866.20 E | 3, 793, 322. 50 | | | | |
| | (LT) | | | | | | |
| Degree = 21° 25′ 20.68" | | | | | | | |

| | 102 01100 | | , | - | 0, 100, 022100 |
|----------------|------------------|------|---------------|---|----------------|
| Delta = | 11° 12′ 37.50" | (LT) | | | |
| Degree = | 21° 25′ 20.68" | | | | |
| Tangent = | 26.25 | | | | |
| Length = | 52.33 | | | | |
| Radius = | 267.46 | | | | |
| External = | 1.28 | | | | |
| Long Chord = | 52.25 | | | | |
| Mid. Ord. = | 1.28 | | | | |
| P.C. Station | 102+11.71 | N | 10,361,886.45 | E | 3,793,305.79 |
| P.T. Station | 102+64.04 | N | 10,361,849.58 | E | 3,793,342.82 |
| C.C. | | N | 10,362,056.62 | E | 3,793,512.13 |
| Back = S | 39° 30′ 53.98" E | | | | |
| Ahead = S | 50° 43′ 31.47" E | | | | |
| Chord Bear = S | 45° 07′ 12.73" E | | | | |
| | | | | | |

Course from PT CR35852 to PC CR35853 S 50° 43' 31.47" E Dist 15.21

Course from PT CR35853 to PC CR35854 S 74° 27' 08.28" E Dist 206.95

| | | Curve De | | | |
|----------------|------------------|----------|------------------|---|-----------------|
| Curve CR35853 | | | | | |
| P.I. Station | 102+90.94 | Ν | 10,361,832.55 | E | 3,793,363.64 |
| Delta = | 23° 43′ 36.81" | (LT) | | | |
| Degree = | 102° 53′ 19.52" | | | | |
| Tangent = | 11.70 | | | | |
| Length = | 23.06 | | | | |
| Radius = | 55.69 | | | | |
| External = | 1.22 | | | | |
| Long Chord = | 22.90 | | | | |
| Mid. Ord. = | 1.19 | | | | |
| P.C. Station | 102+79.24 | N | 10,361,839.95 | E | 3,793,354.59 |
| P.T. Station | 103+02.30 | Ν | 10, 361, 829. 41 | E | 3, 793, 374. 91 |
| C.C. | | Ν | 10,361,883.06 | E | 3, 793, 389. 84 |
| Back = S | 50° 43′ 31.47" E | | | | |
| Ahead = S | 74° 27′ 08.28" E | | | | |
| Chord Bear = S | 62° 35′ 19.88" E | | | | |

| Curve CR35854 P.I. Station Delta = Degree = Tangent = Length = Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S Chord Bear = S | 105+63.91 4° 31′ 54.64" 4° 08′ 50.07" 54.67 109.27 1,381.54 1.08 109.25 1.08 105+09.25 106+18.52 74° 27′ 08.28" E 78° 59′ 02.92" E 76° 43′ 05.60" E | N (LT) N N N | 10, 361, 759. 29 E 10, 361, 773. 94 E 10, 361, 748. 84 E 10, 363, 104. 93 E | 3, 7 3, 7 3, 7 3, 7 |
|---|--|--------------------------|--|------------------------------|
| Course from PT C | R35854 to PC CR358 | 55 S 78° | 59' 02.92" E Dist 67.24 | |
| | | Curve D | | |
| Curve CR35855 P.I. Station Delta = Degree = Tangent = Length = | 107+27.49 21° 58′ 17.17" 26° 38′ 57.12" 41.74 82.45 | N (RT) | 10,361,728.02 E | 3,7 |
| Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S Chord Bear = S | 215.00 4.01 81.94 3.94 106+85.76 107+68.20 78° 59' 02.92" E 57° 00' 45.75" E 67° 59' 54.33" E | N N N | 10, 361, 736. 00 E 10, 361, 705. 30 E 10, 361, 524. 96 E | 3, 7 3, 7 3, 7 |
| Course from PT CA | R35855 +o PC CR358 | 56 S 57° | 00' 45.75" E Dist 117.0 | 0 |
| | | Curve D * | | |
| Curve CR35856 P.I. Station Delta = Degree = Tangent = Length = | 109+11.53 12°01′13.78" 22°55′05.92" 26.32 52.45 | N (LT) | 10,361,627.27 E | 3,7 |
| Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S Chord Bear = S | 250.00 1.38 52.35 1.37 108+85.20 109+37.65 57° 00' 45.75" E 69° 01' 59.53" E 63° 01' 22.64" E | N N N | 10,361,641.60 E 10,361,617.85 E 10,361,851.30 E | 3, 7 3, 7 3, 7 |
| Course from PT (| 235856 to PC CR358 | 57 5 69" | 01' 59.53" F Dist 52.53 | |

Curve Data *----*

Curve CR35854 P.I. Station

| Course from PT C | R35856 to PC CR358 | 357 S 69° | 01′ 59.53" E D |)ist 52.53 |
|---|--|-------------|---|------------|
| | | Curve [|)ata * | |
| Curve CR35857 P.I. Station Delta = | 110+26.73 5°28′07.28" | N (RT) | 10,361,585.97 | E 3,7 |
| Degree = Tangent = Length = Radius = | 7°29′09.33" 36.55 73.05 765.38 | | | |
| External = Long Chord = Mid. Ord. = | 0.87 73.03 0.87 | | | |
| P.C. Station P.T. Station C.C. | 109+90.18 110+63.23 | N N N | 10,361,599.05 10,361,569.70 10,360,884.35 | E 3,7 |
| Back = S Ahead = S Chord Bear = S | 69° 01′ 59.53" E 63° 33′ 52.25" E 66° 17′ 55.89" E | | | |

Course from PT CR35857 to PC CR35858 S 63° 33' 52.25" E Dist 21.65

TBL

3,793,626.95

, 793, 574. 29 , 793, 680. 61 , 793, 944. 59

,793,787.57

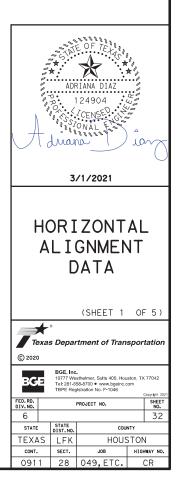
, 793, 746.61 , 793, 822.58 , 793, 705.52

,793,942.80

, 793, 920. 72 , 793, 967. 38 , 794, 056. 83

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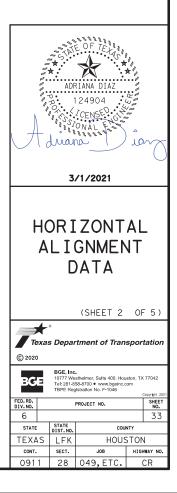
794,016.42 794,083.29 793,742.55



WRIGHT CREEK BRIDGE (CSJ 0911-28-060), CONTINUED

| | | | • | | |
|--|---|----------------|--|------------|---|
| | | Curve [| | | |
| Curve CR35858 P.I. Station Delta = Degree = Tangent = Length = Radius = | 111+42.47 22°51′22.65" 20°06′43.78" 57.59 113.64 284.88 | N | 10,361,534.43 | E | 3, 794, 154. 24 |
| External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S | 5.76 112.89 5.65 110+84.88 111+98.52 63° 33′ 52.25″ E | N N N | 10,361,560.06 10,361,490.77 10,361,304.97 | E | 3, 794, 102.67 3, 794, 191.80 3, 793, 975.85 |
| Course from PT (| R35858 to PC CR358 | 359 S 40° | 42′ 29.60" E D |)ist 18.01 | |
| | | Curve [| | | |
| Curve CR35859 P.I. Station Delta = Degree = Tangent = Length = Radius = | 112+44.67 4° 15′ 58.18" 7° 34′ 59.42" 28.14 56.26 755.56 | N | 10,361,455.79 | Ε | 3, 794, 221. 90 |
| External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S Chord Bear = S | | N N N | 10,361,477.12 10,361,433.15 10,360,984.34 | | 3, 794, 203. 54 3, 794, 238. 61 3, 793, 630. 79 |
| | | Curve [| | | |
| Curve CR358510 P.I. Station Delta = Degree = Tangent = Length = Radius = | 113+10.69 1°00'24.49" 1°19'41.79" 37.90 75.80 4,313.55 | * N (LT) | * 10,361,402.66 | Ε | 3,794,261.13 |
| | 0.17 75.80 0.17 112+72.79 113+48.59 36° 26′ 31.42" E 37° 26′ 55.91" E 36° 56′ 43.67" E | | 10, 361, 433. 15 10, 361, 372. 57 10, 363, 995. 44 | E | 3, 794, 238. 61 3, 794, 284. 17 3, 797, 708. 68 |
| Course from PT (| R358510 to PC CR35 | 58511 S 3 | 7° 26′ 55.91" E | Dist 219 | . 94 |
| | | Curve [| | | |
| Curve CR358511 P.I. Station Delta = Degree = Tangent = Length = Padius = | 116+16.08 1° 02' 45.79" 1° 06' 00.21" 47.55 95.09 | * N (RT) | * 10,361,160.21 | E | 3, 794, 446. 82 |
| Radius = External = Long Chord = Mid. Ord. = P.C. Station P.T. Station C.C. Back = S Ahead = S | | | 10,361,197.96 10,361,121.94 10,358,030.96 | E | 3, 794, 417. 91 3, 794, 475. 04 3, 790, 282. 96 |

Ending chain CR3585 description



| | HICKORY CREEK BRIDGE (C | | | |
|--------------------------------|--|---|--|--|
| | Beginning chain CR1050 descriptio | | | Curve Data Continued ** |
| | Point CR105001 N 10,503 | 5,079.13 E 3,861,639.39 Sta | 99+99.69 | External = 0.15 Long Chord = 36.60 Mid. Ord. = 0.15 |
| | Course from CR105001 to PC CR1050 | 01 N 3° 22′ 49.13" W Dist 404.18 Curve Data | | P.C. Station 109+98.68 N 10,504,063.31 E 3,861,687.8 P.T. Station 110+35.28 N 10,504,097.88 E 3,861,699.8 C.C. N 10,504,442.07 E 3,860,654.1 |
| ayscale.plt | Curve CR10501 P.I. Station 104+62.05 Delta = 4°26′30.49" Degree = 3°49′10.99" | (LT) | 3,861,612.12 | Back = N 20° 07' 27.39" E Ahead = N 18° 13' 09.97" E Chord Bear = N 19° 10' 18.68" E Course from PT CR10504 to PC CR10505 N 18° 13' 09.97" E Dist 162.50 |
| grays | Tangent = 58.17 Length = 116.29 | | | Curve Data |
| t:\pcsetup\PL0TDRV\T×D0T_pdf_ | Radius = 1,500.00 External = 1.13 Long Chord = 116.26 Mid. Ord. = 1.13 P.C. Station 104+03.88 P.T. Station 105+20.16 C.C. Back = Back = N 3° 22' 49.13" W | N 10,503,482.60 E N 10,503,598.31 E N 10,503,394.16 E | 3,861,615.55 3,861,604.21 3,860,118.16 | ** Curve CR10505 P.I. Station 112+16.23 N 10,504,269.76 E 3,861,756.4 Delta = 0° 41′ 10.56" (LT) Degree = 1° 51′ 34.84" Tangent = 18.45 Length = 36.90 Radius = 3,080.95 |
| DRIVER: t:\pcsetup | Ahead = N 7° 49′ 19.62" W Chord Bear = N 5° 36′ 04.38" W Course from PT CR10501 to PC CR10 | I | | External = 0.06 Long Chord = 36.90 Mid. Ord. = 0.06 P.C. Station 111+97.78 N 10,504,252.23 E 3,861,750.6 P.T. Station 112+34.68 N 10,504,287.35 E 3,861,761.9 C.C. N 10,505,215.51 E 3,858,824.1 |
| PLOT DRIV | Curve CR10502 P.I. Station 106+27.31 Delta = 20° 52′ 34.88" Degree = 19° 05′ 54.94" | ** N 10,503,704.46 E (RT) | 3,861,589.62 | Back = N 18° 13′ 09.97" E Ahead = N 17° 31′ 59.41" E Chord Bear = N 17° 52′ 34.69" E Course from PT CR10505 to PC CR10506 N 17° 31′ 59.41" E Dist 90.40 |
| | Tangent = 55.27 Length = 109.31 | | | Curve Data |
| | Radius = 300.00 External = 5.05 Long Chord = 108.70 Mid. Ord. = 4.96 P.C. Station 105+72.04 P.T. Station 106+81.35 C.C. Back = Back = N 7° 49' 19.62" W Ahead = N 13° 03' 15.25" E | N 10,503,649.70 E N 10,503,758.30 E N 10,503,690.53 E | 3,861,597.15 3,861,602.11 3,861,894.35 | ** Curve CR10506 P.I. Station 113+35.16 N 10,504,383.16 E 3,861,792.2 Delta = 1° 31′ 11.15" (RT) Degree = 7° 32′ 43.43" Tangent = 10.07 Length = 20.14 Radius = 759.35 External = 0.07 |
| | Chord Bear = N 2° 36' 57.81" E Course from PT CR10502 to PC CR10 | | 00 | Long Chord = 20.14 Mid. Ord. = 0.07 P.C. Station 113+25.09 N 10,504,373.56 E 3,861,789.1 P.T. Station 113+45.23 N 10,504,392.68 E 3,861,795.5 |
| | Curve CR10503 P.I. Station 108+80.24 | Curve Data ** N 10,503,952.04 E | 3,861,647.03 | C.C. Back = N 17° 31′ 59.41″ E Ahead = N 19° 03′ 10.55″ E Chord Bear = N 18° 17′ 34.98″ E |
| | Delta = 7° 04' 12.13" Degree = 15° 27' 51.99" | (RT) | 0,001,01100 | Course from PT CR10506 to PC CR10507 N 19° 03′ 10.55" E Dist 164.25 |
| | Tangent = 22.89 Length = 45.72 | | | Curve Data |
| 0_HADØ3.dgn | Radius = 370.50 External = 0.71 Long Chord = 45.69 Mid. Ord. = 0.70 P.C. Station 108+57.35 P.T. Station 109+03.07 C.C. Back = Back = N | N 10,503,929.75 E N 10,503,973.53 E N 10,503,846.06 E | 3,861,641.86 3,861,654.91 3,862,002.79 | ** Curve CR10507 P.I. Station 115+30.20 N 10,504,567.51 E 3,861,855.8 Delta = 1°59′04.41"(RT) Degree = 4°47′22.24" Tangent = 20.72 Length = 41.44 Radius = 1,196.28 |
| 1:03:42 PM z\dms40625\HOUCC | Ahead = N 20° 07′ 27.39" E Chord Bear = N 16° 35′ 21.32" E Course from PT CR10503 to PC CR10 | | | External = 0.18 Long Chord = 41.43 Mid. Ord. = 0.18 P.C. Station 115+09.48 N 10,504,547.93 E 3,861,849.1 P.T. Station 115+50.91 N 10,504,586.85 E 3,861,863.3 C.C. N 10,504,157.41 E 3,862,979.8 Back = N 19° 03' 10.55" E |
| \bge_pw\kcru | Curve CR10504 P.I. Station 110+16.98 Delta = 1° 54′ 17.42" Degree = 5° 12′ 16.81" Tangent = 18.30 | (LT) | 3,861,694.10 | Ahead = N 21° 02′ 14.97" E Chord Bear = N 20° 02′ 42.76" E Course from PT CR10507 to CR105002 N 21° 02′ 14.97" E Dist 84.69 |
| hdir 'kdir | Length = 36.60 Radius = 1,100.85 | | | Point CR105002 N 10,504,665.89 E 3,861,893.73 Sta 116+35.60 |
| : 3/1/2021 c:\pwworl | , | | | Ending chain CR1050 description |

7.80 9.82 4.16

5.40

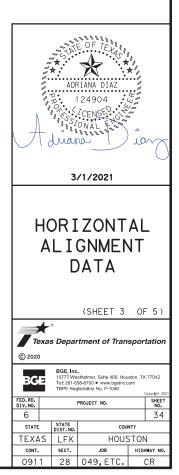
0.63 1.96 4.14

2.22

9.19 5.51 8.26

. 89

9.13 3.33 9.87



| HICKORY CREEK TRIBUTARY BRIDGE (CSJ 0911-28-049) | | |
|---|--|--|
| Beginning chain CR1060 description | Curve Data ** | |
| Point CR106001 N 10,504,600.98 E 3,873,775.42 Sta 99+99.73 Course from CR106001 to PC CR10601 N 62° 18′ 21.55″ E Dist 145.81 Curve Data | Curve CR10604 P.I. Station 107+26.62 N 10,504,870.42 E 3,874,448.67 Delta = 12° 45′ 45.41" (RT) Degree = 19° 05′ 54.94" Tangent = 33.55 Length = 66.82 | |
| ** Curve CR10601 P.I. Station 102+35.85 N 10,504,710.72 E 3,873,984.49 Delta = 7° 15′ 07.24" (RT) Degree = 4° 01′ 14.00" Tangent = 90.31 Length = 1,425.07 External = 2.86 Long Chord = 180.25 Mid. Ord. = 2.85 | Radius = 300.00 External = 1.87 Long Chord = 66.69 Mid. Ord. = 1.86 P.C. Station 106+93.07 N 10,504,859.18 E 3,874,417.06 P.T. Station 107+59.89 N 10,504,874.40 E 3,874,481.99 C.C. N 10,504,576.52 E 3,874,517.59 Back = N 70° 25' 18.15" E Ahead = N 83° 11' 03.56" E Chord Bear = N 76° 48' 10.86" E | |
| P.C. Station 101+45.54 N 10,504,668.75 E 3,873,904.53 P.T. Station 103+25.91 N 10,504,742.26 E 3,874,069.11 C.C. N 10,503,406.93 E 3,874,566.83 Back = N 62° 18' 21.55" E | Course from PT CR10604 to PC CR10605 N 83° 11′ 03.56" E Dist 147.00 Curve Data ** | |
| Ahead = N 69° 33′ 28.79" E Chord Bear = N 65° 55′ 55.17" E Course from PT CR10601 to PC CR10602 N 69° 33′ 28.79" E Dist 45.08 | Curve CR10605 P.I. Station 109+51.03 N 10,504,897.08 E 3,874,671.78 Delta = 16°44′23.36″(RT) Degree = 19°05′54.94″ Tangent = 44.14 | |
| Curve Data ** Curve CR10602 P.I. Station 104+19.50 N 10,504,774.94 E 3,874,156.80 Delta = 3° 14′ 56.09" (RT) Degree = 3° 20′ 58.52" Tangent = 48.51 Length = 96.99 Radius = 1,710.53 | Length = 87.65 Radius = 300.00 External = 3.23 Long Chord = 87.34 Mid. Ord. = 3.20 P.C. Station 109+06.89 N 10,504,891.85 E 3,874,627.95 P.T. Station 109+94.54 N 10,504,889.48 E 3,874,715.26 C.C. N 10,504,593.97 E 3,874,663.55 Back = N 83° 11′ 03.56″ E | |
| External = 0.69 Long Chord = 96.98 Mid. Ord. = 0.69 P.C. Station 103+70.99 N 10,504,758.00 E 3,874,111.35 P.T. Station 104+67.98 N 10,504,789.28 E 3,874,203.14 | Ahead = S 80° 04′ 33.08" E Chord Bear = S 88° 26′ 44.76" E Course from PT CR10605 to PC CR10606 S 80° 04′ 33.08" E Dist 139.61 | |
| C.C. N 10,503,155.19 E 3,874,708.76 Back = N 69° 33′ 28.79" E Ahead = N 72° 48′ 24.88" E Chord Bear = N 71° 10′ 56.84" E | Curve Data ** Curve CR10606 P.I. Station 111+53.38 N 10,504,862.10 E 3,874,871.72 Delta = 0° 50′ 31.68" (RT) | NE OF TELL |
| Course from PT CR10602 to PC CR10603 N 72° 48′ 24.88" E Dist 121.29 Curve Data ** Curve CR10603 | Degree = 2° 11′ 24.03" Tangent = 19.23 Length = 38.45 Radius = 2,616.24 External = 0.07 | |
| P.I. Station 106+08.07 N 10,504,830.69 E 3,874,336.97 Delta = 2° 23′ 06.73" (LT) 2° 23′ 06.73" (LT) Degree = 6° 20′ 46.36" 37.58 Tangent = 18.80 Length = 37.58 Radius = 0.20 Long Chord = 37.58 | Long Chord = 38.45 Mid. Ord. = 0.07 P.C. Station 111+34.16 N 10,504,865.42 E 3,874,852.78 P.T. Station 111+72.61 N 10,504,858.51 E 3,874,890.61 C.C. N 10,502,288.33 E 3,874,401.89 Back = S 80° 04' 33.08" E Ahead = S 79° 14' 01.40" E Chord Bear = S 79° 39' 17.24" E | ADRIANA DIAZ 124904 CENSCO AL CONAL CANANA 3/1/2021 |
| Mid. Ord. = 0.20 P.C. Station 105+89.27 N 10,504,825.13 E 3,874,319.01 P.T. Station 106+26.86 N 10,504,836.99 E 3,874,354.68 C.C. N 10,505,687.62 E 3,874,052.14 Back = N 72° 48' 24.88" | Course from PT CR10606 to PC CR10607 S 79° 14′ 01.40" E Dist 60.11 Curve Data ** | HOR I ZONTAL AL I GNMENT |
| Ahead = N 70° 25′ 18.15" E Chord Bear = N 71° 36′ 51.52" E Course from PT CR10603 to PC CR10604 N 70° 25′ 18.15" E Dist 66.21 | Curve CR10607 P.I. Station 113+26.09 N 10,504,829.84 E 3,875,041.39 Delta = 2° 31′ 09.97" (LT) Degree = 1° 20′ 57.55" Tangent = 93.37 Length = 186.72 | (SHEET 4 OF 5) |
| | Radľus = 4,246.28 External = 1.03 Long Chord = 186.70 Mid. Ord. = 1.03 P.C. Station 112+32.72 N 10,504,847.28 E 3,874,949.65 P.T. Station 112+32.72 N 10,504,816.45 E 3,875,133.79 P.T. Station 114+19.44 N 10,504,816.45 E 3,875,742.87 C.C. N 10,509,018.81 E 3,875,742.87 Back = S 81° 45' 11.36" Chord Bear = S 80° 29' 36.38" E | BGE, Inc. Orrange 10777 Westheetner, Suite 400, Houston, TX 77042 TEX.85 200 # FED, R0. FED, R0. FED, R0. PROJECT NO. STATE DIST, NO. COUNTY TEXAS LFK HOUSTON CONT SECT JOB HIGHNAY NO. O911 28<049, ETC. |

-ds\Tables\Pen\TXD0T.TBL ts\St ale.plt Ø3, NPLOTDRVNT×DOT. PEN TABLE: pw:\\bge-PLOT DRIVER: t:\pcsetu

MODEL NAME: HORIZONTAL ALIGNMENT DATA DATE: 3/1/2021 FILE: c:/.uwworkdari/hos ow/krcuity/ms402251

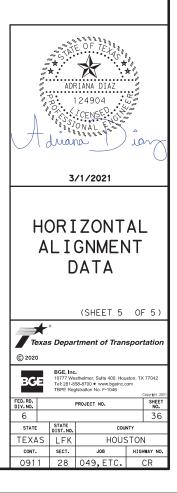
HICKORY CREEK TRIBUTARY BRIDGE (CSJ 0911-28-049), CONTINUED

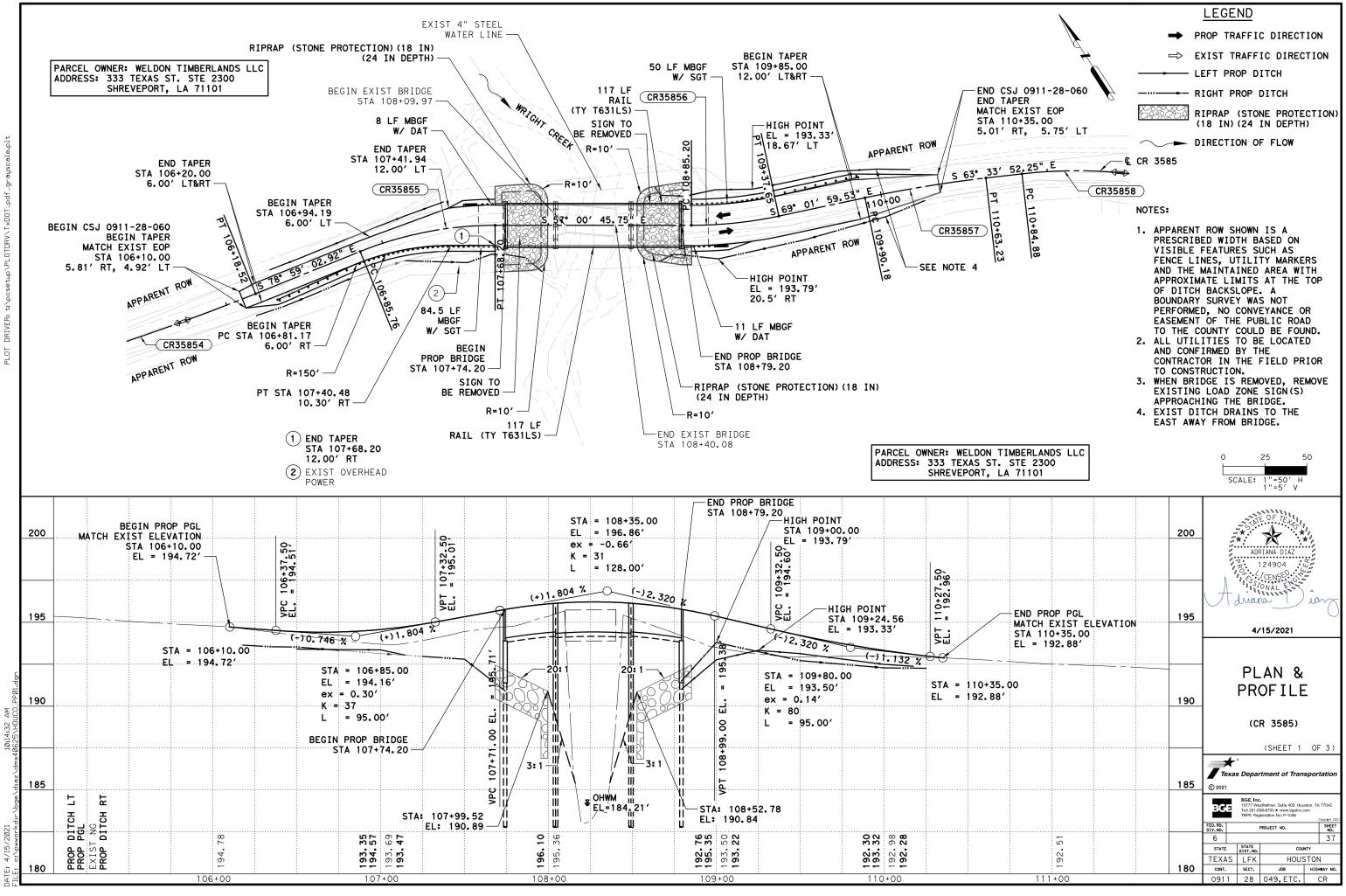
| | | Curve * | Data * | | |
|-----------------------------|----------------------------|------------|------------------|---------|--------------|
| Curve CR10608 | 114.00.07 | | 10 504 005 00 | - | 7 075 010 00 |
| P.I. Station Delta = | 114+99.07 4° 37′ 34.08" | | 10,504,805.02 | E | 3,875,212.60 |
| Degree = Tanaent = | 2° 54′ 22.75" 79.63 | | | | |
| Tangent = Length = | 159.18 | | | | |
| Radius = External = | 1,971.42 | | | | |
| Long Chord = | 159.13 | | | | |
| Mid. Ord. = P.C. Station | 1.61 114+19.44 | N | 10,504,816.45 | F | 3,875,133.79 |
| P.T. Station | 115+78.61 | N | 10,504,799.99 | E | 3,875,292.07 |
| C.C. Back = S | 81° 45′ 11.36" E | N | 10,506,767.48 | E | 3,875,416.57 |
| Ahead = S | 86° 22′ 45.44" E | | | | |
| Chord Bear = S | 84° 03′ 58.40" E | | | | |
| Course from PT C | R10608 to CR106002 | 2 S 86° | 22' 45.44" E Dis | † 53.91 | |
| Point CR106002 | N 10,504, | 796.59 | E 3, 875, 345. | 87 Sta | 116+32.52 |
| | | | | | |

Ending chain CR1060 description

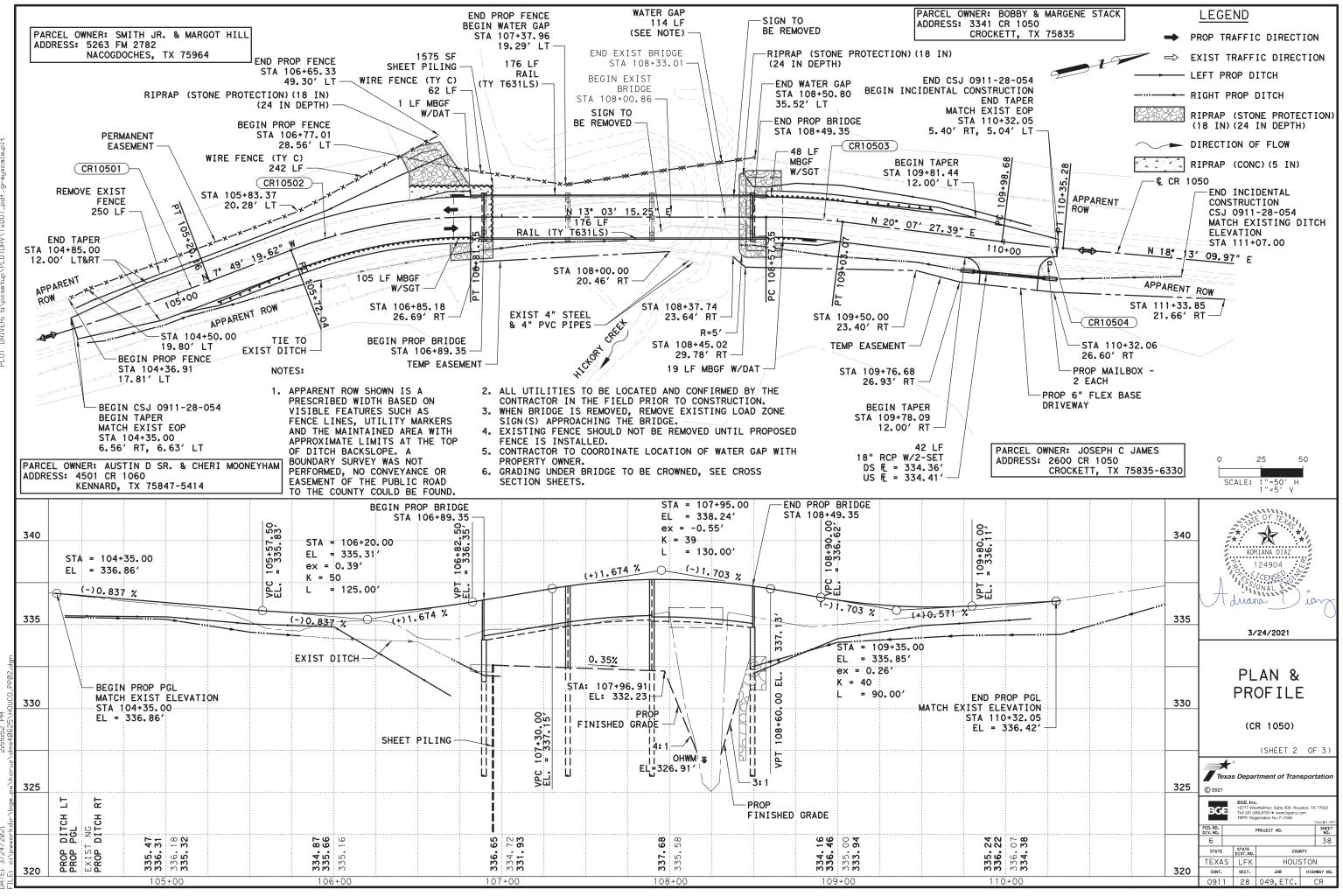
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ale.plt



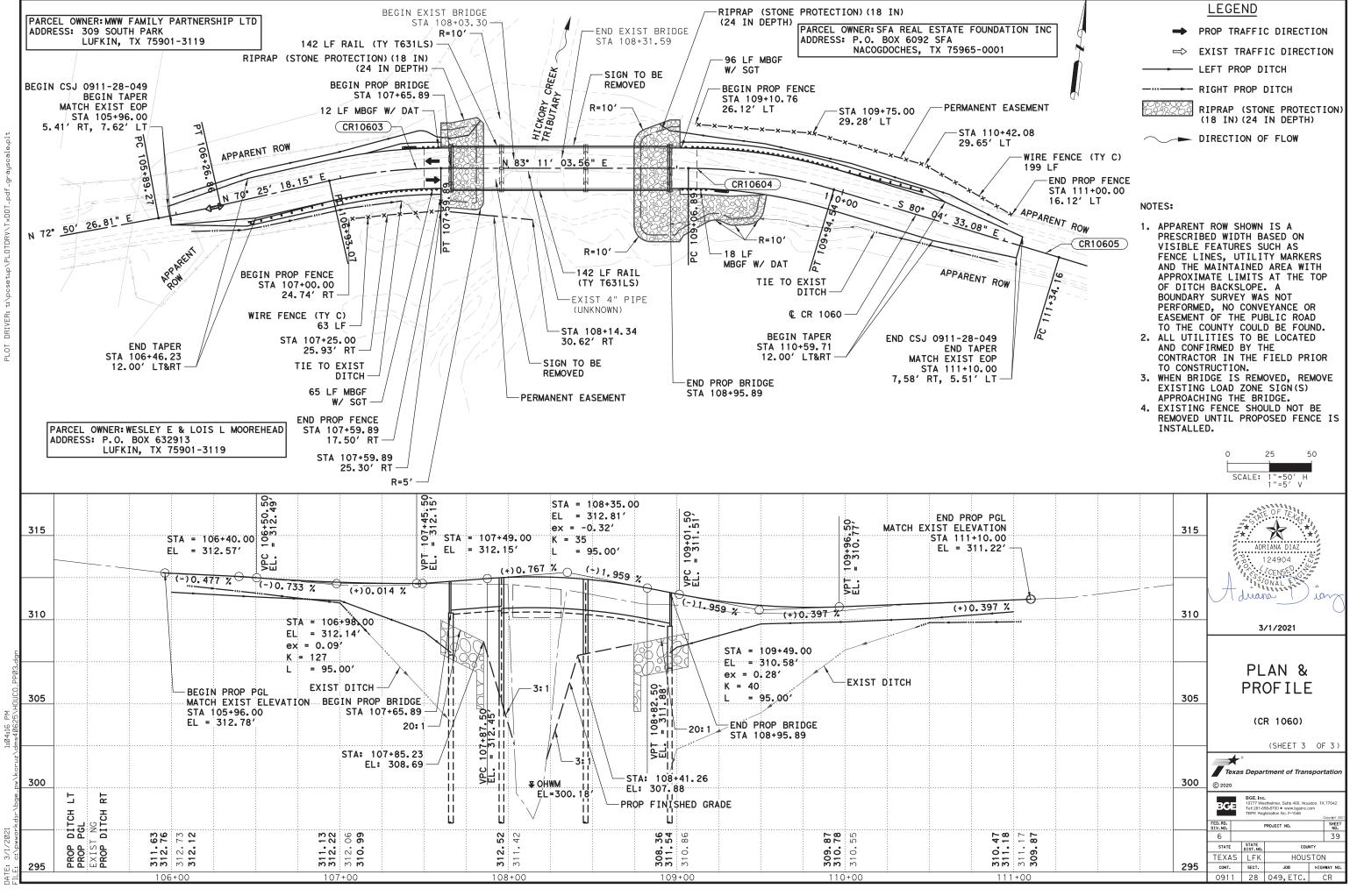


DEL NAME: PLAN () PROFILE TE: 4/15/2021



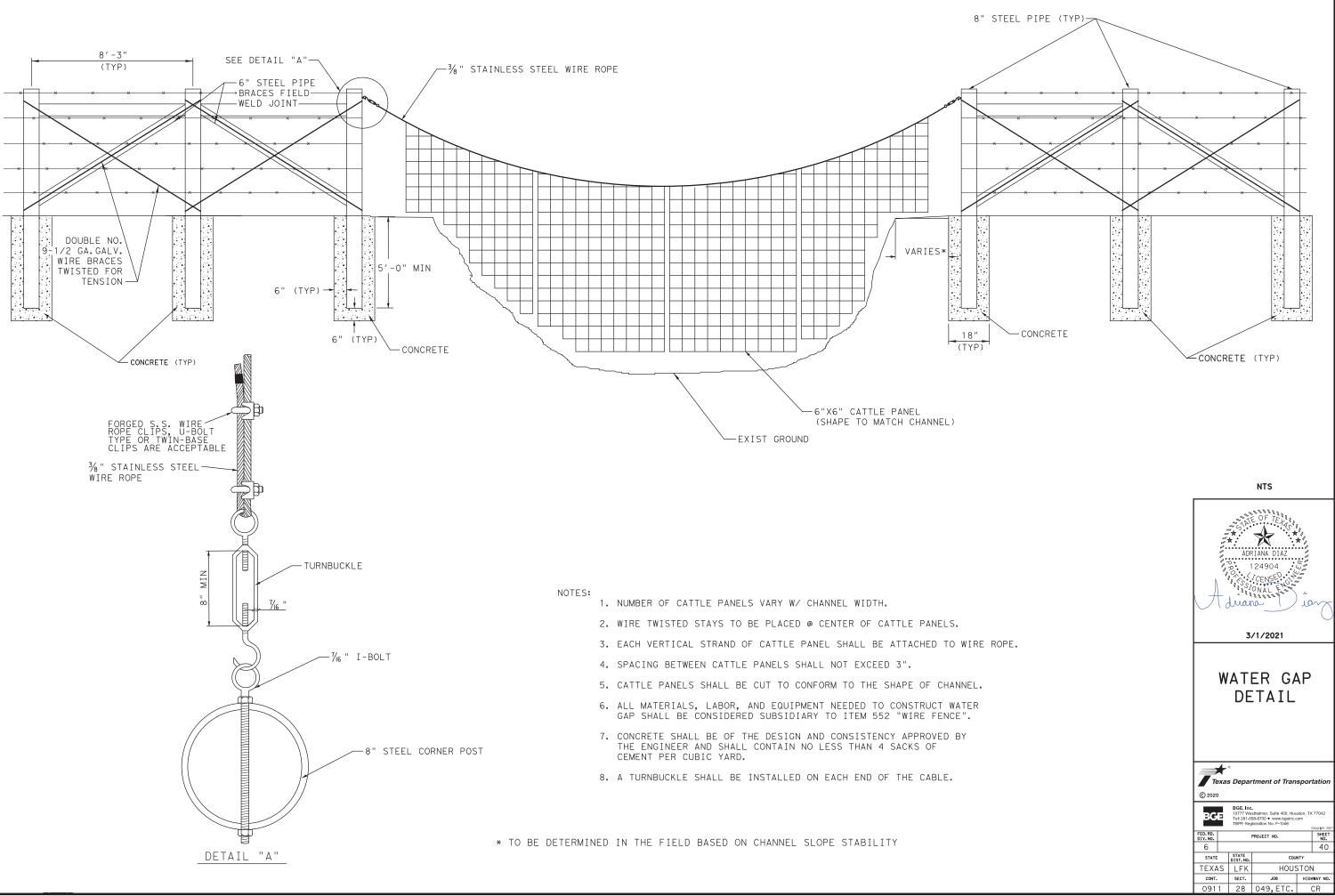
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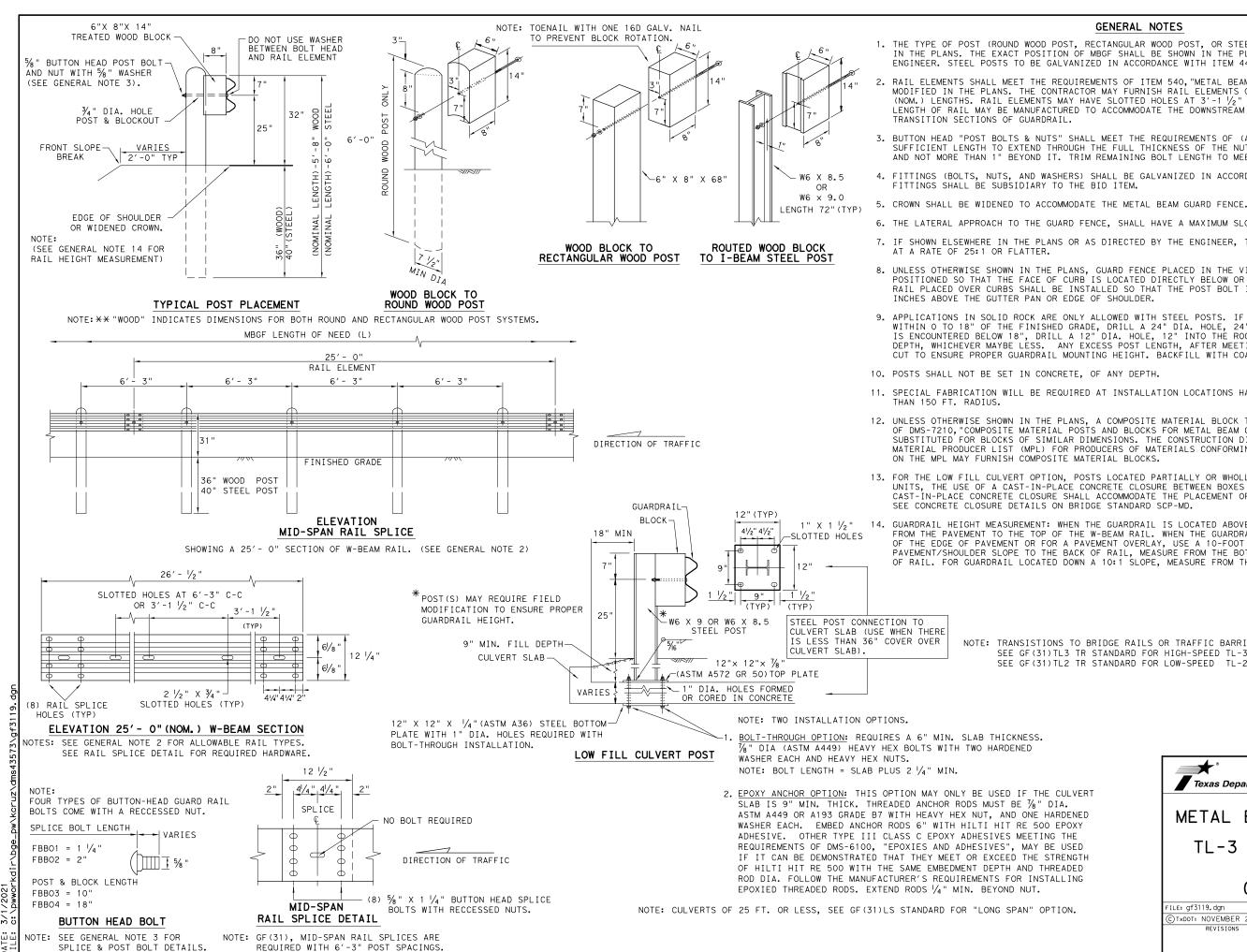
>)EL NAME: PLAN () PROFILE 16: 3/24/2021 2



DRIV







DISCLAIMER: THE USE OF TXDOT ASSUM

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GENERAL NOTES

1. THE TYPE OF POST (ROUND WOOD POST, RECTANGULAR WOOD POST, OR STEEL POST) WILL BE AS SHOWN IN THE PLANS. THE EXACT POSITION OF MBGF SHALL BE SHOWN IN THE PLANS OR AS DIRECTED BY THE ENGINEER. STEEL POSTS TO BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING.

RAIL ELEMENTS SHALL MEET THE REQUIREMENTS OF ITEM 540, "METAL BEAM GUARD FENCE" EXCEPT AS MODIFIED IN THE PLANS. THE CONTRACTOR MAY FURNISH RAIL ELEMENTS OF 25'- 0", OR 12'- 6" (NOM.) LENGTHS. RAIL ELEMENTS MAY HAVE SLOTTED HOLES AT $3'-1 \frac{1}{2}$ " C-C OR 6'-3" C-C. A SPECIAL LENGTH OF RAIL MAY BE MANUFACTURED TO ACCOMMODATE THE DOWNSTREAM ANCHOR TERMINAL (DAT) AND THE

3. BUTTON HEAD "POST BOLTS & NUTS" SHALL MEET THE REQUIREMENTS OF (ASTM A307), AND SHALL BE OF SUFFICIENT LENGTH TO EXTEND THROUGH THE FULL THICKNESS OF THE NUT AND 5/8" WASHER (FWC16g) AND NOT MORE THAN 1" BEYOND IT. TRIM REMAINING BOLT LENGTH TO MEET REQUIRED LENGTH.

4. FITTINGS (BOLTS, NUTS, AND WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING. FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM.

6. THE LATERAL APPROACH TO THE GUARD FENCE, SHALL HAVE A MAXIMUM SLOPE OF 1V:10H.

7. IF SHOWN ELSEWHERE IN THE PLANS OR AS DIRECTED BY THE ENGINEER, THE GUARD FENCE MAY BE FLARED

8. UNLESS OTHERWISE SHOWN IN THE PLANS. GUARD FENCE PLACED IN THE VICINITY OF CURBS SHALL BE POSITIONED SO THAT THE FACE OF CURB IS LOCATED DIRECTLY BELOW OR BEHIND THE FACE OF THE RAIL. RAIL PLACED OVER CURBS SHALL BE INSTALLED SO THAT THE POST BOLT IS LOCATED APPROXIMATELY 25

9. APPLICATIONS IN SOLID ROCK ARE ONLY ALLOWED WITH STEEL POSTS. IF SOLID ROCK IS ENCOUNTERED WITHIN O TO 18" OF THE FINISHED GRADE, DRILL A 24" DIA. HOLE, 24" INTO THE ROCK. IF SOLID ROCK IS ENCOUNTERED BELOW 18", DRILL A 12" DIA. HOLE, 12" INTO THE ROCK OR TO THE STANDARD EMBEDMENT DEPTH, WHICHEVER MAYBE LESS. ANY EXCESS POST LENGTH, AFTER MEETING THESE DEPTHS, MAY BE FIELD CUT TO ENSURE PROPER GUARDRAIL MOUNTING HEIGHT. BACKFILL WITH COARSE AGGREGATE MATERIAL.

11. SPECIAL FABRICATION WILL BE REQUIRED AT INSTALLATION LOCATIONS HAVING A CURVATURE OF LESS

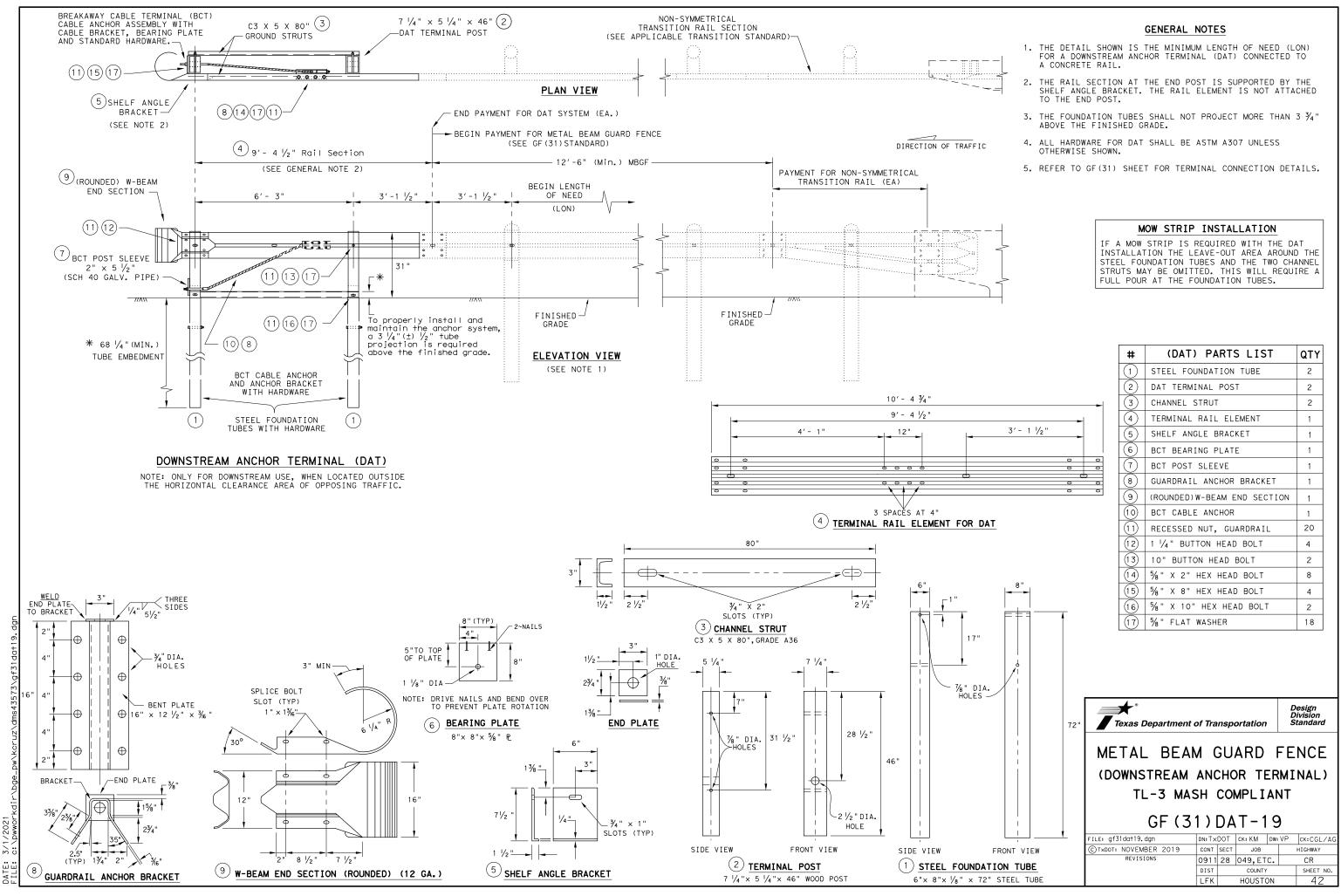
12. UNLESS OTHERWISE SHOWN IN THE PLANS, A COMPOSITE MATERIAL BLOCK THAT MEETS THE REQUIREMENTS OF DMS-7210, "COMPOSITE MATERIAL POSTS AND BLOCKS FOR METAL BEAM GUARD FENCE" MAY BE SUBSTITUTED FOR BLOCKS OF SIMILAR DIMENSIONS. THE CONSTRUCTION DIVISION, TXDOT MAINTAINS A MATERIAL PRODUCER LIST (MPL) FOR PRODUCERS OF MATERIALS CONFORMING TO DMS-7210 ONLY PRODUCERS

13. FOR THE LOW FILL CULVERT OPTION, POSTS LOCATED PARTIALLY OR WHOLLY BETWEEN PRECAST BOX CULVERT UNITS, THE USE OF A CAST-IN-PLACE CONCRETE CLOSURE BETWEEN BOXES IS REQUIRED. THE LENGTH OF THE CAST-IN-PLACE CONCRETE CLOSURE SHALL ACCOMMODATE THE PLACEMENT OF THE LOW FILL CULVERT OPTION.

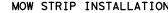
1" X 1 ¹/2" 14. GUARDRAIL HEIGHT MEASUREMENT: WHEN THE GUARDRAIL IS LOCATED ABOVE PAVEMENT, MEASURE THE HEIGHT OF THE EDGE OF PAVEMENT OR FOR A PAVEMENT OVERLAY, USE A 10-FOOT STRAIGHTEDGE TO EXTEND THE PAVEMENT/SHOULDER SLOPE TO THE BACK OF RAIL, MEASURE FROM THE BOTTOM OF STRAIGHTEDGE TO THE TOP OF RAIL. FOR GUARDRAIL LOCATED DOWN A 10:1 SLOPE, MEASURE FROM THE NOMINAL TERRAIN.

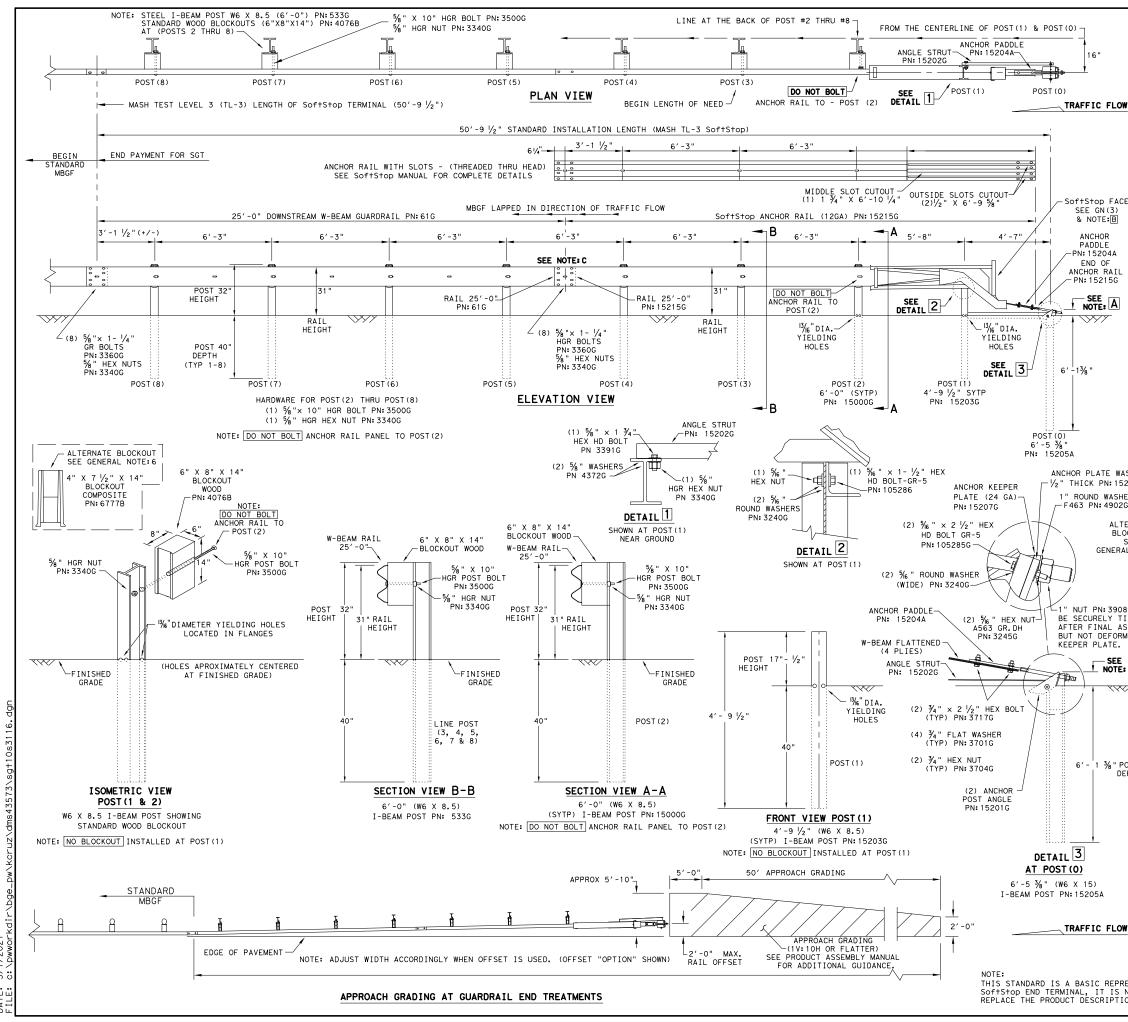
> NOTE: TRANSISTIONS TO BRIDGE RAILS OR TRAFFIC BARRIERS. SEE GF (31) TL3 TR STANDARD FOR HIGH-SPEED TL-3 TRANSITIONS. SEE GF (31) TL2 TR STANDARD FOR LOW-SPEED TL-2 TRANSITIONS.





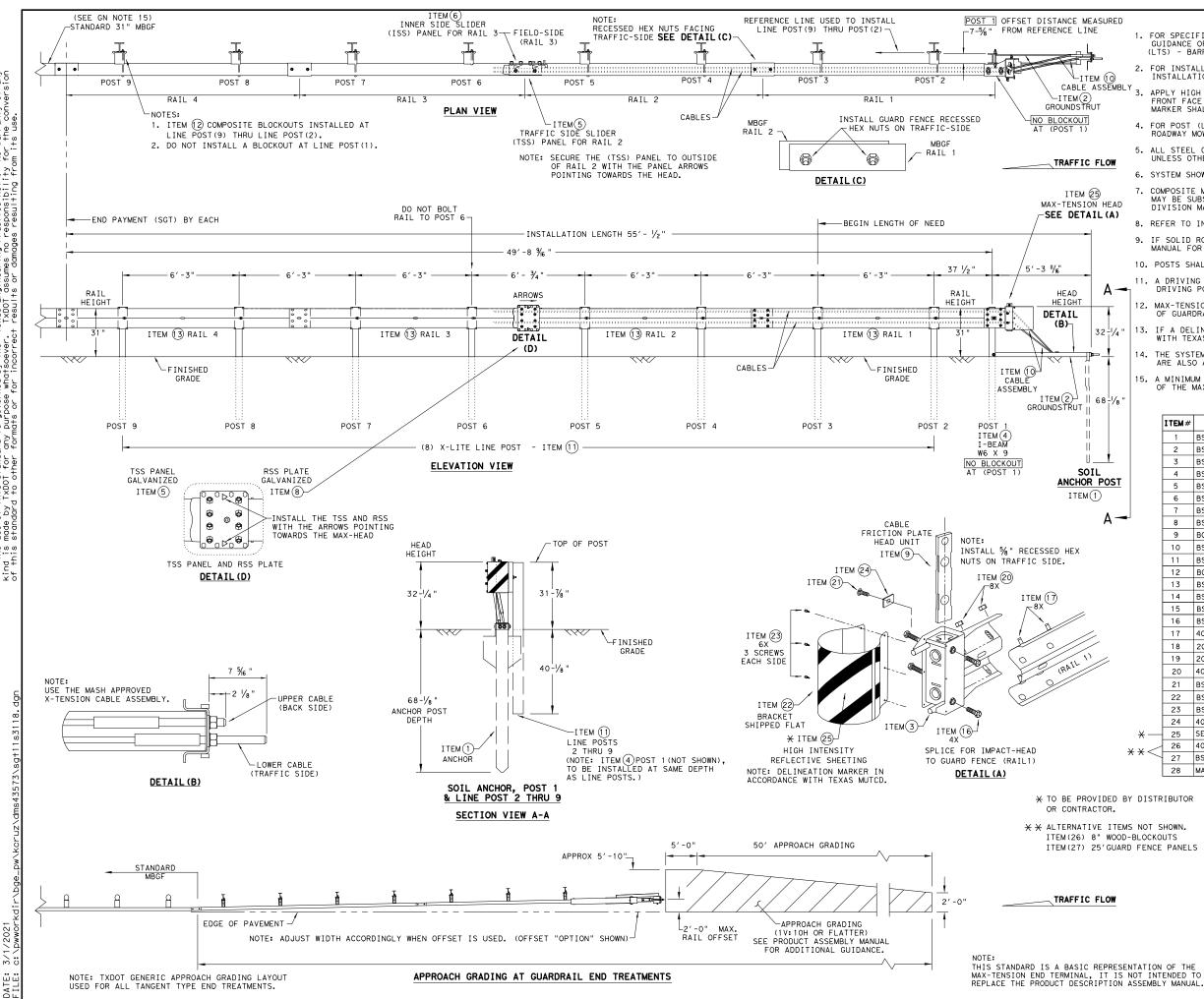
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| | | | GENERAL NOTES | | | | | |
|----------------------------|---|-------------------------------|--|--------------------------------|--|--|--|--|
| (| OF THE SY | STEM, C | DRMATION REGARDING INSTALLATION AND TECHNIC DNTACT: TRINITY HIGHWAY AT 1(888)323-6374. FREEWAY, DALLAS, TX 75207 | AL GUIDANCE | | | | |
| 2. [| FOR INSTA SoftStop | LLATION END TER | , REPAIR AND MAINTENANCE REFER TO THE; MINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL. | PN: 620237B | | | | |
| f | FRONT FAC | E OF TH | SITY REFLECTIVE SHEETING, "OBJECT MARKER" O E DEVICE PER MANUFACTURER'S RECOMMENDATIONS | | | | | |
| .0W 4. F | FOR POST | (LEAVE- | ALL CONFORM TO THE STANDARDS REQUIRED IN TE DUT) INSTALLATION AND GUIDANCE SEE TXDOT'S > STANDARD. | | | | | |
| | | | NUTS, & WASHERS) SHALL BE GALVANIZED IN AC IZING". FITTINGS SHALL BE SUBSIDIARY TO THE | CORDANCE WITH BID ITEM. | | | | |
| Ν | A COMPOSITE MATERIAL BLOCKOUT THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS, SEE CONSTRUCTION | | | | | | | |
| | DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS. IF SOLID ROCK IS ENCOUNTERED SEE THE MANUFACTURER'S INSTALLATION MANUAL AND REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE. | | | | | | | |
| 102 | | | BE SET IN CONCRETE. | | | | | |
| 9. 3 | IT IS ACC | EPTABLE | TO INSTALL THE Sof+Stop IMPACT HEAD PARALL TH AN UPWARD TILT. | EL TO THE | | | | |
| | | | E SoftStop SYSTEM DIRECTLY TO A RIGID BARRI | ER. | | | | |
| n 11. l | | CIRCUMS | TANCES SHALL THE GUARDRAIL WITHIN THE SOF+S | | | | | |
| 12. | A FLARE R FROM ENCR ELIMINATE | ATE OF OACHING D FOR SI | JP TO 25:1 MAY BE USED TO PREVENT THE TERMI ON THE SHOULDER. THE FLARE MAY BE DECREASE PECIFIC INSTALLATIONS, IF DIRECTED BY THE E | NAL HEAD D OR NGINEER. | | | | |
| | | | TALLATION HEIGHT OF FULLY ASSEMBLED ANCHOR DM 3- $\frac{7}{4}$ " MIN. TO 4" MAX. ABOVE FINISHED GRAN | | | | | |
| | NOTE: B | PART PN | 5852B RIGHT-SIDE (HIGH INTENSITY REFLECTIV | E SHEETING) | | | | |
| | | | 5851B LEFT-SIDE (HIGH INTENSITY REFLECTIV SPLICE LOCATED BETWEEN LINE POST(4)AND LINE | | | | | |
| | | GUARDRA | IL PANEL 25'-0" PN:61G RAIL 25'-0" PN:15215G | | | | | |
| | | | RDRAIL IN DIRECTION OF TRAFFIC FLOW. | | | | | |
| | PART | QTY | MAIN SYSTEM COMPONENTS | | | | | |
| | 620237B | 1 | PRODUCT DESCRIPTION ASSEMBLY MANUAL (LATE | ST REV.) | | | | |
| | 15208A 15215G | 1 | SoftStop HEAD (SEE MANUAL FOR RIGHT-LEFT SoftStop ANCHOR RAIL (12GA) WITH CUTOUT | | | | | |
| WASHER | 616 | 1 | SoftStop DOWNSTREAM W-BEAM RAIL (12GA) (| | | | | |
| 15206G | 15205A | 1 | POST #0 - ANCHOR POST (6'- 5 1/8") | | | | | |
| SHER D2G | 15203G 15000G | 1 | POST #1 - (SYTP) (4' - 9 1/2") POST #2 - (SYTP) (6' - 0") | | | | | |
| | 533G | 6 | POST #3 THRU #8 - I-BEAM (W6 × 8.5) (6'- | 0") | | | | |
| LTERNATE | 4076B | 7 | BLOCKOUT - WOOD (ROUTED) (6" x 8" x 14") | | | | | |
| SEE RAL NOTE:6 | 6777B 15204A | 7 | BLOCKOUT - COMPOSITE $(4" \times 7 \frac{1}{2}" \times 14")$ ANCHOR PADDLE | | | | | |
| INAL NOTED | 15207G | 1 | ANCHOR KEEPER PLATE (24 GA) | | | | | |
| | 152066 | 1 | ANCHOR PLATE WASHER (1/2" THICK) | | | | | |
| | 15201G 15202G | 1 | ANCHOR POST ANGLE (10" LONG) ANGLE STRUT | | | | | |
| 08G SHALL | | | HARDWARE | | | | | |
| TIGHTENED ASSEMBLY, | 4902G | 1 | 1" ROUND WASHER F436 | | | | | |
| ORMING THE | 3908G | 1 | 1" HEAVY HEX NUT A563 GR.DH | | | | | |
| | 3717G 3701G | 2 | ¾" × 2 ½" HEX BOLT A325 ¾" ROUND WASHER F436 | | | | | |
| E A | 3701G 3704G | 2 | $\frac{74}{4}$ HEAVY HEX NUT A563 GR.DH | | | | | |
| | 3360G | 16 | 5/8" × 1 1/4" W-BEAM RAIL SPLICE BOLTS HGR | | | | | |
| ~~~ | 3340G 3500G | 25 7 | % "W-BEAM RAIL SPLICE NUTS HGR % " × 10" HGR POST BOLT A307 | | | | | |
| | 3391G | 1 | 5/8 × 1 3/4 " HEX HD BOLT A325 | | | | | |
| | 4489G | 1 | % × 9" HEX HD BOLT A325 | | | | | |
| | 4372G 105285G | 4 | 5% " WASHER F436 5% " × 2 ½ " HEX HD BOLT GR-5 | | | | | |
| B 00- | 1052850 105286G | 1 | $\frac{716}{5}$ x 2 /2 HEX HD BOLT GR-5 | | | | | |
| POST DEPTH | 3240G | 6 | % "ROUND WASHER (WIDE) | | | | | |
| | 3245G 5852B | 3 | % " HEX NUT A563 GR.DH HIGH INTENSITY REFLECTIVE SHEETING - SEE | NOTE: B | | | | |
| | | Г | | | | | | |
| | | | Texas Department of Transportation | Design Division Standard | | | | |
| | | | TRINITY HIGHWA | Y | | | | |
| | | | SOFTSTOP END TERM | INAL | | | | |
| | | | MASH - TL-3 | | | | | |
| . <u>OW</u> | | | SGT (10S) 31-16 | | | | | |
| | | FI | LE: Sg†10S3116 DN:TxDOT CK:KM DW: | | | | | |
| | | | TXDOT: JULY 2016 CONT SECT JOB | HIGHWAY | | | | |
| PRESENTATIO S NOT INTEN | NDED TO | | REVISIONS 0911 28 049, ETC. | CR | | | | |
| TION ASSEME | BLY MANUA | ·· | LFK HOUSTON | SHEET NO. | | | | |
| | | | | | | | | |



DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any kind is made by TXDOT for any purpose whatsoever. TXDOT assumes no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use.

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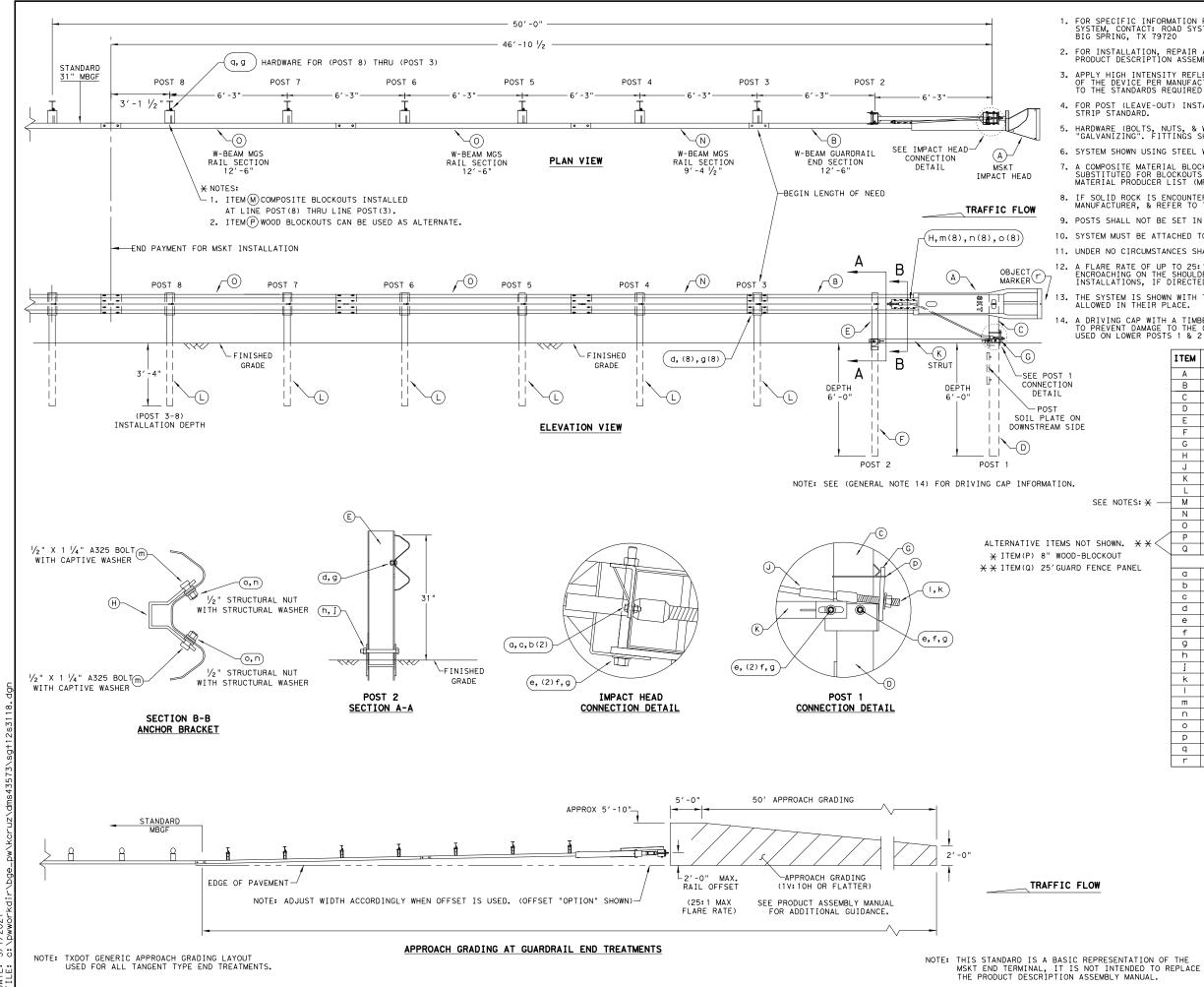
| URED | | | | GENERAL NOTES | | | | | |
|--|--|---|------------------------------------|---|---|--|--|--|--|
| | (| GUIDANCE | OF THE SYSTEM, | CONTACT: LINDSAY TRANSPORTATION S(CONTACT: LINDSAY TRANSPORTATION S(INC. AT (707) 374-6800 | CAL DLUTIONS | | | | |
| | | OR INSTA | ALLATION, REPAI TION INSTRUCTIO | R, & MAINTENANCE REFER TO THE; MAX- N MANUAL. P/N MANMAX REV D (ECN 35 | TENSION 6). | | | | |
| SEMBLY | ¹ 3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURE'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD. | | | | | | | | |
| | | | (LEAVE-OUT) IN MOW STRIP STAND | STALLATION AND GUIDANCE SEE TXDOT'S | LATEST | | | | |
| LOW | | | _ COMPONENTS AR THERWISE STATED | E GALVANIZED PER ASTM A123 OR EQUIN | ALENT | | | | |
| | 6. 5 | SYSTEM SH | HOWN USING STEE | L WIDE FLANGE POST WITH COMPOSITE E | BLOCKOUTS. | | | | |
| HEAD . (A) | N | MAY BE S | UBSTITUTED FOR | KOUT THAT MEETS THE REQUIREMENTS OF BLOCKOUTS SIMILAR DIMENSIONS, SEE (ICER LIST(MPL)FOR CERTIFIED PRODUCE | CONSTRUCTION | | | | |
| | | | | ANUAL FOR SPECIFIC PANEL LAPPING GU | | | | | |
| | | | OR INSTALLATION | TERED SEE THE MANUFACTURER'S INSTAL I GUIDANCE. | LATION | | | | |
| | | | HALL NOT BE SET | | | | | | |
| A – | | DRIVING | POST TO PREVEN | IMBER OR PLASTIC INSERT SHALL BE US IT DAMAGE TO THE GALVANIZING ON TOP | OF THE POST. | | | | |
| Ŧ | 12. | MAX-TENS OF GUAR | | LL NEVER BE INSTALLED WITHIN A CURV | ED SECTION | | | | |
| 2-1/4 " | 13. | IF A DEL WITH TE | INEATION MARKE | R IS REQUIRED, MARKER SHALL BE IN A | CCORDANCE | | | | |
| + | 14. | | TEM IS SHOWN WI O ALLOWED. | TH 12'-6" MBGF PANELS, 25'-0" MBGF | PANELS | | | | |
| | 15. | | JM OF 12'-6" OF MAX-TENSION SYS | 12GA. MBGF IS REQUIRED IMMEDIATELY | DOWNSTREAM | | | | |
| 8 1/8 " | | OF THE | MAX-TENSION 313 |) LW. | | | | | |
| | | ITEM# | PART NUMBER | DESCRIPTION | QTY | | | | |
| | | 1 | BSI-1610060-00 | SOIL ANCHOR - GALVANIZED | 1 | | | | |
| | | 2 | BSI-1610061-00 | GROUND STRUT - GALVANIZED | 1 | | | | |
| - | | 3 | BSI-1610062-00 | MAX-TENSION IMPACT HEAD | 1 | | | | |
| POST | | 4 | BSI-1610063-00 | W6x9 I-BEAM POST 6FTGALVANIZED | 1 | | | | |
| | | 6 | BSI-1610064-00 BSI-1610065-00 | TSS PANEL - TRAFFIC SIDE SLIDER ISS PANEL - INNER SIDE SLIDER | 1 | | | | |
| | | 7 | BSI-1610065-00 | TOOTH - GEOMET | 1 | | | | |
| Α- | | 8 | BSI-1610067-00 | RSS PLATE - REAR SIDE SLIDER | 1 | | | | |
| | | 9 | B061058 | CABLE FRICTION PLATE - HEAD UNIT | 1 | | | | |
| | | 10 | BSI-1610069-00 | CABLE ASSEMBLY - MASH X-TENSION | 2 | | | | |
| | | 11 | BSI-1012078-00 | X-LITE LINE POST-GALVANIZED | 8 | | | | |
| | | 12 | B090534 | 8" W-BEAM COMPOSITE-BLOCKOUT XT110 | 8 | | | | |
| | | 13 | BSI-4004386 | 12'-6" W-BEAM GUARD FENCE PANELS 12 | 2GA. 4 | | | | |
| | | 14 | BSI-1102027-00 | X-LITE SQUARE WASHER | 1 | | | | |
| | | 15 | BSI-2001886 | 5% X 7" THREAD BOLT HH (GR.5) GEOME | | | | | |
| | | 16 | BSI-2001885 | 34" X 3" ALL-THREAD BOLT HH (GR.5) | | | | | |
| | | 17 | 4001115 2001840 | $\frac{5}{8}$ " X 1 $\frac{1}{4}$ " GUARD FENCE BOLTS (GR.2 $\frac{5}{8}$ " X 10" GUARD FENCE BOLTS MGAL | MGAL 48 | | | | |
| /. | | 19 | 2001636 | 5% WASHER F436 STRUCTURAL MGAL | 2 | | | | |
| | | 20 | 4001116 | % " RECESSED GUARD FENCE NUT (GR.2) | | | | | |
| | | 21 | BSI-2001888 | 5% " X 2" ALL THREAD BOLT (GR.5)GEON | | | | | |
| | | 22 | BSI-1701063-00 | DELINEATION MOUNTING (BRACKET) | 1 | | | | |
| | | 23 | BSI-2001887 | 1⁄4" X ⅔4" SCREW SD HH 410SS | 7 | | | | |
| | | 24 | 4002051 | GUARDRAIL WASHER RECT AASHTO FWR03 | 1 | | | | |
| | X – | 25 | SEE NOTE BELOW | HIGH INTENSITY REFLECTIVE SHEETING 8" W-BEAM TIMBER-BLOCKOUT, PDB01B | 8 | | | | |
| ÷ | 1 | 20 | | 25' W-BEAM GUARDRAIL PANEL, 8-SPACE, | | | | | |
| | * * < | 27 | BSI-4004431 | | 12GA. 2 | | | | |
| | * * < | 27 28 | BSI-4004431 MANMAX Rev-(D) | MAX-TENSION INSTALLATION INSTRUCTIO | | | | | |
| | * * < | 27 | | | | | | | |
| OR. ITEMS WOOD- | Ó DIS S NOT BLOCI | TRIBUTOR SHOWN. KOUTS | MANMAX Rev- (D) | | | | | | |
| OR. ITEMS WOOD- | Ó DIS S NOT BLOCI | TRIBUTOR | MANMAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION | DNS 1 Design Division Standard | | | | |
| OR. ITEMS WOOD- | Ó DIS S NOT BLOCI | TRIBUTOR SHOWN. KOUTS | MANMAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION | DNS 1 Design Division Standard | | | | |
| OR. ITEMS WOOD- 'GUARD | Ó DIS S NOT BLOCI | TRIBUTOR SHOWN. KOUTS | MANMAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION ************************************ | DNS 1 Design Division Standard | | | | |
| OR. ITEMS WOOD- 'GUARD | Ó DIS S NOT BLOCI | TRIBUTOR SHOWN. KOUTS | MANMAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION ************************************ | DNS 1 Design Division Standard | | | | |
| OR. ITEMS WOOD- 'GUARD | Ó DIS S NOT BLOCI | TRIBUTOR SHOWN. KOUTS | S MAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION xas Department of Transportation (-TENSION END TER MASH - TL-3 SGT (11S) 31-18 | DNS 1 Design Division Standard | | | | |
| OR. ITEMS WOOD- 'GUARD | ′ DIS 5 NOT BLOCI 0 FEN | TRIBUTOR SHOWN. KOUTS CE PANEL | S MAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION ★* xas Department of Transportation X-TENSION END TER MASH - TL-3 SGT (11S) 31-18 11s3118. dgn DN: TXDOT CK: KM DW: FEBRUARY 2018 CONT SECT JOB | Design Division Standard | | | | |
| OR. ITEMS WOOD- 'GUARD LOW | Ý DIS 3 NOT BLOCI 9 FEN 9 FEN 10 TI | TRIBUTOR SHOWN. KOUTS | S MAX Rev- (D) | MAX-TENSION INSTALLATION INSTRUCTION ************************************ | Design Division Standard MINAL | | | | |

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ATSOE WHA I TS FOR ANY PURPOSE RESULTING FROM MADE BY TXDOT TS OR DAMAGES OF ANY KIND IS INCORRECT RESUL . NO WARRANTY FORMATS OR FOR THE "TEXAS ENGINEERING PRACTICE ACT" CONVERSIONOF THIS STANDARD TO OTHER GOVERNED BY _ITY FOR THE DISCLAIMER: THE USE OF THIS STANDARD IS TXDOT ASSUMES NO RESPONSIBIL

Ř DATE:

GENERAL NOTES

1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: ROAD SYSTEMS, INC. (432)263-2435. 3616 OLD HOWARD COUNTY AIRPORT, BIG SPRING, TX 79720

2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE; MSKT END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL (PUBLICATION~062717).

3. APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER" ON THE FRONT FACE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD.

4. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

5. HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. 6. SYSTEM SHOWN USING STEEL WIDE FLANGE POSTS WITH COMPOSITE BLOCKOUTS.

7. A COMPOSITE MATERIAL BLOCKOUTS THAT MEETS THE REQUIREMENTS OF DMS-7210, MAY BE SUBSTITUTED FOR BLOCKOUTS OF SIMILAR DIMENSIONS. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

8. IF SOLID ROCK IS ENCOUNTERED IN THE AREA OF (POST 1) AND / OR (POST 2) CONTACT THE MANUFACTURER, & REFER TO THE LATEST ROADWAY MBGF STANDARD FOR INSTALLATION GUIDANCE 9. POSTS SHALL NOT BE SET IN CONCRETE.

10. SYSTEM MUST BE ATTACHED TO STANDARD 31" MBGF.

11. UNDER NO CIRCUMSTANCES SHALL THE GUARDRAIL WITHIN THE MSKT SYSTEM BE CURVED.

12. A FLARE RATE OF UP TO 25:1 MAY BE USED TO PREVENT THE TERMINAL HEAD FROM ENCROACHING ON THE SHOULDER. THE FLARE MAY BE DECREASED OR ELIMINATED FOR SPECIFIC INSTALLATIONS, IF DIRECTED BY THE ENGINEER.

13. THE SYSTEM IS SHOWN WITH TWO 12'-6" MBGF PANELS, ONE 25'-0" MBGF PANEL IS ALSO ALLOWED IN THEIR PLACE.

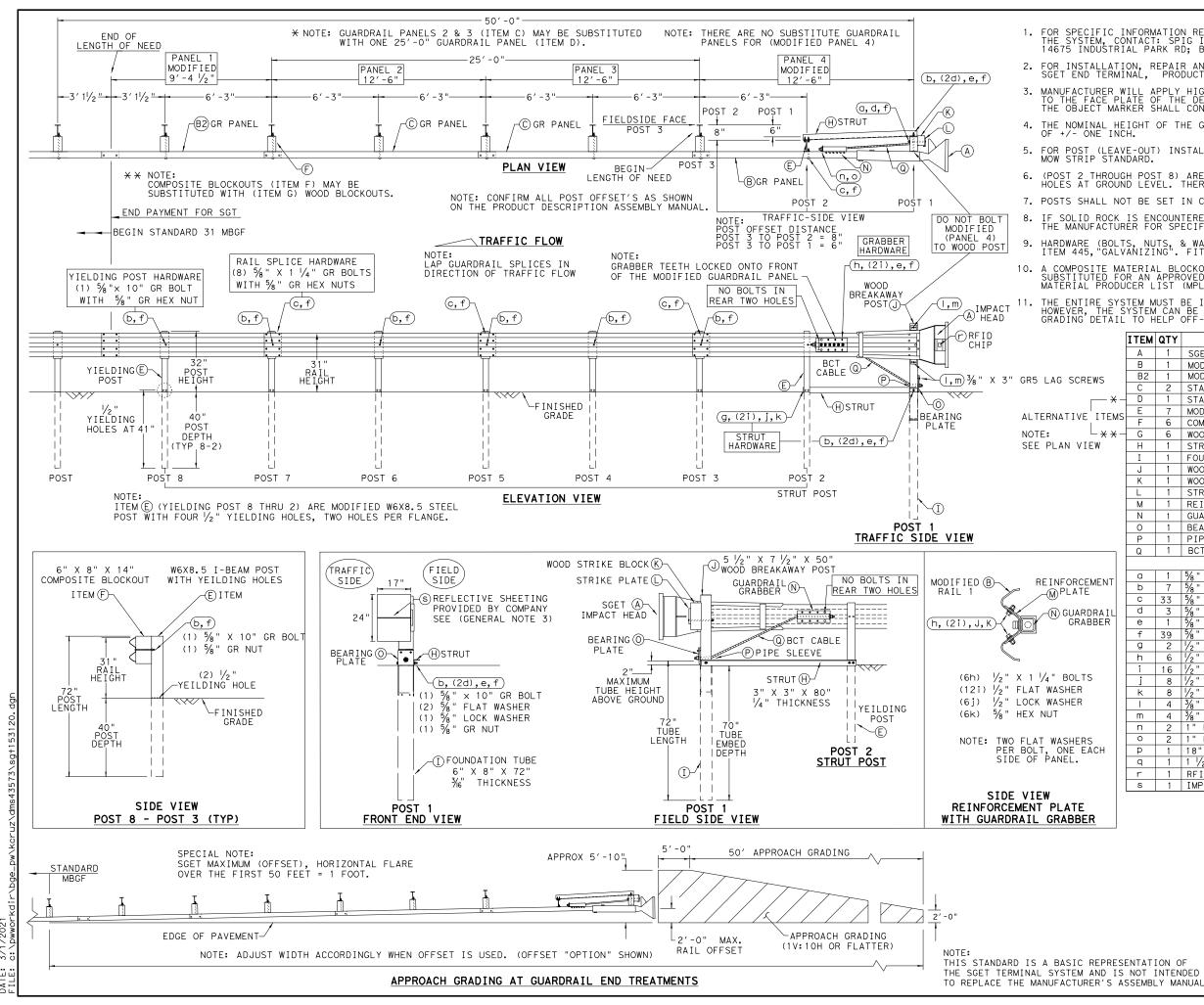
A DRIVING CAP WITH A TIMBER OR PLASTIC INSERT SHALL BE USED WHEN DRIVING POSTS 3-8 TO PREVENT DAMAGE TO THE GALVANIZING ON TOP OF THE POST. SPECIAL DRIVING CAP TO BE USED ON LOWER POSTS 1 & 2 TO PREVENT DAMAGE TO THE WELDED PLATES.

| | ITEM | QTY | MAIN SYSTEM COMPONENTS | ITEM NUMBERS |
|----------|------|-----|--|--------------------------------|
| | Α | 1 | MSKT IMPACT HEAD | MS3000 |
| | В | 1 | W-BEAM GUARDRAIL END SECTION, 12 Ga. | SF1303 |
| | С | 1 | POST 1 - TOP (6" X 6" X 1/8" TUBE) | MTPHP1A |
| | D | 1 | POST 1 - BOTTOM (6' W6X15) | MTPHP1B |
| | E | 1 | POST 2 - ASSEMBLY TOP | UHP2A |
| | F | 1 | POST 2 - ASSEMBLY BOTTOM (6' W6X9) | HP2B |
| | G | 1 | BEARING PLATE | E750 |
| | н | 1 | CABLE ANCHOR BOX | S760 |
| | J | 1 | BCT CABLE ANCHOR ASSEMBLY | E770 |
| | К | 1 | GROUND STRUT | MS785 |
| | L | 6 | W6×9 OR W6×8.5 STEEL POST | P621 |
| NOTES: 🗙 | М | 6 | COMPOSITE BLOCKOUTS | CBSP-14 |
| | N | 1 | W-BEAM MGS RAIL SECTION (9'-4 1/2") | G12025 |
| | 0 | 2 | W-BEAM MGS RAIL SECTION (12'-6") | G1203A |
| | Р | 6 | WOOD BLOCKOUT 6" X 8" X 14" | P675 |
| DWN. ★★< | Q | 1 | W-BEAM MGS RAIL SECTION (25'-0") | G1209 |
| | | | SMALL HARDWARE | |
| E PANEL | a | 2 | 5%6 " × 1" HEX BOLT (GRD 5) | B5160104A |
| | b | 4 | 5/16 " WASHER | W0516 |
| | С | 2 | 5/6 " HEX NUT | N0516 |
| | d | 25 | 5% " Dia. × 1 ¼ " SPLICE BOLT (POST 2) | B580122 |
| | е | 2 | 5%∥ Dia. × 9″ HEX BOLT (GRD A449) | B580904A |
| | f | 3 | 5%/" WASHER | W050 |
| | g | 33 | 5%∥ Dia. H.G.R NUT | N050 |
| | h | 1 | ¾ " Dia. × 8 ½ " HEX BOLT (GRD A449) | B340854A |
| | j | 1 | ¾" Dia. HEX NUT | N030 |
| | k | 2 | 1 ANCHOR CABLE HEX NUT | N100 |
| | I | 2 | 1 ANCHOR CABLE WASHER | W100 |
| | m | 8 | 1/2" × 1 1/4" A325 BOLT WITH CAPTIVE WASHER | SB12A |
| | n | 8 | 1/2" STRUCTURAL NUTS | N012A |
| | 0 | 8 | 1 1/16 " O.D. × %6 " I.D. STRUCTURAL WASHERS | W012A |
| | P | 1 | BEARING PLATE RETAINER TIE | CT-100ST |
| | q | 6 | 5% " × 10" H.G.R. BOLT | B581002 |
| | r | 1 | OBJECT MARKER 18" X 18" | E3151 |
| | | Γ | Texas Department of Transportation | Design Division Standard |

SINGLE GUARDRAIL TERMINAL MSKT-MASH-TL-3

SGT (12S) 31-18

| FILE: sg+12s3118.dgn | DN:T× | DOT | ск:км | DW: | /P | CK:CL |
|----------------------|-------|------|--------|-----|----|-----------|
| C TxDOT: APRIL 2018 | CONT | SECT | JOB | | H | IGHWAY |
| REVISIONS | 0911 | 28 | 049,ET | c. | | CR |
| | DIST | | COUNTY | , | | SHEET NO. |
| | LFK | | HOUSTO | DN | | 45 |



2021 Ř DATE:

GENERAL NOTES

1. FOR SPECIFIC INFORMATION REGARDING INSTALLATION AND TECHNICAL GUIDANCE OF THE SYSTEM, CONTACT: SPIG INDUSTRY, INC. AT 1(267) 644-9510. 14675 INDUSTRIAL PARK RD; BRISTOL, VA 24202

2. FOR INSTALLATION, REPAIR AND MAINTENANCE REFER TO THE MANUFACTURER'S; SGET END TERMINAL, PRODUCT DESCRIPTION ASSEMBLY MANUAL.

3. MANUFACTURER WILL APPLY HIGH INTENSITY REFLECTIVE SHEETING, "OBJECT MARKER' TO THE FACE PLATE OF THE DEVICE PER MANUFACTURER'S RECOMMENDATIONS. THE OBJECT MARKER SHALL CONFORM TO THE STANDARDS REQUIRED IN TEXAS MUTCD. 4. THE NOMINAL HEIGHT OF THE GUARDRAIL BEAM IS 31 INCHES WITH A TOLERANCE OF +/- ONE INCH.

5. FOR POST (LEAVE-OUT) INSTALLATION AND GUIDANCE SEE TXDOT'S LATEST ROADWAY MOW STRIP STANDARD.

6. (POST 2 THROUGH POST 8) ARE MODIFIED STEEL-YIELDING POSTS WITH YIELDING HOLES AT GROUND LEVEL. THERE ARE NO SUBSTITUTE POSTS. 7. POSTS SHALL NOT BE SET IN CONCRETE.

8. IF SOLID ROCK IS ENCOUNTERED FOR ANY OF THE POSTS IN THE SYSTEM, CONTACT THE MANUFACTURER FOR SPECIFIC INSTALLATION GUIDANCE.

HARDWARE (BOLTS, NUTS, & WASHERS) SHALL BE GALVANIZED IN ACCORDANCE WITH ITEM 445, "GALVANIZING". FITTINGS SHALL BE SUBSIDIARY TO THE BID ITEM. A COMPOSITE MATERIAL BLOCKOUT THAT MEETS DMS-7210 REQUIREMENTS MAY BE SUBSTITUTED FOR AN APPROVED WOOD BLOCKOUT. SEE CONSTRUCTION DIVISION MATERIAL PRODUCER LIST (MPL) FOR CERTIFIED PRODUCERS.

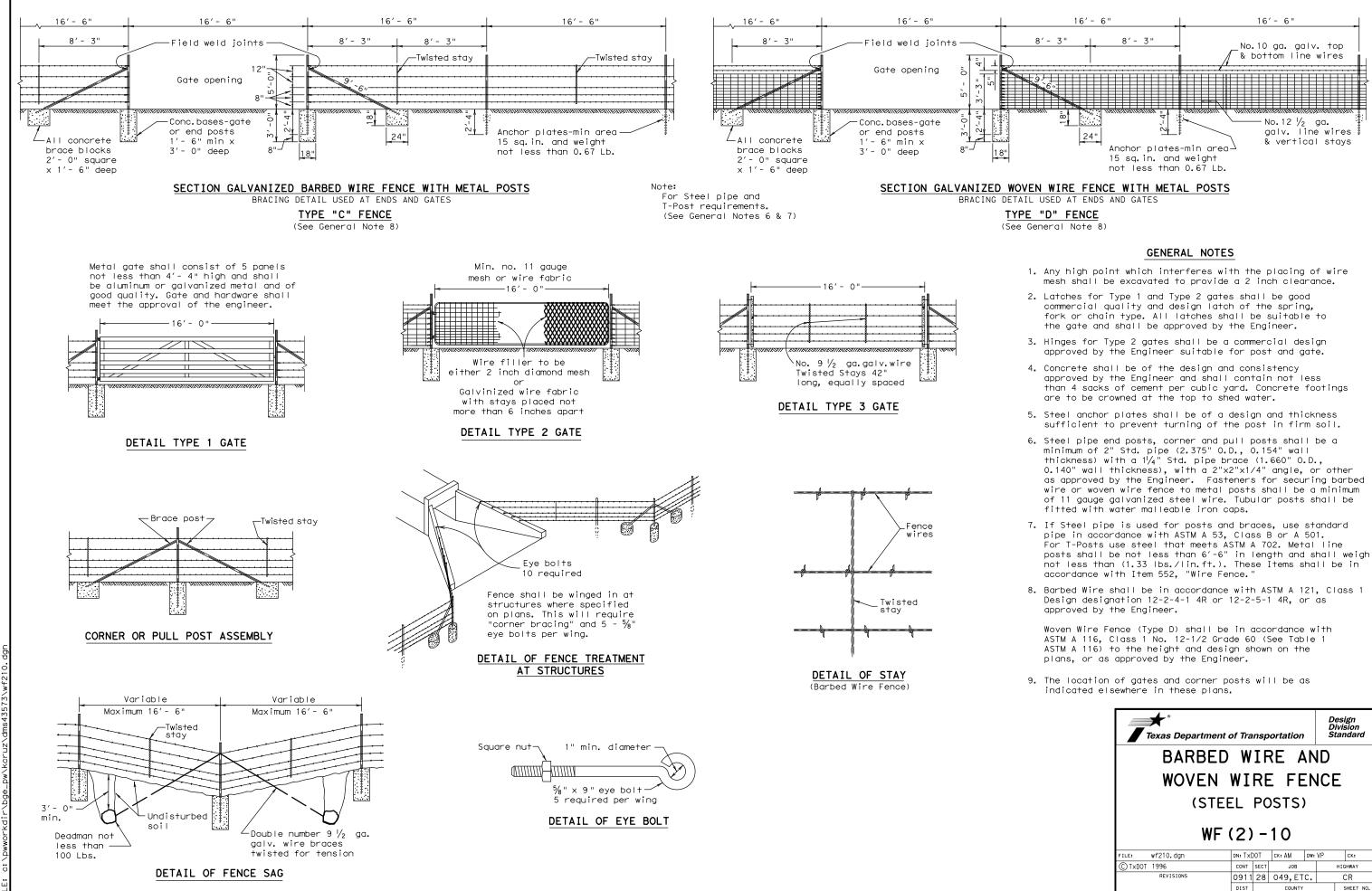
THE ENTIRE SYSTEM MUST BE INSTALLED IN A STRAIGHT LINE WITHOUT ANY CURVE. HOWEVER, THE SYSTEM CAN BE OFFSET BY TWO FEET AS SHOWN ON THE APPROACH GRADING DETAIL TO HELP OFF-SET THE IMPACT HEAD FROM SHOULDER OF THE ROAD.

| 'EM G | QTY | MAIN SYSTEM COMPONENTS | ITEM # |
|-------------------------|------|--|--------------------|
| 4 | 1 | SGET IMPACT HEAD | SIH1A |
| B | 1 | MODIFIED GUARDRAIL PANEL 12'-6" 12GA | 126SPZGF |
| B2 | 1 | MODIFIED GUARDRAIL PANEL 9'-4 1/2" 12GA | GP94 |
| C | 2 | STANDARD GUARDRAIL PANEL 12'-6" 12GA | GP126 |
| D | 1 | STANDARD GUARDRAIL PANEL 25'-0" 12GA | GP25 |
| E | 7 | MODIFIED YIELDING I-BEAM POST W6x8.5 | YP6MOD |
| F | 6 | COMPOSITE BLOCKOUT 6" X 8" X 14" | CB08 |
| G | 6 | WOOD BLOCKOUT 6" X 8" X 14" | WB08 |
| H | 1 | STRUT 3" X 3" X 80" × 1/4" A36 ANGLE | STR80 |
| I | 1 | FOUNDATION TUBE 6" X 8" X 72" × 3/6 " | FNDT6 |
| | 1 | WOOD BREAKAWAY POST 5 $\frac{1}{2}$ " x 7 $\frac{1}{2}$ " x 50" | WBRK50 |
| ĸ | 1 | WOOD STRIKE BLOCK | WSBLK14 |
| | 1 | STRIKE PLATE 1/4" A36 BENT PLATE | SPLT8 |
| | 1 | REINFORCEMENT PLATE 12 GA. GR55 | REPLT17 |
| N I | 1 | GUARDRAIL GRABBER 2 $\frac{1}{2}$ X 2 $\frac{1}{2}$ X 16 $\frac{1}{2}$ | GGR17 |
| | | BEARING PLATE 8" X 8 ½ " X ½ " A36 | |
| 2 | 1 | DEARING FLATE & $X \otimes \frac{7}{8}$ $X \frac{7}{8}$ A36 | BPLT8 |
| > | 1 | PIPE SLEEVE 4 1/4" X 2 3/8" O.D. (2 1/8" I.D.) | PSLV4 |
| ג | 1 | BCT CABLE 3/4" X 81" LENGTH | CBL81 |
| | | SMALL HARDWARE | |
| 3 | 1 | 5% " X 12" GUARDRAIL BOLT 307A HDG | 12GRBLT |
| b | 7 | 5% " X 10" GUARDRAIL BOLT 307A HDG | 10GRBLT |
| | 33 | 5/8 " X 1 1/4 " GR SPLICE BOLTS 307A HDG | 1 GRBLT |
| d | 3 | 5%∥ FLAT WASHER F436 A325 HDG | 58FW436 |
| e | 1 | 5%8 " LOCK WASHER HDG | 58LW |
| f 3 | 39 | 5%∥ GUARDRAIL HEX NUT HDG | 58HN563 |
| g | 2 | 1/2" X 2" STRUT BOLT A325 HDG | 2BLT |
| n | 6 | 1/2" X 1 1/4" PLATE BOLT A325 HDG | 125BLT |
| i · | 16 | 1/2" FLAT WASHER F436 A325 HDG | 12FWF436 |
| j | | 1/2 " LOCK WASHER HDG | 12LW |
| ĸ | | 1/2" HEX NUT A563 HDG | 12HN563 |
| | 4 | 3/8 " X 3" HEX LAG SCREW GR5 HDG | 38LS |
| n | 4 | 3/8" FLAT WASHER F436 A325 HDG | 38FW844 |
| n | 2 | 1" FLAT WASHER F436 A325 HDG | 1FWF436 |
| - - | 2 | 1" HEX NUT A563DH HDG | 1HN563 |
| 5 | 1 | 18" TO 24" LONG ZIP TIE RATED 175-200LB | ZPT18 |
| а — | 1 | 1 1/2 X 4 SCH-40 PVC PIPE | PSPCR4 |
| - | 1 | RFID CHIP RATED MIL-STD-810F | RFID810 |
| S | 1 | IMPACT HEAD REFLECTIVE SHEETING | RF1D810F |
| 5 | | IMPACE HEAD REPLECIIVE SHEELING | INOSOM |
| | | ® | |
| | | | Design Division |
| | | Texas Department of Transportation | Standard |
| | | | |
| | | SPIG INDUSTRY, LI | _C |
| | | · · · · · · | |
| | | SINGLE GUARDRAIL TER | MINA |
| | | | ~ |
| | | SGET - TL-3 - MAS | SH |
| | | SGT (15) 31-20 | h |
| | | | |
| | | | |
| | | | HIGHWAY |
| | | DED 091120 049, ETC. | CR |
| | | | SHEET |
| TATIC DT IN EMBLY | NTEN | F C TxDDT: APRIL 2020 CONT SECT JOB REVISIONS 0911 28 049, ETC. | |

LFK

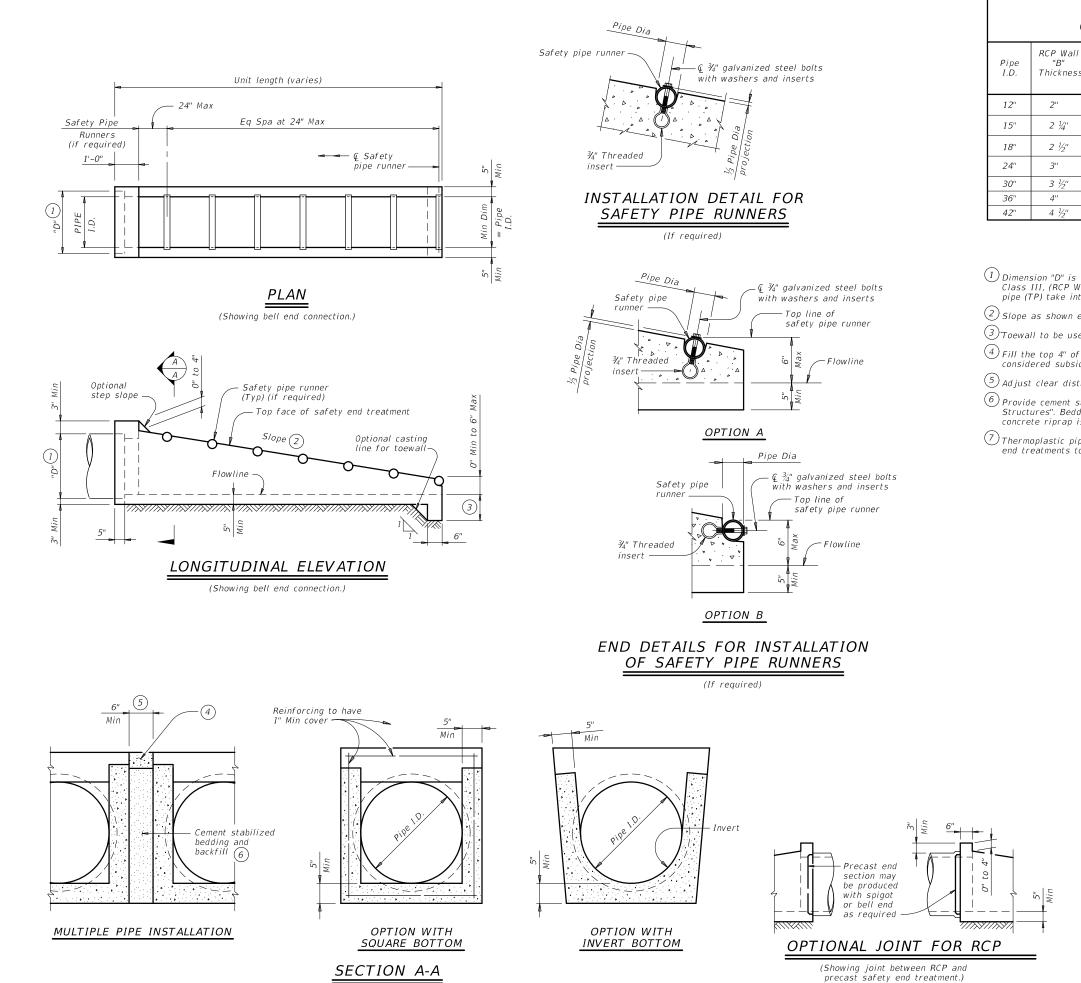
HOUSTON

46



3/1/2021 DATE:

| Texas Department of Transportation | | | | | | | | | |
|------------------------------------|---------|------|--------|-------|----|-----------|--|--|--|
| BARBED WIRE AND | | | | | | | | | |
| WOVEN WIRE FENCE | | | | | | | | | |
| | | | | | | | | | |
| (STEEL POSTS) | | | | | | | | | |
| | | | | | | | | | |
| ₩F | (2) | - | 10 | | | | | | |
| FILE: wf210.dgn | dn: Tx[| DOT | ск: АМ | Dw: V | Ρ | СК: | | | |
| C TxDOT 1996 | CONT | SECT | JOB | | нI | GHWAY | | | |
| REVISIONS | 0911 | 28 | 049,ET | с. | | CR | | | |
| NET ISTONS | | | | | | | | | |
| 121101010 | DIST | | COUNTY | | | SHEET NO. | | | |



REQUIREMENTS FOR CULVERT PIPES AND SAFETY PIPE RUNNERS

| TP Wall | | | Min | Pipe R Requ | | Required | Pipe Run | ner Size |
|------------|----------|-------|------------|----------------|-----------------------|-----------------|----------|----------|
| Thickness | "D" 1 | Slope | Length | Single Pipe | Multiple Pipe | Nominal Dia. | 0.D. | I.D. |
| 1.15" | 17.00" | 6:1 | 4' - 9'' | No | Yes, for > 2 pipes | 3'' STD | 3.500" | 3.068" |
| 1.30" | 20.50" | 6:1 | 6' - 5'' | No | Yes, for > 2 pipes | 3" STD | 3.500" | 3.068" |
| 1.60" | 24.00" | 6:1 | 8' - 0'' | No | Yes, for > 2 pipes | 3" STD | 3.500" | 3.068'' |
| 1.95" | 31.00" | 6:1 | 11' - 3'' | No | Yes, for > 2 pipes | 3" STD | 3.500" | 3.068'' |
| 2.65" | 38.50" | 6:1 | 14' - 8'' | No | Yes | 4'' STD | 4.500" | 4.026" |
| 2.75" | 45.50" | 6:1 | 17' - 11'' | Yes | Yes | 4'' STD | 4.500" | 4.026" |
| N/A | 52.50" | 6:1 | 21' - 2" | Yes | Yes | 4'' STD | 4.500" | 4.026" |

(1) Dimension "D" is based on reinforced concrete pipe (RCP) meeting the requirements of ASTM C-76, Class III, (RCP Wall "B" thickness). Adjust "D" for any other wall thickness used. For thermoplastic pipe (TP) take into account the annular space requirements for grouted connections.

 $^{(2)}$ Slope as shown elsewhere in the plans. Slope of 6:1 or flatter is required for vehicle safety.

3 Toewall to be used only when dimension is shown elsewhere in the plans.

(4) Fill the top 4" of void between precast end treatments with concrete riprap. Concrete riprap is considered subsidiary to the Item 467, "Safety End Treatment".

 $^{(5)}$ Adjust clear distance between pipes to provide for the minimum distance between safety end treatments.

(6) Provide cement stabilized bedding and backfill in accordance with the Item 400, "Excavation and Backfill for Structures". Bedding and backfill is considered subsidiary to the Item 467, "Safety End Treatment". When concrete riprap is specified around the safety end treatment, backfill as directed by Engineer.

(7) Thermoplastic pipe wall thickness may vary. Adjust accordingly. Thermoplastic pipe requires the safety end treatments to have a bell end for grouted connections.

GENERAL NOTES:

Precast safety end treatment for reinforced concrete pipe (RCP), and thermoplastic pipe (TP) may be used for TYPE II end treatment as specified in Item "Safety End Treatment".

When precast safety end treatment is used as a Contractor's alternate to mitered RCP, riprap will not be required unless noted otherwise on the plans

Synthetic fibers listed on the "Fibers for Concrete" Material Producer List (MPL) may be used in lieu of steel reinforcing in riprap concrete unless noted otherwise.

Manufacture this product in accordance with Item 467, "Safety End Treatment" except as noted below

A. Provide minimum reinforcing of #4 at 6" (Grade 40) or #4 at 9" (Grade 60) each way or 6"x6" - D12 x D12 or 5"x5" - D10 x D10 welded wire reinforcement (WWR).

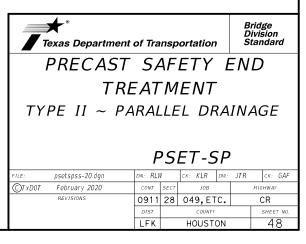
B. For precast (steel formed) sections, provide Class "C" concrete (f'c = 3.600 psi).

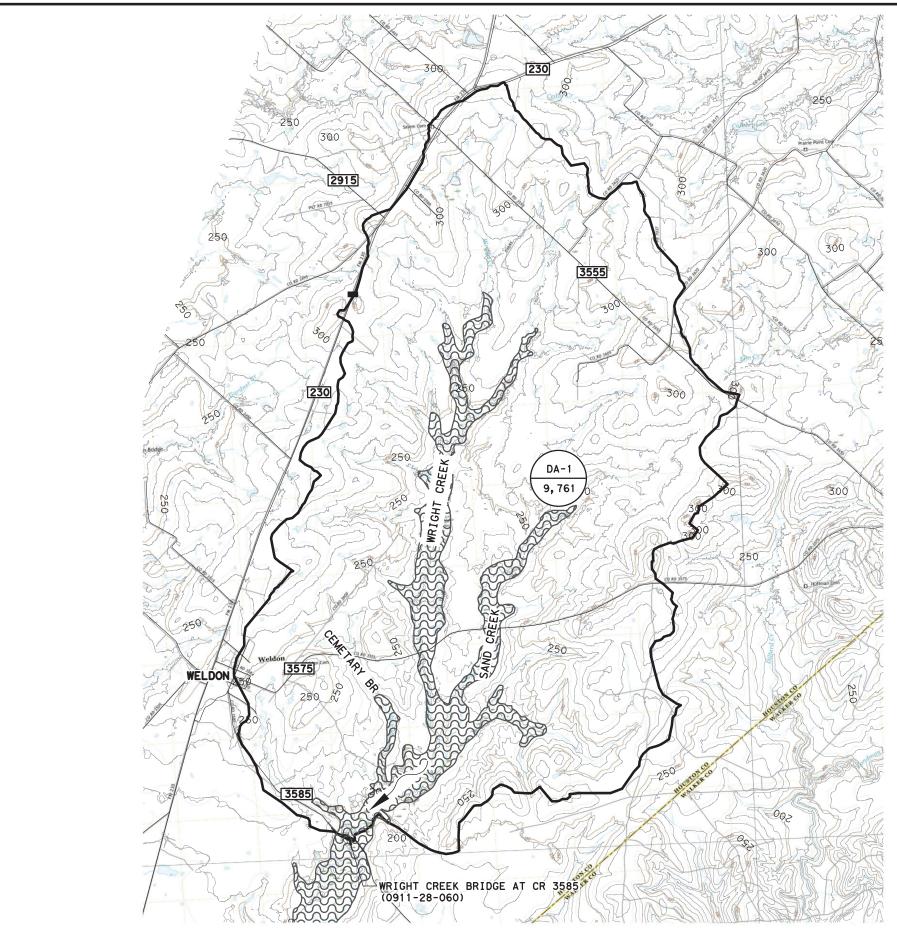
At the option and expense of the Contractor the next larger size of safety end treatment may be furnished; as long as the "D" dimension

cast is that of the required size of pipe. Pipe runners are designed for a traversing load of 10,000 Lbs at yield as recommended by Research Report 280-2F, "Safety Treatment of Roadside Parallel-Drainage Structures", Texas Transportation Institute, March 1981. Provide pipe runners meeting the requirements of ASTM A53 (Type E or S, Grade B), ASTM A500 (Grade B), or API 5LX52.

Galvanize all steel components except reinforcing steel after fabrication. Repair galvanizing damaged during transport or construction in accordance with the specifications.

Connect RCP using the Optional Joint for RCP detail shown or in accordance with Item 464, "Reinforced Concrete Pipe". Connect TP by grouting. See PBGC standard for grouted connections with TP and precast safety end treatment.





NRCS EQUATION Q = QUARF

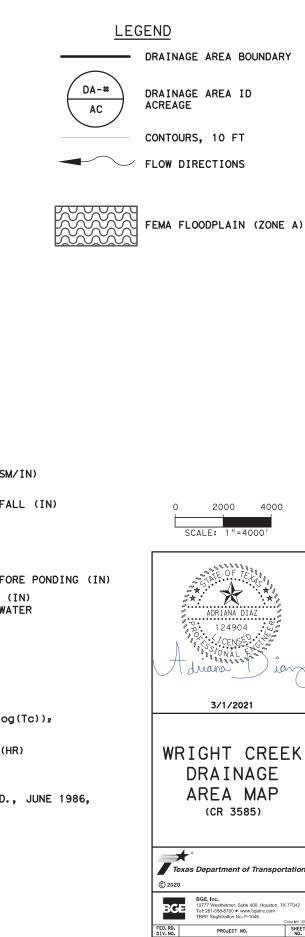
| Q | - | QUARF | |
|----|-----|-------|-----|
| QL | . = | UNIT | PE |
| Α | = | AREA | (M) |
| R | = | ACCUM | UL |
| F | = | ADJUS | TME |
| | | | |

R = (P-Ia) 2/((P-Ia) + S)Ia = 0.2S

S = z(100/CN-1)z = 10 FOR ENGLISH

APPENDIX F

| DRAINAG | GE AREA | HYDROLOGIC METHOD | Тс | Тс | SOIL | RCN | | ſ | RAINFALL I | DEPTH (IN |) | | | | DISCHAR | GE (CFS) | | |
|---------|--------------------|-------------------|--------|------|-------|-----|------|------|------------|-----------|-------|--------|-------|------|---------|----------|--------|--------|
| (AC) | (MI ²) | HIDROLOGIC METHOD | (MIN) | (HR) | GROUP | RCN | 2 YR | 5 YR | 10 YR | 25 YR | 50 YR | 100 YR | 2 YR | 5 YR | 10 YR | 25 YR | 50 YR | 100 YR |
| 9760.61 | 15.26 | NRCS (TR 55) | 267.97 | 4.47 | С | 76 | 3.84 | 5.04 | 6.00 | 7.44 | 8.64 | 10.08 | 2,786 | 4489 | 6,002 | 8,249 | 10,180 | 12,546 |



6 STATE

TEXAS LFK

CONT. SECT.

COUNTY

 CONT.
 SECT.
 JOB
 HIGHWAY NO.

 0911
 28
 049, ETC.
 CR

HOUSTON

PEAK DISCHARGE (CSM/IN) VI2) ATED EXCESS RAINFALL (IN) MENT FACTOR

Id = INITIAL ABSTRACTION BEFORE PONDING (IN) P = TOTAL DEPTH OF RAINFALL (IN)S = POTENTIAL MAX DEPTH OF WATER RETAINED IN WATERSHED

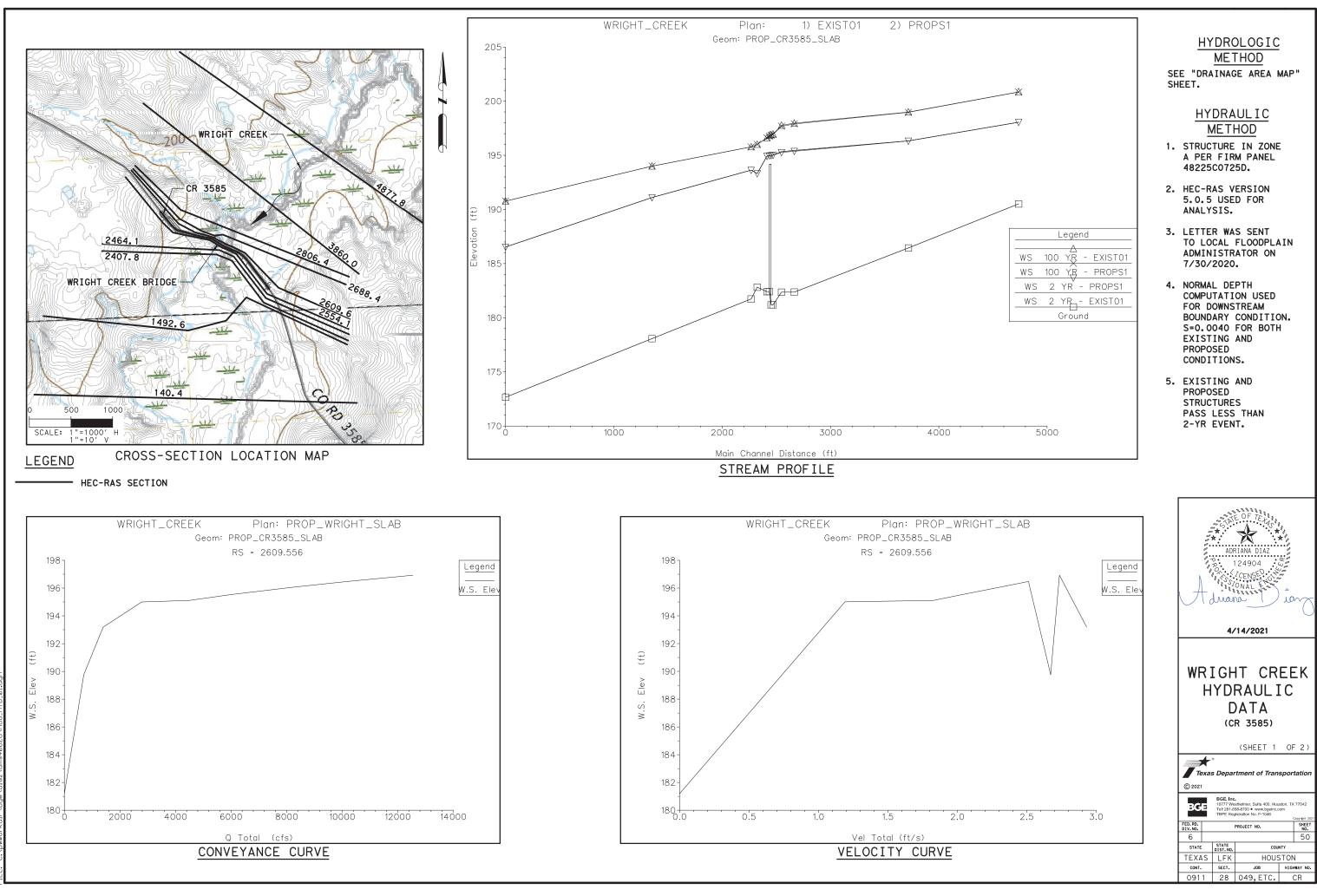
CN = RUNOFF CURVE NUMBER

 $\log(Qu) = C_0 + C_1 \log T_C + C_2 (\log(T_C))_2$

Tc = TIME OF CONCENTRATION (HR) CO, C1, C2 = COEFFICIENTS

FROM 210-VI-TR-55, SECOND ED., JUNE 1986,





ODEL NAME: WRIGHT CREEK HYDRAULIC DATA ATE: 4/14/2021 ILE: c:\pwworkdir\bge\diaz\dms40626\HOULHYD.0

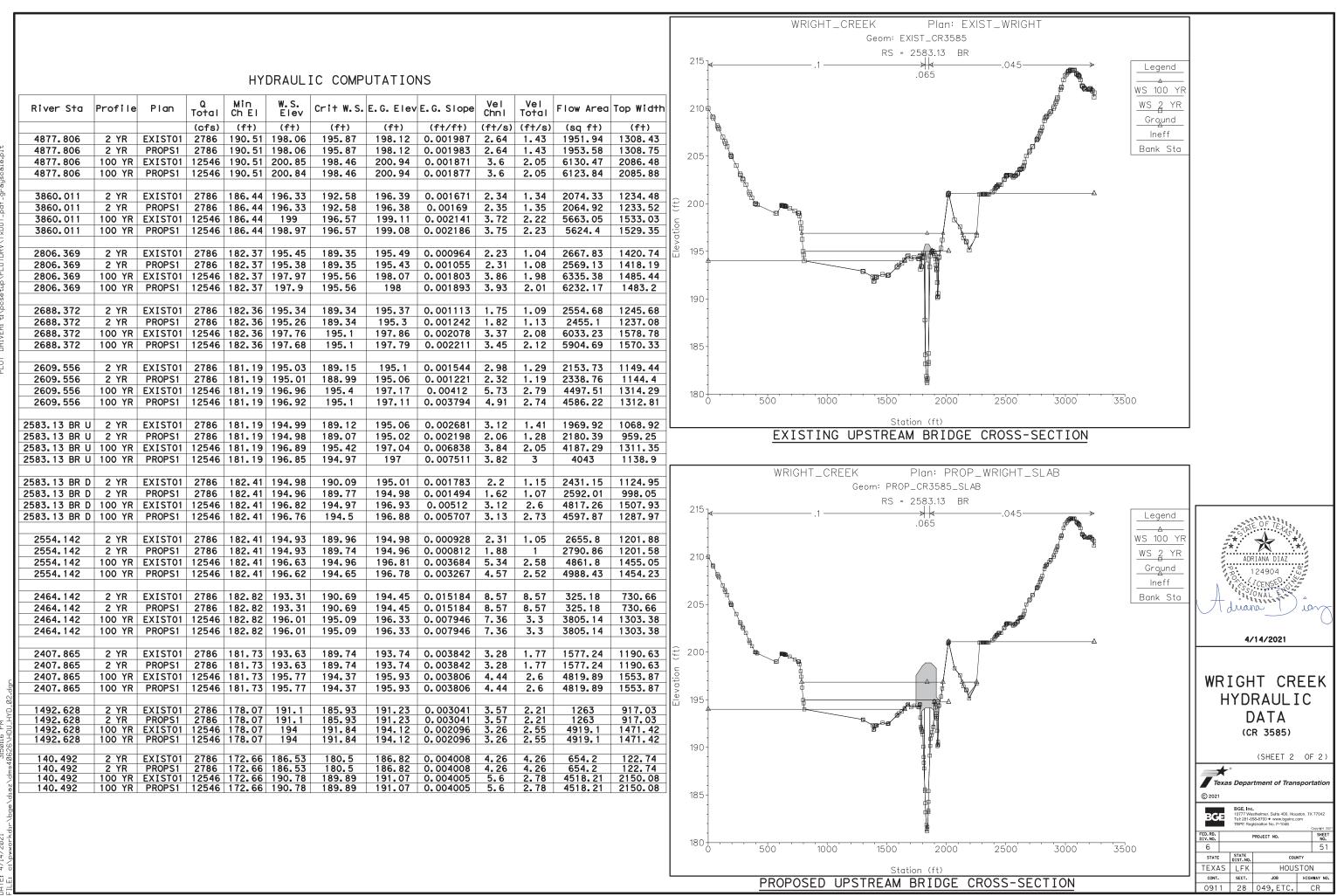


TABLE: T:\PCSETUP\Tables\TXDDT.TBL
 DRIVER: t:\pcsetup\PLDTDRV\TxDDT_pdf_c

EL NAME: WRIGHT CREEK HYDRAULIC DATA E: 4/14/2021 3:50:16 P

LIVE BED CONTRACTION SCOUR ANALYSIS MODIFIED LAURSEN FORMULA

$Y_{a}/Y_{1} = (Q_{a}/Q_{1})^{4.66} (W_{1}/W_{2})^{4} - (Y_{a}/Y_{1})$

- Y, = AVERAGE DEPTH IN THE UPSTREAM MAIN CHANNEL, FT (M)

- Y = AVERAGE DEPTH IN THE OPSTREAM MAIN CHANNEL, FT (M) Y = AVERAGE DEPTH IN THE CONTRACTED SECTION, FT (M) Y = EXISTING DEPTH IN THE CONTRACTED SECTION BEFORE SCOUR, FT (M) Q = FLOW IN THE UPSTREAM CHANNEL TRANSPORTING SEDIMENT, FT'/S (M*/S) Q = FLOW IN THE CONTRACTED CHANNEL, FT'/S (M*/S) W = BOTTOM WIDTH OF THE UPSTREAM MAIN CHANNEL THAT IS TRANSPORTING BED MATERIAL, FT (M)
- W_{a} = BOTTOM WIDTH OF MAIN CHANNEL IN CONTRACTED SECTION LESS PIER WIDTH(S), FT (M)
- K, = EXPONENT DETERMINED BELOW
- $Y_* = Y_* Y_* =$ (AVERAGE CONTRACTION SCOUR DEPTH)

CRITICAL VELOCITY COMPUTATION

JAIN AND FISHER'S EQUATION

| $V_e = K_u Y^{1e} D_{ee}^{1/e}$ | \ | 1. | = | К. | Y 14 | D."" |
|---------------------------------|---|----|---|----|------|------|
|---------------------------------|---|----|---|----|------|------|

- D.,, $Y_{s} = 1.84 \text{ A } F_{s}^{am}(Y / A)^{as}$
- $F_{e} = V_{e} / (GY)^{**}$
- FOR LIVE-BED SCOUR (F $F_* > 0.2$): $Y_{s} = 2 A (F - F_{s})^{sm} (Y / A)^{sm}$

V. = CRITICAL VELOCITY ABOVE WHICH BED MATERIAL OF SIZE D AND SMALLER WILL BE TRANSPORTED, FT/S (M/S)

FOR CLEAR-WATER SCOUR (F - F. <=0.2):

- Y = AVERAGE DEPTH OF FLOW UPSTREAM OF THE BRIDGE, FT (M)
- $D = PARTICLE SIZE FOR V_{e}$, FT (M) $D_{w} = PARTICLE SIZE IN A MIXTURE OF WHICH 50 PERCENT ARE SMALLER, FT (M)$
- $K_{u} = 6.19$ SI UNITS, 11.17 ENGLISH UNITS

PIER SCOUR ANALYSIS

Y./Y.=2K.K.K. (A/Y.)*** FR.**

- Y. = SCOUR DEPTH, FT (M) Y. = FLOW DEPTH DIRECTLY UPSTREAM OF THE PIERFT (M)
- \dot{K} = CORRECTION FACTOR FOR PIER NOSE SHAPE K_z = CORRECTION FACTOR FOR ANGLE OF ATTACK OF FLOW
- K = CORRECTION FACTOR FOR BED CONDITION
- A = PIER WIDTH, FT (M)
- L = LENGTH OF PIER, FT (M)
- FR. = FROUDE NUMBER DIRECTLY UPSTREAM OF THE PIER

LIVE BED CONTRACTION SCOUR RESULTS

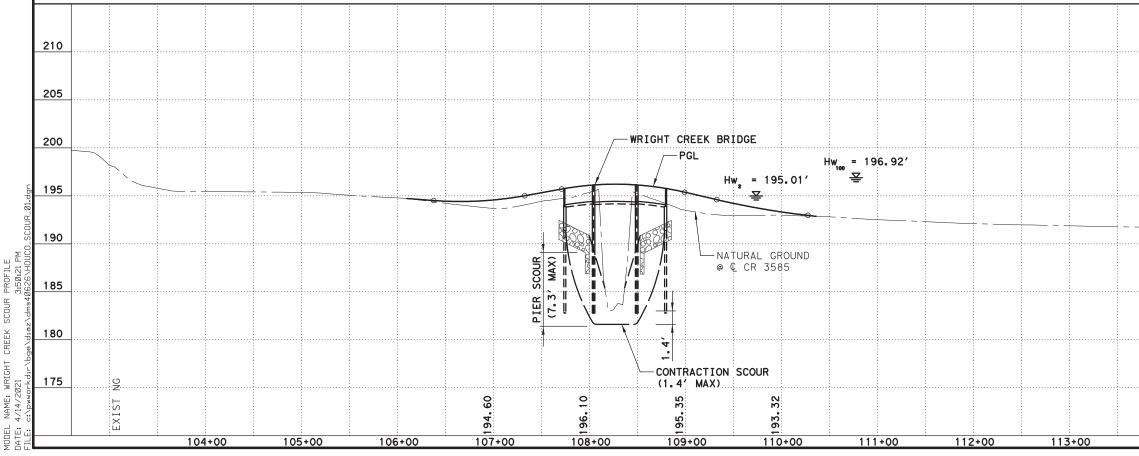
| | | Yı | Yo | Qı | Qz | Wı | W2 |
|------------------|----------|-------|-------|-------|-------|------|------|
| DESIGN CONDITION | Q-BRIDGE | (FT) | (FT) | (CFS) | (CFS) | (FT) | (FT) |
| OVERTOPPING | 1239 | 12.85 | 13.79 | 1239 | 1239 | 130 | 102 |
| 100YR - CHECK | 2291 | 15.26 | 15.66 | 5058 | 2291 | 130 | 102 |

CRITICAL VELOCITY RESULTS

| DESIGN | Y | Dso | Ku | Vc | scou |
|---------------|-------|----------|-------|-------|------|
| CONDITION | (FT) | (IN) | NU | (FPS) | 3000 |
| OVERTOPPING | 12.85 | 0.000246 | 11.17 | 1.07 | LIV |
| 100YR - CHECK | 15.26 | 0.000246 | 11.17 | 1.1 | LIV |

CRITICAL VELOCITY RESULTS

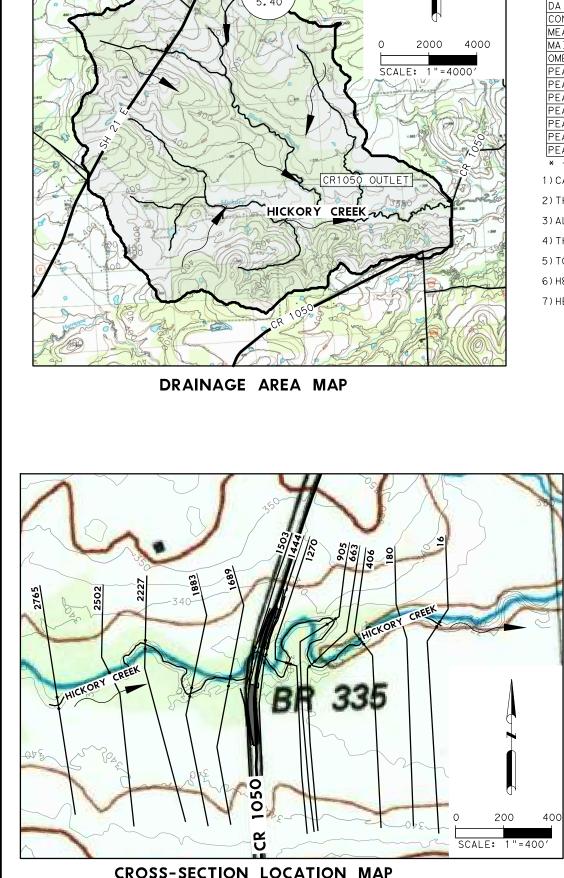
| K1 K2 K3 (FT) 2 1 2 1.1 13.79 100 1 2 1.1 15.66 | FR I (FT) (FPS) 2 0.056 1.19 2 0.122 2.74 | 1.07 5.03 | 0 50 SCALE: 1"= 1"= | 100 100' H |
|---|---|-----------|--|---|
| | | | | |
| 100 1 2 1.1 15.66 | 2 0.122 2.74 | 1.1 7.32 | | |
| | | | | |
| | | | | 107 V |
| | | | 210 210 | TELAS ** |
| | | | 205 | 04 SEP Char |
| | | | 200 / duana | 2021 |
| Hw ₁₀₀ = 196.92′)1′ ≅ | | | 195 | |
| | | | WRIGHT SCO 190 PROF | UR |
| IND | | | 185 | |
| | | | 180 © 2021 | t of Transportation |
| | | | Tel: 281-558-8700 TBPE Registration I TBPE Registration I FED. RD. DIV. NO. PROJECT | No. F-1046 Copyright 2021 T NO. SHEET NO. |
| | | | 6 state state DIST. NO. TEXAS LFK cont. sect. | 52 county HOUSTON JOB HIGHWAY NO. |
| 0 111+00 112+00 11 | 3+00 114+00 | 115+00 | 0911 28 049 | ,ETC. CR |



| UR | TYPE |
|----|------|
| ٧E | BED |
| ٧E | BED |







DA-1050

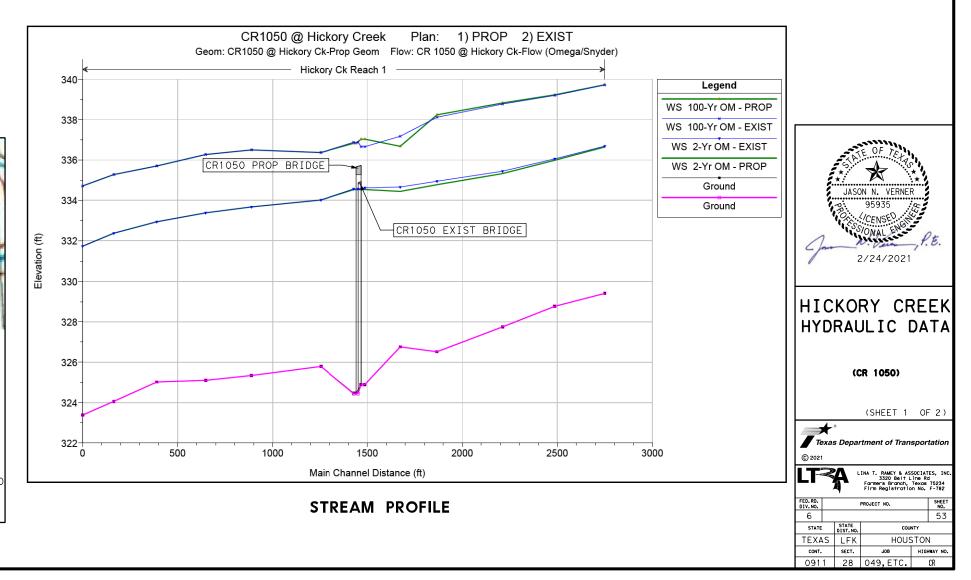
5.40

| OMEGA EM REGRESSION EQUATIONS | |
|------------------------------------|--------|
| A I.D. | CR1050 |
| DNTRIBUTING DRAINAGE AREA, (SQ.MI) | 5.40 |
| EAN ANNUAL PRECIPITATION (IN) | 46 |
| AIN CHANNEL SLOPE (FT/FT) | 0.0043 |
| NEGA EM | -0.127 |
| EAK FLOWRATE (2-YR), (CFS)* | 569* |
| EAK FLOWRATE (5-YR), (CFS) | 1,061 |
| EAK FLOWRATE (10-YR), (CFS) | 1,404 |
| EAK FLOWRATE (25-YR), (CFS) | 1,946 |
| EAK FLOWRATE (50-YR), (CFS) | 2,401 |
| EAK FLOWRATE (100-YR), (CFS) | 2,933 |
| AK FLOWRATE (500-YR), (CFS) | 4,395 |
| - DESIGN YEAR | |

1) CALCULATION ARE BASED ON THE TXDOT HYDRAULIC DESIGN MANUAL (SEPTEMBER 2019) PROCEDURES. 2) THIS SITE IS DESIGNATED AS A ZONE "A" AS SHOWN ON PANEL 48225C0250D, EFFECTIVE DATE: APRIL 4,2011. 3) ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM.

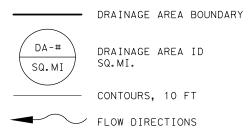
4) THE DOWNSTREAM BOUNDARY CONDITION WAS ESTABLISHED USING NORMAL DEPTH WITH A SLOPE = 0.0043 FT/FT. 5) TOPOGRAPHY DATA UTILIZED FROM TNRIS USGS16-1M-NECHES-BASIN DEMS.

6)H&H FILES WERE SENT TO THE LOCAL FLOODPLAIN ADMINISTRATOR JIM L. LOVELL ON OCTOBER 16,2020. 7) HEC-RAS v5.0.7 WAS UTITLIZED FOR HYDRAULIC DESIGN.



CROSS-SECTION LOCATION MAP

LEGEND

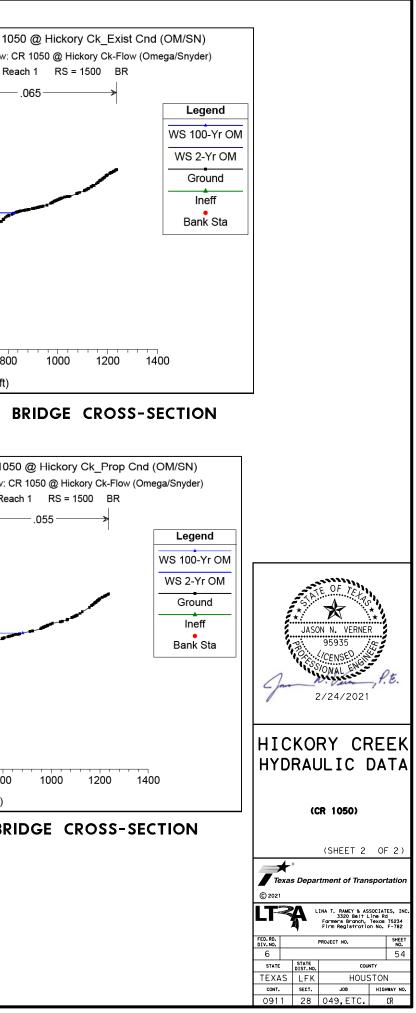


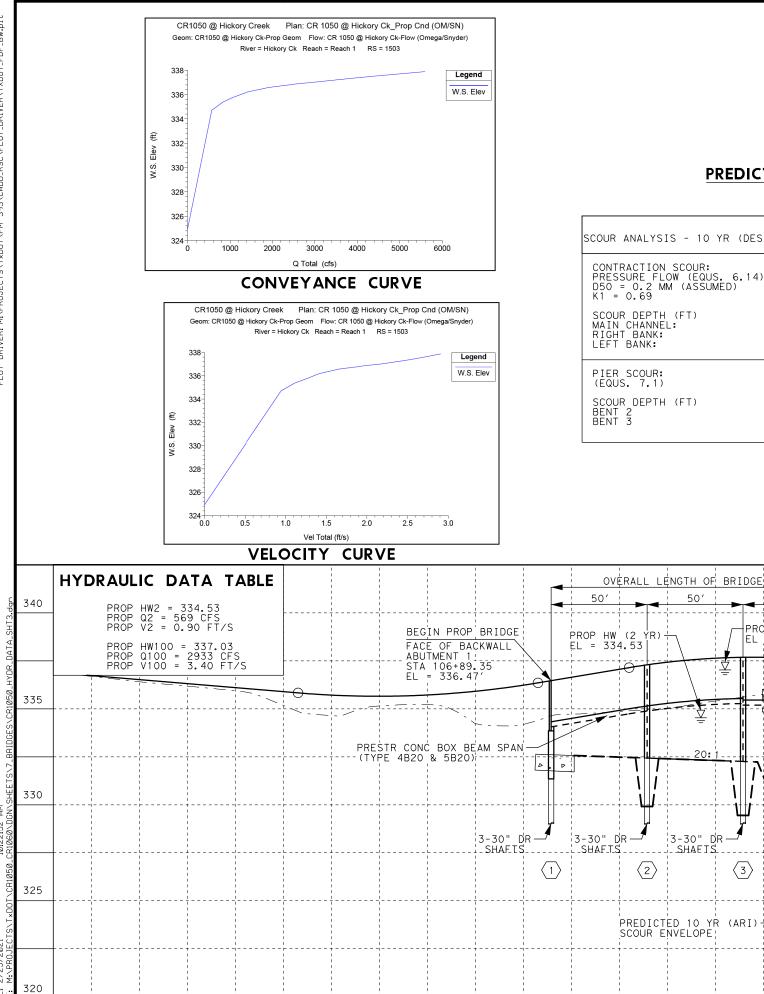
| CR1050 @ Hickory Creek Plan: CR 1050 Geom: CR1050 @ Hickory Ck-Exist Geom Flow: CR | | | | S | TATION | COMPU | ORAULIC | HYD | | | |
|---|----------------|--------------|---|--------------|--------------|------------|-------------------------|------------------|--------------|-----------------------------------|--|
| River = Hickory Ck Reach = Reac | 96 | roude # Chil | Tow Area Top Width F | Vel Totall | Vel Chnll | E.G. Slope | W.S. Flev | Min Ch Fil | Q Total | Profile Plan | River Sta |
| | | | (sq ft) (ft) | (ft/s) | (f†/s) | (f+/f+) | (f+) | (f†) | (cfs) | | |
| s50− .065 | 350 | 0.21 | 266.49 144.30 260.75 133.66 | 2.14 | 2.54 | 0.001288 | 336.69 336.64 | 329.41 329.41 | 569 569 | 2-Yr OM EXIST 2-Yr OM PROP | <u>2765</u> 2765 |
| 4 | | 0.27 | 1353.89 539.23 | 2.17 | 4.19 | 0.001759 | 339.72 | 329.41 | 2933 | 100-Yr OM EXIST | 2765 |
| 5 | | 0.27 | 1361.51 542.07 | 2.15 | 4.17 | 0.001735 | 339.73 | 329.41 | 2933 | 100-Yr OM PROP | 2765 |
| 345 | 34 | 0.32 | 152.08 35.60 | 3.74 | 3.74 | 0.003094 | 336.06 | 328.77 | 569 | 2-Yr OM EXIST | 2502 |
| | | 0.32 | 149.65 35.19 | 3.80 | 3.80 | 0.003252 | 335.99 | 328.77 | 569 | 2-Yr OM PROP | 2502 |
| | 2.44 | 0.30 | 1135.63531.571152.09533.12 | 2.58 | 4.59 | 0.002252 | 339.20 339.23 | 328.77 328.77 | 2933 2933 | 100-Yr OM EXIST 100-Yr OM PROP | <u> 2502 </u> |
| 340- | 340 | | | | | | | | | | |
| | Elevation (ft) | 0.26 | 186.62 58.62 179.92 55.02 | 3.05 3.16 | 3.21 3.30 | 0.001899 | 335.44 | 327.75 | 569 569 | 2-Yr OM EXIST 2-Yr OM PROP | <u>2227</u> 2227 |
| 335- | ie 33 | 0.27 | 179.9255.021241.00622.69 | 2.36 | 4.30 | 0.002067 | 335.33 338.77 | 327.75 327.75 | 2933 | 2-Yr OM PROP 100-Yr OM EXIST | 2227 |
| | eva | 0.26 | 1274.57 628.95 | 2.30 | 4.20 | 0.001682 | 338.83 | 327.75 | 2933 | 100-Yr OM PROP | 2227 |
| | | i | 209.50 46.69 | 2.72 | 2.79 | 0.001255 | 334.95 | 326.52 | 569 | 2-Yr OM EXIST | 1883 |
| 330⊣ ** | 330 | 0.21 | 201.59 45.16 | 2.82 | 2.88 | 0.001401 | 334.77 | 326.52 | 569 | 2-Yr OM PROP | 1883 |
| | | 0.32 | 1087.25 569.37 | 2.70 | 5.35 | 0.002524 | 338.12 | 326.52 | 2933 | 100-Yr OM EXIST | 1883 |
| 225 | 201 | 0.30 | 1157.63 593.15 | 2.53 | 5.11 | 0.002261 | 338.24 | 326.52 | 2933 | 100-Yr OM PROP | 1883 |
| | 32 | 0.23 | 195.64 108.88 | 2.91 | 3.13 | 0.001439 | 334.66 | 326.76 | 569 | 2-Yr OM EXIST | 1689 |
| - | | 0.24 | 184.28 92.79 | 3.09 | 3.28 | 0.001660 | 334.44 | 326.76 | 569 | 2-Yr OM PROP | 1689 |
| 320 | 320 | 0.46 | 801.36 494.20 576.39 386.16 | 3.66 5.09 | 7.50 | 0.005124 | 337.18 336.68 | 326.76 326.76 | 2933 2933 | 100-Yr OM EXIST 100-Yr OM PROP | <u> </u> |
| 0 200 400 600 800 | | | | | | | | | | | |
| Station (ft) | | 0.13 | 338.79 120.86 697.24 210.67 | 1.68 | 2.12 | 0.000307 | 334.62 334.53 | 324.90 324.90 | 569 569 | 2-Yr OM EXIST 2-Yr OM PROP | <u>1503</u> 1503 |
| Station (it) | | 0.39 | 948.40 478.33 | 3.09 | 7.04 | 0.002481 | 336.66 | 324.90 | 2933 | 100-Yr OM EXIST | |
| EXISTING UPSTREAM B | | 0.15 | 1502.49 545.64 | 1.95 | 2.58 | 0.000129 | 337.03 | 324.90 | 2933 | 100-Yr OM PROP | 1503 |
| EXISTING OF STREAM DI | | 0.14 | 233.22 32.00 | 2.44 | 2.58 | 0.000458 | 334.56 | 324.90 | 569 | 2-Yr OM EXIST | 1500 BR U |
| | | 0.05 | 629.32 126.86 | 0.90 | 1.02 | 0.000036 | 334.53 | 324.90 | 569 | 2-Yr OM PROP | 1500 BR U |
| | | 0.33 | 697.07 443.50 915.09 363.49 | 4.21 3.21 | 3.69 3.76 | | 336.66 337.03 | 324.90 324.90 | 2933 2933 | 100-Yr OM EXIST 100-Yr OM PROP | |
| CR1050 @ Hickory Creek Plan: CR 1050 | | 0.20 | 915.09 565.49 | 3.21 | 5.10 | | 337.03 | 524.90 | 2933 | TOU-TR UM PRUP | 1500 BR U |
| Geom: CR1050 @ Hickory Ck-Prop Geom2 Flow: CR | Ge | 0.12 | 267.47 32.00 | 2.13 | 2.13 | 0.000381 | 334.56 | 324.44 | 569 | 2-Yr OM EXIST | 1500 BR D |
| River = Hickory Ck Reach = Reach | | 0.05 | 649.75 126.89 979.08 563.45 | 0.88 3.00 | 1.00 3.06 | 0.000031 | 334.53 336.88 | 324.47 324.44 | 569 2933 | 2-Yr OM PROP 100-Yr OM EXIST | 1500 BR D 1500 BR D |
| <u>۔</u> .055 <u>،</u> .02 .02 | | 0.19 | 935.60 381.55 | 3.13 | 3.68 | | 336.83 | 324.47 | 2933 | 100-Yr OM PROP | |
| 350- | 350 | 0.11 | 401.00 157.40 | 1 40 | 1 0.4 | 0.000215 | 774 57 | 704 44 | 500 | | 1 4 4 4 |
| | | 0.11 | 401.00153.48711.75205.15 | 1.42 | 1.84 0.91 | 0.000215 | 334.57 334.53 | 324.44 324.47 | 569 569 | 2-Yr OM EXIST 2-Yr OM PROP | 1444 |
| | 2.44 | 0.28 | 1351.14 606.10 | 2.17 | 5.01 | 0.001136 | 336.88 | 324.44 | 2933 | 100-Yr OM EXIST | 1444 |
| 345 | 34: | 0.15 | 1588.54 601.15 | 1.85 | 2.59 | 0.000140 | 336.83 | 324.47 | 2933 | 100-Yr OM PROP | 1444 |
| | | 0.39 | 137.81 77.94 | 4.13 | 5.22 | 0.006138 | 334.02 | 325.79 | 569 | 2-Yr OM EXIST | 1270 |
| B40 - N. | 340 | 0.39 | 137.81 77.94 | 4.13 | 5.22 | 0.006138 | 334.02 | 325.79 | 569 | 2-Yr OM PROP | 1270 |
| | (#) | 0.47 | 958.52 670.41 958.52 670.41 | 3.06 | 7.50 | 0.008370 | <u>336.37</u> 336.37 | 325.79 325.79 | 2933 2933 | 100-Yr OM EXIST 100-Yr OM PROP | <u>1270</u> 1270 |
| | uo | | | | | | | | | | |
| 335- | Elevation 33 | 0.16 | 257.39 73.46 | 2.21 | 2.25 | 0.000720 | 333.67 | 325.34 | 569 | 2-Yr OM EXIST | 905 |
| | Elev | 0.16 | 257.39 73.46 1471.44 774.15 | 2.21 | 2.25 | 0.000720 | 333.67 336.50 | 325.34 325.34 | 569 2933 | 2-Yr OM PROP 100-Yr OM EXIST | <u>905</u> 905 |
| 330- | _ | 0.08 | 1471.44 774.15 | 1.99 | 1.38 | 0.000170 | 336.50 | 325.34 | 2933 | 100-Yr OM PROP | 905 |
| | 550 | 0.21 | 208.21 72.73 | 2.73 | 3.01 | 0.001391 | 333.38 | 325.10 | 569 | 2-Yr OM EXIST | 663 |
| 1 | | 0.21 | 208.21 72.73 | 2.73 | 3.01 | 0.001391 | 333.38 | 325.10 | 569 | 2-Yr OM EXIST 2-Yr OM PROP | 663 |
| 325- | 32 | 0.32 | 1314.95 756.45 | 2.23 | 5.48 | 0.002778 | 336.27 | 325.10 | 2933 | 100-Yr OM EXIST | 663 |
| | | 0.32 | 1314.95 756.45 | 2.23 | 5.48 | 0.002778 | 336.27 | 325.10 | 2933 | 100-Yr OM PROP | 663 |
| 220 | 201 | 0.25 | 180.81 68.11 | 3.15 | 3.44 | 0.001742 | 332.94 | 325.02 | 569 | 2-Yr OM EXIST | 406 |
| 0 200 400 600 800 | 320 | 0.25 | 180.81 68.11 | 3.15 | 3.44 | 0.001742 | 332.94 | 325.02 | 569 | 2-Yr OM PROP | 406 |
| | | 0.32 | <u>1307.40</u> 742.69 1307.40 742.69 | 2.24 | 5.35 5.35 | 0.002508 | 335.70 335.70 | 325.02 325.02 | 2933 2933 | 100-Yr OM EXIST 100-Yr OM PROP | <u>406</u> 406 |
| Station (ft) | | | | | | | | | | | |
| PROPOSED UPSTREAM BRID | 1 | 0.30 | 149.64 37.27 | 3.80 | 4.03 | 0.002858 | 332.37 | 324.06 | 569 569 | 2-Yr OM EXIST | <u>180</u> 180 |
| TRUPUSED UPSIKEAM BRIL | I | 0.30 | <u>149.64</u> <u>37.27</u> 1173.45 <u>696.76</u> | 3.80 | 4.03 | 0.002858 | 332.37 335.28 | 324.06 324.06 | 569 2933 | 2-Yr OM PROP 100-Yr OM EXIST | 180 |
| | | 0.31 | 1173.45 696.76 | 2.50 | 5.07 | 0.002572 | 335.28 | 324.06 | 2933 | 100-Yr OM PROP | 180 |
| | | 0.36 | 134.55 50.03 | 4.23 | 4.56 | 0.004301 | 331.73 | 323.38 | 569 | 2-Yr OM EXIST | 16 |
| | | 0.36 | 134.55 50.03 | 4.23 | 4.56 | 0.004301 | 331.73 | 323.38 | 569 | 2-Yr OM PROP | 16 |
| | | 0.38 | 1059.70 754.59 | 2.77 | 6.20 | 0.004301 | 334.71 | 323.38 | 2933 | 100-Yr OM EXIST | 16 |
| | | 0.38 | 1059.70 754.59 | 2.77 | 6.20 | 0.004301 | 334.71 | 323.38 | 2933 | 100-Yr OM PROP | 16 |

CADD_RSC\TABLES\TXDOT.TBL M:\PROJECTS\TxDOT\CR1050_CR1060\ PEN TABLE:

s\T

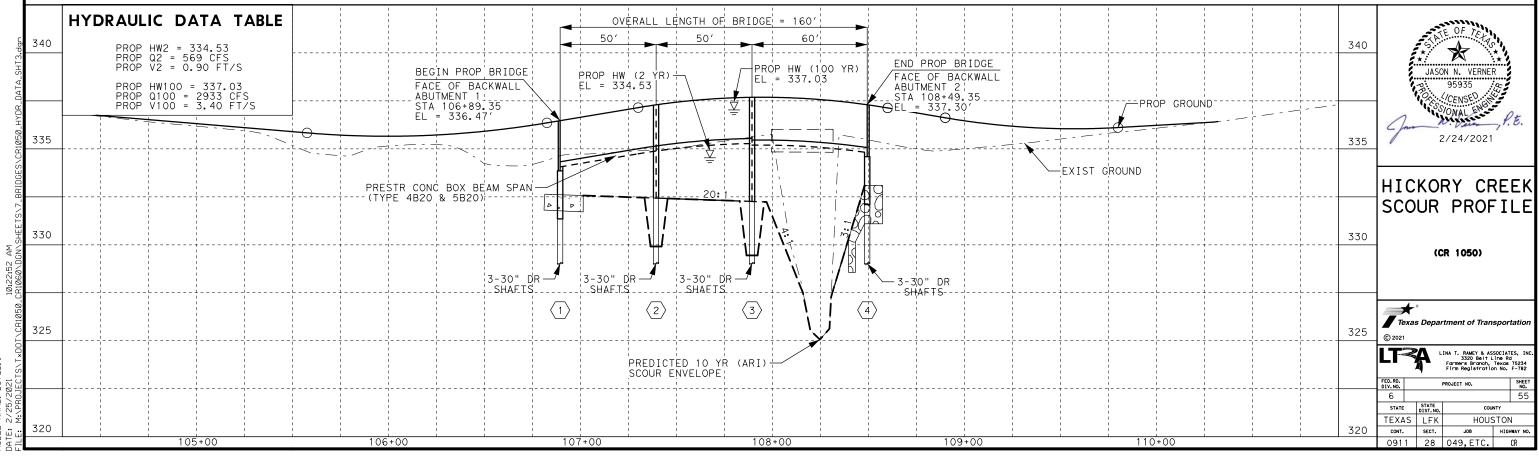
De [02] MODEL DATE:





PREDICTED SCOUR RESULTS

| SCOUR ANALYSIS - 10 YR (DESIGN | FLOW) | SCOUR ANALYSIS - 25 YR (CHECK | FLOW) |
|---|----------------------|---|----------------------|
| CONTRACTION SCOUR: PRESSURE FLOW (EQUS. 6.14) D50 = 0.2 MM (ASSUMED) K1 = 0.69 | | CONTRACTION SCOUR: PRESSURE FLOW (EQUS. 6.14) D50 = 0.2 MM (ASSUMED) K1 = 0.69 | |
| RIGHT BANK: | 0.00 0.00 0.00 | SCOUR DEPTH (FT) MAIN CHANNEL: RIGHT BANK: LEFT BANK: | 0.00 0.00 0.00 |
| PIER SCOUR: (EQUS, 7.1) | | PIER SCOUR: (EQUS. 7.1) | |
| | 2.52 2.81 | SCOUR DEPTH (FT) BENT 2 BENT 3 | 2.86 3.14 |



5) ACCORDING THE TXDOT GEOTECHNICAL MANUAL, THE SCOUR DESIGN FOR 2-YEAR HYDRAULIC DESIGN IS 10-YEAR DESIGN AND 25-YEAR CHECK.

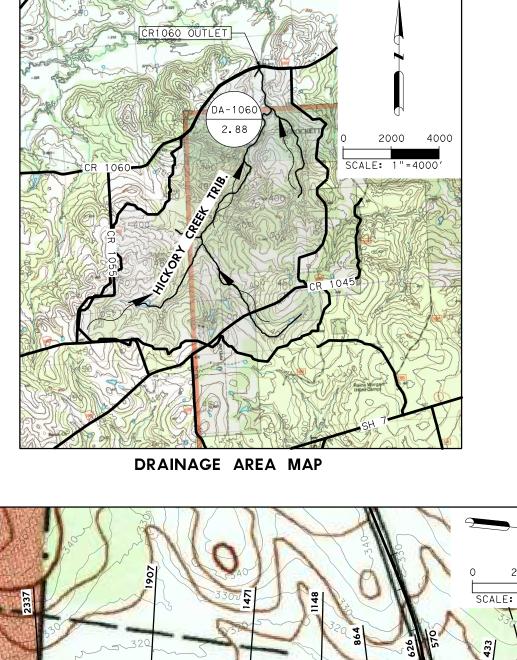
25

SCALE: 1"=50'

0

50

- 4) REFER TO THE H&H REPORT "HYDROLOGIC/HYDRAULIC/SCOUR REPORT FOR HICKORY CREEK" FOR ADDITIONAL INFORMATION.
- SEE DESIGN LAYOUT FOR DRILLED SHAFT DEPTHS AND GEOTECHNICAL DATA.
- 2) BRIDGE ABUTMENTS TO BE ARMORED FOR SCOUR PROTECTION.
- NOTES: 1) FHWA-HYDRAULIC TOOLBOX 4.4 (HEC-18, 5TH EDITION EQUATIONS) UTILIZED FOR THE ANALYSIS.



OMEGA EM REGRESSION EQUATIONS DA I.D. CR1060 CONTRIBUTING DRAINAGE AREA, (SQ.MI) 2.88 MEAN ANNUAL PRECIPITATION (IN) 46 MAIN CHANNEL SLOPE (FT/FT) 0.0066 OMEGA EM -0.127 PEAK FLOWRATE (2-YR), (CFS) 422 PEAK FLOWRATE (5-YR), (CFS) 788 PEAK FLOWRATE (10-YR), (CFS) 1,037 PEAK FLOWRATE (25-YR), (CFS) 1,431 PEAK FLOWRATE (50-YR), (CFS)* 1,760* PEAK FLOWRATE (100-YR), (CFS) 2,148 PEAK FLOWRATE (500-YR), (CFS) 3,212 * - DESIGN YEAR

1) CALCULATION ARE BASED ON THE TXDOT HYDRAULIC DESIGN MANUAL (SEPTEMBER 2019) PROCEDURES.

2) THIS SITE IS DESIGNATED AS A ZONE "A" AS SHOWN ON PANEL 48225C0250D, EFFECTIVE DATE: APRIL 4,2011.

3)ALL ELEVATIONS BASED ON THE NAVD88 VERTICAL DATUM.

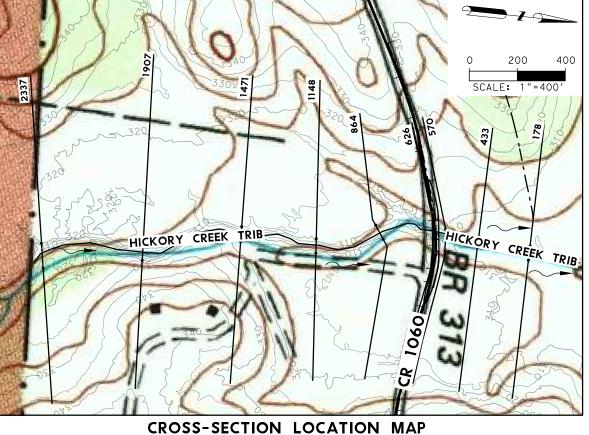
4) THE DOWNSTREAM BOUNDARY CONDITION WAS ESTABLISHED USING NORMAL DEPTH WITH A SLOPE = 0.0066 FT/FT.

5) TOPOGRAPHY DATA UTILIZED FROM TNRIS USGS16-1M-NECHES-BASIN DEMS.

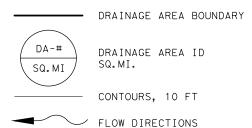
6) H&H FILES WERE SENT TO THE LOCAL FLOODPLAIN ADMINISTRATOR JIM L. LOVELL ON OCTOBER 16,2020.7) HEC-RAS √5.0.7 WAS UTITLIZED FOR HYDRAULIC DESIGN.



MOLL MAMEL OF AUTY DATE: 2/25/2021 FILE: M:VPROJECTSNT×DOTVCR1050_CR1060/DGN\SHEETS\7_BRIDGES\CR1060_HYDR_DATA_SHT1.dgn



<u>LEGEND</u>



2019) PROCEDURES. EFFECTIVE DATE: APRIL 4,2011.

| 2) PROP | | | | | | |
|------------|----------------------|----------------------|--------------------|--|--|---------------------------|
| ega/Snyder | | | | | | |
| > | Levend | | | | | |
| | Legend | | | | | |
| | WS 100 YR OM - EXIST | | | | | |
| | WS 100 YR OM - PROP | | | | | |
| | WS 50 YR OM - EXIST | | | | | |
| | WS 50 YR OM - PROP | | جمعہ جمعہ | E OF TALL | | |
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| 2000 | 2500 | | | (SHEET 1 | OF | 2) |
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| | | © 2021 | | | | |
| | | | A ' | INA T. RAMEY & AS 3320 Beiti Farmers Branch, Firm Registratio | SOCIATE ine Rd Texas on No. I | S, INC. 75234 F-782 |
| | | FED. RD. DIV. NO. | | PROJECT NO. | | SHEET NO. |
| | | 6 STATE | STATE DIST. NO. | cou | NTY | 56 |
| | | TEXAS | LFK | HOUS | | |
| | | CONT. | SECT. | JOB | | WAY NO. |
| | | 0911 | 28 | 049,ETC. | (| R |

| | <u></u> | | | | | | | | | D . |
|---|---|---|---|---|---|---|--|-------|---|--|
| | | | | | | | | Plan | Profile | River Sta |
| | | | | | | | | EVIST | 50 VR OM | 2337 |
| | 603 51 | | | | 316.32 | 303.97 | | | 50 YR OM | 2337 |
| | | 3 05 | | | | | | | | 2337 |
| 88.35 | 695.54 | 3.09 | 3.09 | 0.000924 | 317.38 | 303.97 | 2148 | PROP | 100 YR OM | 2337 |
| E 4 00 | 776 60 | E 07 | - 07 | 0.007004 | 745 77 | 700.70 | 4700 | EVICE | | 4007 |
| | | | | | | | | | | 1907 1907 |
| | | 5.37 | 5.37 | | | | | | | 1907 |
| 58.30 | 391.44 | 5.49 | 5.49 | 0.003901 | 316.31 | 302.76 | 2148 | PROP | 100 YR OM | 1907 |
| 45 11 | 740 41 | E 17 | E 17 | 0.007140 | 717 01 | 701 70 | 1760 | EVICE | | 1471 |
| | | 5 29 | | 0.003140 | | | | | | 1471 |
| | 392.74 | 5 47 | 5 47 | | | | | | | 1471 |
| | | | | | | | | | | 1471 |
| 10.15 | 515.52 | 5.00 | 5.00 | 0.003331 | 514.04 | 301.32 | 2140 | T NOT | | 1771 |
| 40.97 | 273.35 | 6.44 | 6.44 | 0.005697 | 312.23 | 300.01 | 1760 | EXIST | 50 YR OM | 1148 |
| | | | | | | | | | | 1148 |
| | | | | | | | | | | 1148 |
| 49.30 | 294.31 | (.30 | 1.34 | 0.007105 | 312.69 | 300.01 | 2148 | PROP | 100 YR OM | 1148 |
| 74 57 | 317 12 | 5 55 | 5 62 | 0 004546 | 310 90 | 299 73 | 1760 | FXIST | 50 YR OM | 864 |
| | | 7,00 | | | | | | | | 864 |
| | | | | | | | | | | 864 |
| 71.11 | 298.48 | 7.20 | 7.25 | 0.007952 | 310.57 | 299.73 | 2148 | PROP | 100 YR OM | 864 |
| | | | | | | | | | | |
| 70.55 | | 5.97 | | | | | | | | 626 |
| | | | | | | | | | | 626 |
| | 486.81 | | | 0.003093 | | | | | | 626 626 |
| | 0-0.00 | 5.57 | JU | 0.000702 | 510.00 | | | 11101 | | 020 |
| 32.43 | 261.74 | 6.72 | 6.72 | 0.006730 | 309.71 | 297.24 | 1760 | EXIST | 50 YR OM | 600 BR U |
| 125.10 | | | | | | | | | 50 YR OM | 600 BR U |
| 65.24 | 276.21 | | | | | | | | 100 YR OM | 600 BR U 600 BR U |
| | 510.20 | 1.13 | 5.00 | 0.001314 | | | 2170 | 1101 | | |
| 32.43 | 296.16 | 5.94 | 5.94 | 0.004654 | 309.72 | 297.54 | 1760 | EXIST | 50 YR OM | 600 BR D |
| 124.59 | 495.16 | 3.55 | 4.52 | 0.000445 | 309.58 | 297.54 | 1760 | PROP | 50 YR OM | 600 BR D |
| | | 6.91 | 6.91 | 0.010098 | 310.56 | | | | | 600 BR D |
| | 580.65 | 3.70 | 4.85 | 0.001109 | 310.50 | 297.54 | 2148 | PROP | 100 YR OM | 600 BR D |
| 67.49 | 321.51 | 5.47 | 5.56 | 0.002435 | 309.64 | 297.54 | 1760 | EXIST | 50 YR OM | 570 |
| 146.77 | 513.56 | | | | | | | PROP | | 570 |
| 139.27 | 371.57 | | | 0.002561 | | | | | | 570 |
| 228.79 | 687.17 | 3.13 | 4.54 | 0.000398 | 310.51 | 297.54 | 2148 | PROP | 100 YR OM | 570 |
| 76.00 | 267 47 | 6 50 | 6 50 | 0.005410 | 700 01 | 206.07 | 1700 | ГУТСТ | | 127 |
| | | | | 0.005416 | | | | | | 433 433 |
| 150.59 | | | | | | | | | | 433 |
| 150.57 | | | | | 309.73 | | | | | 433 |
| | | | | | | | | | | |
| 56.35 | 267.44 | 6.58 | 6.60 | 0.006606 | 307.61 | 296.77 | 1760 | EXIST | 50 YR OM | 178 |
| | | | | | | | | | | <u>178</u> 178 |
| | | | | | | | | | | 178 |
| | 000.01 | 0.00 | 0.00 | 5.00001 | 000.00 | | | 11101 | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| $\begin{array}{c} \\ \\ 1 \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ $ | (f+) 86.1 85.91 88.61 88.31 54.02 53.61 58.31 45.1 44.55 50.01 44.55 50.01 44.55 50.01 40.99 39.71 59.71 40.55 108.01 71.1 70.51 172.02 261.82 280.2 32.41 125.1 65.21 32.41 124.51 67.44 124.51 67.44 124.51 67.42 128.21 32.47 124.51 67.42 128.21 36.91 3150.51 150.51 | $\begin{array}{c} (sq f+) & (f+) \\ 608.48 & 86.1 \\ 603.51 & 85.9 \\ 704.06 & 88.60 \\ 695.54 & 88.3 \\ \hline \\ 336.69 & 54.0 \\ 331.78 & 53.6 \\ 399.90 & 58.80 \\ 391.44 & 58.30 \\ \hline \\ 399.90 & 58.80 \\ 391.44 & 58.30 \\ \hline \\ 340.41 & 45.1 \\ 332.56 & 44.5 \\ 392.74 & 50.00 \\ 379.32 & 48.7 \\ \hline \\ 273.35 & 40.9 \\ 257.93 & 39.7 \\ 326.29 & 59.7 \\ 294.31 & 49.30 \\ \hline \\ 273.35 & 40.9 \\ 257.93 & 39.7 \\ 326.29 & 59.7 \\ 294.31 & 49.30 \\ \hline \\ 317.12 & 74.5 \\ 251.40 & 61.5 \\ 402.23 & 108.0 \\ 294.31 & 49.30 \\ \hline \\ 317.12 & 74.5 \\ 251.40 & 61.5 \\ 402.23 & 108.0 \\ 294.31 & 49.30 \\ \hline \\ 295.01 & 70.5 \\ 464.06 & 172.0 \\ 486.87 & 261.8 \\ 643.55 & 280.2 \\ \hline \\ 261.74 & 32.4 \\ 437.78 & 125.1 \\ 276.21 & 65.2 \\ \hline \\ 296.16 & 32.4 \\ 495.16 & 124.5 \\ 310.73 & 580.65 \\ \hline \\ \hline \\ 296.16 & 32.4 \\ 495.16 & 124.5 \\ 310.73 & 580.65 \\ \hline \\ \hline \\ 296.16 & 32.4 \\ 495.16 & 124.5 \\ 310.73 & 580.65 \\ \hline \\ \hline \\ 296.16 & 32.4 \\ 495.16 & 124.5 \\ 310.73 & 580.65 \\ \hline \\ \hline \\ 267.47 & 36.9 \\ 267.47 & 36.9 \\ 343.31 & 150.5 \\ 343.28 & 150.5 \\ \hline \\ 267.44 & 56.3 \\ 361.04 & 210.3 \\ \hline \end{array}$ | Vel Total Flow Area Top Wide (ft/s) $(sq ft)$ (ft) 2.89 608.48 86.1 2.92 603.51 85.99 3.05 704.06 88.60 3.09 695.54 88.39 5.23 336.69 54.00 5.30 331.78 53.66 5.37 399.90 58.89 5.49 391.44 58.30 5.47 392.74 50.00 5.66 379.32 48.79 6.44 273.35 40.99 6.82 257.93 39.77 6.58 326.29 59.77 7.30 294.31 49.30 5.55 317.12 74.50 7.00 251.40 61.55 5.34 402.23 108.00 7.20 298.48 71.1 5.97 295.01 70.51 3.79 464.06 172.0 4.41 486.87 261.8 | Vel ChniVel TotalFlow Area Top Wi (ft/s) $(1+/s)$ $(sq ft)$ (rt) 2.892.89608.4886.12.922.92603.5185.933.053.05704.0688.613.093.09695.5488.335.235.23336.6954.015.305.30331.7853.665.375.37399.9058.835.495.49391.4458.315.175.17340.4145.15.295.29332.5644.55.475.47392.7450.015.665.66379.3248.716.446.44273.3540.996.826.82257.9339.716.736.58326.2959.777.347.30294.3149.315.625.55317.1274.557.007.00251.4061.555.635.97295.0170.575.644.41486.87261.84.963.34643.55280.26.726.72261.7432.445.424.02437.78125.17.787.787.787.257.837.15518.206.586.58267.475.645.47321.566.916.91310.734.4523.55495.165.645.82267.476.586.58267.476.58 | E. G. Slope Vel Chni Vel Total Flow Area Top Wi (ft/ft) (ft/s) (sq ft) (ft) 0.000928 2.89 2.89 608.48 86.1 0.000952 2.92 2.92 603.51 85.99 0.000991 3.05 3.05 704.06 88.63 0.000924 3.09 3.09 695.54 88.33 0.003924 5.23 5.23 336.69 54.00 0.004084 5.30 5.30 331.78 53.66 0.003680 5.37 5.37 399.90 58.89 0.003901 5.49 5.49 391.44 58.31 0.003335 5.29 5.29 332.56 44.5 0.003304 5.47 5.47 392.74 50.00 0.003591 5.66 5.66 379.32 48.71 0.005697 6.44 6.44 273.35 40.9 0.005697 6.44 6.44 273.35 40.9 0.005602 6.73 6.58 326.29 59.7 0.005702 6.73 6.58 326.29 59.7 0.005702 6.73 6.58 326.29 59.7 0.005702 6.73 6.58 326.29 59.7 0.0007105 7.34 7.30 294.31 49.37 0.005702 6.73 6.58 326.29 59.7 0.007105 7.25 7.20 28.48 71.1 0.00351 5.64 4.41 486.87 261.8 0.00393 5.64 4.41 486.87 261.8 0.0007952 7.25 7.20 298.48 71.1 0.003051 6.05 5.97 295.01 70.57 0.003051 6.05 5.97 295.01 70.57 0.003051 6.05 5.97 295.01 70.57 0.003051 6.05 5.97 295.01 70.57 0.003093 5.64 4.41 486.87 261.8 0.000795 7.25 7.20 298.48 71.1 0.003051 6.05 5.97 295.01 70.57 0.000659 5.35 3.79 464.06 172.C 0.003093 5.64 4.41 486.87 261.8 0.000492 4.96 3.34 643.55 280.2 0.000730 6.72 6.72 261.74 32.44 0.000679 5.42 4.02 437.78 125.1 0.001574 5.68 4.15 518.20 0.000730 6.72 6.72 261.74 32.44 0.000454 5.94 5.94 296.16 32.47 0.001574 5.68 4.15 518.20 0.000492 4.96 3.37 6.58 267.47 36.97 0.005416 6.58 6.58 267.47 36.97 0.006607 6.60 6.58 267.43 56.33 0.006607 6.60 6.58 267.44 56.33 0.006607 6.60 6.58 267.43 56.33 0.006607 6.60 6.58 267.44 56.33 0.006607 6.60 6.58 267.43 56.33 0.006607 6.60 6.58 267.44 56.33 0.006607 6.60 6.58 267.44 56.33 0.006607 6.60 6.58 267.44 256.33 0.006607 6.60 6.58 267.44 256.33 0.006 | W.S. Elev[E.G. Slope] Vel Chni Vel Total Flow Area Top Wights (ft) (ft/rft) (ft/rs) (ft/s) (ft/s) (ft/s) 316.38 0.000928 2.89 2.89 608.48 86.1 317.48 0.000924 3.09 3.05 3.05 704.06 88.61 317.38 0.000324 5.23 5.23 336.69 54.00 315.24 0.004084 5.30 531.73 399.90 58.81 316.45 0.003600 5.37 5.37 399.90 58.81 316.31 0.003304 5.47 5.47 392.74 50.00 314.91 0.003335 5.29 5.29 332.56 44.5 312.23 0.005697 6.44 6.44 273.35 40.9 312.249 0.005702 6.73 6.58 326.29 57.7 312.69 0.007105 7.34 7.30 244.31 49.33 310.90 0.004546 5.62 5.55 317.12 74.5 309.93 0.003051 6.05 < | $\begin{array}{c c c c c c c c c c c c c c c c c c c $ | | Pian Q Total Min Ch ElW, S. Elevie, G. Slope Vel Chni Vel Total Flow Ared Top Wi. (ofs) (ff) (ff) (ff)/(ff) (ff)/(ff) (ff)/(ff) (ff)/(ff)/(ff)/(ff)/(ff)/(ff)/(ff)/(ff) | Profile Plan Q. Total Min Ch ElW, S. ElevE, G. Slope Vel Chnl Vel Total Flow Area Area |

PROPOSED UPSTREAM BRIDGE CROSS-SECTION

400

600

Station (ft)

200

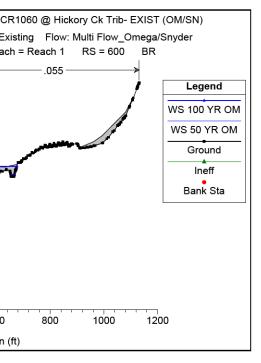
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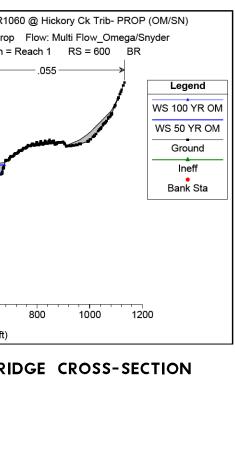
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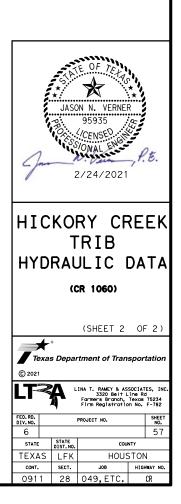
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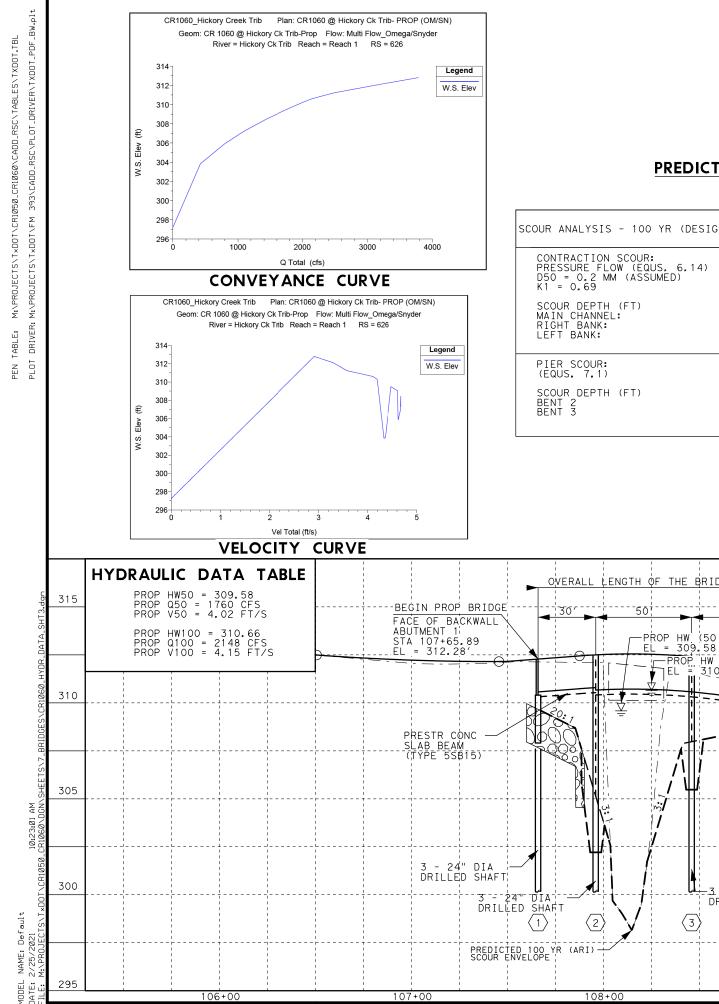
ault Def Ø21 MODEL DATE:



BRIDGE CROSS-SECTION





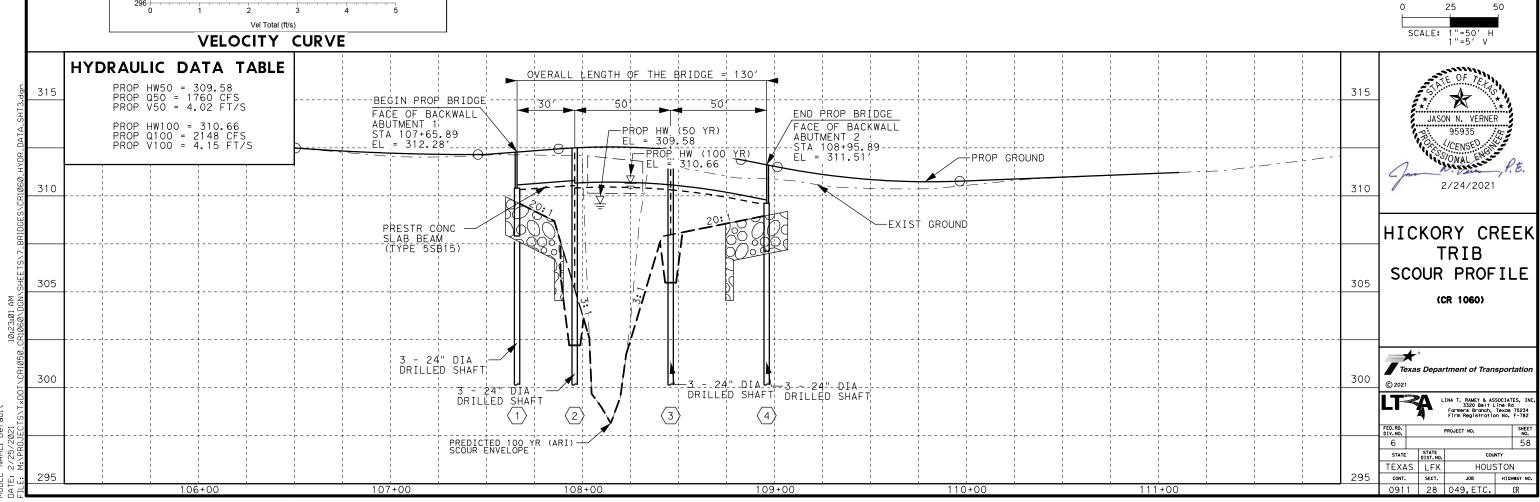


PREDICTED SCOUR RESULTS

| SCOUR ANALYSIS - 100 YR (DESIGN | FLOW) | SCOUR ANALYSIS - 200 YR (CHECK | FLOW) |
|---|----------------------|---|----------------------|
| CONTRACTION SCOUR: PRESSURE FLOW (EQUS. 6.14) D50 = 0.2 MM (ASSUMED) K1 = 0.69 | | CONTRACTION SCOUR: PRESSURE FLOW (EQUS. 6.14) D50 = 0.2 MM (ASSUMED) K1 = 0.69 | |
| SCOUR DEPTH (FT) MAIN CHANNEL: RIGHT BANK: LEFT BANK: | 0.00 0.00 0.00 | SCOUR DEPTH (FT) MAIN CHANNEL: RIGHT BANK: LEFT BANK: | 0.00 0.00 0.00 |
| PIER SCOUR: (EQUS. 7.1) | | PIER SCOUR: (EQUS. 7.1) | |
| SCOUR DEPTH (FT) BENT 2 BENT 3 | 2.93 2.50 | SCOUR DEPTH (FT) BENT 2 BENT 3 | 3.01 2.66 |

NOTES:

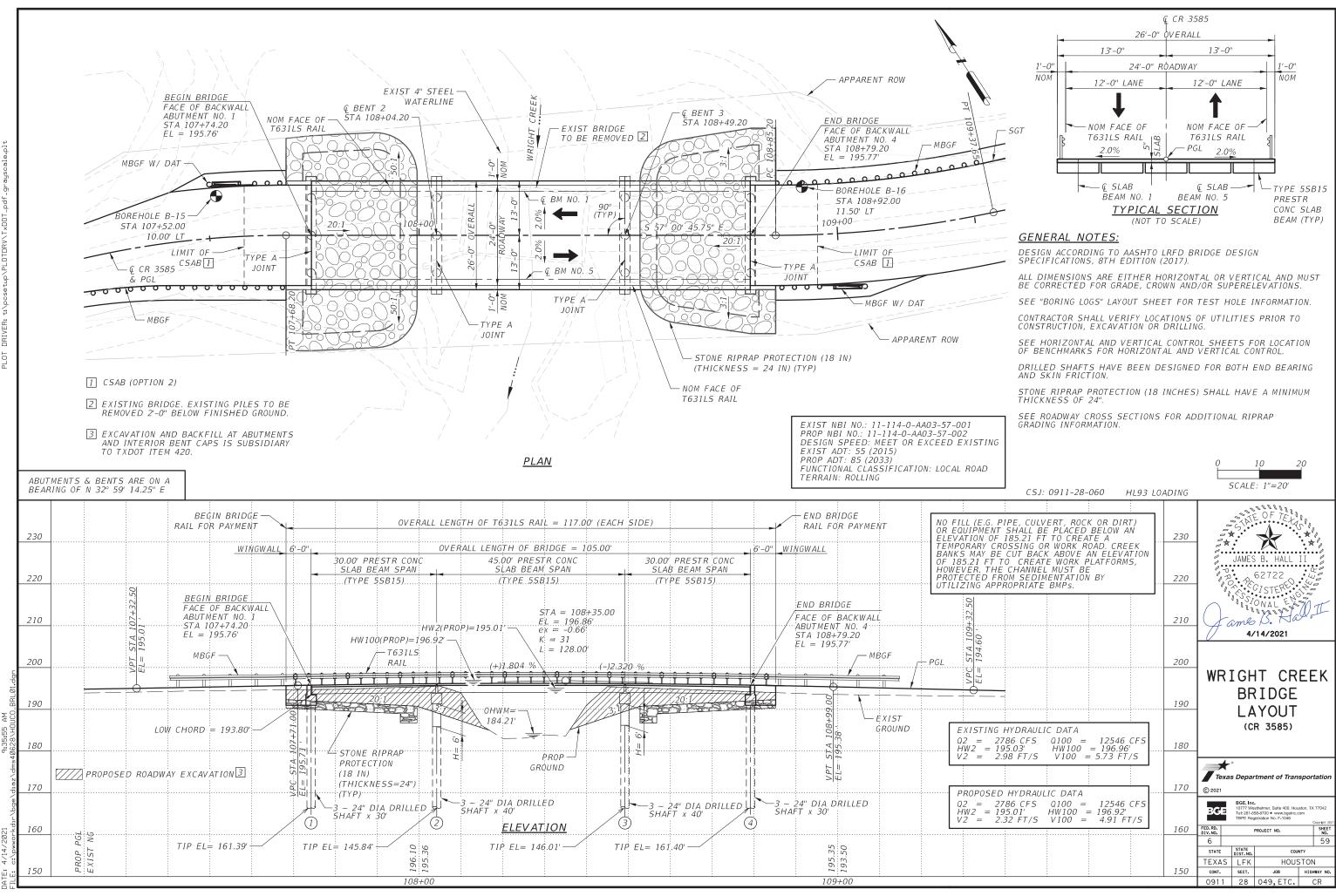
- REPORT FOR



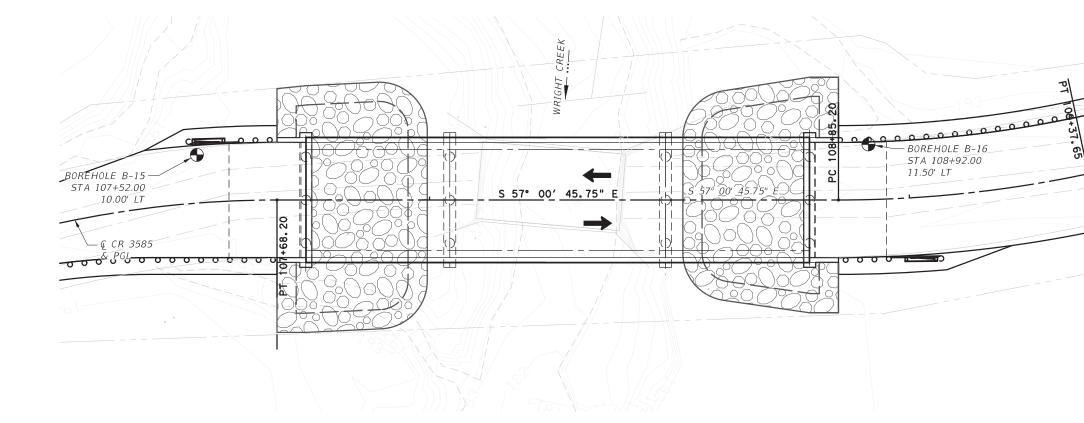
1) FHWA-HYDRAULIC TOOLBOX 4.4 (HEC-18, 5TH EDITION EQUATIONS) UTILIZED FOR THE ANALYSIS. 2) BRIDGE ABUTMENTS TO BE ARMORED FOR SCOUR PROTECTION. 3) SEE BRIDGE DESIGN LAYOUT FOR DRILLED SHAFT DEPTHS AND GEOTECHNICAL DATA. 4) REFER TO THE H&H REPORT "HYDROLOGIC/HYDRAULIC/SCOUR

HICKORY CREEK TRIB. " FOR ADDITIONAL INFORMATION.

5) ACCORDING THE TXDOT GEOTECHNICAL MANUAL, THE SCOUR DESIGN FOR 50-YEAR HYDRAULIC DESIGN IS THE 100-YEAR DESIGN AND 200-YEAR CHECK.



DEL NAME: WRIGHT CREEK BRIDGE LAYO .TE: 4/14/2021 9:35:5



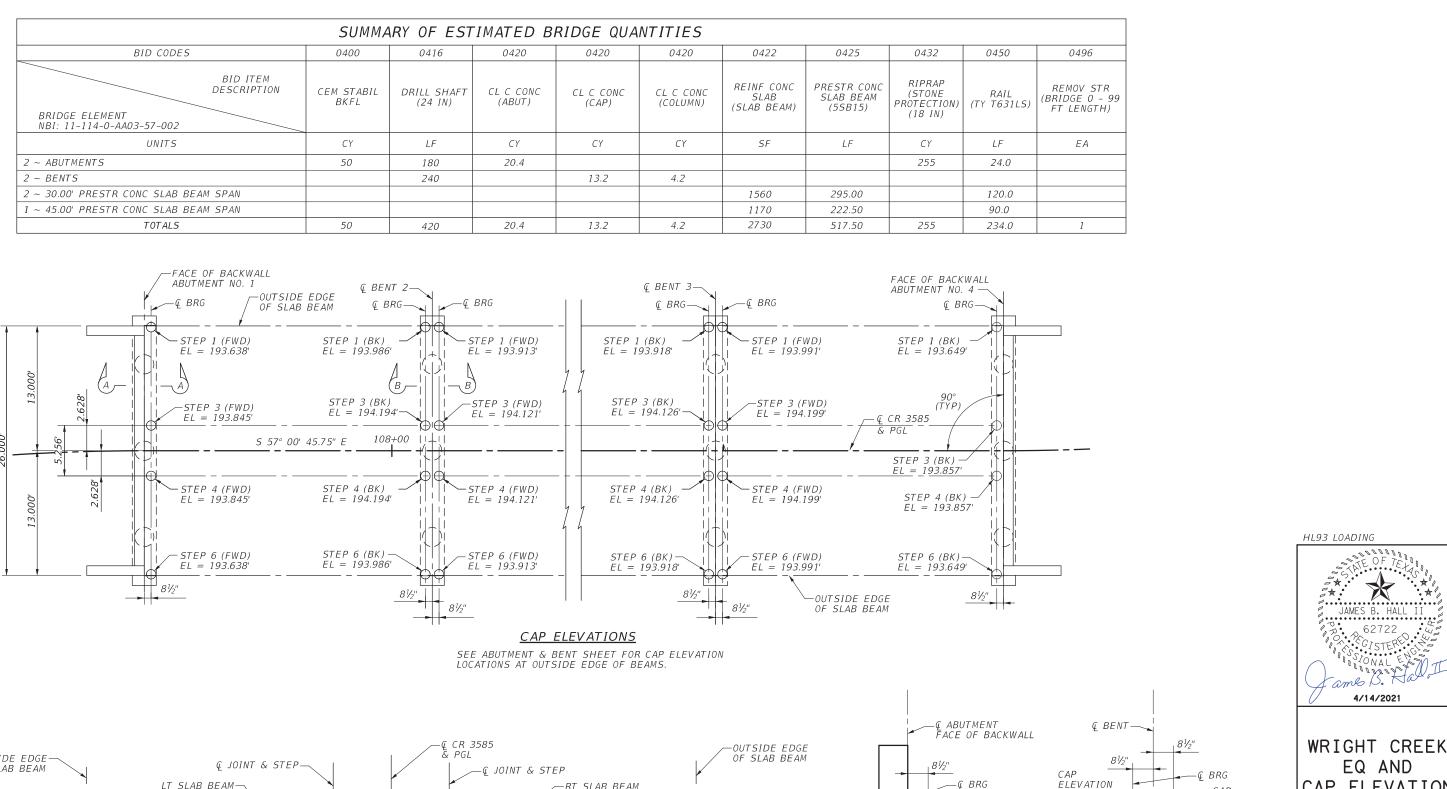
PLAN

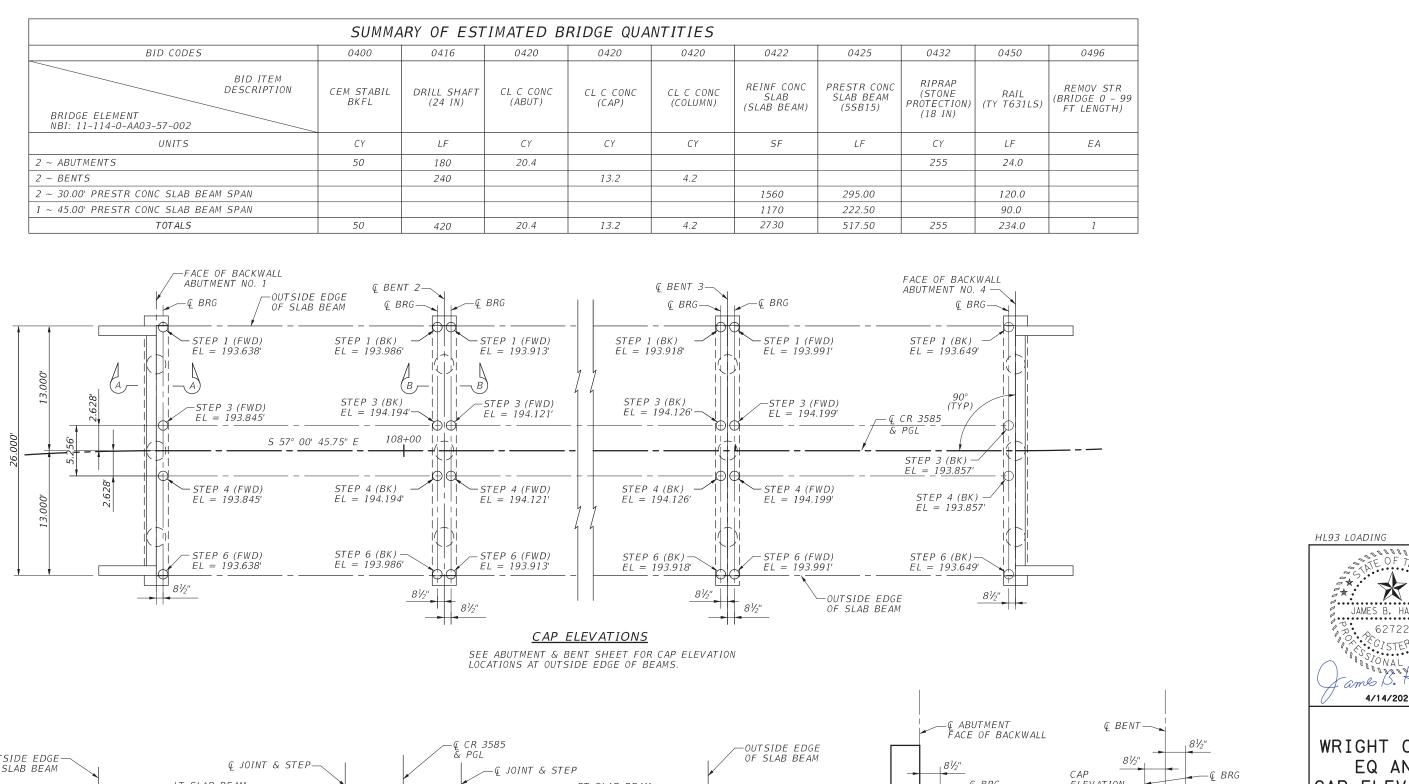
| 200 | | BOREHOLE B-15 EL= 193.70' | |
|-----|----------------|--|--|
| | | | |
| 100 | | SAND, Silty, moist, light brown to 2', light gray | FILL: SAND, Silty, moist, dark brown |
| 190 | 18(6) 20(6) | SAND, Silty, slightly compact, moist, <u>brown to light</u> | <i>FILL: SAND, Sifty, moist, dark brown</i> <i>grained.1</i> . <i>SC.with.Gravel.at.top.(Si</i> <i>CLAY, Lean, soft, moist, brown and gr</i> |
| | 8(6) 7(6) | gray, fine grained, few SC lenses (SM) | 8(6) 9(6) 4 below 8' SAND, Silty, Clayey, loose, moist, bro |
| 180 | 11(6) 20(6) | and gray below 11.5', trace ferrous nodules, trace | 11(6) 11(6) 11(6) |
| | 17(6) 21(6) | SAND, Silty, moist, light brown to 2', light gray below 3', fine grained, traces Grayel and roots to 2' (SM) SAND, Silty, slightly compact, moist, brown to light gray, fine grained, few SC lenses (SM) CLAY, Fat, soft, moist, light brown to 10', brown and gray below 11.5', trace ferrous nodules, trace gypsum below 11.5' CLAY, Fat, stiff, moist, brown, traces gypsum and ferrous nodules | 27(6) 24(6) |
| 170 | 31(6) 37(6) | CLAY, Fat, stiff, moist, dark gray | 22(6) 31(6) |
| | 32(6) 45(6) | CLAY, Fat, very stiff, moist, dark gray, 1" SC seam | 33(6) 40(6) |
| 160 | 38(6) 49(6) | at 32.3' | |
| | 40(6) 50(4.5) | CLAY, Fat, hard, moist, dark gray, trace SC seams, | 50(5.25) 50(4) |
| 150 | 43(6) 50(5.5) | trace shell fragments below 41.1' | |
| | 31(6) 30(6) | CLAY, Fat, very stiff, moist, dark gray, trace shell | 46(6) 50(5) |
| 140 | 43(6) 49(6) | fragments Sandy CH below 51.8' | CLAY, Fat, hard, moist, dark gray, tra 50(5.5) 50(3.5) below 56' |
| | 47(6) 50(5.75) | CLAY, Fat, hard, moist, dark gray, traces SC seams | 42(6) 50(5) |
| 130 | 50(4.5) 50(3) | and shell fragments | |
| | 50(3) 50(3) | CLAY, Fat with Sand to Sandy Fat, hard, moist, dark | 50(3.5) 50(2) trace SC seams |
| 120 | 50(3.5) 50(2) | | |
| n | 50(2) 50(1) | SAND, Clayey, dense to very dense, moist, dark gray, fine grained <u>ELEVATION</u> | 50(2.5) 50(3) |
| 110 | B/ | H = 114.46' | B/H = 113.77' |
| | | | |
| 100 | | | |
| | : : : | 108+00 | 109+00 |

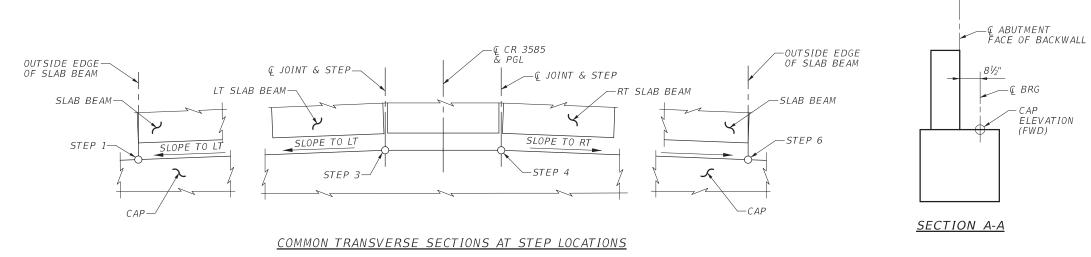
MODEL NAME: WRIGHT CREEK BORING LOGS DATE: 4/14/2021 GENERAL NOTES: BORE HOLE LOCATIONS AND ELEVATIONS APPROXIMATE FROM SURVEY DATA.

DRILLING CONTRACTOR: CORSAIR CONSULTING LLC DRILLED DATE: 12/20/19 & 12/21/19

| | - - - - - - - - - - - - - - - - - - - | | | | | | | | | |
|---------------|---|------|------|-----|--|-----|-------------|--|---------------------|----------------|
| | | | | 200 | | | | | | |
| n and 'SM) | gray, fi | ne | | 190 | l o E | | ALE: | 1"=20' H 1"=20' V | 20 | |
| | Sandy CL | - | | | | | ~~ | OF TEL | | |
| rown, | fine gra | ined | | 180 | e | | STATE | E OF TELA | \$ \$ * | |
| race | Sand sea | ams | | 170 | 111111 | PR | JAMES | 62722 | II 4 | 1 l'ano |
| ау | | | | 170 | | | | SISTEREY ONAL EN | | T. |
| ray | | | | 160 | | t a | emb | , B. Ma | <i>y</i> c 0 | |
| | | | | | \mathcal{O} | | 4/ | 14/2021 | | |
| | | | | 150 | | | | | | |
| race | SC seam | 5 | | 140 | WF | נ ר | BC | IT CR | | ΞK |
| noist, | dark gra | ΨV, | | 130 | | | | .OGS r 3585) | | |
| | | | | 120 | — — — — — — — — — — | | Depar | tment of Tran | spor | tation |
| race | SC seam | 5 | | | BG | E | Tel: 281-55 | sthelmer, Sulte 400, Hou 58-8700 ● www.bgeinc.o istration No. F-1046 | om | Copyright 2021 |
| | | | | 110 | FED.RD. DIV.NO. | | | PROJECT NO. | | SHEET NQ. |
| | • • • | | | | 6 | L | STATE | | | 60 |
| | | | | | STATE TEXA | _ | LFK | HOUS | | |
| | | | | 100 | CONT. | | SECT. | JOB | | WAY NO. |
| | | | | | 091 | _ | 28 | 049,ETC. | | CR |







ATION CAP ELEVA 3:19:50 AM AND ĒŪ CREEK

WRIGHT CREEK EQ AND CAP ELEVATION (CR 3585) Texas Department of Transportatio C 2021 BGE, Inc. 10777 Westhelmer, Sulte 400, Houston, TX 77042 Tel: 281-558-8700 • www.bgeinc.com TBPE Registration No. F-1046 BGE ED.RD. SHEE PROJECT NO. 6 STATE COUNTY DIST.N TEXAS LFK HOUSTON CONT. SECT. HIGHWAY NO JOB 0911 28 049,ETC, CR

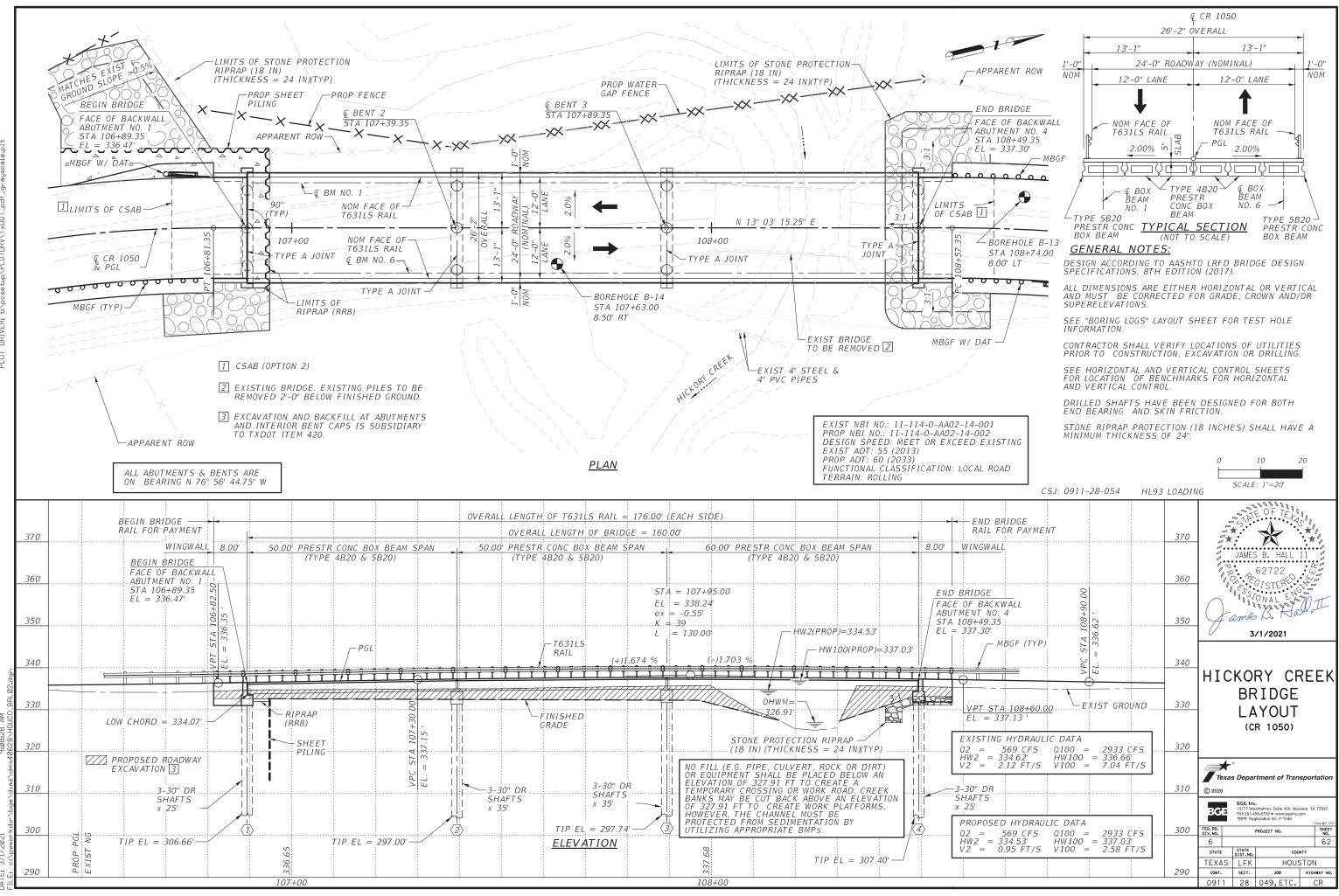
САР

(FWD)

SECTION B-B

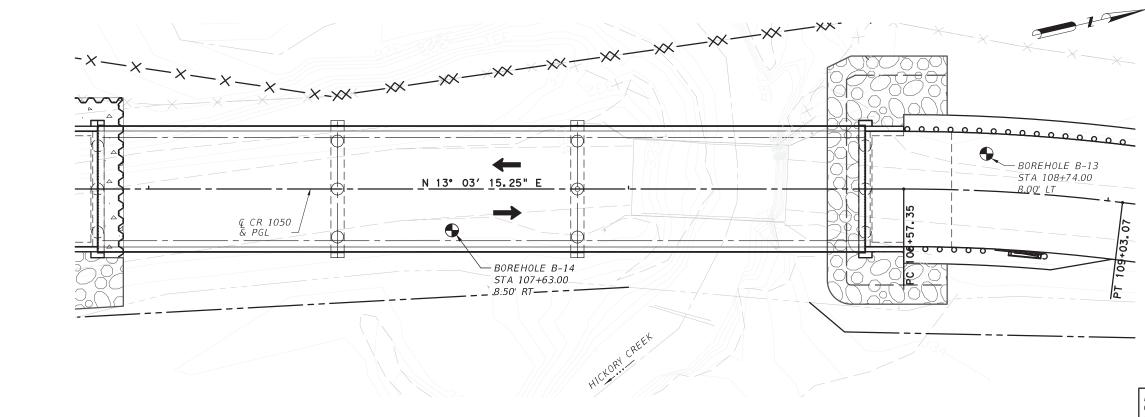
ELEVATION

(BK)



IN TABLE: pw://bge-pw.bentley.com:bge-pw-03/Documents/Standards/Tables/Pen/TXDOT.TBL OT DRIVER: t:/pcsetup/PLOTDRV/TxDOT_pdf_grayscale.plt

> JEL NAME: HICKORY CREEK BRIDGE LAYOU TE: 3/1/2021 9:08:28



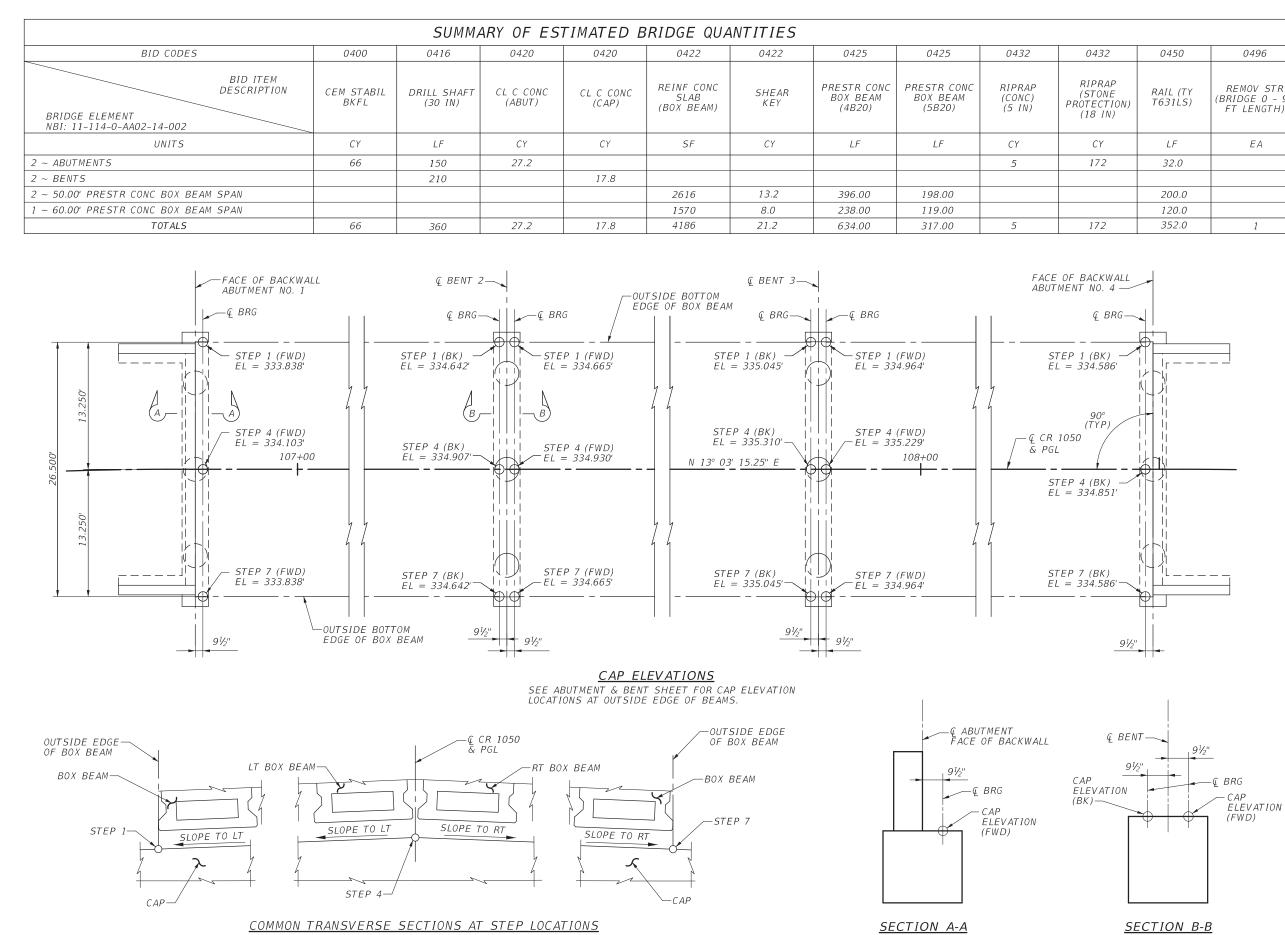
PLAN

| 340 | | | BOREHOLE B-14 EL= 334.65' T/T T T T T T T T T T T T T T T T T T T | • • • • | BOREHOLE B-13 EL= 334.72' | 340 | |
|-----|--------|---------------------------------------|--|--|--|-----|---|
| 330 | | <u> </u> | SAND, Clayey, moist, reddish brown, fine to coarse grained, few Gravel CLAY, Sandy Lean, moist, brown CLAY, Sandy Lean, stiff, moist, gray and brown, few ferrous staining | | CLAY, Lean with Sand, moist, reddish brown, traces roots and ferrous staining CLAY, Fat to Fat with Sand, soft, moist, reddish brown and gray to 5', brown below 6.5', trace to | 330 | |
| 320 | | <u>8(6)</u> 9(6) | SAND, Clayey with Gravel, loose, moist, light gray and reddish brown, fine to coarse grained Sandy CH | 22(6) 30(6) 48(6) 50(5.5) | few ferrous staining below 6.5' CLAY, Fat, very stiff, moist, dark gray, trace SM lenses | 320 | |
| 310 | | | below 12', few Gravel, traces SC lenses and ferrous staining CLAY, Fat, hard, molst, dark gray, trace SM lenses | 47(6) 50(5.75) | CLAY, Fat, hard, moist, dark gray, trace SM lenses | 310 | JAMES B. HALL I |
| 300 | | 5 <i>φ(4) 50(4.5)</i> | CLAY, Fat, very stiff, moist, dark gray, trace SM | 50(3.75) 50(2.5) 50(3.75) 50(3) | CLAY, Fat, hard, moist, dark gray | 300 | ame B. Ha |
| | | 50(2.25) 50(2.5) 50(2.25) 50(1.5) | lenses | 50(2.75) 50(3.25) | CLAY, Fat, hard, moist, dark gray trace SM lenses | 290 | 3/1/2021 |
| 290 | | 50(2.25) 50(2.5) 50(3.75) 50(3.5) | CLAY, Fat, very hard, moist, dark gray, trace SM lenses | 50(2.25) 50(2.25) 50(2) 50(3) | | 200 | HICKORY CF BORING |
| 280 | | 50(3.75) 50(3.25) 46(6) 50(3.5) | CLAY, Fat, hard, moist, dark gray to 52.3', greenish gray below 55.8', trace SC lenses below 55.8' | 50(2.5) 50(2.5) 50(2) 50(2.5) | CLAY, Fat, very hard, moist, dark gray and greenish | | LOGS (CR 1050) |
| 270 | | 50(2.25) 50(3.5) 50(3.75) 50(2.75) | CLAY, Fat, hard, moist, dark gray, trace SC lenses | <u>50(2) 50(2.5)</u> <u>50(1.75) 50(1.75)</u> | gray, trace SC lenses below 55.6 | 270 | Texas Department of Trans |
| 260 | | 50(2) 50(2) | CLAY, Fat, hard, moist, dark gray, trace SC lenses | 50(2.25) 50(2) 50(2) 50(2) | | 260 | © 2020 BCE BGE, Inc. 10777 Westhelmer, Sulte 400, Hous Tel: 281-358-3700 • www.bgeinco. TBPE Regularion No. F-1046 |
| 250 | | | = 252.35' <u>ELEVATION</u> | B/I | H = 255.00' | | FED. RG. PROJECT NO. 01V. NO. PROJECT NO. 6 STATE STATE DIST. NO. COUNT TEXAS |
| 240 | 107+00 | | 108+00 | | | 240 | CONT. SECT. JOB 0911 28 049, ETC. |

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GENERAL NOTES: BORE HOLE LOCATIONS AND ELEVATIONS APPROXIMATE FROM SURVEY DATA.

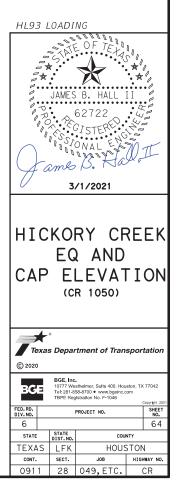
DRILLING CONTRACTOR: CORSAIR CONSULTING LLC DRILLED DATE: 12/20/19 - 12/22/19

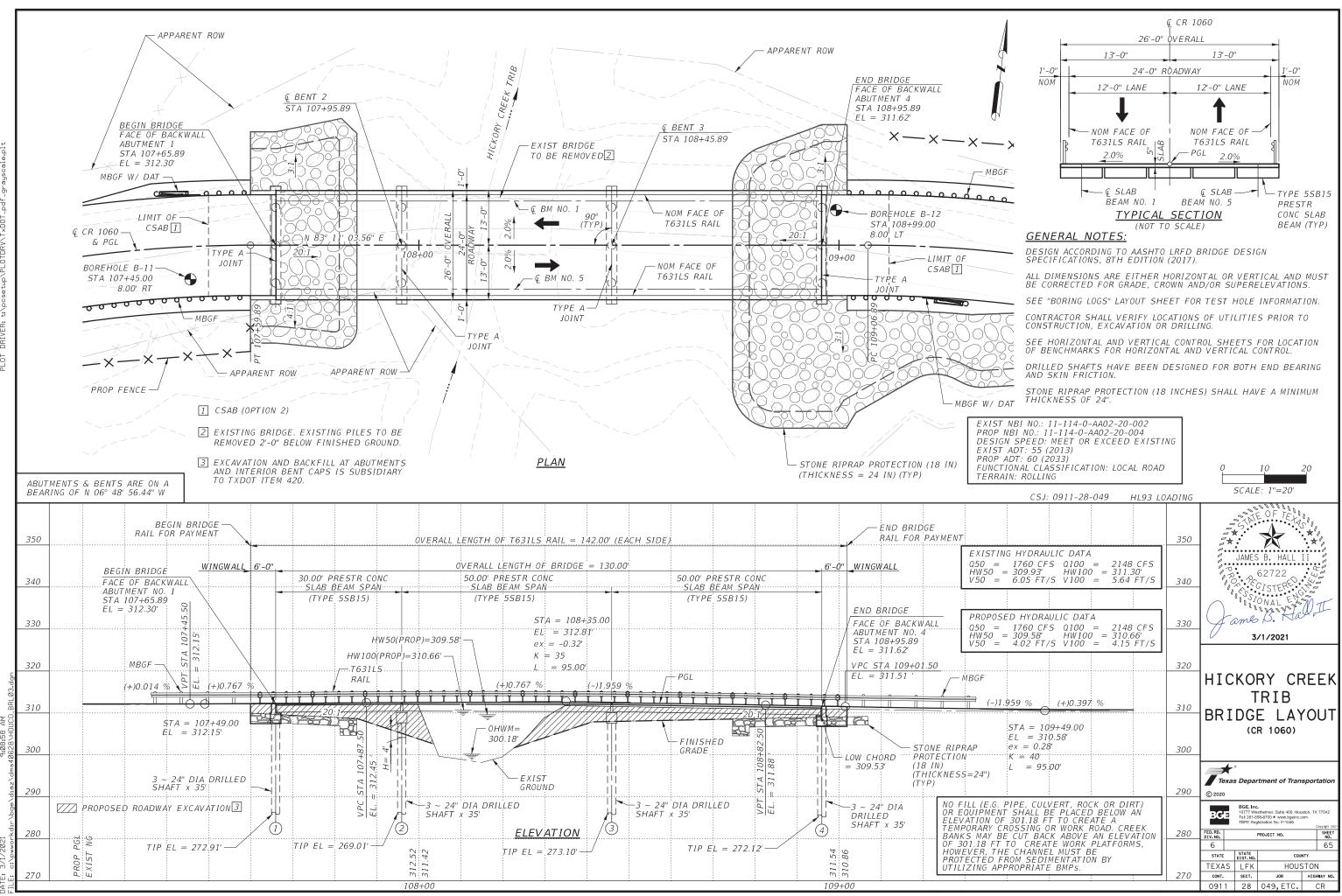


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> ELEVATION CAP AND Ē

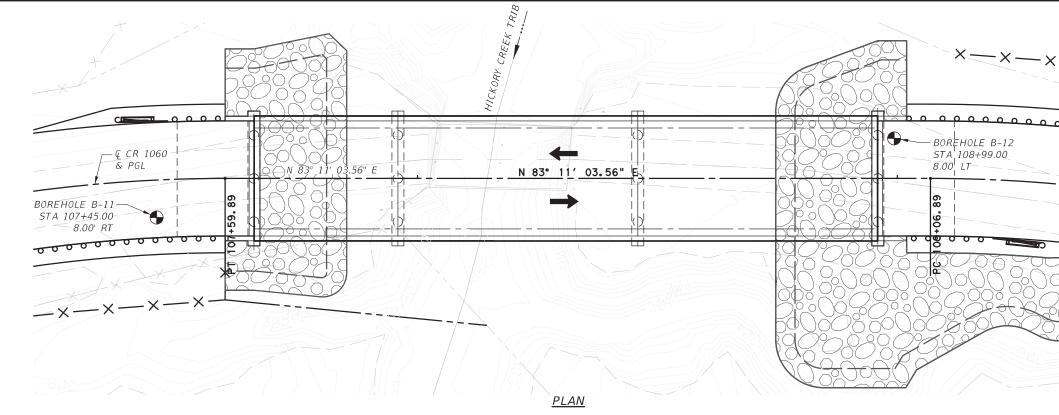
| 32 | 0450 | 0496 |
|-----------------------------|---------------------|---|
| RAP DNE CTION) IN) | RAIL (TY T631LS) | REMOV STR (BRIDGE 0 - 99 FT LENGTH) |
| Y | LF | EA |
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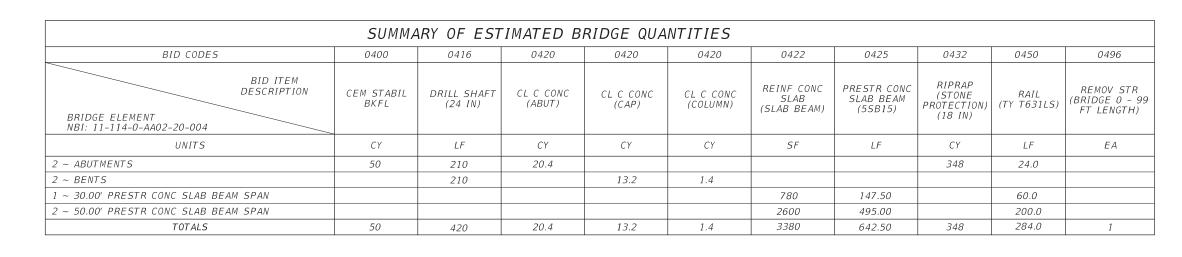


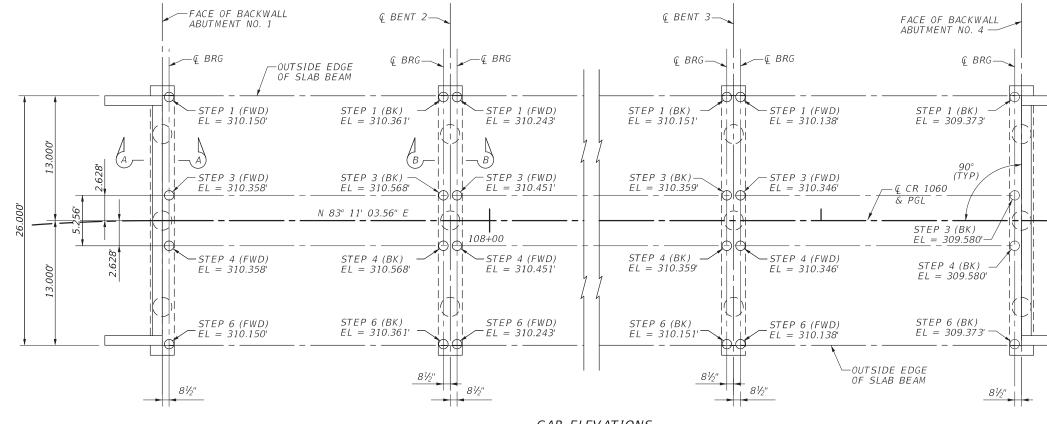
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> DEL NAME: HICKORY CREEK TRIB BRIDGE LAYOU TE: 3/1/2021 9:08:58 AM

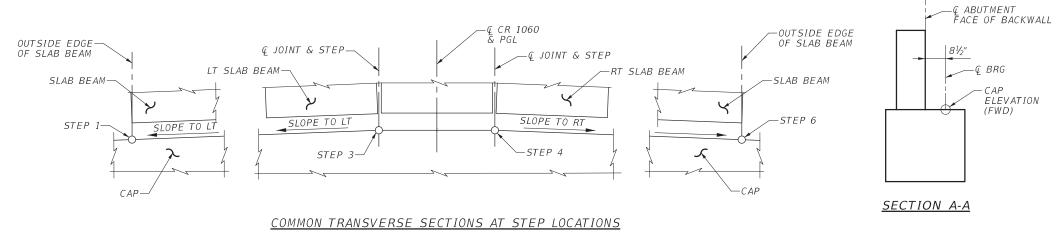


| Image: | The second seco | | CENERAL N BORE HOLE BOREHOLE B-12 STA 108+99.00 BOO' LT CONSTRUCTION C | IOTES: E LOCATIONS AND ELEVATIONS ATE FROM SURVAY DATA. |
|---|---|--|--|---|
| 310 5400 Carly F, mon Yorkin, fine to contre granded, rear University S 210 5400 Clay S, mon Yorkin, fine to contre granded, rear University S 210 | | | CORSAIR CONSULTING LLC DRILLED DATE: 12/20/19 & 1 | |
| 3(b) 6(b) and read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(b) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(c) add read/sh brown below (3, the grange), trace ferrous 3(c) 6(c) add read/sh brown below (2, the grange), trace ferrous 3(c) 5(c) add read/sh brown below (2, the grange), trace ferrous 3(c) 5(c) add read/sh brown below (2, the grange), trace ferrous 3(c) 5(c) add read/sh brown below (2, the grange), trace ferrous 3(c) 5(c) add read/sh brown (c) 5(c) 5(c) add read/sh brown, trace ferrous 5(c) 5(c) add read/sh brown, trace (c) 5(c) 5(c) add read/sh brown, trace (c) 5(c) 2 | 12(6) 18(6) 8(6) 7(6) | CLAY, Sandy Lean, stift, moist, light gray to 3', gray from 6.5' to 8', brown and gray thereafter, traces Gravel and ferrous staining | | STATE OF TELL |
| 280 CLAY, Fat with Sand, hard, moist, brown, trace Sand 3004/30(2.3) CLAY, Fat, hard, moist, dark gray, trace Sand lenses 280 A dome trace 301/2021 50(4) 50(4) 50(3.5) 50(3) 50(3.5) 50(4) 50(3.5) 50(4) 50(3.5) 50(4) 270 270 270 270 50(3.5) 50(4) 270 <td< th=""><th>290 <u>3(6)</u> 6(6) <u>36(6)</u> 39(6)</th><th>and reddish brown below 13', fine grained, trace ferrous staining below 13' CLAY, Sandy Lean, very stiff, moist, gray, few CH Ienses CLAY, Fat with Sand, very stiff, moist, brown, few </th><th>21(6) 28(6) CLAY, Sandy Fat, soft, moist, gray to 14.3', gray and brown thereafter, few Sand lenses, trace ferrous staining, 6" CL seam at 8.8' 20(6) 35(6) CLAY, Fat with Sand, very stiff, moist, dark gray, few to little Sand lenses</th><th>JAMES B. HALL II</th></td<> | 290 <u>3(6)</u> 6(6) <u>36(6)</u> 39(6) | and reddish brown below 13', fine grained, trace ferrous staining below 13' CLAY, Sandy Lean, very stiff, moist, gray, few CH Ienses CLAY, Fat with Sand, very stiff, moist, brown, few | 21(6) 28(6) CLAY, Sandy Fat, soft, moist, gray to 14.3', gray and brown thereafter, few Sand lenses, trace ferrous staining, 6" CL seam at 8.8' 20(6) 35(6) CLAY, Fat with Sand, very stiff, moist, dark gray, few to little Sand lenses | JAMES B. HALL II |
| 260 50(3) 50(2) CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 TRIB BORI 260 50(3) 50(2) 50(3.5) 50(3.25) 260 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 260 CLAY, Fat, hard, moist, dark brown, trace Sand lenses 250 250 50(2) 50(2) CLAY, Fat, hard to very hard, moist, brown, traces 250 260 <t< td=""><td>280 50(4.5) 50(4.5) 50(5) 50(3) 270</td><td>CLAY, Fat with Sand, hard, moist, brown, trace Sand Jenses</td><td>50(4) 50(2.3) CLAY, Fat, hard, moist, dark gray, trace Sand lenses 50(3.5) 50(5.5) SILT, Sandy, dense, wet, gray, few CH lenses (ML)</td><td>280 A amis D. 190 3/1/2021</td></t<> | 280 50(4.5) 50(4.5) 50(5) 50(3) 270 | CLAY, Fat with Sand, hard, moist, brown, trace Sand Jenses | 50(4) 50(2.3) CLAY, Fat, hard, moist, dark gray, trace Sand lenses 50(3.5) 50(5.5) SILT, Sandy, dense, wet, gray, few CH lenses (ML) | 280 A amis D. 190 3/1/2021 |
| 250 50(2) 50(3) CLAY, Fat, hard, moist, dark brown, trace sand renses 250 90 50(2) 50(2.75) 50(2.25) 50(2.75) 50(2.25) 50(3.5) 240 50(2) 50(2.25) 50(2) 50(1.5) 240 250 250 50(2) 50(2.25) 50(2.5) 50(2.75) 240 250 240 50(2) 50(2.25) 50(2.5) 240 240 250 50(2) 50(1.5) 240 240 240 240 | 260 50(3) 50(2) 50(3.5) 50(3.25) 50(3.5) 50(3.25) | CLAY, Fat, hard, moist, brown, trace to few Sand Ienses | <u>50(2.25) 50(2.5)</u> | LOGS |
| Sol(2) Sol(2.25) CLAY, Fat, Very hard, molst, dark brown, trace Sand I lenses | 50 50(3.75) 50(2) 50 50(2.25) 50(3.5) 50 50(2.25) 50(3.5) | CLAY, Fat, hard to very hard, moist, brown, traces Sand lenses and shell fragments | below 55.7', trace shell fragments below 65.6' | 240 Constant of Transportation |
| $\frac{50(2.5) \ 50(2)}{B/H} = 230.86$ $\frac{230 \ \overline{b} B/H}{B/H} = 230.86$ $B/H = 230.86$ | 50(2) 50(2) 50(2) 50(2) | = 232.09 ELEVATION | 50(2.5) 50(2) | 230 FED. RD. DIV. NO. PROJECT NO. SHEET. Copyright 2021 6 STATE DIST. NO. COUNTY |



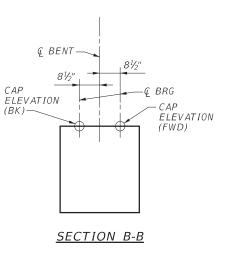


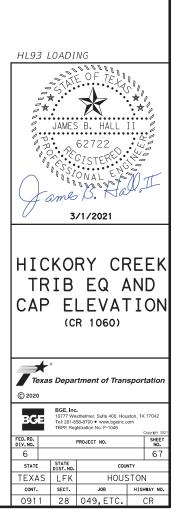
<u>CAP ELEVATIONS</u> SEE ABUTMENT & BENT SHEET FOR CAP ELEVATION LOCATIONS AT OUTSIDE EDGE OF BEAMS.

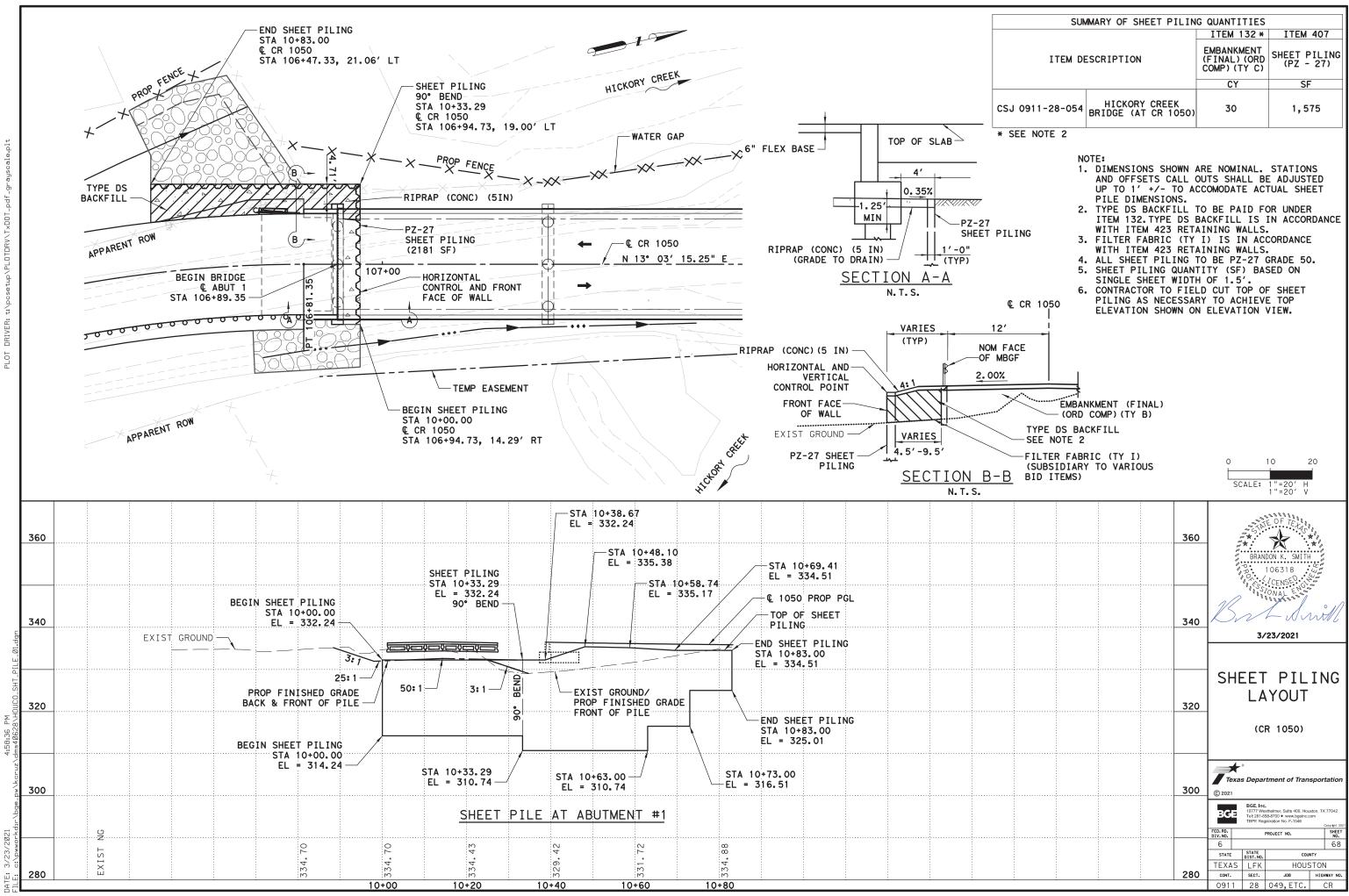


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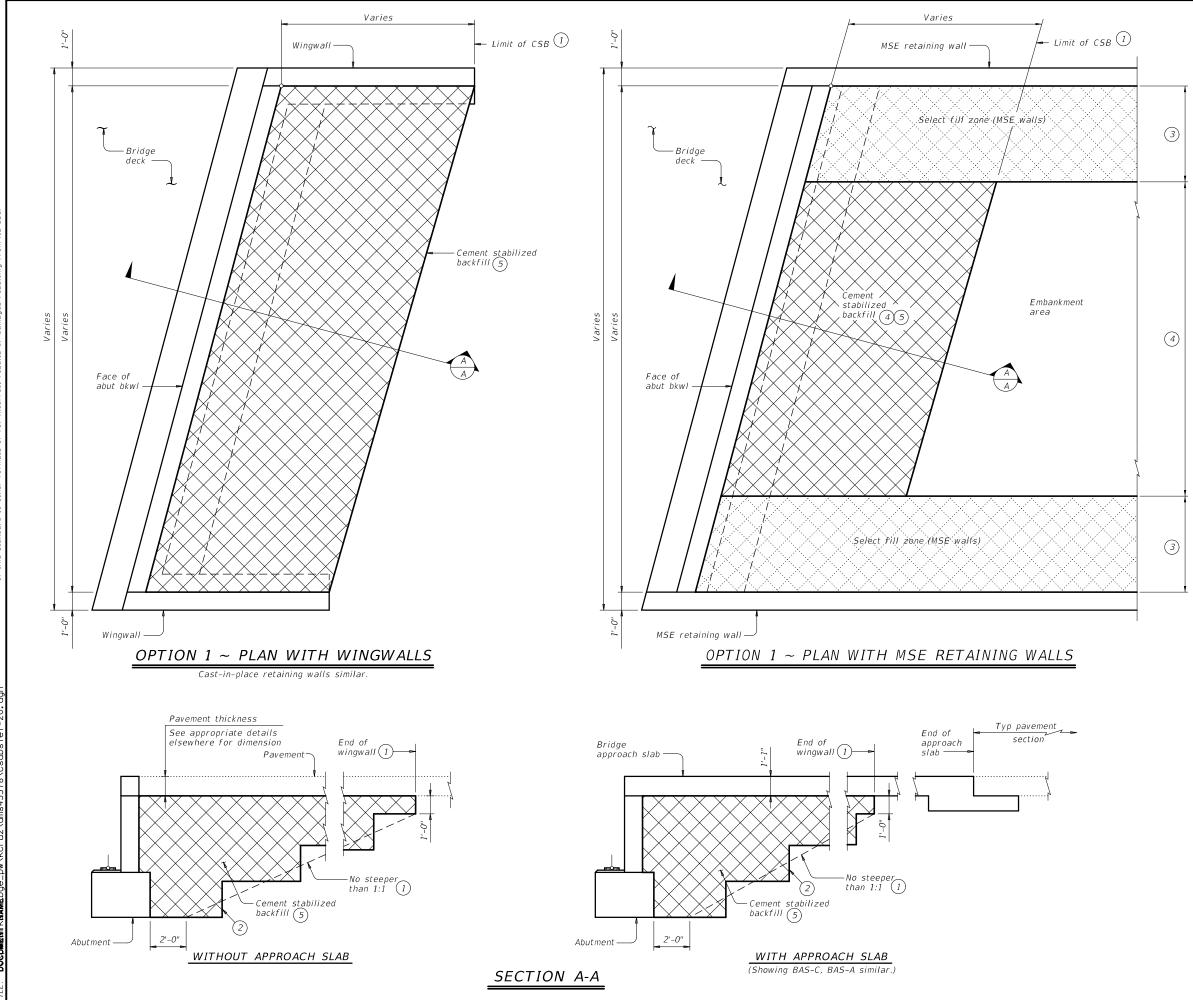
> ОDEL NAME: HICKORY CREEK TRIB ED AND CAP ELEVAT ATE: 3/1/2021 D. 5. 2/2000-2415/bar/415/bar/415/2023/HILICO ВРЕЛИЗ







T×DO OTDR/ DRIVEF ΝEN



DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warn the use of TxDDT for any purpose whatsoever. TxDDT assumes no responsibility for sof this standard to other formats or for incorrect results or damages resulting from its .

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- (1) Usual limit of Cement Stabilized Backfill is at end of wingwall. Extend CSB limits as required to maintain a slope no steeper than 1:1 at bottom of backfill.
- (2) Bench backfill as shown with 12" (approximate) bench depths.
- (3) Where MSE retaining walls are present, adjust CSB limits to accommodate the select fill zone. See retaining wall details for additional information.
- (4) When distance between select fill zones is less than 5'-0", MSE select fill may be substituted for cement stabilized backfill with approval from the Engineer.
- 5 If shown in the plans flowable backfill can be used as a substitute for cement stabilized backfill with the following constraints:

constraints: a). If flowable backfill is to be placed over MSE backfill then a filter fabric will be placed over the MSE backfill prior to placement of the flowable fill; and b). Place flowable fill in lifts not exceeding 2 feet in height, place each successive lift when the previous lift has stiffened/hardened (i.e. has lost its flowability).

GENERAL NOTES:

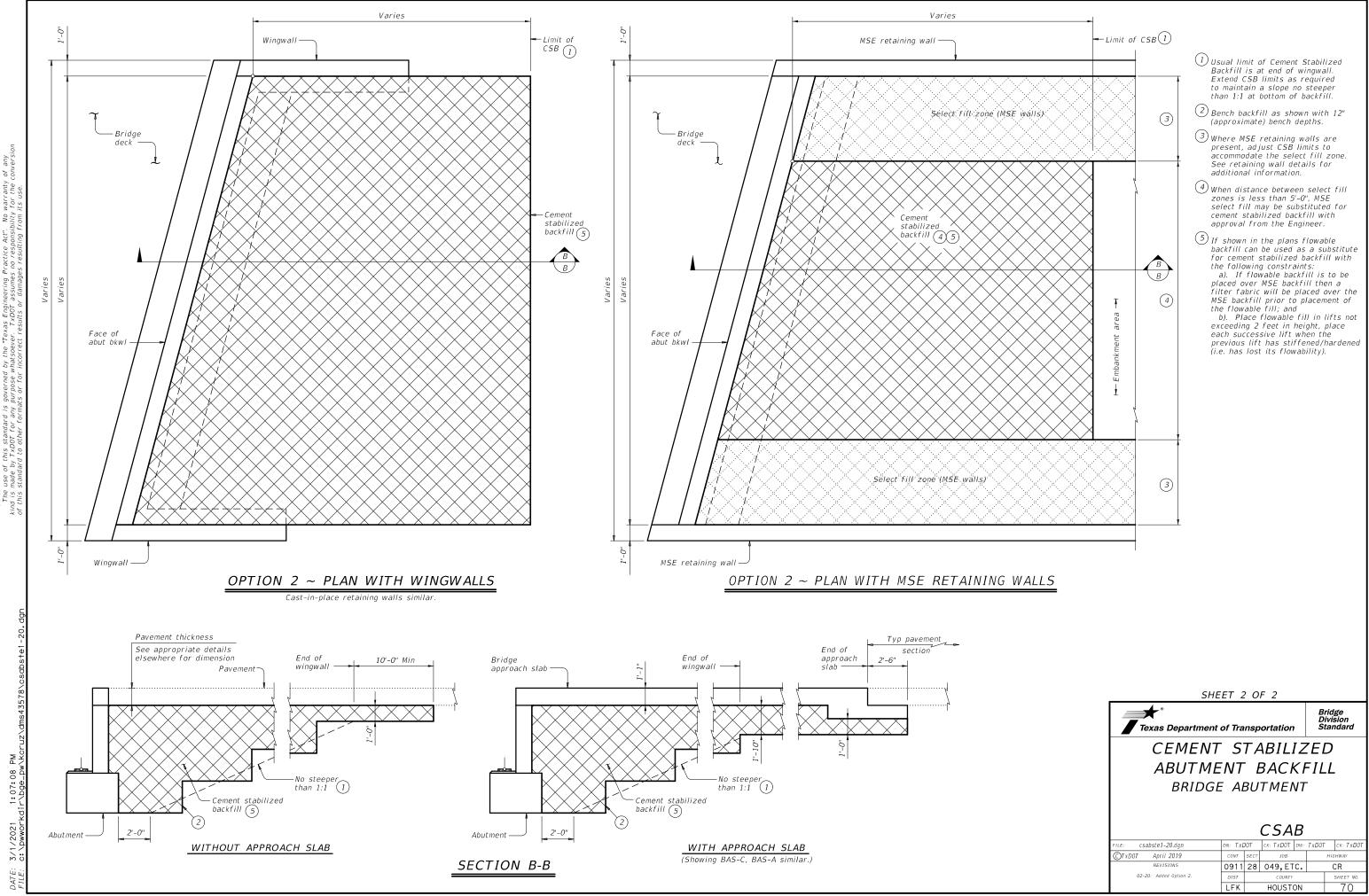
See the Bridge Layout for selected Option. Option 2 is intended for new construction requiring high plasticity embankment fill with a plasticity index (PI) greater than 30 or pavement built in poor native soil. Poor soils are defined as high plasticity clays or expansive clays. Option 1 is intended for construction only requiring PI controlled embankment fill or excavation in competent soils/rocks in order to construct the abutment. Provide Cement Stabilized Backfill (CSB) meeting the

Provide Cement Stabilized Backfill (CSB) meeting the requirements of Item 400, "Excavation and Backfill for Structures", to the limits shown at bridge abutments. If required elsewhere in the plans, provide Flowable

Backfill meeting the requirements of Item 401, "Flowable Backfill", to the limits shown at bridge abutments. Details are drawn showing left forward skew. See Bridge Layout for actual skew direction.

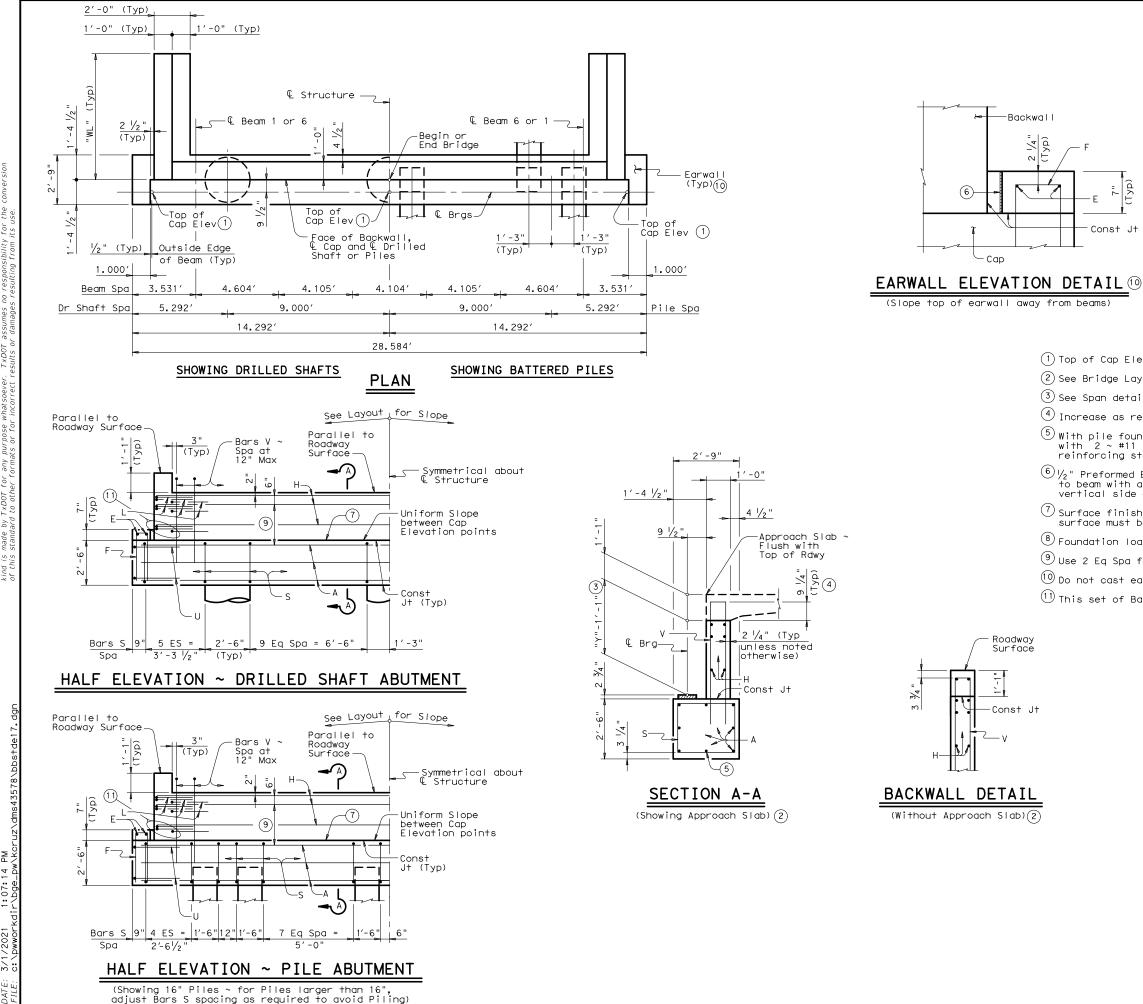
These details do not apply when Concrete Block retaining walls are used in lieu of wingwalls.

| SHE | ET 1 | 0 | F 2 | | | | | | | | |
|------------------------|---------|--------------------------------|--------------|---------|-----------|--|--|--|--|--|--|
| Texas Department | D | Bridge Division Standard | | | | | | | | | |
| CEMENT | - 5 | T, | ABILI. | ΖE | D | | | | | | |
| ABUTME | NT | Ē | ЗАСК | FIL | L | | | | | | |
| BRIDGE ABUTMENT | | | | | | | | | | | |
| | | | | | | | | | | | |
| | | | | | | | | | | | |
| | - | | CSAB | | | | | | | | |
| FILE: csabste1-20.dgn | DN: TXE | D0T | CK: TXDOT DW | : TxD01 | ск: ТхDOT | | | | | | |
| ©TxDOT April 2019 | CONT | SECT | JOB | | HIGHWAY | | | | | | |
| REVISIONS | 0911 | 28 | 049,ETC. | | CR | | | | | | |
| 02-20: Added Option 2. | DIST | | COUNTY | | SHEET NO. | | | | | | |
| | LFK | | HOUSTON | | 69 | | | | | | |



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| TABL WING LENG "W | WALL |
|----------------------------|---------|
| Beam Type | "WL" |
| B20 | 8.000′ |
| B28 | 10.000′ |
| B34 | 11.000′ |
| | |

| | TABLE O DATION | |
|----------------|--------------------------|--------------------------|
| Span Length | Drilled Shaft Load | Battered Pile Load |
| F+ | Tons/DS | Tons/Pile |
| 30 | 50 | 38 |
| 35 | 55 | 41 |
| 40 | 60 | 43 |
| 45 | 64 | 45 |
| 50 | 68 | 47 |
| 55 | 73 | 50 |
| 60 | 77 | 52 |
| 65 | 81 | 54 |
| 70 | 85 | 56 |
| 75 | 89 | 58 |
| 80 | 93 | 60 |
| 85 | 97 | 62 |
| 90 | 101 | 64 |
| 95 | 105 | 66 |

(1) Top of Cap Elevations are based on section depths shown on Span Details.

(2) See Bridge Layout for Joint type and to determine if Approach Slab is present. 3 See Span details for "Y" value.

(5) With pile foundations, replace Bar A, located at bottom centerline of cap with 2 ~ #11 x 5'-0" bars placed between pile groups. Deduct 93 Lbs from reinforcing steel total.

 $\overset{(6)}{\to} /_2"$ Preformed Bituminous Fiber material between beam and earwall. Bond to beam with an approved adhesive. Inside face of earwall to be cast with vertical side of beam.

O Surface finish for the top of Cap will be a textured wood float finish. The surface must be level in the direction of the centerline of Beams.

 $^{(8)}$ Foundation loads are based on B34 beams.

 $^{(9)}$ Use 2 Eq Spa for B28 and B34 beams. Use 1 space for B20 beams.

0 Do not cast earwalls until beams are erected in their final position.

(1) This set of Bars L only required for B28 and B34 beams.

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications.

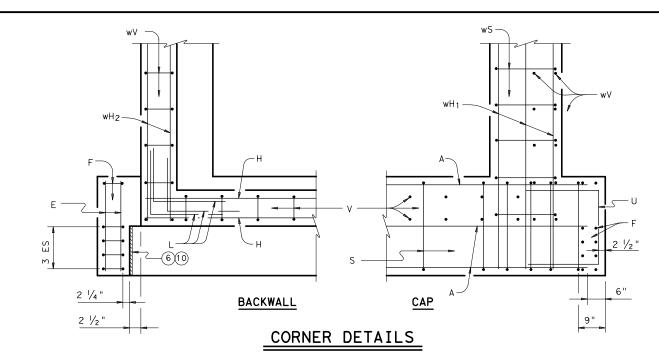
Concrete strength f'c = 3,600 psi. All reinforcing must be Grade 60. Designed for normal embankment header slope of 3:1 or 2:1. See Bridge Layout for beam type and foundation type, size and lenath.

See standard FD for all foundation details and notes. See applicable rail details for rail anchorage cast in wingwalls.

See standard CRR for riprap attachment details, if applicable. These abutment details may be used only with the following

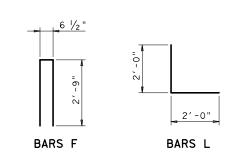
standards: SBBS-B20-24 or SBB0-B20-24 SBBS-B28-24 or SBB0-B28-24 SBBS-B34-24 or SBB0-B34-24

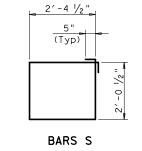
HL93 LOADING SHEET 1 OF 2 Bridge Division Standard × ° Texas Department of Transportation **ABUTMENTS** PRESTR CONC BOX BEAMS 24' RDWY *ABB-24* bbstde17.dgn DN: TXDOT CK: TXDOT DW: TXDOT CK: TXDO C)TxDOT December, 2006 JOB HIGHWA CR 0911 28 049, ETC. 04-11: Span length. IFK HOUSTON 71

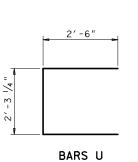


| | | 4 |
|---------------------------|--|---------|
| Bars wV & wS Spa ~ 2 1/4" | Eq Spa at 12" Max | _3" |
| Flush with Top of Rdwy | Permiss Const Jt B WH2 WH2 | 9 Vw |
| | | ws a |
| | →B | |
| | WINGWALL ELEVATION | |

(Earwall omitted for clarity)







| | 7 | 1/2 "_ | | |
|-----|---|--------|-----|---|
| | _ | | | |
| ·6" | | | | |
| | | | | _ |
| s u | | B | ARS | 5 |

| - | |
|-------------------------------|-----------------------|
| "Y" + 1'-2" "Y" + 1'-3 /2" | 1'-7 ½" 5" (Typ |
| > ≩ | |
| V & wV (3) | BARS wS |

SECTION B-B

TABLE OF ESTIMATED

NO. SIZE

8 #11

4 # 5

10 # 5

4 # 6

12 # 6

32 # 4

4 # 6

25 # 5

14 # 6

12 # 6

18 # 5

Class "C" Concrete (w/Slab) CY

2'-0'

Class "C" Concrete (w/ACP)

4

18

Reinforcing Steel

wH2

2 1/4" (Typ unless noted otherwise)

Const Jt

wH₁

BAR

A (5)

F

F

Н

S

U

V

wH 1

wH 2

wS

wV

QUANTITIES (TYPE B20 BEAMS)¹²

LENGTH

27'- 7"

2'- 5"

6'-1"

25'-10"

4'- 0"

9'- 8"

7'- 3"

7'- 6"

9'- 0"

7'- 8"

7'- 9"

7'- 9"

1′-0″

Lb

CY

WEIGHT

1,172

10

63

155

72

207

44

12.3

25 191 V 14 189 wH 1 138 wH 2 16 93 22 wS 145 wV 22 Reinforcing Steel 2,479 12.6 Class "C" Concrete





| | QU | JANT | ESTIMATE ITIES 8 BEAMS | | | | ANT] | STIMATE [TIES 4 BEAMS] | |
|----------|--------|------|------------------------------|--------|----------|--------|-------|------------------------------|--------|
| BAR | NO. | SIZE | LENGTH | WEIGHT | BAR | NO. | SIZE | LENGTH | WEIGHT |
| A (5) | 8 | #11 | 27'- 7" | 1,172 | A (5) | 8 | #11 | 27'- 7" | 1,172 |
| E | 4 | # 5 | 2'- 5" | 10 | E | 4 | # 5 | 2'- 5" | 10 |
| F | 10 | # 5 | 6′-1" | 63 | F | 10 | # 5 | 6'-1" | 63 |
| Н | 6 | # 6 | 25′-10" | 233 | Н | 6 | # 6 | 25'-10" | 233 |
| L | 18 | # 6 | 4′- 0" | 108 | L | 18 | # 6 | 4'- 0" | 108 |
| S | 32 | # 4 | 9'- 8" | 207 | S | 32 | # 4 | 9'- 8" | 207 |
| U | 4 | # 6 | 7'- 3" | 44 | U | 4 | # 6 | 7'- 3" | 44 |
| V | 25 | # 5 | 8'- 9" | 226 | V | 25 | # 5 | 9'-10" | 254 |
| wH 1 | 14 | # 6 | 11'- 0" | 231 | wH 1 | 14 | # 6 | 12'- 0" | 252 |
| wH 2 | 16 | # 6 | 9'- 8" | 232 | wH 2 | 16 | # 6 | 10'- 8" | 256 |
| wS | 22 | # 4 | 7'- 9" | 114 | wS | 24 | # 4 | 7'- 9" | 124 |
| wV | 22 | # 5 | 9'- 0" | 207 | wV | 24 | # 5 | 10' - 1" | 252 |
| Reinforc | ing St | eel | Lb | 2,847 | Reinforc | ing St | eel | Lb | 2,975 |
| Class "C | " Conc | rete | (w/Slab) CY | 14.7 | Class "C | " Conc | orete | (w/Slab) CY | 16.2 |
| Class "C | " Conc | rete | (w/ACP) CY | 14.4 | Class "C | " Cond | crete | (w/ACP) CY | 15.9 |

(3) See Span details for "Y" value.

(5) With pile foundations, replace Bar A, located at bottom centerline of cap, with 2 ~ #11 x 5'-0" bars placed between pile groups. Deduct 93 Lbs from reinforcing steel total.

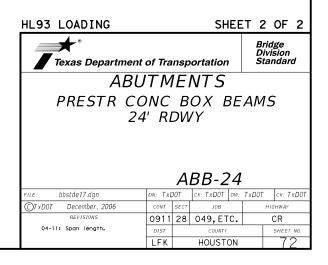
 $^{(6)}{\rm I}_{\rm 2}^{\prime}{\rm "}$ Preformed Bituminous Fiber material between beam and earwall. Bond to beam with an approved adhesive. Inside face of earwall to be cast with vertical side of beam.

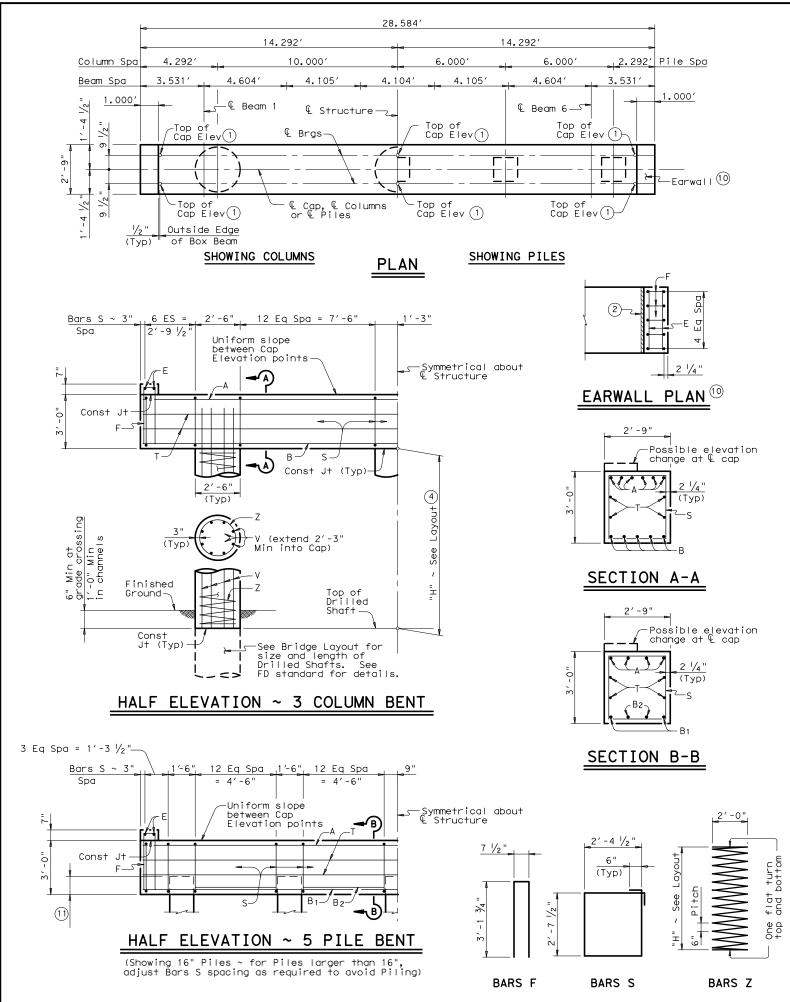
 $^{(9)}$ Use 2 Eq Spa for B28 and B34 beams and 1 space for B20 beams.

0 Do not cast earwalls until beams are erected in their final position.

 $\stackrel{(1)}{11}$ This set of Bars L only required for B28 and B34 beams.

(12) Quantities shown are for one Abutment only (with Approach Slab). With no Approach Slab, add 1.0 CY Class "C" concrete and 78 Lb reinforcing steel for 2 additional Bars H.





| | FOUND | ATION | LOADS | | | | ESTI TIES | | | TABLE OF ESTIMATED QUANTITIES FOR | | | | | | | |
|---|---------------------------|-------------------------------|---------------------------|----------------------|--------------|------------|--------------|---------|--------|--------------------------------------|-------|--------|------|-----|--------|--|--|
| | Average Span Length | Drilled Shaft Loads (5) | Vertical Pile Loads | | | | MN BE | | | 5-PILE BENT | | | | | | | |
| | F† | Tons/DS | Tons/Pile | Bar | No. | Size | Leng† | h | Weight | Bar | No. | Size | Leng | th | Weight | | |
| ſ | 30 | 86 | 50 | А | 6 | #11 28′-3 | | 3" | 901 | А | 4 | #11 | 28′- | 3" | 600 | | |
| | 35 | 95 | 55 | В | 5 #11 28'- 3 | | 3" | 750 | B 1 | 2 | #11 | 28′- | 3" | 300 | | | |
| | 40 | 104 | 61 | E | 4 | # 5 2'-5 | | 5" | 10 | B 2 | 8 | #11 | 4′- | 6" | 191 | | |
| | 45 | 113 | 66 | F | 10 | # 5 | 6′-1 | ′ –11" | | E | 4 | # 5 | 2′- | 5" | 10 | | |
| | 50 | 122 | 72 | S | 40 | # 5 | 11'- (|)" | 459 | F | 10 | # 5 | 6′- | 11" | 72 | | |
| | 55 7 | 131 | 77 | Т | 4 | # 5 28'- 3 | | 3" | 118 | S | 60 | # 5 | 11′- | 0" | 689 | | |
| | 60 (7) | 140 | 82 | V | 24 | # 9 | 32'- 3 | 32'- 3" | | Т | 4 | # 5 | 28′- | 3" | 118 | | |
| | 65 (7) | 148 | 87 | Z | 3 | # 3 | 391′- |)" | 441 | Reinf | orcin | g Stee | 1 | Lb | 1,980 | | |
| | 70 (8) | 157 | 93 | Reinf | orcinq | g Stee | I | Lb | 5,383 | Class | "C" | Conc (| Cap) | CY | 8.9 | | |
| | 75 (8) | 166 | 98 | Class "C" Conc (Cap) | | | | | 8.9 | | | | | | | | |
| | 80 (8) | 174 | 103 | Class | "C" (| Conc ((| Column) | CY | 16.4 | | | | | | | | |
| | 85 (8) | 183 | 108 | | | | | | | | | | | | | | |
| | 90 (9) | 192 | | | | | | | | | | | | | | | |
| | 95 (9) | 201 | | | | | | | | | | | | | | | |

- (1) Top of Cap Elevations are based on section depths shown on span details.
- $^{(2)}$ $^{\prime}_{2}$ " Preformed Bituminous Fiber material between box beam and earwall. Bond to beam with an approved adhesive. Inside face of earwall to be cast with vertical side of beam.
- $\overset{\textcircled{3}}{3}$ Quantities shown are based on an "H" value of 30'. For each linear foot variation in "H" value, make the following adjustments:
- (4) This standard may not be used for "H" heights exceeding 30' or exposed pile heights exceeding the values shown in the tables. In areas of very soft soil or where scour is anticipated, maximum allowable "H" heights or exposed pile heights must be evaluated by the engineer prior to the use of this standard.
- $^{(5)}$ Foundation loads based on "H" = 30'.
- 6 When HP14x117 steel piling is specified in the plans, the Contractor has the option of furnishing either HP14x117 or HP16x101 steel piling.
- \bigodot 16" square concrete piling and HP 14 x 73 piling may not be used.
- $^{(8)}$ 18" square concrete piling and HP 14 x 117 piling may not be used.
- $^{(9)}$ Pile supported bents not allowed at this average span length.
- 0 Do not cast earwalls until beams are erected in their final position.
- (1) See FD standard.

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TABLE OF MAXIMUM ALLOWABLE EXPOSED PILE HEIGHTS AND PILE LOADS

Pile Type Max Ht | Max Load Tons/Pile Concrete Steel F† 75 16" Sq $HP14 \times 73$ 16 18" Sq HP14×117(6) 20 90 20" Sq HP18x135 24 110

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. For Pile Bents supporting unequal spans, the shorter span cannot be less than 80 percent of the longer span. Concrete strength f'c = 3,600 psi.

All cap reinforcing must be Grade 60. Column and drilled shaft reinforcing may be Grade 40.

Bent selected will be based on the average span length rounded up to the next 5-foot increment. See Bridge Layout for foundation type, size and length.

See standard FD for all foundation details and notes.

These standards do not support the use of multi-pile footings as shown on standard FD. These Bent details may be used only with the

following Standards: SBBS-B20-24 or SBB0-B20-24 SBBS-B28-24 or SBBO-B28-24 SBBS-B34-24 or SBBO-B34-24

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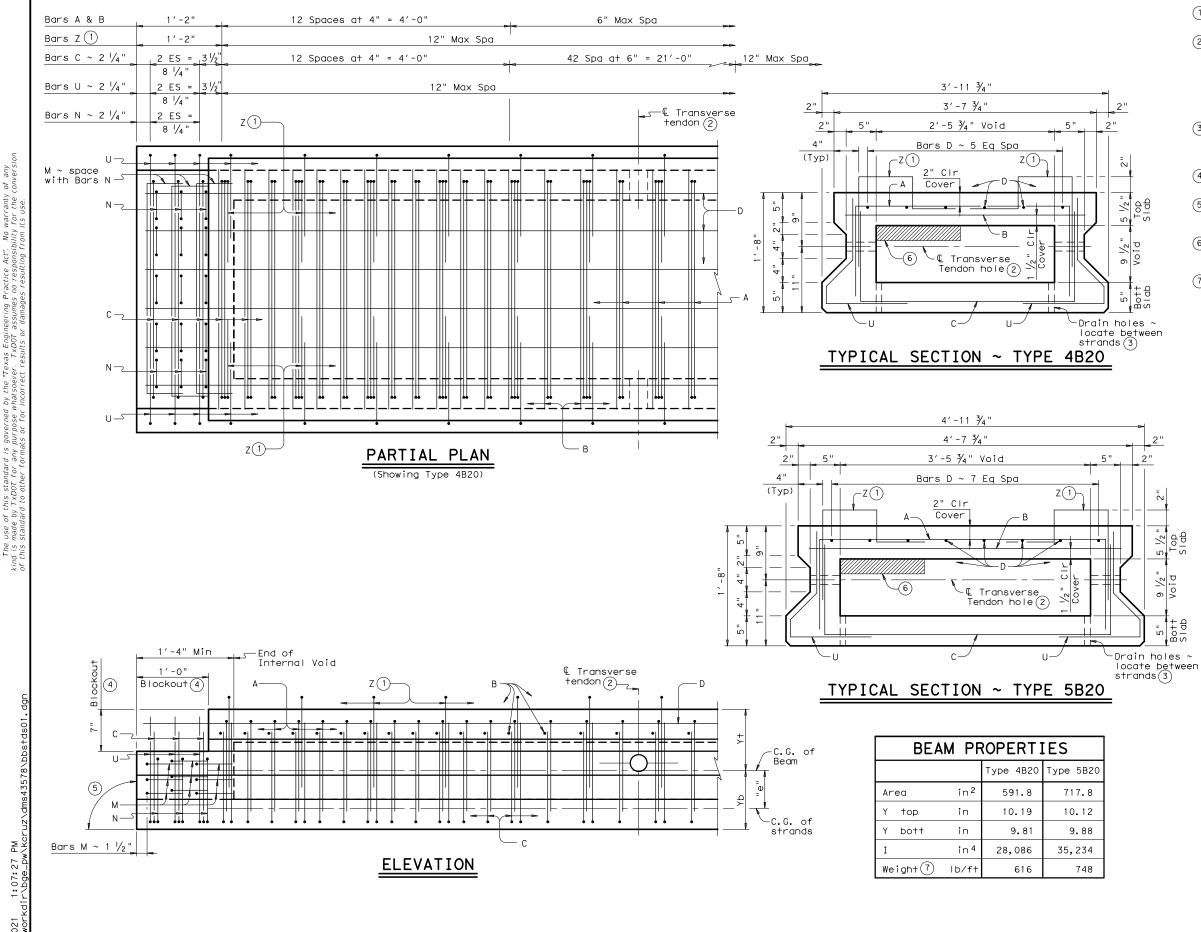
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| Texas Department of Transportation | Bridge Division Standard |
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| INTERIOR BENT. | S |
| PRESTR CONC BOX BE 24' RDWY | AMS |

BBB-24

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| ©TxDOT December, 2006 | CONT | SECT | JOB | | ŀ | HIGHWAY | | | |
| REVISIONS | 0911 | 28 | | CR | | | | | |
| 04-11: Span length. 02-2012: Piles and Notes | DIST | | COUNTY | | | SHEET NO. | | | |
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- (1)Bars Z are required for beams topped with a cast-in-place concrete slab only.
- (2) Post-tensioning tendons are required for beams not topped with a Min 5" cast-in-place concrete slab. See span details for number and spacing of transverse tendons. Cast interior diaphragms in exterior beams and beams that serve temporarily as exterior beams in staged constructed bridges. See "Blockout, Interior Diaphragm, and Drain Details". Form 3" Dia holes in interior beams. See standard BBPT for details.
- $(\overline{3})$ Place drain holes (1" Dia PVC Sch 40 Pipe) as shown in all beam void corners including each side of interior diaphragms. See "Blockout, Interior Diaphragm, and Drain Details".
- (4) Blockouts required at ends of all beams. Extend beam reinforcement into blockouts.
- $(5)90^{\circ}$ at conventional Interior Bents. Ends of beams shall be vertical at Abutment backwall and Inverted Tee Bent Stems.
- 6 Showing void modification required in exterior beams not topped with a Min 5" cast-in-place concrete slab. See standard BBRAO for void modification dimensions.
- $\widehat{(7)}$ Based on 150 pcf weight density of concrete. Weight of end blocks and interior diaphragms is not included.

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications. Use Class H concrete. Use Class H (HPC) if required elsewhere in plans. All reinforcing steel

Two-stage monolithic casting is required. The concrete in the first stage cast (bottom beam flange) must remain plastic until the second stage cast (webs and top beam flange) is placed. Vibrate as required to ensure consolidation between the two

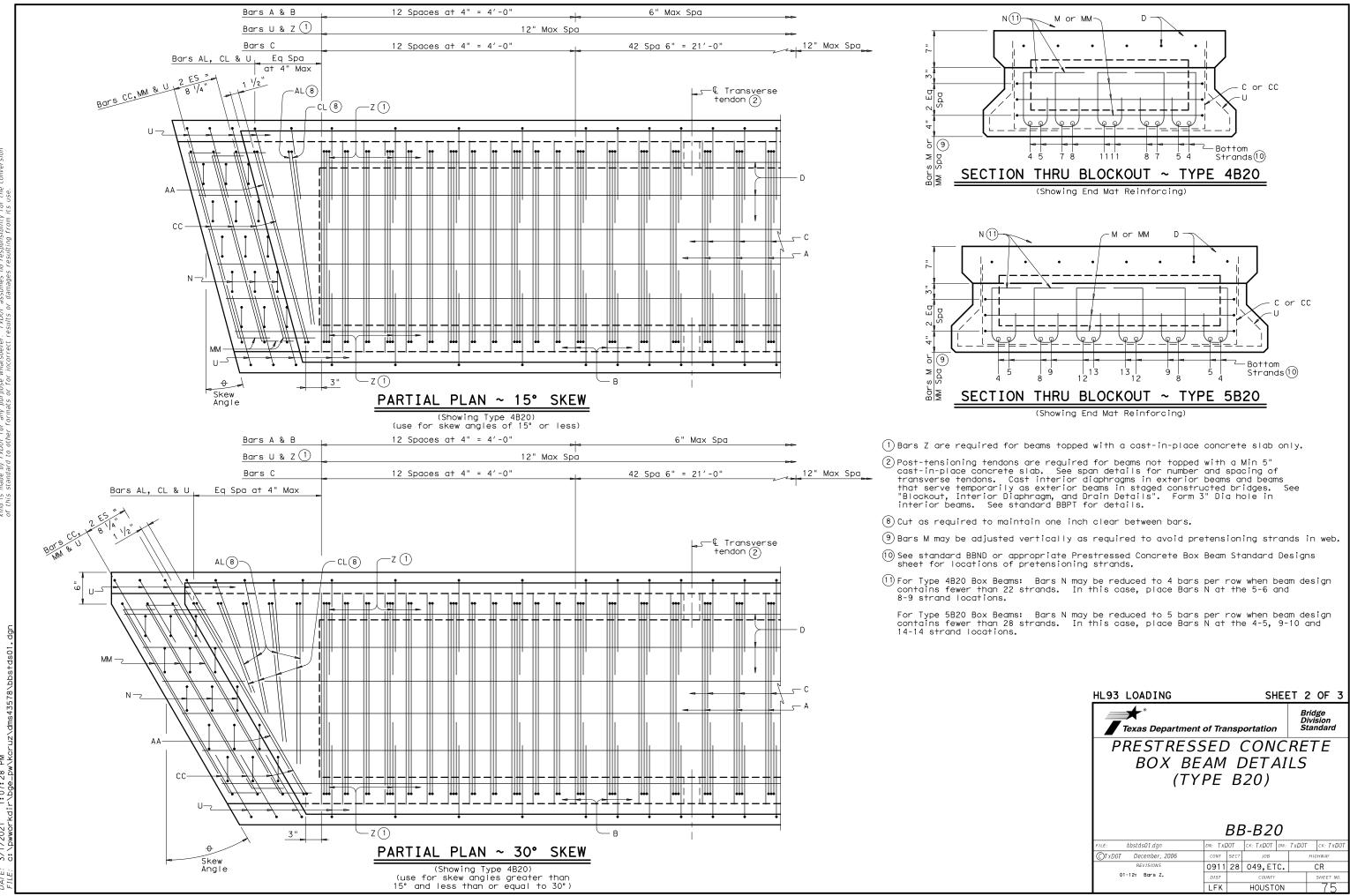
casts. 1 1/4" clear cover to reinforcement is required unless noted otherwise. See standard BBRAS or BBRAO for railing

anchorage at bridge edges to be cast in beams. An equal area of welded wire reinforcement (WWR) meeting the requirements of ASTM A1064 may be substituted for Bars A, B, C, and D. These details are applicable for skews up to 30

degrees only. Chamfer bottom beam corners $\frac{3}{4}$ " or round to

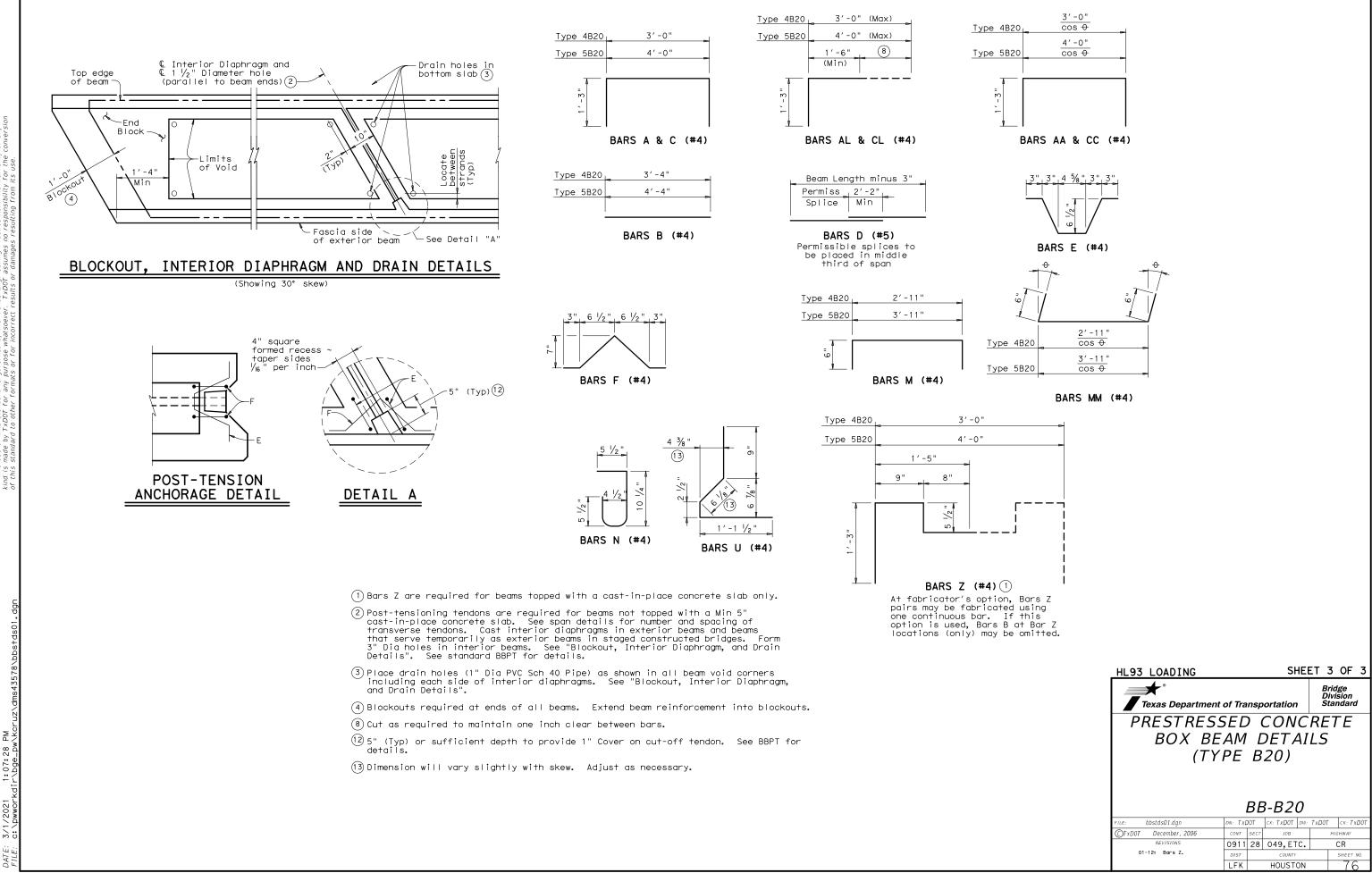
a $\frac{3}{4}$ " radius.

| HL93 LOADING | | | SHE | ET | 1 OF | 3 | | | | | | | |
|-----------------------------|---------|--------------------------------|--------------|---------|---------|-----|--|--|--|--|--|--|--|
| Texas Department | D | Bridge Division Standard | | | | | | | | | | | |
| PRESTRESS BOX BE (TY) | ĀΜ | Ľ | DETA | | | | | | | | | | |
| BB-B20 | | | | | | | | | | | | | |
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| CTxDOT December, 2006 | CONT | SECT | JOB | | HIGHWAY | | | | | | | | |
| REVISIONS | 0911 | 28 | 049,ETC. | | CR | | | | | | | | |
| 01-12: Bars Z. | DIST | | COUNTY | | SHEET N | 10. | | | | | | | |
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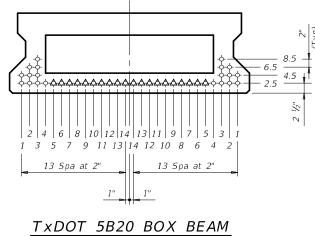
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| | | | | | l | DESIG | NED E | BEAMS (| (STRAIG | HT S | STRAND | S) | | | | | | | | | | OPTION | AL DESIG | N | |
|-------------|----------|--------------|--------------|---------------|----------------|---------|-------------------|----------------------|----------------------|-------------------|------------------------|----------------|---------------|--------|-------------|---------------|------|----|---------------|--------------------------|---------------------------|-------------------------------|--------------------------------|-------------------------|--------------------|
| STANDARD | SPAN | BEAM | BEAM | NON- | F | PRESTRE | ESSING . | STRANDS | | | | | D STRANI | | JMBER | R OF S | TRAN | 05 | RELEASE | RETE MINIMUM | DESIGN LOAD | DESIGN LOAD | REQUIRED MINIMUM | DIST | E LOAD RIBUTION |
| SBBS-B20-24 | LENGTH | NO. | TYPE | STD STRAND | TOTAL NO. | SIZE | STRGTH | "e" ⊈ | "e" END | TOT NO. DEB | DIST FROM BOTTOM | | ANDS | | | BONDE from | | | STRGTH | 28 DAY COMP STRGTH | COMP STRESS (TOP @) | TENSILE STRESS (BOTT Q) | ULTIMATE MOMENT CAPACITY | | ACTOR 2 |
| | (ft) | | | PATTERN | | (in) | fpu (ksi) | (in) | (in) | 525 | (in) | TOTAL | DE- BONDED | 3 | 6 | 9 | 12 | 15 | f'ci (ksi) | f'c (ksi) | (SERVICE I) fct(ksi) | (SERVICE III) fcb(ksi) | (STRENGTH 1) (ft-kips) | Moment | - |
| | 30 30 | 1&6 2 - 5 | 5B20 4B20 | | 8 6 | 0.6 | 270 270 | 7.38 7.31 | 7.38 7.31 | 0 | 2.50 2.50 | 8 6 | 0 | 0 0 | 0 0 | 0 0 | 0 | 0 | 4.000 | 5.000 5.000 | 0.640 0.693 | -0.808 -0.860 | 704 601 | 0.454 0.379 | 0.69 |
| | 35 | 1&6 2 - 5 | 5B20 4B20 | | 8 | 0.6 | 270 | 7.38 7.31 | 7.38 7.31 | 0 | 2.50 | 8 6 | 0 | 0 | 0 0 | 0 0 | 0 | 0 | 4.000 | 5.000 5.000 | 0.838 0.911 | - 1 . 041 - 1 . 111 | 795 615 | 0.440 | 0.68 |
| 24' Roadway | 40 40 | 1&6 2 - 5 | 5B20 4B20 | | 10 8 | 0.6 | 270 270 270 | 7.38 7.31 | 7.38 7.31 | 0 | 2.50 | 10 8 | 0 | 0 0 | 0 0 | 0 0 | | 0 | 4.000 | 5.000 5.000 5.000 | 1.061 1.156 | - 1 . 297 - 1 . 388 | 889 712 | 0.427 0.356 | 0.67 |
| 5" Slab | 45 | 1&6 2 - 5 | 5B20 4B20 | | 10 10 | 0.6 | 270 270 270 | 7.31 7.38 7.31 | 7.31 7.38 7.31 | 0 | 2.50 2.50 2.50 | 10 10 | 0 | 0 | 0 | 0 | 0 | 0 | 4.000 | 5.000 5.000 5.000 | 1.316 1.437 | - 1 . 590 - 1 . 706 | 960 824 | 0.330 0.417 0.348 | 0.66 |
| | 50 50 | 1&6 2 - 5 | 5B20 4B20 | | 12 12 12 | 0.6 | 270 270 270 | 7.38 7.31 | 7.38 7.31 | 0 | 2.50 2.50 2.50 | 12 12 12 | 0 | 0 0 | 0 | 0 | | 0 | 4.000 | 5.000 5.000 5.000 | 1.606 | - 1 . 927 - 2 . 070 | 1147 985 | 0.408 0.340 | 0.65 |
| | 55 | 1&6 2 - 5 | 5B20 4B20 | | 16 14 | 0.6 | 270 270 270 | 7.31 7.38 7.31 | 7.31 7.38 7.31 | 0 | 2.50 2.50 2.50 | 16 14 | 0 | 0 0 | 0 0 | 0 | | 0 | 4.000 | 5.000 5.000 5.000 | 1.921 2.104 | - 2 . 289 - 2 . 464 | 1344 1157 | 0.400 | 0.64 |
| | 60 60 | 1&6 2 - 5 | 5B20 4B20 | | 18 18 18 | 0.6 | 270 270 270 | 7.38 7.31 | 7.31 7.38 7.31 | 02 | 2.50 2.50 2.50 | 18 18 | 0 2 | 0 0 | 0 2 | 0 | | 0 | 4.000 | 5.000 5.000 5.000 | 2.262 | - 2.677 - 2.899 | 1551 1347 | 0.393 | 0.64 |
| | 65 65 | 1&6 2 - 5 | 5B20 4B20 | | 24 20 | 0.6 | 270 | 7.38 7.31 | 7.31 7.38 7.31 | 64 | 2.50 | 24 20 | 6 4 | 2 0 | 2 2 2 | 0 | 22 | 0 | 4.000 | 5.000 5.800 | 2.627 | - 3.091 - 3.368 | 1769 1551 | 0.387 | 0.63 |
| | | 2 5 | 4020 | | 20 | 0.0 | 270 | 7.51 | 7.51 | - | 2.50 | 20 | - | U | 2 | 0 | 2 | | 4.000 | 5.000 | 2.505 | 5.500 | 1551 | 0.555 | 0.40 |
| | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | + | | <i>(4B20</i> | | | | | | | | | | | | | - ¢ 5820 | | - 15 1 | | |

TXDOT 4B20 BOX BEAM

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DISCLAIMER: The use of this standard is governed by the "Texas Engineering Practice Act". No warranty of any tind is made by TXDOT for any purpose whatsoever. TXDOT assumes no responsibility for the conversion of this standard to other formats or for incorrect results or damages resulting from its use.

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skew. FABRICATION NOTES: Provide Class H concrete. Provide Grade 60 reinforcing steel bars. Use low relaxation strands, each pretensioned to 75 percent of fpu. When shown on this sheet, the Fabricator has the option of furnishing either the designed beam or an approved optional beam design. All optional design whether the designed conclusion and the design. submittals and shop drawings must be signed, sealed and dated by a Professional Engineer registered in the State of Texas. Locate strands for the designed beam as low as possible on the 2" grid system unless a non-standard stand pattern is indicated. Fill row "2.5", then row "4.5", then row "6.5", etc. Place strands within a row as follows: 1) Locate a strand in each "1" position. 2) Place strand symmetrically about vertical centerline of box. 3) Space strands as equally as possible across the entire width. Strand debonding must comply with Item 424.4.2.2.2.4. Do not debond strands in position "1". Distribute debonded strands equally

DESIGN NOTES:

Designed in accordance with AASHTO LRFD Bridge Design Specifications. Prestress losses for the designed beams have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform. Beam designs are applicable for 5" concrete slabs without overlay and 0 degree

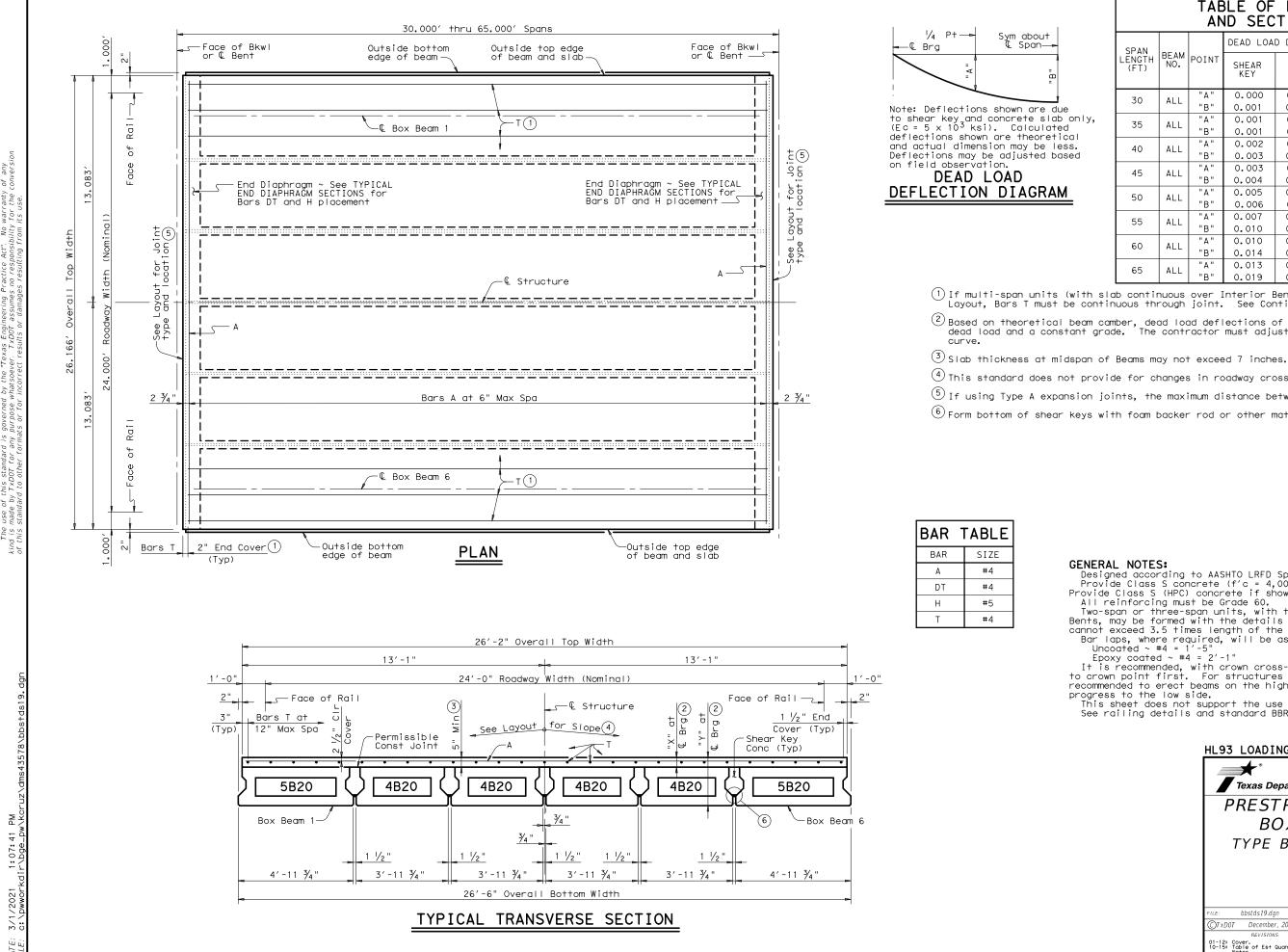
about the vertical centerline. Decrease debonded lengths working inward, with debonding staggered in each row. Full-length debonded strands are only permitted in positions marked Δ .

> 1 Based on the following allowable stresses (ksi): Compression = 0.65 f'ci Tension = $0.24\sqrt{f'ci}$

Optional designs must likewise conform.

2 Portion of full HL93.

| TL95 LUADING | | | | | | | | | |
|--|--------|------|---------|-----|-----|-----------|--|--|--|
| Bridge Division Texas Department of Transportation | | | | | | | | | |
| PRESTR CONC BOX BEAM STANDARD DESIGNS TYPE B20 24' RDWY (WITH SLAB) BBSDS-B20-24 | | | | | | | | | |
| FILE: bbstds11.dgn | DN: SF | RM | ск: ВМР | DW: | SFS | ск: SDB | | | |
| CTxDOT December 2006 | CONT | SECT | JOB | | | HIGHWAY | | | |
| REVISIONS | 0911 | 28 | 049,ET | C. | | CR | | | |
| 04-11: f'ci and LLDF. 01-16: Notes, 0.6" strand designs. | DIST | | COUNTY | | | SHEET NO. | | | |
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| TABLE OF DEFLECTIONS AND SECTION DEPTHS | | | | | | | | | | | |
|--|-----------|------------|----------------|----------------|----------------|----------------------|-----------------------|--|--|--|--|
| SPAN | | | DEAD LOA | D DEFLECT | IONS (FT) | SECTION | DEPTHS | | | | |
| LENGTH (FT) | TH BEAM F | POINT | SHEAR KEY | SLAB | TOTAL | "X" AT € BRG 2 | "Y" AT € BRG 2 | | | | |
| 30 | ALL | "A" "B" | 0.000 0.001 | 0.002 0.002 | 0.002 0.003 | 5 1⁄4 " | 2'-1 /4" | | | | |
| 35 | ALL | "A" "B" | 0.001 0.001 | 0.003 0.004 | 0.004 0.005 | 5 1⁄4 " | 2′-1 /4″ | | | | |
| 40 | ALL | "A" "B" | 0.002 0.003 | 0.005 0.007 | 0.007 0.010 | 5 1⁄4" | 2'-1 1/4" | | | | |
| 45 | ALL | "A" "B" | 0.003 0.004 | 0.009 0.012 | 0.012 0.016 | 5 | 2'-1 /2" | | | | |
| 50 | ALL | "A" "B" | 0.005 0.006 | 0.013 0.019 | 0.018 0.025 | 5 ¾" | 2'-1 ¾" | | | | |
| 55 | ALL | "A" "B" | 0.007 0.010 | 0.019 0.027 | 0.026 0.037 | 6 1⁄4 " | 2′-2 ¼″ | | | | |
| 60 | ALL | "A" "B" | 0.010 0.014 | 0.028 0.039 | 0.038 0.053 | 6 ¾" | 2'-2 3⁄4" | | | | |
| 65 | ALL | "A" "B" | 0.013 0.019 | 0.039 0.054 | 0.052 0.073 | 7" | 2'-3" | | | | |

(1) If multi-span units (with slab continuous over Interior Bents) are indicated on the Bridge Layout, Bars T must be continuous through joint. See Continuous Slab Detail.

⁽²⁾ Based on theoretical beam camber, dead load deflections of 5" Cast-in-place slab, shear key dead load and a constant grade. The contractor must adjust these values for any vertical

4 This standard does not provide for changes in roadway cross slopes within the structure.

 $^{(5)}$ If using Type A expansion joints, the maximum distance between joints is 100 feet.

 $^{(6)}$ Form bottom of shear keys with foam backer rod or other material acceptable to the Engineer.

GENERAL NOTES:

Designed according to AASHTO LRFD Specifications.

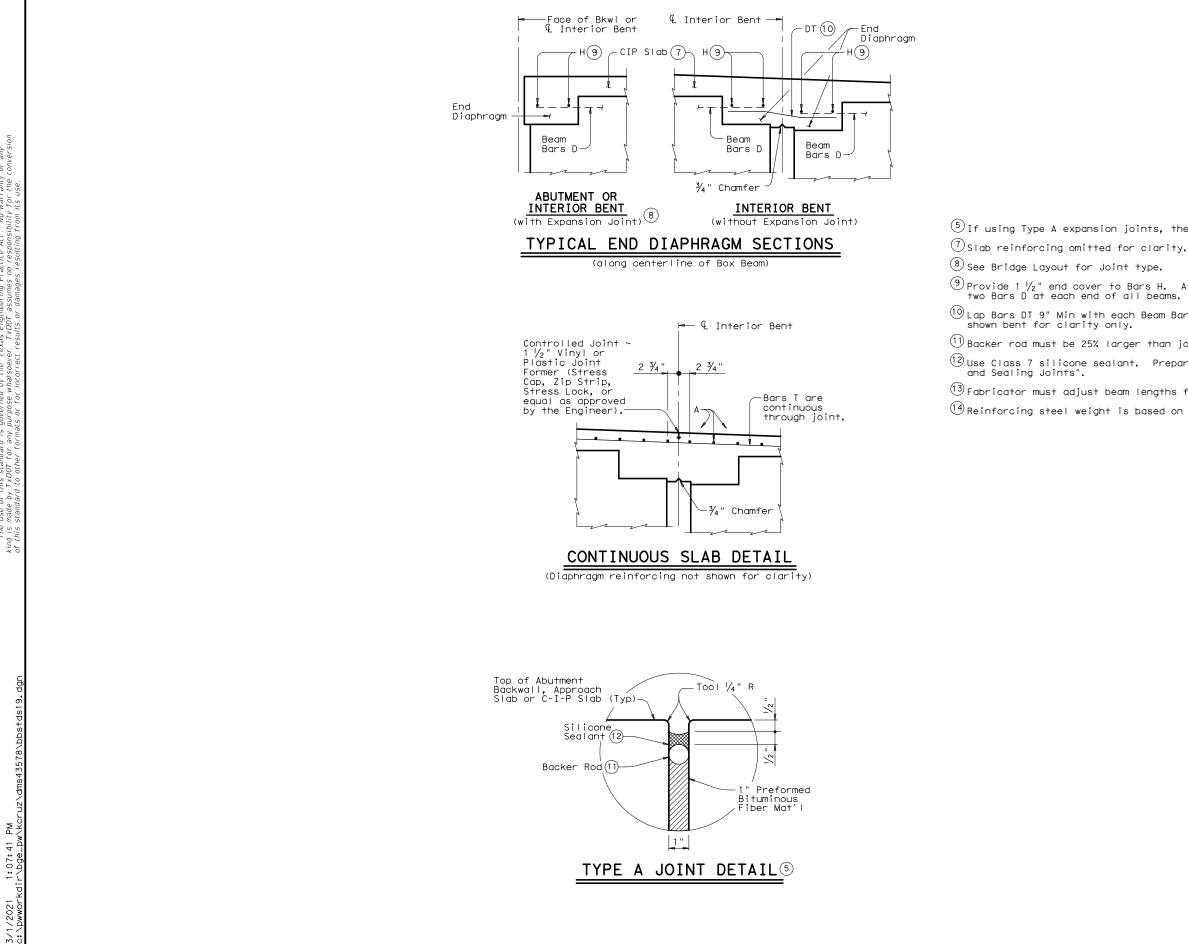
Provide Class S concrete (f'c = 4,000 psi) for slab and shear key. Provide Class S (HPC) concrete if shown elsewhere in the plans. All reinforcing must be Grade 60.

Two-span or three-span units, with the slab continuous over Interior Bents, may be formed with the details on this standard. Unit Length cannot exceed 3.5 times length of the shortest end span. Bar laps, where required, will be as follows:

Bar laps, where required, will be as follows: Uncoated ~ #4 = 1'-5" Epoxy coated ~ #4 = 2'-1" It is recommended, with crown cross-slope, to erect beams adjacent to crown point first. For structures without a crown point, it is recommended to erect beams on the high side of cross-slope first and progress to the low side.

This sheet does not support the use of Transition Bents. See railing details and standard BBRAS for rail anchorage.

| HL93 LOADING | SHEE | ET 1 | OF | 2 | | | |
|---|----------|--------------------------------|----------|-------|---------|------|--|
| Texas Department | ortation | Bridge Division Standard | | | | | |
| PRESTRESSED CONCRETE BOX BEAM SPANS TYPE B20 24' RDWY (WITH SLAB) SBBS-B20-24 | | | | | | | |
| FILE: bbstds19.dqn | DN: TXL | | | TxDOT | ск: Т | хD0T | |
| CTxDOT December, 2006 | CONT | SECT | JOB | | HIGHWAY | | |
| REVISIONS 01-12: Cover. | 0911 | 28 | 049,ETC. | | CR | | |
| 10-15: Table of Est Quantities, Notes. | DIST | | COUNTY | | SHEET | NO. | |
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| TAB | LE OF | ESTIMA | TED Q | UANTIT | IES |
|----------------|--------------|----------------------------------|---|---|-------------------------|
| SPAN LENGTH | SHEAR KEY | REINF CONC SLAB (BOX BEAM) | PRESTR CONCRETE BOX BEAMS (TY 4B20) | PRESTR CONCRETE BOX BEAMS (TY 5B20) | TOTAL REINF STEEL |
| FT | CY | SF | LF | LF | Lb |
| 30 | 4.0 | 785 | 118.00 | 59.00 | 1,570 |
| 35 | 4.6 | 916 | 138.00 | 69.00 | 1,832 |
| 40 | 5.3 | 1,047 | 158.00 | 79.00 | 2,094 |
| 45 | 6.0 | 1,177 | 178.00 | 89.00 | 2,354 |
| 50 | 6.6 | 1,308 | 198.00 | 99.00 | 2,616 |
| 55 | 7.3 | 1,439 | 218.00 | 109.00 | 2,878 |
| 60 | 8.0 | 1,570 | 238.00 | 119.00 | 3,140 |
| 65 | 8.6 | 1,701 | 258.00 | 129.00 | 3,402 |

 $^{(5)}$ If using Type A expansion joints, the maximum distance between joints is 100 ft.

 $^{(9)}$ Provide 1 $/\!\!/_2$ " end cover to Bars H. After all beams have been placed, weld one Bar H to two Bars D at each end of all beams.

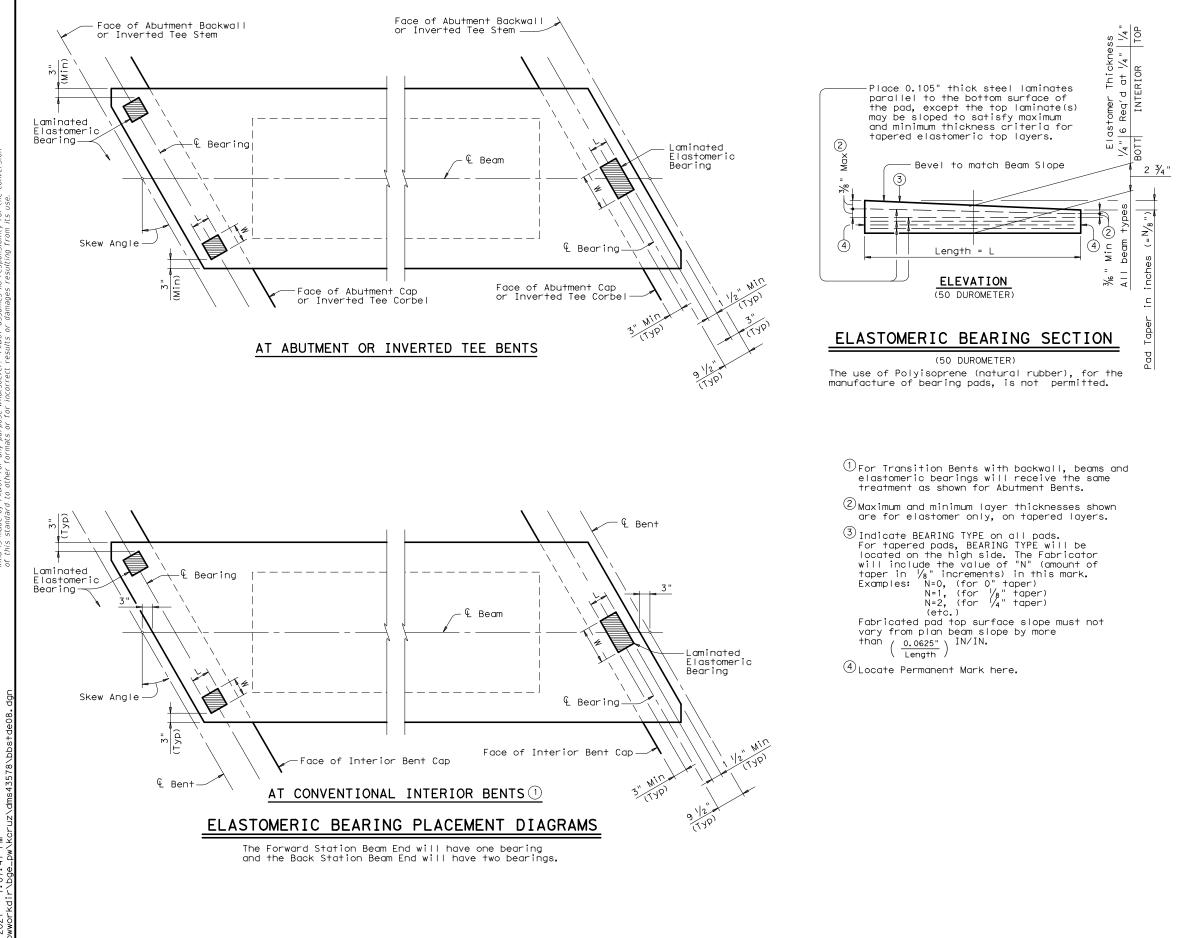
 $\overset{(1)}{\bigcirc}$ Lap Bars DT 9" Min with each Beam Bar D at Interior Bents without Expansion Joints. Bars DT shown bent for clarity only.

1 Backer rod must be 25% larger than joint opening and must be compatible with the sealant. 12 Use Class 7 silicone sealant. Prepare joint and seal in accordance with Item 438 "Cleaning and Sealing Joints".

(3) Fabricator must adjust beam lengths for beam slopes as required.

14 Reinforcing steel weight is based on an approximate factor of 2.0 lbs per square foot of slab.

| HL93 LOADING | | | SHEE | т | 2 | OF | 2 | |
|---|-----------|--------------------------------|---------------|------|----|-------|------|--|
| Texas Department | oortation | Bridge Division Standard | | | | | | |
| PRESTRESSED CONCRETE BOX BEAM SPANS | | | | | | | | |
| TYPE B20 24' RDWY (WITH SLAB) | | | | | | | | |
| SBB | S-E | 32 | 0-24 | | | | | |
| FILE: bbstds19.dgn | DN: TXE | D0T | CK: TXDOT DW: | TxD0 | T | ск: 7 | xD0T | |
| CTxDOT December, 2006 | CONT | SECT | JOB | | HI | GHWAY | | |
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| 10-15: Table of Est Quantities, Notes. | DIST | | COUNTY | | | SHEET | NO. | |
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| ELASTOMETRIC BEARING DIMENSIONS | | | | | | | | |
|------------------------------------|------|--------|--------|------------|------|--|--|--|
| BEARING | BEAM | ONE BI | EARING | TW BEAR | INGS | | | |
| TYPE | TYPE | L | w | L | w | | | |
| B20-"N" | 4B20 | 6" | 12" | 6" | 6" | | | |
| | 5B20 | 6" | 12" | 6" | 6" | | | |
| B28-"N" | 4B28 | 6" | 14" | 6" | 7" | | | |
| D20- N | 5B28 | 6" | 14" | 6" | 7" | | | |
| B34-"N" | 4B34 | 6" | 16" | 6" | 8" | | | |
| 634- N | 5B34 | 6" | 16" | 6" | 8" | | | |
| B40-"N" | 4B40 | 6" | 20" | 6" | 10" | | | |
| 640- N | 5B40 | 6" | 20" | 6" | 10" | | | |

GENERAL NOTES:

Set beams on elastomeric bearings of the dimensions shown. Center bearings as near nominal £ bearing as possible within limits shown.

Constant thickness bearings may be used for moderate beam slopes up to 0.0113 ft/ft. For skewed supports, Bearings beveled for beam

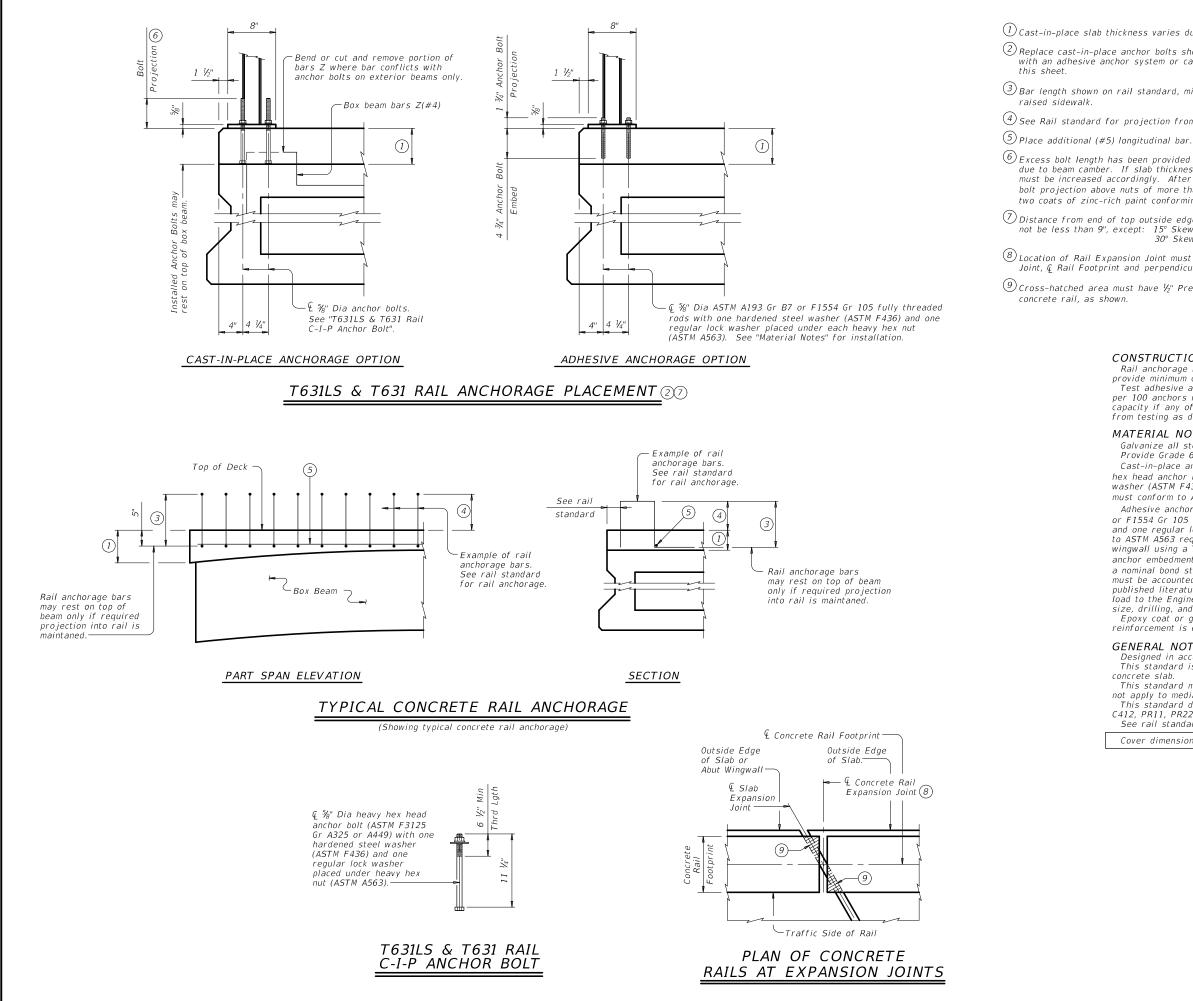
slope may not provide uniform contact. However, predicted contact is considered within allowable tolerances.

Shop drawings for approval are required. A bearing layout which identifies location and orientation of all bearings will be developed by the bearing fabricator. Permanently mark each bearing in accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer.

Cost of furnishing and installing elastomeric bearings is to be included in unit price bid for "Prestressed Concrete Box Beams". Details are drawn showing right forward skew.

See Bridge Layout for actual direction. These details are applicable for skews up to 30 degrees only.

| | HLS | 93 L(| DAD | ING | | | | | |
|-----------|---|--------------|------|-----------|---------|-------|---------|--|--|
| 7 | ■ Texas Department of Transportation ELASTOMERIC | | | | | | | | |
| | ELAS BEARII PRESTR CO | ٧G | D | ETA | ĪL | - | S | | |
| | | | | BB | Εl | В | | | |
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(1) Cast-in-place slab thickness varies due to beam camber (5" minimum)

2 Replace cast-in-place anchor bolts shown on T631LS or T631 Rail standard with an adhesive anchor system or cast-in-place anchor bolts shown on

3 Bar length shown on rail standard, minus 1 ¼". Adjust bar length for a

4 See Rail standard for projection from finished grade or top of sidewalk.

(6) Excess bolt length has been provided to accommodate a variable slab thickness due to beam camber. If slab thickness on span details exceed 10", bolt length must be increased accordingly. After posts have been set and bolts tightened, bolt projection above nuts of more than $\frac{1}{2}$ " must be cut off and painted with two coats of zinc-rich paint conforming to the Item 445 "Galvanizing".

 \bigodot Distance from end of top outside edge of slab to center of first bolt group can not be less than 9", except: 15° Skew: 1'-0" (acute corner only) 30° Skew: 1'-3" (acute corner only)

 $^{(8)}$ Location of Rail Expansion Joint must be at the intersection of m c Slab Expansion Joint, Rail Footprint and perpendicular to slab outside edge.

(9)Cross-hatched area must have V_2 " Preformed Bitumuminous Fiber Material under

CONSTRUCTION NOTES:

Rail anchorage bars may be field bent as required to clear rail reinforcing or provide minimum cover shown on standard rail detail sheets. Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

MATERIAL NOTES:

Galvanize all steel components of steel rail system.

Provide Grade 60 reinforcing steel.

Cast-in-place anchorage system for T631LS and T631 Rail must be 3/8" Dia heavy hex head anchor bolts (ASTM F3125 Gr 325 or A449) with one hardened steel washer (ASTM F436) and one regular lock washer placed under heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed anchor bolts 4 1/2" minimum.

Adhesive anchors for T631LS and T631 Rail must be 5/8" Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 $\frac{3}{4}$ ". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing." Epoxy coat or galvanize reinforcing steel shown on this standard if rail reinforcement is epoxy coated or galvanized.

GENERAL NOTES:

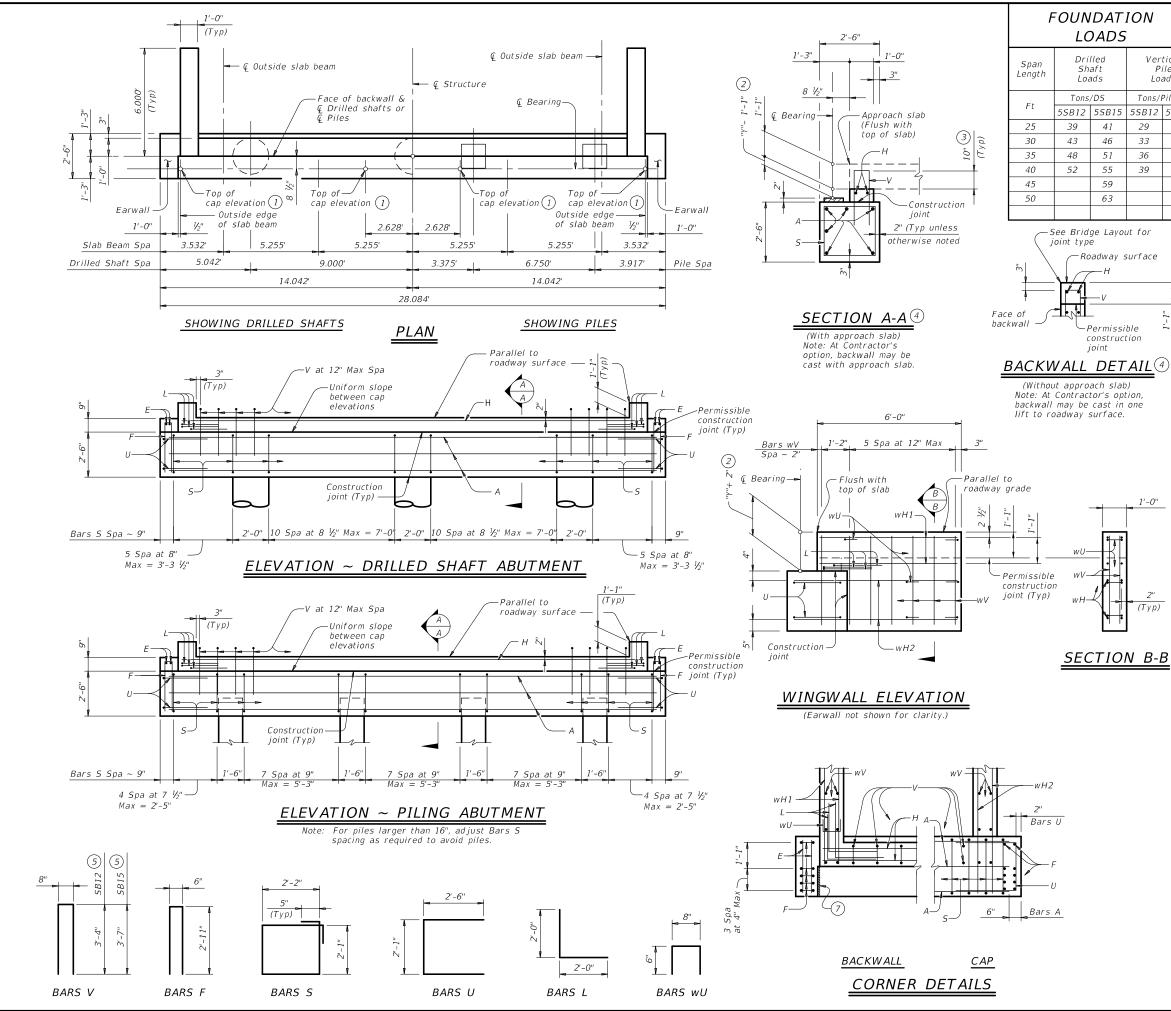
Designed in accordance with AASHTO LRFD Bridge Design Specifications. This standard is for use with structures with a 5" minimum cast-in-place concrete slab.

This standard may require modification for interior rails. This standard does not apply to median barriers.

This standard does not provide details for Type T221P, T224, T80HT, T80SS, C412, PR11, PR22 and PR3 rails on box beam bridges. See rail standards for approved speed restrictions, notes and details not shown.

Cover dimensions are clear dimensions, unless noted otherwise.

| Texas Department | ortation | Bridge Division Standard | | | | | |
|---|-------------|--------------------------------|---------------|-----|-----------|--|--|
| RAIL ANCHORAGE | | | | | | | |
| DETAILS | | | | | | | |
| PRESTR CONC BOX BEAMS | | | | | | | |
| (WI | ΤН | SL | .AB) | | | | |
| | | E | BRAS | | | | |
| FILE: bbstde09-18.dgn | DN: TXL | DOT | CK: TXDOT DW: | JTR | ск: ЈМН | | |
| ©TxD0T December 2006 | CONT | SECT | JOB | | HIGHWAY | | |
| REVISIONS 04-90: Updated for new rails. 01-12: rails anchor bars. | 0911 | 28 | 049,ETC. | CR | | | |
| 07-14: Removed T101 & T6 Added T631 | DIST COUNTY | | | | SHEET NO. | | |
| 03-16: Class D, E, or F epoxy in material notes, T221P & T224 in general notes. 03-18: Updated adhesive anchor notes. | LFK | | HOUSTON | | 81 | | |



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| 405 | | | | | | | | | |
|--------------|---------------------------|-------|--|--|--|--|--|--|--|
| ed t s | Vertical Pile Loads | | | | | | | | |
| S | Tons/Pile | | | | | | | | |
| SB15 | 5SB12 | 5SB15 | | | | | | | |
| 41 | 29 | 31 | | | | | | | |
| 46 | 33 | 34 | | | | | | | |
| 51 | 36 | 38 | | | | | | | |
| 55 | 39 | 41 | | | | | | | |
| 59 | | 44 | | | | | | | |
| 63 | | 47 | | | | | | | |
| | | | | | | | | | |

| T | TABLE OF ESTIMATED | | | | | | | | | |
|-----|--------------------|--------|-------|--------|---|--|--|--|--|--|
| | C | QUANT | ITIES | | | | | | | |
| No. | Size | Length | (5) | Weight | 5 | | | | | |
| NO. | JIZE | | | | | | | | | |

| Par | No | Sizo | Length (5) | |) | Weight | 3 | |
|--------|---|---|--|---|---|--|--|--|
| Dai | NO. | 5120 | 5SB12 | 551 | B15 | 5SB12 | 5SB15 | |
| А | 6 | #11 | 27'-1" | 2 | 7'-1" | 863 | 863 | |
| Ε | 4 | #4 | 2'-2" | | 2'-2" | 6 | 6 | |
| F | 10 | #4 | 6'-4'' | | 6'-4" | 43 | 43 | |
| Н | 2 | #5 | 25'-8" | 2 | 5'-8" | 54 | 54 | |
| L | 6 | #6 | 4'-0" | | 4'-0'' | 36 | 36 | |
| 5 | 34 | #4 | 9'-4" | | 9'-4'' | 212 | 212 | |
| U | 4 | #6 | 7'-1" | | 7'-1" | 43 | 43 | |
| V | 25 | #5 | 7'-4" | 7' | '-10'' | 191 | 204 | |
| wH1 | 8 | #6 | 5'-8'' | | 5'-8" | 68 | 68 | |
| wH2 | 8 | #6 | 6'-11'' | 6 | '-11" | 83 | 83 | |
| wU | 12 | #4 | 1'-8" | | 1'-8" | 14 | 14 | |
| wV | 28 | #5 | 3'-10" | | 4'-1" | 112 | 119 | |
| | | | | | | | | |
| Reinfo | rcing St | eel | | | Lb | 1,725 | 1,745 | |
| CI "C" | Conc (Al | but) | | | СҮ | 8.8 | 9.2 | |
| | E F H L S U V WH1 WH2 WU WV Reinfo | A 6 E 4 F 10 H 2 L 6 S 34 U 4 V 25 wH1 8 wU 12 wV 28 Reinforcing St | A 6 #11 E 4 #4 F 10 #4 H 2 #5 L 6 #6 S 34 #4 U 4 #6 V 25 #5 wH1 8 #6 wH2 8 #6 wU 12 #4 | Bar No. Size 55B12 A 6 #11 27'-1" E 4 #4 2'-2" F 10 #4 6'-4" H 2 #5 25'-8" L 6 #6 4'-0" S 34 #44 9'-4" U 4 #6 7'-1" V 25 #5 7'-4" wH1 8 #6 6'-11" wU 12 #4 1'-8" wV 28 #5 3'-10" Reinforcing Steel 5 3'-10" | Bar No. Size 55B12 55B1 | Bar No. Size $5SB12$ $5SB15$ A 6 #11 $27'-1"$ $27'-1"$ E 4 #4 $2'-2"$ $2'-2"$ F 10 #4 $6'-4"$ $6'-4"$ H 2 #5 $25'-8"$ $25'-8"$ L 6 #6 $4'-0"$ $4'-0"$ S 34 #4 $9'-4"$ $9'-4"$ U 4 #6 $7'-1"$ $7'-1"$ V 25 #5 $7'-4"$ $7'-10"$ wH1 8 #6 $6'-11"$ $6'-11"$ wU 12 #4 $1'-8"$ $1'-8"$ wV 28 #5 $3'-10"$ $4'-1"$ Reinforcing Steel Lb Lb Lb | Bar No. SIZe $5SB12$ $5SB13$ $5SB12$ A 6 #11 $27'-1"$ $27'-1"$ 863 E 4 #4 $2'-2"$ $2'-2"$ 6 F 10 #4 $6'-4"$ $6'-4"$ 43 H 2 #5 $25'-8"$ $25'-8"$ 54 L 6 #6 $4'-0"$ $4'-0"$ 36 S 34 #4 $9'-4"$ $9'-4"$ 212 U 4 #6 $7'-1"$ $7'-1"$ 43 V 25 #5 $7'-4"$ $7'-10"$ 191 wH1 8 #6 $6'-11"$ $6'-11"$ 83 wH2 8 #6 $6'-11"$ $6'-11"$ 83 wU 12 #4 $1'-8"$ $1'-8"$ 14 wV 28 #5 $3'-10"$ $4'-1"$ 112 Reinforcing Steel Lb | |

(1) Top of cap elevations are based on section depths shown on Span Details.

(2) See Span Details for "Y".

(3) Increase as required to maintain 3" from finished grade.

- (4) See Bridge Layout to determine if approach slab is present.
- 5 See Bridge Layout for beam type used in the superstructure.
- (6) Quantities shown are for one abutment only (with approach slab). Without approach slab, add 1.0 CY Class "C" concrete and 54 Lb reinforcing steel for 2 additional Bars H.
- (7) 1/2" preformed bituminous fiber material between slab beam and earwall. Bond to earwall with an approved adhesive. Cast inside face of earwall perpendicular to cap. (Typ)

GENERAL NOTES:

*

Designed according to AASHTO LRFD Bridge Design Specifications. Designed for a normal embankment header slope

- Designed for a normal embankment header slope of 3:1 and a maximum span length of 50 feet. See Bridge Layout for header slope and foundation type, size, and length. See Common Foundation Details (FD) standard sheet for all foundation details and notes. See Concrete Riprap (CRR) standard sheet or Stone Riprap (SRR) standard sheet for riprap attachment details if applicable.
- details, if applicable. See applicable rail details for rail anchorage in
- wingwalls. These abutment details may be used with standard SPSB-24 only.

Cover dimensions are clear dimensions, unless noted otherwise.

Reinforcing bar dimensions shown are out-to-out of bar. MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere in

the plans. Provide Grade 60 reinforcing steel.

Texas Department of Transportation

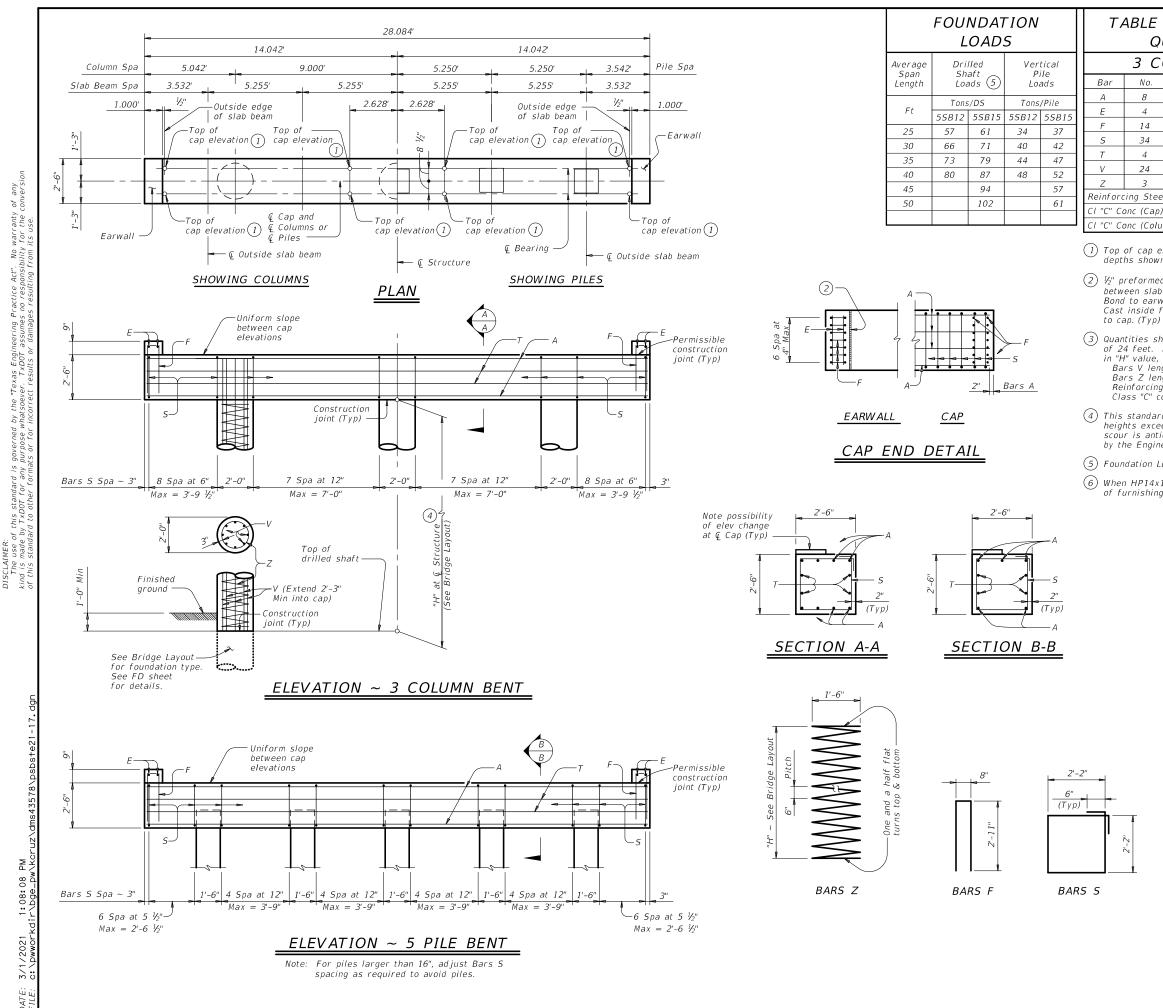
HL93 LOADING

Bridge Division Standard

ABUTMENTS PRESTR CONCRETE SLAB BEAM

24' ROADWAY

| | APSB-24 | | | | | | | |
|---|---------|-----------|--------|----|-----------|----|----------|--|
| FILE: psbste09-17.dgn DN: TxDOT CK: TxDOT DW: TxDOT CK: TxD | | | | | | | | |
| ©TxDOT January 2017 | CONT | SECT | JOB | | HIGHWAY | | IWAY | |
| REVISIONS | 0911 | 28 | 049,ET | с. | CR | | R | |
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TABLE OF ESTIMATED QUANTITIES 3 3 COLUMN BENT

| | JLUM | IN E | | 1 |
|--------|------|------|-------|--------|
| No. | Size | Len | gth | Weight |
| 8 | #11 | 27 | '-9" | 1,180 |
| 4 | #4 | 2 | "-2" | 6 |
| 14 | #4 | 6 | '-6" | 61 |
| 34 | #5 | 9 | '-8'' | 343 |
| 4 | #5 | 27 | '-9" | 116 |
| 24 | #7 | 26 | '-3" | 1,288 |
| 3 | #3 | 242 | "-2" | 273 |
| Stee | / | | Lb | 3,267 |
| (Cap) | | | СҮ | 6.6 |
| (Colui | nn) | | СҮ | 8.4 |

(1) Top of cap elevations are based on section depths shown on Span Details.

- (2) 1/2" preformed bituminous fiber material between slab beam and earwall.
 - Bond to earwall with an approved adhesive. Cast inside face of earwall perpendicular
- (3) Quantities shown are based on an "H" value of 24 feet. For each linear foot variation in "H" value, make the following adjustments: Bars V length, 1'-0"
 - Bars Z length, 9'-6"
 - Reinforcing Steel, 60 Lb Class "C" conc (column), 0.35 CY

TABLE OF ESTIMATED QUANTITIES 5 PILE BENT

| Bar | No. | Size | Ler | ngth | Weight |
|-----------|-----------|------|-----|------|--------|
| А | 5 | #11 | 27' | -9" | 737 |
| Ε | 4 | #4 | 2' | -2" | 6 |
| F | 14 | #4 | 6' | -6" | 61 |
| S | 34 | #5 | 9' | -8" | 343 |
| Т | 4 | #5 | 27' | -9" | 116 |
| | | | | | |
| | | | | | |
| Reinford | ing Stee | 1 | | Lb | 1,263 |
| CI "C" Co | onc (Cap) | | | СҮ | 6.6 |
| | | | | | |

TABLE OF MAXIMUM ALLOWABLE EXPOSED PILE HEIGHTS AND PILE LOADS ④

| Pile | Туре | Max Ht | Max Load |
|----------|------------|--------|-----------|
| Concrete | Steel | Ft | Tons/Pile |
| 16" Sq | HP14x73 | 16 | 75 |
| 18" Sq | HP14x117 6 | 20 | 90 |

(4) This standard may not be used for "H" heights exceeding 24 feet or exposed pile heights exceeding the values shown in the table. In areas of very soft soil or where scour is anticipated, allowable "H" heights or exposed pile heights must be evaluated by the Engineer prior to the use of this standard.

(5) Foundation Loads based on "H" = 24 feet.

6 When HP14x117 steel piling is specified in the plans, the Contractor has the option of furnishing either HP14x117 or HP16x101 steel piling.

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Bent selected must be based on the average span length rounded up to the next 5-foot increment.

For pile bents supporting unequal spans, the shorter span cannot be less than 80 percent of the longer span.

See Bridge Layout for foundation type, size, and length. See Common Foundation Details (FD) standard sheet for all foundation details and notes.

These bent details do not support the use of multi-pile footings shown on the FD standard.

These bent details may be used with standard SPSB-24 only.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

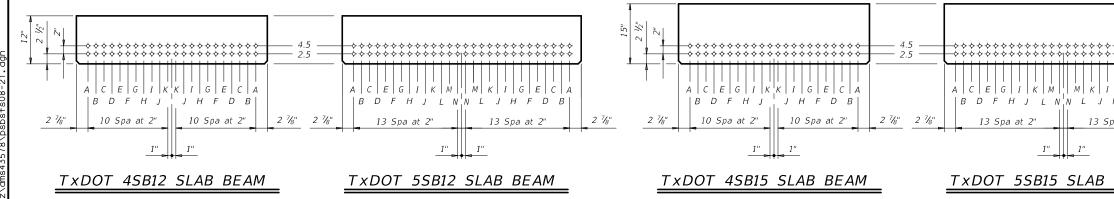
MATERIAL NOTES:

Provide Class C concrete (f'c = 3,600 psi). Provide Class C (HPC) concrete if shown elsewhere in the plans

Provide Grade 60 reinforcing steel.

| HL93 | 3 LO, | 4DI | NG | | | |
|-----------------------|--------|------|--------------|-------|------|-------------------------|
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| PRESTR CON | CRE | TE | E SLA | ١B | ΒE | AM |
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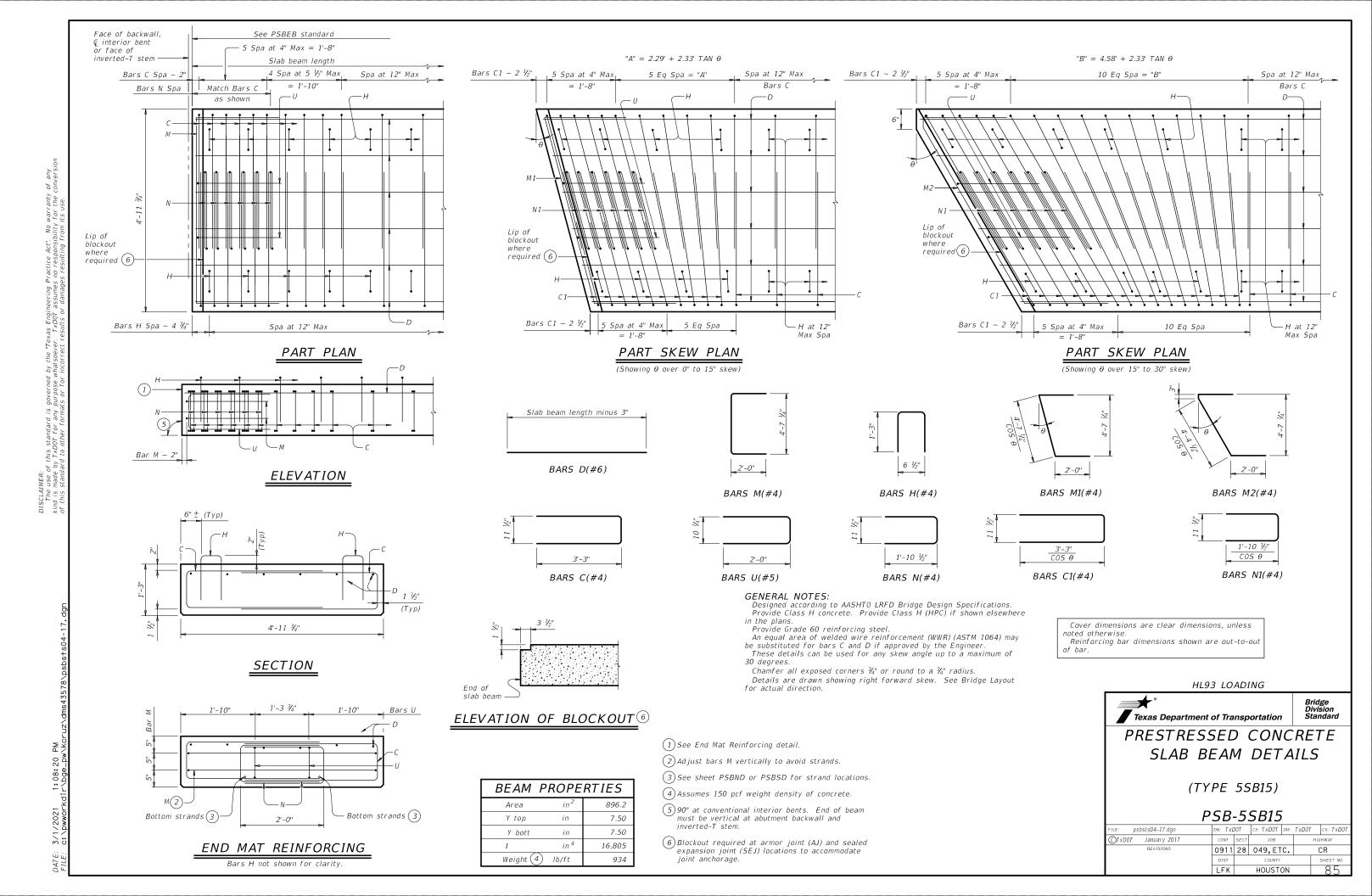
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| | | 1 | | | | | | | | (STRA | IGHT S | TRAND | | | | | | | | | | | | AL DESIG | | | LC | AD RA | TING | |
| STRUCTURE | SPAN LENGTH (ft) | BEAM NO. | BE TY | AM NON PE STL STRA PATTE | D T AND | | | SSING STRGTH fpu (ksi) | STRANDS "e" (in) | "e" END (in) | DEB | DIST FROM BOTTOM (in) | NC | ONDED STR . OF ANDS DE- BONDED | NUM | IBER OF DEBON (ft fro | F STRA | <u> </u> | | GTH 2 D s | TE INIMUM 28 DAY COMP STRGTH f'c (ksi) | DESIGN LOAD COMP STRESS (TOP Q) (SERVICE I) fct (ksi) | DESIGN LOAD TENSILE STRESS (BOTT @) (SERVICE III) fcb (ksi) | REQUIRED MINIMUM ULTIMATE MOMENT CAPACITY (STRENGTH I) (kip-ft) | DISTR | LOAD IBUTION CTOR 2) Shear | STRE | NGTH I Opr | SERVICE III Inv | |
| 24' ROADWAY SB12 BEAM | 25 30 35 40 | ALL ALL ALL ALL | 5S 5S | B12 B12 B12 B12 B12 | | 8 10 14 18 | 0.6 0.6 0.6 0.6 | 270 270 270 270 270 | 3.50 3.50 3.50 3.50 3.50 | 3.50 3.50 3.50 3.50 3.50 | | 2.5 2.5 2.5 2.5 2.5 | 8 10 14 18 | 0 0 0 0 | 0 0 | 0 0 0 0 0 0 0 0 | 0 0 | 0 | 4.0 4.0 4.0 | 00 . 00 . | 5.000 5.000 5.000 5.000 | 0.914 1.292 1.730 2.218 | -1.217 -1.685 -2.219 -2.796 | 448 530 675 820 | 0.450 0.450 0.450 0.440 | 0.450 0.450 0.450 0.440 | 1.40 1.25 1.33 1.34 | 1.82 1.62 1.73 1.74 | 1.71 1.29 1.23 1.12 | |
| 24' ROADWAY SB15 BEAM | 25 30 35 40 45 50 | ALL ALL ALL ALL ALL ALL | 5S 5S 5S 5S | B15 B15 B15 B15 B15 B15 B15 | | 8 8 10 14 18 24 | 0.6 0.6 0.6 0.6 0.6 0.6 | 270 270 270 270 270 270 270 | 5.00 5.00 5.00 5.00 5.00 5.00 | 5.00 5.00 5.00 5.00 5.00 5.00 | 0 0 0 0 0 0 0 0 0 2 | 2.5 2.5 2.5 2.5 2.5 2.5 2.5 | 8 8 10 14 18 24 | 0 0 0 2 8 | 0 0 0 2 | 0 0 0 0 0 0 0 0 0 0 4 0 | | | 4.0 4.0 4.0 4.0 4.0 4.0 | 00 00 00 00 | 5.000 5.000 5.000 5.000 5.000 5.000 | 0.725 1.020 1.361 1.739 2.179 2.680 | -0.897 -1.244 -1.640 -2.068 -2.574 -3.153 | 551 574 708 864 1054 1276 | 0.450 0.450 0.450 0.440 0.440 0.440 | 0.450 0.450 0.450 0.440 0.440 0.440 | 1.77 1.23 1.15 1.32 1.34 1.33 | 2.29 1.59 1.49 1.71 1.73 1.72 | 2.41 1.45 1.14 1.19 1.08 1.11 | 1 Based on the following allowable stresses (ksi): Compression = 0.65 f'ci Tension = 0.24 $\sqrt{f'ci}$ Optional designs must likewise conform. |
| 28' ROADWAY SB12 BEAM | 25 30 35 40 | ALL ALL ALL ALL | 5S 5S | B12 B12 B12 B12 B12 | | 8 10 12 18 | 0.6 0.6 0.6 0.6 | 270 270 270 270 270 | 3.50 3.50 3.50 3.50 3.50 | 3.50 3.50 3.50 3.50 | | 2.5 2.5 2.5 2.5 | 8 10 12 18 | 0 0 0 0 | 0 0 | 0 0 0 0 0 0 0 0 | | 0 | 4.0 4.0 4.0 | 00 . 00 . | 5.000 5.000 5.000 5.000 | 0.903 1.276 1.708 2.200 | -1.184 -1.639 -2.159 -2.744 | 444 508 647 799 | 0.430 0.430 0.430 0.430 | 0.430 0.430 0.430 0.430 0.430 | 1.47 1.32 1.18 1.37 | 1.91 1.71 1.53 1.78 | 1.80 1.37 1.02 1.17 | 2 Portion of full HL93. |
| 28' ROADWAY SB15 BEAM | 25 30 35 40 45 50 | ALL ALL ALL ALL ALL ALL | 5S 5S 5S 5S | B15 B15 B15 B15 B15 B15 B15 | | 8 8 10 14 18 22 | 0.6 0.6 0.6 0.6 0.6 0.6 | 270 270 270 270 270 270 270 | 5.00 5.00 5.00 5.00 5.00 5.00 | 5.00 5.00 5.00 5.00 5.00 5.00 |) 0) 0) 0) 2 | 2.5 2.5 2.5 2.5 2.5 2.5 2.5 | 8 8 10 14 18 22 | 0 0 0 2 6 | 0 0 0 | 0 C 0 C | | | 4.0 4.0 4.0 4.0 4.0 4.0 | 00 00 00 00 | 5.000 5.000 5.000 5.000 5.000 5.000 | 0.716 1.007 1.343 1.725 2.149 2.643 | -0.874 -1.212 -1.598 -2.032 -2.508 -3.073 | 529 570 680 842 1013 1227 | 0.430 0.430 0.430 0.430 0.420 0.420 | 0.430 0.430 0.430 0.430 0.420 0.420 | 1.85 1.29 1.21 1.36 1.41 1.33 | 2.40 1.67 1.57 1.76 1.82 1.72 | 2.53 1.53 1.22 1.24 1.16 1.01 | DESIGN NOTES: Designed according to AASHTO LRFD Bridge Design Specifications. Load rated using Load and Resistance Factor Rating according to AASTHO Manual for Bridge Evaluation. Prestress losses for the designed beams have been calculated for a relative humidity of 60 percent. Optional designs must likewise conform. FABRICATION NOTES: |
| 30' ROADWAY SB12 BEAM | 25 30 35 40 | ALL ALL ALL ALL | 45 45 | B12 B12 B12 B12 B12 | | 6 8 10 14 | 0.6 0.6 0.6 0.6 | 270 270 270 270 270 | 3.50 3.50 3.50 3.50 3.50 | 3.50 3.50 3.50 3.50 | | 2.5 2.5 2.5 2.5 | 6 8 10 14 | 0 0 0 0 | 0 0 | 0 0 | 0 0 | 0 | 4.0 4.0 4.0 | 00 . 00 . | 5.000 5.000 5.000 5.000 | 0.904 1.277 1.711 2.205 | -1.187 -1.646 -2.169 -2.758 | 341 407 518 640 | 0.340 0.340 0.340 0.340 | 0.340 0.340 0.340 0.340 | 1.38 1.32 1.24 1.34 | 1.79 1.71 1.60 1.73 | 1.67 1.37 1.08 1.11 | Provide Class H concrete. Provide Grade 60 reinforcing steel. Use low relaxation strands, each pretensioned to 75 percent of fpu. Full-length debonded strands are not permitted in positions "A" and "B". Strand debonding must comply with Item 424.4.2.2.2.4. When shown on this sheet, the Fabricator has the option of furnishing either the designed beam or an approved optional beam design. All |
| 30' ROADWAY SB15 BEAM | 25 30 35 40 45 50 | ALL ALL ALL ALL ALL ALL | 45 45 45 45 | B15 B15 B15 B15 B15 B15 | | 6 6 8 12 14 18 | 0.6 0.6 0.6 0.6 0.6 0.6 | 270 270 270 270 270 270 270 | 5.00 5.00 5.00 5.00 5.00 5.00 | 5.00 5.00 5.00 5.00 5.00 5.00 | 0 0 0 0 0 0 0 0 0 2 | 2.5 2.5 2.5 2.5 2.5 2.5 2.5 | 6 6 12 14 18 | 0 0 0 2 4 | 0 0 0 | 0 0 0 0 0 0 0 0 2 0 | | 0 0 0 0 | 4.0 4.0 4.0 4.0 4.0 4.0 | 00 00 00 00 | 5.000 5.000 5.000 5.000 5.000 5.000 | 0.723 1.017 1.346 1.729 2.166 2.665 | -0.888 -1.231 -1.605 -2.043 -2.542 -3.115 | 431 438 545 675 823 998 | 0.350 0.350 0.340 0.340 0.340 0.340 | 0.350 0.350 0.340 0.340 0.340 0.340 | 1.69 1.16 1.21 1.47 1.33 1.32 | 2.19 1.50 1.57 1.91 1.73 1.71 | 2.32 1.37 1.21 1.38 1.06 1.02 | optional design submittals and shop drawings must be signed, sealed and dated by a Professional Engineer registered in the State of Texas. Locate strands for the designed beam as low as possible on the 2" grid system unless a non-standard strand pattern is indicated. Fill row "2.5", then row "4.5". Place strands within a row as follows: 1) Locate a strand in each "A" position. 2) Place strand symmetrically about vertical centerline of beam. 3) Space strands in position "A". Distribute debonded strands symmetrically about the vertical centerline. Increase debonded lengths |
| | \$ \$ \$ C E G 3 D F F 10 Spa at 0 Spa At | $\diamond \diamond \diamond \diamond \diamond$ | + | \$ | | 2 7%" 2 7%" | | | ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ ♦ | \rightarrow \Leftrightarrow \Leftrightarrow \Leftrightarrow \Rightarrow G I G I = H J = $=$ $=$ $=$ $=$ $=$ $=$ $=$ $=$ $=$ | K M K M L N $2^{"}$ | ¢ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ | $\diamond \diamond \diamond \diamond$ | • • • • • • • • • • • • • • • • • • • | 2 7/5 | * 15" | <u>2</u> 7/8 | | | фффф | → | \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ | | 4.5 2.5 | \diamond \diamond \diamond \diamond \diamond \diamond \diamond \diamond ϕ | \$\$\$\$ G I k F H J Spa at 2" | > | ¢ | - - - - - - - - - - - - - - | HL93 LOADING |

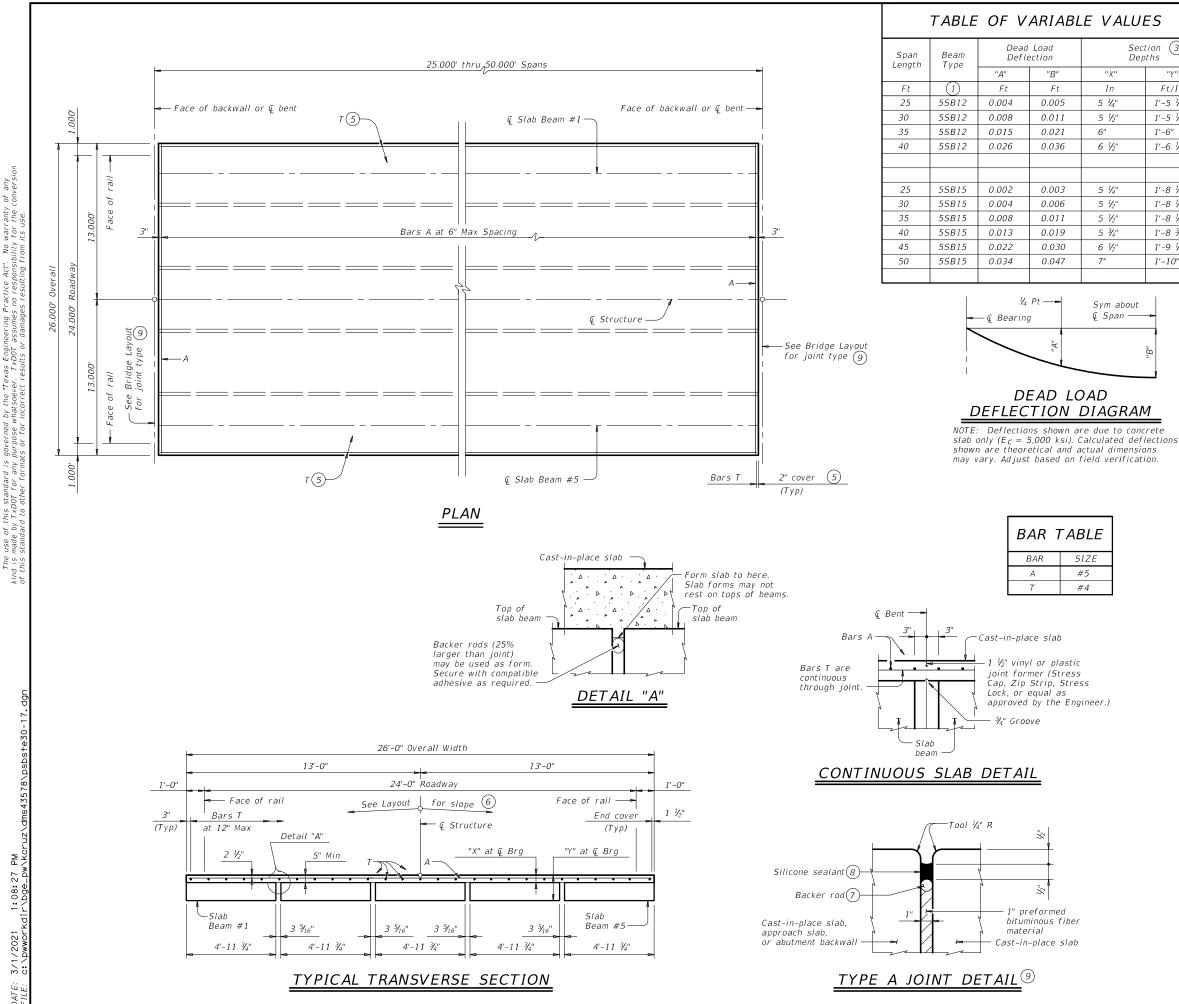


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|--------|---------------------------|----------------------|------------------------------------|--------------------|--------------------|
| SPAN | REINF CONCRETE SLAB | | PRESTR CC SLAB BEA B12 OR 55 | M | TOTAL (2) REINF |
| LENGTH | (SLAB (SLAB BEAM) | ABUT TO INT BT | INT BT TO INT BT | ABUT TO ABUT | STEEL |
| Ft | SF | LF (4) | LF (4) | LF (4) | Lb |
| 25 | 650 | 122.50 | 122.50 | 122.50 | 1,820 |
| 30 | 780 | 147.50 | 147.50 | 147.50 | 2,180 |
| 35 | 910 | 172.50 | 172.50 | 172.50 | 2,550 |
| 40 | 1,040 | 197.50 | 197.50 | 197.50 | 2,910 |
| 45 | 1,170 | 222.50 | 222.50 | 222.50 | 3,280 |
| 50 | 1,300 | 247.50 | 247.50 | 247.50 | 3,640 |

- (1) See Bridge Layout for beam type used in the superstructure. These standards do not provide for the use of both SB12 and SB15 beams within the same structure.
- (2) Reinforcing steel weight is calculated using an approximate factor of 2.8 Lbs/SF.
- (3) Based on theoretical beam camber, dead load deflections of 5" cast-in-place concrete slab and a constant grade. The Contractor will adjust these values for any vertical curve.
- $^{(4)}$ Fabricator will adjust beam lengths for beam slopes as required
- (5) Where slab is continuous over Interior Bents, Bars T are continuous through Joint. See "Continuous Slab Detail".
- (6) This standard does not provide for changes in roadway cross-slopes within the structure.
- 1 ¼" backer rod must be compatible with joint sealant. Use of multiple pieces to create a backer rod cross section is not permitted. Top of backer rod must be convex as shown.
- $\overset{\textcircled{\mbox{(8)}}}{\longrightarrow}$ Class 7 silicone sealant that conforms to DMS-6310. Install when ambient temperature is between 55°F and 85°F and rising. Engineer to determine allowable hours for sealant application.
- (9) See Bridge Layout for expansion joint locations. If using Type A expansion joints, the maximum distance between joints is 100 feet. Type A joints are subsidiary to Item 422, "Concrete Superstructures".

GENERAL NOTES:

Designed according to AASHTO LRFD Bridge Design Specifications. Two- or three-span units, with slab continuous over interior bents. may be formed with the details shown on this sheet. See applicable rail details for rail anchorage in slab.

This standard does not support the use of transition bents.

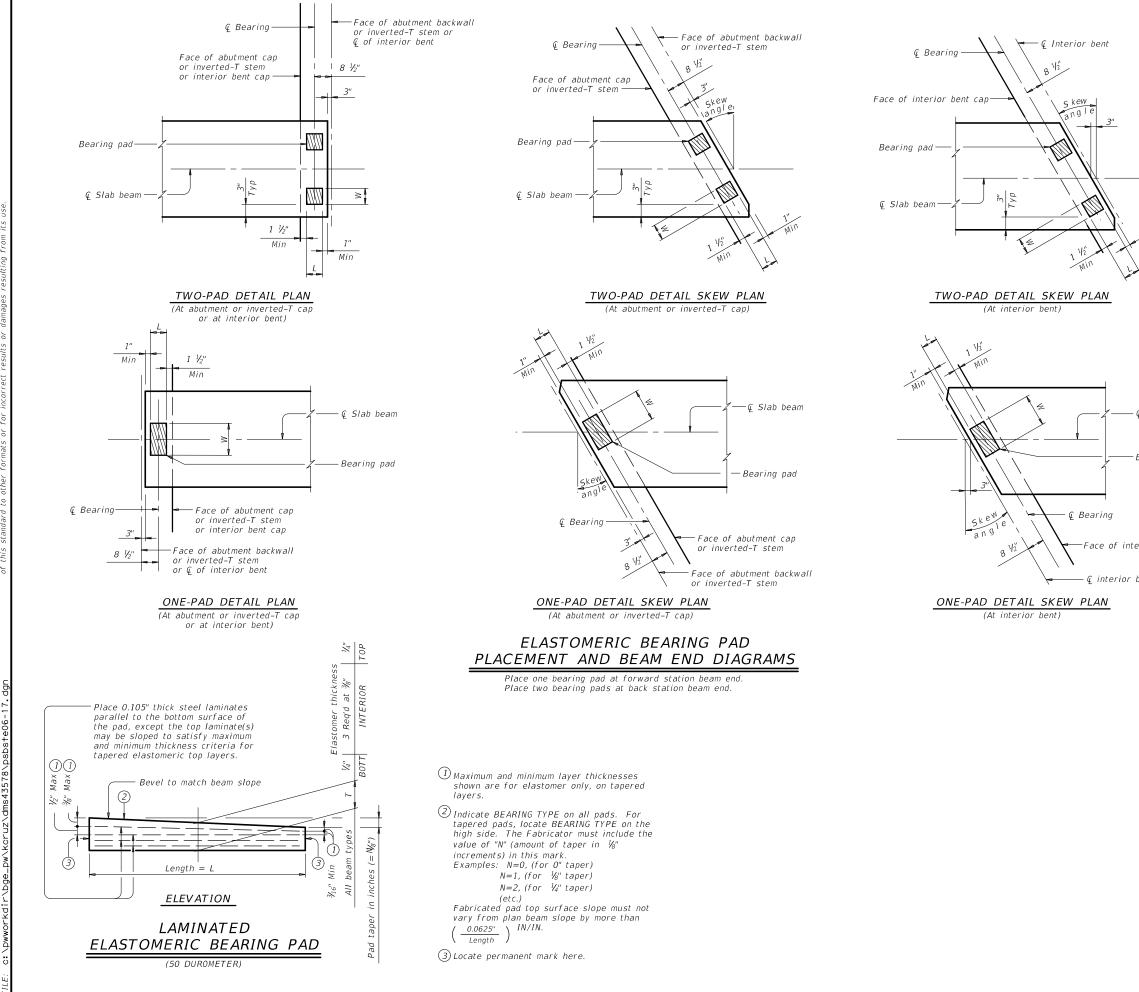
Cover dimensions are clear dimensions, unless noted otherwise.

MATERIAL NOTES:

Provide Class S concrete (f'c = 4,000 psi). Provide Class S (HPC) concrete if shown elsewhere in the plans. Provide Grade 60 reinforcing steel. Provide bar laps, where required, as follows: Uncoated $\sim #4 = 1'-7''$ $\sim #5 = 2'-0''$ Epoxy coated $\sim #4 = 2'-5''$ $\sim #5 = 3'-0'$

Deformed welded wire reinforcement (WWR) (ASTM A1064) of equal size and spacing may be substituted for Bars A or T unless noted otherwise.

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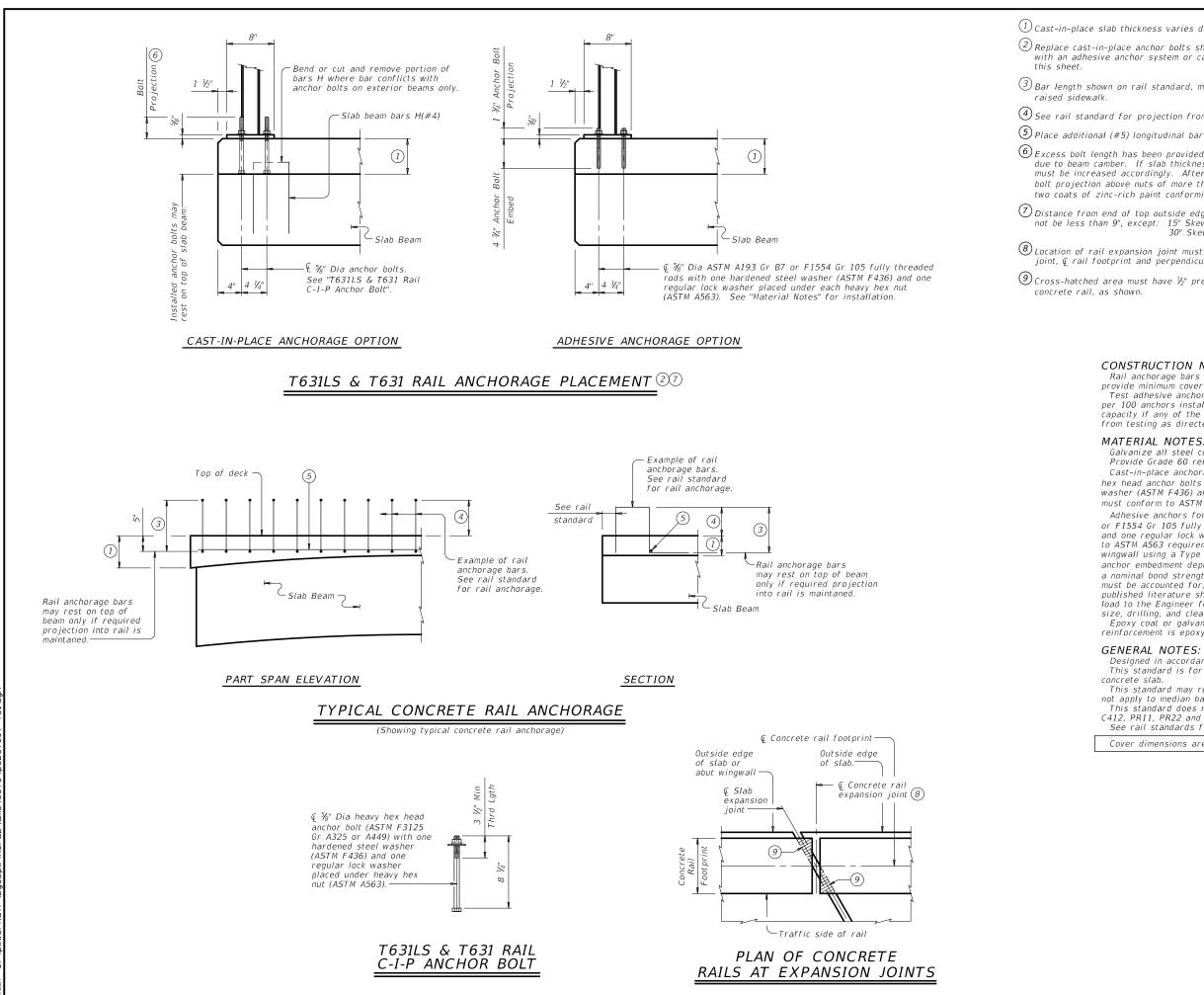


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| | TABLE OF BEARING PAD DIMEN | | |
|----------------|---|--------------------------|-----------------------------|
| | (ALL PRESTR CONC SLAB One-Pad (Ty SB1-"N") (2) Two-Pad | ВМ Г d (Ty SB2 | , |
| | W L T W | L | Т |
| | 14" 7" 2" 7" | 7" | 2" |
| | Pad sizes shown are applicable for following conditions: | the | |
| | (1) All one, two and three span un where the minimum span lengt not less than 25' and the maxi span is not more than 50'. (2) Skews less than or equal to 30 | h is imum | |
| 1" Min | | | |
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| | | | |
| Ç Slab beam | | | |
| Bearing pad | | | |
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| | | | |
| erior bent cap | GENERAL NOTES: These details accommodate skew angles up to 30°. Shop drawings for approval are required | 1. | |
| bent | A bearing layout which identifies location and orientation of all bearings must be developed by the bearing fabricator. Permanently mark each bearing in accordance with the bearing layout. A copy of the bearing layout is to be provided to the Engineer. Cost of furnishing and installing elastom | y | |
| | bearings must be included in unit price bid "Prestressed Concrete Slab Beams". | | |
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(1) Cast-in-place slab thickness varies due to beam camber (5" minimum).

(2) Replace cast-in-place anchor bolts shown on T631LS and T631 Rail standard with an adhesive anchor system or cast-in-place anchor bolts shown on

3 Bar length shown on rail standard, minus 1 ¼". Adjust bar length for a

(4) See rail standard for projection from finished grade or top of sidewalk.

Excess bolt length has been provided to accommodate a variable slab thickness due to beam camber. If slab thickness on span details exceed 7", bolt length must be increased accordingly. After posts have been set and bolts tightened, bolt projection above nuts of more than $\frac{1}{2}$ must be cut off and painted with two coats of zinc-rich paint conforming to the Item 445 "Galvanizing".

Distance from end of top outside edge of slab to center of first bolt group can not be less than 9", except: 15° Skew: 1'-0" (acute corner only) 30° Skew: 1'-3" (acute corner only)

(a) Location of rail expansion joint must be at the intersection of Q slab expansion joint, Q rail footprint and perpendicular to slab outside edge.

(9)Cross-hatched area must have $\frac{1}{2}$ " preformed bitumuminous fiber material under

CONSTRUCTION NOTES:

Rail anchorage bars may be field bent as required to clear rail reinforcing or provide minimum cover shown on standard rail detail sheets. Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

MATERIAL NOTES:

Galvanize all steel components of steel rail system.

Provide Grade 60 reinforcing steel.

Cast-in-place anchorage system for T631LS and T631 Rail must be 3/8" Dia heavy hex head anchor bolts (ASTM F3125 Gr 325 or A449) with one hardened steel washer (ASTM F436) and one regular lock washer placed under heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed anchor bolts 4 1/2" minimum.

Adhesive anchors for T631LS and T631 Rail must be $\frac{5}{8}$ " Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 $\frac{3}{4}$ ". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing." Epoxy coat or galvanize reinforcing steel shown on this standard if rail reinforcement is epoxy coated or galvanized.

GENERAL NOTES:

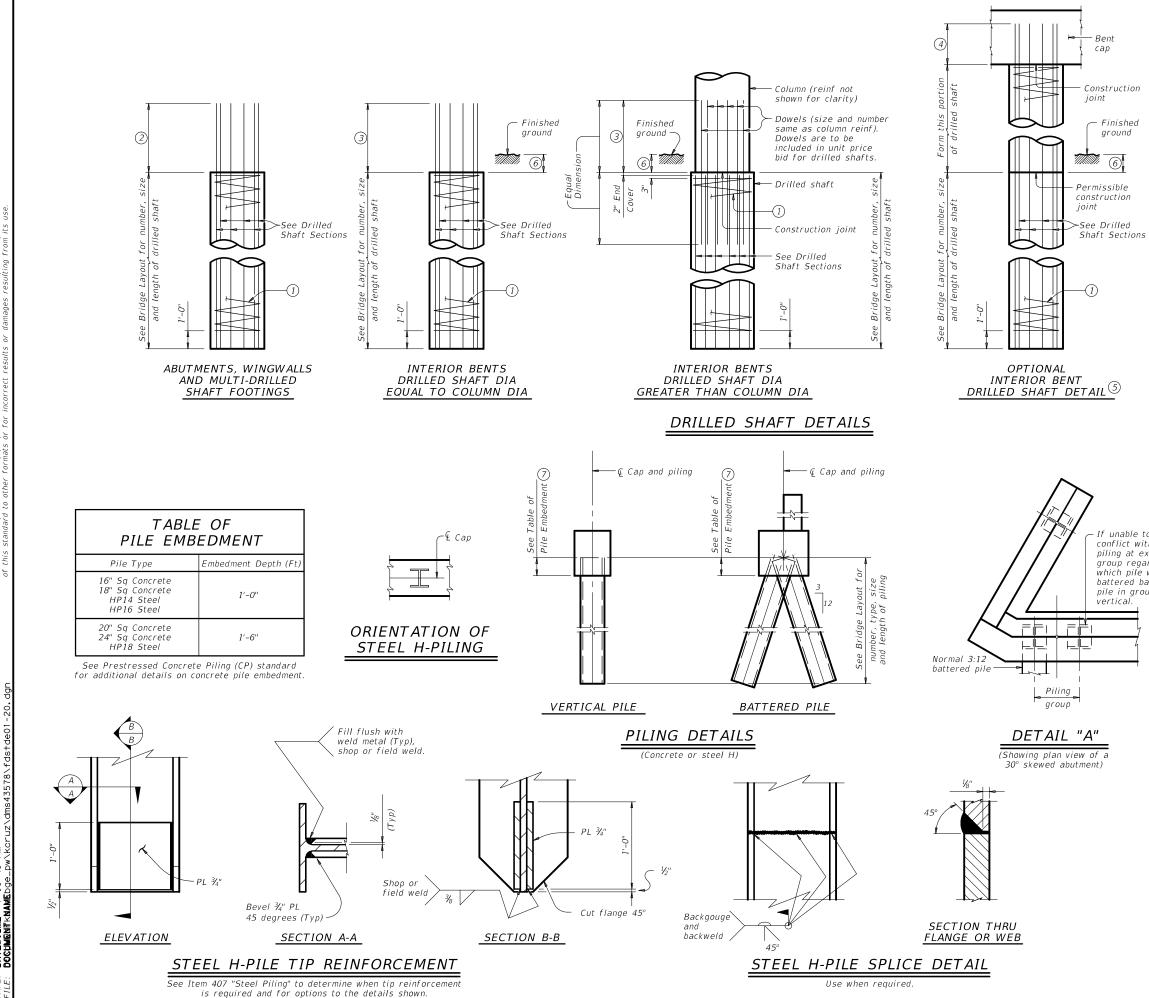
Designed in accordance with AASHTO LRFD Bridge Design Specifications. This standard is for use with structures with a 5" minimum cast-in-place concrete slab.

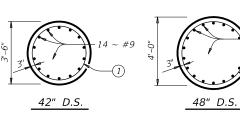
This standard may require modification for interior rails. This standard does not apply to median barriers.

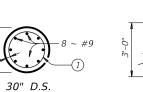
This standard does not provide details for Type T221P, T224, T80HT, T80SS, C412, PR11, PR22 and PR3 rails on slab beam bridges. See rail standards for approved speed restrictions, notes and details not shown.

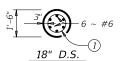
Cover dimensions are clear dimensions, unless noted otherwise.

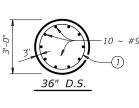
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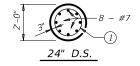








 $18 \sim \#9$



1) #3 spiral at 6" pitch (one and a half flat turns top and bottom).

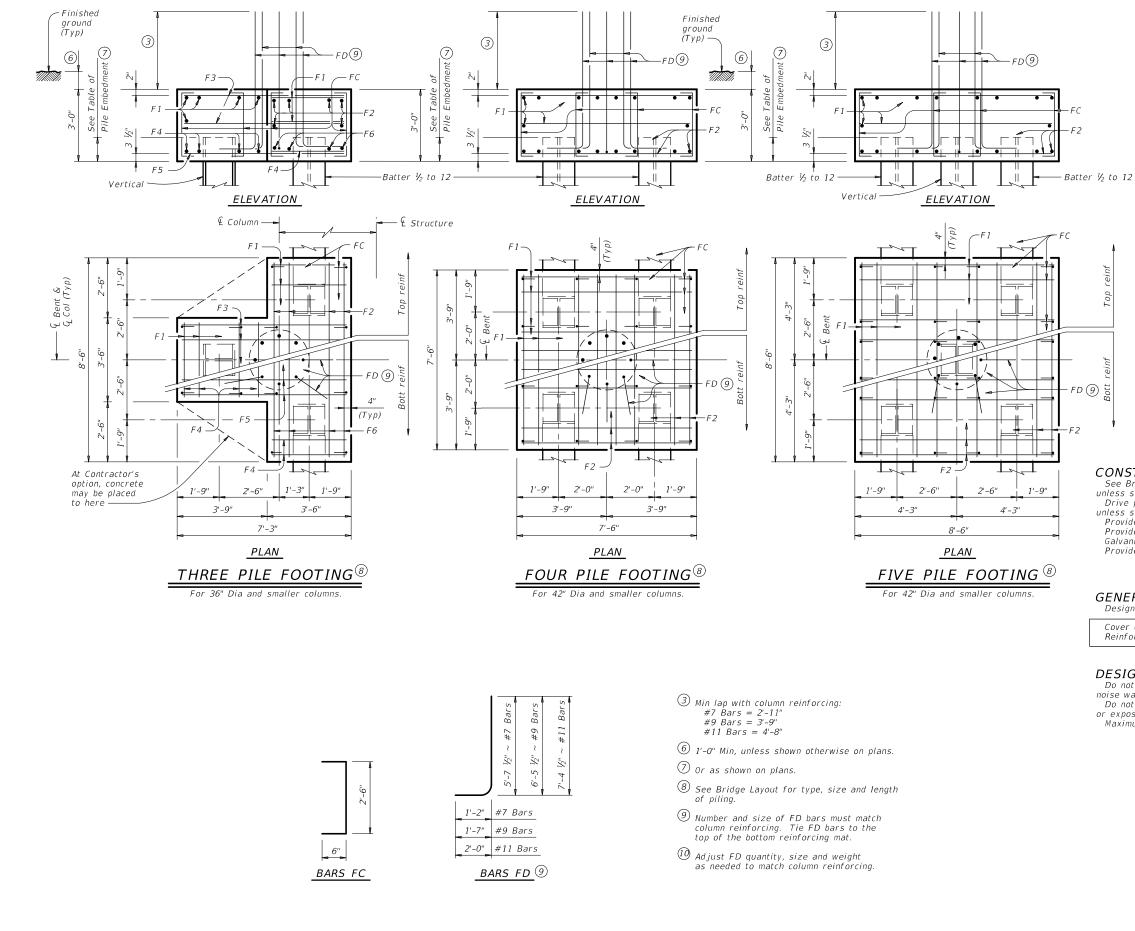
- 2 Min extension into supported element: #6 Bars = 1'-11" #7 Bars = 2'-0" #9 Bars = 2'-3"
- ③ Min lap with column reinf. #7 Bars = 2'-11" #9 Bars = 3'-9" #11 Bars = 4'-8"

DRILLED SHAFT SECTIONS

- (4) Min extension into supported element: #6 Bars = 1'-11" #7 Bars = 2'-3"
- #9 Bars = 2'-9''5 Drilled shafts may extend to the bottom of
- bent caps for "H" heights of 6 ft and less (as shown on the Bridge Layout), if approved. This option can only be used when the drilled shaft diameter equals the column diameter. Obtain approval of the forming method above the ground line prior to construction. No adjustments in payment will be made if this option is used.
- 6 1'-0" Min, unless shown otherwise on plans.
- 🗇 Or as shown on plans.

| SHE | ET 1 | 0 | F 2 | | | | |
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| COMMON FOUNDATION DETAILS | | | | | | | |
| | | | | FĽ |) | | |
| FILE: fdstde01-20.dgn | DN: TXL | D0T | ск: ТхD0Т | DW: | TxD0T | ск: ТхD0Т | |
| ©TxDOT April 2019 | CONT | SECT | JOB | | | HIGHWAY | |
| REVISIONS | 0911 | 28 | 049,ET | С. | | CR | |
| 01-20: Added #11 bars to the FD bars. | DIST | | COUNTY | | | SHEET NO. | |
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If unable to avoid conflict with wingwall piling at exterior pile group regardless of which pile would be battered back, one pile in group may be



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| TABLE OF FOOTING QUANTITIES FOR 30" COLUMNS | | | | | | | | | | |
|---|--------|--------|-----------|-----|--------|--|--|--|--|--|
| ONE 3 PILE FOOTING | | | | | | | | | | |
| Bar | No. | Size | Lengti | ካ | Weight | | | | | |
| F 1 | 11 | #4 | 3'- 2 | u | 23 | | | | | |
| F2 | 6 | #4 | 8'- 2 | u | 33 | | | | | |
| F3 | 6 | #4 | 6'- 11 | " | 28 | | | | | |
| F4 | 8 | #9 | 3'- 2 | u | 86 | | | | | |
| F5 | 4 | #9 | 6'- 11 | " | 94 | | | | | |
| F6 | 4 | #9 | 8'- 2 | u | 111 | | | | | |
| FC | 12 | #4 | 3'- 6 | u | 28 | | | | | |
| FD (10) | 8 | #9 | 8'- 1 | u | 220 | | | | | |
| Reinf | orcing | Steel | | Lb | 623 | | | | | |
| Class | "С" Сс | ncrete | | СҮ | 4.8 | | | | | |
| | | ONE 4 | PILE FOOT | ING | | | | | | |
| Bar | No. | Size | Lengti | h | Weight | | | | | |
| F 1 | 20 | #4 | 7'- 2 | u | 96 | | | | | |
| F2 | 16 | #8 | 7'- 2 | u | 306 | | | | | |
| FC | 16 | #4 | 3'- 6 | u | 37 | | | | | |
| FD (10) | 8 | #9 | 8'- 1 | u | 220 | | | | | |
| Reinf | orcing | Steel | | Lb | 659 | | | | | |
| Class | "С" Сс | ncrete | | СҮ | 6.3 | | | | | |
| | | ONE 5 | PILE FOOT | ING | | | | | | |

CONSTRUCTION NOTES:

See Bridge Layout for foundation type required. Use these foundation details unless shown otherwise. Drive piling under abutment wingwalls to a minimum resistance of 10 Tons/Pile

Bar

F 1

F2

FC

FD (10)

No. Size

8 #9

#4

#9

#4

20

16

24

Reinforcing Steel

Class "C" Concrete

unless shown otherwise.

Provide Class C Concrete (f'c = 3,600 psi), unless shown otherwise. Provide Grade 60 reinforcing steel. Galvanize reinforcing if shown elsewhere in the plans.

Provide bar laps for drilled shaft reinforcing, where required, as follows: Uncoated or galvanized (#6) ~ 2'-6" Uncoated or galvanized (#7) ~ 2'-11" Uncoated or galvanized (#9) ~ 3'-9"

Length

8'- 2"

8'- 2"

3'- 6"

8'- 1"

Lb

СҮ

Weight

109

444

56

220

829

8.0

GENERAL NOTES: Designed according to AASHTO LRFD Bridge Design Specifications.

Cover dimensions are clear dimensions, unless noted otherwise. Reinforcing bar dimensions shown are out-to-out of bar.

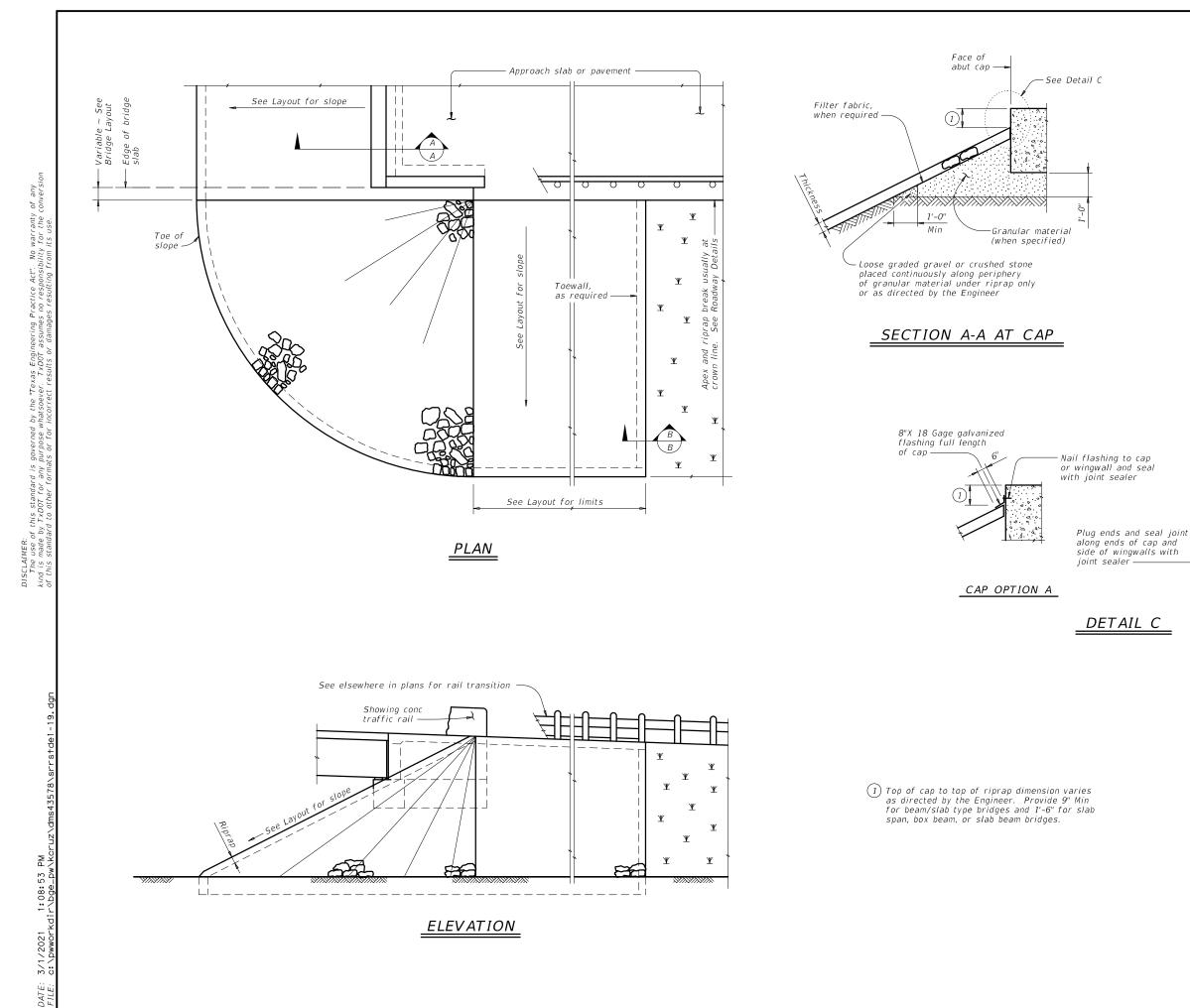
DESIGNER NOTES: Do not use the drilled shaft details shown on this standard for retaining wall,

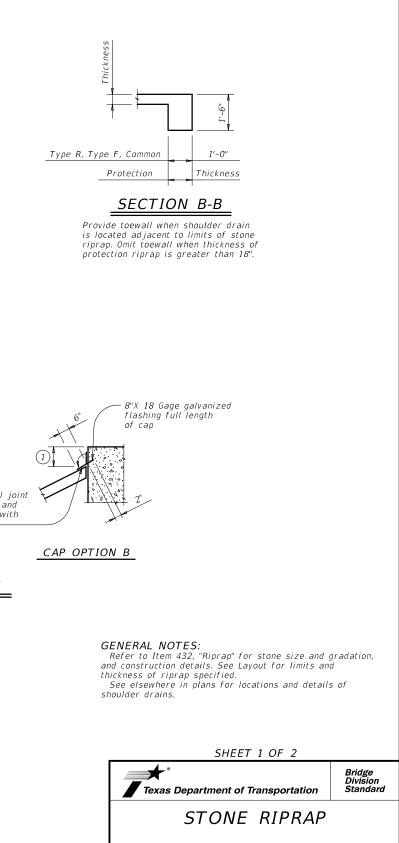
Do not use the footings shown on this standard in direct contact with salt water or exposed to salt water spray.

Maximum allowable pile loads for the footings shown are:

| 51101 | vii ui c. | | | | |
|-------|-----------|------|-----|-----|---------|
| 72 | Tons/Pile | with | 24" | Dia | Columns |
| 80 | Tons/Pile | with | 30" | Dia | Columns |
| 100 | Tons/Pile | with | 36" | Dia | Columns |
| 120 | Tons/Pile | with | 42" | Dia | Columns |
| | | | | | |

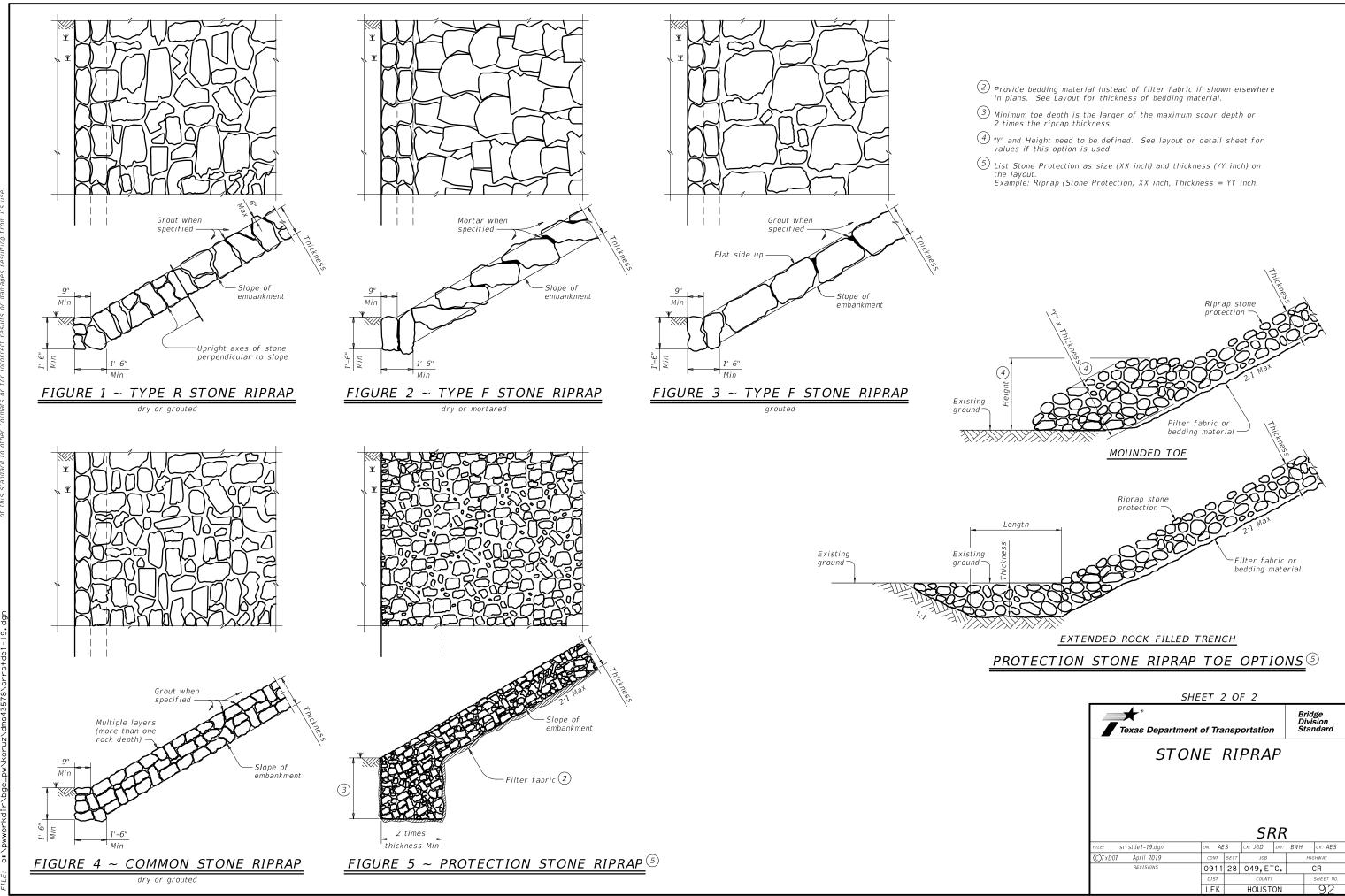
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| ©TxDOT April 2019 | CONT | SECT | JOB | | HIGHWAY | | | |
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| 01-20: Added #11 bars to the FD bars. | DIST | | COUNTY | | SHEET NO. | | | |
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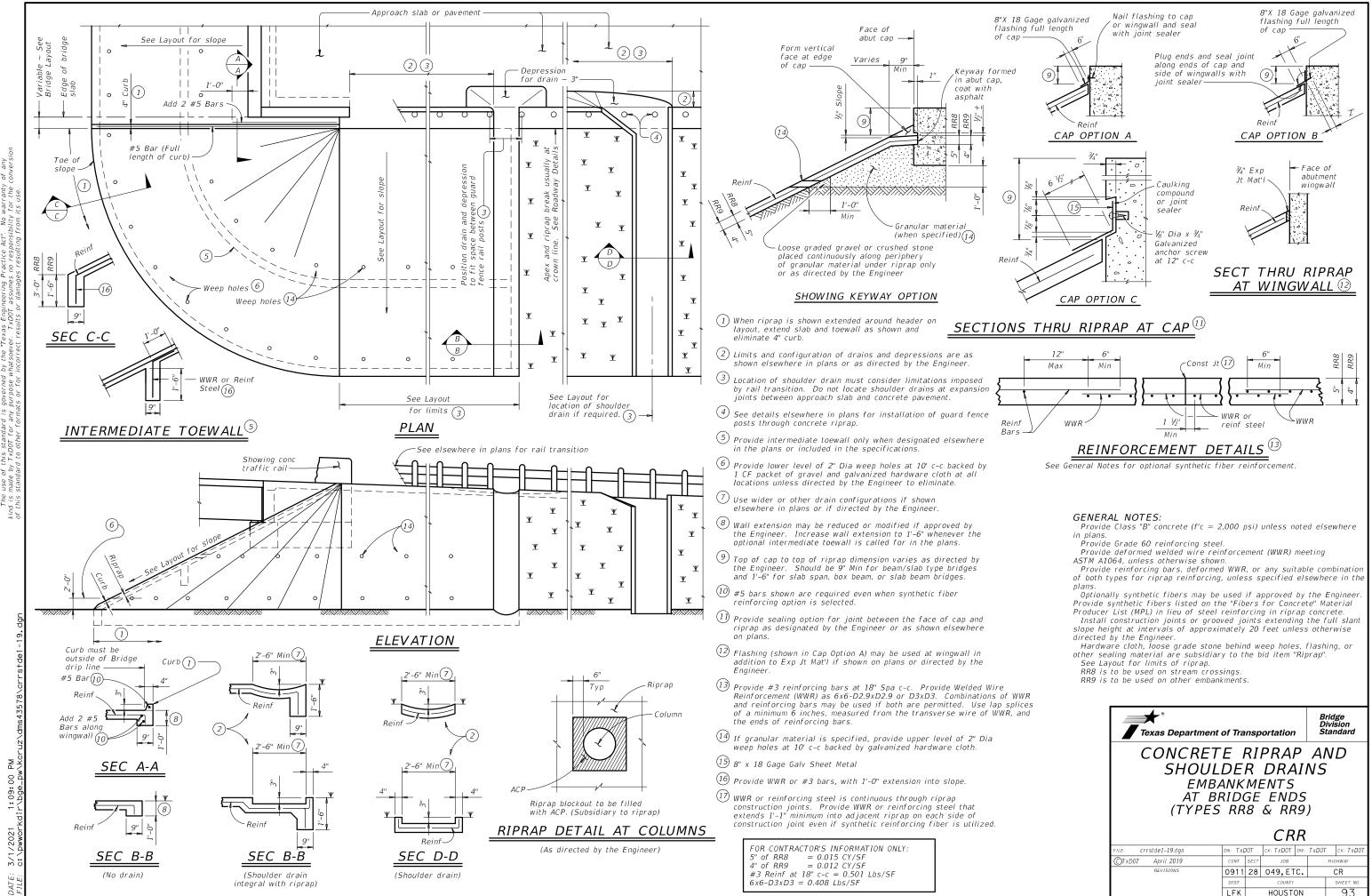




| | SF | R | I. | |
|-----|---------|-----|-----|--|
| AES | ск: JGD | DW: | BWH | |
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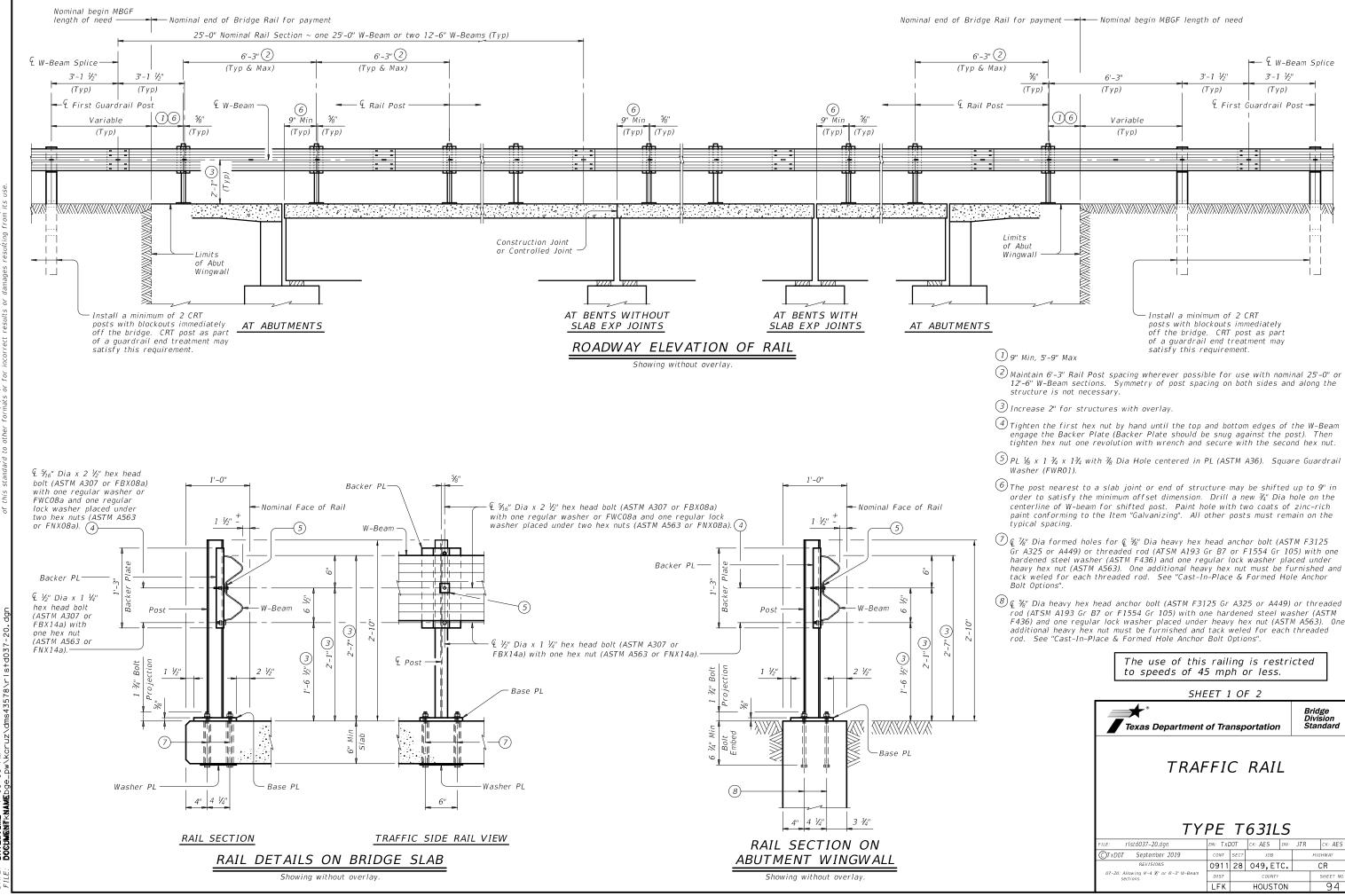
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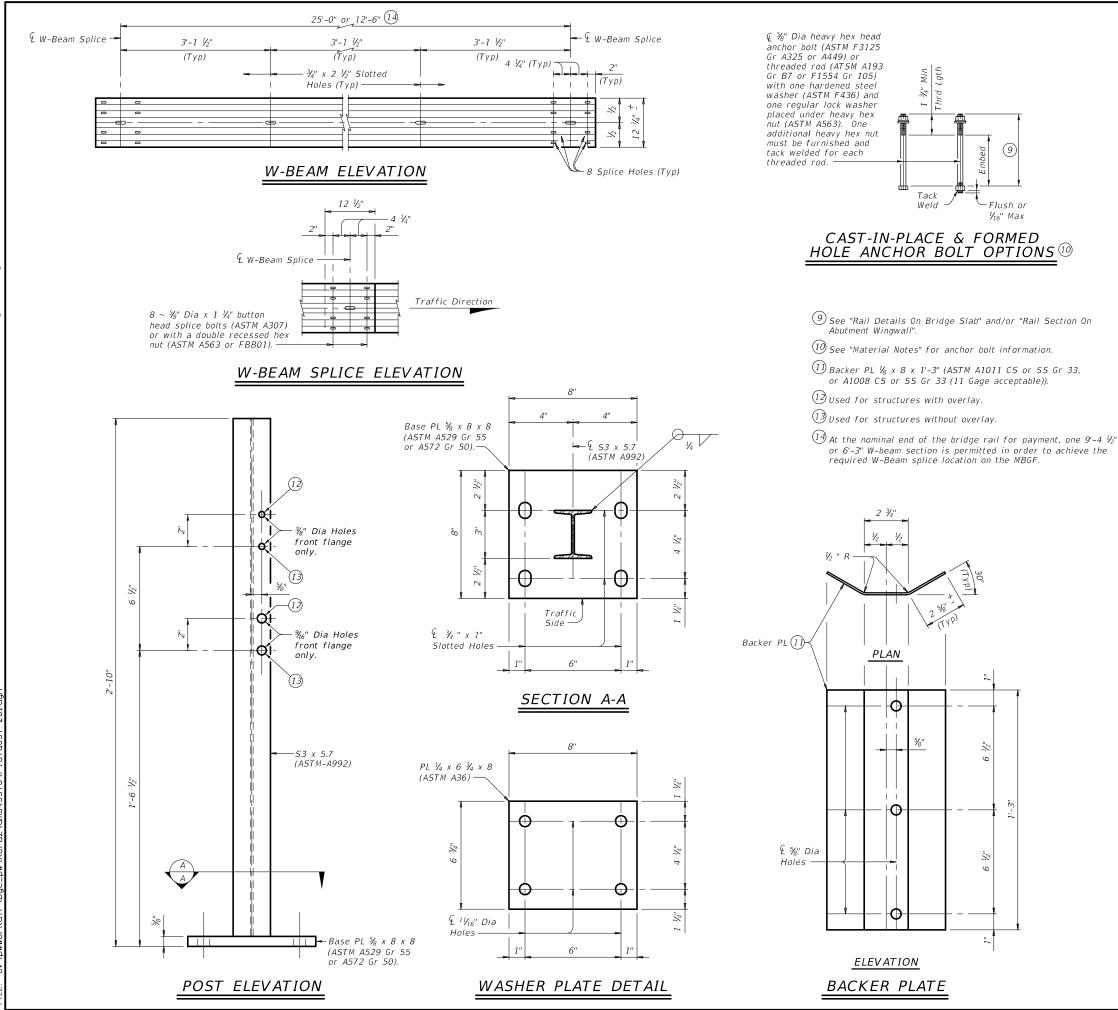


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- F436) and one regular lock washer placed under heavy hex nut (ASTM A553). One additional heavy hex nut must be furnished and tack weled for each threaded

| The use of this railing is restricted to speeds of 45 mph or less. | | | | | | | | | | |
|--|--------------|------|------------|--------|-----------------|--|--|--|--|--|
| SHEET 1 OF 2 | | | | | | | | | | |
| Texas Department of Transportation Standard | | | | | | | | | | |
| | TRAFFIC RAIL | | | | | | | | | |
| FILE: rlstd037-20.dgn | DN: TXE | DOT | CK: AES DI | N: JTR | CK: AES | | | | | |
| ©TxDOT September 2019 | CONT | SECT | JOB | | HIGHWAY | | | | | |
| REVISIONS 07-20: Allowing 9'-4 ½" or 6'-3" W-Beam | 0911 | 28 | 049,ETC | | CR | | | | | |
| or-20: Allowing 9−4 ½° or 6−3° w-Beam sections. | DIST LFK | | HOUSTON | ١ | SHEET NO. 94 | | | | | |



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MBGF AND END TREATMENT NOTES:

This traffic railing must be anchored by metal beam guard fence (MBGF) and/or guard fence end treatments. Determine MBGF length of need in accordance with the Roadway Design Manual, unless otherwise specified. The minimum MBGF length of need required for anchoring the railing is: SGT; or DAT plus 12.5' of MBGF, as applicable. Provide CRT posts as shown in "Roadway Elevation of Rail."

CONSTRUCTION NOTES:

Face of rail post must be plumb unless otherwise approved by the Engineer. Post must be perpendicular to adjacent roadway grade. Use epoxy mortar under post base plates if gaps larger than V_{16} " exist.

Fully anchored guardrail must be attached to each end of rail. A metal beam guard fence transition is not used with this rail. At the Contractor's option anchor bolts may be an adhesive

anchor system. See "Material Notes". Test adhesive anchors in accordance with Item 450.3.3, "Tests". Test 3 anchors per 100 anchors installed. Perform corrective measures to provide adequate capacity if any of the tests do not meet the required test load. Repair damage from testing as directed.

It is recommended to show a Rail Layout with rail posts and W-beam splices. Fabricator must submit erection drawings to the Engineer for approval.

Round or chamfer exposed edges of rail post and backer plate to approximately $~{\mathcal Y}_{\rm 16}{}^{\rm c}$ by grinding.

Shop drawings are not required for this rail.

MATERIAL NOTES:

Galvanize all steel components.

Anchor bolts for base plate must be ½" Dia ASTM F3125 Gr A325 or A449 bolts (or ASTM A193 Gr B7 or F1554 Gr 105 threaded rods with one tack welded heavy hex nut each) with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements.

Optional adhesive anchorage system must be $\frac{7}{6}$ " Dia ASTM A193 Gr B7 or F1554 Gr 105 fully threaded rods with one hardened steel washer (ASTM F436) and one regular lock washer placed under each heavy hex nut. Nuts must conform to ASTM A563 requirements. Embed fully threaded rod into slab and/or abutment wingwall using a Type III, Class C, D, E, or F anchor adhesive. Minimum adhesive anchor embedment depth is 4 $\frac{3}{4}$ ". Anchor adhesive chosen must be able to achieve a nominal bond strength in tension of a single anchor, Na, of 8 kips (edge distance must be accounted for). Submit signed and sealed calculations or the manufacturer's published literature showing the proposed anchor adhesive's ability to develop this load to the Engineer for approval prior to use. Anchor installation, including hole size, drilling, and clean out, must be in accordance with Item 450, "Railing."

W-beam must meet the requirements of Item 540, "Metal Beam Guard Fence" except as modified in the plans. The Contractor may furnish rail elements of 25-0" or 12'-6" (Nominal) lengths and a single rail element of 9'-4 $\frac{1}{2}$ " or 6'-3" (Nominal) length.

W-Beam must have slotted holes at 3'-1 $\frac{1}{2}$ ".

Some part numbers from the "Task Force 13" Guide to Standardized Highway Barrier Hardware have been furnished for quick reference.

GENERAL NOTES:

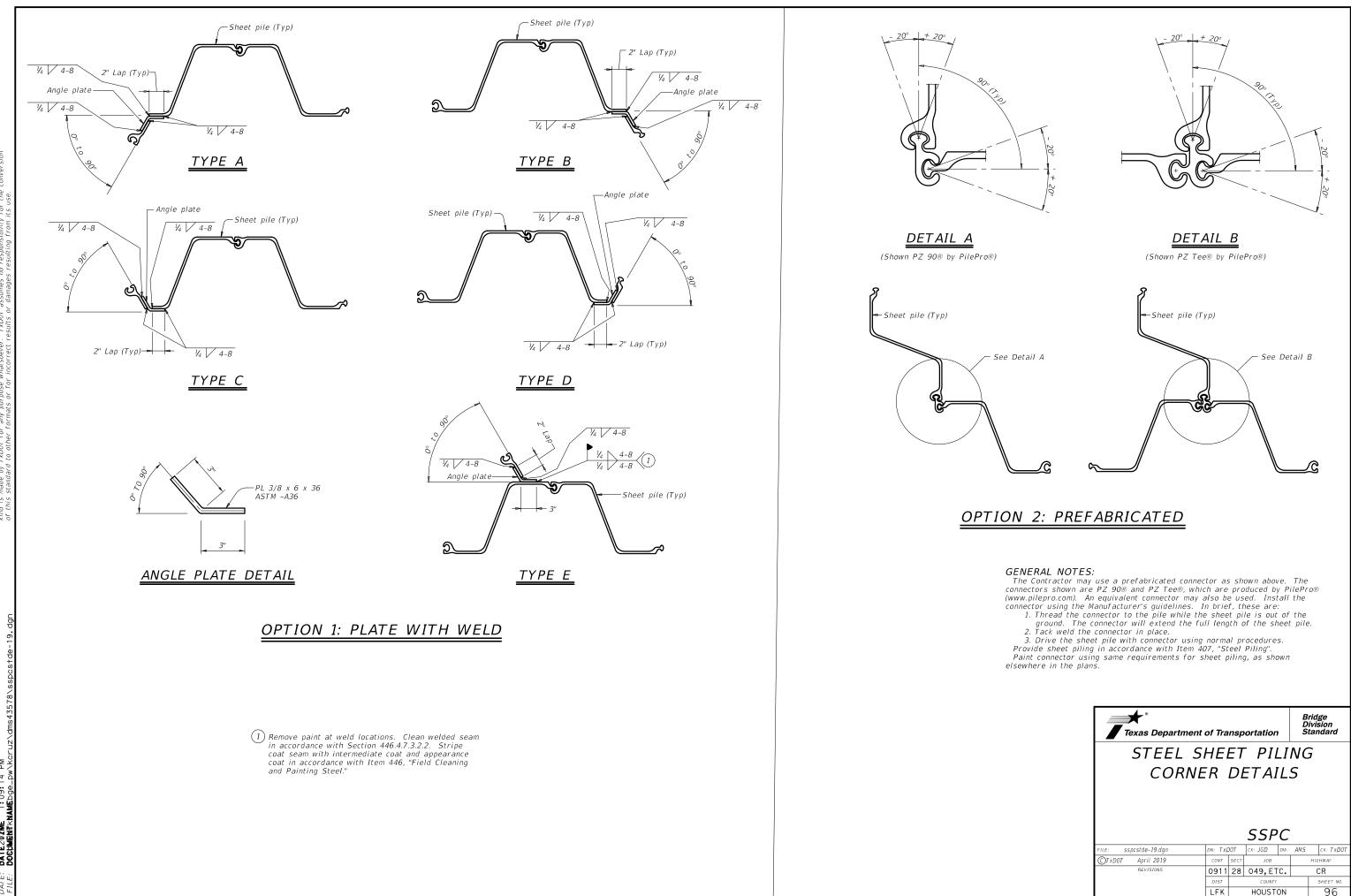
This railing has been successfully evaluated by full-scale crash test to meet MASH TL-2 criteria. This railing can be used for speeds of 45 mph and less.

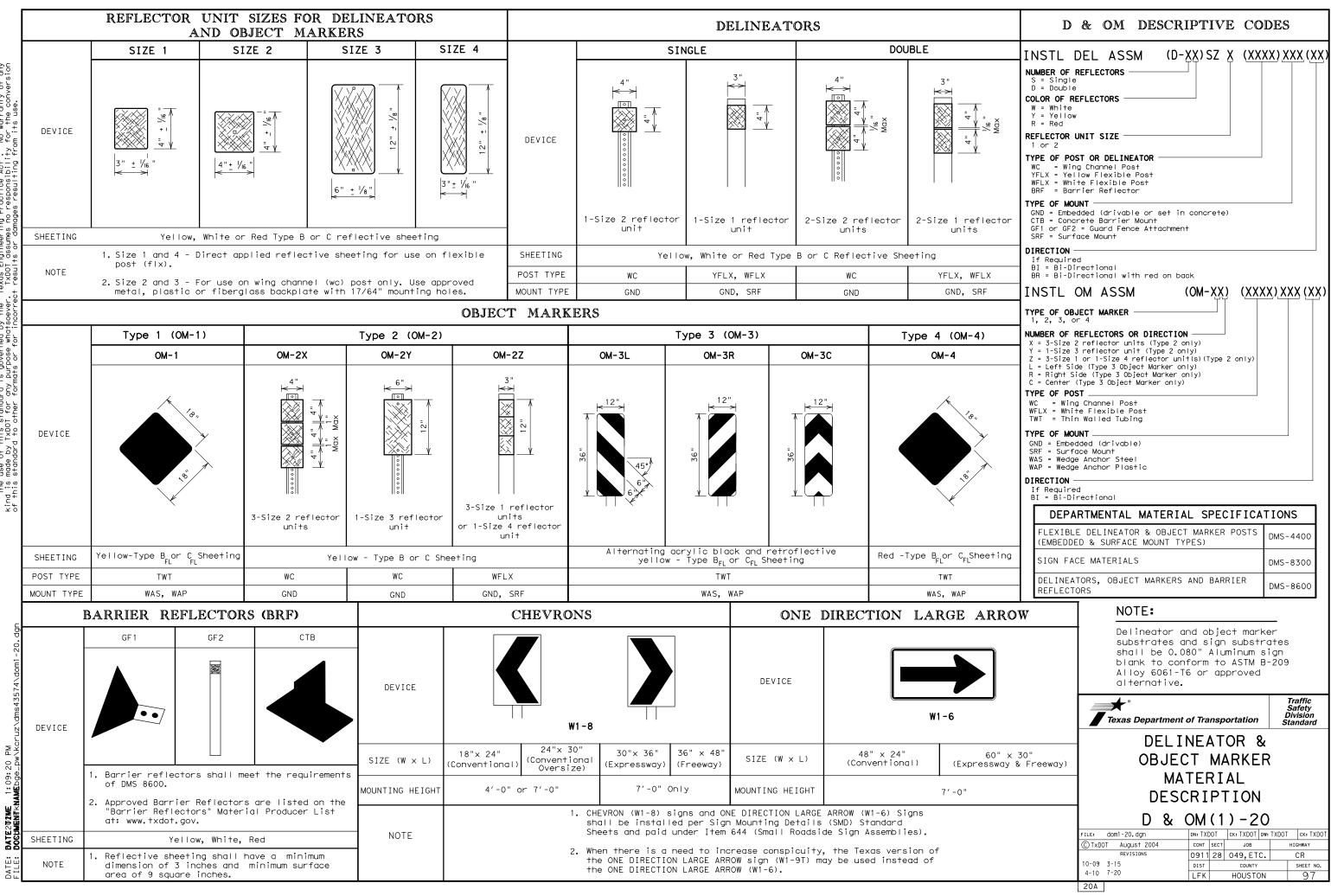
This rail is designed to deflect approximately 2' to 2'-6" as it contains and redirects the errant vehicle. This rail may not be installed on top of or behind curbs that project above finished grade, on bridges with expansion joints providing more than 5" movement, on retaining walls, or on grade separations and interchanges.

Repairs to impact-damaged post and base plate unit are not permitted. Replace all impact-damaged posts with a new post and base plate unit.

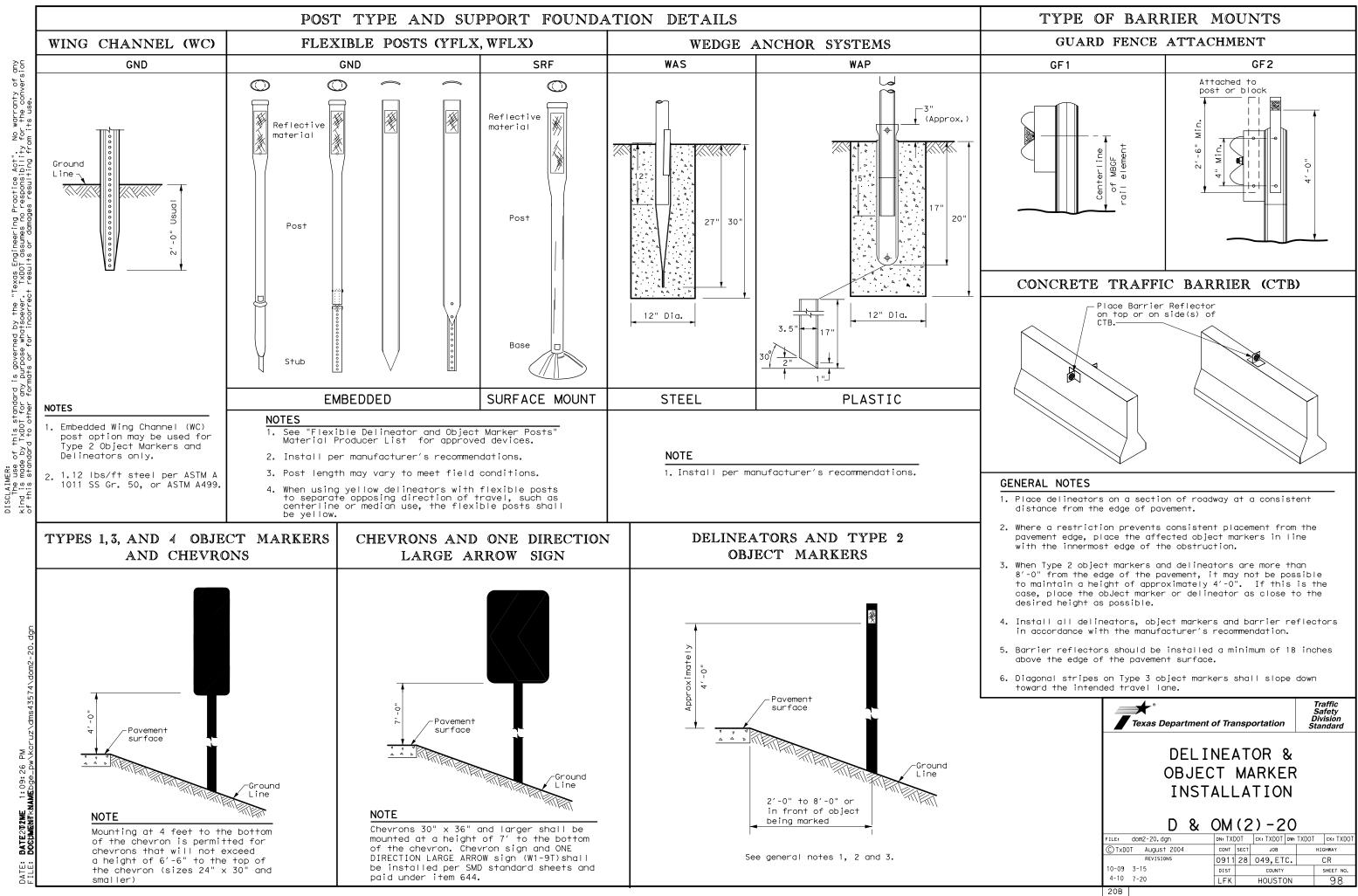
Average weight of railing with no overlay: 13 plf total.

| SHE | ET 2 | ? 0 | F 2 | | | | | | |
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| Texas Department of Transportation | | | | | | | | | |
| TRAFFIC RAIL | | | | | | | | | |
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| ©TxDOT September 2019 | CONT | SECT | JOB | | | HIGHWAY | | | |
| REVISIONS | 0911 | 28 | 049,ET | С. | | CR | | | |
| 07-20: Allowing 9'-4 ½" or 6'-3" W-Beam sections. | DIST | | COUNTY | | | SHEET NO. | | | |
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MINIMUM WARNING DEVICES AT CURVES WITH ADVISORY SPEEDS

| | WITH ADVISORY | SPEEDS |
|-----------------------------------|--|---|
| Amount by which Advisory Speed | Curve Advi | sory Speed |
| is less than Posted Speed | Turn (30 MPH or less) | Curve (35 MPH or more) |
| 5 MPH & 10 MPH | • RPMs | • RPMs |
| 15 MPH & 20 MPH | • RPMs and One Direction Large Arrow sign | RPMs and Chevrons; or RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles prevent the installation of chevrons. |
| 25 MPH & more | RPMs and Chevrons; or RPMs and One Direction Large Arrow sign where geometric conditions or roadside obstacles preven the installation of | • RPMs and Chevrons |
| SUGGES | Chevrons TED SPACING FOR ON HORIZONTAL | |
| | Extension of t centerline of tangent sectio approach lane NOTE ONE DIRECTION LARGE ARROW should be located at appro- perpendicular to the exten- centerline of the tangent approach lane. | (M1-6) sign (M1-6) sign(M1-6) sign(M1-6) sign(M1-6) sign(M1-6) sign(M1-6) s |
| | ESTED SPACING FO ON HORIZONTAL | |
| | at of vature | Point of tangent B B B |
| | | |
| | NOTE | |

At least one chevron pair is installed beyond the point of tangent in tangent

section.

| DEGREE Radius | SPAC | ND CHEV ING | RON | CONDITION | REQUI | |
|------------------|--|--|--|--|--|--|
| Radius | | | | | | |
| | | OR RADIUS I | S KNOWN | Frwy./Exp. Tangent | RPMs | |
| | | FEET | | | | |
| | Spacing | Spacing | Chevron Spacina | Frwy./Exp. Curve | Single del | |
| of Curve | in Curve | in Straightaway | in | Frwy/Exp.Ramp | Single del side of ro of curves) | |
| | A | 2A | В | | or curves) | |
| 5730 2865 | 225 160 | 450 320 | | Acceleration/Deceleration | Double de | |
| 1910 | 130 | 260 | 200 | Lane | on D&OM(4) | |
| 1433 | 110 | 220 | 160 | Truck Escape Ramp | Single red | |
| 1146 | 100 | 200 | 160 | | Bi-Direct | |
| | | | | Bridge Rail (steel or | undivided direction | |
| | | | 160 | concrete) and Metal | | |
| 637 | 75 | 150 | 120 | Beam Guard Fence | Single De lanes eact | |
| 573 | 70 | 140 | 120 | | | |
| | | | | | Barrier re | |
| | | | | | | |
| 409 | 55 | 110 | 80 | Cable Barrier | Reflectors | |
| 382 | 55 | 110 | 80 | | or me edg | |
| 358 | 55 | 110 | 80 | | Divided h approach | |
| 302 | | 100 | 80 | Guard Rail Terminus/Impact | | |
| | | | | hedd | Undivided Object ma | |
| 151 | 30 | 60 | 40 | | departure | |
| 101 | 20 | 40 | 40 | Bridges with pe Approach | Type 3 Ob | |
| | | | | | Markers () delineator | |
| | | | | Culverts without MBGF | Type 2 Ob | |
| | | | | Crossovers | Double ye | |
| ELINEA | | | RON | Pavement Narrowing (lane merge) on Freeways/Expressway | Single de to affect length of | |
| EGREE OF | CURVE C | R RADIUS IS | NOT KNOWN | NOTES | | |
| ory Spac | bing s | pacing | | 1. Unless indica | ted otherwi | |
| | | in | in | to the color or barrier re | of the pave | |
| A | | 2×A | B | 2. Barrier reflec | ctors may b | |
| 13 | 0 | 260 | 200 | | • | |
| | 0 | 220 | 160 | | | |
| | | 200 | 160 | | | |
| | | | | | | |
| | | | | | | |
| | - | 120 | 120 | | | |
| | | 110 | 80 | | LEGEN | |
| | | 100 | 80 | | | |
| | | 80 | 80 | | Bi-dire Delinea | |
| | | | | | • | |
| | | be determine | | | · · · · · · · · · · · · · · · · · · · | |
| | 1146 955 819 716 637 573 521 478 441 409 382 358 302 249 198 151 101 elineato should at 2A. T ring des ree of c ory Space i) Cur A 131 111 100 A 133 111 100 A 133 111 100 A 70 60 51 52 51 61 52 54 55 54 55 54 55 54 <t< td=""><td>1146 100 955 90 819 85 716 75 573 70 521 65 478 60 441 60 409 55 382 55 302 50 249 40 198 35 151 30 101 20 elineator approarshould include at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 300 PEGREE OF CURVE O</td><td>1146 100 200 955 90 180 819 85 170 716 75 150 573 70 140 521 65 130 478 60 120 441 60 120 409 55 110 382 55 110 302 50 100 249 40 80 198 35 70 151 30 60 101 20 40 elineator approach and depar should include 3 delineator at 2A. This spacing should be ring design preparation or where of curve is known. fin Curve Straightaway A 2×A 130 260 110 220 100 200 85 170 75 150 70 140 60</td><td>1146 100 200 160 955 90 180 160 819 85 170 160 716 75 150 120 573 70 140 120 521 65 130 120 478 60 120 120 441 60 120 120 409 55 110 80 382 55 110 80 302 50 100 80 302 50 100 80 249 40 80 80 198 35 70 40 101 20 40 40 elineator approach and departure should include 3 delineators at 2A. This spacing should be ring design preparation or when ree of curve is known. pring in Curve Straightaway Chevron Spacing in Curve A 2xA B 130 260 200 110 220</td><td>1146 100 200 160 955 90 180 160 819 85 170 160 716 75 150 160 637 75 150 120 573 70 140 120 573 70 140 120 478 60 120 120 409 55 110 80 302 50 100 80 302 50 100 80 302 50 100 80 302 50 100 80 198 35 70 40 151 30 60 40 101 20 40 40 819 85 70 40 101 20 40 40 80 80 80 80 101 20 40 102</td></t<> | 1146 100 955 90 819 85 716 75 573 70 521 65 478 60 441 60 409 55 382 55 302 50 249 40 198 35 151 30 101 20 elineator approarshould include at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 2A. This spacering design preprese of curve is at 300 PEGREE OF CURVE O | 1146 100 200 955 90 180 819 85 170 716 75 150 573 70 140 521 65 130 478 60 120 441 60 120 409 55 110 382 55 110 302 50 100 249 40 80 198 35 70 151 30 60 101 20 40 elineator approach and depar should include 3 delineator at 2A. This spacing should be ring design preparation or where of curve is known. fin Curve Straightaway A 2×A 130 260 110 220 100 200 85 170 75 150 70 140 60 | 1146 100 200 160 955 90 180 160 819 85 170 160 716 75 150 120 573 70 140 120 521 65 130 120 478 60 120 120 441 60 120 120 409 55 110 80 382 55 110 80 302 50 100 80 302 50 100 80 249 40 80 80 198 35 70 40 101 20 40 40 elineator approach and departure should include 3 delineators at 2A. This spacing should be ring design preparation or when ree of curve is known. pring in Curve Straightaway Chevron Spacing in Curve A 2xA B 130 260 200 110 220 | 1146 100 200 160 955 90 180 160 819 85 170 160 716 75 150 160 637 75 150 120 573 70 140 120 573 70 140 120 478 60 120 120 409 55 110 80 302 50 100 80 302 50 100 80 302 50 100 80 302 50 100 80 198 35 70 40 151 30 60 40 101 20 40 40 819 85 70 40 101 20 40 40 80 80 80 80 101 20 40 102 | |

for each Advisory Speed (MPH).

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R AND OBJECT MARKER APPLICATION AND SPACING

LEGEND

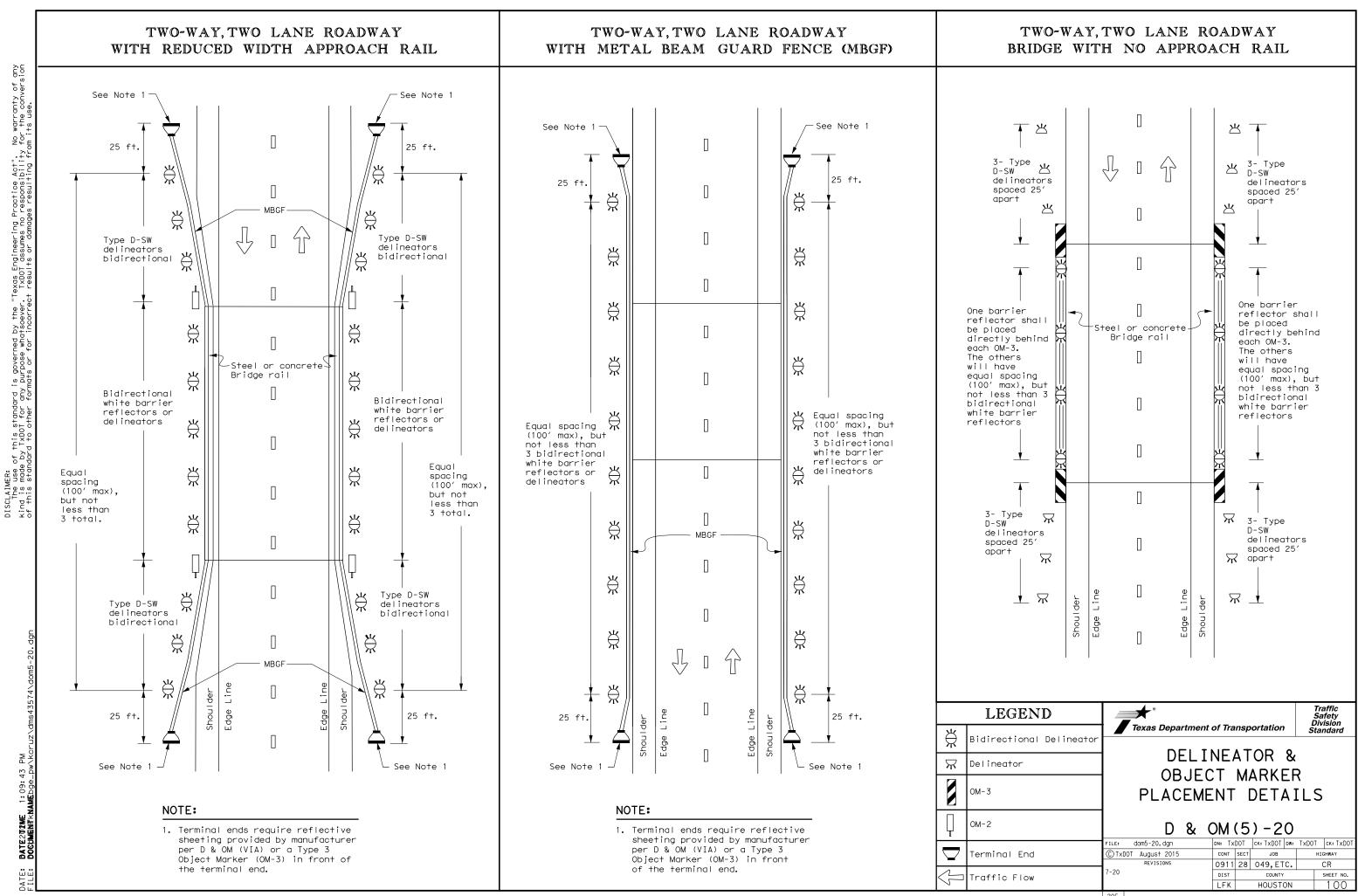
| | REQUIRED TREATMENT | MINIMUM SPACING |
|--------------|--|--|
| | RPMs | See PM-series and FPM-series standard sheets |
| | Single delineators on right side | See delineator spacing table |
| | Single delineators on at least one side of ramp (should be on outside of curves) (see Detail 3 on D&OM(4)) | 100 feet on ramp tangents Use delineator spacing table for ramp curves ("straightway spacing" does not apply to ramp curves) |
| on | Double delineators (see Detail 3 on D&OM(4)) | 100 feet (See Detail 3 on D & OM (4)) |
| | Single red delineators on both sides | 50 feet |
| | Bi-Directional Delineators when undivided with one lane each direction Single Delineators when multiple lanes each direction | Equal spacing (100'max) but not less than 3 delineators |
| r (CTB) r | Barrier reflectors matching the color of the edge line | Equal spacing 100′ max |
| | Reflectors matching the color of the edge line | Every 5th cable barrier post (up to 100'max) |
| oact | Divided highway - Object marker on approach end Undivided 2-lane highways - Object marker on approach and departure end | Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) and D & OM (6) |
| h | Type 3 Object Marker (OM-3) at end of rail and 3 single delineators approaching rail | See D & OM(5) |
| es to | Type 2 and Type 3 Object Markers (OM-3) and 3 single delineators approaching bridge | Requires reflective sheeting provided by manufacturer per D & OM (VIA) or a Type 3 Object Marker (OM-3) in front of the terminal end See D & OM (5) |
| | Type 2 Object Markers | See Detail 2 on D & OM(4) |
| | Double yellow delineators and RPMs | See Detail 1 on D & OM (4) |
| | Single delineators adjacent to affected lane for full length of transition | 100 feet |

indicated otherwise, the delineator or barrier reflector color shall conform color of the pavement edge line on the side of the road where the delineators ier reflectors are placed.

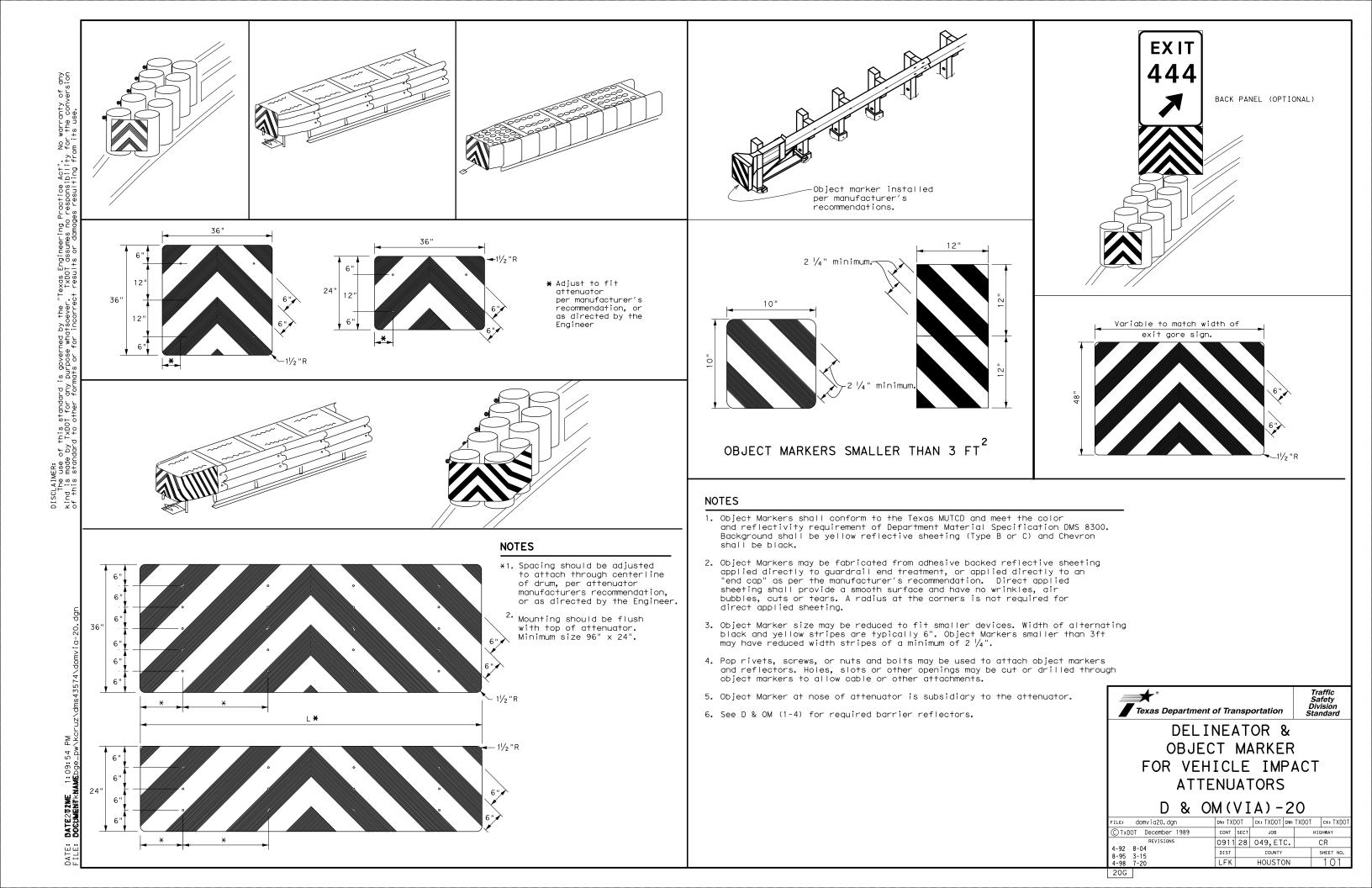
reflectors may be used to replace required delineators.

red delineators may be mounted on the back side of delineator posts for wrong

| | Texas Depar | rtment o | of Tra | nsp | ortation | S Di | raffic afety vision andard | |
|------------------------------|------------------------------------|----------|---------|------|---------------|---------|-------------------------------------|--|
| LEGEND | | | | | OR & | | | |
| Bi-directional Delineator | OBJECT MARKER PLACEMENT DETAILS | | | | | | | |
| Delineator | | | | _ | | | | |
| Sign | | & (| | |) -20 | | | |
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| | © TxDOT August 2004 | | CONT | SECT | JOB | н | IGHWAY | |
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| | 8-15 7-20 | | LFK | | HOUSTON | | 99 | |
| | 200 | | | | | | | |



20E



HICKORY CREEK BRIDGE AT CR 1060 (CSJ 0911-28-049)

THE TOTAL DISTURBED AREA FOR HICKORY CREEK BRIDGE IS 0.648 ACRES. THE DISTURBED AREA IN THIS PROJECT AND THE CONTRACTOR PROJECT SPECIFIC LOCATIONS (PSLs) WITHIN 1 MILE OF THE PROJECT LIMITS FOR THE CONTRACT WILL FURTHER ESTABLISH THE AUTHORIZATION REQUIREMENTS FOR STORM WATER DISCHARGES. AS THE DISTURBED AREA INCLUDING PSLS IS LESS THAN 1 ACRE, THE TPDES CGP DOES NOT APPLY, HOWEVER, THE CONTRACTOR WILL ADHERE TO THE REQUIREMENTS OF THE SWP3 LAYOUTS. IF THE TOTAL AREA DISTURBED SHOWN IN THE PLANS AND PSLS WITHIN 1 MILE OF THE PROJECT LIMITS EXCEEDS 1 ACRE, THE ENGINEER WILL DEVELOP AN SWP3 SITE PLAN AND POST A SMALL CONSTRUCTION SITE NOTICE FOR THE CONSTRUCTION ACTIVITIES.

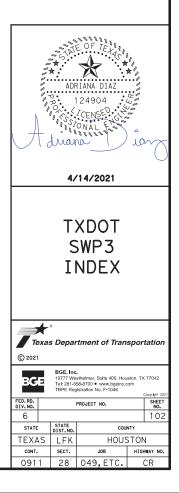
HICKORY TRIB CREEK BRIDGE AT CR 1050 (CSJ 0911-28-054)

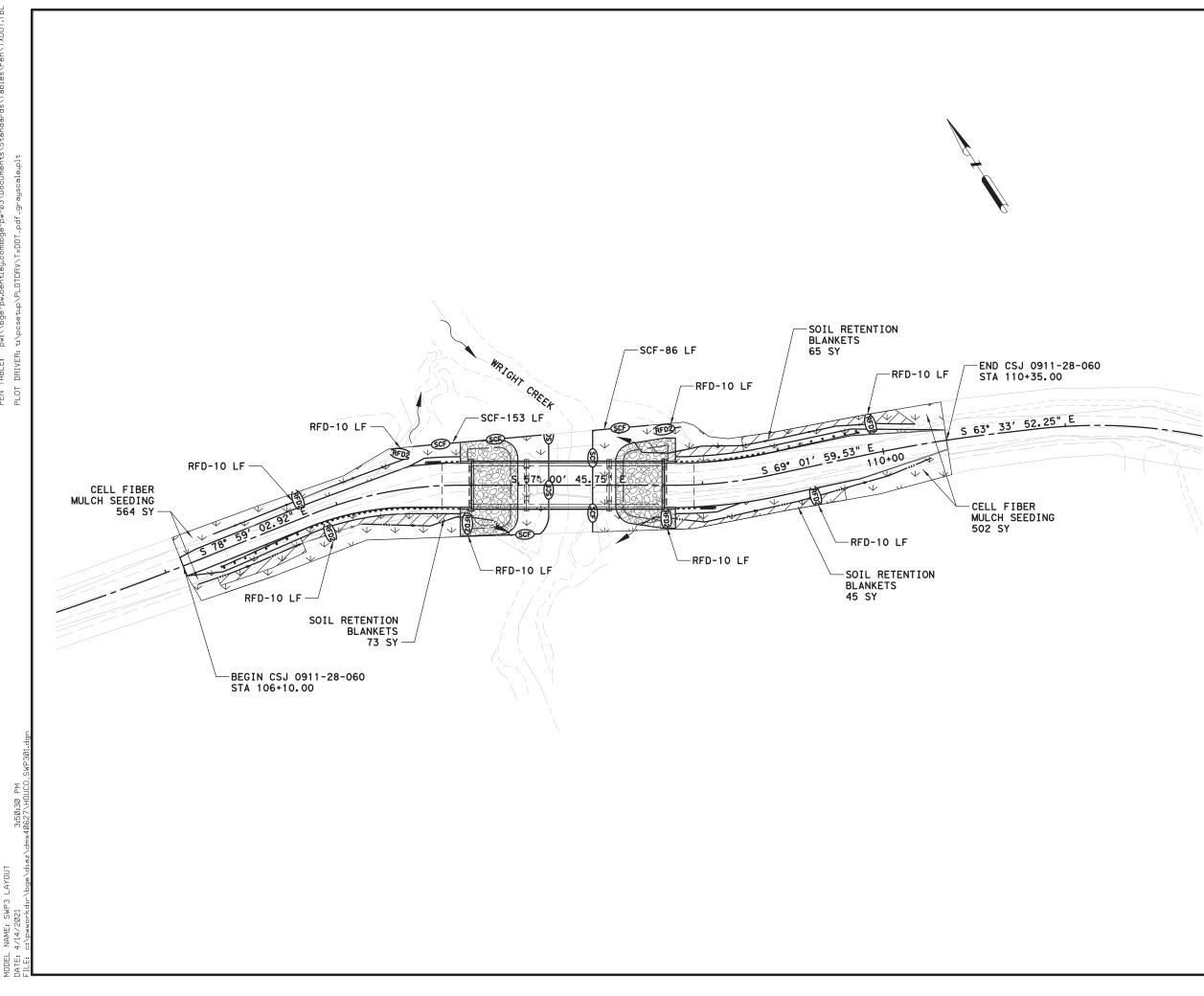
THE TOTAL DISTURBED AREA FOR HICKORY TRIB CREEK BRIDGE IS 0.757 ACRES. THE DISTURBED AREA IN THIS PROJECT AND THE CONTRACTOR PROJECT SPECIFIC LOCATIONS (PSLs) WITHIN 1 MILE OF THE PROJECT LIMITS FOR THE CONTRACT WILL FURTHER ESTABLISH THE AUTHORIZATION REQUIREMENTS FOR STORM WATER DISCHARGES. AS THE DISTURBED AREA INCLUDING PSLS IS LESS THAN 1 ACRE, THE TPDES CGP DOES NOT APPLY, HOWEVER, THE CONTRACTOR WILL ADHERE TO THE REQUIREMENTS OF THE SWP3 LAYOUTS. IF THE TOTAL AREA DISTURBED SHOWN IN THE PLANS AND PSLS WITHIN 1 MILE OF THE PROJECT LIMITS EXCEEDS 1 ACRE, THE ENGINEER WILL DEVELOP AN SWP3 SITE PLAN AND POST A SMALL CONSTRUCTION SITE NOTICE FOR THE CONSTRUCTION ACTIVITIES.

WRIGHT CREEK BRIDGE AT CR 3585 (CSJ 0911-28-060)

THE TOTAL DISTURBED AREA FOR WRIGHT CREEK BRIDGE IS 0.420 ACRES. THE DISTURBED AREA IN THIS PROJECT AND THE CONTRACTOR PROJECT SPECIFIC LOCATIONS (PSLs) WITHIN 1 MILE OF THE PROJECT LIMITS FOR THE CONTRACT WILL FURTHER ESTABLISH THE AUTHORIZATION REQUIREMENTS FOR STORM WATER DISCHARGES. AS THE DISTURBED AREA INCLUDING PSLS IS LESS THAN 1 ACRE, THE TPDES CGP DOES NOT APPLY, HOWEVER, THE CONTRACTOR WILL ADHERE TO THE REQUIREMENTS OF THE SWP3 LAYOUTS. IF THE TOTAL AREA DISTURBED SHOWN IN THE PLANS AND PSLS WITHIN 1 MILE OF THE PROJECT LIMITS EXCEEDS 1 ACRE, THE ENGINEER WILL DEVELOP AN SWP3 SITE PLAN AND POST A SMALL CONSTRUCTION SITE NOTICE FOR THE CONSTRUCTION ACTIVITIES.

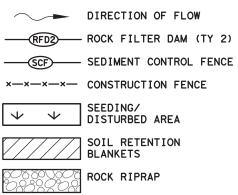
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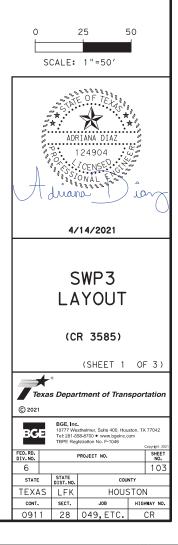
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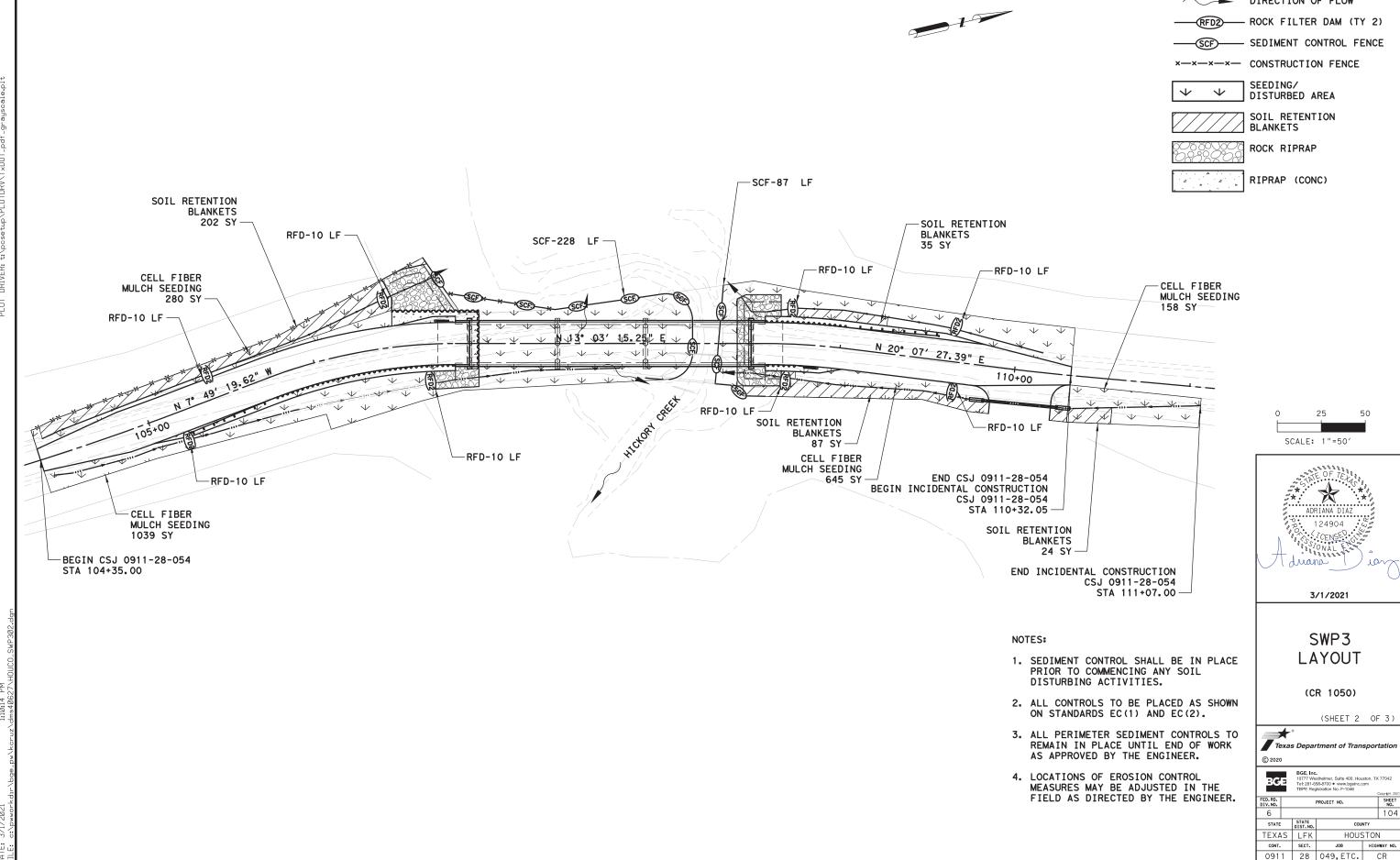
LEGEND



NOTES:

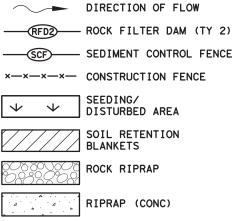
- 1. SEDIMENT CONTROL SHALL BE IN PLACE PRIOR TO COMMENCING ANY SOIL DISTURBING ACTIVITIES.
- 2. ALL CONTROLS TO BE PLACED AS SHOWN ON STANDARDS EC(1) AND EC(2).
- 3. ALL PERIMETER SEDIMENT CONTROLS TO REMAIN IN PLACE UNTIL END OF WORK AS APPROVED BY THE ENGINEER.
- LOCATIONS OF EROSION CONTROL 4. MEASURES MAY BE ADJUSTED IN THE FIELD AS DIRECTED BY THE ENGINEER.

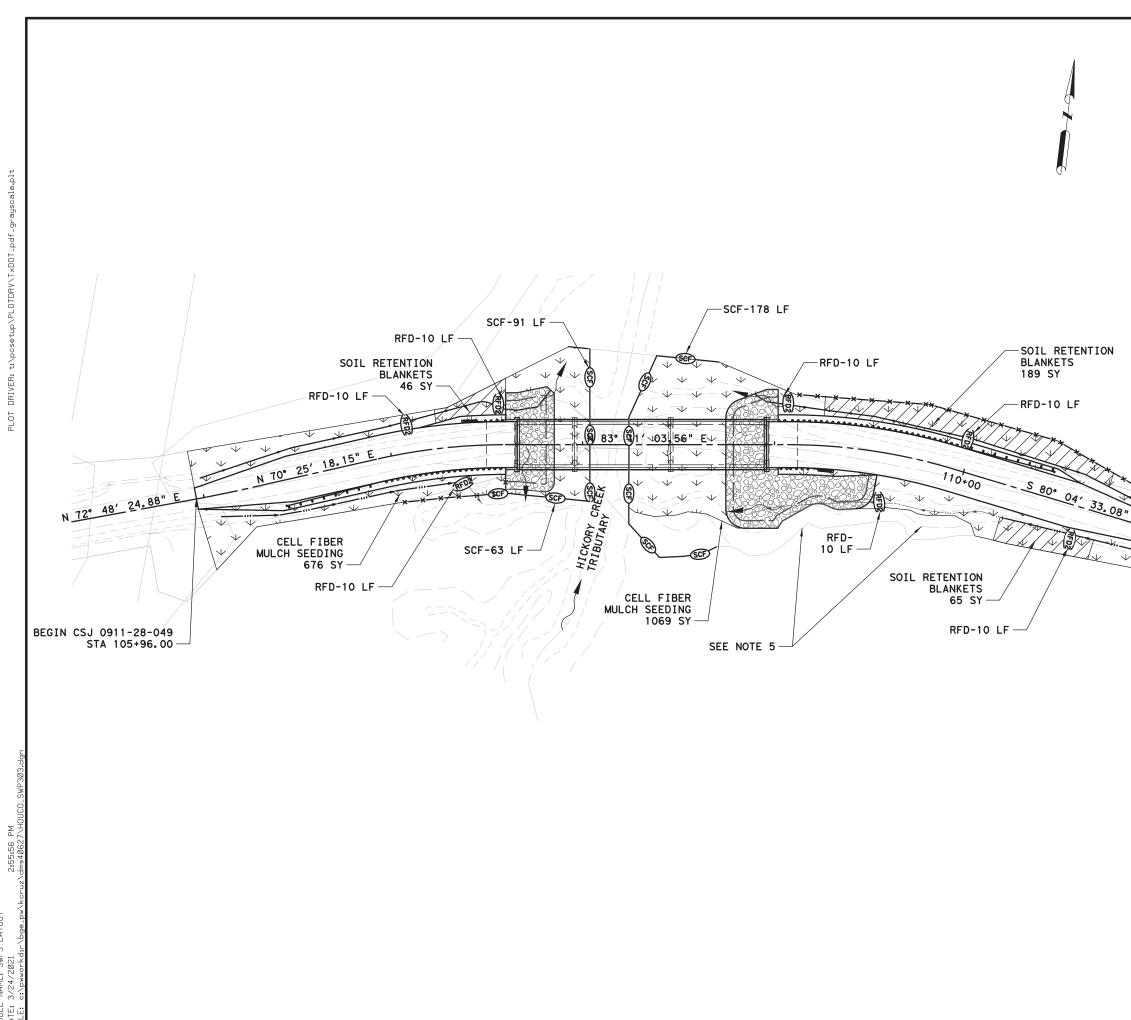




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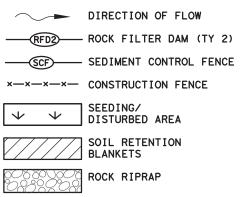






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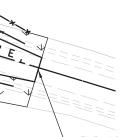


NOTES:

- 1. SEDIMENT CONTROL SHALL BE IN PLACE PRIOR TO COMMENCING ANY SOIL DISTURBING ACTIVITIES.
- 2. ALL CONTROLS TO BE PLACED AS SHOWN ON STANDARDS EC(1) AND EC(2).
- 3. ALL PERIMETER SEDIMENT CONTROLS TO REMAIN IN PLACE UNTIL END OF WORK AS APPROVED BY THE ENGINEER.
- 4. LOCATIONS OF EROSION CONTROL MEASURES MAY BE ADJUSTED IN THE FIELD AS DIRECTED BY THE ENGINEER.
- 5. DO NOT DISTURB DITCH BACKSLOPE. SEE ROADWAY CROSS SECTIONS SHEETS FOR LIMITS. 25 50

SCALE: 1"=50' ADRIANA DIA 124904 CENSE LONAL S Inana ian 3/24/2021 SWP3 LAYOUT (CR 1060) (SHEET 3 OF 3) Texas Department of Transportation © 2021 BGE, Inc. 10777 Westhelmer, Sulte 400, Houston, TX 77042 Tel: 281-558-8700 • www.bgeinc.com TBPE Registration No. F-1046 BGE ED.RD. PROJECT NO. 6 STATE COUNTY DIST. NO TEXAS LFK HOUSTON
 CONT.
 SECT.
 JOB
 HIGHWAY NO

 0911
 28
 049, ETC.
 CR



-END CSJ 0911-28-049 STA 111+10.00

| | I. STORMWATER POLLUTION F | PREVENTION-CLEAN WATER | ACT SECTION 402 | III. CULTURAL RESOURCES | | VI. HAZARDOUS M |
|-------------|---|--|---|---|---|--|
| | required for projects with | r Discharge Permit or Constr 1 or more acres disturbed so for erosion and sedimentati | oil. Projects with any | archeological artifacts are found d | ons in the event historical issues or uring construction. Upon discovery of nt rock, flint, pottery, etc.) cease act the Engineer immediately. | General (appli Comply with the Haz hazardous materials making workers awar |
| | • | nay receive discharges from ed prior to construction act | | 🛛 No Action Required 🗌 | Required Action | provided with perso Obtain and keep on- |
| nse | 1. N/A | | | Action No. | | used on the project Paints, acids, solv |
| :†s | No Action Required | Required Action | | 1. N/A | | compounds or additi |
| from | | | | T. IVA | | products which may Maintain an adequat |
| resulting 1 | Action No. 1. The proposed work of this at CR 1060, CR 1050, and CR discrete construction projec and will be treated as separ | 3585 in Houston County.This its separated a minimum $\frac{1}{4}$ m | project consists of ile by undisturbed areas | IV. <u>VEGETATION RESOURCES</u> Preserve native vegetation to the exte | ent practical. | In the event of a s in accordance with immediately. The Cc of all product spil |
| or damages | each location and Contractor the project limits will furt storm water discharges. As e TPDES CGP does not apply, ho | project specific locations her establish the authoriza- ach disturbed area including wever the contractor will ac | (PSLs) within 1 mile of tion requirements for g PSLs are less than 1 acre, dhere to the requirements | 164, 192, 193, 506, 730, 751, 752 in a | n Specification Requirements Specs 162, order to comply with requirements for ing, and tree/brush removal commitments. | Contact the Enginee * Dead or distr * Trash piles, * Undesirable s * Evidence of 1 |
| sults | of the SWP3 layouts and plac by the Engineer to comply wi | th water quality requirement | ts associated with section | No Action Required | Required Action | Does the project replacements (br |
| ě. | 401. If the total area distu project limits exceeds 1 acr | | | Action No. | | Yes |
| rect | a small construction site no | tice for the construction ac | ctivities. | 1. N/A | | If "No", then r |
| ncor | | | | | | If "Yes", then Are the results |
| fori | II. WORK IN OR NEAR STRE | | ETLANDS CLEAN WATER | V. FEDERAL LISTED, PROPOSED THREA CRITICAL HABITAT, STATE LISTED | • • • | Are the results |
| s or | ACT SECTIONS 401 AND | | | AND MIGRATORY BIRDS. | | If "Yes", then the notification |
| forma | water bodies, rivers, cre | filling, dredging, excavati eks, streams, wetlands or we | et areas. | If any of the listed species are observed on the disturb species or habitat and o | | activities as ne 15 working days |
| her | the following permit(s): | e to all of the terms and co | onaltions associated with | No Action Required 🛛 🕅 Re | equired Action | If "No", then 1 |
| 0 0 | | | | | | scheduled demoli In either case, |
| + + | No Permit Required | | | Action No. | | activities and/o |
| standa | Nationwide Permit 14 - wetlands affected) | PCN not Required (less than | 1/10th acre waters or | In order to maintain compliance with Wildlife Code and Migratory Bird Treaty activities that may affect nests (i.e. | (Act (MBTA), construction | asbestos consult Any other evider |
| his | 🗌 Nationwide Permit 14 - | PCN Required (1/10 to <1/2 | acre, 1/3 in tidal waters) | work) shall be conducted outside of the September 15). In the event birds or c | - | on site. Hazaro |
| 0f + | 🗌 Individual 404 Permit F | | | present) are encountered, contact the e | | No Action |
| | Other Nationwide Permit | f Required: NWP# | | TPWD Commitment Notes: | | |
| | | ers of the US permit applies | | 1. Cajun Chorus frog may occur in the pro | bject area. Avoid harming | Suspect pain at CR 1060 cd |
| | and check Best Management and post-project TSS. | Practices planned to control | l erosion, sedimentation | species if encountered. PSLs proposed be located in uplands away from aquat | | (BRL) or <10p request a cop |
| | 1. CR 3585 at Wright Creek | NWP #14 DOD-PCN | | disturbing or removing downed trees, | rotting stumps, and leaf litter, | the Area Eng |
| _ | 2. CR 1050 at Hickory Cree | k NWP #14 non-PCN | | which may be refugia for amphibians, 2. Eastern spotted skunk, Plains spotted | | |
| . dgr | 3. CR 1060 at Hickory Cree | k Tributary NWP#14 non-PCN | | Southern short-tailed shrew, Swamp ro in the project area. Avoid harming sp | , . | |
| IC01 | The should be a fully and | | | minimize disturbing or removing logs, | | VII. OTHER ENVI |
| 0_EP | | ary high water marks of any ers of the US requiring the | · - | where feasible. 3. Eastern box turtle, Slender glass lize | | (includes reg |
| fouc | permit can be found on the | Bridge Layouts. | | rattlesnake may occur in the project encountered.If reptiles are found on | area. Avoid harming species if project site, allow species to safely | No Action |
| 27/1 | Best Management Practic | ces: | | leave the project area. Visually insp wildlife prior to backfilling Avoid c | ect excavation areas for trapped or minimize disturbing or removing down | |
| s406 | Erosion | Sedimentation | Post-Construction TSS | trees, rotting stumps, and leaf litte | er, where feasible. | |
| mp/ | Temporary Vegetation | 🗙 Silt Fence | Vegetative Filter Strips | · · · | avoid impact WOTUS, etc.) around creeks | |
| oruz | Blankets/Matting | 🗙 Rock Berm | Retention/Irrigation Systems | that cross the project area to avoid | impacts to aquatic wildlife. | |
| ×/× | Mulch | Triangular Filter Dike | Extended Detention Basin | | | |
| 1-e0 | Sodding | ☐ Sand Bag Berm ☐ Straw Bale Dike | Constructed Wetlands | LIST OF ABBREVI | ATIONS | |
| ir/b | Diversion Dike | Brush Berms | Erosion Control Compost | | PCC: Spill Prevention Control and Countermeasure P3: Storm Water Pollution Prevention Plan | |
| rkd | Erosion Control Compost | Erosion Control Compost | ☐ Mulch Filter Berm and Socks | DSHS: Texas Department of State Health Services PC FHWA: Federal Highway Administration PS | Pre-Construction Notification BL: Project Specific Location | |
| OWWC | Mulch Filter Berm and Socks | Mulch Filter Berm and Socks | Compost Filter Berm and Socks | MOA: Memorandum of Agreement TC | EQ: Texas Commission on Environmental Quality DES: Texas Pollutant Discharge Elimination System | |
| ブ ;; | Compost Filter Berm and Socks | s 🗌 Compost Filter Berm and Socks | <u> </u> | MS4: Municipal Separate Stormwater Sewer System TP | | |
| ü | | Stone Outlet Sediment Traps | | NOT: Notice of Termination T& | Areatened and Endangered Species Army Corps of Engineers | |
| Η | | Sediment Basins | Grassy Swales | | FWS: U.S. Fish and Wildlife Service | |

DA

MATERIALS OR CONTAMINATION ISSUES

ies to all projects):

zard Communication Act (the Act) for personnel who will be working with by conducting safety meetings prior to beginning construction and re of potential hazards in the workplace. Ensure that all workers are onal protective equipment appropriate for any hazardous materials used. -site Material Safety Data Sheets (MSDS) for all hazardous products t, which may include, but are not limited to the following categories: vents, asphalt products, chemical additives, fuels and concrete curing ives. Provide protected storage, off bare ground and covered, for be hazardous. Maintain product labelling as required by the Act. te supply of on-site spill response materials, as indicated in the MSDS. spill, take actions to mitigate the spill as indicated in the MSDS, safe work practices, and contact the District Spill Coordinator ontractor shall be responsible for the proper containment and cleanup lls.

er if any of the following are detected: ressed vegetation (not identified as normal) drums, canister, barrels, etc. smells or odors

leaching or seepage of substances

t involve any bridge class structure rehabilitation or ridge class structures not including box culverts)?

No No

no further action is required. TxDOT is responsible for completing asbestos assessment/inspection.

of the asbestos inspection positive (is asbestos present)? 🛛 No

TxDOT must retain a DSHS licensed asbestos consultant to assist with n, develop abatement/mitigation procedures, and perform management ecessary. The notification form to DSHS must be postmarked at least prior to scheduled demolition.

TxDOT is still required to notify DSHS 15 working days prior to any ition.

the Contractor is responsible for providing the date(s) for abatement or demolition with careful coordination between the Engineer and tant in order to minimize construction delays and subsequent claims.

nce indicating possible hazardous materials or contamination discovered dous Materials or Contamination Issues Specific to this Project:

Required Action Required

t on the columns of bridge over Hickory Creek Tributary ontains lead at a concentration below reporting limit ppm, therefore abatement is not required. Contractor may py of the Asbestos and Lead Paint Inspection Report from ineer.

RONMENTAL ISSUES

jional issues such as Edwards Aquifer District, etc.)

Required

Required Action

| Texas Department | of Tra | nsp | ortation | | D | esign ivision tandard |
|--|---------|------|----------|-------|----|-----------------------------|
| ENVIRONME | ΝT | AL | . PE | R | М] | ιτs, |
| ISSUES ANI |) (| 0 | MMI | ΤI | ME | NTS |
| E | ΡI | С | | SHE | ΕT | 1 OF 2 |
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| CTxDOT: February 2015 | CONT | SECT | JOB | | | HIGHWAY |
| REVISIONS 12-12-2011 (DS) | 0911 | 28 | 049,ET0 | с. | | CR |
| 12-12-2011 (05) | | | | | | SHEET NO. |
| 05-07-14 ADDED NOTE SECTION IV. 01-23-2015 SECTION I (CHANGED ITEM 1122 | DIST | | COUNTY | | | SHEET NO. |

NWP GENERAL CONDITIONS

AS APPLICABLE TO THIS PROJECT

2. AQUATIC LIFE MOVEMENTS. NO ACTIVITY MAY SUBSTANTIALLY DISRUPT THE NECESSARY LIFE CYCLE MOVEMENTS OF THOSE SPECIES OF AQUATIC LIFE INDIGENOUS TO THE WATERBODY, INCLUDING THOSE SPECIES THAT NORMALLY MIGRATE THROUGH THE AREA. UNLESS THE ACTIVITY'S PRIMARY PURPOSE IS TO IMPOUND WATER.

3. SPAWNING AREAS. ACTIVITIES IN SPAWNING AREAS DURING SPAWNING SEASONS MUST BE AVOIDED TO THE MAXIMUM EXTENT PRACTICABLE. ACTIVITIES THAT RESULT IN THE PHYSICAL DESTRUCTION (E.G., THROUGH EXCAVATION, FILL, OR DOWNSTREAM SMOTHERING BY SUBSTANTIAL TURBIDITY) OF AN IMPORTANT SPAWNING AREA ARE NOT AUTHORIZED.

6. SUITABLE MATERIAL. NO ACTIVITY MAY USE UNSUITABLE MATERIAL (E.G., TRASH, DEBRIS, CAR BODIES, ASPHALT, ETC.). MATERIAL USED FOR CONSTRUCTION OR DISCHARGED MUST BE FREE FROM TOXIC POLLUTANTS IN TOXIC AMOUNTS (SEE SECTION 307 OF THE CLEAN WATER ACT).

8. ADVERSE EFFECTS FROM IMPOUNDMENTS. IF THE ACTIVITY CREATES AN IMPOUNDMENT OF WATER, ADVERSE EFFECTS TO THE AQUATIC SYSTEM DUE TO ACCELERATING THE PASSAGE OF WATER, AND/OR RESTRICTING ITS FLOW MUST BE MINIMIZED TO THE MAXIMUM EXTENT PRACTICABLE.

9. MANAGEMENT OF WATER FLOWS. TO THE MAXIMUM EXTENT PRACTICABLE, THE PRE-CONSTRUCTION COURSE, CONDITION, CAPACITY, AND LOCATION OF OPEN WATERS MUST BE MAINTAINED FOR EACH ACTIVITY, INCLUDING STREAM CHANNELIZATION AND STORM WATER MANAGEMENT ACTIVITIES, EXCEPT AS PROVIDED BELOW. THE ACTIVITY MUST BE CONSTRUCTED TO WITHSTAND EXPECTED HIGH FLOWS. THE ACTIVITY MUST NOT RESTRICT OR IMPEDE THE PASSAGE OF NORMAL OR HIGH FLOWS, UNLESS THE PRIMARY PURPOSE OF THE ACTIVITY IS TO IMPOUND WATER OR MANAGE HIGH FLOWS. THE ACTIVITY MAY ALTER THE PRE-CONSTRUCTION COURSE, CONDITION, CAPACITY, AND LOCATION OF OPEN WATERS IF IT BENEFITS THE AQUATIC ENVIRONMENT (E.G., STREAM RESTORATION OR RELOCATION ACTIVITIES).

11. EQUIPMENT. HEAVY EQUIPMENT WORKING IN WETLANDS OR MUD FLATS MUST BE PLACED ON MATS. OR OTHER MEASURES MUST BE TAKEN TO MINIMIZE SOIL DISTURBANCE.

12. SOIL EROSION AND SEDIMENT CONTROLS. APPROPRIATE SOIL EROSION AND SEDIMENT CONTROLS MUST BE USED AND MAINTAINED IN EFFECTIVE OPERATING CONDITION DURING CONSTRUCTION, AND ALL EXPOSED SOIL AND OTHER FILLS, AS WELL AS ANY WORK BELOW THE ORDINARY HIGH WATER MARK OR HIGH TIDE LINE, MUST BE PERMANENTLY STABILIZED AT THE EARLIEST PRACTICABLE DATE. PERMITTEES ARE ENCOURAGED TO PERFORM WORK WITHIN WATERS OF THE UNITED STATES DURING PERIODS OF LOW-FLOW OR NO-FLOW.

13. REMOVAL OF TEMPORARY FILLS. TEMPORARY FILLS MUST BE REMOVED IN THEIR ENTIRETY AND THE AFFECTED AREAS RETURNED TO PRE-CONSTRUCTION ELEVATIONS. THE AFFECTED AREAS MUST BE REVEGETATED, AS APPROPRIATE.

14. PROPER MAINTENANCE. ANY AUTHORIZED STRUCTURE OR FILL SHALL BE PROPERLY MAINTAINED, INCLUDING MAINTENANCE TO ENSURE PUBLIC SAFETY AND COMPLIANCE WITH APPLICABLE NWP GENERAL CONDITIONS, AS WELL AS ANY ACTIVITY-SPECIFIC CONDITIONS ADDED BY THE DISTRICT ENGINEER TO AN NWP AUTHORIZATION.

23. MITIGATION. THE DISTRICT ENGINEER WILL CONSIDER SEVERAL FACTORS WHEN DETERMINING APPROPRIATE AND PRACTICABLE MITIGATION NECESSARY TO ENSURE THAT ADVERSE EFFECTS ON THE AQUATIC ENVIRONMENT ARE MINIMAL.

25. WATER QUALITY. WHERE STATES AND AUTHORIZED TRIBES, OR EPA WHERE APPLICABLE, HAVE NOT PREVIOUSLY CERTIFIED COMPLIANCE OF AN NWP WITH CWA SECTION 401, INDIVIDUAL 401 WATER QUALITY CERTIFICATION MUST BE OBTAINED OR WAIVED (SEE 33 CFR 330.4(C)). THE DISTRICT ENGINEER OR STATE OR TRIBE MAY REQUIRE ADDITIONAL WATER QUALITY MANAGEMENT MEASURES TO ENSURE THAT THE AUTHORIZED ACTIVITY DOES NOT RESULT IN MORE THAN MINIMAL DEGRADATION OR WATER QUALITY.

27. REGIONAL AND CASE-BY-CASE CONDITIONS. THE ACTIVITY MUST COMPLY WITH ANY REGIONAL CONDITIONS THAT MAY HAVE BEEN ADDED BY THE DIVISION ENGINEER (SEE 33 CFR 330.4(E)) AND WITH ANY CASE SPECIFIC CONDITIONS ADDED BY THE CORPS OR BY THE STATE, INDIAN TRIBE, OR U.S. EPA IN ITS SECTION 401 WATER QUALITY CERTIFICATION, OR BY THE STATE IN ITS COASTAL ZONE MANAGEMENT ACT CONSISTENCY DETERMINATION.

AS APPLICABLE TO THIS PROJECT

ACTIVITIES REQUIRED FOR CROSSINGS OF WATERS OF THE UNITED STATES ASSOCIATED WITH THE CONSTRUCTION, EXPANSION, MODIFICATION, OR IMPROVEMENT OF LINEAR TRANSPORTATION PROJECTS (E.G., ROADS, HIGHWAYS, RAILWAYS, TRAILS, AIRPORT RUNWAYS, AND TAXIWAYS) IN WATERS OF THE U.S. FOR LINEAR TRANSPORTATION PROJECTS IN NON-TIDAL WATERS, THE DISCHARGE CANNOT CAUSE THE LOSS OF GREATER THAN 1/2-ACRE OF WATERS OF THE U.S. ANY STREAM CHANNEL MODIFICATION, INCLUDING BANK STABILIZATION, IS LIMITED TO THE MINIMUM NECESSARY TO CONSTRUCT OR PROTECT THE LINEAR TRANSPORTATION PROJECT; SUCH MODIFICATIONS MUST BE IN THE IMMEDIATE VICINITY OF THE PROJECT.

THIS NWP ALSO AUTHORIZES TEMPORARY STRUCTURES, FILLS, AND WORK NECESSARY TO CONSTRUCT THE LINEAR TRANSPORTATION PROJECT. APPROPRIATE MEASÚRES MUST BE TAKEN TO MAINTAIN DOWNSTREAM FLOWS AND MINIMIZE FLOODING TO THE MAXIMUM EXTENT PRACTICABLE, WHEN TEMPORARY STRUCTURES, WORK, AND DISCHARGES, INCLUDING COFFERDAMS, ARE NECESSARY FOR CONSTRUCTION ACTIVITIES, ACCESS FILLS, OR DEWATERING OF CONSTRUCTION SITES. TEMPORARY FILLS MUST CONSIST OF MATERIALS, AND BE PLACED IN A MANNER THAT WILL NOT BE ERODED BY EXPECTED HIGH FLOWS. TEMPORARY FILLS MUST BE REMOVED IN THEIR ENTIRETY AND THE AFFECTED AREAS RETURNED TO PRE-CONSTRUCTION ELEVATIONS. THE AREAS AFFECTED BY TEMPORARY FILLS MUST BE REVEGETATED, AS APPROPRIATE.

THIS NWP CANNOT BE USED TO AUTHORIZE NON-LINEAR FEATURES COMMONLY ASSOCIATED WITH TRANSPORTATION PROJECTS, SUCH AS VEHICLE MAINTENANCE OR STORAGE BUILDINGS, PARKING LOTS, TRAIN STATIONS, OR AIRCRAFT HANGARS.

NOTIFICATION: THE PERMITTEE MUST SUBMIT A PRE-CONSTRUCTION NOTIFICATION (PCN) TO THE DISTRICT ENGINEER PRIOR TO COMMENCING THE ACTIVITY IF: (1) THE LOSS OF WATERS OF THE U.S. EXCEEDS 1/10-ACRE; OR (2) THERE IS A DISCHARGE IN A SPECIAL AQUATIC SITE, INCLUDING WETLANDS.

NOTE:

THE PROJECT CROSSES JURISDICTIONAL WATERS OF THE U.S. AND A NWP #14 WITH NO PCN HAS BEEN UTILIZED. THIS PERMIT AUTHORIZES THE ACTIVITIES WHICH WILL IMPACT WATERS OF THE U.S. THE NWP GENERAL CONDITIONS AND THE NWP #14 LIMITS MUST BE FOLLOWED IN ORDER TO MAINTAIN COMPLIANCE WITH THE NWP. NO COORDINATION HAS TAKEN PLACE WITH THE USACE BECAUSE IMPACTS WILL NOT EXCEED THE ABOVE CRITERIA. IF COORDINATION MAY BE NEEDED, CONTACT THE TXDOT LUFKIN DISTRICT ENVIRONMENTAL SECTION AT 1-800-687-8087.

ENVIRONMENTAL PERMITS, (EPIC) ISSUES AND COMMITMENTS

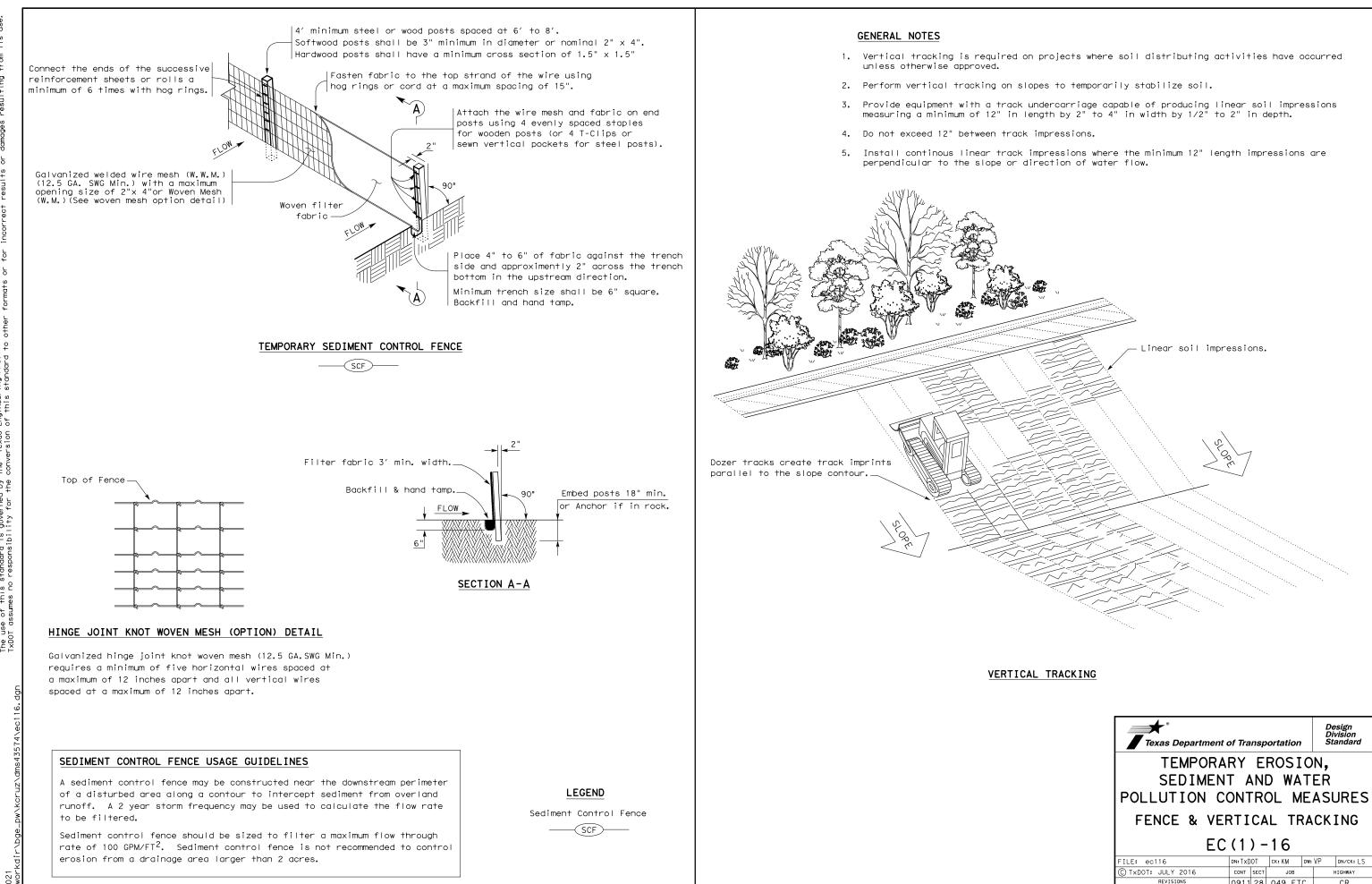
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FOR A COMPLETE LIST OF GENERAL CONDITIONS GO TO:

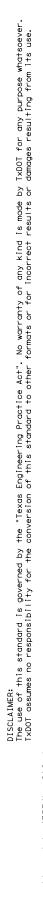
http://www.swf.usace.army.mil/Missions/Regulatory/Permitting/NationwideGeneralPermits.aspx

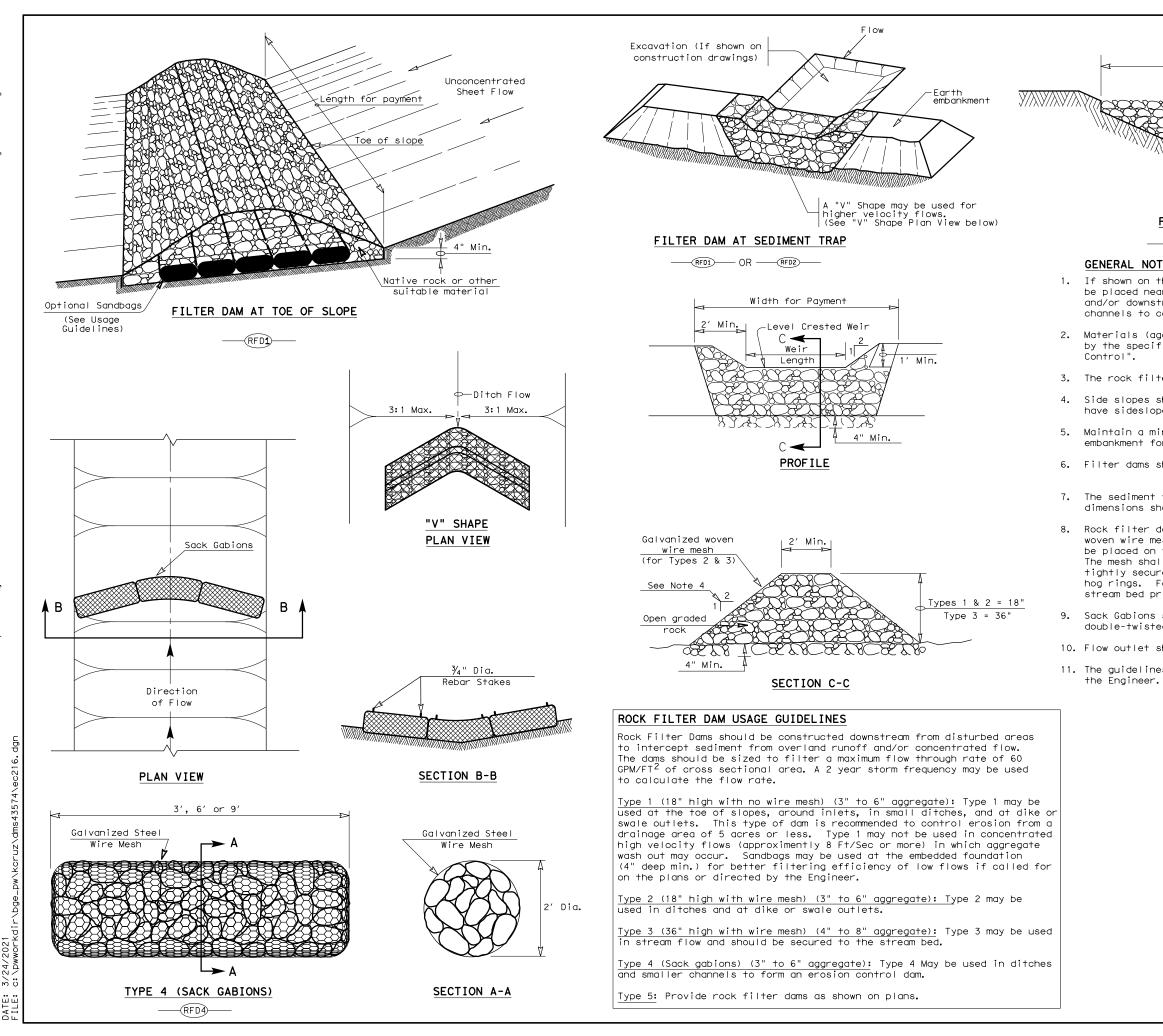
USACE - PERMIT #14

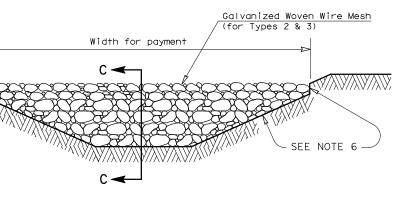
Texas Department of Transportation EPIC (ENVIRONMENTAL PERMITS, **ISSUES AND COMMITMENTS)** SHEET 2 OF 2 DN: TXDOT CK: RG DW: VP CK: AR ILE: epic.dgn C)TxDOT: February 2015 CONT SECT JOB HIGHWAY REVISION 091128 049,ETC. CR F 12-12-2011 (DS) -07-14 ADDED NOTE SECTION IV. DIST SHEET N -23-2015 SECTION I (CHANGED ITEM 1122) ITEM 506, ADDED GRASSY SWALES. LEK HOUSTON 107



| Texas Departmen | nt of Transj | oortation | Design Division Standard |
|---|-----------------|---------------------|--------------------------------|
| TEMPORA SEDIMEN POLLUTION (FENCE & VE | NT AN CONTR | D WAT OL ME | EŔ ASURES |
| E | C(1)- | -16 | |
| FILE: ec116 | DN: TxDOT | CK:KM DW | VP DN/CK: LS |
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| GINDON BOLL ZOID | | | |
| REVISIONS | 0911 28 | 049,ETC. | CR |
| 0 | 0911 28 DIST | 049, ETC. COUNTY | CR SHEET NO. |







FILTER DAM AT CHANNEL SECTIONS

GENERAL NOTES

1. If shown on the plans or directed by the Engineer, filter dams should be placed near the toe of slopes where erosion is anticipated, upstream and/or downstream at drainage structures, and in roadway ditches and channels to collect sediment.

2. Materials (aggregate, wire mesh, sandbags, etc.) shall be as indicated by the specification for "Rock Filter Dams for Erosion and Sedimentation

3. The rock filter dam dimensions shall be as indicated on the SW3P plans.

4. Side slopes should be 2:1 or flatter. Dams within the safety zone shall have sideslopes of 6:1 or flatter.

5. Maintain a minimum of 1' between top of rock filter dam weir and top of embankment for filter dams at sediment traps.

6. Filter dams should be embedded a minimum of 4" into existing ground.

7. The sediment trap for ponding of sediment laden runoff shall be of the dimensions shown on the plans.

8. Rock filter dam types 2 & 3 shall be secured with 20 gauge galvanized woven wire mesh with 1" diameter hexagonal openings. The aggregate shall be placed on the mesh to the height & slopes specified. The mesh shall be folded at the upstream side over the aggregate and tightly secured to itself on the downstream side using wire ties or hog rings. For in stream use, the mesh should be secured or staked to the stream bed prior to aggregate placement.

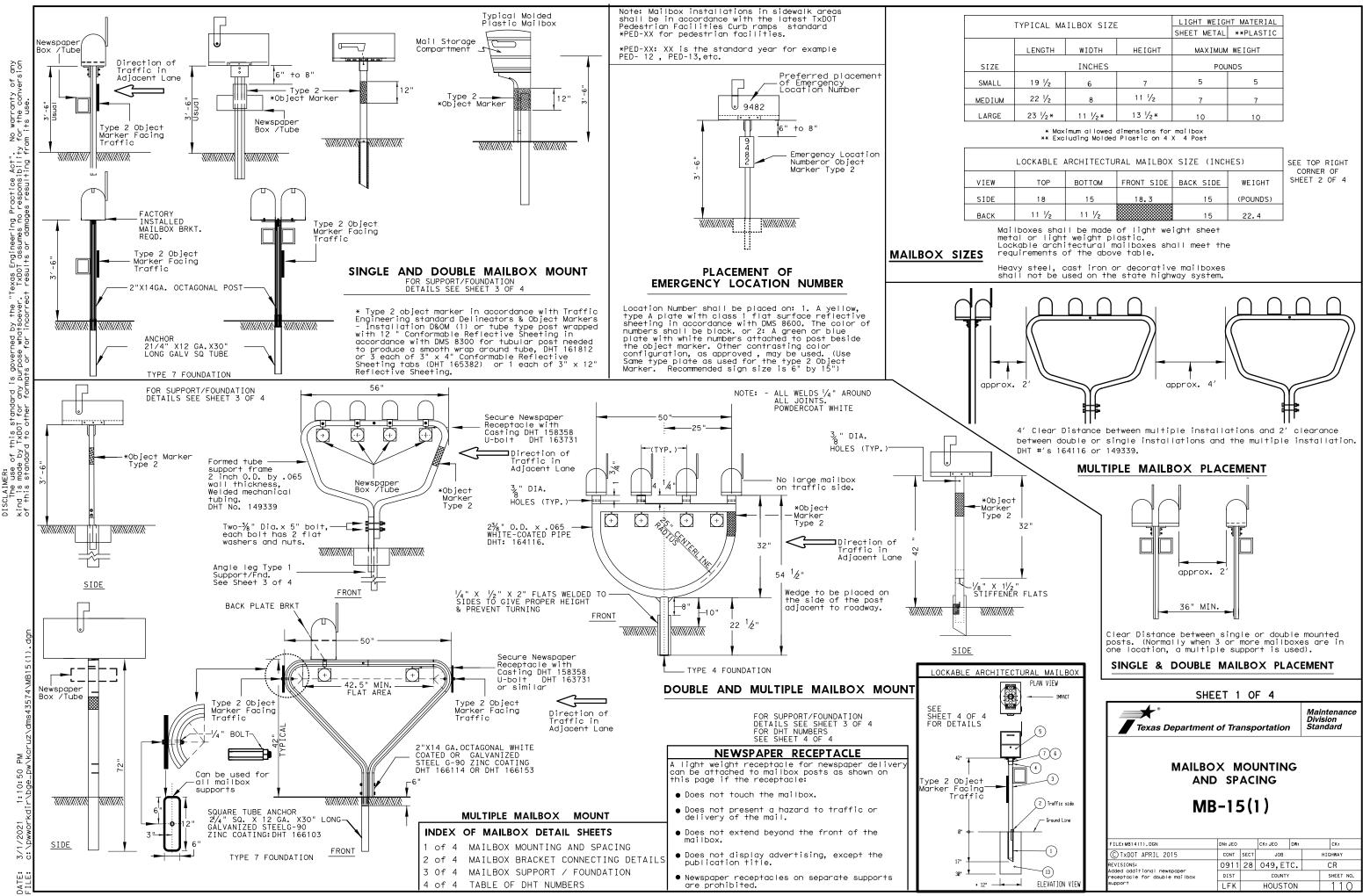
9. Sack Gabions should be staked down with $\frac{3}{4}$ " dia. rebar stakes, and have a double-twisted hexagonal weave with a nominal mesh opening of 2 $\frac{1}{2}$ x 3 $\frac{1}{4}$

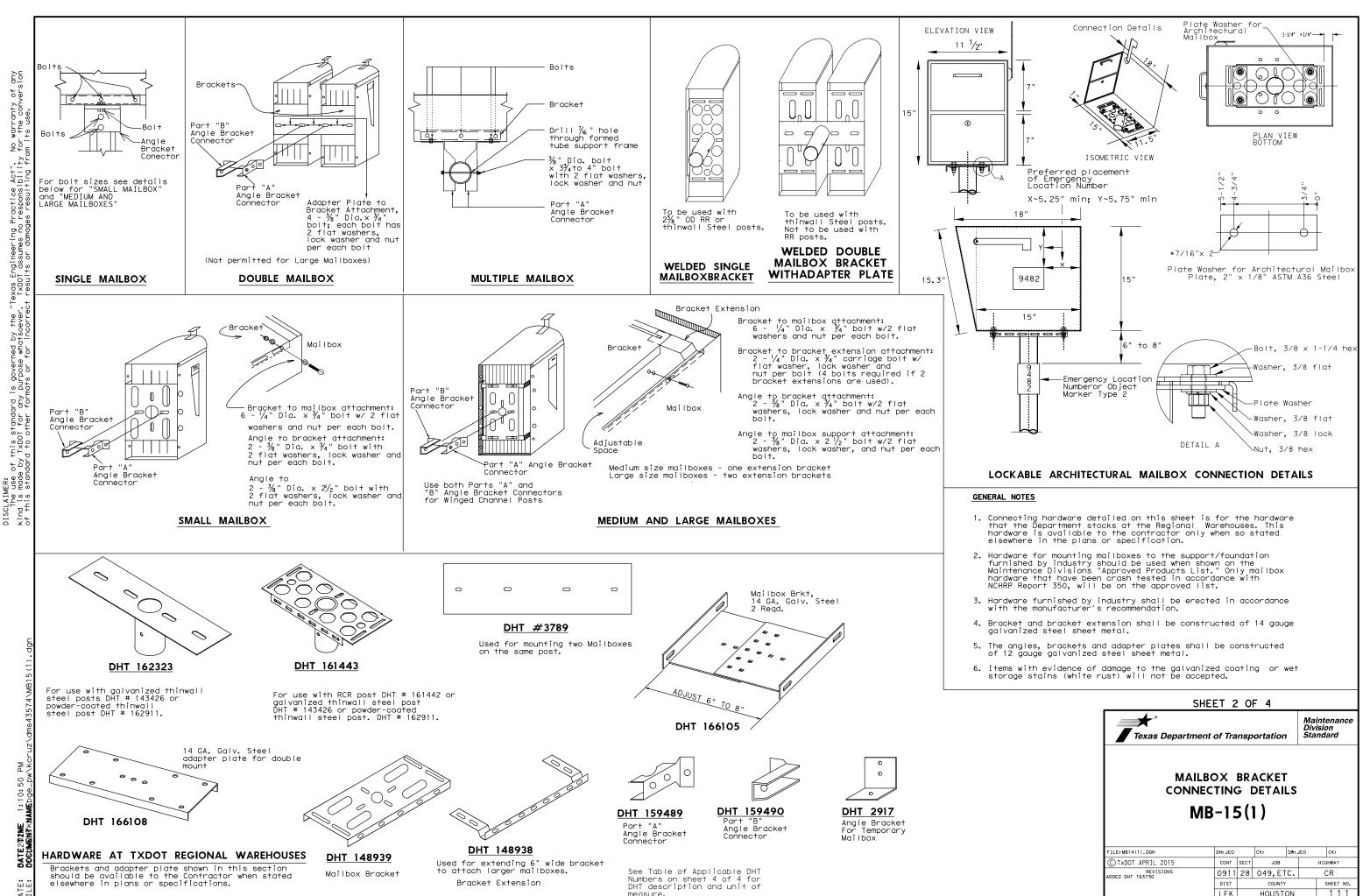
10. Flow outlet should be onto a stabilized area (vegetation, rock, etc.).

11. The guidelines shown hereon are suggestions only and may be modified by

PLAN SHEET LEGEND

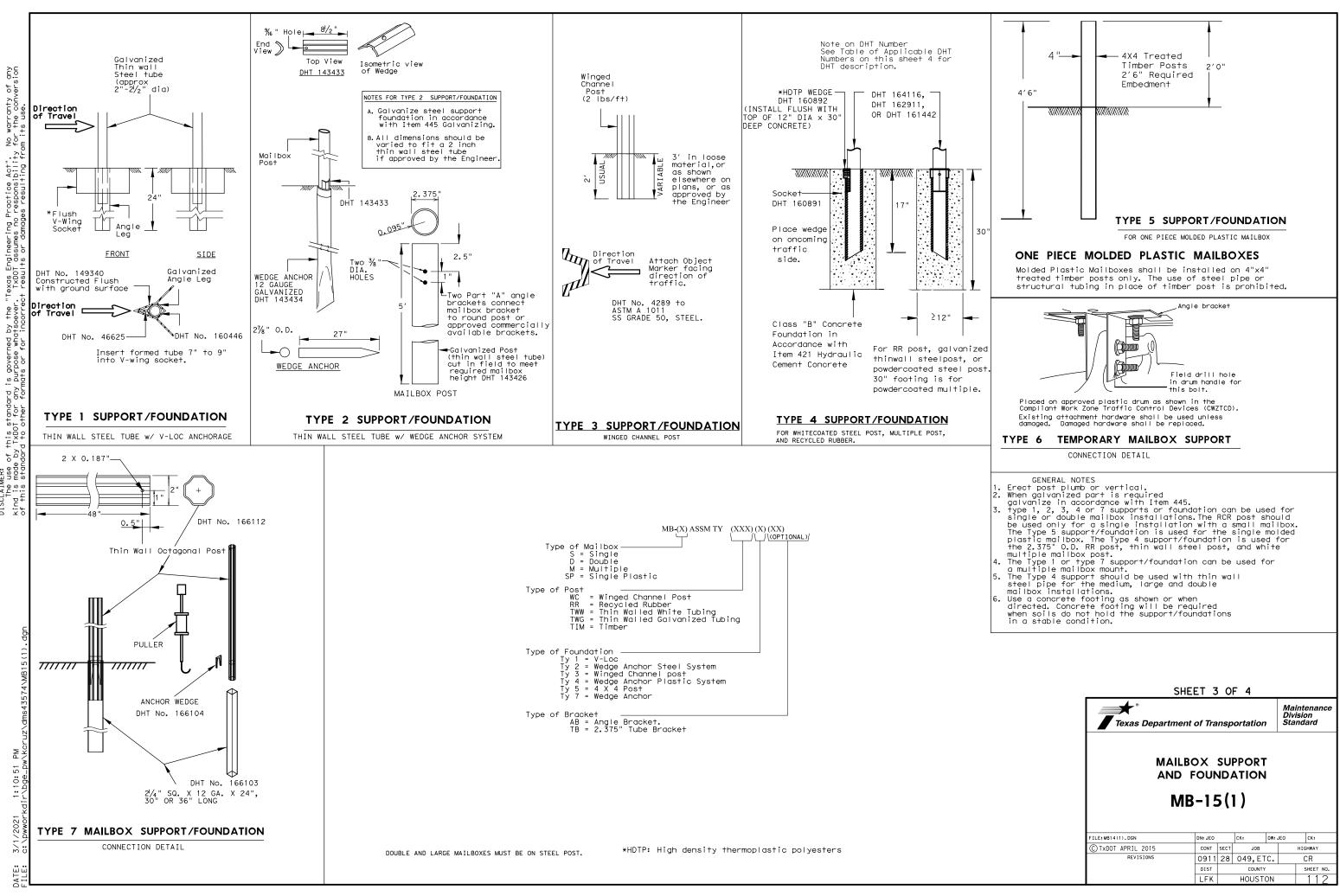
| Type 1 Rock Filter Dam | י ––(| RFD1 | | |
|---|-----------|----------------|------------|---------------------------|
| Type 2 Rock Filter Dam | י —(| RFD2 | | |
| Type 3 Rock Filter Dam | י —(| RFD3 | | |
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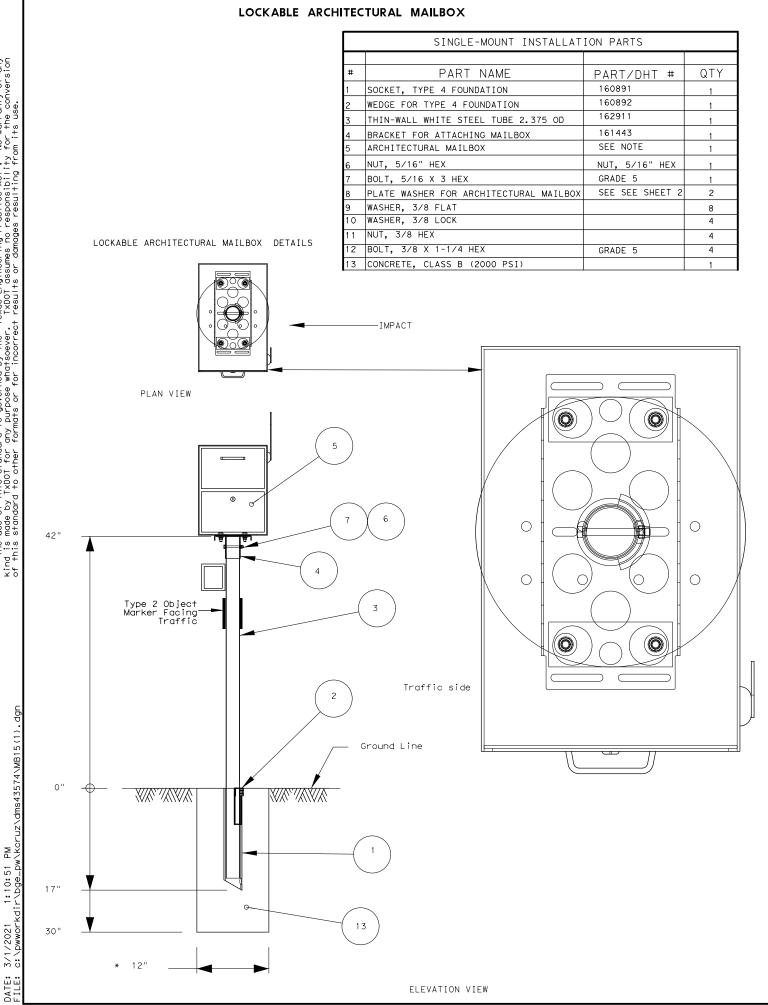


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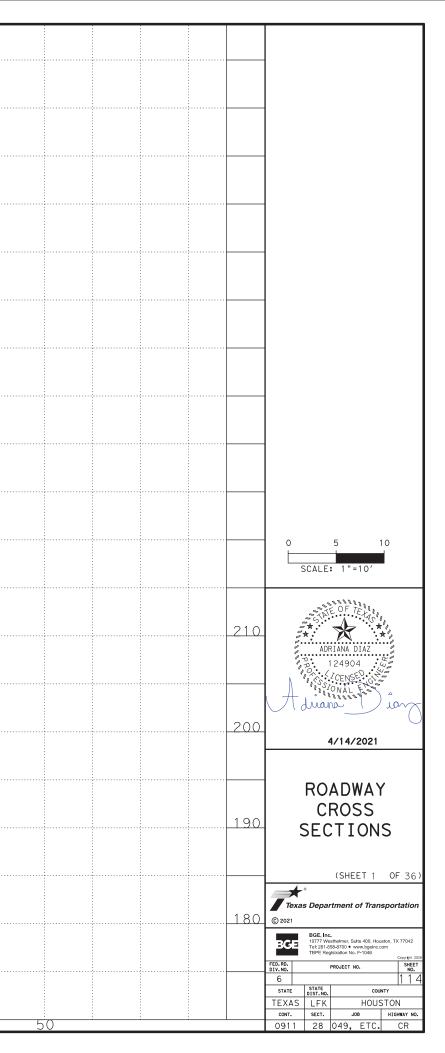
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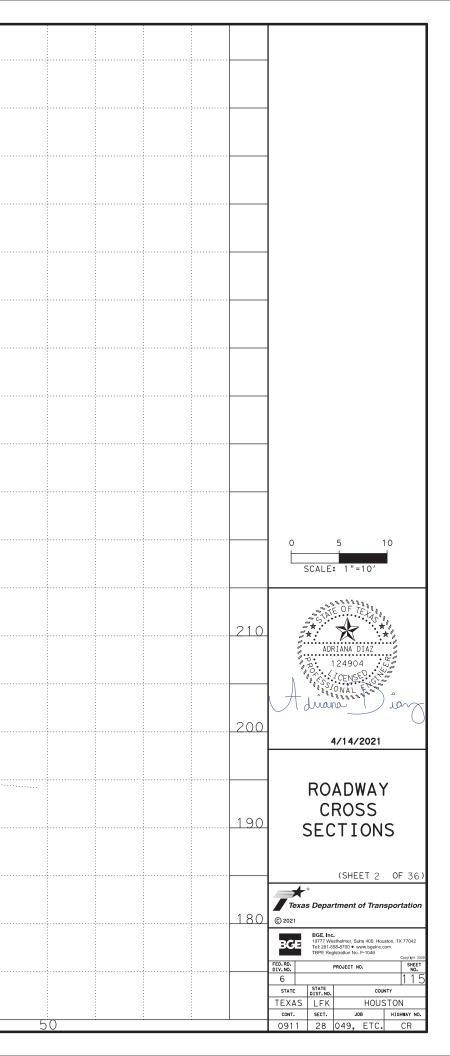
| | TABLE OF APPLICABLE DHT NUMBERS |
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| DHT NUMBER | DESCRIPTION |
| | FOUNDATIONS |
| 46625 | WEDGE FOR V-WING SOCKET FOR TYPE 1 FOUNDATION |
| 149340 | V-WING SOCKET FOR TYPE 1 FOUNDATION |
| 143433 | WEDGE FOR TYPE 2 FOUNDATION |
| 143434 | ANCHOR FOR TYPE 2 FOUNDATION |
| 166103 | ANCHOR FOR TYPE 7 FOUNDATION |
| 160891 | SOCKET FOR TYPE 4 FOUNDATION |
| 160892 | WEDGE FOR TYPE 4 FOUNDATION |
| 166104 | WEDGE FOR TYPE 7 FOUNDATION |
| | POSTS |
| 4289 | WINGED CHANNEL MAILBOX POST |
| 149339 | MULTIPLE MAILBOX POST (GALVANIZED TUBING) |
| 164116 | MULTIPLE MAILBOX POST (WHITE COATED) |
| 166114 | MULTIPLE MAILBOX POST (WHITE COATED OCTAGONAL) |
| 166153 | MULTIPLE MAILBOX POST (GALVANIZED OCTAGONAL) |
| 161442 | RECYCLED RUBBER POST. FOR SMALL MAILBOX ONLY |
| 143426 | THIN-WALL GALVANIZED STEEL TUBE 2.375" OUTER DIAMETER |
| 162911 | THINWALL WHITE STEEL TUBE 2.375" OUTER DIAMETER |
| | SINGLE OR DOUBLE THIN-WALL MAILBOX POST GALVANIZED |
| 166152 | 2" OCTAGONAL |
| | SINGLE OR DOUBLE THIN-WALL MAILBOX POST WHITECOATED |
| 166112 | 2" OCTAGONAL |
| | REFLECTIVE SHEETING |
| 161812 | REFLECTIVE SHEETING FOR EMERGENCY LOCATION NUMBER PANEL |
| | CONNECTING HARDWARE |
| 2917 | ANGLE BRACKET USED FOR TEMPORARY MAILBOX SUPPORT |
| 166105 | BRACKET FOR SINGLE MOUNTING OF MAILBOXES (MOUNTING KIT) |
| 3789 | PLATE FOR DOUBLE MOUNTING OF MAILBOXES |
| 166108 | BRACKET FOR DOUBLE MOUNTING OF MAILBOXES (MOUNTING KIT) |
| 166111 | BRACKET FOR MULTIPLE MOUNTING OF MAILBOXES (MOUNTING KIT) |
| 148939 | BRACKET FOR ATTACHING SMALL OR MEDIUM SIZE MAIL BOX |
| 148938 | EXTENDER TO BRACKET FOR ATTACHING LARGE MAILBOX |
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| 159489 | |
| 159489 159490 | ANGLE BRACKET PART A |
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| 159490 | ANGLE BRACKET PART A ANGLE BRACKET PART B BRACKET FOR DOUBLE MOUNTING OF MAILBOXES ON THINWALL STEEL POST, GALVANIZED OR POWDERCOATED. |
| 159490 162323 | ANGLE BRACKET PART A ANGLE BRACKET PART B BRACKET FOR DOUBLE MOUNTING OF MAILBOXES ON THINWALL STEEL POST, GALVANIZED OR POWDERCOATED. BRACKET FOR ATTACHING MAILBOX TO RECYCLED RUBBER POST |
| 159490 162323 161443 | ANGLE BRACKET PART A ANGLE BRACKET PART B BRACKET FOR DOUBLE MOUNTING OF MAILBOXES ON THINWALL STEEL POST, GALVANIZED OR POWDERCOATED. BRACKET FOR ATTACHING MAILBOX TO RECYCLED RUBBER POST AND TO MULTIPLE WHITE MAILBOX POST |
| 159490 162323 161443 158358 | ANGLE BRACKET PART A ANGLE BRACKET PART B BRACKET FOR DOUBLE MOUNTING OF MAILBOXES ON THINWALL STEEL POST, GALVANIZED OR POWDERCOATED. BRACKET FOR ATTACHING MAILBOX TO RECYCLED RUBBER POST AND TO MULTIPLE WHITE MAILBOX POST CASTING (NEWSPAPER RECEPTACLE BRACKET) |
| 159490 162323 161443 158358 163731 | ANGLE BRACKET PART A ANGLE BRACKET PART B BRACKET FOR DOUBLE MOUNTING OF MAILBOXES ON THINWALL STEEL POST, GALVANIZED OR POWDERCOATED. BRACKET FOR ATTACHING MAILBOX TO RECYCLED RUBBER POST AND TO MULTIPLE WHITE MAILBOX POST CASTING (NEWSPAPER RECEPTACLE BRACKET) U-BOLT (NEWSPAPER RECEPTACLE BRACKET) |
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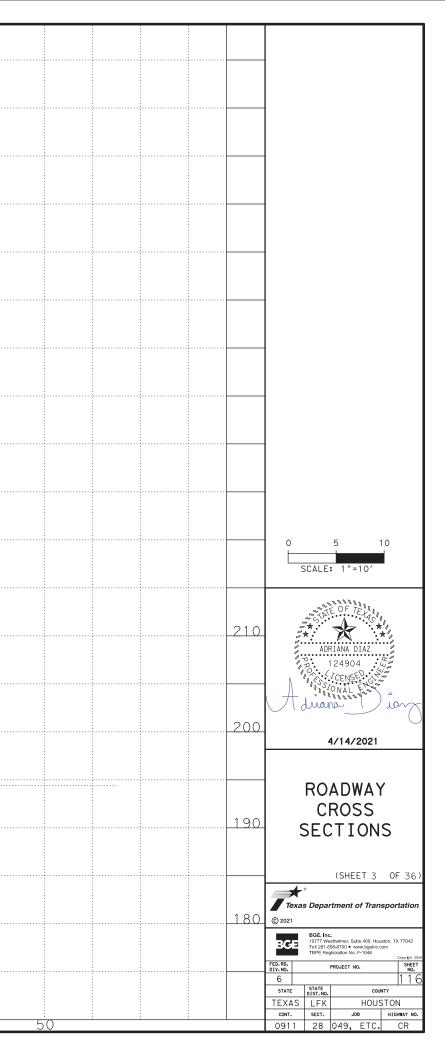
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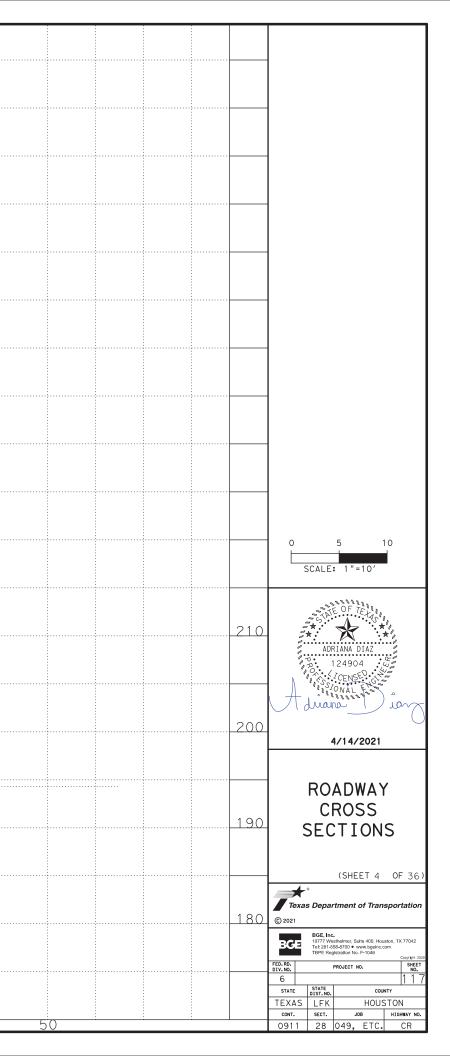
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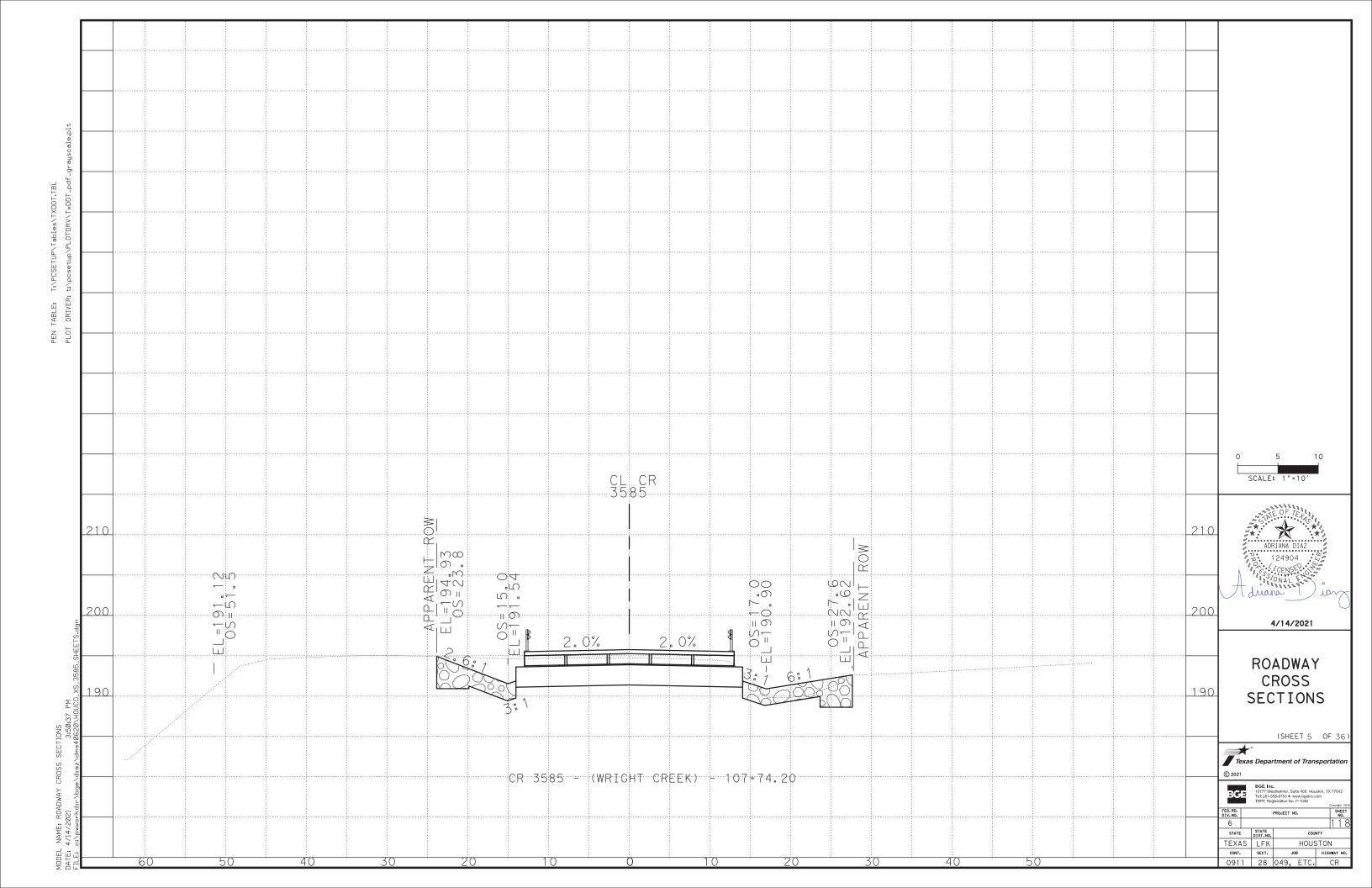


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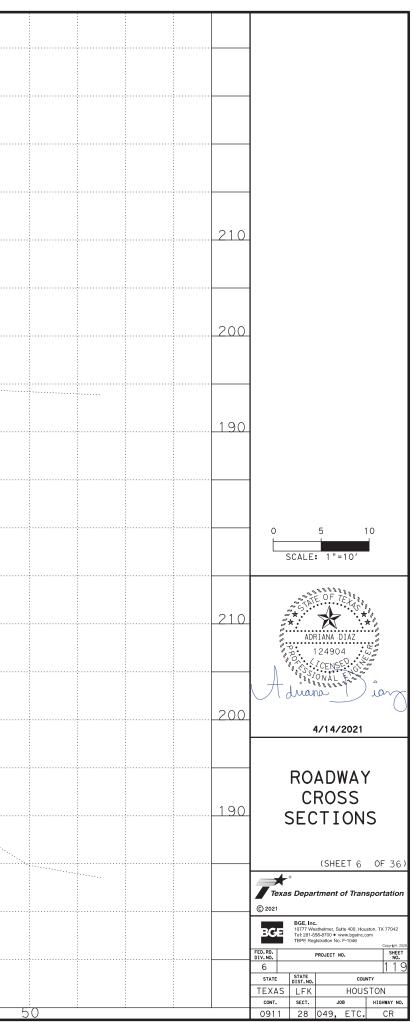


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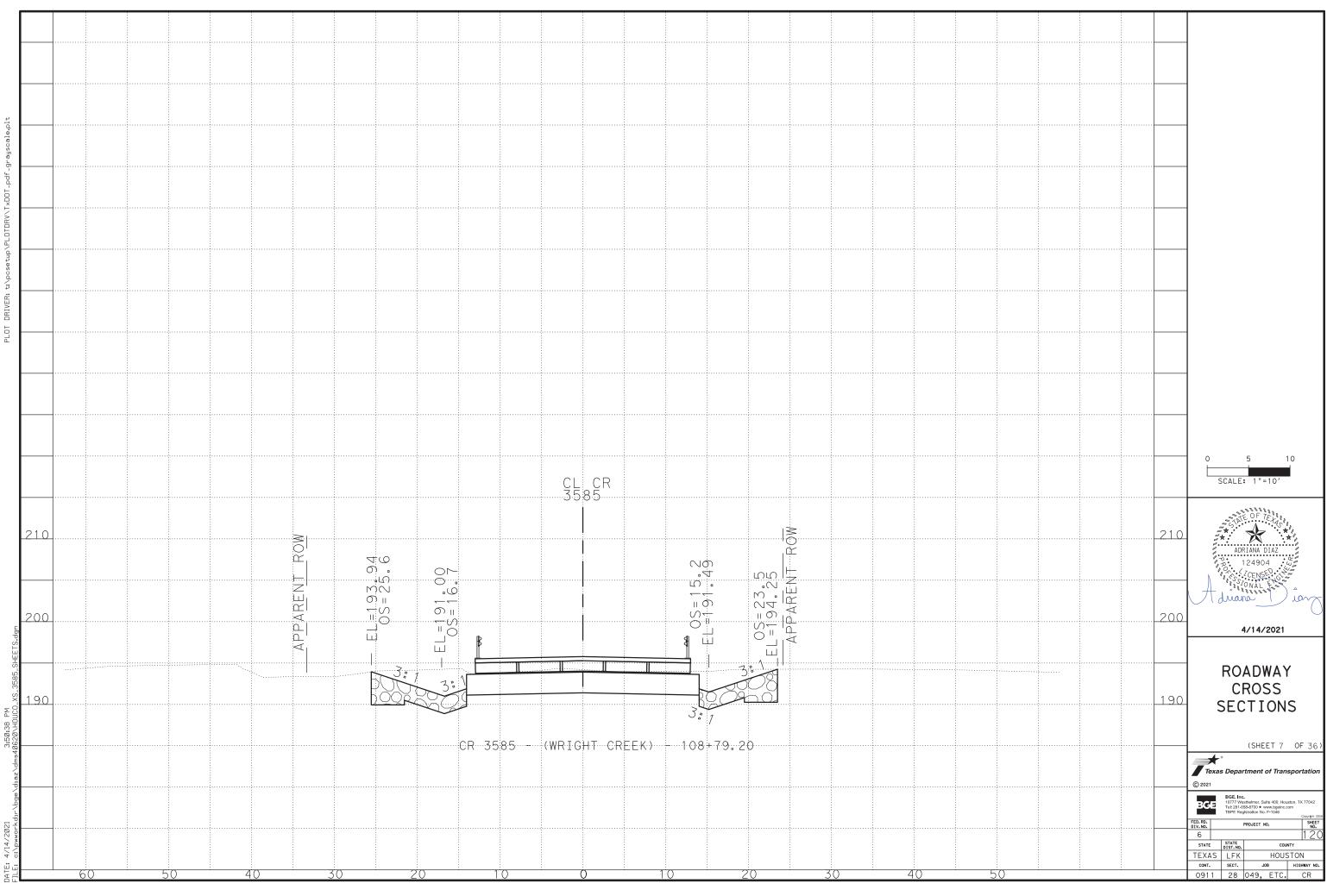


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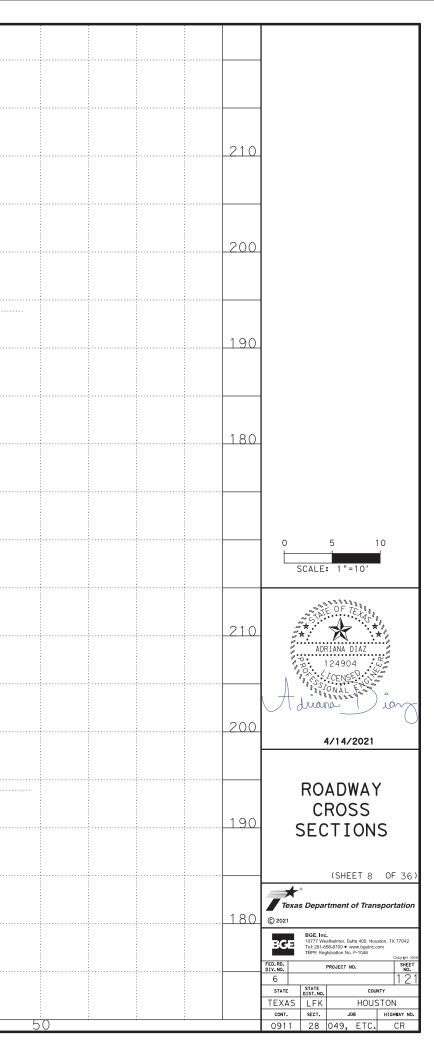




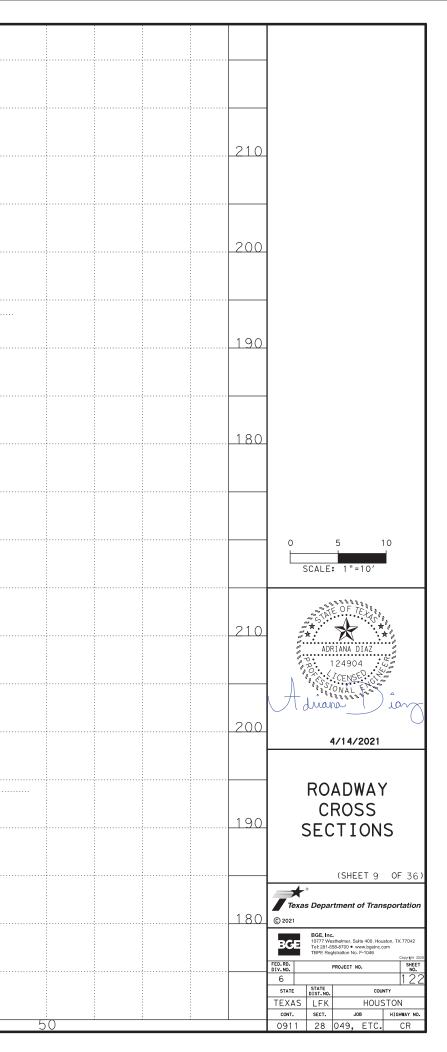
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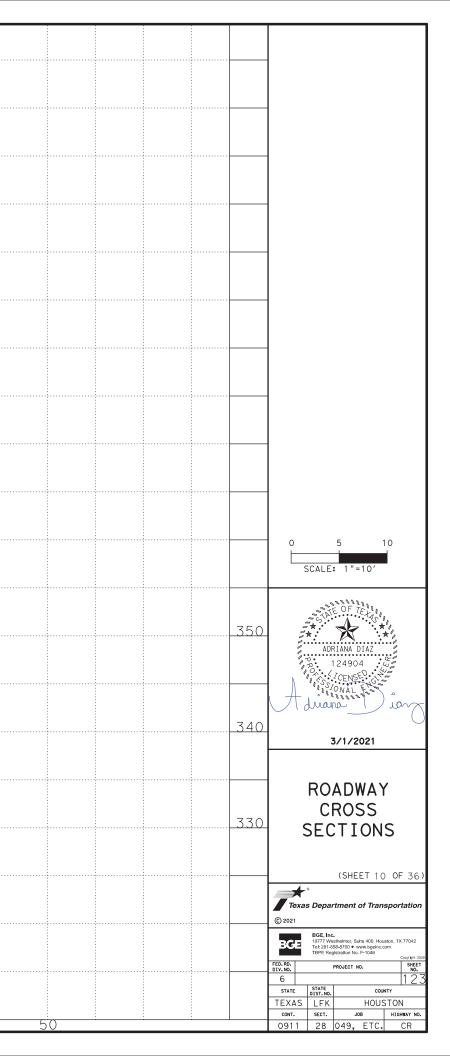
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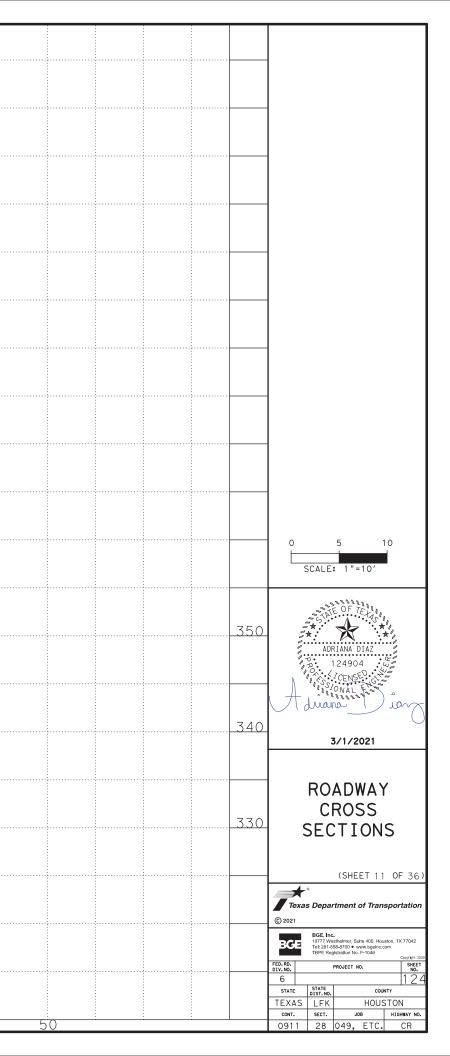
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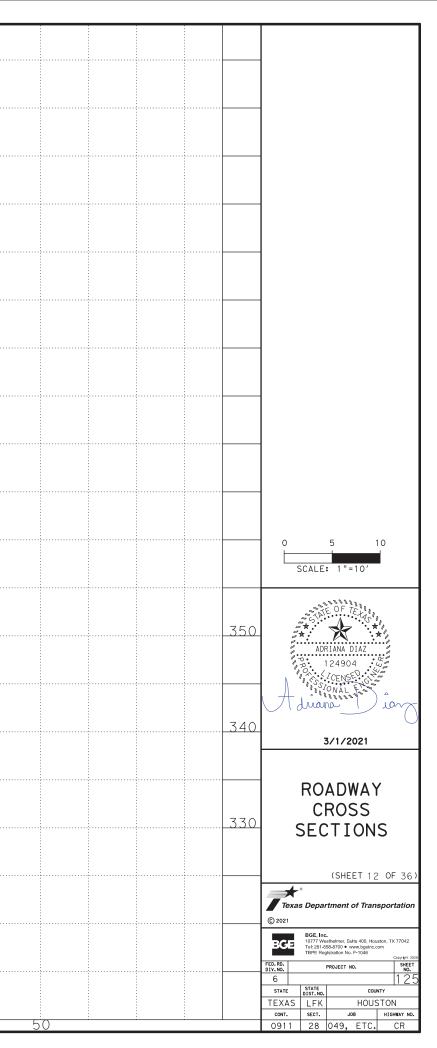
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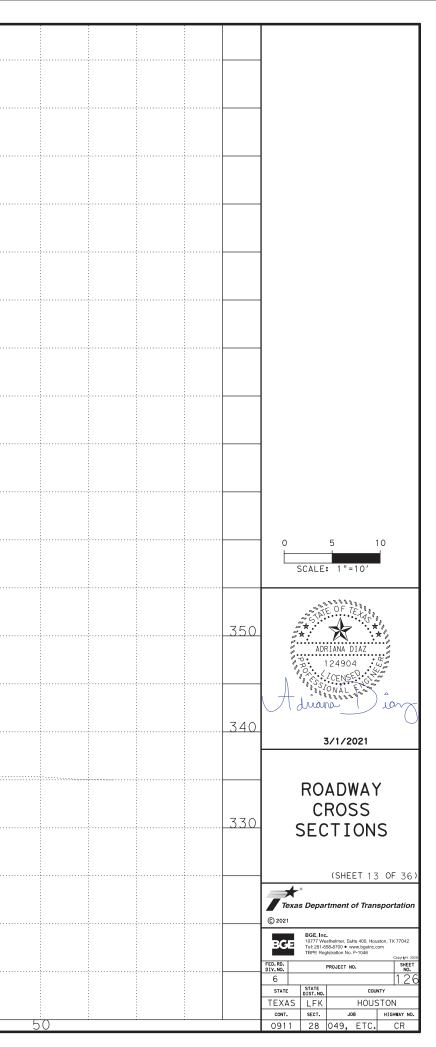
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| | | PERMANENT Easement Prop fence | | | | | | |
| | EL=337.25 | -1 | ROW | | | | | |
| CR 10 | EL=335.52 | | | | | | | |
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| (HICK | | . 0% | CL 10 | | | | | |
| ORY C | | 2.0 | CR 50 | | | | | |
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| - 10 | DS=12.6 =335.42 | • 0 | | | | | | |
| 4+50. | ©S=17.5 | | | | | | | |
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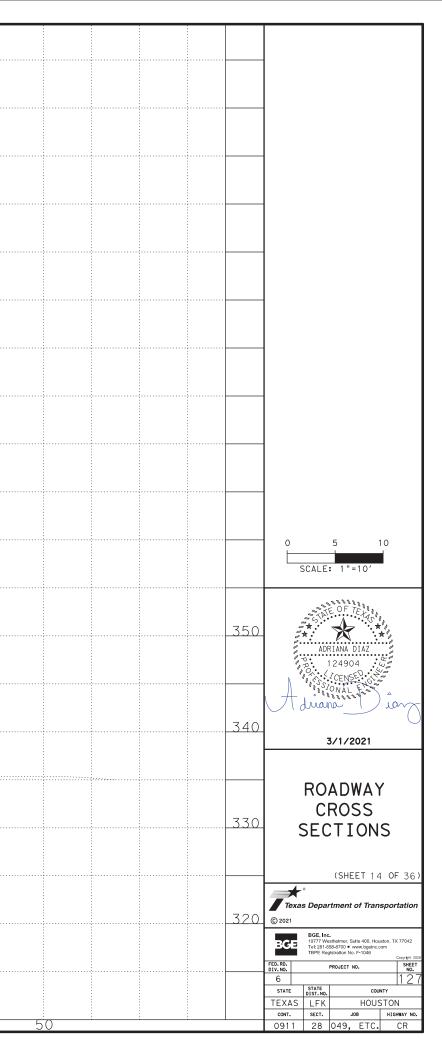


| MODEL NAME: ROADW DATE: 3/1/2021 FILE: c:\pwworkdir\ | MODEL NAME: ROADWAY CROSS SECTIONS DATE: 3/1/2021 FILE: c:\pwworkdir\bge_pw\kcruz\dms40620\HOUCO_XS_1 | 050_SHEETS.dgn | | | PEN T | TABLE: T:\PCSE DRIVER: t:\pcset | 1:\PCSETUP\Tables\TXD01.TBL t:\pcsetup\PL0TDRV\TxD0T_pdf | p | ayscale.plt | |
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| 20 | EL=336.87 | | PERMANENT EASEMENT PROP FENCE | | | | | | | |
| 10 | L = 335. 47 EL = 335. 47 EL = 335. 47 CR = 14. 4 CR = 1020 - | | APPAKENI KOW 05=12.0 EL=336.07 | | | | | | | |
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| 10 | | E = 3 | 36. 07 | | | | | | | |
| 2 | 0 0S=15.2 +4 EL=335.32 0S=18.9 EL=336.26 | 0S=1 | 2 | | | | | | | |
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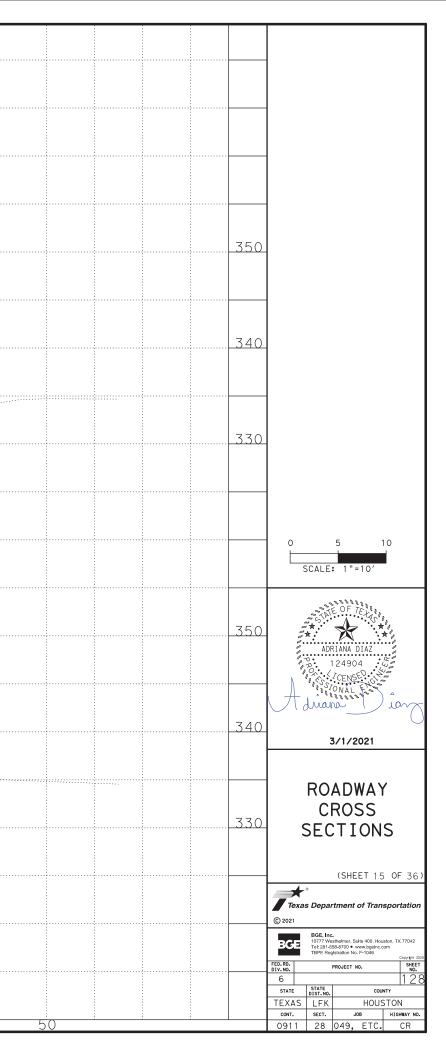


| MODEL NAME: ROADWAY CROSS SECTIONS DATE: 3/1/2021 FILE: c:\pwworkdir\bge_pw\kcruz\dms40620\F | CROSS SECTIONS Z:56:18 PM w\kcruz\dms40620\HOUCO_XS_10E | i0_SHEETS,dgn | | | PEN TABLE: Plot driver: | T:\PCSETUP\Ta t:\pcsetup\PLC | UP\Tables\TXDOT.TBL p\PLOTDRV\T×DOT_pdf | If_grayscale.pl | lt | |
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| | EL=335.04 OS=14.5 | Δ | - 05=12:0 EL=335:65 | | | | | | | |
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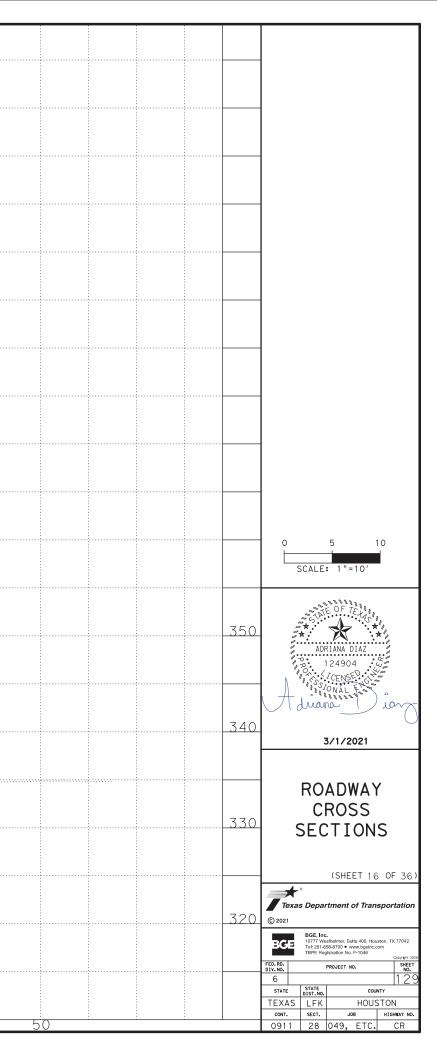




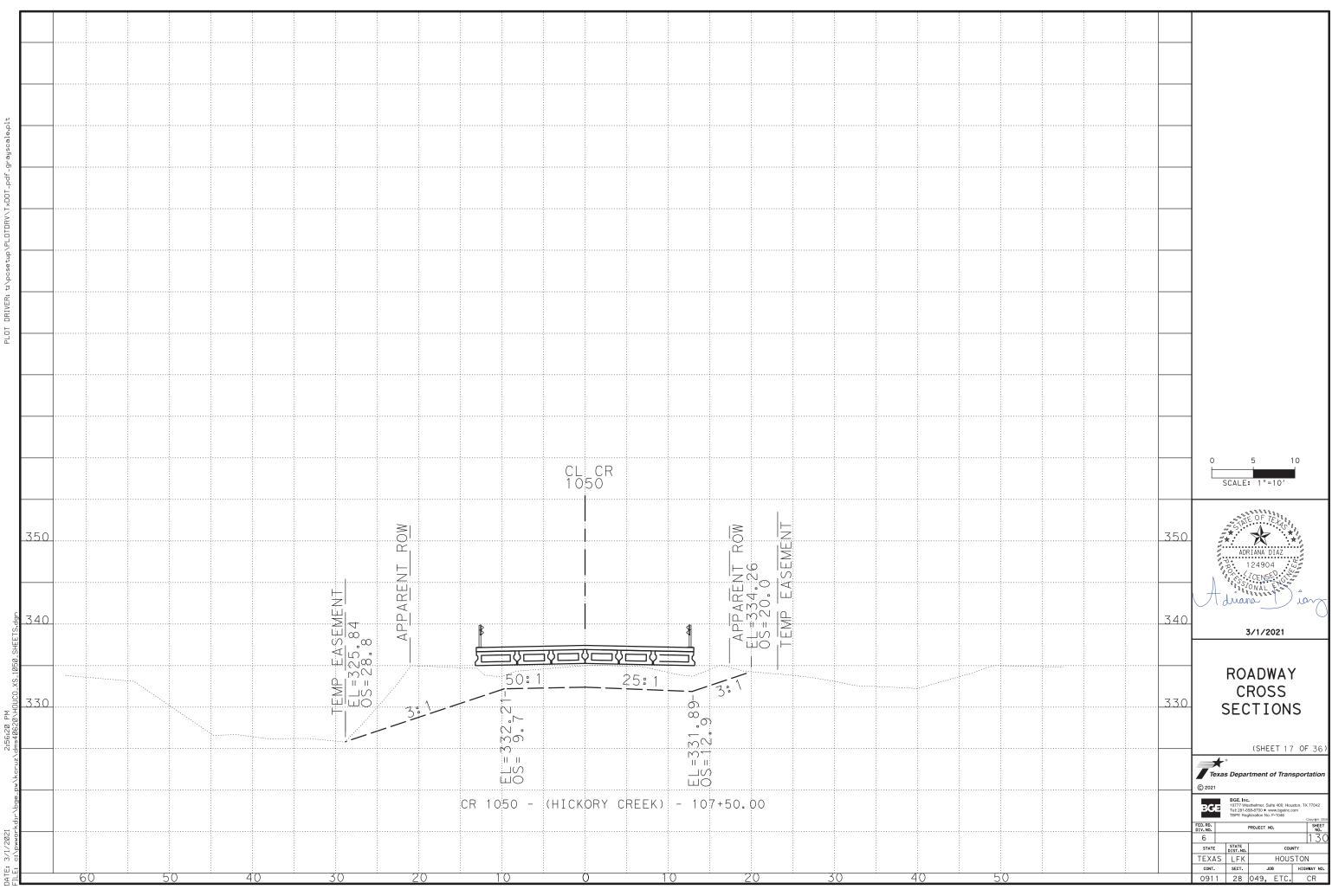
| DATE: 3/1/2021 FILE: c:\pwworkdir\bge_pw\kcruz\dr | 2:56:19 PM .dms40620\HOUC0_XS_105 | Ø_SHEETS.dan | | | PLOT DRIVER: t:/pcset | PLOT DRIVER: t:\pcsetup\PLOTDRV\TxDOT_pdf_grayscale.plt | -grayscale.plt | |
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| 330 Image: State of the | EL=335.06- |

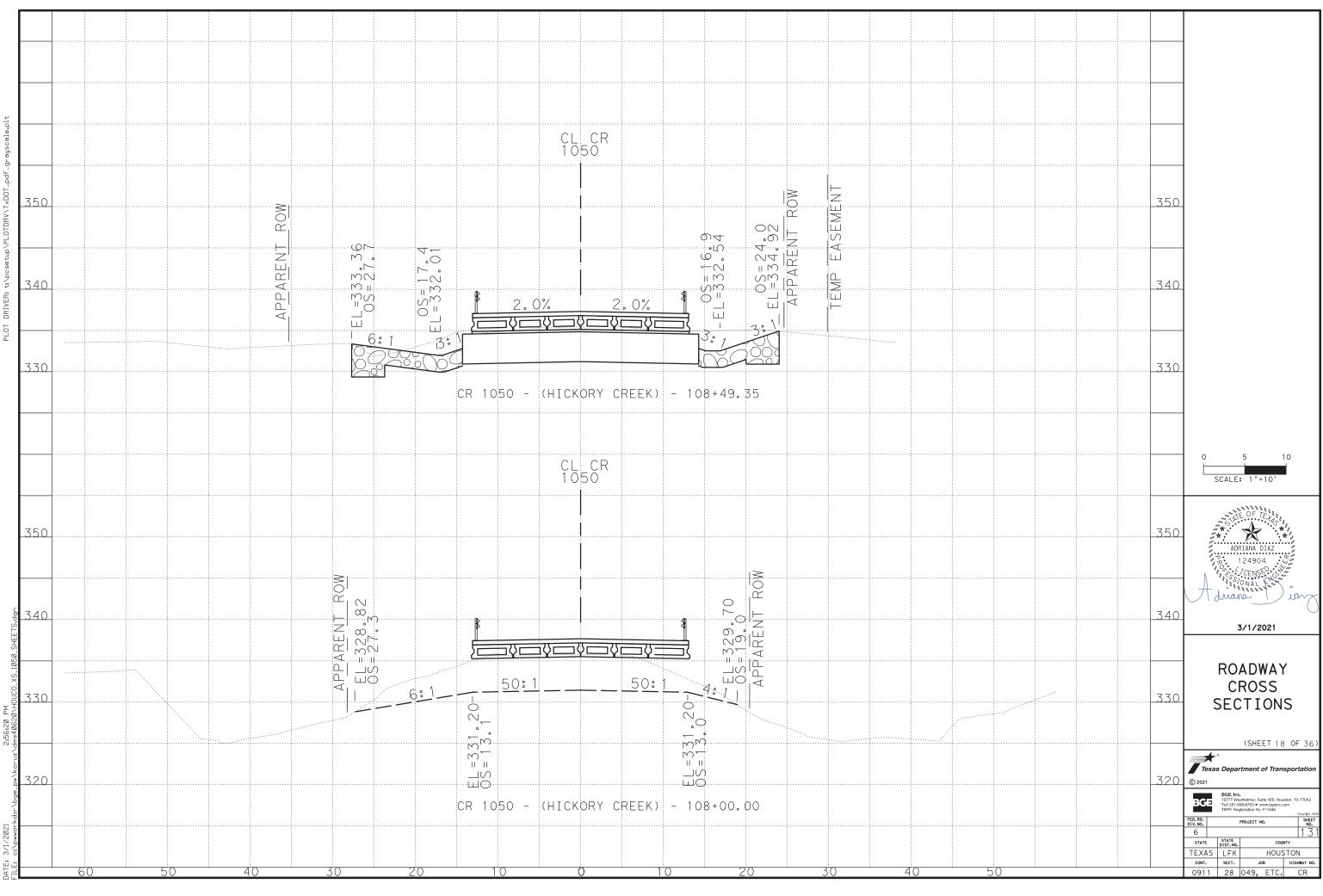






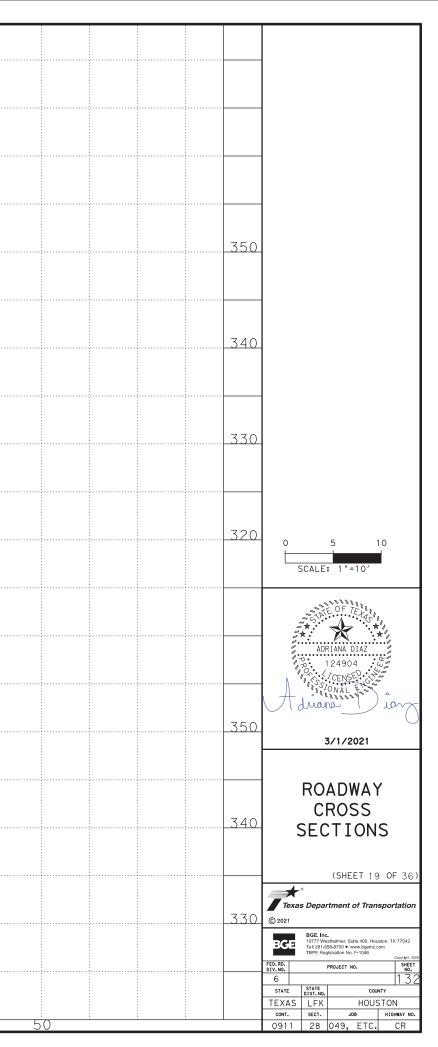


PEN TABLE: T:/PCSETUP/Tables/IXDOT.TBL PLOT DRIVER: t:/pcsetup/PLOTDRV/IxDOT_pdf_graysc:

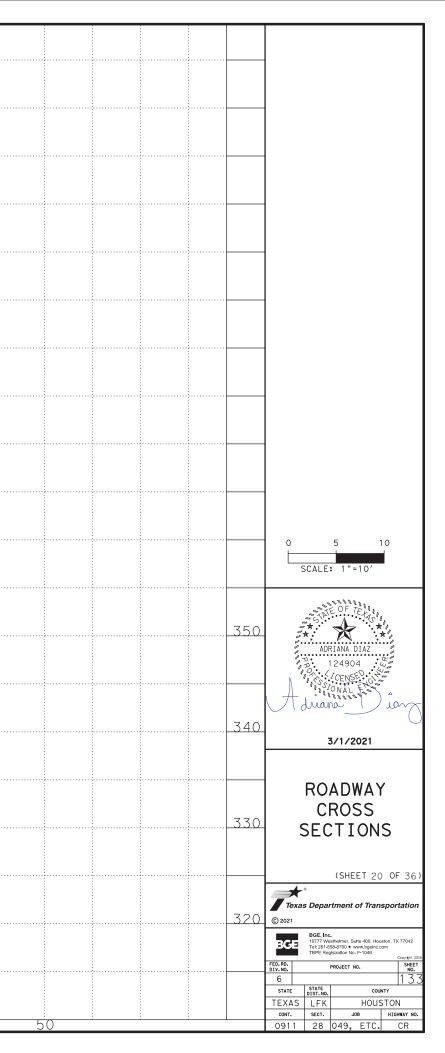


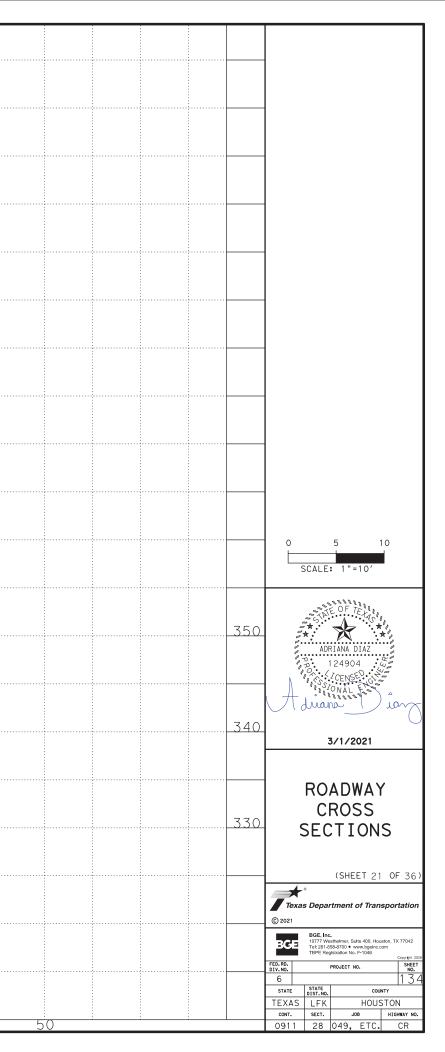
MODEL NAME: ROADWAY CROSS SECTIONS

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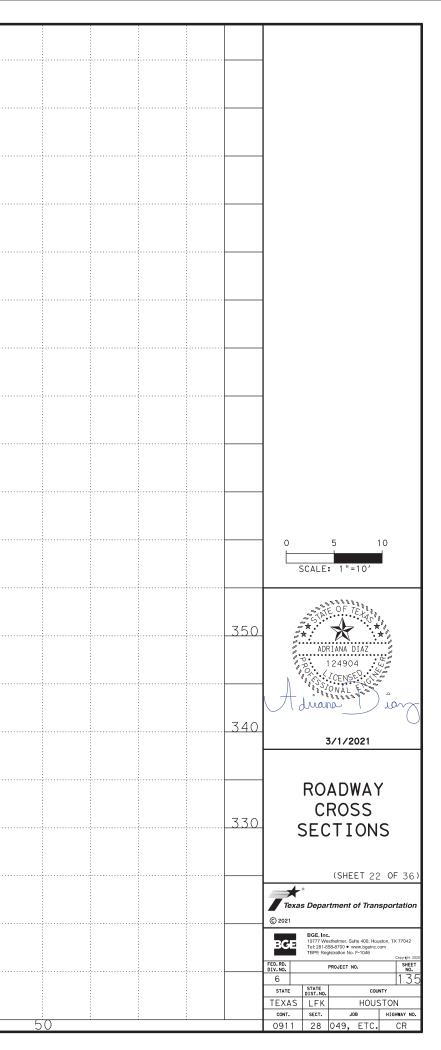


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| | EL=334.93 OS=18.7 | | 1 | | | | | |
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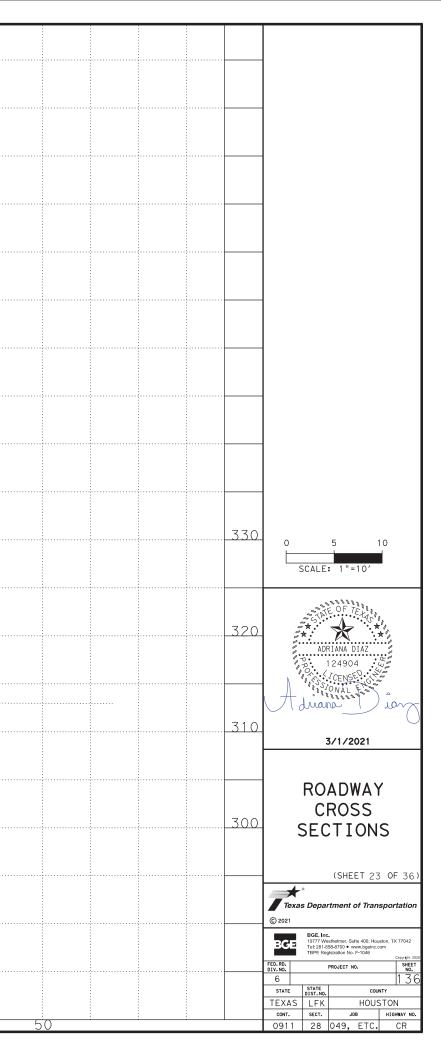




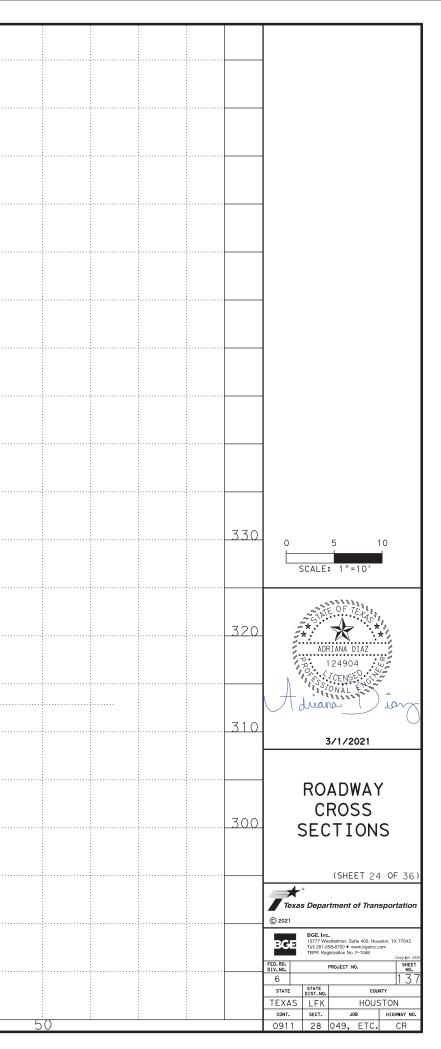
| 5.0 | RON NON | CL_C 1050 | <u></u> | RENT ROW EASEMENT | |
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| 30 | | EL=335.48- 05=11.548- EL=335.79 | EL=334.41 | EL=337.43 | |



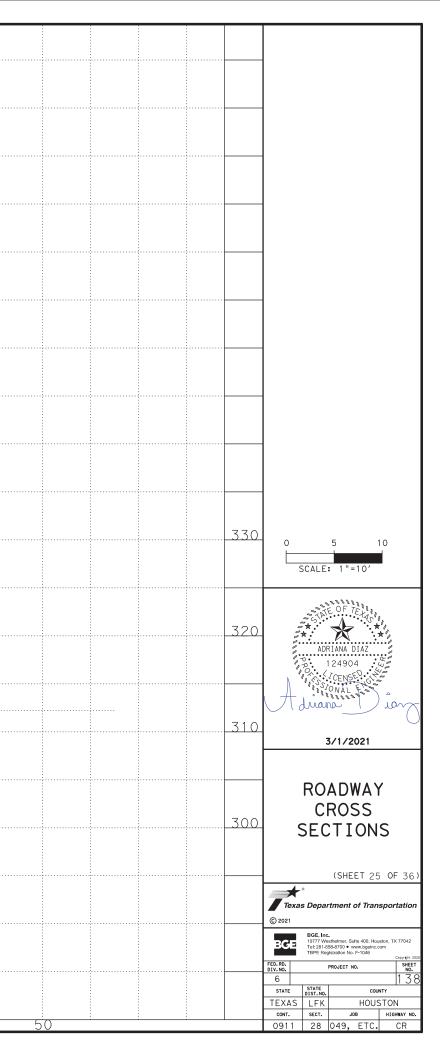
| DATE: 3/1/2021 FILE: c:\pwworkdir\bge_pw\k | 2:56:28 PM kcruz\dms40620/HOUC0_XS_1060_SH | HEETS.dgn | _32 | _33 | PLOT DRI | DRIVER: t:\pcsetup/PL | tup/PL0TDRV/T×D0T_pdf_gr | -grayscale.plt | |
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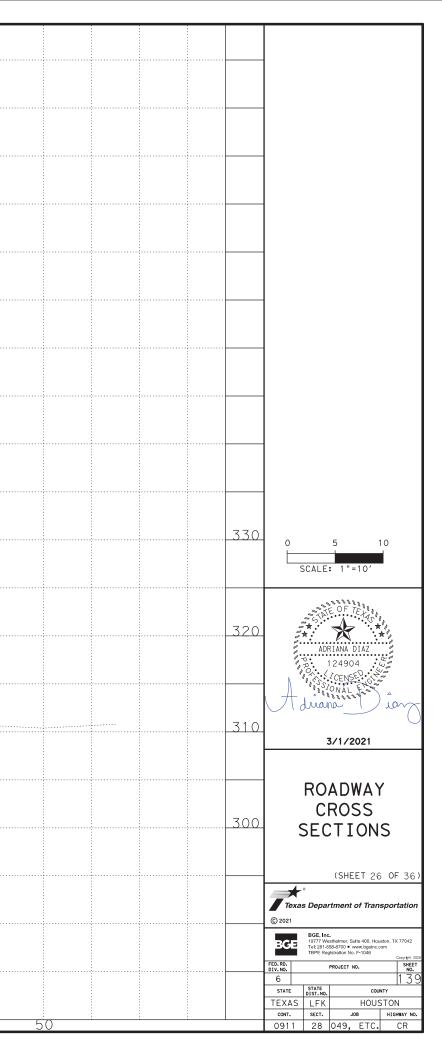
| DATE: 3/1/2021 FILE: c:\pwworkdır\bge_pw | 2:56:29 PM W\kcruz\dms40620\HOUC0_XS_1060_S | HEETS.dgn | | | PLOT DRIVE | DRIVER: t:\pcsetup\PLOTDRV\TxD0T_pdf_grayscale.plt | TDRV\T×D0T_pdf_ | grayscale.plt | |
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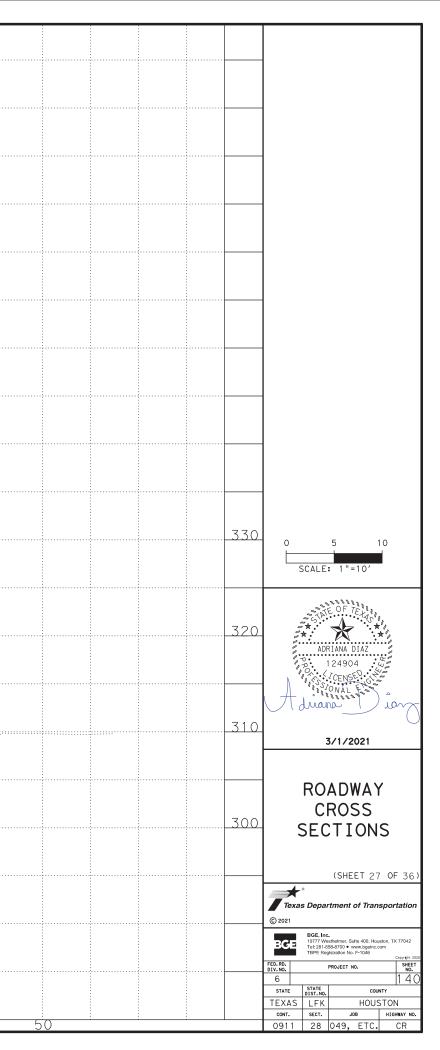
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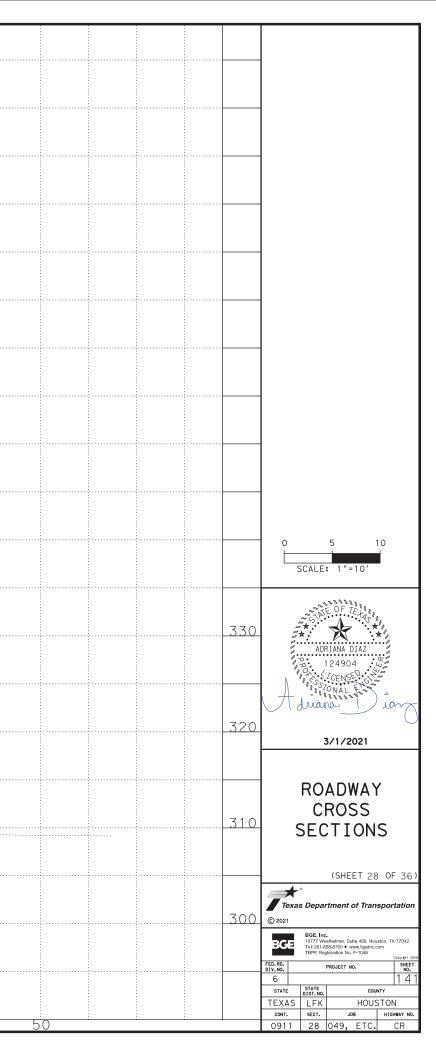


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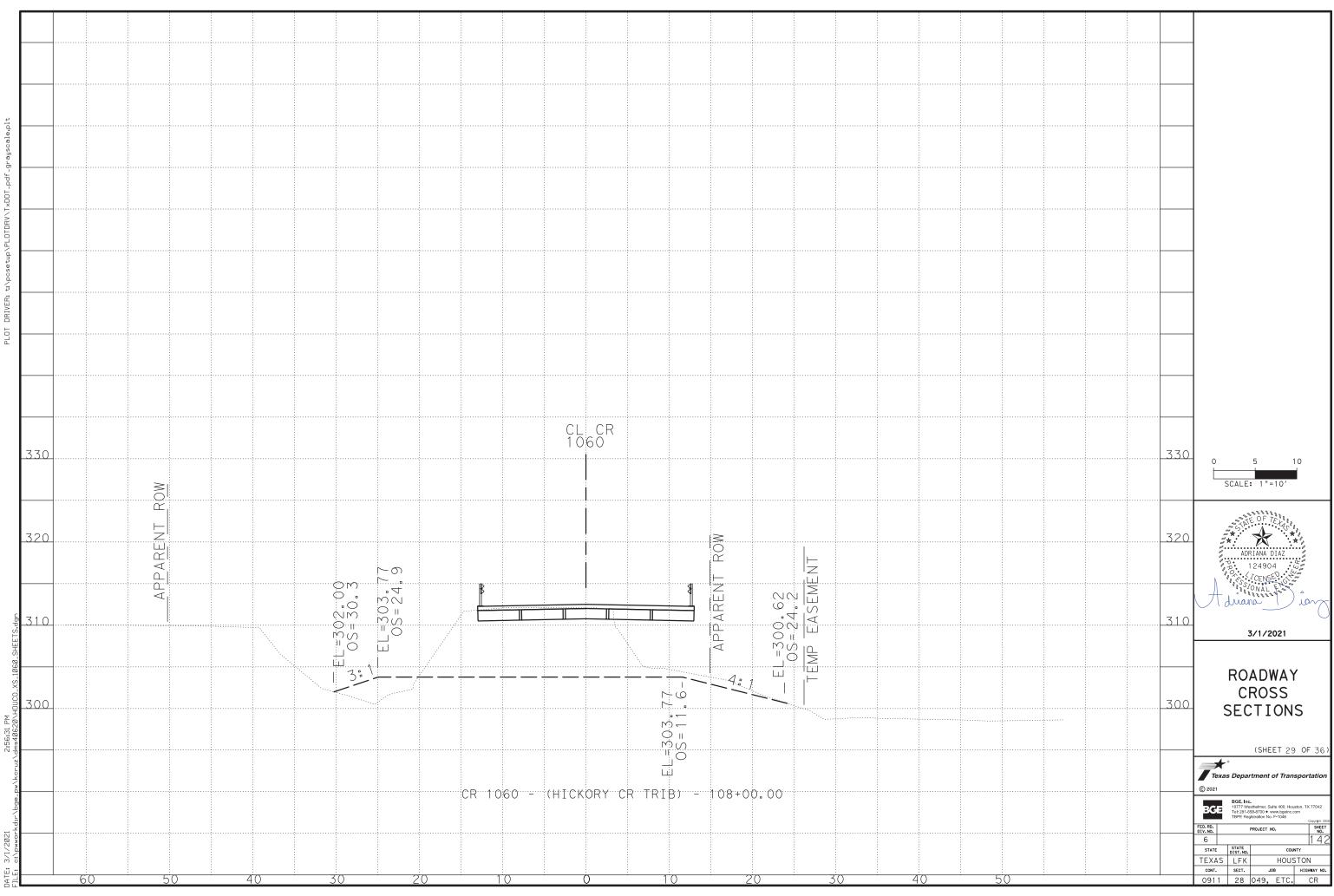




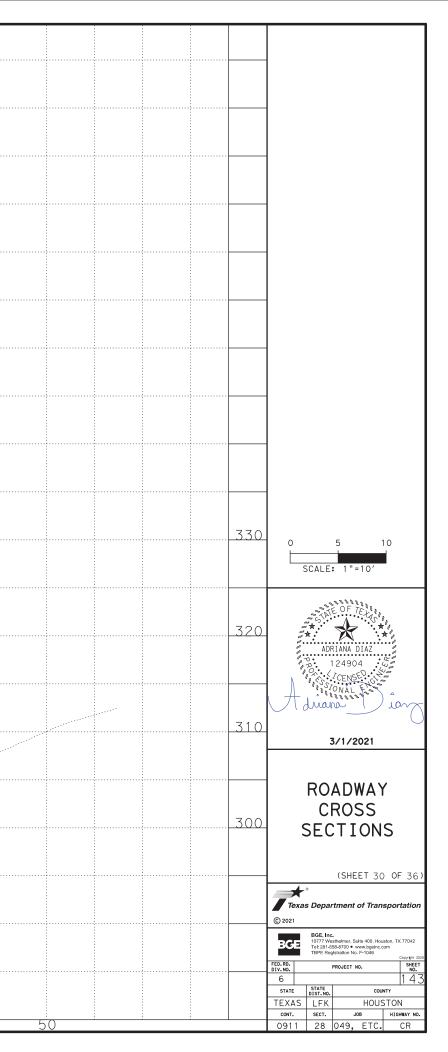
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| 1860 - 340 | APPARENT | =308 =308 | | OS=0.0 | A PPARENT | EL=307.09 TEMP EASEMENT | | |
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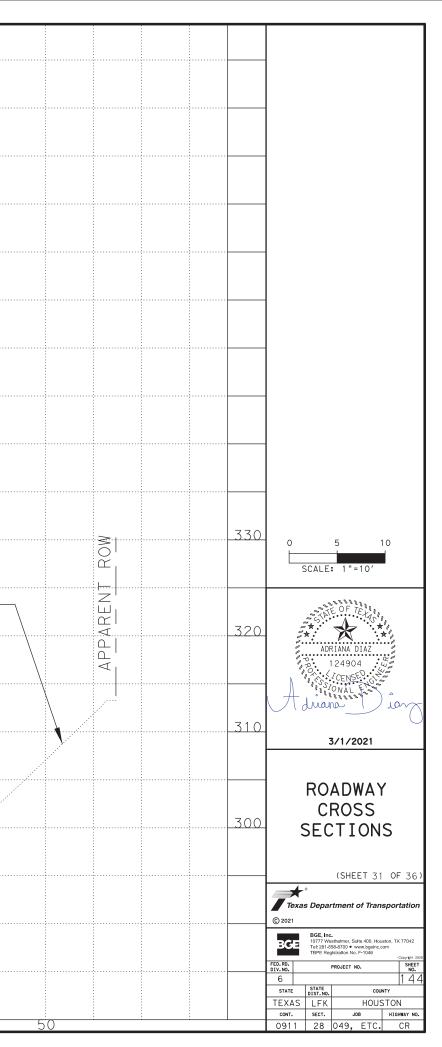
MODEL NAME: ROADWAY CROSS SECTIONS DATE: 3/1/2021 2:56:311 DATE: 3/1/2021 2:56:331



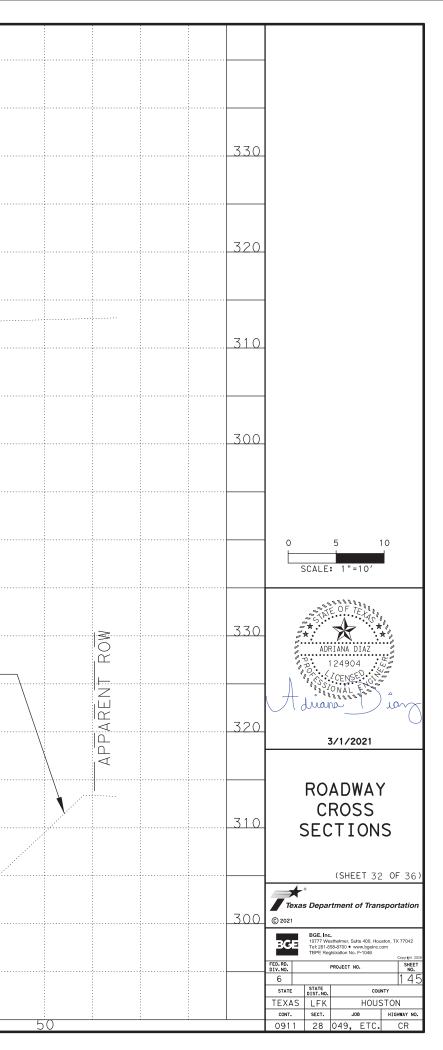
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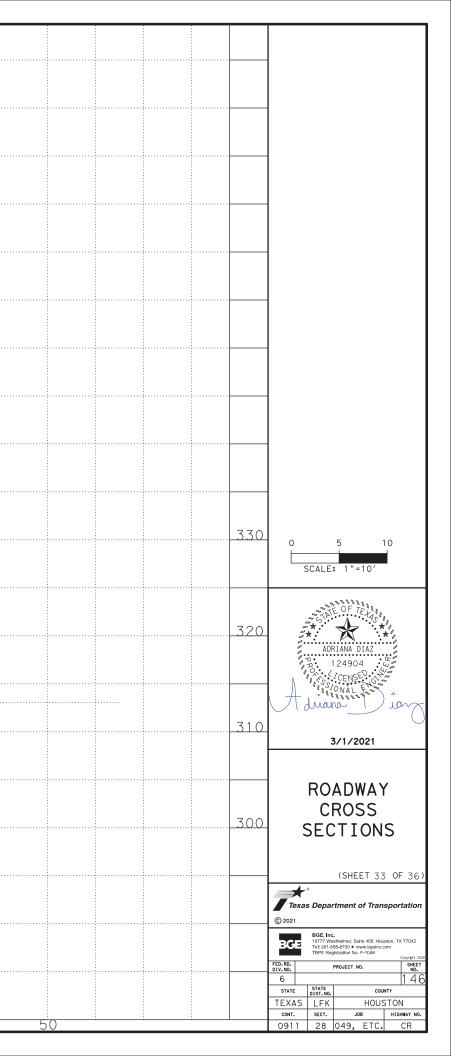
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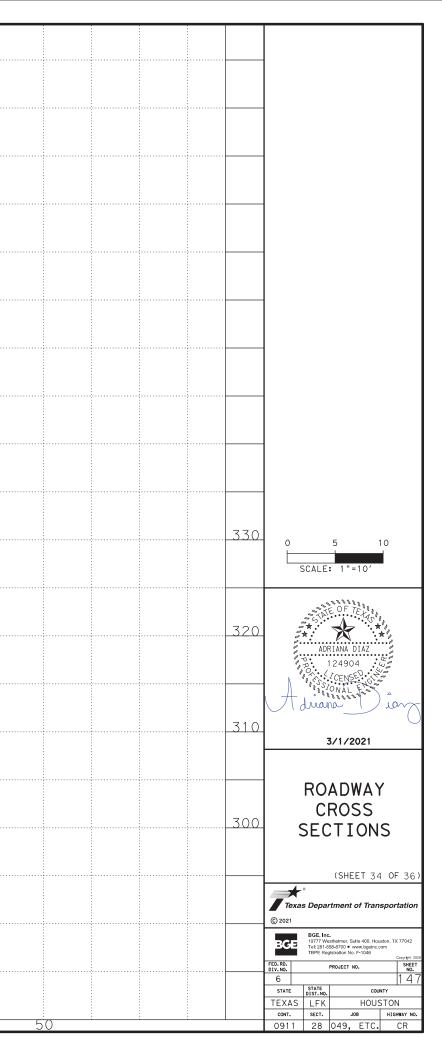
| 330 | PERMANENT EASEMENT PROPFENCE APPARENT ROW | S S = 15 5 = 3 0 5 5 4 | CL CR DO NOT DITCH BA | ACKSLOPE |
|-----|--|---------------------------------------|-----------------------------------|---------------------------|
| 310 | EL=311.18 | EL=309°.75 | 2.0% | |
| 330 | 66 66 80 80 | 0 | CL CR 1060 8 2 • 0 | DO NOT DIS DITCH BACKS |
| 320 | EL=308 | | | = 302 302 31. 0 |



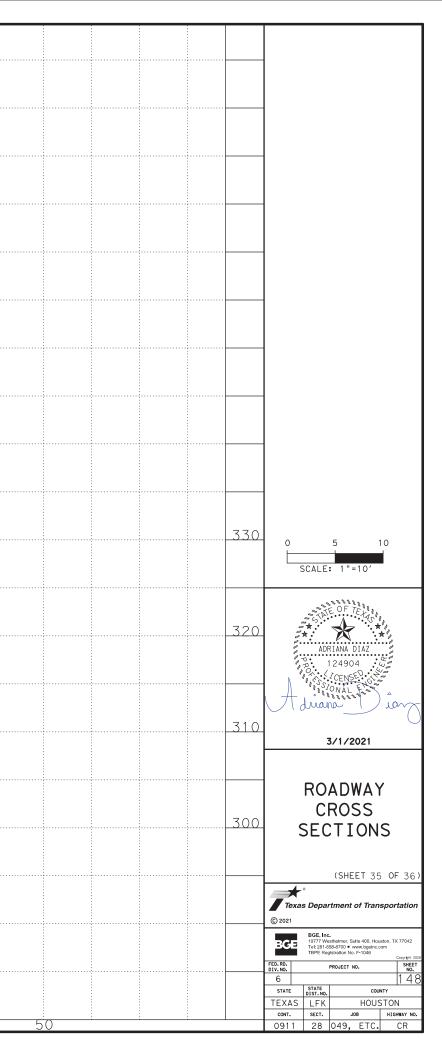
| DATE: 3/1/2021 FILE: c:\pwworkdır\bge_pw\kcru | 2:56:32 PM uz\dms40620\HOUC0_XS_1060 | 0_SHEETS.dgn | | | PLOT DRIVER: t:\pcsetup | up\PL0TDRV\T×D0T_pdf. | -grayscale.plt | | ſ |
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| | EL=312.89 | | | | | | | | |
| | | 3.01 | | | | | | | |
| | EL=309.87 05=1709 | <u>A: 1</u> | 05=15.5 EL=310.47 | | | | | | |
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| | (HICK |)% | | CL 1-0 | | | | | |
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